



# Influence of Mass Moment of Inertia on Normal Modes of Preloaded Solar Array Mast

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(NASA-TP-3273) INFLUENCE OF MASS MOMENT OF INERTIA ON NORMAL MODES OF PRELOADED SOLAR ARRAY MAST (NASA) 12 p

N92-33476

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NASA Technical Paper 3273

1992

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#### Summary

Earth-orbiting spacecraft often contain solar arrays or antennas supported by a preloaded mast. Because of weight and cost considerations, the structures supporting the spacecraft appendages are extremely light and flexible; therefore, it is vital to investigate the influence of all physical and structural parameters that may influence the dynamic behavior of the overall structure. The study reported in this paper primarily focuses on the mast for the space station solar arrays, but the formulations and the techniques developed in this study apply to any large and flexible mast in zero gravity. Furthermore, to determine the influence on the circular frequencies, the mass moment of inertia of the mast has been incorporated into the governing equation of motion for bending. A finite element technique (MSC/NASTRAN) has been used to verify the formulation derived in this paper. Results indicate that when the mast is relatively flexible and long, the mast mass moment of inertia influences the circular frequencies.

#### Introduction

With the evolution of man-tended or manned spacecraft such as Space Station Freedom comes the need for generating more electrical power than for an ordinary satellite. More electrical power simply translates to having a greater number of solar arrays with larger surface areas. In order to have a large surface area solar array and yet be able to package the folded solar array in a launch vehicle, the solar arrays should be designed with a relatively large aspect ratio. Because solar arrays for manned spacecraft require a large aspect ratio, their supporting structures (preloaded masts) will be as tall as and much more flexible than the mast for an ordinary satellite. Therefore, the assumptions made to formulate the governing equation of motion for a compact solar array will not be applicable for a solar array with a large aspect ratio. Although extensive studies have been performed on preloaded mast vibrations (refs. 1 and 2), the influence of the mast mass moment of inertia has not been considered.

Our studies focused on developing the governing equation of motion for a preloaded beam, representing a typical mast, with a tip mass, representing the storage boxes of the solar cells (fig. 1.) The equation of motion for the beam was used to derive the eigen condition (characteristic equation) whose roots are the circular frequencies of the structure in question.



Figure 1.--Space station split blanket solar array.

The accuracy of these circular frequencies is the key in determining the forces and moments resulting from an outside excitation, such as space shuttle docking loads. The loads determined from the circular frequencies with high modal mass participation are often used to design the structure.

#### Symbols

- C constant
- EA axial stiffness
- EI<sub>cs</sub> flexural stiffness

- F force
- I moment of inertia of mast cross section
- K preload parameter
- $K_2$  tip mass preload parameter
- L length of mast
- M moment
- $M_T$  tip mass
- m mass of mast
- m' distributed mass
- P preload
- PD component of preload in lateral direction
- Q shear load
- T function of time
- t time
- V lateral deformation as function of beam length and time
- v lateral deformation as function of beam length
- x distance
- $\beta$  frequency parameter
- $\delta$  axial deflection
- $\Theta$  slope of deflection
- $\lambda$  characteristic value
- $\rho$  mass density of the mast
- $\omega$  circular frequency
- a partial derivative

#### Subscript:

n nth root

Superscripts:

- ' differentiation with respect to length of mast
- . differentiation with respect to time

## Theory

The solar arrays masts are essentially bars that are subject to bending vibrations. The source of the vibrations may be the interaction of the propulsion system, and/or the shuttle docking. Since a bar is an elastic body, its mass and stiffness are distributed continuously along its length. For an elastic body the number of the generalized coordinates needed to describe the motion of the bar becomes infinite, and, as a result, the number of modes and mode shapes become infinite.

