

An analytical–numerical approach to simulate the dynamic behaviour of arbitrarily laminated composite plates

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Abstract

A general analytical–numerical approach developed for the dynamical analysis of unsymmetrically laminated plates of general quadrilateral planforms is presented in this work. An arbitrary quadrilateral thin flat laminate is mapped onto a square basic one, so that a unique macro-element is constructed for the whole plate. The Ritz method is applied to evaluate the governing equation in which the coupling effects of bending and stretching are contained. All possible transverse boundary conditions combining with the different in-plane constraints are considered in the analysis. The resulting algorithm possesses great flexibility, it is easy to program and it needs minimal input information. For these reasons, the proposed methodology results convenient for large scale structural design and analysis where repeated calculations are often required.

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1. Introduction

Fibre-reinforced composite laminates are very important in many engineering applications. In addition to their high strength/light-weight, another important advantage of composite laminates is that structural properties can be tailored through changing the fibre angle and/or the number and sequence of plies. Particularly, laminated plates of different shapes made of advanced fibre-reinforced composite materials have many excellent advantages and are widely used as high-performance structural components. The accurate and efficient determination of the natural vibration frequencies and mode shapes of laminated plates components are essentials to the design and performance evaluation of a mechanical system. Moreover, the plate resonant frequencies and vibration mode shapes are often

used to establish the dynamic response of complex engineering systems.

General angle-ply laminated plates exhibit various coupling effects, such as stretching–bending, stretching–shear and bending–twisting couplings, due to the anisotropy of the individual lamina and unsymmetrical layering [1]. Existence of these couplings and the general quadrilateral plate formulation are the source of analytical difficulties and of complicated mathematical structures for the boundary conditions making difficult the exact analysis for even the simplest cases. Consequently, most studies on these laminates employ approximate analytical or numerical methods.

A deep revision among the references about free vibration of thin unsymmetrically laminated composite plates reveals that most papers are concerned with rectangular plates (see e.g. [2–7]). Only limited literature can be found for unsymmetrically laminated plates of general quadrilateral shapes. In this topic, for instance, Liew and Lim [8] and Lim et al. [9] investigated the free vibration of trapezoidal multi-layered laminates with different combinations of boundary conditions using two-dimensional orthogonal

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polynomials as the trial functions in the Ritz method. For the study of free vibration of skew laminates Wang [10] used B-spline Rayleigh–Ritz method, while Krishna Reddy and Palaninathan [11] used a general high precision triangular plate bending finite element.

In general, it is observed that the Rayleigh–Ritz method has had a frequent application because of its high accuracy and relatively small computational cost, mainly related with the use of only one single super element in the whole process. The fundamental difficulty associated with the Ritz method in complex laminates is the choice of suitable functions to approximate the deflected shape. In a previous paper [12] the authors developed a general algorithm based on the Ritz method in conjunction with natural coordinates to express the geometry of laminated plates of general quadrilateral shapes in a simple form. That methodology was limited to the analysis of symmetrically laminated composite plates, where only the bending–twisting coupling needed to be included. In view of the aforementioned limitation, the present work has been undertaken to extend the mentioned algorithm to embrace laminates with unsymmetrical layering. The procedure involves the reformulation of the use of the geometric natural-to-Cartesian mapping concept in the Rayleigh–Ritz method including the effect of the bending–stretching coupling and the different transverse boundary conditions combining with four kinds of in-plane constraints. The general algorithm obtained offers much simplicity and automation in dealing with general laminated plates of several quadrilateral geometries and boundary conditions. It does not require any mesh discretization and it needs only very minimal input data for computations. Besides, the value of the analytical solution obtained here is that it allows getting insight into the behaviour on complex laminated plates.

To show the accuracy and the correctness of the presented method, convergence tests are carried out for several selected plate problems and, when it is possible, some results are compared with those published by other authors.

The algorithm developed can be applied to the analysis of a wide range of laminates with different shapes, aspect ratios, boundary conditions, number of layers, stacking sequences and angles of fibre orientation. The number of parameters is too high and the possibility of combination among them is infinite. For these reasons natural frequencies and nodal patterns of free vibration are presented for selected representative cases which can also be useful as benchmark comparison for future investigations in this topic.

2. Formulation

2.1. Energy functional components

Consider a flat, thin arbitrary-shaped quadrilateral fibre reinforced composite laminated plate of uniform thickness h as shown in Fig. 1a. The plane x – y is placed at the middle

surface of the plate thickness, while z remains normal to it. In each layer of the laminate β denotes the angle of fibre orientation and the major and minor principal material axes are denoted by L and T , respectively. In consequence, the material constants are denoted by E_L , E_T , ν_{LT} and G_{LT} .

The fundamental displacements are the three mid-surface translational displacements u , v and w along the x , y and z directions, respectively. It is necessary that the two in-plane mid-surface translational displacements u and v are included in the analysis due to the coupling between in-plane and out-of-plane behaviour in laminates with unsymmetrical layup. Assuming that the Kirchhoff hypothesis holds, the translational displacements \bar{u} , \bar{v} , \bar{w} , at a general point in the laminate are given by:

$$\begin{aligned}\bar{u}(x, y, z, t) &= u(x, y, t) - z \frac{\partial w(x, y, t)}{\partial x}; \\ \bar{v}(x, y, z, t) &= v(x, y, t) - z \frac{\partial w(x, y, t)}{\partial y}; \\ \bar{w}(x, y, z, t) &= w(x, y, t),\end{aligned}\quad (1)$$

where t is the time dimension.

