RIVER MODELLING FOR FLOOD RISK MAP PREDICTION:
CASE STUDY OF SUNGAI KAYU ARA

by

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Dissertation submitted in fulfilment of the
requirements for the degree of
Master of Science

Universiti Sains Malaysia

July 2009
ACKNOWLEDGEMENTS

First and foremost I thank the Universiti Sains Malaysia (USM), especially School of Civil Engineering, for the last 26 months. To Assoc. Prof. Dr. Rozi bin Abdullah and Assoc. Prof. Dr. Hj. Ismail Abustan, my supervisors, I owe a great deal of gratitude for supporting my efforts with their precious time. Their lessons, guidance, supervision and unparalleled knowledge shared will not be soon forgotten. My thanks also due to Prof. Hamidi bin Abdul Aziz and Assoc. Prof. Badorul Hisham bin Abu Bakar, Dean and Deputy Dean of School of Civil Engineering. The assistance provided by the Institute of Postgraduate Studies (IPS) is very much appreciated.

To my parents, Mr. Ali Akbar Alaghmand and Madam Seyedeh Beigom Banikamali for making all of this possible through their support and love without which the thesis would not have come to fruition. I would like to thank them who understand best my commitment and tolerate my long time absence. They were not near me here but they have always been in my mind and heart to set me on the right path and for that I am eternally grateful. Their sacrifices give me courage and let me be the reasons that I have been able to succeed.

I have made every effort to identify the original sources of information sated but, if there have been any accidental errors of missions, I apologises to those concerned.

Sina Alaghmand

July 2009
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LIST OF SYMBOLS

\( \bar{R}_t \) Average rainfall intensity (mm/hr) for ARI \( R \) and duration \( t \)

\( R \) Average recurrent interval (years)

\( HR \) River Flood Hazard Rating

\( V \) Velocity (m/s)

\( D \) Depth (m)

\( DF \) Debris factor

\( f_t \) Loss during period \( t \)

\( K \) Saturated hydraulic conductivity

\( S_f \) Wetting front suction

\( F_t \) Cumulative loss at time \( t \)

\( t_r \) Rainfall duration

\( t_p \) River basin lag-time

\( A \) Watershed drainage area

\( C_p \) UH peaking coefficient

\( C \) Conversion constant

\( Cp \) Peak flow coefficient,

\( Q_0 \) Initial base-flow (at time zero)

\( K \) an exponential decay constant

\( S_f \) Energy gradient

\( S_0 \) Bottom slope

\( V \) Velocity

\( y \) Hydraulic depth

\( x \) Distance along the flow path

\( t \) Time (Duration)
$g$  Acceleration due to gravity  
$Q$  Flow discharge  
$R$  Hydraulic radius  
$A$  Cross-sectional area  
$B$  Water surface width  
$q$  Lateral inflow per unit length of channel  
$F$  Goodness-of-fit  
$q_i$  Observed value $i$  
$r_i$  Concurrent simulated value $i$  
$n$  Number of data pairs  
$Y$  Depth of water at the cross section  
$Z$  Elevation of the main channel inverts  
$V$  Average velocities, respectively  
$h_e$  Energy head loss  
$L_{lob}$  Cross section reach length specified for flow in the left overbank  
$L_{ch}$  Cross section reach length specified for flow in the main channel  
$L_{rob}$  Cross section reach length specified for flow in the right overbank
LIST OF PUBLICATION BASED ON THIS THESIS