To formulate the equation of motion for and analyze a bar under a bending vibration, the following assumptions were made: (1) the material is homogeneous and isotropic, and it follows Hooke's law; (2) the continuum representing the mast is conservative, and the structural damping and viscous damping (frictional forces) are nearly zero; (3) the magnitude of displacement is small when compared to the mast length, so the sine and tangent of the angle are equal to the angle of deflection; (4) the mast has a constant cross-sectional area and constant bending stiffness along its length; and (5) the supporting spacecraft is rigid and massive when compared to the solar array mast, and the mast is fixed at its base. It is important to note that the boundary conditions for the mast are derived based on the last assumption.

The mechanism for preloading the mast (fig. 1) can be described as the tensioning of the solar array substrates by some force at their edges; a compressive force of equal magnitude will act on the mast. The substrates are tensioned to achieve flat surfaces and to increase their natural frequency to some nonzero value.

In this study the assumption has been made that the masses of the solar cell storage boxes are concentrated at only one point, the tip of the mast. Such an assumption is appropriate when analyzing the mast under a bending vibration since the storage boxes will experience the same direction and magnitude of motion during any bending mode. In contrast, one tip mass is not a valid assumption when analyzing the mast under a torsional vibration, and an appropriate mass moment inertia of the solar cell storage boxes should be considered. In summary, the continuum representing the mast is a preloaded mast that is fixed at one end and free at the other, where the tip mass is placed.



Figure 2.--Preloaded beam with a tip mass.

### **Governing Equation of Motion**

Figure 2 shows a mast of length L, whose cross-sectional area is A and bending stiffness is  $EI_{cs}$ . The preload has a magnitude **P** in the direction shown, and the tip mass has a magnitude  $M_T$ . One very important point to be made here is that because of the mechanism for preloading the mast, the direction of the load always remains towards the root of the mast, regardless of the deflection and the mast mode shapes. This occurs because the solar cell substrates are essentially membranes and they react only to tensile loading. The magnitude of the preload remains constant, regardless of the magnitude of deflection, since the solar cell substrates contain negator

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Figure 3.—Free-body diagram of a differential element of the preloaded beam.

spring mechanisms at their edges that keep the magnitude of the preload constant.

Figure 3 shows the free-body diagram of a differential element of the preloaded mast with all the forces and moments acting on its body. The sum of the loads can be expressed as follows:

$$\sum \mathbf{F}_{\mathbf{x}} = 0$$
, which results in  $\mathbf{P} = \mathbf{P}$  (1)

$$\Sigma \mathbf{F}_{\mathbf{v}} = 0$$
, which results in  $\frac{\partial \mathbf{Q}}{\partial \mathbf{x}} = -\rho A \frac{\partial^2 \mathbf{V}}{\partial t^2}$  (2)

$$\sum \mathbf{M}_{o} = 0$$
, which results in  $\mathbf{Q} = \frac{\partial \mathbf{M}}{\partial \mathbf{x}} + \mathbf{P} \frac{\partial \mathbf{V}}{\partial \mathbf{x}} - \rho I \ddot{\Theta}$  (3)

Using the elementary theory of strength of material gives

$$\mathbf{M} = EI_{cs} \frac{\partial^2 \mathbf{V}}{\partial \mathbf{x}^2} \tag{4}$$

and

$$\Theta = \frac{\partial \mathbf{V}}{\partial \mathbf{x}} \tag{5}$$

By differentiating equation (4) with respect to x once, differentiating equation (5) with respect to t twice, and then substituting the results into equation (3), we obtain

$$\mathbf{Q} = EI_{cs} \frac{\partial^3 \mathbf{V}}{\partial \mathbf{x}^3} + \mathbf{P} \frac{\partial \mathbf{V}}{\partial \mathbf{x}} - \rho I \frac{\partial}{\partial \mathbf{x}} \left( \frac{\partial^2 \mathbf{V}}{\partial t^2} \right)$$
(6)

