

Optimal measurement of ice-sheet deformation
from surface-marker arrays

by



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Abstract

Surface strain rate is best observed by fitting a strain-rate ellipsoid to the measured movement of a stake network, or other collection of surface features, using a least-squares procedure. Error of the resulting fit varies as $(L\Delta t\sqrt{n})^{-1}$, where L is the stake separation, Δt is the time period between initial and final stake survey, and n is the number of stakes in the network. This relation suggests that if n is sufficiently high, the traditional practice of re-visiting stake-network sites on successive field seasons may be replaced by a less costly single-year operation. A demonstration using Ross Ice Shelf data shows that reasonably accurate measurements can be obtained from 12 stakes after only 4 days of deformation. The least-squares procedure may also aid airborne photogrammetric surveys in that reducing the time interval between survey and re-survey could permit better surface-feature recognition.

INTRODUCTION

Horizontal strain rate, vertical vorticity and horizontal velocity observed at the surface of large ice sheets and ice shelves constitute field data essential to the study of large-scale ice-flow dynamics and mass balance. These data are traditionally acquired by implanting stake networks and surveying their deformation over a known time interval [Nye, 1959; Zumberg and others, 1960; Thomas and others, in press; Drew and Whillans, in press]. In practice, a one-year time interval is required to allow relative stake displacements in excess of measurement resolution. This one-year period necessitates the deployment of survey parties on two successive field seasons. This paper presents a technique, based on prior work by Nye [1957, 1959], that reduces the time interval between initial and final survey and eliminates costly survey party re-deployment.

Typical stake networks used to measure the general flow of the Ross Ice Shelf and its grounded margins, for example, consist of four individual stakes arranged in either a rosette or a rectangular pattern [Thomas and others, in press]. These networks provide a degree of data redundancy because only three stakes are required to measure horizontal strain rates and the vertical component of vorticity. In practice, no significance is assigned to apparent deformation gradients within the network, so this redundancy is generally treated by averaging the results of the four triangular sub-arrays [Thomas and others, in press].

This study examines the consequence of expanded data redundancy within compact stake networks to determine whether the time interval between survey and re-survey can be shortened to less than one field season. Figure (1) shows an example of such a stake network. The results demonstrate that greater redundancy can shorten field operations and

suppress aliasing errors that may occur when small-scale strain-rate fluctuations are superimposed on the average large-scale deformation. The stake-network design features that achieve these advantages may additionally be useful for certain remote sensing programs that employ large numbers of natural surface features which can become obscured from one year to the next.

Sources of measurement error:

The horizontal strain-rate tensor components $\dot{\epsilon}_{ij}$ ($i = 1, 2; j = 1, 2$) and vertical vorticity component $\dot{\omega}_z$ are defined by the following expressions:

$$(1) \quad \dot{\epsilon}_{ij} = \frac{1}{2} \left[\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right] \quad i, j = 1, 2$$

and,

$$(2) \quad \dot{\omega}_z = \left[\frac{\partial u_2}{\partial x_1} - \frac{\partial u_1}{\partial x_2} \right]$$

where $\underline{u} = (u_1, u_2, u_3)$ is the ice velocity and subscripts 1, 2, and 3 refer to a orthogonal coordinate system (x, y, z) having unit vectors \hat{e}_x , \hat{e}_y and \hat{e}_z such that \hat{e}_z is perpendicular to the geoid and \hat{e}_x and \hat{e}_y define the horizontal plane.

Measurement of stake displacements by conventional surveying equipment establishes the velocity gradients required to solve Equations (1) and (2) for $\dot{\epsilon}_{ij}$ and $\dot{\omega}_z$. Sufficient time must be allowed between initial and final survey so that the displacements will exceed measurement error and any unnatural stake disturbances caused by wind or sunlight. If, for example, $\Delta L = 0.02$ m is the stake position uncertainty, $L = 1.5 \times 10^3$ m is the stake separation, and $\dot{\epsilon} = 1 \times 10^{-10} \text{ sec}^{-1}$ is the strain rate scale (typical for

Ross Ice Shelf conditions), then a time interval of

$$(3) \quad \Delta t = \frac{100 \Delta L}{L \dot{\epsilon}} \approx 0.4 \text{ years}$$

is required between initial and final stake survey to achieve a displacement measurement accuracy of 1%. This time span is generally too long to be fit within a single field season; thus a second field season, incurring the duplicate cost of field party re-deployment, is required.

An alternative to allowing one year between survey and re-survey is to plant more stakes in each network. From elementary statistics, the measurement error should reduce roughly as $\frac{1}{\sqrt{n}}$ where n is the number of stakes (or where n is the number of independent measurements of a single stake) [Mendenhall and Scheaffer, 1973]. It is thus conceivable that, by choosing n sufficiently high, Δt can be reduced to several days or weeks.

A second potential advantage gained by using stake arrays with large numbers of stakes is the reduction of aliasing errors caused by small-scale strain rate and vorticity fluctuations superimposed on the large-scale deformation. Defining $\dot{\epsilon}_{ij}^a$ as the strain rate averaged over a suitably chosen large-scale distance and $\dot{\epsilon}_{ij}^v$ as the fluctuation around this average, the total strain rate $\dot{\epsilon}_{ij}$ may be defined by

$$(4) \quad \dot{\epsilon}_{ij} = \dot{\epsilon}_{ij}^a + \dot{\epsilon}_{ij}^v$$

Uniaxial strain-rate data from the Ross Ice Shelf, shown in Figure (2), demonstrates that $\dot{\epsilon}_{ij}^v$ is not necessarily smaller than $\dot{\epsilon}_{ij}^a$ and can vary over a length scale commensurate with the actual 1-2 km stake-separations generally used on ice shelves [Thomas and others, in press].

