

Anisotropic permeabilities evolution of reservoir rocks under pressure:

New experimental and numerical approaches



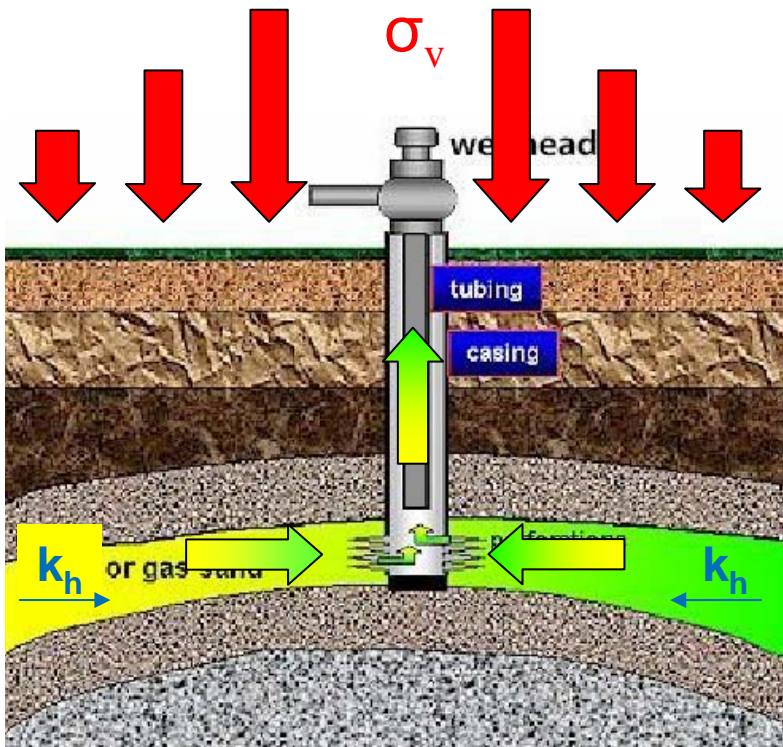
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Context of our study :

Reservoir permeability drop due to compaction during the production



- Primary recuperation \rightarrow Pore Pressure P_p decreases
- Effective stress increases
$$\sigma_{eff} = \frac{2\sigma_h + \sigma_v}{3} - P_p$$
- Effective vertical stress increases (dependent of the distance to the borehole)
- Horizontal permeability dependency of the production

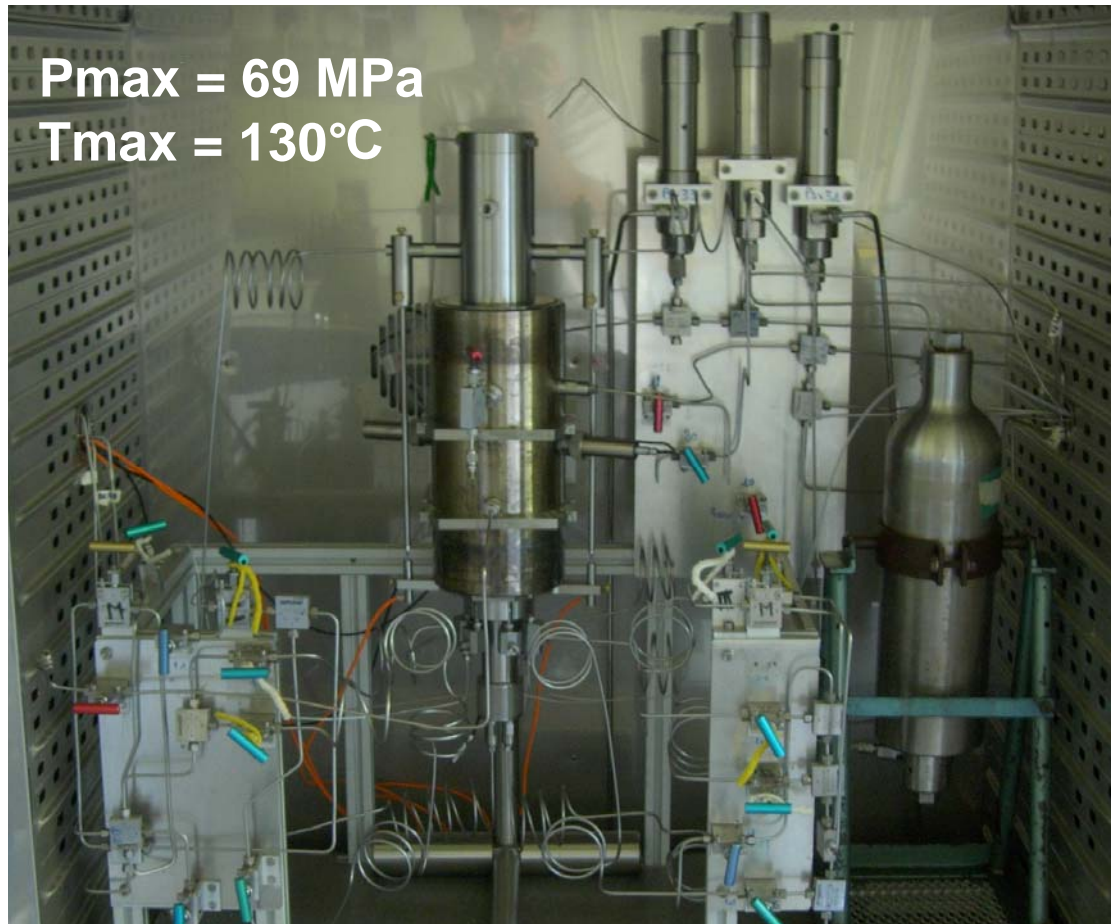
Motivations :

Relation between the evolution of the stress field anisotropy and the transport properties anisotropy ?

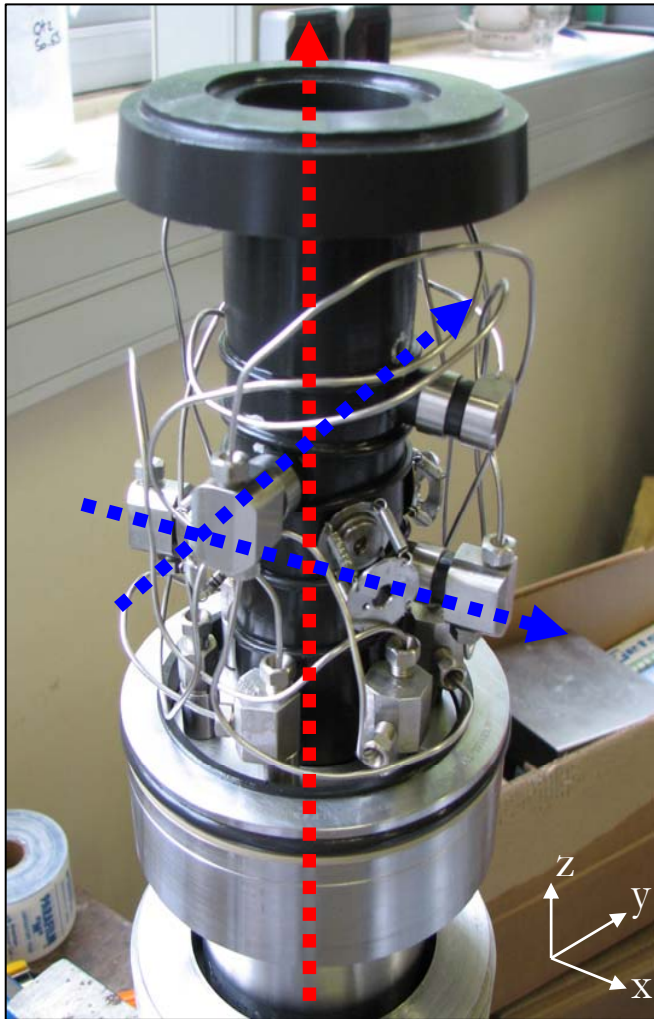
Effects of the stress path on reservoir compressibility ? \rightarrow Reservoir simulation

EXPERIMENTAL SET-UP

Triaxial cell specially designed to directional
permeabilities measurements



Special Core sleeve equipment



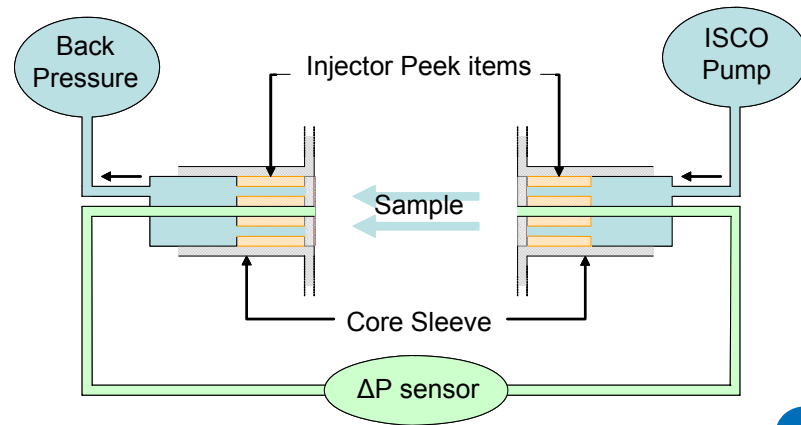
Tridirectional Permeabilities:

Axial permeability measurements: $k_{az,FL}$ & $k_{az,ML}$

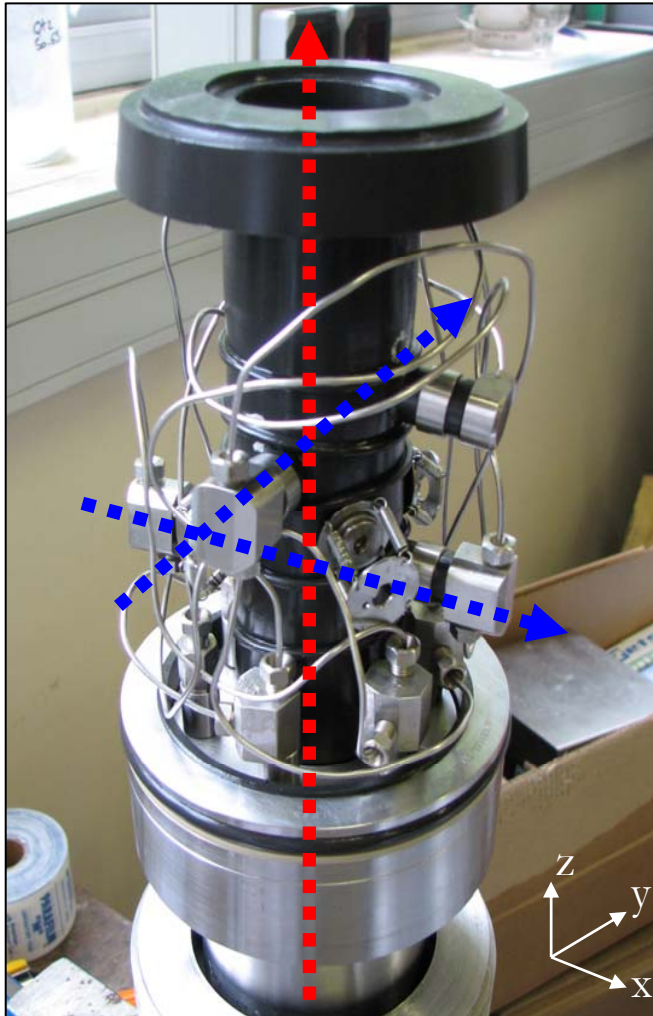
- Classical between inlet and outlet of the sample
- Pore pressure sampling at the mid-length of the sample

Radial permeability measurements: k_{rx} & k_{ry}

- 2 pairs of injector/receptor at the contact of lateral sample surface.



Special Core sleeve equipment



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- Classical between inlet and outlet of the sample
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Complementary measurements:

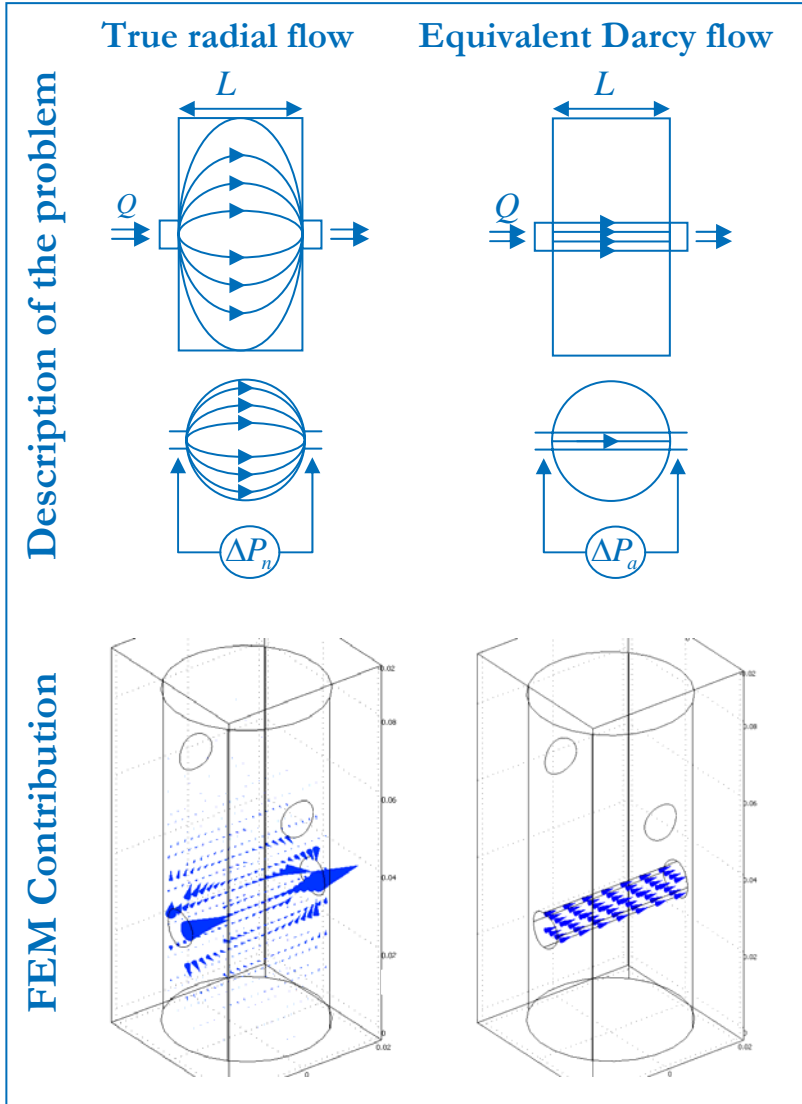
Sample strains:

- Axial displacement of the upper piston : external LVDT
- Radial strains : Cantilever fixed on the core sleeve

Porosity Evolution:

- ΔV_p recorded by ISCO Pump during each confining pressure increase.

Modified Darcy law:
Geometric Factor Calculation using Finite Elements Method



Modified Darcy law : $\frac{Q}{A_a} = -G \frac{k_r}{\mu} \frac{\Delta P}{D}$

True radial flow

$$Q_n = A_n \frac{k_n \Delta P_n}{\mu D}$$

Effective cross-section Area

Equivalent Darcy flow

$$Q_a = A_a \frac{k_a \Delta P_a}{\mu D}$$

Injector Area

Considering an isotropic permeability case :

Geometric factor $G = \frac{A_a}{A_n} = \frac{\Delta P_n}{\Delta P_a}$

FEM simulation $\rightarrow G = 0.18$





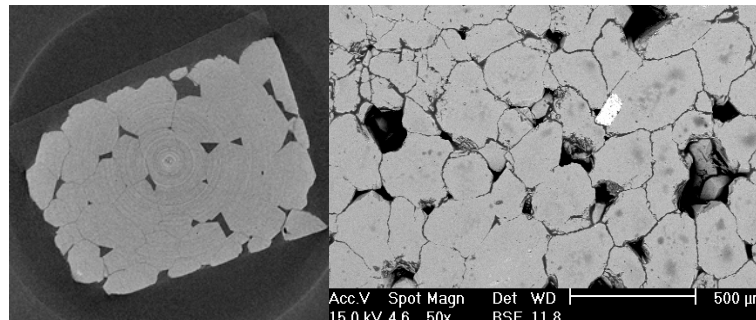
EXPERIMENTAL RESULTS

Tested Samples

Fontainebleau Sandstones:

Porosity: 5.4 to 8% Permeability: 2.5 to 30mD

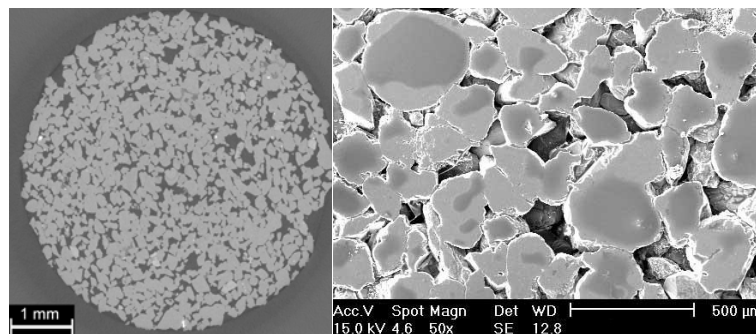
→ Hydrostatic loading



Bentheimer Sandstones:

Porosity: 24% Permeability: 3000 mD

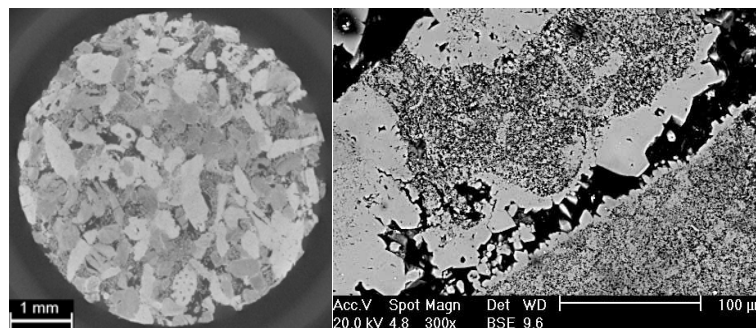
→ Hydrostatic and Deviatoric loading at low confining pressure



Estailades Limestones:

Porosity: 27% Permeability: 150mD

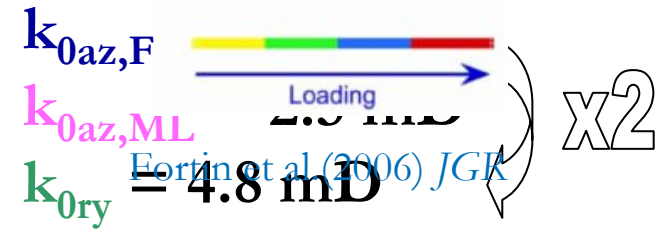
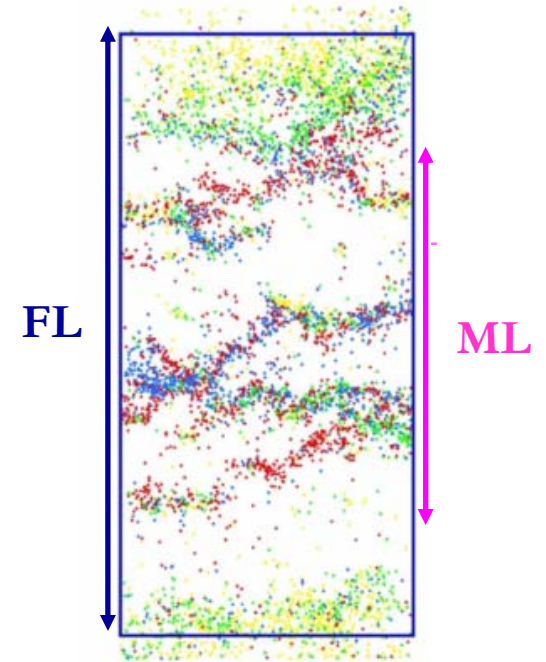
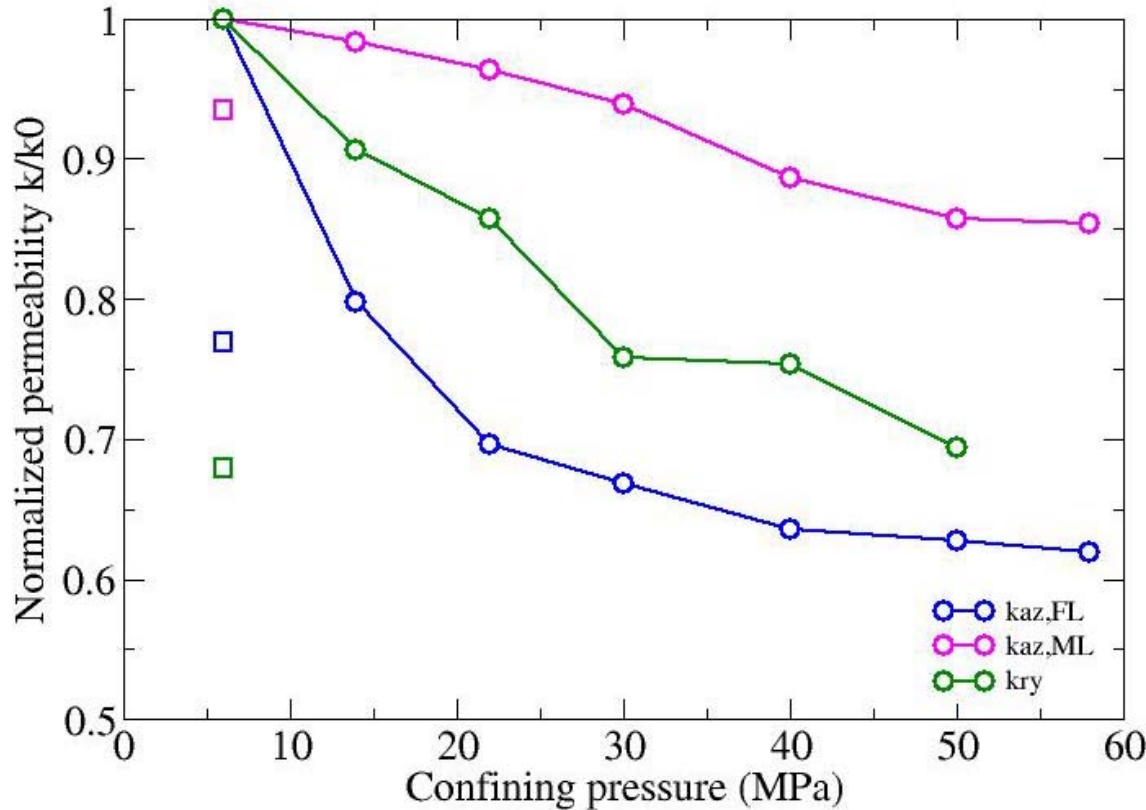
→ Hydrostatic and Deviatoric loading at low confining pressure



Experimental results : Low permeability sandstone (Fontainebleau)

HYDROSTATIC LOADING

SAMPLE 1 : $\phi = 5.4\%$



$k_{0ry} = 4.8 \text{ mD}$

Ref: David C.(1993) *JGR*; Korsnes et al.(2006) *Tectonophysics*

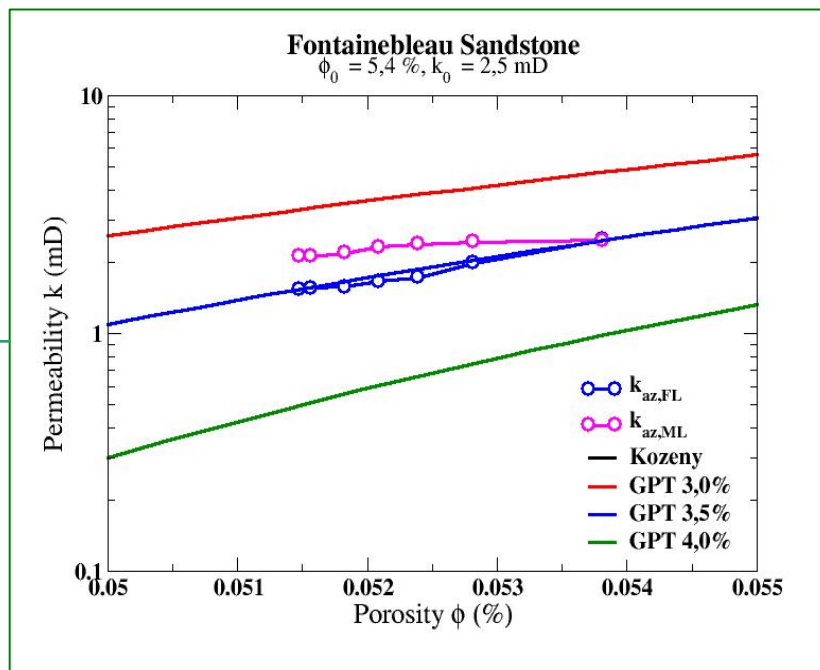
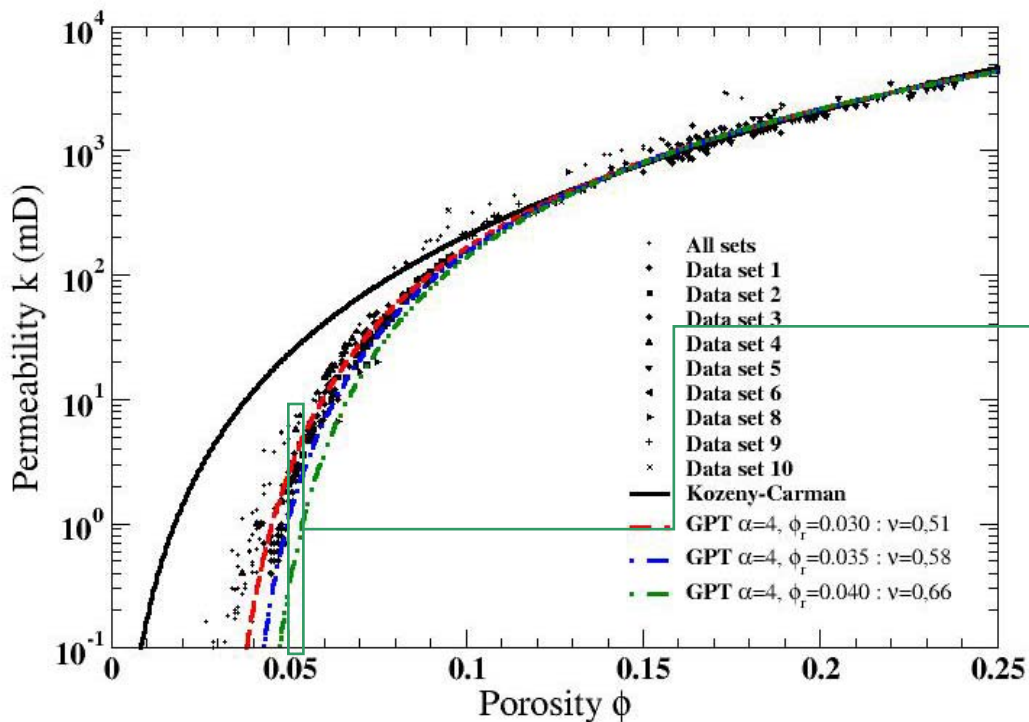
Experimental measurements validation on Fontainebleau sandstones

Confrontation of measured $k-\phi$ and a model of diagenetic compression of Quartz aggregates

