# Reports of the Department of Geodetic Science

Report No. 196

# GEOMETRIC ADJUSTMENT OF THE SOUTH AMERICAN SATELLITE DENSIFICATION (PC-1000) NETWORK

by

Ivan I. Mueller and M. Kumar

**Prepared for** 

National Aeronautics and Space Administration Washington, D. C.

Contract No. NGR 36-008-093 OSURF Project No. 2514



The Ohio State University Research Foundation Columbus, Ohio 43212

February, 1973

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#### PREFACE

This project is under the supervision of Ivan I. Mueller, Professor of the Department of Geodetic Science at The Ohio State University, and is under the technical direction of James P. Murphy, Special Programs, Code ES, NASA Headquarters, Washington, D. C. The contract is administered by the Office of University Affairs, NASA, Washington, D. C. 20546.

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#### 1. INTRODUCTION

The basic purpose of this experiment is to compute reduced normal equations from the observational data of the South American Satellite Densification (PC-1000) Network obtained from the Defense Mapping Agency Aerospace Center, St. Louis. These reduced normal equations are to be combined with reduced normal equations of other satellite networks of National Geodetic Satellite Program [Mueller et al., 1973] so as to provide station coordinates from a single least square adjustment.

Details of this network, including instrumentation, are given in Huber [1971].

#### 2. DATA

#### 2.1 Terrestrial Data

Terrestrial data, which include base-lines, heights and survey coordinates of stations, provide the necessary relative position constraints between "collocated" stations of BC-4 World-net and the South-American Densification Net (Figure 1). Survey information regarding the observation stations is summarized in Table 2.1-1. Constraints used in this solution are given in Tables 2.1-2, 2.1-3, 2.1-4 and 2.1-5 [Mueller, et al. 1973]. Geoidal undulations (Table 2.1-5) are computed by using formula and constants as given in [Rapp, 1973].

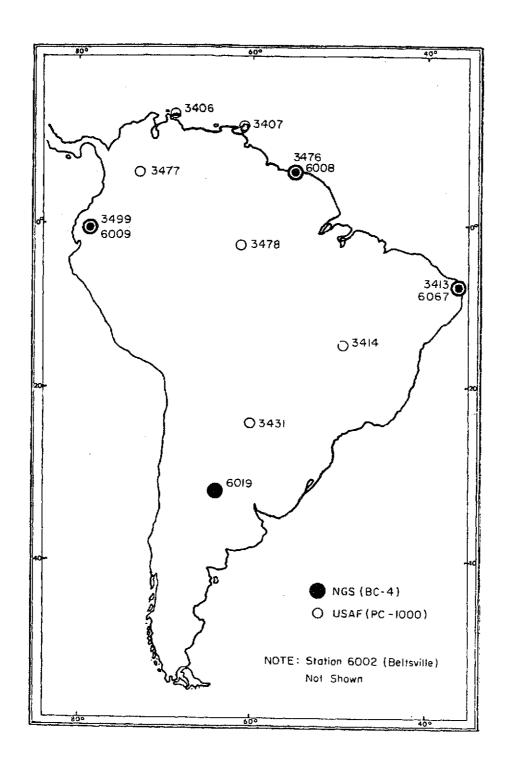


Fig. 1 South American densification net.

## Table 2.1-1

SURVEY	TNEODMATION	N OF OBSEDUATION	

STATION	MUTAD		s u	RVEY	C	0 0	RDIN	ATES <sup>2</sup>	I MSL <sup>3</sup>	I INSTR.	INSTR.	SOURCE
NO I NAME	CODE		LATI	TUDE	L	ONGI	TUDE	(ELL. H(M)	   (M)	HEIGHT*	TYPE	CODE 5
3406   CURACAD 3407   TRINIDAD 3413   NATAL 3414   BRASILIA 3431   ASUNCION 3476   PARAMARIBD 3477   BOGOTA 3478   MANAUS 3499   QUITO 6002   BELTSVILLE 6008   PARAMARIBO 6009   QUITO 6019   VILLA DOLORES 6067   NATAL	41   41   41   41   41   41   41   41	12 10 - 5 -15 -25 5 4 - 3 - 0 39 5 - 31 - 5	5 44 51 18 24 8 51 26 56 55	26.843 35.844 56.253 35.540 56.192 54.645 2.379 44.820 50.468 39.003 55.325 50.468 33.954 37.414	291 298 324 312 302 304 285 304 281 294 324	9 23 49 6 25 47 55 0 34 10 47 34 53 50	45.803 25.652 57.605 2.679 15.376 44.226 35.482 59.620 49.212 26.942 42.832 49.212 41.342 6.200	-4.0 237.0 63.0 1059.0 162.0 8.6 2586.0 2706.4 45.0 8.7 2706.7 621.0 66.7	6.83 254.80 36.90 1058.25 149.74 18.27 2557.90 83.60 2681.80 44.30 18.38 2682.10 608.18 40.63	1.25   1.25   *   1.44   1.65   1.25   1.25   1.25   1.25   1.50	PC-1000 PC-1000 PC-1000 PC-1000 PC-1000 PC-1000 PC-1000 PC-1000 BC-4 BC-4 BC-4 BC-4 BC-4	1 1 1 2 2 1 2 1 1 1 1 1 2 2 1 1 1 1 1 1