During free vibration, the displacements are assumed split in the spatial and temporal parts, being the last one periodic in time; i.e.,

$$\begin{aligned}u(x, y, t) &= U(x, y) \sin \omega t, \\ v(x, y, t) &= V(x, y) \sin \omega t, \quad w(x, y, t) = W(x, y) \sin \omega t,\end{aligned}\quad (2)$$

where ω is the radian natural frequency.

Taking into account Eq. (2), the maximum strain energy of the unsymmetrically laminated plate can be written as

$$\begin{aligned}U_{\max} &= \frac{1}{2} \iint_R \left[A_{11} \left(\frac{\partial U}{\partial x} \right)^2 + 2A_{12} \frac{\partial U}{\partial x} \frac{\partial V}{\partial y} + A_{22} \left(\frac{\partial V}{\partial y} \right)^2 \right. \\ &\quad + 2A_{16} \left(\frac{\partial U}{\partial x} \frac{\partial U}{\partial y} + \frac{\partial U}{\partial x} \frac{\partial V}{\partial x} \right) \\ &\quad + 2A_{26} \left(\frac{\partial U}{\partial y} \frac{\partial V}{\partial y} + \frac{\partial V}{\partial y} \frac{\partial V}{\partial x} \right) + A_{66} \left(\frac{\partial U}{\partial y} + \frac{\partial V}{\partial x} \right)^2 \\ &\quad - 2B_{11} \frac{\partial U}{\partial x} \frac{\partial^2 W}{\partial x^2} - 2B_{12} \left(\frac{\partial U}{\partial x} \frac{\partial^2 W}{\partial y^2} + \frac{\partial V}{\partial y} \frac{\partial^2 W}{\partial x^2} \right) \\ &\quad - 2B_{22} \frac{\partial V}{\partial y} \frac{\partial^2 W}{\partial y^2} \\ &\quad - 2B_{16} \left(\frac{\partial V}{\partial x} \frac{\partial^2 W}{\partial x^2} + \frac{\partial U}{\partial y} \frac{\partial^2 W}{\partial x^2} + 2 \frac{\partial U}{\partial x} \frac{\partial^2 W}{\partial x \partial y} \right) \\ &\quad - 2B_{26} \left(\frac{\partial U}{\partial y} \frac{\partial^2 W}{\partial y^2} + \frac{\partial V}{\partial x} \frac{\partial^2 W}{\partial y^2} + 2 \frac{\partial V}{\partial y} \frac{\partial^2 W}{\partial x \partial y} \right) \\ &\quad - 4B_{66} \frac{\partial^2 W}{\partial x \partial y} \left(\frac{\partial U}{\partial y} + \frac{\partial V}{\partial x} \right) + D_{11} \left(\frac{\partial^2 W}{\partial x^2} \right)^2 \\ &\quad + 2D_{12} \frac{\partial^2 W}{\partial x^2} \frac{\partial^2 W}{\partial y^2} + D_{22} \left(\frac{\partial^2 W}{\partial y^2} \right)^2 + 4D_{16} \left(\frac{\partial^2 W}{\partial x^2} \frac{\partial^2 W}{\partial x \partial y} \right) \\ &\quad \left. + 4D_{26} \left(\frac{\partial^2 W}{\partial y^2} \frac{\partial^2 W}{\partial x \partial y} \right) + 4D_{66} \left(\frac{\partial^2 W}{\partial x \partial y} \right)^2 \right] dx dy, \quad (3)\end{aligned}$$

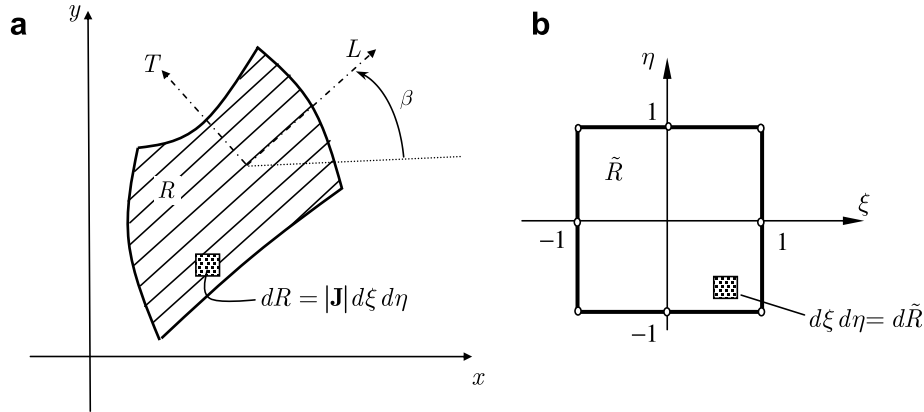


Fig. 1. (a) Geometry of a quadrilateral laminated plate in Cartesian coordinate system and (b) square parent domain in natural coordinate system.

where A_{ij} , B_{ij} and D_{ij} ($i, j = 1, 2, 6$) denote the stretching, stretching–bending coupling and bending stiffness coefficients, respectively [1,13] and R is the mid-surface area (Fig. 1a).

The maximum kinetic energy for free vibrations of the plate is given by

$$T_{\max} = \frac{\rho h \omega^2}{2} \iint_R (U^2 + V^2 + W^2) dx dy, \quad (4)$$

where ρ is the material density which is considered here to be uniform through the volume of the laminate.

2.2. Mapping technique and energy functional

Some authors have used the mapping technique, as commonly employed in finite element analysis, in conjunction with other methods to study the dynamical behaviour of plates of various geometrical shapes, see for instance Ref. [14–19]. In all of these cases isotropic plates were considered. More recently, Nallim et al. [12] combined the mapping technique and the Ritz method to derive the eigenfrequency equation of symmetrically laminated plates. This methodology is extended and generalized here to be applied to unsymmetrically laminated plates.