Grain Pore Throat Model*

$$k \propto (\phi^{1-\nu} - \phi_r^{1-\nu})^4$$

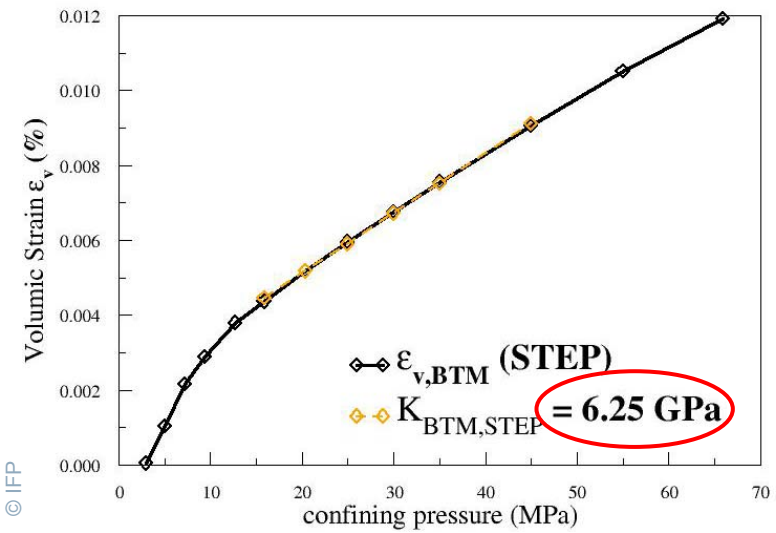
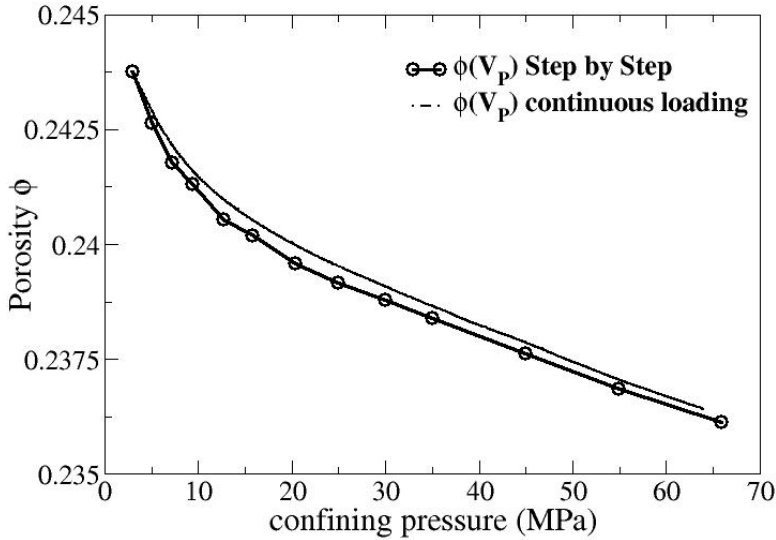
ϕ_r : Residual Porosity; ν : Geometrical Exponent defined as $S \propto \phi^\nu$



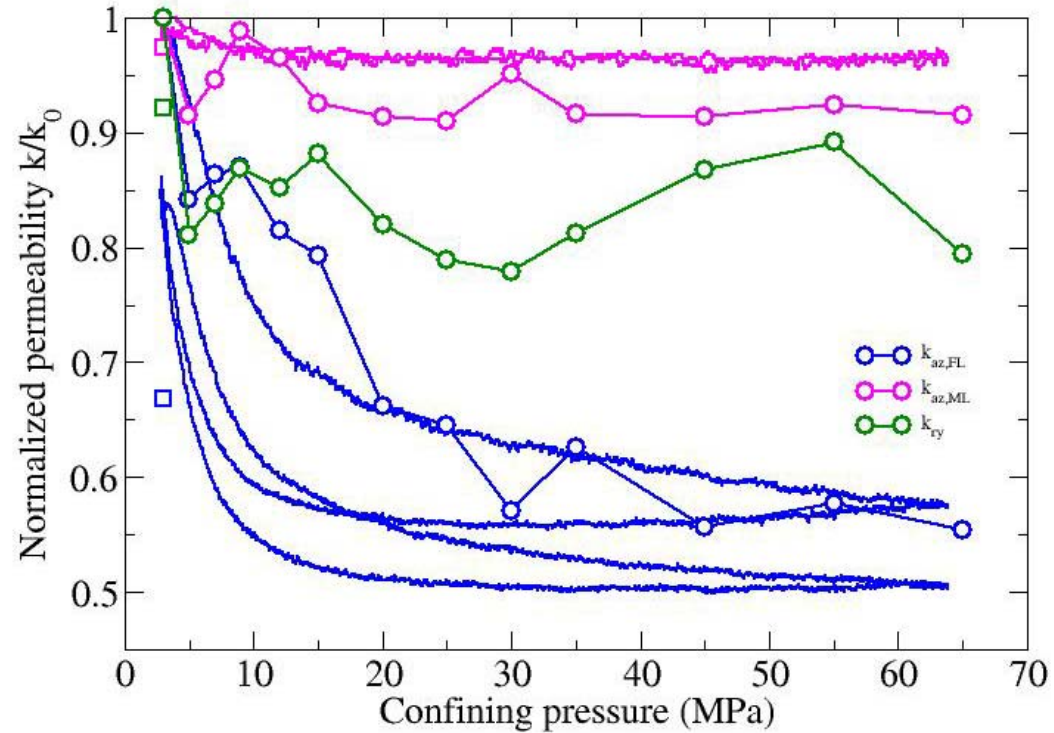
→ Verified for 3 Fontainebleau Samples (low porosity and low permeability)

* Chauveteau G. (2002) SPE#73736

Experimental results : High permeability sandstone (Bentheimer)

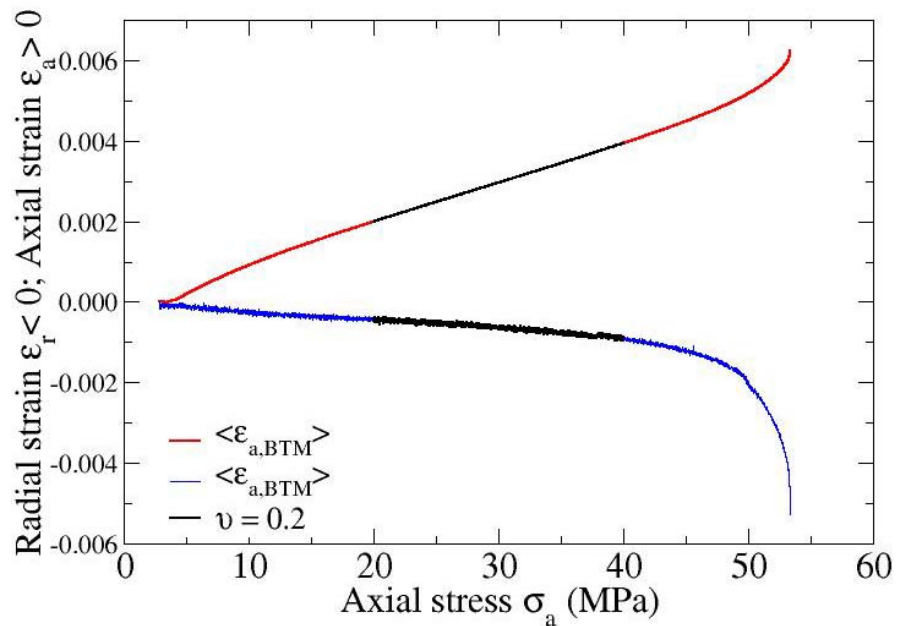
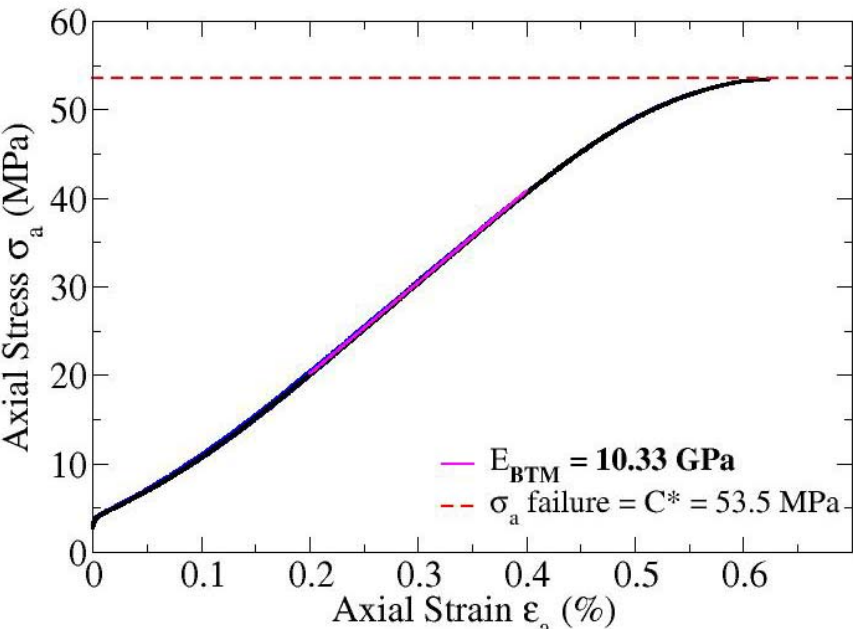
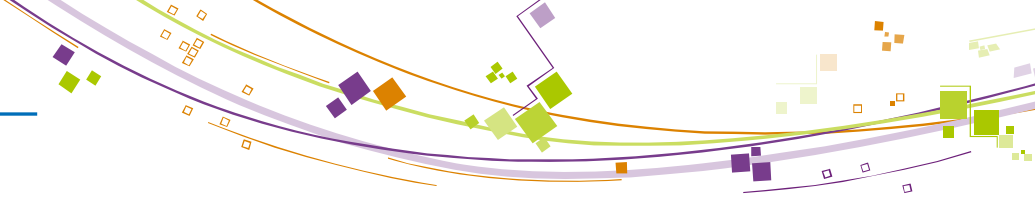


HYDROSTATIC LOADING



$k_{0az,FL} = 1840 \text{ mD}$; $k_{0az,ML} = 2900 \text{ mD}$
 $k_{0ry} = 2825 \text{ mD}$

« UNIAXIAL » LOADING



Brittle failure: $\sigma_a = 53.5 \text{ MPa}$

Effective Elastic moduli calculated in the range of axial stress [20:40] MPa :