- \* INSUFFICIENT DATA
- I DATUM CODE :

29 -- NAO 1927 41 -- SAD 1969

- 2 GEODETIC COORDINATES OF THE INSTRUMENTAL REFERENCE POINT (OPTICAL/ELECTRONIC CENTER.ETC.)
  ON THE LOGAL GEODETIC DATUM
- 3 MEAN SEA LEVEL HEIGHT OF THE INSTRUMENTAL REFERENCE POINT
- 4 HEIGHT OF INSTRUMENTAL REFERENCE POINT ABOVE SURVEY MONUMENT
- 5 SOURCE CODE :

1 -- (CSC,1971)

2 -- (CSC, 1972/73)

3 -- (HUBER, 1971)

NOTE : ZERO IN THE LAST DIGIT MAY INDICATE THAT THE DIGIT IS UNKNOWN.

Table 2.1-2

Relative Position Constraints

C#-1:	Relative	Weights			
Stations	Δu	Δv Δw		(1/o <sup>2</sup> )	
3413-6067	-48.64	-289.13	1258.05	3.00	
3476-6008	36.31	22.94	-20.80	3.00	
3499-6009	0.00	0.00	0.00	100.00	

Table 2.1-3
Station Position Constraints

	Station	Coordinates (N	Weights			
Stations	u	v	w	p <sub>u</sub>	$^{\mathrm{p}}\mathrm{_{v}}$	P <sub>w</sub>
6002	1130 764.85	-4830 831.87	3994 704.05	0.2415	0.3425	0.2725
6008	3623 241.00	-5214 233.74	601 536.05	0.2212	0.2591	0.1163
6009	1280 834.24	-6250 955.94	-10 800.59	0.0776	0.0852	0.0593
6019	2280 627.09	-4914 543.17	3355 402.77	0.1779	0.1366	0.0741
6067	5186 397.12	-3653 933.25	-654 276.92	0.2315	0.2160	0.1464

Weights ( $^{1}/\sigma^{2}$ ) based as per variances obtained in OSU WN14 Solution [Mueller et al., 1973]

Table 2.1-4
Chord Constraint

Station-Station	Chord Distance (Meters)	σ x 10 <sup>6</sup>
6009-6067	4734 132.44	1,00

The chord length and the sigma were computed from the adjusted coordinates of the stations from OSU WN14 Solution [Mueller et al., 1973]

Table 2.1-5

Geoidal Undulations and Heights Used in the Constraints

		<del>-</del>		····
	STATION		HCONSTR <sup>2</sup>	HCON- STR <sup>3</sup>
NO.	NAME	(m)	(m)	(m)
0400		20.40	4.4	
3406	Curacao	-29.19	-41.02	4.0
3407	<b>Trin</b> idad	-38.57	194.88	4.0
3413	Natal	~12.03	-5.87	6.0
3414	Brasilia	-9.88	1021.23	6.0
3431	Asuncion	11.98	137.72	6.0
3476	Paramaribo	-28.31	-34.02	6.0
3477	Bogota	10.71	2551.44	6.0
3478	Manaus	-7.17	<b>53.</b> 63	6.0
3499	Quito	16.73	2682.74	6.0
6002	Beltsville	-36.90	-6.73	2.5
6008	Paramaribo	-28.31	-33.91	4.0
6009	Quito	16.73	2683.04	6.0
6019	Villa Dolores	22.80	609.43	6.0
6067	Natal	-12.03	-2.14	6.0

- 1. From [Rapp, 1973]
- 2. HCONSTR = MSL+NREF+ $\Delta N$ , where  $\Delta N$  is a correction term for the differences of position and size of the ellipsoids used [Mueller et al., 1973]
- 3. Used in Computing the Weights of the Height Constraints

#### 2.2 Satellite Observational Data

Data of South America Densification Net was obtained from the Defense Mapping Agency/Aerospace Center, St. Louis, Missouri. This data, which is a punched card-deck, is used without any modification. No major blunders are detected. (See Table 2.2-1)

#### 3. THEORETICAL BACKGROUND

### 3.1 Normal Equations for Optical Observations

A set of reduced normal equations of the form

$$\underline{N}X + \underline{\Omega} = 0$$

is obtained from the optical observations. The symmetric coefficient matrix is composed of 3 x 3 blocks of the form [Mueller, 1968]

$$\overline{N}_{kk} = \sum_{j} M_{kj}^{2} - \sum_{j} M_{kj}^{2} (\sum_{i} M_{ij}^{2})^{2} M_{kj}^{4} + P_{k}$$

$$\overline{N}_{k,1} \approx -\sum_{i} M_{k,j}^{-1} (\sum_{i} M_{i,j}^{-1})^{-1} M_{1,j}^{-1}$$

where

$$M_{i,j} = B_{i,j} P_{i,j}^{a} B_{i,j}'$$

$$B_{i,j} = SR_{3} (-\alpha) R_{2} (-90^{\circ} + \delta) \begin{bmatrix} 1 & 0 & 0 \\ 0 & -\cos \delta & 0 \\ 0 & 0 & -1 \end{bmatrix}$$