Many field programs are designed to measure $\dot{\epsilon}_{ij}^a$ rather than $\dot{\epsilon}_{ij}^v$; however, it may not be possible to select a stake placement that will average out the effects of the unknown $\dot{\epsilon}_{ij}^v$. Aliasing errors will consequently arise in the measured value of $\dot{\epsilon}_{ij}^a$. The reduction of aliasing errors can be achieved through either increasing the stake separation or increasing the number of stakes to allow, in effect, averaging of individual aliasing errors. Planting stakes with large separations may be ruled out by equipment limitations, so increasing the number of stakes may again provide the best alternative.

Strain-rate estimation from an over-determined data set

An n-leg generalization of the 3-leg rosette stake network used by Thomas and others [in press] on the Ross Ice Shelf will be examined to determine the advantages of redundancy and demonstrate the data-analysis technique. Each outlying stake of this rosette defines a position vector (x_i, y_i) $i = 1, n$ that will change by $(\delta x_i, \delta y_i)$ $i = 1, n$ as a result of horizontal ice-shelf strain in the time Δt_i , $i = 1, n$ between initial survey and re-survey. For purposes of this analysis, (i) the net strain is infinitesimal and (ii) the strain-rate variations over the length scale spanned by the stake network are considered noise. Rosette design procedures other than those described here should be adopted if either of these conditions are not met.

The set of n observed stake displacements are converted into the four horizontal velocity component gradients $\frac{\partial u_i}{\partial x_j}$ $i, j = 1, 2$ by solving the following linear equation:

$$(5) \quad \underline{A} \underline{s} = \underline{d} - \underline{z}$$

Here \underline{s} is the column vector $\left(\frac{\partial u_x}{\partial x}, \frac{\partial u_x}{\partial y}, \frac{\partial u_y}{\partial x}, \frac{\partial u_y}{\partial y} \right)^T$ representing the true local average of the horizontal velocity gradients, \underline{d} is the column vector $\left(\frac{\delta x_1}{\Delta t_1}, \frac{\delta x_2}{\Delta t_2}, \dots, \frac{\delta x_n}{\Delta t_n}, \frac{\delta y_1}{\Delta t_1}, \frac{\delta y_2}{\Delta t_2}, \dots, \frac{\delta y_n}{\Delta t_n} \right)^T$ that represents the measured stake displacements resulting from the true ice strain \underline{s} , \underline{z} is the column vector $\left(\frac{\sigma_1}{\Delta t_1}, \frac{\sigma_2}{\Delta t_2}, \dots, \frac{\sigma_n}{\Delta t_n}, \frac{\sigma_{n+1}}{\Delta t_1}, \frac{\sigma_{n+2}}{\Delta t_2}, \dots, \frac{\sigma_{2n}}{\Delta t_n} \right)^T$ that represents the disturbance of the true stake displacements resulting from inaccurate measurement and from actual strain rate variation over the rosette, and \underline{A} is the $2n \times 4$ stake-position matrix given by

$$(6) \quad \underline{A} = \begin{bmatrix} x_i & y_i & 0 & 0 \\ \cdot & \cdot & \cdot & \cdot \\ x_n & y_n & 0 & 0 \\ 0 & 0 & x_1 & y_1 \\ \cdot & \cdot & \cdot & \cdot \\ 0 & 0 & x_n & y_n \end{bmatrix}$$

The noise vector \underline{z} is assumed to be a random variable characterized by the following $2n \times 2n$ covariance matrix $\langle \underline{z} \underline{z}^T \rangle$:

inaccessible data elements.

Pre-weighting and scaling

Before deriving the inverse of \underline{A} , Equation (5) is modified so that the covariance matrix $\langle \underline{z} \underline{z}^T \rangle$ has equal diagonal elements and the variables are scaled dimensionless quantities of order one [Wunsch, 1978]. The dimensionless weighting matrix \underline{W} defined by

$$(8) \quad W_{ij} = \frac{\sigma^2 \Delta t_i^2}{\sigma_i^2} \delta_{ij}; \quad i, j = 1, 2n$$

where $\sigma^2 = \frac{1}{2n} \sum_{i=1}^{2n} \sigma_i^2 / \Delta t_i^2$, $\delta_{ij} = 0$ if $i \neq j$, and $\delta_{ij} = 1$ if $i = j$, is used to transform Equation (5) as follows:

$$(9) \quad \underline{W}^{1/2} \underline{A} \underline{s} = \underline{W}^{1/2} \underline{d} - \underline{W}^{1/2} \underline{z}$$

where $W_{ij}^{1/2} = \sqrt{W_{ij}}$. Observe that now $\langle \underline{W}^{1/2} \underline{z} \underline{z}^T \underline{W}^{1/2} \rangle = \sigma^2 \underline{I}$. To avoid unnecessary notational complication, however, the factors $\underline{W}^{1/2}$ will be henceforth dropped from Equation (11).