$E = 10.3 \text{ GPa}$
 $\nu = 0.2$

Rupture influence on 3D permeabilities

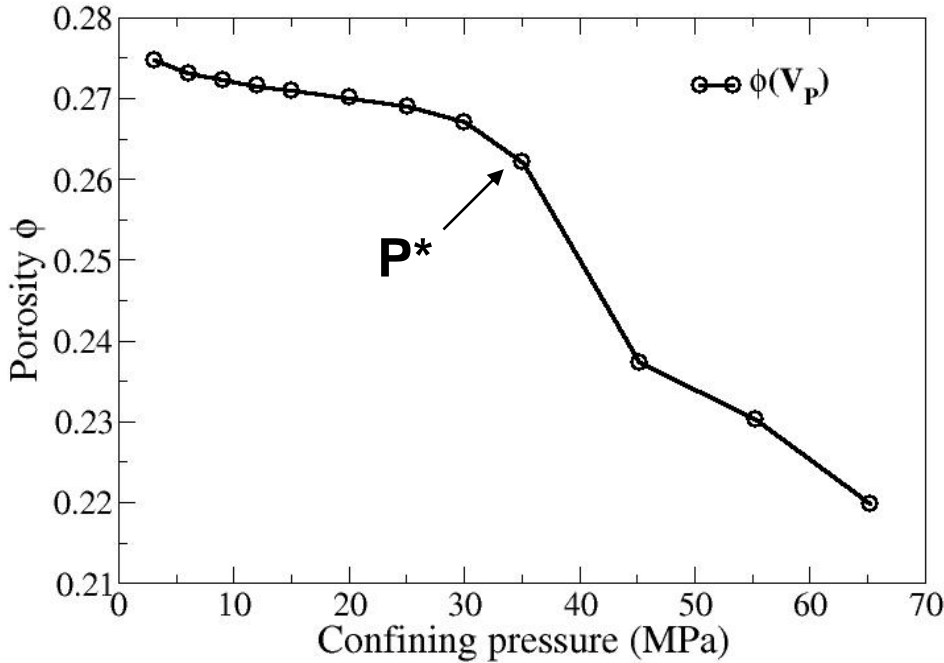
Axial: $k_{az,FL} \text{ before failure} = 1185 \text{ mD}$
 $k_{az,FL} \text{ after failure} = 1560 \text{ mD}$ ↗↗

Radial: $k_{rx} \text{ before failure} = 2139 \text{ mD}$
 $k_{rx} \text{ after failure} = 631 \text{ mD}$ ↘↘

Sulem et Ouffroukh (2005) *Rock Mech. and rock eng.*

Experimental results : intermediate permeability limestone (Estailades)

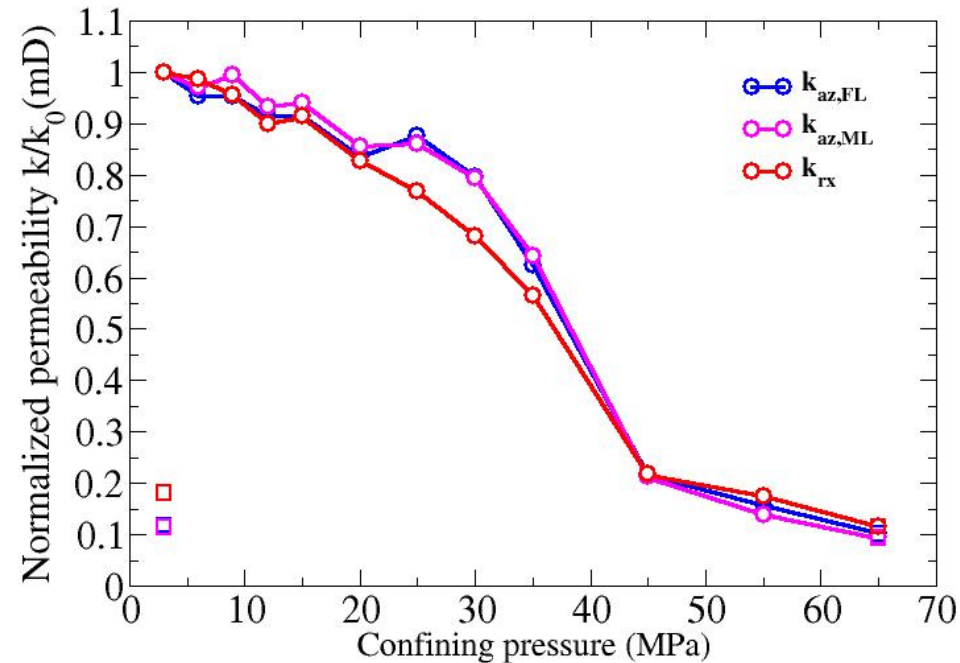
Porosity evolution - hydrostatic loading



Homogeneous Pore Collapse

$P^* = 30 \text{ MPa}$

Permeability evolution - hydrostatic loading



$k_{0az,FL} = 152 \text{ mD}$

$k_{0az,ML} = 162 \text{ mD}$

$k_{0ry} = 70 \text{ mD}$



$k_{0az,FL} = 20 \text{ mD}$

$k_{0az,ML} = 20 \text{ mD}$

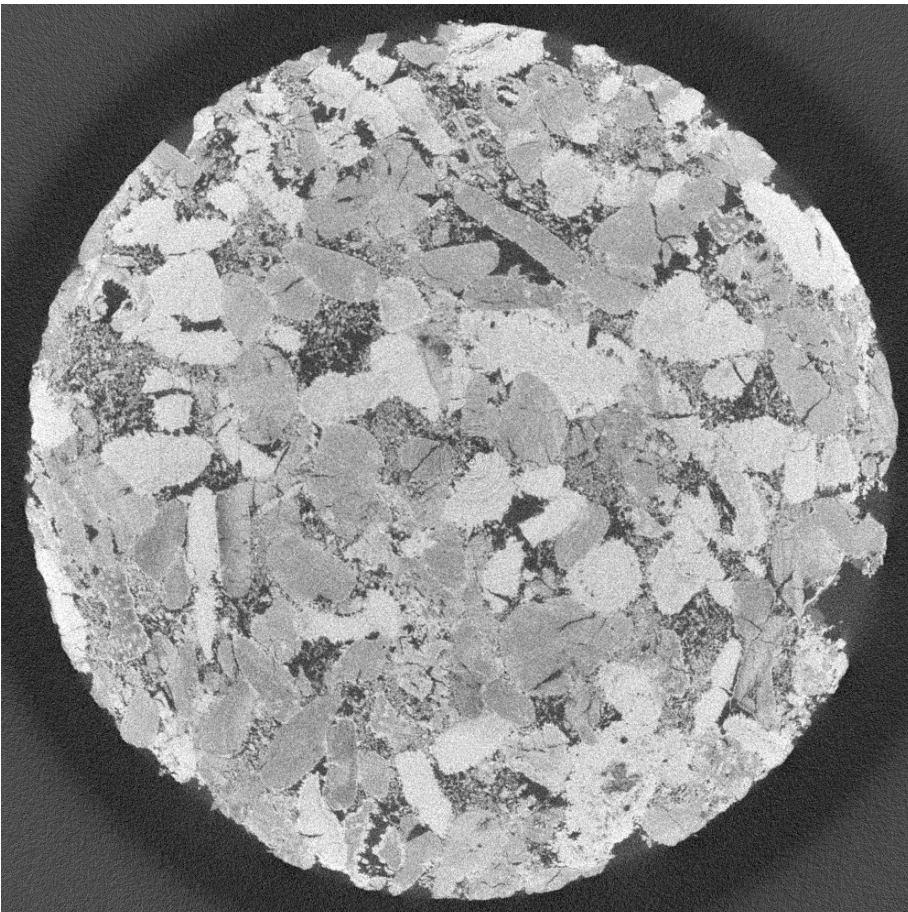
$k_{0ry} = 13 \text{ mD}$

Experimental results : Intermediate permeability limestone (Estailades)

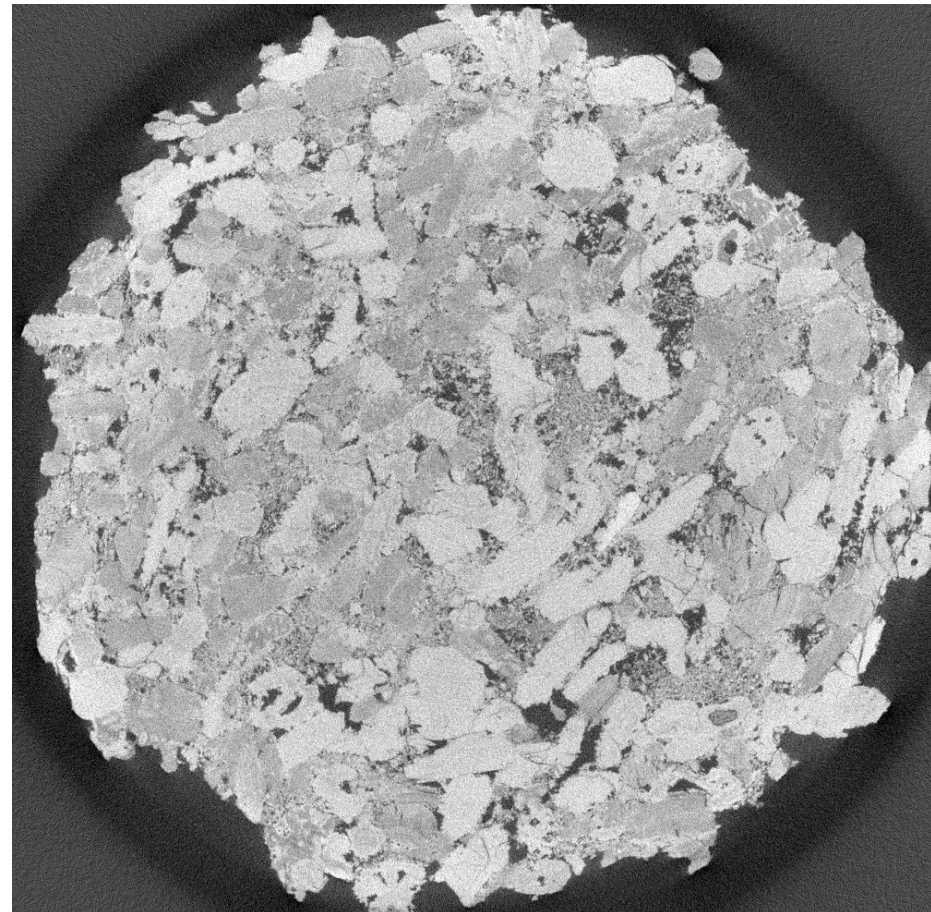
High Resolution Micro-Scanner Slides (3 μm resolution)