S = Polar motion matrix

and

k,1 Denotes particular ground stations

Denotes particular simultaneous event

Denotes any ground station participating in an event

 $\Sigma$  Is the summation over all ground stations involved in event j

Σ Is the summation over all events observed by ground station k and/or 1

Pk Weight matrix associated with any particular ground station

 $P_{i,j}$  Weight matrix of any observed direction

Table 2.2-1
Summary of Simultaneous Observations by Line (SA Network)

Line Station-Station	No. of Pairs	Line Station-Station	No. of Pairs
6002-6008 6002-3406 6002-3407 6002-3477 6008-6009 6008-6019 6008-6067 6008-3406	23 14 11 7 7 10 36 14 26	3406-3478 3406-3499 3407-3431 3407-3476 3407-3477 3407-3478 3413-3431 3414-3431	14 4 16 19 23 9 29 2 2
6008-3477 6008-3478 6009-6019 6009-3407 6009-3476 6009-3477 6009-3499 6019-6067 6019-3406 6019-3407	3 6 7 14 6 6 5 9 35 19 38	3476-3477 3477-3478 3477-3499	15 2 5
6019-3431 6019-3476 6019-3477 6067-3407 3406-3407 3406-3413 3406-3414 3406-3431 3406-3476 3406-3477	4 19 6 3 9 25 41 53 20 13		

Finally, the vector of constant terms is expressed by

$$\overline{U}_{k} = \sum_{i} M_{k} \int_{j}^{1} [X_{k}^{\circ} - (\sum_{i} M_{i})^{1}]^{2} \sum_{i} M_{i} \int_{j}^{1} X_{i}^{\circ}]$$

where as usual, the superscript (°) denotes initial approximate values.

#### 3.2 "Constraints" Contributions to the Normal Equations

#### 3.21 General

Since the coefficient matrix of normal equations is singular, a unique least squares solution is not possible. A minimal set of constraints to the normal equations provides a unique solution [Blaha, 1971].

Two alternative definitions exist for the term "constraints". The absolute constraints represent certain conditions which have to be fulfilled exactly and with no uncertainties while the relative constraints (or weighted constraints) have the same characteristics as the observations.

In general the contribution of the functional constraint equations

$$G(X, L_c) = 0$$

to the normal equations can be found by bordering the normal equations matrix

$$\begin{vmatrix} \overline{N} & C' \\ C & -P_c^{-1} \end{vmatrix} \begin{vmatrix} X \\ -K_c \end{vmatrix} + \begin{vmatrix} \overline{U} \\ W^c \end{vmatrix} = 0, \text{ where } C = \frac{\partial G}{\partial X}.$$

After elimination of K<sub>c</sub>, it is easy to find

$$[\overline{N} + C'P_c C]X + \overline{U} + C'P_c W^c = 0$$

or

$$[\overline{N} + N^c]X + \overline{U} + U^c = 0$$
 (1)

where  $N^c$  and  $U^c$  are the contributions to the coefficient matrix and constant vector of the normal equations due to the application of constraints. The quantities  $\overline{N}$  and  $\overline{U}$  correspond to the original normal equations without constraints (Section 3.1).

After the constraints are added the normal equations will take the usual form

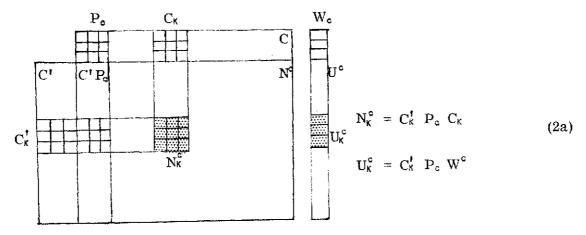
$$NX + U = 0$$

and we are in the position to obtain the contribution from a new set of constraints.

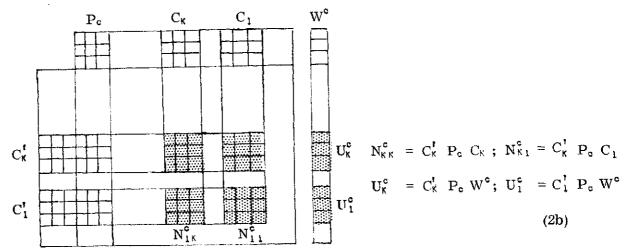
Constraints can be applied between two station k and 1 or to a single station. The contribution of these constraints to the matrix  $\overline{N}$  (3 x 3) blocks) and  $\overline{U}$  (3 x 1 blocks) can be schematically expressed in two different ways.

Assuming that the matrix  $P_{\text{c}}$  is always diagonal it is possible to express:

(a) Contribution to the normals due to the constraint applied to station k



(b) Contribution to the normals due to the constraint between stations k and 1



These blocks obtained as indicated above for the corresponding case will be the only ones computed and added to the original normal equations as expressed by formula (1).

#### 3.22 Relative Position Constraints

Relative position constraints are used in order to combine the normal equations obtained from various satellite nets and to constrain "double" stations or closely situated stations of the same net. The expression for the combination of normals can be written as follows:

$$[\overline{N} + N^R] X + \overline{U} + U^R = 0$$

where  $N^R$  and  $U^R$ , computed from (2a), (2b), are the contribution to the original normal equations  $(\overline{N}X + \overline{U} = 0)$ .