The purpose of the weighting matrix $\underline{W}^{1/2}$ is to render each component of each observed stake displacement rate in a form having the same uncertainty. In practice, the displacement rate vectors $(\frac{\delta x_i}{\Delta t_i}, \frac{\delta y_i}{\Delta t_i})$ $i=1, \dots, n$ are measured in polar coordinates $(\frac{\delta R_i}{\Delta t_i}, \frac{\delta \theta_i}{\Delta t_i})$ $i=1, \dots, n$ where the uncertainty of $\delta \theta_i$ is considerably higher than $\frac{\delta R_i}{R_i}$. This disparity results from the practical limitations of conventional theodolite and electronic distance measuring devices. Typically, accuracies of $\pm 0.5 \times 10^{-2} \text{m}$ and $\pm 10''$ are attainable for δR_i and $\delta \theta_i$, respectively. Unless the outlying stakes are placed closer than $\sim 100\text{m}$ from the central stake, these disparate measurement uncertainties will render the uncertainty of the tangential component of $(\delta x_i, \delta y_i)$ greater than that of the radial component.

The advantage of using the weighting matrix $\underline{W}^{1/2}$ is that it allows the data elements of \underline{d} representing radial displacements to weigh more heavily than those representing tangential displacements.

Nondimensionalization of variables is accomplished by defining the length and strain-rate scales L , $\dot{\epsilon} = [\frac{1}{2} \dot{\epsilon}_{ij} \epsilon_{ij}]^{1/2}$, and by transforming the variables as follows:

$$(10) \quad \begin{aligned} A_{ij} &\longrightarrow (L) A_{ij} \\ \frac{\partial u_i}{\partial x_j} &\longrightarrow \dot{\epsilon} \frac{\partial u_i}{\partial x_j} \\ \left(\frac{\delta x_\ell}{\Delta t_\ell}, \frac{\delta y_\ell}{\Delta t_\ell} \right) &\longrightarrow \left(\frac{\dot{\epsilon} L}{\Delta t} \right) (\delta x_\ell, \delta y_\ell) \end{aligned}$$

and,

$$z_\ell \longrightarrow \sigma z_\ell$$

where A_{ij} , $\frac{\partial u_i}{\partial x_j}$, δx_ℓ , δy_ℓ , and z_ℓ are now of order one. Equation (5) may be expressed in terms of the non-dimensional uncertainty parameter $\left(\frac{\sigma}{\dot{\epsilon} L \Delta t} \right)$:

$$(11) \quad \underline{A} \underline{s} = \underline{d} - \left(\frac{\sigma}{\dot{\epsilon} L \Delta t} \right) \underline{z}$$

Scaling Equation (9) in this way will simplify the forthcoming discussion on how to best select Δt and L from prior estimates of $\dot{\epsilon}$ and σ .

Singular value decomposition

The estimate \underline{S}_e that minimizes the error $\| \underline{A} \underline{S}_e - \underline{d} \|$ defined by:

$$(12) \quad || \underline{\underline{A}} \underline{\underline{s}}_e - \underline{\underline{d}} || = \left[\sum_{i=1}^{2n} \left(\sum_{\ell=1}^4 A_{i\ell} s_{e\ell} - d_i \right)^2 \right]^{1/2}$$

is obtained by the singular value decomposition method [Lawson and Hanson, 1974; Wunch, 1978]. According to this method, $\underline{\underline{A}}$ is decomposed into the product of three associated matrices that are more readily invertible:

$$(13) \quad \underline{\underline{A}} = \underline{\underline{Q}} \underline{\underline{\Gamma}} \underline{\underline{R}}^T$$

The 4 x 4 matrix $\underline{\underline{\Gamma}}$ is diagonal and is composed of the eigenvalues λ_i , $i=1,4$ associated with the following linear equations

$$(14) \quad \underline{\underline{A}}^T \underline{\underline{A}} \underline{\underline{r}}_\ell = \lambda_\ell^2 \underline{\underline{r}}_\ell \quad \ell = 1,4$$

and

$$(15) \quad \underline{\underline{A}} \underline{\underline{A}}^T \underline{\underline{q}}_\ell = \begin{cases} \lambda_\ell^2 \underline{\underline{q}}_\ell & \ell = 1,4 \\ 0 & \ell = 5, \dots, 2n \end{cases}$$

The normalized eigenvectors of $\underline{\underline{A}} \underline{\underline{A}}^T$ and $\underline{\underline{A}}^T \underline{\underline{A}}$, $\underline{\underline{q}}_\ell$ and $\underline{\underline{r}}_\ell$, respectively, comprise the columns of $\underline{\underline{Q}}$ and the rows of $\underline{\underline{R}}^T$ respectively. The inverse matrices $\underline{\underline{R}}^{T-1}$ and $\underline{\underline{Q}}^{-1}$ are easily obtained by taking the transposes of $\underline{\underline{R}}^T$ and $\underline{\underline{Q}}$. This simplicity results from the orthonormality of the eigenvectors $\underline{\underline{q}}_\ell$ and $\underline{\underline{r}}_\ell$.

The inverse of $\underline{\underline{A}}$ is¹

$$(16) \quad \underline{\underline{A}}^{-1} = \underline{\underline{R}} \underline{\underline{\Gamma}}^{-1} \underline{\underline{Q}}^T$$

¹The complex algebraic manipulations required to perform singular value decomposition of $\underline{\underline{A}}$ are easily done by computer programs now available on most university computer systems [Dongarra and others, 1979].

where $\underline{\Gamma}^{-1}$ is the 4 x 4 diagonal matrix composed of diagonal elements $\Gamma_{ii}^{-1} = 1/\lambda_i$ $i = 1,4$. Equation (9) is solved by applying \underline{A}^{-1} to the observed data \underline{d} :

$$(17) \quad \underline{Se} = \underline{A}^{-1} \underline{d} = \underline{R} \underline{\Gamma}^{-1} \underline{Q}^T \underline{d}$$

The expectation value of the error variance, $\langle (\underline{se}-\underline{s})(\underline{se}-\underline{s})^T \rangle_{ii}$ $i=1,4$, is given by:

$$(18) \quad \langle (\underline{Se}-\underline{S})(\underline{Se}-\underline{S})^T \rangle_{ii} = \langle \underline{R} \underline{\Gamma}^{-1} \underline{Q}^T \underline{z} \underline{z}^T \underline{Q} \underline{\Gamma}^{-1} \underline{R}^T \rangle_{ii}$$

$$= \left[\sum_k R_{ik}^2 / \lambda_{kk}^2 \right] \frac{\sigma^2}{(\dot{\epsilon} L \Delta t)^2}$$

Equation (18) states that the error of each component of \underline{Se} depends on the eigenvalues of $\underline{\Gamma}$ and the eigenvectors \underline{r}_i , $i=1,4$.