Finally, differentiating equation (6) with respect to x and substituting it into equation (2) gives the equation of motion for a preloaded mast with a tip mass:

$$EI_{cs}\frac{\partial^{4}\mathbf{V}}{\partial\mathbf{x}^{4}} + \mathbf{P}\frac{\partial^{2}\mathbf{V}}{\partial\mathbf{x}^{2}} - \rho I\frac{\partial}{\partial t^{2}}\left(\frac{\partial^{2}\mathbf{V}}{\partial\mathbf{x}^{2}}\right) = -\rho A\frac{\partial^{2}\mathbf{V}}{\partial t^{2}} \qquad (7)$$

Since the displacement V is a function of length x and time t, the method of separation of variables is used:

$$\mathbf{V} = f(\mathbf{x}, t) = \mathbf{v}(\mathbf{x})T(t) \tag{8}$$

For a harmonic excitation

$$T(t) = \sin \omega t \tag{9}$$

where  $\omega$  is the circular frequency of motion.

Substituting equation (9) into equation (8) and then differentiating with respect to x and t and substituting the results into equation (7) gives the following:

$$\mathbf{v}'''' + \left(\frac{\mathbf{P} + \rho I \omega^2}{E I_{cs}}\right) \mathbf{v}'' - \left(\frac{\rho A \omega^2}{E I_{cs}}\right) \mathbf{v} = 0$$
(10)

Equation (10) is similar to equation (7), but it represents the motion at any given time, since v is a function only of x.

#### **Discussion of Terms in Equation (10)**

When the preload and the mass moment of inertia are zero, equation (10) reduces to the equation of motion for a simple beam. In equation (10) the first term, v-9, represents the bending motion of a differential element of length dx; the

third term, 
$$\left(\frac{\rho A \omega^2}{E I_{cs}}\right) \mathbf{v}$$
, represents the linear inertia of a differ-

ential element of length dx that opposes the motion; and the

second term, 
$$\left(\frac{\mathbf{P} + \rho I \omega^2}{E I_{cs}}\right) \mathbf{v}''$$
, is the contribution of the preload

and the mass moment of inertia. The physical meaning of the second term can be described as follows: (1) When the mast is deformed laterally, the preload **P** tends to deform (bend) it even further, and the magnitude of this added displacement is proportional to the preload times the displacement lever arm; and (2) when the mast is deformed laterally, the angular acceleration tends to assist the bending of the mast in the same direction as the preload. It is interesting to note that the magnitude of this bending effect is influenced by the displacement **v** and by the circular frequency at which the system is vibrating. For high circular frequencies, this effect dominates the influence of preload.

## **General Solution**

For simplicity, let us define

$$K^{2} = \frac{\mathbf{P} + \rho I \omega^{2}}{E I_{cs}} \tag{11}$$

and

$$\beta^4 = \frac{\rho A \omega^2}{E I_{cs}} \tag{12}$$

where  $\beta$  is the frequency parameter (ref. 2).

Substituting equations (11) and (12) into equation (10) results in

$$\mathbf{v}^{\prime\prime\prime\prime} + K^2 \mathbf{v}^{\prime\prime} - \beta^4 \mathbf{v} = 0 \tag{13}$$

Equation (13) is a fourth order ordinary differential equation with the general solution

$$\mathbf{v} = C_1 \cosh \lambda_1 \mathbf{x} + C_2 \sinh \lambda_1 \mathbf{x} + C_3 \cos \lambda_2 \mathbf{x} + C_4 \sin \lambda_2 \mathbf{x}$$
(14)

where

$$\lambda_{1} = \sqrt{\frac{-K^{2} + \sqrt{K^{4} + 4\beta^{4}}}{2}}$$
(15)

and

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$$\lambda_2 = \sqrt{\frac{K^2 + \sqrt{K^4 + 4\beta^4}}{2}}$$
(16)

The constants  $C_1$  to  $C_4$  can be determined from the boundary conditions and the initial condition.

