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CYLINDRICAL AND SPHERICAL SHELLS WITH CRACKS
by

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## SUMMARY

The symmetric problem for the cylindrical and spherical shells containing a meridional crack is considered. The problem is solved for a uniform membrane load and a uniform bending moment applied to the surface of the crack. The extensional and bending components of the stress intensity factor ratio are obtained as functions of shell parameter and are tabulated. The results are also plotted in order to compare them with the existing asymptotic solutions.

## INTRODUCTION

A meridional crack in cylindrical and spherical shells subjected to internal pressure was considered by Folias [1,2]. After obtaining the respective systems of integral equations for the cylinder and the sphere, Folias gave asymptotic expressions for the stress intensity factors for both cases. These expressions are valid only for small values of shell parameter $\lambda$, hence their range of appication is limited. In this paper, the integral equations are solved numerically and the stress intensity factors are evaluated for up to $\lambda=8$ in cylindrical shells and up to $\lambda=5$ in spherical shells.

## THE SOLUTION AND RESULTS

To analyze the stress singularities at the ends of meridional cracks in isotropic and homogeneous thin shells, in [1] and [2], the following linearized shallowshell equations were used, which are due to Marguerre [3] and Reissner [4]:

$$
\begin{align*}
& \frac{E h a^{2}}{R} \frac{\partial^{2} W}{\partial x^{2}}+\nabla^{4} F=0 \\
& \nabla^{4} W-\frac{a^{2}}{R D} \frac{\partial^{2} F}{\partial x^{2}}=\frac{q}{D} a^{4} \tag{1}
\end{align*}
$$

for cylindrical shells, and

$$
\begin{align*}
& -\frac{E h a^{2}}{R} \nabla^{2} W+\nabla^{4} F=0  \tag{2}\\
& \nabla^{4} W+\frac{a^{2}}{R D} \nabla^{2} F=\frac{q}{D} a^{4}
\end{align*}
$$

for spherical shells. In (1) and (2), $X, Y, Z$ are the rectangular coordinates with $Z$ normal to the surface and $X$ in the plane of the crack, $W$ is the displacement in $Z$ direction, $x=X / a, y=Y / a$ are dimensionless coordinates, $a \operatorname{is}$ the halfcrack length, $R$ is the mean radius, $h$ is the shell thickness, $F$ is the stress function, $q$ is the normal traction acting on the shell surface, $E$ is the Young's modulus and $D$ is the flexural rigidity, $D=E h^{3} / 12\left(1-\nu^{2}\right)$, $v$ being the Poisson's ratio. In the usual manner, the components of bending moment $M_{x}, M_{y}$, $M_{x y}$ and transverse shear $Q_{x}, Q_{y}$ are given in terms of $W$ and
the membrane forces $N_{x}, N_{y}, N_{x y}$ are given in terms of $F$.
In general, the singular solution of the cracked shell problem may be reduced to a perturbation problem in which self-equilibrating forces and moments acting on the crack surfaces are the only external loads. Thus, in one class of important symmetric problems, the homogeneous systems obtained from (1) and (2) will have to be solved with the following boundary conditions on the crack surface

$$
\begin{align*}
& M_{y}=M_{0}(x), V_{y}=Q_{y}+\frac{\partial M_{x y}}{\partial x}=0,  \tag{3}\\
& N_{y}=N_{0}(x), N_{x y}=0,(-1<x<1, y=0)
\end{align*}
$$

and vanishing stresses away from the crack. Due to the conditions of symmetry, in [1] and [2], the functions $F(x, y)$ and $W(x, y)$ are expressed in terms of Fourier cosine integrals, and the resulting dual integral equations are reduced to a system of singular integral equations of the following form*

$$
\begin{equation*}
\int_{-1}^{1} \sum_{i}^{2} h_{i j}(x, t) u_{j}(t) d t=f_{i}(x),(i=1,2 ;-1<x<1) \tag{4}
\end{equation*}
$$

where the kernels $h_{i j}$ contain Cauchy-type singularities, $u_{j}$,

[^0]$u_{2}$ are conveniently defined unknown functions and $f_{1}, f_{2}$ are related to the external loads. For example, for constant $M_{y}$ and $N_{y}$ in (3), letting
\[

$$
\begin{equation*}
M_{0}=\frac{m_{0} D}{a^{2}}, N_{0}=\frac{n_{0}}{a^{2}} \tag{5}
\end{equation*}
$$