PERMODELAN SUNGAI UNTUK RAMALAN PETA RISIKO BANJIR: KAJIAN KES SUNGAI KAYU ARA

ABSTRAK

Penyelidikan ini memberikan tumpuan terhadap kepentingan kebanjiran sungai di kawasan bandar yang menyebabkan kehilangan nyawa dan kerosakan harta benda. Pengetahuan tindak balas tadian sungai terhadap kejadian hujan yang menghasilkan airlarian ribut adalah kritikal di dalam praktis kejuruteraan untuk perancangan dan pembangunan di kawasan bandar. Pemetaan bahaya kebanjiran sungai merupakan gandingan permodelan hidrologik, permodelan hidraulik dan paparan melalui GIS. Sungai Kayu Ara yang terletak di Damansara dijadikan kajian kes di dalam penyelidikan ini. Kesah magnitudo hujan (20 tahun, 50 tahun dan 100 tahun ARI) dan tempoh (60, 120, 180 dan 360 minit) untuk keadaan pembagunan sedia ada, pertengahan dan puncak dinilai menggunakan 36 senario yang telah dikenal pasti. Keputusan dari simulasi model hidrologik menunjukan peningkatan magnitudo hujan bolih menghasilkan pertambahan isipadu dan puncak kadaralir airlarian ribut, manakala peningkatan tempoh peristiwa hujan menyebabkan pertambahan isipadu airlarian ribut tetapi penurunan puncak kadaralir. Isipadu dan puncak kadaralir yang tinggi dihasilkan oleh keadaan pembangunan puncak (90% kawasan tidak telap air) jika dibandingkan dengan keadaan pembangunan sedia ada dan pertengahan. Penjanaan peta bahaya kebanjiran sungai adalah berdasarkan pada kedalaman air, halaju aliran, dan gandingan kedalaman air dan halaju aliran. Peta tersebut menunjukan impak kedalaman air adalah lebih tinggi jika dibandingkan dengan halaju aliran semasa kejadian kebanjiran sungai. Sehubungan dengan itu, bahaya yang disebabkan oleh kedalaman air adalah lebih signifikan dari halaju aliran. Peta bahaya kebanjiran merupakan asas untuk ramalan risiko kebanjiran.
RIVER MODELLING FOR FLOOD RISK MAP PREDICTION: CASE STUDY OF SUNGAI KAYU ARA

ABSTRACT

The research illustrates an importance of river flood in urban areas which cause loss of lives and properties damages. Knowledge on the river basin response to rainfall events of runoff is vital in engineering practices for urban planning and development. Flood hazard map prediction is a combination of hydrological modelling, hydraulic modelling and river flood visualization using GIS. The case study of this research is Sungai Kayu Ara located in Damansara, Kuala Lumpur. A total of 36 scenarios are identified in order to assess the effects of rainfall magnitude (20 year, 50 year and 100 year ARI) and duration (60, 120, 180 and 360 minutes) for existing, intermediate and ultimate development conditions. The results of hydrological model simulation indicated that, an increase in the rainfall magnitude leads to increase of runoff volume and peak discharge while increase of rainfall event duration increases the runoff volume but decreases the runoff peak discharge. Furthermore, an ultimate river basin development conditions (90% imperviousness) generate higher runoff volume and peak discharge in comparison with existing and intermediate development conditions. The river flood hazard maps are generated based on water depth, flow velocity and combination of water depth and flow velocity. These maps showed that the impact of water depth is more considerable than flow velocity during river flood. Hence, hazard attributed to water depth is more significant in comparison with flow velocity. River flood hazard maps are the base of the river flood risk prediction. River flood risk maps are considered as the function of river flood hazard, vulnerability and exposure. In this case, land-use type, main road accessibility and debris flow are involved to reflect the terms
vulnerability and exposure in river flood risk map prediction. These four elements are provided as GIS raster layers in which all pixels indicate the severity value of each element. The generated river flood risk map is the result of combination of four main elements, river flood hazard, land-use type, main road accessibility and debris flow. The suggested method for river flood risk map prediction recommends four classes of severity for river flood consists of, low, medium, high and extreme. The established flood risk prediction map has shown that the river flood hazard, debris flow hazard, land-use type and main road accessibility have significant impact and able to facilitate the planning and management of river flooding in urban areas. The variation of predicted river flood risk pattern is a function of river flood hazard and debris flow hazard patterns; as the distribution of hazards produced by land-use type and main road accessibility is uniform.
CHAPTER 1
INTRODUCTION

1.1 Background and Motivation

In recent years there have been a number of significant riverine floods in the rest of the world, which resulted in tragic loss of life and in enormous material damage (Figures 1.1 and 1.2). In the past decades, thousands of lives have been lost, directly or indirectly, by flooding. In fact, of all natural risks, floods pose the most widely distributed natural risk to life today. River flood risk management is the process under which different bodies try to reduce the current and the future vulnerability of human society to natural risks. Flood risk management measures can be structural where the risk is modified for example dam and reservoir construction, channel improvements, by-pass channels and artificial levees. Non-structural where the flood damage and disruption is modified for example setting up flood plain management regulations such as zoning, building codes and measures where both the methods are applied. It is clear that no protection work can offer a hundred percent security against floods. There is always the possibility that a threshold is surpassed and that floodwater will enter into areas where it should not go, e.g. by overtopping or breaching of dikes.