Optimal rosette design

The task of designing the best rosette amounts to choosing the adjustable parameters Δt , L and (x_i, y_i) $i=1,n$ that satisfy a design criterion given by Equation (18) restated as follows:

$$(19) \quad \text{Max}_i \left[\sum_k R_{ik}^2 / \lambda_{kk}^2 \right] \frac{\sigma^2}{(\dot{\epsilon} L \Delta t)^2} \leq E$$

where E is the desired accuracy level (expressed in units scaled by $\dot{\epsilon}^2$).

Here, σ^2 is a parameter combining information about the surveying equipment and natural strain-rate variation, and $\dot{\epsilon}$ is estimated or determined from prior data. In practice, it is best to choose L as long as possible (~2 km); therefore only n and Δt will be considered adjustable in the following discussion.

To derive the design criterion, the n-leg rosette will be idealized as a system of n outlying stakes placed regularly on a circle at angular intervals of $\Delta\theta = \frac{2\pi}{n}$. In practice, however, allowances must be made for deviations from this ideal configuration. Assuming that δR and $L\delta\theta$ are the intrinsic radial and tangential uncertainties of observed stake displacements, the non-dimensional and pre-weighted version of the matrix A is given by:

$$(19) \quad A_{ij} = \begin{cases} \frac{\sigma}{(\delta R^2 \cos^2 i\Delta\theta + L^2 \delta\theta^2 \sin^2 i\Delta\theta)^{1/2}} \times \begin{cases} \cos i\Delta\theta & j=1 \text{ and } i \leq n \\ \sin i\Delta\theta & j=2 \text{ and } i \leq n \\ 0 & j=3,4 \text{ and } i \leq n \end{cases} \\ \times \\ \frac{\sigma}{(\delta R^2 \sin^2 i\Delta\theta + L^2 \delta\theta^2 \cos^2 i\Delta\theta)^{1/2}} \times \begin{cases} 0 & j=1,2 \text{ and } i > n \\ \cos i\Delta\theta & j=3 \text{ and } i > n \\ \sin i\Delta\theta & j=4 \text{ and } i > n \end{cases} \end{cases}$$

$$\text{where } \sigma^2 = \frac{1}{2n} \sum_{i=1}^n \left[(\delta R^2 \cos^2 i\Delta\theta + L^2 \delta\theta^2 \sin^2 i\Delta\theta)^{1/2} + (\delta R^2 \sin^2 i\Delta\theta + L^2 \delta\theta^2 \cos^2 i\Delta\theta)^{1/2} \right]$$

The value of $\text{Max}_i \left[\frac{\sum_k R_{ik}^2}{\lambda_k^2} \right] [\sigma^2 / (\epsilon L \Delta t)^2]$ expressed as a percentage of ϵ is presented as a function of n and Δt in Figure (3) for parameter values $L=2$ km and $\epsilon=10^{-10} \text{ sec}^{-1}$. Observe that this error value varies as $(\Delta t \sqrt{n})^{-1}$. As shown in Figure (3), a choice of $n=13$ would yield 3% accuracy within a time span of 4 weeks.

Demonstration

A stake network planted upstream of the Cray Ice Rise on the Ross Ice Shelf demonstrates the data-analysis procedure associated with the n-leg rosette. Figure (4) shows the disposition of four 3-leg rosettes (O,E,F,G)

planted in 1983 as part of a field project by Robert Bindshadler and others (in preparation) to measure strain rates upstream of an ice rise. The position of a much older 3-leg rosette planted in 1973 (G_8) falls within the area enclosed by the four new rosettes. This older rosette was re-surveyed in 1974 as part of the Ross Ice Shelf Geophysical and Glaciological Survey (RIGGS) [Thomas and others, in press]; and yielded an accurate strain-rate determination ($\pm 9\%$) with which data from the new rosettes may be compared.

The four rosettes planted in 1983 were each surveyed and re-surveyed within a time interval spanning 4.2 days. This re-survey was conducted primarily to check survey equipment and to scan for possible zones of ultra-high deformation. Here, however, the re-survey data will be used to test the ability of a 12-leg rosette to yield an accurate measurement in a short time interval. The imaginary 12-leg rosette is synthesized from the four smaller rosettes by treating each of their central stakes as one. Actual strain-rate gradients expected within the field area will degrade the accuracy of the synthesized 12-leg rosettes because the smaller 3-leg rosettes are separated from each other by up to 15 km. Nevertheless, the synthesization provides an otherwise unavailable opportunity to demonstrate a 12-leg rosette measurement.

Figure (5) shows the stake disposition for the synthesized 12-leg rosette, and indicates the observed stake velocities relative to the combined central stakes.