\]

we have

$$
\begin{equation*}
f_{1}(x)=\frac{2 \pi n_{0}}{i \sqrt{E D h}} x, f_{2}(x)=-2 \pi m_{0} x \tag{6}
\end{equation*}
$$

The kernels $h_{i j}(x, t)$ in (4) contain modified Bessel functions of the form, $k_{j}\left(e^{\mp \pi i / 4}|t-x| \lambda / n\right),(j=0,1)$, where $n=2$ for cylindrical shells, $n=1$ for spherical shells and $\lambda$ is the shell parameter defined by

$$
\lambda=\left[12\left(1-v^{2}\right)\right]^{1 / 4} \frac{a}{\sqrt{R h}}
$$

Here $\nu$ is the Poisson's ratio, a is the half crack length, $R$ is shell radius of curvature and $h$ is the shell thickness.

Studying the asymptotic vilues of the kernels for small arguments, $i t$ is easily shown that at $t=x, h_{i j}$ are singular and the leading terms are of the form $a_{i j} /(t-x)$, where $\mathbf{a}_{\mathbf{i j}},(\mathbf{i}, \mathbf{j}=1,2)$, are constant. For convenience in the numerical analysis, we replace the modified Bessel functions having complex arguments by the Kelvin functions with real arguments by using the following relation:

$$
\begin{equation*}
e^{-\frac{1}{2} \nu \pi i} K_{\nu}\left(x e^{\pi i / 4}\right)=\operatorname{ker}_{\nu} x+i k e i_{\nu} x \tag{7}
\end{equation*}
$$

Around zero, the functions $\mathrm{ker}_{1}$ and $\mathrm{kei}_{\mathrm{j}}$ behave like $1 / \mathrm{x}$ which provide the singularity of the integral equations and may easily be separated. By using the polynomial approximations for the functions $k e r_{v}$ and $k e i_{v},(\nu=1,2)$ given in [7], (4) may be put in the following form:

$$
\begin{gather*}
\int_{-1}^{1}\left\{\begin{array}{l}
2 \\
a_{i j} u_{j}(t) \frac{d t}{t-x}+\int_{-1}^{1} k_{i j}(x, t) u_{j}(t) d t=f_{i}(x) \\
i=1,2,|x|<1
\end{array}\right. \tag{8}
\end{gather*}
$$

where the coefficients $a_{i j}$ are known constants and the kernels $k_{i j}$ are bounded known functions.

From the definitions of the auxiliary functions $u_{1}$ and $u_{2}$ as given in [1] and [2], it can be shown that $u_{1}$ and $u_{2}$ are even functions and the index of the system of equations ( 8 ) is -1 , that is $u_{1}$ and $u_{2}$ may be expressed as

$$
\begin{equation*}
u_{j}(x)=\sqrt{1-x^{2}} A_{j}\left(x^{2}\right),(j=1,2) \tag{9}
\end{equation*}
$$

where the functions $A_{1}, A_{2}$ are regular. Again, referring to [1] and [2] for details, we simply note that the extensional and bending components of the stress intensity factors are directly related to the values of the functions $A_{j}$ at $x=1$. Hence the solution of ( 8 ) is sufficient for the evaluation of
stress intensity factors.

The system of singular integral equations (8) may be regularized by using the method of Muskhelishvili [8] or Carlemann and Vekua [9]. However, because of the complexity of the kernels $k_{i j}$, the process is quite complicated and the solution of the resulting system of integral equations witn weakly singular kernels can only be solved approximately. Therefore, to solve (8), we use the method described in [10], which is based on the observation that the fundamental function of the system (8) is the weight of Chebishev polynomials. Thus, the singularities of (8) may be removed by expressing the functions $A_{j}$ as infinite series in these orthogonal polynomials. The technique is rather easy to apply and allows introducing into the program an automatic convergence scheme for a desired degree of accuracy in the computed quantities (by simply specifying the number of significant digits in stress intensity ratios which should be repeated as the number of terms in the series is increased).