Figure 1.1 Flood in Jalan Sultan Ismail, Kuala Lumpur, June 2007
Starting in the year 2000s, extreme rainfall events with high intensity are not longer a new issue in Malaysian urban cities, especially in the West Coast area (Figures 1.3 and 1.4). This phenomenon is formed mostly through convection process (Embi et. al., 2004). The main motivation of this research is an importance of river flood events in urban areas which cause in large number lost of lives and properties damages. Knowledge on the river basin response to rainfall events which is in the form of runoff is vital in engineering practices for urban planning and management. River flood modelling is a combination of hydrological modelling, hydraulic modelling and river flood visualization using GIS.

Flooding is one of the major natural hazards affecting communities across Malaysia and has caused damages worth millions of dollars every year. The required allocation for flood mitigation projects has increased almost 600% (RM 6000 million) for the 8th Malaysian Plan compared to RM 1000 million during the 7th Malaysian Plan (Abdullah, 2000).
Floods are recurring phenomena, which form a necessary and enduring feature of all river basin and lowland coastal system. Major floods are the largest cause of economic losses from natural disasters mainly in more developed countries. And they are also a major cause of disaster-related deaths, mainly in the less developed countries. Despite recent advances in the understanding of the relevant climatologically, fluvial and marine mechanisms, and a greater investment in flood reduction measures, floods take a larger number of lives and damage more properties each year, mainly, because of unwise land management practices and growing human vulnerability (Smith and Ward, 1998).

Knowing the fact that the floods are part of human being life and that this natural phenomena can’t be fully controlled, it’s important to focus and improve knowledge about the prevention. In order to achieve this issue it is crucial that, more specific and scientific work must be developed to a better understanding of the flooding phenomena.
and their related geographical, hydrological and geomorphologic causes. Vaz (2000) and Jaarsma et al. (2001) emphasized, respectively, the need to define a strategy that includes a judicious combination of structural and none structural measures, based on a careful analysis of the past floods and improvements in floods forecasting.

The main objectives of flood mapping can be sorted as follows: to prevent loss of life, to minimize property damage, to minimize social disruption and to encourage coordinated approach for land/water use. The role of flood mapping in river engineering is an important feature in planning and management: basis for managing flood plains, engineering & planning tool, first step in flood plain management, part of legislation for regulating development and basis for pursuing structural and non-structural measures.

Figure 1.4 Flood in Kuala Lumpur, Malaysia, January 2008
1.2 Objectives

This research involves the integration of three models:

1. The HEC Hydrologic Modelling System (HEC-HMS3.1) as a hydrologic model to simulate rainfall-runoff process.

2. The HEC River Analysis System (HEC-RAS4.0) as a hydraulic model to route the runoff through stream channels to determine water surface profiles at specific locations along the stream network.

3. MIKE11 as a hydraulic model to develop a model for floodplain determination and representation.

Furthermore, Geography Information System (GIS) is widely used as a powerful tool toward reaching to the study objectives. For instance, in order to link the HEC-HMS, HEC-RAS and MIKE11 to GIS environment, HEC-GeoHMS, HEC-GeoRAS and MIKE11GIS are applied. The objectives of this research have been set as follows:

i. To develop rainfall-runoff modelling using HEC-GeoHMS and HEC-HMS3.1 as hydrological model for Sungai Kayu Ara river basin.

ii. To develop hydraulic modelling applying MIKE11GIS, MIKE11, HEC-GeoRAS and HEC-RAS4.0 based on the results of HEC-HMS 3.1 for Sungai Kayu Ara river basin.

iii. To compare two hydraulic models, MIKE11 and HEC-RAS4.0, in terms associated with river flood risk mapping.
iv. To establish river flood hazard mapping in Sungai Kayu Ara river basin.

v. To predict river flood risk map in Sungai Kayu Ara river basin.

Note that, hydrological modelling, hydraulic modelling, river flood mapping and river flood risk mapping will be conducted for the 20 years, 50 years and 100 years ARI flood events in existing, intermediate and ultimate river basin development conditions using rainfall events with four different durations (60 minutes, 120 minutes, 180 minutes and 360 minutes). Table 1.1 indicates the thirty six study scenarios for Sungai Kayu Ara river basin.