Except for two stakes corresponding to rosette "0", tangential displacements were not measured. Angular displacements required

to observe the tangential components would have been below the accuracy level ($\pm 10''$) of the theodolites used in this project, so any unknown tangential displacements were arbitrarily assumed to be zero.

The results of the 12-leg rosette analysis are compared with the known strain rates in Figure (5) and in Table (1). The two principle strain-rate components (denoted $\dot{\epsilon}_{11}$ and $\dot{\epsilon}_{22}$) given by the 12-leg rosette differ from the known values by 9% and 14%, respectively; and their orientations are displaced by 6° . The unsatisfactory results produced by the 3-leg rosette (E) closest to G_8 , using the data analysis procedure outlined by Thomas and others [in press], are also shown in Figure (5) and Table (1) as a contrast to the results of the 12-leg rosette.

Practical considerations

It would be a mistake to adopt rosettes with large n without first considering the limitations imposed by logistic support, survey technology and the various benefits of a multi-year field program not emphasized in this study. It is, of course, always desirable to plant rosettes with large n . This desire conflicts, however, with the need to minimize the field-party workload, especially when aircraft-support limits the time a field party can spend at each field site. If field programs encompass a large number of field sites, the 3 or 4 leg rosette may be best because field operations would likely span several years regardless of whether individual sites were revisited in the same, or in a succeeding, field season. For field programs covering a small area, such as that conducted upstream of the Crary Ice Rise, rosettes with greater numbers of stakes may be preferable so that the entire field project can be accomplished in several weeks.

In view of the need to reduce the field-party workload, several design modifications to the n-leg rosette may prove useful. Figure (1) shows, for example, how planting k central stakes and m outlying stakes will achieve a high data redundancy while reducing the total number of stakes and the surveying workload. Surveying the m outlying stakes from k independent central locations yields $k \cdot m = n$ independent measurements of stake displacement. Only $m+k < n$ stakes are required by this non-ideal n-leg rosette; and if appropriate measures are taken (e.g., assigning a separate corner-reflector prism to each outlying stake) the surveying can be accomplished within a reasonable time frame.

Conclusion

The stake-network designs that best measure surface strain rates and vorticity encompass a large degree of redundancy. The more stakes included in each network, the less individual stake displacement errors will effect the overall measurement. The advantage gained at the expense of the redundancy is the reduced time interval between survey and re-survey.

Redundancy may also be a key element in the design of future field programs based on remote sensing techniques such as airborne photogrammetry. Brecher [1982], for example, has used photographs of natural features to identify the motion of the Byrd Glacier. These natural features are available in virtually unlimited number; so provide, in principle, a natural equivalent of a large- n rosette. A shortened time interval between initial and final aerial photography missions may reduce the camouflaging of natural features by drifting snow; and may, therefore, ultimately achieve greater accuracy by virtue of higher data redundancy.

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FIGURE CAPTIONS

Figure 1. The idealized n-leg rosette considered in this study for the purpose of error analysis consists of n outlying stakes surrounding a central stake (0) at equal angular intervals and with equal radial separation. Typical rosettes used on the Ross Ice Shelf had n=3 [Thomas and others, in press]. Also shown above is a modified n-leg rosette that uses $m+k < n$ stakes. This rosette design achieves n-legs by having $k = \frac{n}{m}$ central stakes, and may be more practical under actual field conditions.

Figure 2. Uniaxial strain rate measured on the Ross Ice Shelf at station C-16 (81°11'38"S, 189°30'09"W) is plotted as a function of distance along a line bearing 312°T. The relative ice-shelf surface elevation along the line is also plotted. Open circles denote stake positions. The error level of the observed strain rate is approximately 1.0×10^{-4} /yr.

Figure 3. The maximum error of the velocity gradients, expressed as percent of $\dot{\epsilon}$, is plotted as a function of n and Δt . Scale factors used in the calculation of these graphs are $\dot{\epsilon} = 10^{-10}$ /sec and $L = 2.0 \times 10^3$ m. Notice that 3% accuracy can be achieved by a 13-leg rosette in 28 days or by a 3-leg rosette in 84 days. Under some circumstances, it may be preferable to deploy rosettes having $n \geq 15$ so that all measurements can be accomplished within one field season. This diagram, and other similar ones, can be used by field glaciologists to design their field program to maximize the data return for a given logistic effort.

Figure 4. Four 3-leg rosettes (O, E, F, G) planted upstream of Crary Ice Rise on the Ross Ice Shelf were re-surveyed after an average time period of 4.3 days. A fifth rosette (G_g) planted during RIGGS provides an accurate measurement of the actual strain rate to be used as a reference. In this study, the four 3-leg rosettes are combined into one 12-leg rosette to see how the subsequent strain rate measurement provided by the 4.2 day period compares with the actual strain rate.

Figure 5. Stake positions of the 12-leg rosette are mapped along with the relative stake velocities. The principal axes of the strain rate measured using the 12-leg rosette are compared with those measured during RIGGS (G_g) [Thomas and others, in press]. Also shown are the principle axes resulting from the 3-leg rosette E using a 3.9 day interval between survey and re-survey. Uncertainty of the observed velocities is variable. The shaded ellipse represents, for example, the velocity uncertainty for the three most outlying stakes. The ellipse should be oriented perpendicular to the position vector.