Here, we give the numerical results for four different cases, namely the cylindrical and the spherical shells with the loading conditions $N_{0} \neq 0, M_{0}=0$ and $N_{0}=0, M_{0} \neq 0$. Let the stress intensity factors in shells be

$$
\begin{aligned}
& k_{s}=\left(A_{e}+A_{b}\right) k_{p} \text { (outer surface) } \\
& k_{s}=\left(A_{e}-A_{b}\right) k_{p}(\text { inner surface })
\end{aligned}
$$

where $k_{p}$ is the stress intensity factor in the corresponding flat plate and is given by

$$
\begin{align*}
& k_{p}=\frac{N_{0}}{\hbar} \sqrt{a}, \text { for } N_{0} \neq 0, M_{0}=0 \\
& k_{p}=\frac{6 M_{0}}{h^{2}} \sqrt{a}, \text { for } M_{0} \neq 0, N_{0}=0 \tag{11}
\end{align*}
$$

The constants $A_{e}$ and $A_{b}$ are the extensional and bending components of the stress intensity factor and are given in Tables 1 and 2, either directly ( $A_{e}$ for $N_{0} \neq 0, M_{0}=0$ and $A_{b}$ for $N_{0}=0, M_{0} \neq 0$ ) or through the following relations

$$
\begin{equation*}
A_{b}=\frac{(3+v)}{\sqrt{\left(1-v^{2}\right) / 3}} a_{b} \text { for } N_{0} \neq 0, M_{0}=0 \tag{12}
\end{equation*}
$$

$$
A_{e}=\sqrt{\left(1-\nu^{2}\right) / 3} a_{e} \text { for } N_{0}=0, M_{0} \neq 0
$$

Regarding the results given by Tables 1 and 2, the following should be noted: a) the Poisson's ratio $v$ appears explicitly as well as through $\lambda$ in the espressions for $k_{i j}$. Thus the numerical results are obtained for one value of $v$ only, which was selected to be $v=1 / 3$. b) Because of the assumption of Kirchhoff type boundary conditions in the formulation of the problem, the angular distributions of extensional and bending stresses around the crack tip are not the same, hence the superposition given by (10) is valid only for $\theta=0$, that is, along the prolongation of the crack. c) The numerical
analysis is carried out by using poiynomial approximations for Kelvin functions which are valid in $-8 \leq x \leq 8$ with a residual error of less than $10^{-7}, x$ being the argument of the functions. In the cylindrical shells, the argument $x$ varies in $0 \leq x \leq \lambda$, and in spherical shells, in $0 \leq x \leq \lambda / 2$. Since we required only four significant digit accuracy in the calculated quantities, technically, we could obtain useful results for values of $\lambda$ somewhat larger than 8 in cylinders and 4 in spheres. To obtain more reliable results for higher values of $\lambda$, in addition to polynomial approximations valid for small $x$, one can also introduce the asymptotic expressions for the Kelvin functions valid for $|x|>8$ (also given in

The results given in Tables 1 and 2 are shown in Figures 1-8. The figures also include the results of the asymptotic solutions obtained in [1] and [2].

Comparison of the two sets of curves clearly indicates the inadequacy of the asymptotic solutions for moderately large values of $\lambda$. Note that at $\lambda=0$, as expected, all stress intensity ratio curves have zero slope. Note also that the effect of shell curvature on the stress intensity factors is much more significant if the shell is subjected to membrane loading rather than bending.

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Iable 1 - Stress Intensity Factor Ratios For
$N_{0} \neq 0, M_{0}=0$

|  | CYLINDE |  | SPHERE |  |
| :---: | :---: | :---: | :---: | :---: |
| $\lambda$ | $A_{e}$ | $a_{b}$ | $A_{e}$ | $\mathrm{a}_{\mathrm{b}}$ |
| . 2 | 1.0096 | . 00410 | 1.0112 | . 00611 |
| . 4 | 1.0371 | . 01124 | 1.0422 | . 01693 |
| . 6 | 1.0795 | . 01902 | 1.0887 | . 02919 |
| . 8 | 1.1344 | . 02659 | 1.1479 | . 04186 |
| 1.0 | 1.1893 | . 03359 | 1.2174 | . 05448 |
| 1.2 | 1.2723 | . 03985 | 1.2956 | . 06685 |
| 1.4 | 1.3519 | . 04529 | 1.3812 | . 07886 |
| 1.6 | 1.4367 | . 04990 | 1.4731 | . 09045 |
| 1.8 | 1.5256 | . 05368 | 1.5706 | . 10155 |
| 2.0 | 1.6177 | . 05664 | 1.6729 | . 11216 |
| 2.2 | 1.7122 | . 05883 | 1.7795 | . 12223 |
| 2.4 | 1.8085 | . 06018 | 1.8899 | . 13172 |
| 2.6 | 1.9060 | . 06090 | 2.0038 | . 14058 |
| 2.8 | 2.0045 | . 06083 | 2.1208 | . 14879 |
| 3.0 | 2.1035 | . 06014 | 2.2408 | . 15630 |
| 3.25 | 2.2276 | . 05832 | 2.3947 | . 16463 |
| 3.50 | 2.3519 | . 05549 | 2.5526 | . 17172 |
| 3.75 | 2.4761 | . 05172 | 2.7143 | . 17751 |
| 4.00 | 2.5999 | . 04700 | 2.8796 | . 18194 |
| 4.25 | 2.7232 | . 04154 | 3.0485 | . 18483 |
| 4.50 | 2.8459 | . 03512 | 3.2208 | . 18644 |
| 5.00 | 3.0895 | . 02012 | 3.5750 | . 18493 |
| 5.50 | 3.3303 | . 00234 | 3.9446 | . 17802 |
| 6.00 | 3.5681 | . .02222 |  |  |
| 6.50 | 3.8029 | -. 04130 |  |  |
| 7.00 | 4.0347 | -. 06622 |  |  |
| 7.50 | 4.2637 | -. 09350 |  |  |
| 8.00 | 4.4895 | -. 12279 |  |  |