At the fixed end of the mast, the displacement and the slope of the displacement are zero; thus the first two boundary conditions are

$$\mathbf{v}(0) = 0 \tag{17}$$

$$\left. \frac{d\mathbf{v}}{d\mathbf{x}} \right|_{\mathbf{x}=0} = 0 \tag{18}$$

Because the boundary condition at the free end, where the tip mass is placed, is more complex, a free-body diagram of



Note: For a small lateral deflection  $\theta_1 = \theta = \tan(\theta) = \sin(\theta)$ 

Figure 4.-Deflected shape of the preloaded beam centerline.

the free end is required. Figure 4 shows the mast as loaded; in this figure,  $P_D$  is a component of preload whose magnitude can be determined as follows:

$$\tan \Theta \cong \frac{\mathbf{V}(L)}{L-\delta} \tag{19}$$

and

$$\mathbf{P}_{\mathbf{D}} = \mathbf{P} \sin \Theta \tag{20}$$

Note that when the mast is deflected, its length is shortened by some magnitude  $\delta$ .

For a small deflection,  $\Theta \cong \tan \Theta \equiv \sin \Theta$ ; therefore

$$\mathbf{P}_{\mathbf{D}} = \mathbf{P} \sin \Theta \cong \mathbf{P}\Theta = \mathbf{P} \frac{\mathbf{V}(L)}{L - \delta}$$
 (21)



Figure 5.—Free-body diagram at free end of the preloaded beam.

where  $\delta$  is the axial deflection whose magnitude is influenced by the axial preload. Its magnitude (ref. 3) is

$$\delta = \frac{\mathbf{P}L}{EA} \tag{22}$$

Substituting equation (22) into (21) gives

$$\mathbf{P}_{\mathbf{D}} = \mathbf{P} \frac{\mathbf{V}(L)}{L - \frac{\mathbf{P}L}{EA}}$$
(23)

Figure 5 depicts the free-body diagram at the mast's free end. To eliminate the time function, the terms from equations (8) and (9) should be substituted for the terms in this free-body diagram. Summing the forces and moments at the mast free end results in

$$\mathbf{Q}(L) + \frac{\mathbf{P}\mathbf{v}(L)}{L - \frac{\mathbf{P}L}{EA}} - M_T \omega^2 \mathbf{v}(L) = 0$$
(24)

and

$$M(L) = EI_{cs} \left. \frac{d^2 \mathbf{v}}{d \mathbf{x}^2} \right|_{\mathbf{x}=L} = 0$$
 (25)

Note that since the direction of the preload always remains towards the mast fixed end, the moment caused by the preload about the mast fixed end is zero.

If the mast is tall and flexible so that  $EA \gg PL$ , then equation (24) can be simplified to

$$Q(L) + \frac{\mathbf{P}\mathbf{v}(L)}{L} - M_T \omega^2 \mathbf{v}(L) = 0$$
 (26)

Therefore, equations (25) and (26) are the boundary conditions to be used at the mast free end. Using the first two boundary conditions (eqs. (17) and (18)) results in

$$C_3 = -C_1$$
 (27)

and

$$C_4 = -\frac{\lambda_1}{\lambda_2} C_2 \tag{28}$$

Employing the third condition (eq. (26)) and equation (6) gives

$$C_{I}\left[\left(\lambda_{1}^{3}+\lambda_{1}\lambda_{2}^{2}\right)\sinh\lambda_{1}L+\left(\lambda_{2}+\lambda_{1}^{2}-\lambda_{2}^{3}\right)\sin\lambda_{2}L\right.$$
$$+K_{2}\left(\cosh\lambda_{1}L-\cos\lambda_{2}L\right)$$
$$+C_{2}\left[\left(\lambda_{1}^{3}+\lambda_{1}\lambda_{2}^{2}\right)\cosh\lambda_{1}L\right.$$
$$+\left(-\lambda_{2}\lambda_{1}^{2}+\lambda_{2}^{3}\right)\frac{\lambda_{1}}{\lambda_{2}}\cos\lambda_{2}L$$
$$+K_{2}\left(\sinh\lambda_{1}L-\frac{\lambda_{1}}{\lambda_{2}}\sin\lambda_{2}L\right)\right]=0$$
(29)

where

$$K_2 = \frac{M_T \omega^2 - \frac{\mathbf{P}}{L}}{E I_{cs}}$$
(30)

