UNIVERSITI TEKNOLOGI MARA

EFFECTIVENESS OF DIFFERENT PERCENTAGE OF BENTONITE IN SOIL LINER ON INTERFACE SHEAR STRENGTH WITH GEOSYNTHETIC

FARAH HAFIFEE BINTI AHMAD

.

• Thesis submitted in fulfilment of the requirements for the degree of Master of Science in Civil Engineering (Geotechnique)

Faculty of Civil Engineering

JUN 2013

Candidate's Declaration

I declare that the work in this thesis entitled "Effectiveness of Different Percentage of Bentonite in Soil Liner on Interface Shear Strength with Geotextile" is the result of my own work except as cited in the references. It was carried out in accordance with the regulations of Universiti Teknologi MARA and has not been submitted to any other academic institution or non-academic institution for any other degree or qualification.

In the event that my thesis is found to violate the condition above, I voluntarily waive the right of conferment of my degree and agree to be subjected to the disciplinary rules and regulations of Universiti Teknologi MARA.

	Soil with Different Percentage of Bentonite	
Thesis Title	:	Interface Shear Strength of Geosynthetic and
Faculty	•	Civil Engineering
Programme	:	MSc. Civil Engineering (Geotechnique)
Candidate's ID no.	:	2011109555
Name of Candidate	:	Farah Hafifee Binti Ahmad

Signature of Candidate	:	••••••
Date	:	Jun 2013

ABSTRACT

Interface shear performance of various percentages of bentonite and fine soil were evaluated for landfill stability by conducting Direct Shear Test. The focus of this study is placed on interface shear strength of fine soil and soil with different percentage of bentonite. Bentonite is currently placed for use as buffer and backfill materials in landfill because these materials create an impermeable zone where it reduced the potential of soil contamination from leachate. In this study, direct shear box with dimension of 60 mm x 60 mm was used to determine the interface shear strength of soil with different percentage of bentonite at optimum moisture content. Normal stresses used were between 100 kPa to 300 kPa to represent the depth of 20 m metres of solid waste. Results showed that the most suitable percentage of bentonite was 5% due to the highest interface shear strength of the mixture with non woven geosynthetic. Moreover, even the presence of bentonite in the sample is proved to give higher interface shear strength to the tested soil, additional bentonite tends to decreased it shear strength.

ACKNOWLEDGEMENT

First of all, I would like to thank Allah S.W.T. for his blessing and kindness upon completing this research which is both fruitful and beneficial in a successful manner.

I would also like to further my thanks to my supervisor Dr Mazidah Binti Mukri for her guidance, readiness and ever willing to help me all his other candidates with fundamental information throughout this research. Furthermore, extended gratitude is in order to all the laboratory technicians who were involved in helping and making my research a success by giving me free will to conduct tests in the laboratory with ease.

In addition, to my beloved parents, Ahmad Bin Sahadar and Husna Binti Idris who gave me their full support and consent to accomplish this research even during inconvenient times such as on the weekends and public holidays.

Last but not least, I wish to thank all the others especially to all my friends who have in one way or another guided me with useful tips, valuable advices and kind assistance towards producing this research.

THANK YOU.

Farah Hafifee Ahmad Jun 2013

TABLE OF CONTENTS

		Page
TITI	LE PAGE	
Can	didate's Declaration	ii
ABS	STRACT	iii
AC	KNOWLEDGEMENT	iv
TAE	BLE OF CONTENTS	v
LIST OF TABLES		ix
LIS	Г OF FIGURES	X
LIS	Γ OF ABBREVIATIONS	xiii
CHA	APTER 1	1
INT	RODUCTION	1
1.0	Background of Study	1
1.1	Problem Statement	4
1.2	Objectives	5
1.3	Scope of Works	5
1.4	Significance of Study	6
CHA	APTER 2	7
LIT	ERATURE REVIEW	7
2.1	Introduction of Clay	7
2.2	Basic Properties of Clay Soil	7
	2.2.1 Silica Tetrahedron	9
	2.2.2 Aluminium Octahedron	9
2.3	Atterberg Limit of Clay	10
2.4	Shrinkage Limit of Clay	12
2.5	5 Swelling Behaviour of Bentonite	
2.6	Mineralogy	14
2.7	Introduction of Geosynthetic Clay Liners (GCLs)	17
2.8	Types of Geosynthetic Clay Liner	17
	2.8.1 Unreinforced Geosynthetic Clay Liner	17