The geometric mapping of a curvilinear quadrilateral region in the Cartesian x – y plane (Fig. 1a) is accomplished from a square parent domain $-1 \leq \xi \leq 1$ and $-1 \leq \eta \leq 1$ in the natural ξ – η plane (Fig. 1b), using the coordinate transformation:

$$x = \sum_{i=1}^{n_p} N_i(\xi, \eta) x_i, \quad y = \sum_{i=1}^{n_p} N_i(\xi, \eta) y_i, \quad (5)$$

where (x_i, y_i) , $i = 1, \dots, n_p$ are the coordinates of n_p points on the boundary of the quadrilateral region R and $N_i(\xi, \eta)$ are the interpolation functions [20].

Applying the chain rule of differentiation it can be shown that the first and the second derivatives of a function are related by

$$\begin{aligned} \begin{bmatrix} \frac{\partial}{\partial x} \\ \frac{\partial}{\partial y} \end{bmatrix} &= \mathbf{J}^{-1} \begin{bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{bmatrix}, \quad \begin{bmatrix} \frac{\partial^2}{\partial x^2} \\ \frac{\partial^2}{\partial y^2} \\ \frac{\partial^2}{\partial x \partial y} \end{bmatrix} = \frac{1}{|\mathbf{J}|^2} [\text{Op}^{(1)}] \begin{bmatrix} \frac{\partial^2}{\partial \xi^2} \\ \frac{\partial^2}{\partial \eta^2} \\ \frac{\partial^2}{\partial \xi \partial \eta} \end{bmatrix} \\ &+ \frac{1}{|\mathbf{J}|^3} [\text{Op}^{(2)}] \begin{bmatrix} \frac{\partial}{\partial \xi} \\ \frac{\partial}{\partial \eta} \end{bmatrix}, \end{aligned} \quad (6)$$

where \mathbf{J} is the Jacobian $\partial(x, y)/\partial(\xi, \eta)$, $|\mathbf{J}|$ is the determinant of this Jacobian matrix and the elements of the matrices $[\text{Op}^{(1)}]$ and $[\text{Op}^{(2)}]$ are given in Appendix A.

The elemental area $dx dy$ in the Cartesian domain R is transformed into $|\mathbf{J}| d\xi d\eta$. Consequently, the maximum kinetic energy expression given by Eq. (4) reduce to:

$$T_{\max} = \frac{h \rho \omega^2}{2} \int_{-1}^1 \int_{-1}^1 (U^2 + V^2 + W^2) |\mathbf{J}| d\xi d\eta \quad (7)$$

where now $U = U(\xi, \eta)$, $V = V(\xi, \eta)$ and $W = W(\xi, \eta)$.

Finally, substitution of the derivatives given by Eq. (6) into Eq. (3) leads to the following expressions for the maximum strain energy

$$U_{\max} = U_{\max}^{(1)} + U_{\max}^{(2)} + U_{\max}^{(3)} \quad (8)$$

where $U_{\max}^{(1)}$ contains the terms that correspond to the in-plane effects, $U_{\max}^{(2)}$ those that correspond to the in-plane and out-of-plane coupling effects and $U_{\max}^{(3)}$ is the part of the strain energy that involves the bending and the twisting effects. Each one of these strain energies is given by the following expressions

$$\begin{aligned} U_{\max}^{(1)} &= \frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left(A_1^* \left(\frac{\partial U}{\partial \xi} \right)^2 + A_2^* \left(\frac{\partial U}{\partial \eta} \right)^2 + A_3^* \frac{\partial U}{\partial \xi} \frac{\partial U}{\partial \eta} \right. \\ &+ A_4^* \frac{\partial U}{\partial \xi} \frac{\partial V}{\partial \eta} + A_5^* \frac{\partial U}{\partial \xi} \frac{\partial V}{\partial \xi} + A_6^* \frac{\partial U}{\partial \eta} \frac{\partial V}{\partial \eta} + A_7^* \frac{\partial U}{\partial \eta} \frac{\partial V}{\partial \xi} \\ &\left. + A_8^* \left(\frac{\partial V}{\partial \xi} \right)^2 + A_9^* \left(\frac{\partial V}{\partial \eta} \right)^2 + A_{10}^* \frac{\partial V}{\partial \xi} \frac{\partial V}{\partial \eta} \right) |\mathbf{J}|^{-1} d\xi d\eta \end{aligned} \quad (9a)$$

$$\begin{aligned}
U_{\max}^{(2)} = & -\frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left(\frac{\partial U}{\partial \xi} \frac{\partial^2 W}{\partial \xi^2} B_1^* + \frac{\partial U}{\partial \xi} \frac{\partial^2 W}{\partial \eta^2} B_2^* + \frac{\partial U}{\partial \eta} \frac{\partial^2 W}{\partial \xi^2} B_3^* \right. \\
& + \frac{\partial U}{\partial \xi} \frac{\partial^2 W}{\partial \xi \partial \eta} B_4^* + \frac{\partial U}{\partial \eta} \frac{\partial^2 W}{\partial \eta^2} B_5^* + \frac{\partial U}{\partial \eta} \frac{\partial^2 W}{\partial \xi \partial \eta} B_6^* \\
& + \frac{\partial V}{\partial \xi} \frac{\partial^2 W}{\partial \xi^2} B_{11}^* + \frac{\partial V}{\partial \xi} \frac{\partial^2 W}{\partial \eta^2} B_{12}^* + \frac{\partial V}{\partial \eta} \frac{\partial^2 W}{\partial \xi^2} B_{13}^* \\
& \left. + \frac{\partial V}{\partial \xi} \frac{\partial^2 W}{\partial \xi \partial \eta} B_{14}^* + \frac{\partial V}{\partial \eta} \frac{\partial^2 W}{\partial \eta^2} B_{15}^* + \frac{\partial V}{\partial \eta} \frac{\partial^2 W}{\partial \xi \partial \eta} B_{16}^* \right) \\
& \times |\mathbf{J}|^{-2} d\xi d\eta - \frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left(\frac{\partial U}{\partial \xi} \frac{\partial W}{\partial \xi} B_7^* + \frac{\partial U}{\partial \xi} \frac{\partial W}{\partial \eta} B_8^* \right. \\
& + \frac{\partial U}{\partial \eta} \frac{\partial W}{\partial \xi} B_9^* + \frac{\partial U}{\partial \eta} \frac{\partial W}{\partial \eta} B_{10}^* + \frac{\partial V}{\partial \xi} \frac{\partial W}{\partial \xi} B_{17}^* + \frac{\partial V}{\partial \xi} \frac{\partial W}{\partial \eta} B_{18}^* \\
& \left. + \frac{\partial V}{\partial \eta} \frac{\partial W}{\partial \xi} B_{19}^* + \frac{\partial V}{\partial \eta} \frac{\partial W}{\partial \eta} B_{20}^* \right) |\mathbf{J}|^{-3} d\xi d\eta \quad (9b)
\end{aligned}$$