Note that the following relations, which can easily be checked, simplify equation (29):  $\lambda_1 \lambda_2 = \beta^2$ ;  $\lambda_1^2 + K^2 = \lambda_2^2$ ; and  $\lambda_2^2 - K^2 = \lambda_1^2$ . Thus equation (29) becomes

$$C_{1}\left[\lambda_{1}\lambda_{2}^{2}\sinh\lambda_{1}L - \lambda_{1}^{2}\lambda_{2}\sin\lambda_{2}L + K_{2}\left(\cosh\lambda_{1}L - \cos\lambda_{2}L\right)\right] + C_{2}\left[\lambda_{1}\lambda_{2}^{2}\cosh\lambda_{1}L + \lambda_{1}^{3}\cos\lambda_{2}L\right] + K_{2}\left(\sinh\lambda_{1}L - \frac{\lambda_{1}}{\lambda_{2}}\sinh\lambda_{2}L\right) = 0$$
(31)

Employing the fourth condition (eq. (25)) results in

$$C_{1}\left(\lambda_{1}^{2} \cosh \lambda_{1}L + \lambda_{2}^{2} \cos \lambda_{2}L\right)$$
$$+ C_{2}\left(\lambda_{1}^{2} \sinh \lambda_{1}L + \lambda_{1}\lambda_{2} \sin \lambda_{2}L\right) = 0 \qquad (32)$$

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[rcf. 4]	
Length, in	]
Width, in	l
Beam <i>El</i> <sub>10</sub> , psi2.85x10 <sup>4</sup>	l
Bcam, psi6.9x10 <sup>6</sup>	
Beam weight, lb	
Blanket weight, lb766	
Tip weight, lb	I
Tip inertia, lb-in. <sup>2</sup>	l

Table I. - Parameters Which Define the

Idealized Solar Array Structure

Equations (31) and (32) represent an eigenvalue problem. The determinant of  $C_1$  and  $C_2$  must be zero, and this condition establishes the characteristics equation (having an infinite number of roots) that determines the characteristic number or frequency value. Setting the determinant of equations (31) and (32) to zero gives

$$2\beta^{6} - K^{2}\beta^{4} \sinh \lambda_{1}L \sin \lambda_{2}L$$
$$+\beta^{2} \left(K^{4} + 2\beta^{4}\right) \cosh \lambda_{1}L \cos \lambda_{2}L - K_{2} \left(\lambda_{1}^{2} + \lambda_{2}^{2}\right)$$
$$\times \left(\lambda_{1} \cosh \lambda_{1}L \sin \lambda_{2}L - \lambda_{2} \sinh \lambda_{1}L \cos \lambda_{2}L\right) = 0$$
(33)

Equation (33) is similar to the eigen condition derived in reference 2, with some minor differences in definition of parameters. Note that all parameters in equation (33) are functions of  $\omega$ , and employing the constants from table I will result in a highly nonlinear equation with an infinite number of roots.

#### **Mode Shapes**

To determine the mode shapes, the constants  $C_1$  to  $C_4$ should be evaluated and substituted into equation (14). The boundary conditions of the mast can provide the necessary information to evaluate any set of three constants, but the last constant can be evaluated only with the initial condition. Since we are interested in the mode shapes at any instant of time and since the last constant can have any value, depending on the initial condition, an arbitrary value of 1.0 for the last constant is assumed in this study.