If the relative position ( $\Delta u^{\circ}$ ,  $\Delta v^{\circ}$ ,  $\Delta w^{\circ}$ ) of two stations is known, along with the standard deviation of these relative positions, the constraints can be formed. In this case the functional constraint equations are

$$\mathbf{u}_{\mathbf{k}}^{\circ} - \mathbf{u}_{1}^{\circ} = \Delta \mathbf{u}^{\circ}$$

$$\mathbf{v}_{\mathbf{k}}^{\circ} - \mathbf{v}_{\mathbf{i}}^{\circ} = \Delta \mathbf{v}^{\circ}$$

$$\mathbf{w}_{k}^{\circ} - \mathbf{w}_{l}^{\circ} = \Delta \mathbf{w}^{\circ}$$

Therefore

$$C_k^R = I$$
;  $C_1^R = -I$ 

and

$$U_k^R = 0$$
;  $U_1^R = 0$  because  $W^R = G^R (X^O, L_c^O) = 0$ 

where

$$\mathbf{P}_{R} = \begin{bmatrix} \frac{1}{\sigma_{\Delta}^{2} \mathbf{u}^{\circ}} & 0 & 0 \\ 0 & \frac{1}{\sigma_{\Delta \mathbf{v}^{\circ}}^{2}} & 0 \\ 0 & 0 & \frac{1}{\sigma_{\Delta \mathbf{w}^{\circ}}^{2}} \end{bmatrix}$$

and

$$N_{KK}^{R} = I P_{R} I = P_{R}$$
 $3x3$ 
 $N_{I1}^{R} = I P_{R} I = P_{R}$ 
 $3x3$ 
 $N_{K1}^{R} = N_{IK}^{R} = I P_{R} (-I) = -P_{R}$ 
 $3x3$ 
 $3x3$ 

Thus, the diagonal elements of  $P_{\overline{N}}$  are added to each element of the diagonal of the blocks  $\kappa\kappa$  and 11 of the matrix of the combined normals  $\overline{N}$ , and subtracted from the diagonal elements of the blocks  $\kappa$  1 and 1 $\kappa$  of  $\overline{N}$ . There is no contribution to the vector  $\overline{U}$ .

#### 3.23 Length (Chord) Constraints

Chord constraints are introduced when scalar information is available between ground stations (e.g., distances determined through high precision geodimeter traversing). The functional constraint equation in this case is

 $G^{c}(X, L_{c}) = 0$ 

or

$$[(u_{K} - u_{1})^{2} + (v_{K} - v_{1})^{2} + (w_{K} - w_{1})^{2}] = L_{K1}$$

$$C_{K}^{c} = \left[\frac{u_{K}^{\circ} - u_{1}^{\circ}}{L_{K1}^{\circ}}, \frac{v_{K}^{\circ} - v_{1}^{\circ}}{L_{K1}^{\circ}}, \frac{w_{K}^{\circ} - w_{1}^{\circ}}{L_{K1}^{\circ}}\right]$$
(3)

and

$$C_{1 \text{ x3}}^{c} = - \begin{bmatrix} \underline{u_{\text{K}}^{\circ} - \underline{u_{1}^{\circ}}} & , & \underline{v_{\text{K}}^{\circ} - \underline{v_{1}^{\circ}}} & , & \underline{w_{\text{K}}^{\circ} - \underline{w_{1}^{\circ}}} \\ \underline{L_{\text{K}1}^{\circ}} & & \underline{L_{\text{K}1}^{\circ}} & & \end{bmatrix}$$

and

$$P_c^1 = \frac{\sigma_{K1}^2}{\sigma_0^2} = \frac{\text{variance of the chord}}{\text{a priori variance of unit weight}}$$

Then the contribution to the normals are obtained by applying (2a) and (2b)

as under:

The first three expressions in the above are added respectively to the blocks  $N_{K\,K}$ ,  $N_{1\,1}$  and  $N_{K\,1}$  of  $\overline{N}$ ; the last two expressions are added respectively to the constant vectors  $U_K$  and  $U_1$  of  $\overline{U}$ .

#### 3.24 Station Position Constraints

Station position constraint is used for the purpose of defining the origin of the coordinate system. If the station coordinates  $(u_k^\circ, v_k^\circ, w_k^\circ)$  of station  $\kappa$  are to be constrained and if the computed (known) variances of its known coordinates are  $\sigma_{u_k^\circ}^2$ ,  $\sigma_{v_k^\circ}^2$ ,  $\sigma_{u_k^\circ}^2$ , then the equations given in section 3.22 are valid by merely deleting the terms with index 1, i.e.,  $\Delta u^\circ = u_k^\circ$ ,  $\Delta v^\circ = v_k^\circ$ ,  $\Delta w_k^\circ = w_k^\circ$ . Hence

$$N_{KK}^{s} = I P_{s} I = P_{s}$$

where

$$\mathbf{P}_{S} = \begin{bmatrix} \frac{1}{\sigma_{\mathbf{u}_{K}}^{2}} & 0 & 0 \\ 0 & \frac{1}{\sigma_{\mathbf{v}_{K}}^{2}} & 0 \\ 0 & 0 & \frac{1}{\sigma_{\mathbf{w}_{K}}^{2}} \end{bmatrix}$$