$$\begin{aligned}
U_{\max}^{(3)} = & \frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left(\left(\frac{\partial^2 W}{\partial \xi^2} \right)^2 D_1^* + \left(\frac{\partial^2 W}{\partial \eta^2} \right)^2 D_2^* + \frac{\partial^2 W}{\partial \xi^2} \frac{\partial^2 W}{\partial \eta^2} D_3^* \right. \\
& + \left. \left(\frac{\partial^2 W}{\partial \xi \partial \eta} \right)^2 D_4^* + \frac{\partial^2 W}{\partial \xi^2} \frac{\partial^2 W}{\partial \xi \partial \eta} D_5^* + \frac{\partial^2 W}{\partial \eta^2} \frac{\partial^2 W}{\partial \xi \partial \eta} D_6^* \right) |\mathbf{J}|^{-3} d\xi d\eta \\
& + \frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left(\frac{\partial^2 W}{\partial \xi^2} \frac{\partial W}{\partial \xi} D_7^* + \frac{\partial^2 W}{\partial \eta^2} \frac{\partial W}{\partial \eta} D_8^* + \frac{\partial^2 W}{\partial \xi^2} \frac{\partial W}{\partial \eta} D_9^* \right. \\
& + \left. \frac{\partial^2 W}{\partial \eta^2} \frac{\partial W}{\partial \xi} D_{10}^* + \frac{\partial^2 W}{\partial \xi \partial \eta} \frac{\partial W}{\partial \xi} D_{11}^* + \frac{\partial^2 W}{\partial \xi \partial \eta} \frac{\partial W}{\partial \eta} D_{12}^* \right) \\
& \times |\mathbf{J}|^{-4} d\xi d\eta + \frac{1}{2} \int_{-1}^1 \int_{-1}^1 \left(\left(\frac{\partial W}{\partial \xi} \right)^2 D_{13}^* + \left(\frac{\partial W}{\partial \eta} \right)^2 D_{14}^* \right. \\
& \left. + \frac{\partial W}{\partial \xi} \frac{\partial W}{\partial \eta} D_{15}^* \right) |\mathbf{J}|^{-5} d\xi d\eta \quad (9c)
\end{aligned}$$

where $A_i^* = A_i^*(\xi, \eta)$, ($i = 1, \dots, 10$), $B_j^* = B_j^*(\xi, \eta)$, ($j = 1, \dots, 20$) and $D_k^* = D_k^*(\xi, \eta)$, ($k = 1, \dots, 15$) are functions that depend on the problem parameters, i.e., plate geometry and material properties, and are defined in Appendix B.

The energy functional for free vibration of the plate is given by:

$$F = U_{\max} - T_{\max}, \quad (10)$$

which is to be minimized according to the Ritz principle, as will be discussed in Section 3.

2.3. Boundary conditions and approximating functions

The situation for unsymmetrically laminated plates is more complex than for symmetric ones because the transverse and in-plane vibrations are coupled. Actually, there are four types of boundary conditions that can be called simply supported (S), clamped (C) or free edges (F) [21]. The various combinations of these constraints are summarized in Table 1.

In the application of the Ritz method only the essential boundary conditions are required to be satisfied by the assumed functions [22]. The fact that the natural boundary conditions need not be satisfied by the chosen coordinate functions is a very important characteristic of the Ritz

method, specially when dealing with problems for which these satisfaction is very difficult to achieve [23,24].

The use of beam orthogonal polynomials to study anisotropic plates is very satisfactory, as has been demonstrated through some works [12,25,26] since the procedure has a quick convergence to the solution and takes place practically without oscillations. For this reason, in the present paper the three field displacement components are expressed in terms of the natural coordinates system by sets of beam characteristic orthogonal polynomials, $\{p_i^{(u)}(\xi)\}$, $\{q_j^{(u)}(\eta)\}$, $\{p_i^{(v)}(\xi)\}$, $\{q_j^{(v)}(\eta)\}$ and $\{q_j^{(w)}(\eta)\}$, as

$$U(\xi, \eta) \approx U_{MN}(\xi, \eta) = \sum_{i=1}^M \sum_{j=1}^N c_{ij}^{(u)} p_i^{(u)}(\xi) q_j^{(u)}(\eta), \quad (11a)$$

$$V(\xi, \eta) \approx V_{MN}(\xi, \eta) = \sum_{i=1}^M \sum_{j=1}^N c_{ij}^{(v)} p_i^{(v)}(\xi) q_j^{(v)}(\eta), \quad (11b)$$

$$W(\xi, \eta) \approx W_{MN}(\xi, \eta) = \sum_{i=1}^M \sum_{j=1}^N c_{ij}^{(w)} p_i^{(w)}(\xi) q_j^{(w)}(\eta), \quad (11c)$$

where $c_{ij}^{(u)}$, $c_{ij}^{(v)}$ and $c_{ij}^{(w)}$ are the unknown coefficients, and M and N are the numbers of polynomials in each natural coordinate.