Table 1.1 Study Scenarios

<table>
<thead>
<tr>
<th>Development Conditions</th>
<th>Design Rainfall</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>20 year ARI</td>
</tr>
<tr>
<td>Existing</td>
<td>60, 120, 180 and 360 minutes</td>
</tr>
<tr>
<td>Intermediate</td>
<td>60, 120, 180 and 360 minutes</td>
</tr>
<tr>
<td>Ultimate</td>
<td>60, 120, 180 and 360 minutes</td>
</tr>
</tbody>
</table>

1.3 Structure of the Thesis

This thesis is divided into seven chapters. Chapter 1 includes a brief introduction, problem statement and objectives of the research. The methods for river flood risk mapping and analysis and related theories are reviewed in Chapter 2. The case study of this research will be described in Chapter 3. This chapter also gives a general methodology of the research. The detailed description of hydrological and hydraulic
models is presented in Chapters 4 and 5. Chapters 4 and 5 discuss on the introduction, methodology and development of HEC-HMS3.1 hydrologic model and HEC-RAS4.0 and MIKE11 hydraulic models for Sungai Kayu Ara river basin, respectively. Chapter 6 discusses and illustrates the generated river flood hazard mapping and river flood risk mapping for Sungai Kayu Ara river basin. Finally, chapter 7 presents the findings of the research, problems, a brief research outlook for the future and conclusions.
CHAPTER 2
LITERATURE REVIEW

2.1 River Flood

Water is a basic requirement for sustaining life and development of society. Proper management, protection and development of the water resources are challenges imposed by population growth, increasing pressure on the water and land resources by competing usage, and degradation of scarce water resources in many parts of the world.

River flood is defined as a high flow that exceeds or over-tops the capacity either the natural or the artificial banks of a stream (Hoyt and Langbein, 1958; Walesh, 1989; Knight and Shiono, 1996; Omen, et. al. 1997; Smith and Ward, 1998). Flooding results from excessive rain on the land, streams overflowing channels or unusual high tides or waves in coastal areas. Some of the most important factors that determine the features of floods are rainfall event characteristics, depth of the flood, the velocity of the flow, and duration of the rainfall event (Smith, 1996). Most floods are caused by intense precipitation combined with other factors such as: snow melt, inadequate drainage, water-saturated ground or unusually high tides or waves. As mentioned in Figure 2.1, floods are the most damaging phenomena that effect to the social and economic of the population (Smith and Ward, 1998). There are many different types of flooding. The most common types are: river floods, flash floods, coastal floods, urban floods and ice jams.
Every year, floods claim many lives and adversely affect around 75 million of people worldwide (Figures 2.2 and 2.3). The reason lies in the widespread geographical distribution of river floodplains and low-lying coasts, together with their long-standing attractions for human settlement (Ologunorisa and Abawua, 2005). Many factors cause floods. In general, the reasons for increasing flooding in many parts of the world are climatologically, changes in land-use and increasing population and land subsidence (Smith and Ward, 1998). Problems related to flooding and vulnerability of the population have greatly increased in recent decades due to several factors including changes in land-use in the hinterlands, urbanization of flood-prone sites, squatter settlements and sub-standard constructions, and increased household density (Munich, 2002; Pelling, 2003).
There is a relationship between urbanization and hydrological characteristics, such as decrease of infiltration, increase of overland flow, increase in frequency and height of flood peak, increase in range of discharge (variability) and decrease lag time. The dangers of floodwaters are associated with a number of different characteristics of
the flood such as depth of water, duration, velocity, sediment load, rate of rise and frequency of occurrence (Kingma, 2002).

Floods result from a combination of meteorological and hydrological extremes as indicated in the Table 2.1. In most cases, floods are additionally influenced by human factors. Although these influences are very diverse, they generally tend to aggravate flood hazards by accentuating river flood peaks. Thus river flood hazards in built environments have to be seen as the consequence of natural and man-made factors. The factors contributing to river flood can be categorized into three classes; meteorological factors, hydrological factors and human factors. Table 2.1 shows the factors contributing to river flood.