Table 1. Principal strain-rates ($\dot{\epsilon}_{11}$ and $\dot{\epsilon}_{22}$) measured by various rosettes

<u>Year of Survey</u>	<u>Rosette</u>	<u>Δt</u>	<u>$\dot{\epsilon}_{11}$</u>	<u>$\dot{\epsilon}_{22}$</u>	<u>Bearing of $\dot{\epsilon}_{11}$</u>	<u>$\dot{\omega}_z$</u>
1973	G ₈	1 year	$1.72 \pm 0.15 \times 10^{-10} \text{ sec}^{-1}$	$-1.57 \pm 0.03 \times 10^{-10} \text{ sec}^{-1}$	50°	-
1984	12-leg	4.2 days	1.87	-1.79	44°	$1.77 \times 10^{-10} \text{ sec}^{-1}$
1984	E	3.9 days	2.67	-2.74	42°	-

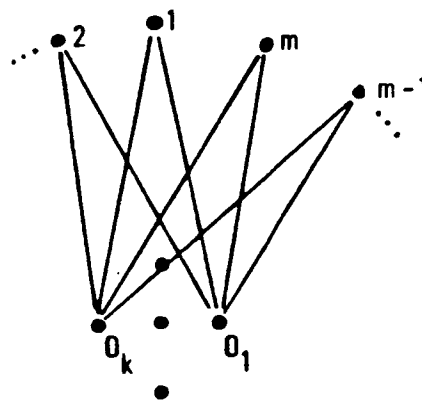
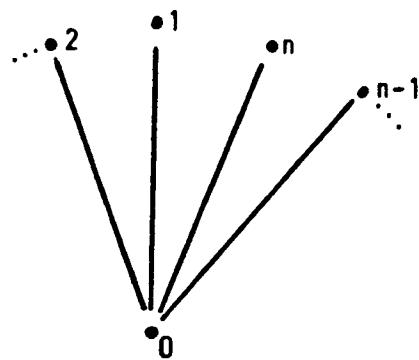