The procedure for the construction of the orthogonal polynomials has been developed by Bhat [27,28]. The first members of the sets, $p_1^{(\cdot)}(\xi)$ and $q_1^{(\cdot)}(\eta)$ ($\cdot = u, v, w$) are obtained as the simplest polynomials that satisfy all the geometrical boundary conditions of the plate in their respective ξ and η -directions. The higher members of each set are constructed by employing the Gram–Schmidt orthogonalization procedure. The coefficients of the polynomials are chosen in such a way as to make the polynomials orthonormal.

It is important to point out that working with the master element in natural coordinates allows us to use the same sets of orthogonal polynomials for plates of different shapes. This fact makes possible a unified treatment.

3. Application of the Ritz method for the study of free vibration

The Ritz method is applied to determine analytical approximate solutions for general laminated plates of different shapes. For the dynamical analysis the Ritz procedure requires the minimization of the energy functional (Eq. (10)) with respect to each of the $c_{ij}^{(u)}$, $c_{ij}^{(v)}$ and $c_{ij}^{(w)}$ coefficients

$$\frac{\partial F}{\partial c_{ij}^{(u)}} = 0, \quad \frac{\partial F}{\partial c_{ij}^{(v)}} = 0, \quad \frac{\partial F}{\partial c_{ij}^{(w)}} = 0, \quad (i, j = 1, \dots, M, N) \quad (12)$$

Substituting Eqs. (11a)–(11c) into the expressions for U_{\max} (Eq. (8)) and T_{\max} (Eq. (7)), and subsequently applying Eq. (12) result in the following governing eigenvalue equation:

Table 1

Notations for various combinations of boundary conditions, in which n and s indicate the directions normal and tangential to the respective plate edges

Transverse boundary supports	In-plane constraints			
	$u_n = 0, u_s = 0$	$N_n = 0, u_s = 0$	$u_n = 0, N_{ns} = 0$	$N_n = 0, N_{ns} = 0$
Clamped: $w = 0; \partial w / \partial n = 0$	C ₁	C ₂	C ₃	C ₄
Simply supported: $w = 0; M_n = 0$	S ₁	S ₂	S ₃	S ₄
Free: $M_n = 0; \partial M_{ns} / \partial s + Q_n = 0$	F ₁	F ₂	F ₃	F ₄

$$(\mathbf{K} - \omega^2 \mathbf{M})\{\mathbf{C}\} = \{\mathbf{0}\}. \tag{13}$$

In Eq. (13), \mathbf{K} is the stiffness matrix, \mathbf{M} is the mass matrix and $\{\mathbf{C}\}$ is the unknown coefficient vector, and are respectively given by

$$\mathbf{K} = \begin{bmatrix} [K^{uu}] & [K^{uv}] & [K^{uw}] \\ & [K^{vv}] & [K^{vw}] \\ \text{symm} & & [K^{ww}] \end{bmatrix} \tag{14a}$$

$$\mathbf{M} = \begin{bmatrix} [M^{uu}] & [0] & [0] \\ & [M^{vv}] & [0] \\ \text{symm} & & [M^{ww}] \end{bmatrix} \tag{14b}$$

$$\{\mathbf{C}\} = \begin{Bmatrix} \{c^{(u)}\} \\ \{c^{(v)}\} \\ \{c^{(w)}\} \end{Bmatrix} \tag{14c}$$

where the elements of these matrices are,

$$K_{ijmn}^{uu} = \int_{-1}^1 \int_{-1}^1 (2(A_1^* P_{im}^{uu11} Q_{jn}^{uu00} + A_2^* P_{im}^{uu00} Q_{jn}^{uu11}) + A_3^* (P_{im}^{uu01} Q_{jn}^{uu10} + P_{im}^{uu10} Q_{jn}^{uu01})) |\mathbf{J}|^{-1} d\xi d\eta,$$

$$K_{ijmn}^{uv} = \int_{-1}^1 \int_{-1}^1 (A_4^* P_{im}^{uv10} Q_{jn}^{uv01} + A_5^* P_{im}^{uv11} Q_{jn}^{uv00} + A_6^* P_{im}^{uv00} Q_{jn}^{uv11} + A_7^* P_{im}^{uv01} Q_{jn}^{uv10}) |\mathbf{J}|^{-1} d\xi d\eta,$$

$$K_{ijmn}^{uw} = - \int_{-1}^1 \int_{-1}^1 (B_1^* P_{im}^{uw12} Q_{jn}^{uw00} + B_2^* P_{im}^{uw10} Q_{jn}^{uw02} + B_3^* P_{im}^{uw02} Q_{jn}^{uw10} + B_4^* P_{im}^{uw11} Q_{jn}^{uw01} + B_5^* P_{im}^{uw00} Q_{jn}^{uw12} + B_6^* P_{im}^{uw01} Q_{jn}^{uw11}) |\mathbf{J}|^{-2} d\xi d\eta - \int_{-1}^1 \int_{-1}^1 (B_7^* P_{im}^{uw11} Q_{jn}^{uw00} + B_8^* P_{im}^{uw10} Q_{jn}^{uw01} + B_9^* P_{im}^{uw01} Q_{jn}^{uw10} + B_{10}^* P_{im}^{uw00} Q_{jn}^{uw11}) |\mathbf{J}|^{-3} d\xi d\eta,$$