2.2 Risk and Hazard

Risk is widely recognized as precisely what it implies as a possibility and often referred in term of probability (ACS, 1998). Risk also can be defined as the probability of harmful consequences or expected loss (of lives, people injured, property, livelihoods, economic activity disrupted or environment damaged) resulting from interactions between natural or human-induced hazards and vulnerable conditions. Risk is an integral part of life. It is impossible to live in a risk-free environment. Risk is sometimes taken as synonymous with hazard but risk has additional implication of the chance and probability a particular hazard actually occurring (Omen et al., 1997).
Table 2.1 Factors Contributing to River Flood

<table>
<thead>
<tr>
<th>Meteorological Factors</th>
<th>Hydrological factors</th>
<th>Human factors</th>
</tr>
</thead>
<tbody>
<tr>
<td>▪ Rainfall</td>
<td>▪ Soil moisture level</td>
<td>▪ surface sealing due to urbanization, deforestation) increase run-off and may be sedimentation</td>
</tr>
<tr>
<td>▪ Cyclonic storms</td>
<td>▪ Groundwater level prior to storm</td>
<td>▪ Occupation of the flood plain obstructing flows</td>
</tr>
<tr>
<td>▪ Small-scale storms</td>
<td>▪ Natural surface infiltration rate</td>
<td>▪ Inefficiency or non-maintenance of infrastructure</td>
</tr>
<tr>
<td>▪ Temperature</td>
<td>▪ Presence of impervious cover</td>
<td>▪ Too efficient drainage of upstream areas increases flood peaks</td>
</tr>
<tr>
<td>▪ Snowfall and snowmelt</td>
<td>▪ Channel cross-sectional shape and roughness</td>
<td>▪ Climate change affects magnitude and frequency of precipitations and floods</td>
</tr>
<tr>
<td></td>
<td>▪ Presence or absence of over bank flow, channel network</td>
<td>▪ Urban microclimate may enforce precipitation events</td>
</tr>
<tr>
<td></td>
<td>▪ Synchronization of run-offs from various parts of watershed</td>
<td></td>
</tr>
<tr>
<td></td>
<td>▪ High tide impeding drainage</td>
<td></td>
</tr>
</tbody>
</table>

Hazard is defined as threatening event, or the probability of occurrence of a potentially damaging phenomenon within a given time period and area, while risk is expected losses (of lives, persons injured, property damaged, and economic activity disrupted) due to a particular hazard for a given area and reference period. In other words, risk is the actual exposure of something of human value to hazard and often regarded as the combination of probability and loss (Chow, 1958). Hazard refers to the probability of a potentially dangerous phenomenon occurring in a given location within
a specified period of time (Alexander, 1993). Risk does not exist if exposure to a harmful situation does not or will not occur (Omen et al., 1997). Thus, it can be defined that hazard (or cause) as a potential threat to humans and their welfare and risk (or consequence) as the probability of the specific hazard occurrence (Smith, 1996; Sinnakaudan et al., 2003).

Hazards include geophysical events, hydro-meteorological phenomena, and technological circumstances that relate to accidents or failures in industrial, military and energy generation activities. While some hazards can be considered to be exclusively natural in origin, the spatial and temporal patterns of hazard occurrence are increasingly correlated with patterns of human behaviour and relationship with their natural environment. Human practices such as the alteration of natural drainage, the creation of landfills, or the destruction of the natural environments and increased groundwater extraction may radically alter the pattern of the hazard behaviour (Otieno, 2004). Results of human habitation such as unplanned rapid urban development, uncontrolled logging of natural forests or major changes in land-use can influence the spatial and temporal pattern of the hazards. In this research, river flood is considered as a natural hazard and river flood mapping is conducted for river flood risk map prediction.

2.3 River Flood Modelling

River flood extent mapping is the process of determining inundation extents and depth by comparing river water levels with ground surface elevation. The process requires the understanding of flow dynamics over the flood plain, topographic
relationships and the sound judgments of the modeller (Noman et al., 2001; Sinnakaudan et al., 2003). Flood hazard maps produced may include water depth, flood extent, flow velocity and flood duration. This is a basic and important indicator for the flood plain land use development planning and regulations (Walesh, 1989).

Flood hazard mapping was first initiated in 1988 in the United States by the Hydrologic Engineering Centre (HEC) of the U.S. Army Corps of Engineers (Smith, 1996; Feldman & Owen. 1997). The purpose of the study is mainly to produce flood hazard maps for the National Insurance Program (NFIP) due to the reluctance of private insurance industry in providing insurance policies as a result of catastrophic losses (Smith, 1996).