BEFORE LOADING

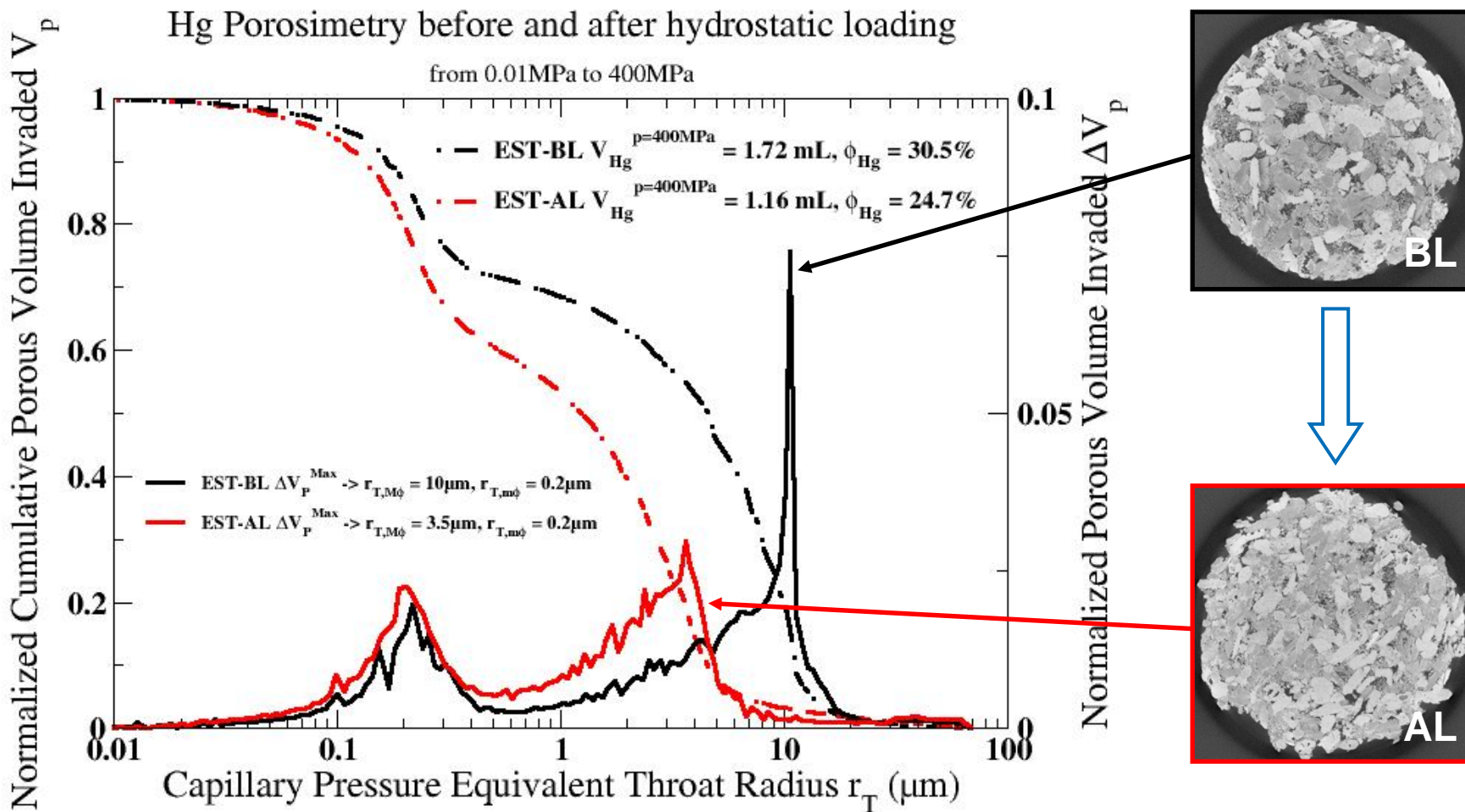
AFTER LOADING



5 mm



5 mm





CONCLUSIONS #1

- Simultaneous radial and axial permeability measurements are feasible.
- Classical axial permeability measurements may be affected by end effects.
- The pressure dependency of permeabilities is well captured.

ON GOING EXPERIMENTAL WORK:

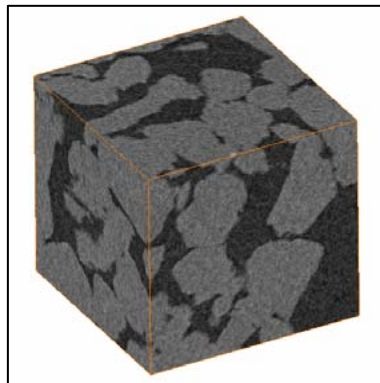
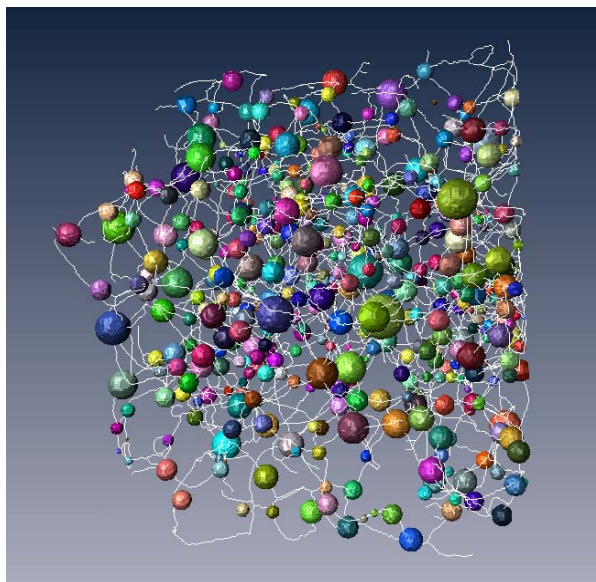
- *Investigation of the influence of strains localization on flow properties (In-situ Observations)*
- *Focus on stress paths more representative of reservoir conditions.*



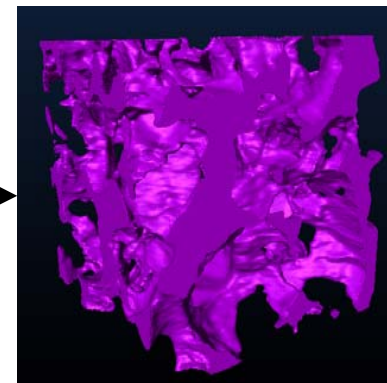
PORE SCALE MECHANISMS MODELISATION

Modelisation of pore-scale mechanisms

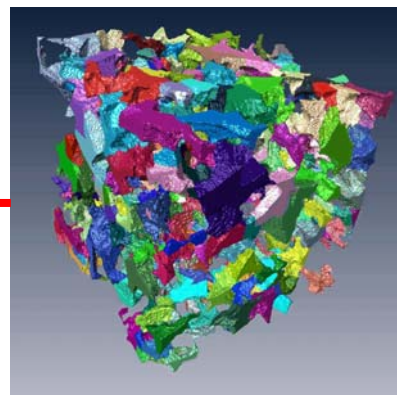
Equivalent Pore Network extraction* :



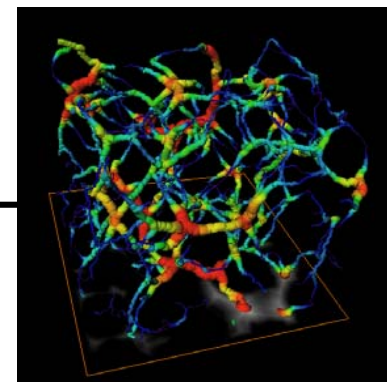
Microtomography 3D Reconstruction



Porosity threshold



Individual Pore Indexation



Pore Network Skeletonization
local minimum radius

Pores: Equivalent Volume spheres
Throats: Cylindrical channels

Output data:

Throats dimension: L_T , r_T & AR
Equivalent pores volumes: ϕ
Network connectivity