$$K_{ijmn}^{vv} = \int_{-1}^1 \int_{-1}^1 (2(A_8^* P_{im}^{vv11} Q_{jn}^{vv00} + A_9^* P_{im}^{vv00} Q_{jn}^{vv11}) + A_{10}^* (P_{im}^{vv10} Q_{jn}^{vv01} + P_{im}^{vv01} Q_{jn}^{vv10})) |\mathbf{J}|^{-1} d\xi d\eta,$$

$$K_{ijmn}^{vw} = - \int_{-1}^1 \int_{-1}^1 (B_{11}^* P_{im}^{vw12} Q_{jn}^{vw00} + B_{12}^* P_{im}^{vw10} Q_{jn}^{vw02} + B_{13}^* P_{im}^{vw02} Q_{jn}^{vw10} + B_{14}^* P_{im}^{vw11} Q_{jn}^{vw01} + B_{15}^* P_{im}^{vw00} Q_{jn}^{vw12} + B_{16}^* P_{im}^{vw01} Q_{jn}^{vw11}) |\mathbf{J}|^{-2} d\xi d\eta - \int_{-1}^1 \int_{-1}^1 (B_{17}^* P_{im}^{vw11} Q_{jn}^{vw00} + B_{18}^* P_{im}^{vw10} Q_{jn}^{vw01} + B_{19}^* P_{im}^{vw01} Q_{jn}^{vw10} + B_{20}^* P_{im}^{vw00} Q_{jn}^{vw11}) |\mathbf{J}|^{-3} d\xi d\eta,$$

$$K_{ijmn}^{ww} = \int_{-1}^1 \int_{-1}^1 (2D_1^* P_{im}^{ww22} Q_{jn}^{ww00} + 2D_2^* P_{im}^{ww00} Q_{jn}^{ww22} + D_3^* (P_{im}^{ww02} Q_{jn}^{ww20} + P_{im}^{ww20} Q_{jn}^{ww02}) + 2D_4^* P_{im}^{ww11} Q_{jn}^{ww11} + D_5^* (P_{im}^{ww12} Q_{jn}^{ww10} + P_{im}^{ww21} Q_{jn}^{ww01}) + D_6^* (P_{im}^{ww10} Q_{jn}^{ww12} + P_{im}^{ww01} Q_{jn}^{ww21})) |\mathbf{J}|^{-3} d\xi d\eta + \int_{-1}^1 \int_{-1}^1 (D_7^* Q_{jn}^{ww00} (P_{im}^{ww12} + P_{im}^{ww21}) + D_8^* P_{im}^{ww00} (Q_{jn}^{ww12} + Q_{jn}^{ww21}) + D_9^* (P_{im}^{ww02} Q_{jn}^{ww10} + P_{im}^{ww20} Q_{jn}^{ww01}) + D_{10}^* (P_{im}^{ww10} Q_{jn}^{ww02} + P_{im}^{ww01} Q_{jn}^{ww20}) + D_{11}^* P_{im}^{ww11} (Q_{jn}^{ww01} + Q_{jn}^{ww10}) + D_{12}^* Q_{jn}^{ww11} (P_{im}^{ww01} + P_{im}^{ww10})) |\mathbf{J}|^{-4} d\xi d\eta + \int_{-1}^1 \int_{-1}^1 (2D_{13}^* P_{im}^{ww11} Q_{jn}^{ww00} + 2D_{14}^* P_{im}^{ww00} Q_{jn}^{ww11} + D_{15}^* (P_{im}^{ww01} Q_{jn}^{ww10} + P_{im}^{ww10} Q_{jn}^{ww01})) |\mathbf{J}|^{-5} d\xi d\eta,$$

$$M_{ijmn}^{uu} = 2 \int_{-1}^1 \int_{-1}^1 P_{im}^{uu00} Q_{jn}^{uu00} |\mathbf{J}| d\xi d\eta,$$

$$M_{ijmn}^{vv} = 2 \int_{-1}^1 \int_{-1}^1 P_{im}^{vv00} Q_{jn}^{vv00} |\mathbf{J}| d\xi d\eta,$$

$$M_{ijmn}^{ww} = 2 \int_{-1}^1 \int_{-1}^1 P_{im}^{ww00} Q_{jn}^{ww00} |\mathbf{J}| d\xi d\eta;$$

$$i, m = 1, \dots, M; \quad j, n = 1, \dots, N,$$

in which

$$P_{im}^{uurs} = \frac{d^r p_i^{(u)}}{d\xi^r} \frac{d^s p_m^{(u)}}{d\xi^s}, \quad Q_{jn}^{uurs} = \frac{d^r q_j^{(u)}}{d\eta^r} \frac{d^s q_n^{(u)}}{d\eta^s},$$

$$P_{im}^{uvrs} = \frac{d^r p_i^{(u)}}{d\xi^r} \frac{d^s p_m^{(v)}}{d\xi^s}, \quad Q_{jn}^{uvrs} = \frac{d^r q_j^{(u)}}{d\eta^r} \frac{d^s q_n^{(v)}}{d\eta^s},$$

$$P_{im}^{uwrs} = \frac{d^r p_i^{(u)}}{d\xi^r} \frac{d^s p_m^{(w)}}{d\xi^s}, \quad Q_{jn}^{uwrs} = \frac{d^r q_j^{(u)}}{d\eta^r} \frac{d^s q_n^{(w)}}{d\eta^s},$$

$$P_{im}^{vurs} = \frac{d^r p_i^{(v)}}{d\xi^r} \frac{d^s p_m^{(v)}}{d\xi^s}, \quad Q_{jn}^{vurs} = \frac{d^r q_j^{(v)}}{d\eta^r} \frac{d^s q_n^{(v)}}{d\eta^s},$$