#### 3.25 Height Constraints

If the geodetic (ellipsoidal) heights  $H_{\kappa}$  of the station  $\kappa$  is to be constrained, then

$$N_{KK}^{H} = (C_K^H)^{\dagger} P_H C_K^H$$

where

$$C_{K}^{H} = [\cos \varphi_{K}^{\circ} \cos \lambda_{K}^{\circ}, \cos \varphi_{K}^{\circ} \sin \lambda_{K}^{\circ}, \sin \varphi_{K}^{\circ}]$$

and

$$P_{H} = \frac{1}{\sigma_{H_K}^2}.$$

Here  $\phi_K^o$  and  $\lambda_K^o$  are the approximate geodetic coordinates and  $\sigma_{H_K}^2$  is the variance of the height for station  $\kappa$ .

The constant vector UK can be computed from

$$\mathbf{U}_{K}^{H} = (\mathbf{C}_{K}^{H})^{\dagger} \mathbf{P}_{H} \mathbf{W}^{H}$$

where

$$W^{H} = H_{K} - H_{K}^{o}$$
,  $H_{K}^{o}$  being the approximate height.

#### 3.26 Inner Constraints (Free Adjustment)

Even though the selection of a coordinate system is arbitrary in the case of a minimum constraint adjustment, e.g., in the case of ranging, the selection of the six coordinates (at more than two stations) to be constrained is very critical, since one set of constraints would give a different solution than another set. The "best" solution is arrived at in a coordinate system defined through the use of a set of constraint equations called "inner" constraints [Rinner et al., 1967]. In this sense, "best" means resulting in the smallest covariance matrix for the unknowns. Covariance matrices may be compared by means of their traces, and the inner constraint equations are characterized by the property that the trace of the covariance matrix obtained with their use is a minimum among those obtained by adjusting a given set of observations augmented by a minimal set of constraint equations. This property also implies that the mean square uncertainty of the

unknowns is smaller when the inner adjustment equations are used. The resulting adjustment is called a "free" one. The functional inner constraints equations can be written as

$$C^{I}X = 0$$

where X is the set of corrections of the approximate coordinates of the unknown points and in the most general application when the "best" origin, orientation and scale are sought

$$C^{T} = \begin{bmatrix} C_{1}^{T} \\ C_{2}^{T} \\ C_{3}^{T} \end{bmatrix} = \begin{bmatrix} \frac{1}{3 \times 3} & \frac{1}{3$$

The symbols  $(u_1^{\circ}, v_1^{\circ}, w_1^{\circ})$  denote the approximate coordinates of the ith unknown point where both the ground points and the satellite positions are considered.

It is also possible to design a set of constraints that will result in the "best" solution for only a subset of the points. In the adjustment reported here we were only interested in the gound station unknowns implying that the trace of only that portion of the covariance matrix corresponding to the ground station unknowns should be minimized, while the variances of the satellite position unknowns should not be included in the minimum sum. The constraint equations that will produce such a solution have the same form as those producing the "best" solution for all the points; however, 3 x 3 blocks of zeros are inserted into those positions of I which correspond to unknowns whose variances are not to be included in the minimum sum.

The inner adjustment constraint equations can be given a geometrical interpretation that appeals to intuition. Let  $X_i^{\text{O}}$  denote the set of approximate coordinates of the ith unknown point,  $dX_i$  denote the corrections to

these coordinates, and X<sub>1</sub> denote the adjusted coordinates, i.e.,

$$X_1 = X_1^0 + dX_1$$

The first set of constraint equations,  $C_1^{\uparrow}X = 0$ , is then equivalent to the set of conditions

$$\sum_{i} dX_{i} = 0$$

The geometrical interpretation of these conditions is that the center of gravity of all the points will not change after adjustment, i.e.,

$$\sum_{i} X_{i}^{\circ} \times dX_{i} = 0$$

If the center of the system remains fixed, then the cross products  $X_1^{\circ} \times dX_1$  reflect rotations of the points around the fixed center. These constraint equations insure that the sums of the rotations around all three coordinate axes are zero. The corresponding geometrical interpretation is that the mean orientation of the system of points will not change after adjustment either.

Thus, the respective equations  $C_1^IX = 0$  and  $C_2^IX = 0$  effectively specify the origin and the orientation of the adjustment coordinate system. A third "inner adjustment" equation  $C_3^IX = 0$  specifies the scale of the system. However, this scale equation is only used when the observations themselves do not determine the scale.

A more complete description of the inner adjustment is described in [Blaha, 1971].

In summary, if the normal equations with the contribution of all the constraints (except inner constraints) are represented by

$$[\overline{N} + N^{R} + N^{C} + N^{S} + N^{M}]X + \overline{U} + U^{R} + U^{C} + U^{E} + U^{H} = 0$$
 (5)

$$NX + U = 0$$

or

then the inner adjustment can be obtained by bordering the coefficient matrix N of the normal equations as

$$\begin{bmatrix} \mathbf{N} & (\mathbf{C}^{\mathbf{I}})^{\dagger} \\ \mathbf{C}^{\mathbf{I}} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{X} \\ -\mathbf{K} \end{bmatrix} = \begin{bmatrix} -\mathbf{U} \\ \mathbf{0} \end{bmatrix}$$
 (6)

Upon the addition of any kind of constraint to the normal equations, it becomes necessary to consider also its contribution to  $\Sigma$  V'PV. The degrees of freedom change as well. In order to compute the proper variance of unit weight the latter must be taken into consideration.