Figure 1

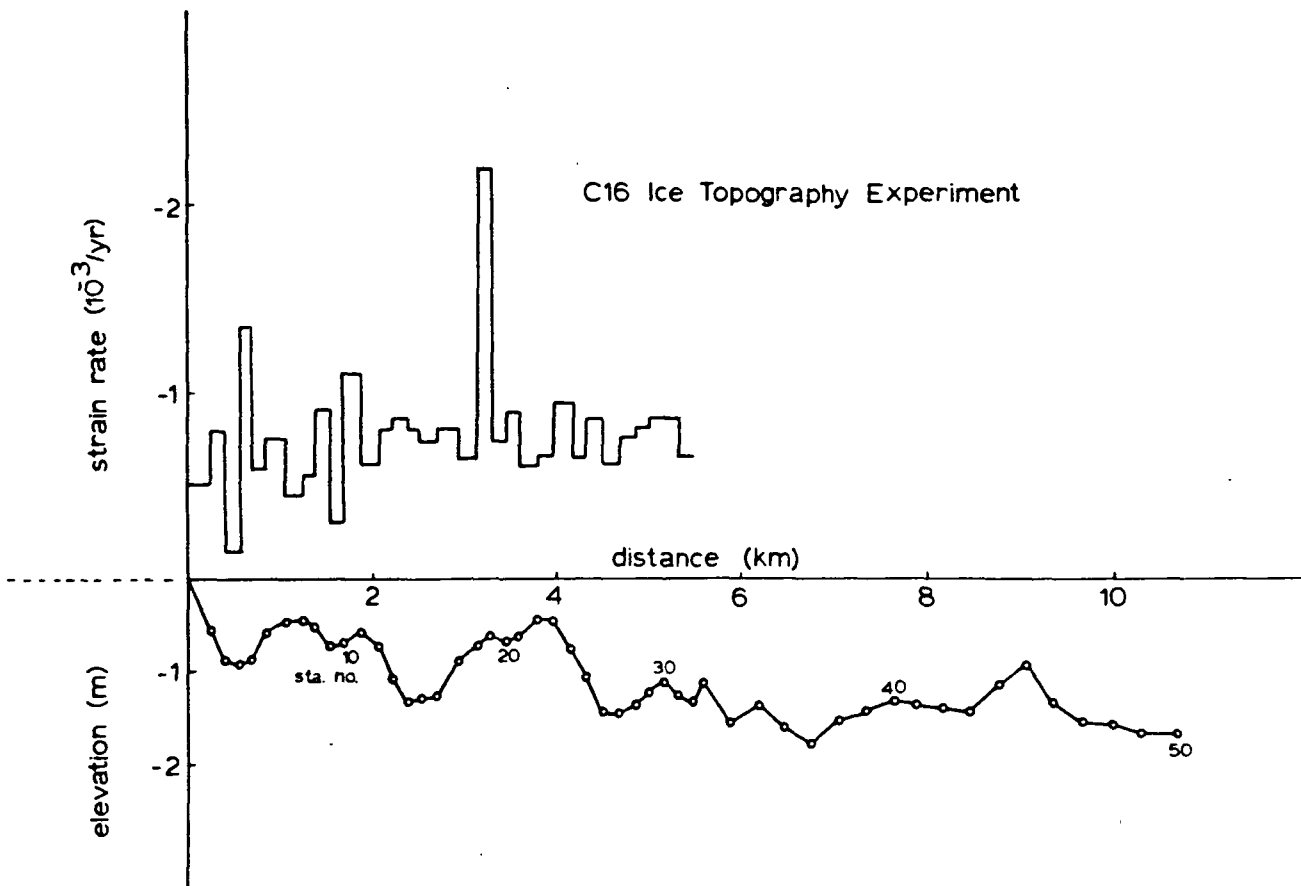


Figure 2

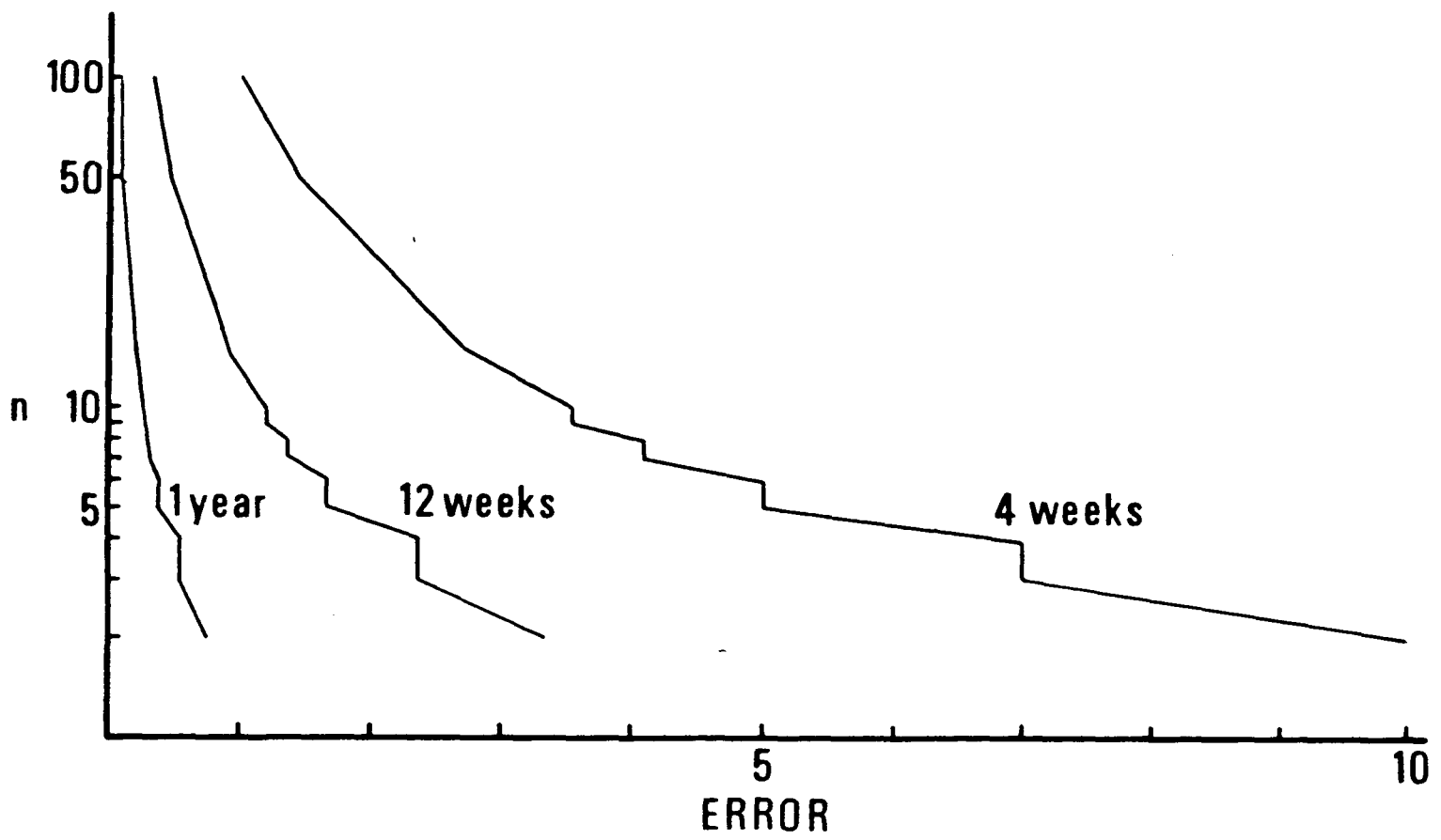