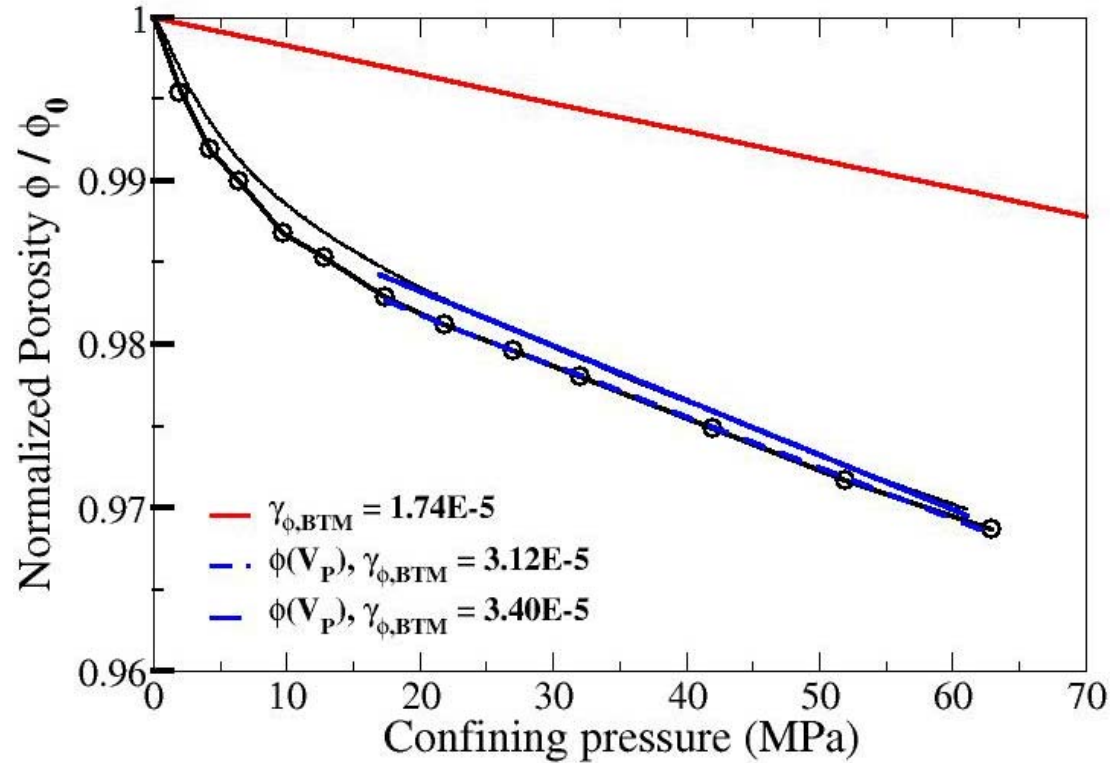
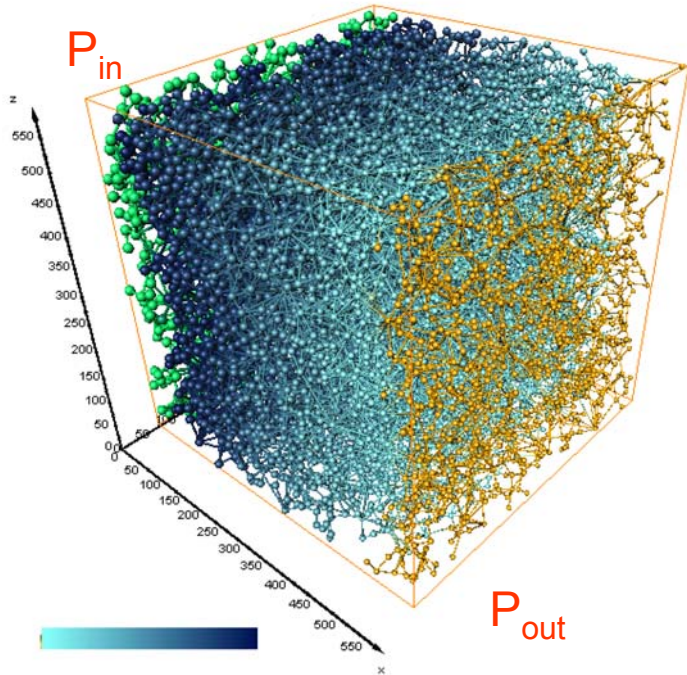
* Youssef et al. (2007) *SCA*

Modelisation of pore-scale mechanisms : Fluid flows and compaction coupling

Transport properties simulation	Network compaction implementation
<p>Individual channel conductance :</p> $g = \frac{\pi r^4}{8 L}$ <p>Problem formulation :</p> <p>In the throat between pores i and j $q_{ij} = g_{ij} (P_i - P_j)$</p> <p>In the Pores : $\sum_{i \rightarrow j} q_{ij} = 0$</p> <p>Matrix formulation : $G \bullet \vec{P} = \vec{S}$</p> <p>→ Resolution of network effective hydraulic conductivity</p>	<p><u>Spherical Pores:</u></p> $r_p \cong r_{p,0} (1 - \gamma_p (p - p_0))^*$ $\rightarrow \gamma_p = \frac{(1 + \nu)^{**}}{2E}$ <p><u>Cylindrical Pore Throats:</u></p> $r_T \cong r_{T,0} (1 - \gamma_T (p - p_0))^*$ $\rightarrow \gamma_T = \frac{(1 + \nu^2)^{**}}{E}$ <p>l_T pressure dependency neglected</p> $g_T(P) \rightarrow G(P) \rightarrow k(P)$ <p>* Bernabé et al. (1982) <i>Mech. of Materials.</i> ; Bernabé et al. (1995) <i>JGR</i> ** Jaeger et Cook (1976) <i>Fundamental of Rock Mechanics.</i></p>

Modelisation of pore-scale mechanisms : Bentheimer Sandstone Example

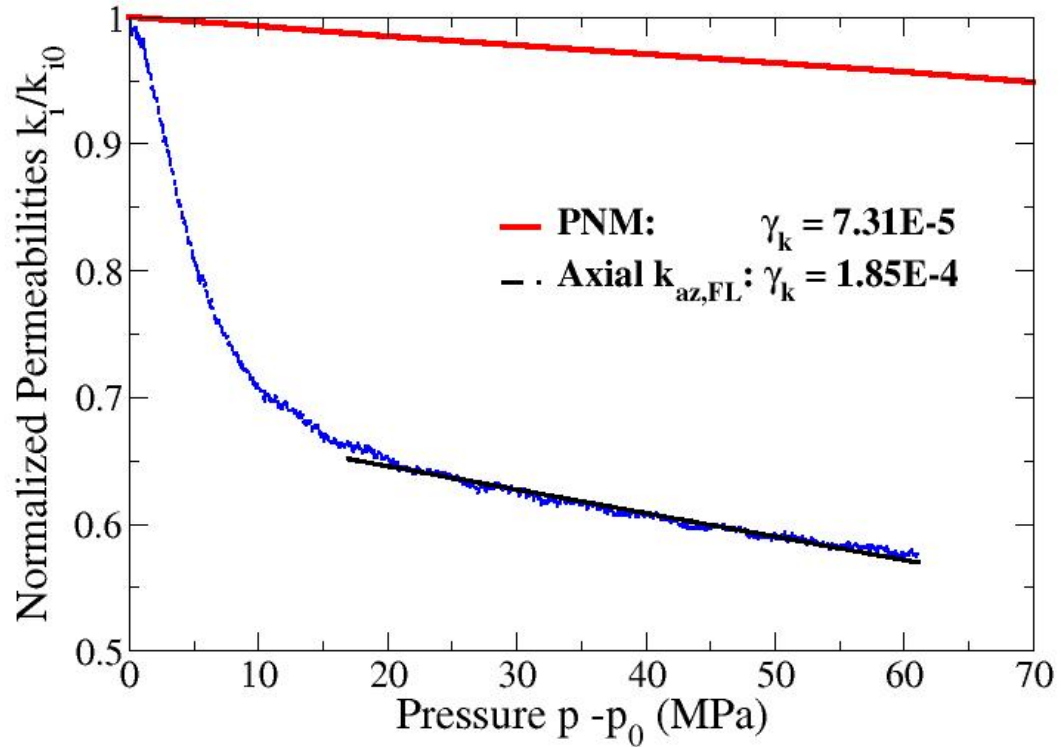
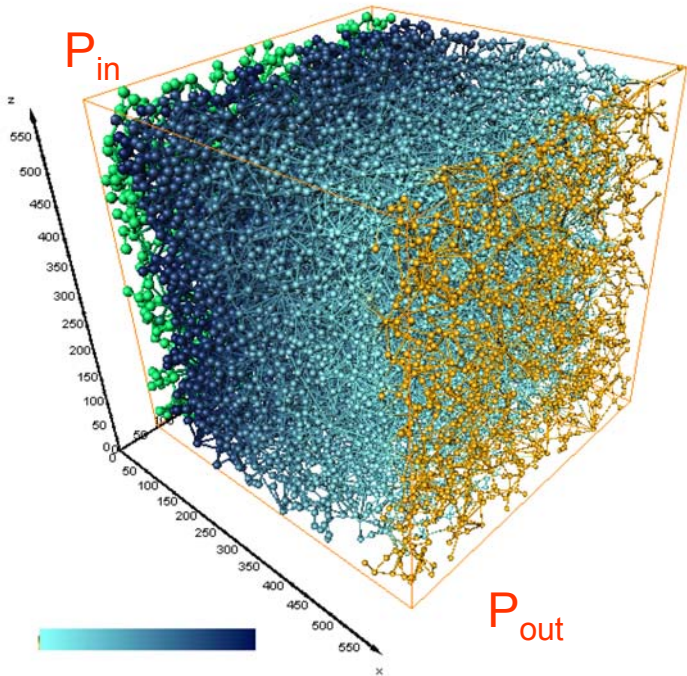
Extracted equivalent pore network
Volume = 500x500x500 x6μm



$$\phi_{\text{exp}} = 24.5\% \quad \longleftrightarrow \quad \phi_{\mu\text{CT}} = 24.4\%$$

Modelisation of pore-scale mechanisms : Bentheimer Sandstone Example

Extracted equivalent pore network
Volume = 500x500x500 x6μm



$$k_{exp} = 3000 mD$$

$$k_{\mu CT} = 847 mD$$

$$A_{k, \mu CT} < 10\%$$

Discrepancy lies to the definition of r_T
(minimum local pore throat radius)

$$g_h = \frac{\pi r_T^4}{8 L}$$

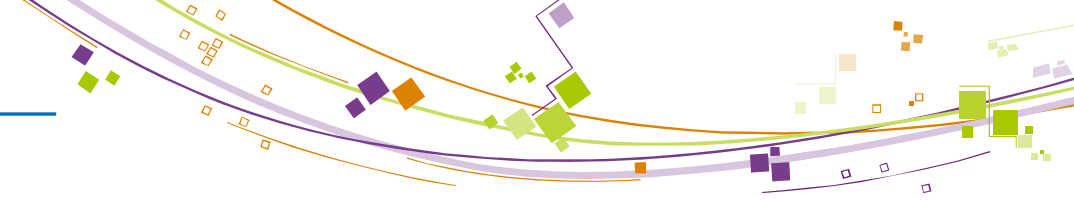


CONCLUSIONS #2 : MICRO-TOMOGRAPHY CONTRIBUTION

- Simple pressure dependency model can be applied on the equivalent pore network.