Flood mapping uses a map to predict the probable extent of flooding; flood hazard maps can be based on known, recorded, or prehistoric events. Floodplain Mapping Process (FMP) includes gathering required data, hydrologic analysis, hydraulic analysis and floodplain mapping using output data sets and base maps. Figures 2.4 and 2.5 show river flood hazard mapping in Germany and England, respectively.

Figure 2.4 Flood Hazard Mapping in Germany (Adapted from: http://flood-risk-assessment.com/insurance-solutions.html/ accessed on 22/11/2007, 10.40 a.m.)
The boundary of a hazard area may vary according to the frequency of the flooding event, such as the 10-year, the 100-year or the 500-year flood. River flood hazard mapping is an inherently complicated process, full of uncertainties due to complexities in the hydrological/hydraulic models used, the availability and quality of data, and the subjectivity of human judgment in the process (Jones et al., 1998). Some example of vital data required for flood hazard mapping are past and previous discharge records, channel hydraulic structure, flood plain geometry and roughness, historic flood stage or area of inundation (from ground photos, aerial photos/remote sensing data and scattered discrete ground observation). The results will be more credible if they have been calibrated and compared to the actual data (Walesh, 1989).

Figure 2.5 Flood Risk Map for Part of the London, England (Adopted from: http://www.floodlondon.com/floodlt.htm/ accessed on 22/11/2007, 10.40 a.m.)

The first step in flood risk analysis is to find out what are the problems. After this has been done, the risk management is implemented through flood protection measures. Once a potential risk has been identified, it is important to know its characteristics. The knowledge about the characteristics of the flood is the base of the flood risk
managements. The assessment of these characteristics requires historical data, but as a matter of fact, in most rivers sufficient observations are not available. Therefore, to determine these values recourse must be made to some sort of predictive model. The reliable prediction of the hydrodynamics of flooding events forms an indispensable basis to fulfil risk characteristics (Stelling et al, 1998).

There are four different strategies in regarding to the reduction of flood damages. The first approach, “Keep the flood away from people”, has an objective to reduce flood effect by implementing structural and non-structural measures such as constructing dams and bunds along river and applying river basin management programs. The second approach, “Keep the people away from flood “, has an objective avoiding the use of flood prone areas. The third approach, “Accept floods and clean up afterwards” is based on the acceptance of flooding conditions as natural phenomenon and continue live in the risky areas. The final approach is the combination of the three approaches, which could be implemented based on the site-specific requirements (Petry, 2002).

2.4 River Flood Mapping

All the existing methods for flood plain mapping can be grouped into the following three major categories (Smith & Ward, 1998) namely the analytical method, the historical method and the physiographic method. All these three methods share two common steps for flood plain mapping; determination of water surface profiles and transfer of water elevation from profiles to maps. Essentially these three methods use the same procedure to delineate flood plain boundaries by determining the flood elevation at
each river cross section. The boundaries are then interpolated between the cross section. The three methods differ only in their way of determining the water surface profile. The analytical method determines a T-years surface profile by obtaining solutions to the dynamic equation to a T-year flood. The historical method involves the adjustment of water surface profiles according to historic flood. This method requires detailed historical flooding information. Predicted flood hazard zones are largely based on mathematical or statistical theory and use the historical record of the past events to estimate the future probability or recurrence of similar events. The results are expressed in terms of average probability. There are no precise indications of when any particular event may occur (Smith, 1996). The physiographic method derives a T-year water surface profile by a depth-frequency relationship and uses projection of depth as elevations on a map (Sinnakaudan et al., 2003).

Historical and physiographic approaches which are similar to DID’s modified method, may be used to get the basic idea about the river flood hazard for planning purposes, but are inadequate for detailed design and floodplain mapping for insurance rating. However there is no evidence on the provision of flood insurance schemes in Malaysia although it is considered as a possible alternative or complementary components of the overall flood proofing designs (DID, 2000). Only the analytical approach can meet the requirement of the Urban Storm-water Management Manual for Malaysia (USMM), as specified in Volume 4, Chapter 11 which requires that any new development proposals should include base flood elevation (BFE) information (DID, 2000). These three methods are labour-intensive, involving the manual interpretation of aerial photos and contour maps and full of uncertainties during the entire mapping
process. Because of the high cost incurred, flood plain maps are very difficult to update using these traditional manual methods (Sinnakaudan et al., 2003).