#### 4. THE SOLUTION

Using the specified constraint of Section 3.2, SA-10 solution is computed. A general information of this solution is given in Table 4-1. The solution is obtained by using the general OSUGOP program (Reilly et al., 1972).

The station coordinates of SA-10 solution are given in Table 4-2 with their standard deviations.

Table 4-1

General Information on the SA-10 Geometric Adjustment

No. of observing stations	14
σ <sub>s</sub> (a priori)	1.0
No. of degrees of freedom	649
ΣV'PV	721.76
ο̂, (a posteriori)	2.50
Number of constraints used:	
Relative position constraints	3
Length (Chord) constraint	1
Station position constraints	5
Height constraints	14
Inner constraint defines the origin of the coordinate system.	

Table 4-2
Cartesian and Geodetic Coordinates
(Solution SA 10)

Sta. No.	u	$\sigma_{\mathtt{u}}$	v	$\sigma_{\rm v}$	w	O,
	φ	$\sigma_{\!arphi}$	λ	$\sigma_{\lambda}$	Н	OH.
		aa	$A_{\mathbf{e}}$	$\mathbf{r}_{\mathbf{s}}$		
		$\mathbf{a}_{\mathtt{b}}$	$\mathbf{A}_{b}$	$\mathbf{r}_{\mathtt{b}}$		
		$\mathbf{a}_{\mathrm{c}}$	$A_c$	$\mathbf{r_c}$		

- u, v, w Cartesian coordinates in meters (Orientation: u = the Greenwich meridian as defined by the B.I.H.;  $v = \lambda = 90^{\circ}$  (E); w = Conventional International Origin).
- Geodetic latitude and longitude in angular units (degrees, minutes and seconds of arc) computed from the Cartesian coordinates and referred to a rotational ellipsoid of a =  $6378155.00 \,\mathrm{m}$  and b =  $6356769.70 \,\mathrm{m}$ .
- H Geodetic (ellipsoidal) height in meters referred to the same ellipsoid.
- $\sigma_{\!\scriptscriptstyle U}$  ,  $\sigma_{\!\scriptscriptstyle V}$  ,  $\sigma_{\!\scriptscriptstyle W}$  Standard deviations of the Cartesian coordinates in meters.
- $\sigma_{\varphi}$ ,  $\sigma_{\lambda}$  Standard deviations of the geodetic coordinates in seconds of arc.
- $\sigma_{H}$  Standard deviations of the geodetic height in meters.
- a, A, r, Altitude (elevation angle), azimuth and magnitude of the major semi axis of the error ellipsoid, respectively. Angles in degrees, magnitude in meters. Altitude is positive above the horizon. Azimuth is positive east reckoned from the north.
- $a_{\text{b}}$ ,  $A_{\text{b}}$ ,  $r_{\text{b}}$  Same as above for the mean axis of the error ellipsoid.
- ac, Ac, rc Same as above for the minor axis of the error ellipsoid.

Table 4.2 (Con't)

			501/000 01	2.04	1227210 20	7.37
3406	2251789.08	5.57   0.24	-5816902.96 291 9 43.15	3.94 0.20	1327210.38 -46.08	3.53
<u></u>	12 5 26.57	0.74	271 7 43.13	0.20	-40.00	3.75
		8.88	-32.18	8.16		
İ		-5.04	57.03	4.83		
1		-79.77	-62.24	3.33		
<del></del>			and the second of the second o		The state of the s	
3407	2979880.94	7.18	-5513533.18	4.49	1181138.43	6.20
	10 44 35.20	0.20	<b>298 23 23.</b> 08	0.25	185.59	3.71
		- 10	110 77	0.21		
		-3.40	118.77	8.31 5.44		
		20.28 -69.41	30.03 19.66	3.37		
		-07 +41	17,00	2.2.		
3413	5186345.51	2.00	-3654218.56		-653018.68	2.71
	- 5 54 57.55	0.09	324 49 55.45	0.07	-4.76	1.97
		2 70	3.66	2.71		
		-3.79 -8.55	94.23	2.14		
		80.64	69.95	1.96		
		60.04	07475	10,0		
						~ 1 ~
3414	4114977.26		-4554124.02	5.71	-1732149.84	
	-15 51 37.37	0.24	312 6 0.26	0.27	1002.05	4.88
		2.88	50.36	9.10		
		-11.13	-39.07	5.84		
		-78.50	126.06	4.82		
					4714654 40	10 61
3431	3093028.20		-4870063.97	6.08	-2710839.99	10.51 4.94
	<b>-25 18 58.25</b>	0.36	302 25 12.20	0.26	130.54	4.74
		-5.20	11.62	11.19		
		-1.83	101.79	7.07		
		-84.48	-148.88	4.85		
•		- · - · ·	•	• •		
9,9,	2/2227	2 1/	_E21/200 00	1 07	601517.56	2 • 84
3476	3623275.84 5 26 52.81		~5214208,28 304 47 41.51	1.87 0.07	-39.47	
	5 40 54.61	U <b>+U</b> 7	DO4 11 41+51	0407		1901
		3.15	-6.32	2.85		
		10.83	84.28	2.20		
		-78.72	67.69	1.79		