Substituting equations (27) and (28) into equation (14) results in

$$\mathbf{V} = C_1 \left(\cosh \lambda_1 \mathbf{x} - \cos \lambda_2 \mathbf{x}\right) + C_2 \left(\sinh \lambda_1 \mathbf{x} - \frac{\lambda_1}{\lambda_2} \sin \lambda_2 \mathbf{x}\right)$$
(34)

Solving equation (29) for  $C_2$  and substituting that into equation (34) yields

$$= C_1 \left\{ \left( \cosh \lambda_1 \mathbf{x} - \cos \lambda_2 \mathbf{x} \right) \right.$$

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$$-\left[\frac{\lambda_1\lambda_2^2\sinh\lambda_1L-\lambda_1^2\lambda_2\sin\lambda_2L+K_2(\cosh\lambda_1L-\cos\lambda_2L)}{\lambda_1\lambda_2^2\cosh\lambda_1L+\lambda_1^3\cos\lambda_2L+K_2\left(\sinh\lambda_1L-\frac{\lambda_1}{\lambda_2}\sin\lambda_2L\right)}\right]$$

$$\times \left( \sinh \lambda_1 \mathbf{x} - \frac{\lambda_1}{\lambda_2} \sin \lambda_2 \mathbf{x} \right) \right\}$$
 (35)

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Equation (35) is the equation of motion for individual circular frequencies. When a circular frequency obtained from equation (33) is substituted into equation (35), the mode shape for that particular frequency can be determined. Note that, as stated previously, the value for  $C_1$  is assumed to be 1.0 prior to mode shapes determination.

When the mast is excited, it will vibrate through all frequencies; therefore its motion, which is the sum of all mode shapes, can be expressed as

$$\mathbf{v} = \sum_{n=1}^{\infty} C_n \left\{ \left( \cosh \lambda_{1n} \mathbf{x} - \cos \lambda_{2n} \mathbf{x} \right) - \left[ \frac{\lambda_{1n} \lambda_{2n}^2 \sinh \lambda_{1n} L - \lambda_{1n}^2 \lambda_{2n} \sin \lambda_{2n} L + K_2 \left( \cosh \lambda_{1n} L - \cos \lambda_{2n} L \right)}{\lambda_{1n} \lambda_{2n}^2 \cosh \lambda_{1n} L + \lambda_{1n}^3 \cos \lambda_{2n} L + K_2 \left( \sinh \lambda_{1n} L - \frac{\lambda_{1n}}{\lambda_{2n}} \sin \lambda_{2n} L \right) \right] \right\}$$

$$\times \left( \sinh \lambda_{1n} \mathbf{x} - \frac{\lambda_{1n}}{\lambda_{2n}} \sin \lambda_{2n} \mathbf{x} \right) \right\}$$
(36)

where the subscript *n* refers to the *n*th circular frequency.

Again, it should be emphasized that equation (36) is the mode shape at any instant of time; to obtain the mode shape as function of the mast length and time, use equations (8) and (9), which yield

$$\mathbf{V} = \sum_{n=1}^{\infty} C_n \left\{ \left( \cosh \lambda_{1n} \mathbf{x} - \cos \lambda_{2n} \mathbf{x} \right) \right\}$$

$$\left[\frac{\lambda_{1n}\lambda_{2n}^2\sinh\lambda_{1n}L - \lambda_{1n}^2\lambda_{2n}\sin\lambda_{2n}L + K_2(\cosh\lambda_{1n}L - \cos\lambda_{2n}L)}{\lambda_{1n}\lambda_{2n}^2\cosh\lambda_{1n}L + \lambda_{1n}^3\cos\lambda_{2n}L + K_2\left(\sinh\lambda_{1n}L - \frac{\lambda_{1n}}{\lambda_{2n}}\sin\lambda_{2n}L\right)}\right]$$

$$\times \left( \sinh \lambda_{1n} \mathbf{x} - \frac{\lambda_{1n}}{\lambda_{2n}} \sin \lambda_{3n} \mathbf{x} \right) \left\{ \sin \omega_n t \qquad (37) \right\}$$

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where  $C_n$  is obtained by using the initial condition.