ON GOING NUMERICAL WORK :

- *Alternative description of throats dimensions*
- *Investigation of the anisotropic distribution of the channels*
- *FEM simulation of the coupled effects of deforming matrix and fluid flows (TRUE GEOMETRY OF THE POROSITY)*



THANKS FOR YOUR
ATTENTION

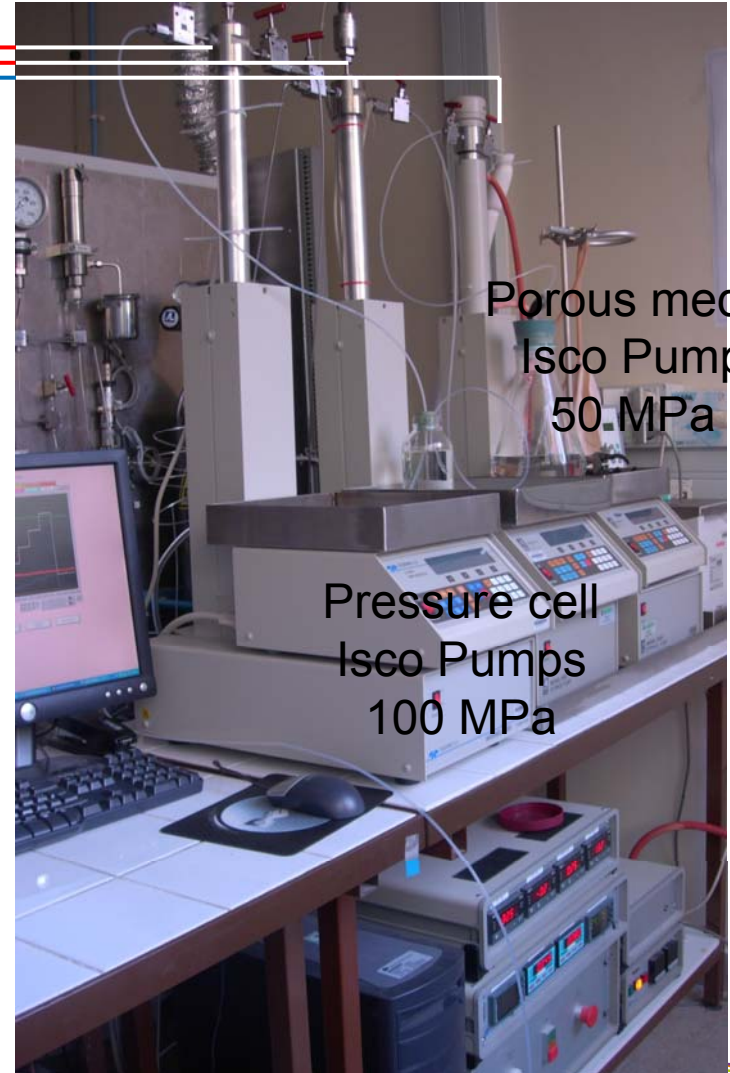
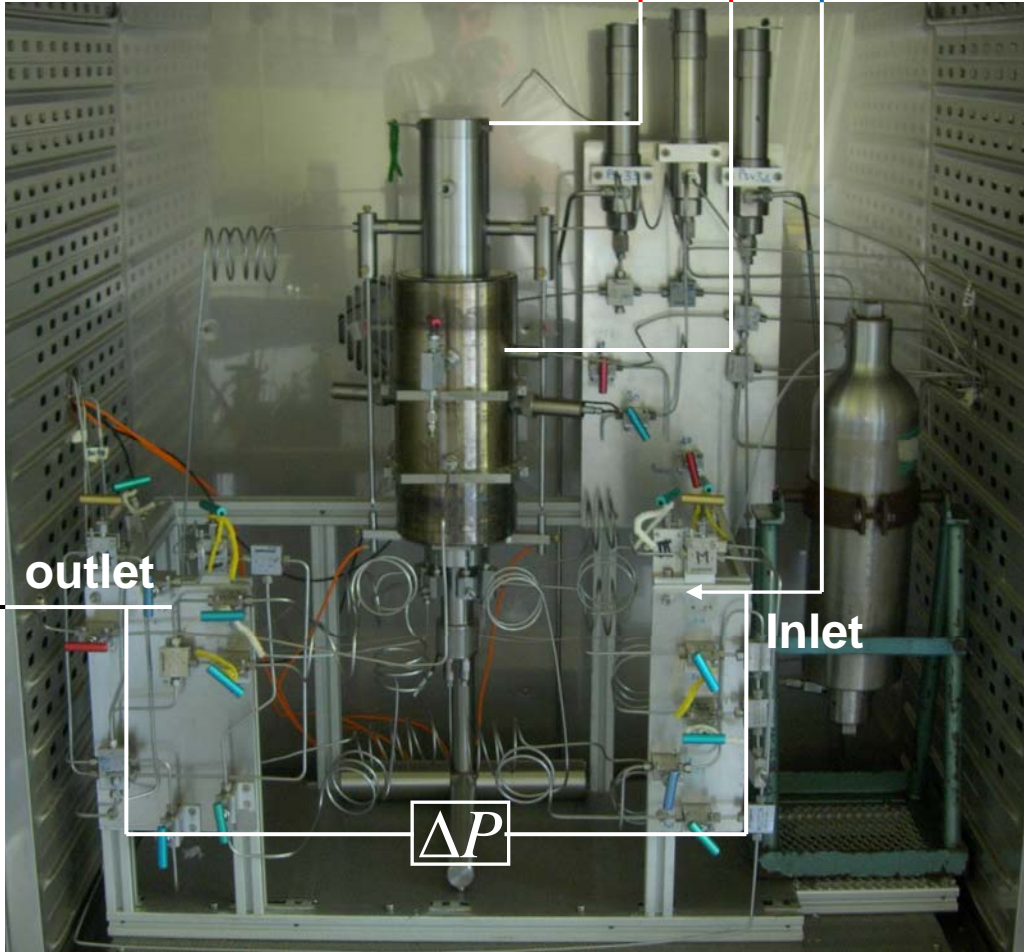


Anisotropic permeabilities evolution of reservoir rocks under pressure

New Experimental Set-up : Triaxial cell specially designed to directional permeabilities measurements

$P_{max} = 69 \text{ MPa}$

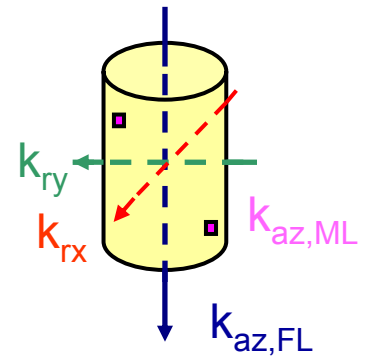
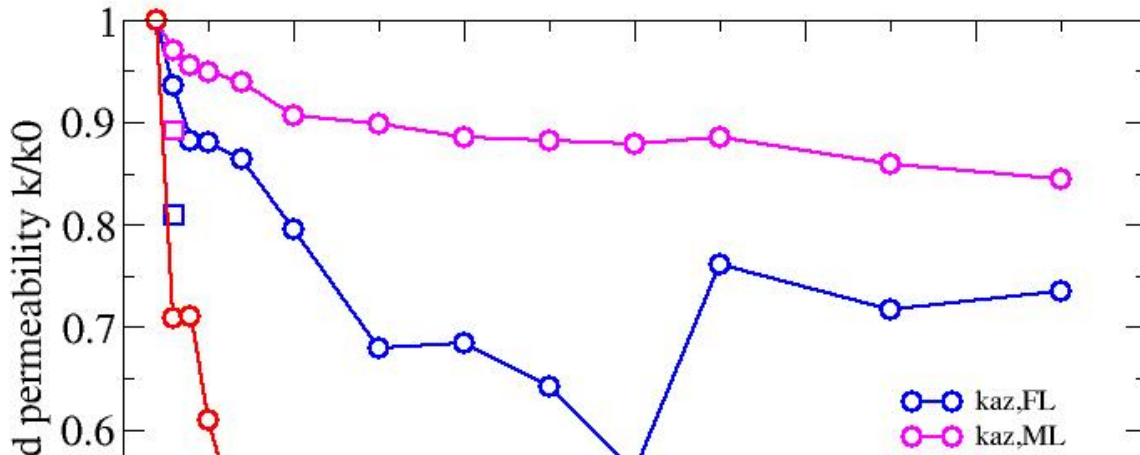
Max Using Temperature = 130°



Experimental results : Low permeability sandstone (Fontainebleau)

Sample 2 : $\phi = 8\%$

Directional permeability evolution SAMPLE 2



Preliminary Experimental Conclusions :

- Radial and axial permeabilities values differences due to G calculation
- Intermediate axial permeability measurements looks more consistent than classical measurements