$$P_{im}^{vwrs} = \frac{d^r p_i^{(v)}}{d\xi^r} \frac{d^s p_m^{(w)}}{d\xi^s}, \quad Q_{jn}^{vwrs} = \frac{d^r q_j^{(v)}}{d\eta^r} \frac{d^s q_n^{(w)}}{d\eta^s},$$

$$P_{im}^{wurs} = \frac{d^r p_i^{(w)}}{d\xi^r} \frac{d^s p_m^{(w)}}{d\xi^s}, \quad Q_{jn}^{wurs} = \frac{d^r q_j^{(w)}}{d\eta^r} \frac{d^s q_n^{(w)}}{d\eta^s},$$

$$r, s = 0, 1, 2$$

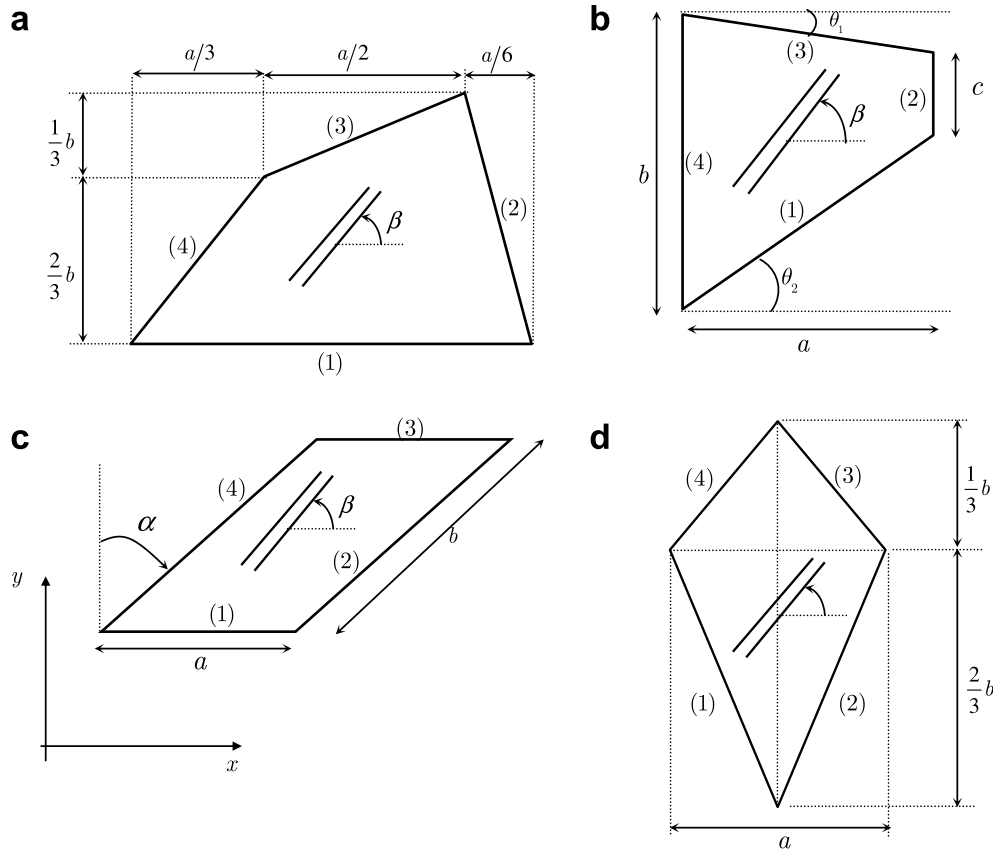


Fig. 2. Laminated plates of (a) general quadrilateral planform, (b) trapezoidal planform, (c) skew planform and (d) rhomboidal planform.

Eq. (13) yields an eigenvalue determinant, whose zeros give the natural frequencies of the plate. Back substitution yields the coefficient vectors; and finally substitution of these coefficient vectors into Eq. (11) gives the corresponding free vibration mode shapes.

4. Verification of the formulation and its numerical applications

4.1. General description

The variational algorithm developed in this paper was programmed and used for the free vibration analysis of unsymmetrically laminated thin plates having different shapes, geometric parameters, stacking sequences, material properties and boundary conditions. The examples considered in this study are confined to laminates with layers of equal thickness, even though the procedure was formulated for plies with arbitrary thickness.

Let us introduce the terminology to be used throughout the remainder of the paper for describing the boundary conditions of the considered plates. The designation $C_i S_i$, $F_i S_i$, for example, identifies a plate with edges (1) clamped, (2) simply supported, (3) free and (4) simply supported (see Fig. 2), the subscript i ($i = 1, \dots, 4$) identifies the in-plane constraints according to Table 1. The reference flexural stiffness is $D_\beta = E_L h^3 / 12(1 - \nu_{LT} \nu_{TL})$ and the results are

presented for a representative high modulus graphite/epoxy material having each laminae elastic properties $E_L/E_T = 40$; $G_{LT}/E_T = 0.5$ and $\nu_{LT} = 0.25$.

The main purposes of the numerical applications presented in this section are twofold. One is to demonstrate the accuracy, the flexibility and the efficiency of the proposed method and the other is to produce some results which may be regarded as benchmark solutions for other academic research workers and design engineers.