#### **Finite Element Approach**

Since the normal mode solution of MSC/NASTRAN (Version 65c) does not have the capability of predicting the normal modes of a preloaded structure in one computer run, the general solution was divided into two separate runs. First the stiffness matrix of the preloaded mast was generated by using the geometric nonlinearity of solution 64 of MSC/ NASTRAN, and then the superelement normal mode, solution 63, was used to determine the modes.

# Static Nonlinear and Normal Mode Solutions

The finite element model of the mast, shown in figure 6, is approximately a 100 degree-of-freedom model, which contains BEAM elements. BEAM elements are two-noded elements with the capability of deforming axially, laterally, and torsionally. For this study the torsional deformation capability of the finite element model was eliminated by constraining all the nodes from rotating about the x-direction. Also the model was forced to bend in only one direction by



Figure 6.—A representation of the finite element model of the preloaded beam. Note: The mast and single beam element are physically at the same location, but in this figure they have been shown separately to emphasize the physics of loading. Node 50 is a shared node with a hinge.

constraining all the nodes in the z-direction. Node 1, which represents the fixed end of the mast, is constrained in all directions.

To generate the preload on the mast, and yet force the direction of the preload to remain towards the fixed end of the mast, a single BEAM element with a very large stiffness was generated, running the entire length of mast. The single BEAM element shares node 50 with the mast. The large stiffness of the single BEAM element ensures that no mode coupling will occur between the mast and the single BEAM element. The node at the other end of the single BEAM element is node 100, which has been located 0.094 in. above node 1, in the positive x-direction. The reason for this offset is that when a preload (see table I) is applied to the mast, because of the axial flexibility of the mast, the entire assembly will deform by 0.094 in. towards the negative x-direction. To enforce the motion of node 100 in the direction of the preload, a constraint in the v-direction was applied to this node. In addition, to eliminate the bending action between the single BEAM element and the mast, a hinge was placed at node 50.

Finally, the database containing the preloaded stiffness matrix of the mast was used in solution 63 to obtain the circular frequencies. The Generalized Dynamic Reduction technique was used to extract the circular frequencies.

#### Results

A Fortran program was developed to solve equation (33) for circular frequencies by using the Newton method. The circular frequencies from the Fortran program and the finite element model of the mast are shown in tables II to IV. In these tables the values for the mass moment of inertia and the preload vary from zero to their maximum values. The circular frequencies from equation (33) and from the finite element model of the mast are nearly identical at each mode; thus the Fortran program was verified for accuracy.

Table II. – Comparison of Frequencies Obtained From MSC/NASTRAN and Equation (33) With I = 0 in.<sup>4</sup> and  $\mathbf{P} = 0$ 

 $[M_1 = 67 \text{ lb}]$ 

Modes	Frequency, Hz		
	MSC/NASTRAN	Equation (33)	
First	1.27	1.27	
Second	9.07	9.07	
Third	26.86	26.87	

Table III. - Comparison of Frequencies Obtained From MSC/NASTRAN and Equation (33) With I = 0 in.<sup>4</sup> and P = 150 lb  $[M_{\chi} = 67$  lb]

Modes	Frequency, Hz		
	MSC/NASTRAN	Equation (33)	
First	1.25	1.25	
Second	8.88	8.90	
Third	26.44	26.71	

Table IV. – Comparison of Frequencies Obtained From MSC/NASTRAN and Equation (33) With  $I = 38 \text{ in.}^4$  and  $\mathbf{P} = 150 \text{ lb}$  $[M_{\pi} = 67 \text{ lb}]$ 

Modes	Frequenc	y, Hz
	MSC/NASTRAN	Equation (33)
First	1.25	1.25
Second	8.88	8.90
Third	26.44	26.67



Figure 7.—Frequency differences from table V as a function of mode number.