Table 4.2 (Con't)

3477	1744632.76 9 4 49 0.44 0	.75 -6 .32 285	114278.13 55 31.56	6.57 0.34	532213•19 2542•42	
	-2. -43. 46.	15 1	47.57 40.16 34.64	13.26 5.79 4.90		
3478	3185743.08 11 - 3 8 43.73 0				-347640.88 36.97	
•	-0. -21. 68.	96 😘	30.23 59.85 59.31	23.24 7.91 5.31		
3499	1280825.47 2 - 0 5 51.25 0	.996 .13 281	250950 ±21. 34 46 •84	2.60	-10793.24 2676.43	
•	2. 10. -79.	43 1	14.32 04.78 90.96	4.07 2.90 2.60		
6002	1130763.54 2 39 1 39.39 0		830831.02 10 27.00	1.63	3994704.49 -7.30	
	1. 11. -78.	74	05.75 15.36 24.27	2.11 1.93 1.52		
6008	3623239.59 2 5 26 53.49 0		214231.39		601538•31 -39•20	2.79 1.74
	3. 11. -78.	0.7	-5.65 84.98 68.21	2.80 2.12 1.71		
6009	1280825.47 2 - 0 5 51.25 0	.13 281	250950.22 34 46.84	2.59 0.10	-10793.24 2676.43	4.01 2.61
	2. 10. -79.	48 <b>1</b>	14.32 .04.78 91.01	4.07 2.90 2.59		
6019		=	914540.71 53 38.34	2.64 0.10	-3355401.90 603.72	3.65 2.77
	-24. -46. 33.	19 1	-0.19 .17.61 72.26	3.67 2.63 2.41		

Table 4.2 (Con't)

In the tables the rotations  $\omega$ ,  $\psi$ , and  $\epsilon$  are about the w, v, and u axes, respectively. The scale factor  $\Delta$  is in units of ppm. The unit in the variance-covariance matrix, for the elements corresponding to the rotations, is radian squared.

#### 5. COMPARISON WITH OTHER SOLUTIONS

Table 5.1-1 summarizes the transformation parameters between SA 10 and South American 1969 Datum, and Table 5.1-2 between SA 10 and WN14 Solution [Mueller et al., 1973].

Table 5.1-1

#### SA10 -TO- SAD-69 \*\*\*\*\*\*\*\*\*\*\*\*

#### SCALE FACTOR AND ROTATION PARAMETERS CONSTRAINED

# SOLUTION FOR 3 TRANSLATION, 1 SCALE AND 3 ROTATION PARAMETERS

#### (USING VARIANCES ONLY)

DU METERS	DV DW S METERS METERS				PSI SECONDS		
59.65	16.52	36.33	5.80	-0.79	0.12	0.21	

#### VARIANCE - COVARIANCE MATRIX

 $\sigma_o^2 = 1.13$ 

-0.1260-0f	-0.2770-05	0.967D-05	-0.447D-05	0.106D+02	0.5840+01	0.857D+02
0.1300-05	-0.358D-06	0.564D-05	0.738D-05	0.1860+01	0.7490+02	0.584D+01
-0.9790-05	-0.644D-05	0.1610-05	0.137D-05	0.9230+02	0.186D+01	0.1060+02
0.1100-13	0.3190-14	0.7210-14	0.145D-11	0.1370-05	0.738D-05	<b>-0.447</b> D-05
-0.1520-12	<b>-0.26</b> 5D <b>-12</b>	0.1870-11	0.7210-14	0.1610-05	0.564D-05	0.9670-05
0.440D-12	0.1360-11	-0.2680-12	0.3190-14	-0.644D-05	-0.358D-06	-0.277D-05
0.1680-11	0.440D-12	-0.1520-12	0.1100-13	-0.979D-05	0.1300-05	-0.1260-05

#### COEFFICIENTS OF CORRELATION

0.1000+01	0.7300-01	0.1190+00	-0.401D+00	0.764D+00	-0.2570+00	-0.105D+00
0.7300-01	0.1000+01	0.224D-01	0.7080+00	0.476D+00	-0.3550-01	0.1160+00
0.1190+00	0.224D-01	0.1000+01	0.118D+00	0.122D+00	-0.576D+00	-0.7889+00
-0.4010+00	0.7080+00	0.1180+00	0.1000+01	0.4380-02	0.2270-02	0.7050-02
0.7640+00	0.4765+00	0.1220+00	0.4380-02	0.1000+01	-0.168D+00	-0.8560-01
-0.2570+00	-0.355D-01	-0.576D+00	0.2270-02	-0.168D+00	0.1000+01	0.2920+00
	0-1160+00	-0-788D+00	0.7050-02	-0.8560-01	0 - 2920+00	0.1000+01

Table 5.1-1 (Con't)