2.5 Computer Models

Models are required where the characteristics and, accuracy of the boundary conditions and the input data determine the outcome of the computations. Such models show the effects of different boundary conditions or input data on the results. Hence in river flood modelling by looking at different inputs (hydrological data), the behaviour (hydraulic characteristics) of the river flood risk at given instances of a period of time can be determined and investigated. The computer model simulations lead into better decision making in the management of the risks and disasters.

Nowadays, scientists and engineers take advantages from computer modelling techniques in determining river flood modelling. Computer models for the determination of river flood generally consists of four parts (Snead, 2000), these are:

i. The hydrologic model which develops rainfall-runoff from a design rainfall or historic rainfall event.

ii. The hydraulic model which routes the runoff through stream channels to determine water surface profiles (including depth and velocity) at specific locations along the stream network.

iii. The extraction of geospatial data for use in the hydrological and hydraulic models
iv. A tool for floodplain mapping and visualization.

Combination of the hydraulic series data within a spatial interface, such as a Geographical Information System (GIS) and Remote Sensing (RS), are the key to graphical visualizations on the hydraulic modelling. The increasing availability of very high performance GIS software packages such as ArcviewGIS offers new opportunities for engineers to perform flood inundation analysis in conjunction with hydraulic models with interactive visualization within immerse decision support environments (Tate, 1999; Ab. Ghani et al., 1999; ESRI, 1992, 1996, 1997 & 2001; Sinnakaudan et al., 2003.). The GIS technology has the ability to capture, store, manipulate, analyze, and visualize the diverse sets of geo-referenced data (Burrough, 1986; Aronoff, 1989; Goodchild, 1993; Sinnakaudan, 1999). On the other hand, hydraulic is inherently spatial and hydraulic models have large spatially distributed data requirements (USFEMA, 1997; Graf, 1998; Jones et al., 1998; Noman et al., 2001; Horrit & Bates, 2002). The integration of hydraulic model and GIS is therefore quite natural. The GIS allows modulation and simulation of different scenarios and the graphic representation of the different alternatives.

Nowadays the integration between GIS software and hydrological modelling software has been developed for various purposes. One of them is HEC-GeoHMS, which is an ArcviewGIS extension specially designed to process geospatial data for use with the Hydrological Engineering Center- Hydrological Modelling System (HEC-HMS). The other one is MIKE11GIS which is the linking extension between ArcviewGIS and MIKE11 hydraulic model.
2.6 Integration of River Flood Hazard Modelling and GIS

Integrating the hydraulic model outputs into a GIS environment has improved flood analysis in recent years. Numerous modelling techniques have been studied in an attempt to find an optimum combination of various methods. In an attempt to link the model outputs to a spatial interface, Djokic et. al. (1994) developed an interface between the Hydrologic Engineer Centre’s HEC-2 1-D, steady-state hydraulic model and the Arc/info spatial GIS. The interface, Known as ARC/HEC2, exports the terrain data from Arc/info into HEC-2. The ARC/HEC2 interface converts HEC-2 water surface elevations into GIS coverage in Arc/info.

In recent years, efforts have been made to integrate hydraulic models and GIS to facilitate the manipulation of the model output which led to the establishment of a new branch of hydraulics and hydrology, namely, hydro-informatics (Karimi & Houston, 1996; Yang et al., 2002). Hydro-informatics encompasses the use of advanced information technology procedures to improve the level of technology in predicting the governing processes of water science and engineering (Abbott, 1999). Traditional computational hydraulic tools which use the FORTRAN programming language running under the MS-DOS system can now be presented in more usable forms. The introduction of framework based system integrating the object-oriented methodologies in creating modelling tools using Windows Graphical User Interfaces (GUIs) may provide an understandable and highly visualized output for both the hydraulic experts and non-specialist users (Lam et al., 1996; Karimi & Houston, 1996; Pullar & Springer, 2000; Yang, et al., 2002; Huang & Jiang 2002). This concept has also alleviated the desired modularity and re-use of the existing modules in software developments (Ye et al., 1996;
A hydraulic model, like any other model, is intended to be a realistic representation of the physical processes over time in a river channel or flood plain and gives decision makers an indication of the outcome for different options (Pullar & Springer, 2000). The hydraulic model usually has the capacity to analyze, to predict and to solve engineering problems without taking into consideration the geographical prospective (McKinney & Cai, 2002). Under these circumstances, GIS becomes a valuable tool (Pullar & Springer, 2000; McKinney & Cai, 2002). Zerger (2002) notes that there are strong grounds for believing that GIS has an important function to play because natural hazards are multi-dimensional phenomena which has a spatial component.