Figure 3

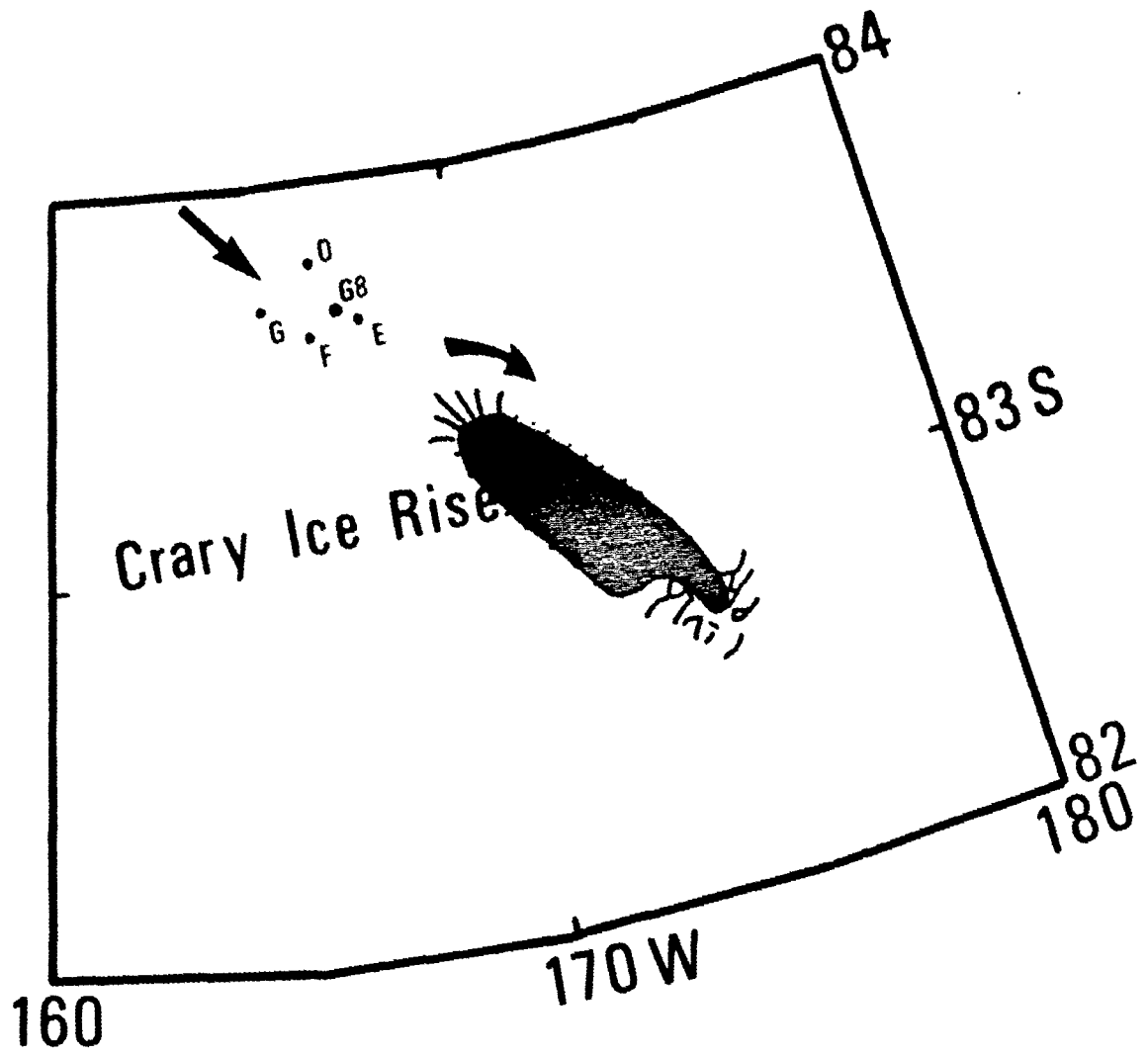


Figure 4

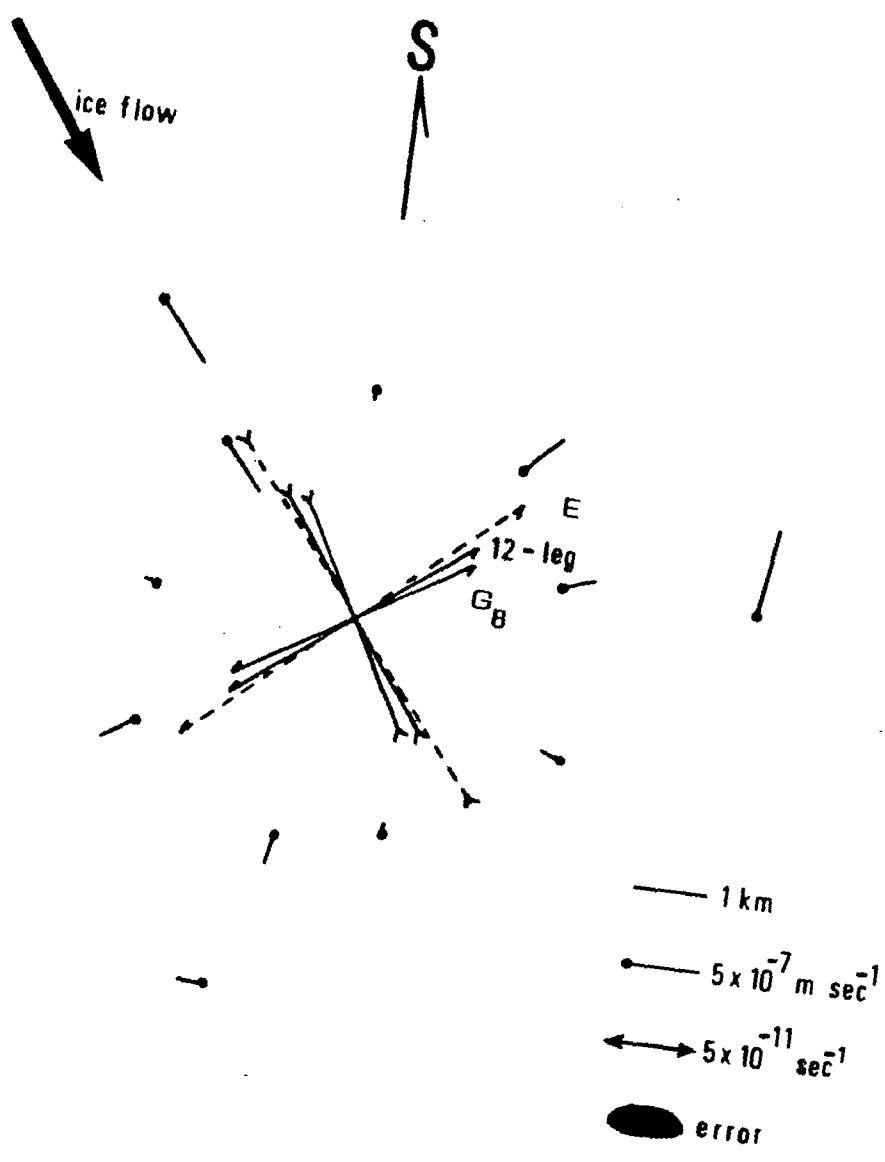


Figure 5