## Table 2 - Stress Intensity Factor Ratios For

$$
N_{0}=0, M_{0} \neq 0
$$

## CYLINDER

## SPHERE

| $\lambda$ | $a_{e}$ | $A_{b}$ | $a_{e}$ | $A_{b}$ |
| :--- | :---: | :---: | :---: | :---: |
| .2 | .006161 | .99816 | .00842 | 1.0020 |
| .4 | .01695 | .99340 | .02249 | 1.0070 |
| .6 | .02897 | .98660 | .03749 | 1.0137 |
| .8 | .04107 | .97846 | .05202 | 1.0211 |
| 1.0 | .05283 | .96946 | .06557 | 1.0287 |
| 1.2 | .06406 | .95986 | .07799 | 1.0364 |
| 1.4 | .07473 | .94993 | .08935 | 1.0439 |
| 1.6 | .08482 | .93976 | .09964 | 1.0512 |
| 1.8 | .09435 | .92956 | .10895 | 1.0583 |
| 2.0 | .1033 | .91936 | .11740 | 1.0652 |
| 2.2 | .1118 | .90923 | .12519 | 1.0718 |
| 2.4 | .1198 | .89926 | .13228 | 1.0783 |
| $2 . F$ | .1273 | .88940 | .13876 | 1.0845 |
| 2.8 | .1344 | .87970 | .14475 | 1.0905 |
| 3.0 | .1410 | .87023 | .15030 | 1.0964 |
| 3.25 | .1488 | .85863 | .15668 | 1.1035 |
| 3.50 | .1551 | .84740 | .16260 | 1.1103 |
| 3.75 | .1628 | .83643 | .1681 | 1.1170 |
| 4.00 | .1691 | .82440 | .1732 | 1.1233 |
| 4.25 | .1750 | .81542 | .1780 | 1.1297 |
| 4.50 | .1803 | .80539 | .1826 | 1.1357 |
| 5.00 | .1903 | .78616 | .1905 | 1.1470 |
| 5.50 | .2005 | .76832 | .2000 | 1.1580 |
| 6.00 | .2068 | .75079 |  |  |
| 6.50 | .2137 | .73446 |  |  |
| 7.00 | .2200 | .71879 |  |  |
| 7.50 | .2255 | .7080 | .6897 |  |


ricure 1 . Stress intensity ratio in cylinder ( $0_{0} \neq 0, \gamma_{0}=0$ )


Pipure 2. 3tress intensity ratio in cylinder $\left(A_{0} \neq 0,{ }_{0}=0\right)$

ifigure 3. Stress Intensity ratio in sphere ( $N_{0} \neq 0, \pi_{0}=0$ )


Pigure 4. Stress intensity ratio in sphere ( $H_{0} \neq 0, T_{0}=0$ )


Fisure 5. Stress intensity ratio in cylinder ( $N_{0}=0, T_{0} \neq 0$ )


Figure 6. Stress intensity ratio in cylinder ( $H_{0}=0$, ito $_{0} \neq 0$ )


Mipure 7. Stress intensity ratio in sphere ( $\pi_{0}=0, T_{0} \neq 0$ )


Pigure 3. Stress intensity ratio in sphere (if $=0, ?_{0} \neq 0$ )


[^0]:    *For details of the analysis and explicit forms of the kernels $h_{i j}$, see [5] and [6].