# RESIDUALS V

	V1( SA10 )			V2( SAD-69 )				V1 - V2		
3414	. 0.4	-2 •8	1.3	3414	-0.7	8.5	-2.6	1.2	-11.3	3.9
3431	3.0	-0.1	7.8	3431	-5.9	0.3	-7.1	9.0	<del>~</del> 0.4	14.9
3477	17.7	0.9	8.3	3477	-18.7	-2.1	-8.7	36.4	3.1	16.9
6008	-0.2	0.3	1.0	6008	3.8	-8.2	-12.5	-4.0	8 • 4	13.5
6009	-0.9	-0.3	-2.6	6009	10.0	4.9	16.4	-10.9	-5.2	-19.0
6019	-0.4	0.5	-0.9	6019	7.4	-7.3	7.1	-7.8	7.8	-6.1
		-0.2		6067	4.1	3.9	7.3	-4.3	-4.0	-7.8

#### Table 5.1-2

#### SA10 -TO- WN14 \*

## SCALE FACTOR AND ROTATION PARAMETERS CONSTRAINED

# SOLUTION FOR 3 TRANSLATION, 1 SCALE AND 3 ROTATION PARAMETERS

#### (USING VARIANCES ONLY)

. DN nv DW - DELTA OMEGA P S I **EPSILON** METERS METERS (X1.0+6) SECONDS SECONDS SECONDS -4.92 -7.78 -0.44-0.13 0.07 0.10 1.63

#### VARIANCE - COVARIANCE MATRIX

 $\sigma_0^2 = 0.92$ 

0.365D+01 0.521D+00 0.880D+00 -0.171D-06 0.403D-06 -0.339D-07 -0.134D-06 
0.521D+00 0.315D+01 0.462D+00 0.244D-06 0.286D-06 -0.304D-07 -0.120D-06 
0.880D+00 0.462D+00 0.363D+01 -0.444D-07 0.162D-06 -0.168D-06 -0.277D-06 
-0.171D-06 0.244D-06 -0.444D-07 0.505D-13 -0.155D-15 0.390D-15 0.351D-15 
0.403D-06 0.286D-06 0.162D-06 -0.155D-15 0.856D-13 -0.884D-14 -0.284D-13 
-0.339D-07 -0.304D-07 -0.168D-06 0.390D-15 -0.884D-14 0.431D-13 0.740D-14 
-0.134D-06 -0.120D-06 -0.277D-06 0.351D-15 -0.284D-13 0.740D-14 0.535D-13

#### COEFFICIENTS OF CORRELATION

0.100D+01 0.154D+00 0.242D+00 -0.398D+00 0.720D+00 -0.855D-01 -0.302D+00 0.154D+00 0.100D+01 0.137D+00 0.612D+00 0.551D+00 -0.825D-01 -0.292D+00 0.242D+00 0.137D+00 0.100D+01 -0.104D+00 0.291D+00 -0.424D+00 -0.629D+00 -0.398D+00 0.612D+00 -0.104D+00 0.100D+01 -0.236D-02 0.835D-02 0.675D-02 0.720D+00 0.551D+00 0.291D+00 -0.236D-02 0.100D+01 -0.145D+00 -0.420D+00 -0.855D-01 -0.825D-01 -0.424D+00 0.835D-02 -0.145D+00 0.100D+01 0.154D+00 -0.302D+00 -0.292D+00 -0.629D+00 0.675D-02 -0.420D+00 0.154D+00 0.100D+01

Table 5.1-2 (Con't)

# RESIDUALS V

	V1 (	SALO	}		V2( W	N14 J		VI	- V2		
	********				***				7% 4% 45 T T		
3406	6.6	-5.3	-14.5	3406	-1.2	1.5	3.0	7.8	-6.7	-17.6	
3407	4.9	3.5	-4.3	3407	-2.1	-1.9	3.1	7.0	5.4	-7.4	
3413	0.1	-0.2	1.0	3413	-0.1	0.3	-0.9	0.2	-0.5	1.9	
3414	-1.4	-6.6	-1.2	3414	1.5	7.6	1.2	-2.9	-14.2	-2.4	
3431	6.3	-5.8	9.1	3431	-7.1	6.7	-9.7	13.3	-12.5	18.8	
3476	8 • 0	0.3	-0.3	3476	0.8	-0.4	0.3	-1.7	0.7	-0.6	
3477	6.6	-2.3	-1.5	3477	-7.2	2.4	1.4	13.8	-4.7	-2.0	
3478	8.0	2.4	-15.4	3478	-22.4	-5.8	45.4	30.4	8.2	-60.7	
3499	2.0	-0.5	-2.8	3499	-2.9	0.9	2.9	4.9	-1.4	-5.7	
6002	-0.5	0.7	1.1	6002	0.5	-0.8	-1.1	-1.0	1.5	2.2	
6008	-0.8	0.4	-0.3	6008	0.9	-0.5	0.3	-1.7	0.8	-0.6	
6019	-1.8	1.6	0.5	6019	1.7	-1.7	-0.5	-3.5	3.4	1.1	
6067	0.1	-0.2	0.9	6067	-0.1	0.2	-0.9	0.1	-0.3	1.9	

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