#### Table V. -- Comparison of Frequencies Obtained From Equation (33) for Two Cases: I = 0, and I = 38 in.<sup>4</sup> [P = 150 lb and $M_{\odot} = 67$ lb]

Mode	Frequency, Hz			Frequency, H	
number	<i>I</i> = 38 in. <sup>4</sup>	$I = 0 \text{ in.}^4$	Difference		
1	1.2455	1.2455	0		
2	8.8963	8.9005	.0042		
3	26.6714	26.7064	.0350		
4	54.1339	54.2774	.1435		
5	91.3582	91.7687	.4105		
6	138.2606	139.2063	.9457		
7	194.7165	196.603	1.8865		
8	160.5686	263.9654	3.3968		
9	335.6321	341.2965	5.6644		
10	419.6985	428.5986	8.9001		

To determine the influence of the mast mass moment of inertia on circular frequencies, the first 10 circular frequencies were extracted and tabulated in table V for 2 mass moments of inertia input to the Fortran program, namely, I = 0 in.<sup>4</sup> and I = 38 in.<sup>4</sup> The differences between these circular frequencies are shown in table V and plotted in figure 7; note that as the mode number increases, the influence of the mast mass moment of inertia becomes more apparent. The relationship between the mode number and the frequency difference in figure 7 appears to be that of a second order polynomial. MSC/NASTRAN was not included in the study performed for table V, since for higher frequencies the mast finite element model demands higher fidelity. This model will need to be further refined.

To incorporate the influence of the mass moment of inertia into the finite element model of the mast, the following methods were investigated: (1) the mass moment of inertia of each BEAM element of the mast was calculated and applied to the BEAM property card as the nonstructural mass moment of inertia; and (2) the mass moments of inertia of the BEAM elements (calculated in step 1) were applied to concentrated mass elements (CONM2), and these elements were placed on nodes 1 to 50. As expected, the circular frequencies from the finite element model of the mast were not affected at all, because the influence of the mass moment of inertia has not been included in the Euler beam formulation.

For a cantilevered beam such as the structure of the mast, where most of modal energy is stored in its first few modes, it is crucial to predict the first few circular frequencies accurately. The reason for this is that the dynamic loads are generally proportional to the inverse of the circular frequencies. As can be observed in tables II and IV, the frequencies obtained from MSC/NASTRAN are accurate. However as the demand for higher circular frequencies grows, MSC/NASTRAN becomes less accurate.

# Conclusions

From the work reported herein, the following conclusions may be drawn:

1. Equation (33) is the eigen condition for the characteristic equation of the preloaded mast with a tip mass whose infinite number of roots are the circular frequencies of the structure.

2. According to our formulations and analyses, MSC/ NASTRAN is accurate for predicting the bending circular frequencies of masts as long as the influence of the mass moment of inertia of the beam is small.

Lewis Research Center National Aeronautics and Space Administration Cleveland, Ohio, February 3, 1992

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REPORT	DOCUMENTATION F	PAGE	Form Approved OMB No. 0704-0188
Public reporting burden for this collection of gathering and maintaining the data needed, collection of information, including suggestion Davis Highway, Suite 1204, Arlington, VA 2	information is estimated to average 1 hour pe and completing and reviewing the collection of ins for reducing this burden, to Washington He 2202(-4302, and to the Office of Management	r response, including the time for revil if information. Send comments regard sedquarters Services, Directorate for in and Budget, Papework Reduction Pro	wing instructions, searching existing data sources, ing this burden estimate or any other aspect of this formation Operations and Reports, 1215 Jefferson ject (0704-0188), Washington, DC 20503.
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	August 1992		Technical Paper
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Preloaded Solar Array Mas	st		
AUTHOR(S)			WU-474-46-10
Sasan C. Armand and Paul	l Lin		
PERFORMING ORGANIZATION	NAME(S) AND ADDRESS(ES)		B. PERFORMING ORGANIZATION
National Aeronautics and 5	Space Administration		
Lewis Research Center			E-6847
Cleveland, Ohio 44135-3	3191		
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I. SUPPLEMENTARY NOTES			
Sasan C. Armand, NASA I	Lewis Research Center; Paul Lin	, Cleveland State Universi	ty, Cleveland, Ohio 44115.
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