Further more, for the past two decades many GIS integrated modelling applications have capitalized on using the GIS as a database manager and visualization tools (Westervelt & Shapiro, 2000; Karimi & Houston, 1996). Data requirements, search method, governing algorithms, flood inundation extent and depth are the main area where these procedures might need to be modified and differ from the manual flood hazard map delineation processes (Noman et al., 2001). These techniques depend on the spatial capabilities of GIS, produce consistent modelling inputs as well as continual quality control (before, during and after the modelling process) where the benefits are nearly impossible to be obtained using the spreadsheets or other non-graphic methods of data organization. Moreover, once data is available in the GIS, they can be extracted, combined with other data, reformatted as needed for various modelling processes and
even used to generate other inputs needed by the models (Robbins & Phipps, 1996). Figure 2.6 represents an application of GIS in flood mapping in United States.

The incorporation of river basin models into GIS involves three major components: (1) spatial data construction; (2) integration of spatial model layers and; (3) GIS and model interface. The integration of GIS has improved matters by streamlining data input, assist in visualization, design, calibration, modification/comparison and providing better interpretation of model outputs. There are a number of methods to integrate GIS and analytical models which will be reviewed in the next section. The link between GIS and a tailor made hydraulic model should be close enough to allow automatic data transfer, but at the same time should be loose enough to let the user replace the hydraulic model with an alternative (Noman al et., 2001). Numerous
modelling techniques for integrating environmental models with GIS have been discussed and analyzed by many researchers to find an optimum combination of various methods (Burrough, 1986; Goodchild, 1993; Karimi & Houston, 1996; McDonnell, 1996; Abbott, 1999; Alfredsen & Sather, 2000; Huang & Jiang, 2002).

Evans (1998) developed a data exchange format to transfer physical element descriptions between hydrologic and hydraulic software packages and GIS software. The package studied was HEC-RAS, with the ability to import cross-section locations as XYZ coordinates from terrain models to develop channel and reach geometry. Upon completion of the hydraulic calculations, HEC-RAS exports the data back to a GIS for comparison with the terrain model. In 1998, ESRI translated and improved Evans’ code and added some utilities to facilitate its use. The result was an ArcviewGIS extension called AVRas.

Tate (1999) further investigated how to improve upon the HEC-RAS model’s accuracy by incorporating field surveyed, stream geometry and control structures into a GIS-based terrain model (Figure 2.7). The research led to the development of Avenue scripts for ArcviewGIS that integrate such data. The terrain model Tate used for his study was based on very accurate digital orthography. Andrysiak (2000) applied Tate’s Avenue scripts to a larger study area using a digital elevation model (DEM) with 30-meter accuracy of the terrain model. When studying both cases, one can deduce that terrain model refinement is limited to the accuracy of the data. In addition, accuracy of the geo-referencing of the surveyed cross-sections and control structures is imperative in the development of an optimum terrain model.
Azagra-Camino (1999) focused on a smaller study area using more precise terrain data from the development of a Triangulated Irregular Network (TIN) in ArcviewGIS (Figure 2.8). The TIN was created from aerial photography, which resulted in a highly accurate terrain depiction of the study area. Using the AVRas extension, Azagra-Camino (1999) extracted topographic information from the TIN and imported it as channel and stream geometry for use in the HEC-RAS model. The flood visualization results provided highly accurate 2D and 3D flood maps. Azagra and Camino’s method was limited to one output in time for each run from the steady state HEC-RAS model, making the process of developing flood animations tedious. The created animations required multiple runs of the HEC-RAS model and importing the data into the TIN. Additionally, Azagra-Camino (1999) extracted the cross-section data directly from the terrain model. Since the terrain data were based on aerial photography, the cross section