4.2. Validation and convergence studies

In order to evaluate the accuracy and reliability of the present method, comparison and convergence studies are carried out in this section. Convergence studies have been undertaken for simply supported ($S_2 S_2 S_2 S_2$), fully clamped ($C_1 C_1 C_1 C_1$), cantilever ($F_4 F_4 F_4 C_1$) and for $C_3 S_3 C_3 S_3$ general quadrilateral laminated plates with $a/b = 2$ (Fig. 2a). Two layer laminates (in this case in-plane stretching effects are more important) with a width-thickness $a/h = 1000$ are considered. Results for antisymmetric angle-ply ($30^\circ/-30^\circ$) laminated plates are given in Table 2. The number of polynomials in each natural coordinate for u , v and w are stepped steadily from 6 to 12 to demonstrate the downward convergence of the first eight no dimensional frequency parameters $\Omega_i = \omega_i a^2 \sqrt{\rho h / D_\beta}$. It can be seen that the eigenfrequencies converge monotonically from above as

Table 2

Convergence of frequency parameters $\Omega = \omega a^2 \sqrt{\rho h / D_\beta}$ for general quadrilateral angle-ply ($30^\circ/-30^\circ$) graphite/epoxy plates ($a/b = 2$, $h/a = 0.001$)

$M \times N$	Ω_1	Ω_2	Ω_3	Ω_4	Ω_5	Ω_6	Ω_7	Ω_8
S₂S₂S₂S₂								
6 × 6	31.327	54.395	70.445	97.240	115.548	137.559	147.174	181.248
7 × 7	31.266	54.255	70.006	94.603	114.142	135.206	142.983	169.465
8 × 8	31.213	54.171	69.842	94.330	113.360	134.373	139.487	165.261
9 × 9	31.173	54.134	69.715	94.147	113.219	134.611	138.865	163.449
10 × 10	31.140	54.104	69.613	94.128	113.088	134.537	138.519	163.183
11 × 11	31.112	54.080	69.528	94.117	113.014	134.505	138.500	162.930
12 × 12	31.089	54.061	69.457	94.110	112.954	134.482	138.485	162.906
C₁C₁C₁C₁								
6 × 6	45.505	79.021	97.883	124.339	146.878	171.814	184.440	217.912
7 × 7	45.392	79.490	97.121	123.365	145.582	171.265	178.549	209.197
8 × 8	45.344	79.397	97.951	123.252	145.948	170.587	177.495	206.356
9 × 9	45.320	79.369	97.880	123.945	145.182	170.082	176.846	205.641
10 × 10	45.305	79.353	97.844	123.911	145.995	170.953	176.044	204.852
11 × 11	45.297	79.344	97.826	123.903	145.924	170.910	176.849	204.236
12 × 12	45.292	79.338	97.815	123.901	145.886	170.892	176.822	204.076
F₄F₄F₄C₁								
6 × 6	2.8963	9.1009	15.993	27.095	36.035	45.279	60.738	70.460
7 × 7	2.8857	8.9957	15.912	26.816	35.721	44.852	60.172	68.407
8 × 8	2.8770	8.9385	15.866	26.774	35.610	44.691	59.062	67.620
9 × 9	2.8709	8.9030	15.841	26.748	35.549	44.604	59.018	67.533
10 × 10	2.8661	8.8792	15.825	26.730	35.510	44.555	58.982	67.502
11 × 11	2.8623	8.8631	15.815	26.717	35.484	44.522	58.965	67.482
12 × 12	2.8591	8.8517	15.807	26.706	35.466	44.500	58.953	67.469
C₃S₃C₃S₃								
6 × 6	38.701	64.461	91.873	104.481	134.183	166.778	168.978	207.459
7 × 7	38.627	64.252	91.006	102.484	130.632	155.746	166.459	189.537
8 × 8	38.577	64.150	90.961	101.703	129.835	152.688	163.003	181.470
9 × 9	38.541	64.089	90.924	101.592	129.367	150.685	162.879	179.664
10 × 10	38.515	64.046	90.908	101.570	129.228	150.424	162.722	178.213
11 × 11	38.496	64.014	90.896	101.562	129.128	150.315	162.709	178.067
12 × 12	38.481	63.990	90.887	101.558	129.056	150.299	162.703	177.940

the increase of the number of terms of the trial functions. In general, 10–12 terms of the trial functions can give sufficiently satisfactory results for first eight eigenfrequencies.

The accuracy and reliability of the results obtained with the present approach are next demonstrated by comparing them with some selected values published by other researchers for unsymmetrically laminated trapezoidal and skew plates.

The comparison presented in Table 3, authenticates the validity of the present method for antisymmetric laminated plates with symmetric trapezoidal planform $\theta_1 = \theta_2$ (see Fig. 2b). The first eight non-dimensional frequencies $\omega a^2 \sqrt{\rho h / D_\beta}$ for F₄F₄F₄C₁ two-ply ($30^\circ/-30^\circ$) and four-ply ($30^\circ/-30^\circ/30^\circ/-30^\circ$) antisymmetric laminated graphite-epoxy plates are compared with those of Liew and Lim [8], and very good agreement is obtained.

The second example verifies the accuracy of the eigenvalues for thin skew fibre reinforced antisymmetric angle-ply ($45^\circ/-45^\circ/45^\circ/-45^\circ$) plates and cross-ply ($90^\circ/0^\circ/90^\circ/0^\circ$) plates. The plate geometry is defined by means of a , b and α as shown in Fig. 2c. The material properties of each lamina correspond to graphite/epoxy and two skew angles, i.e., $\alpha = 30^\circ$, 45° are used for the comparisons. The first

eight non-dimensional frequencies $\hat{\Omega}_i = (\omega_i a^2 / h \pi^2) \sqrt{\rho / E_T}$ obtained with the present approach for fully clamped (C₁C₁C₁C₁), are compared with the solutions of Wang [10] in Table 4, and excellent agreement is achieved between both solutions.

From the convergence analysis and the comparisons performed it is clear that M , $N = 10$ produces no drastic change in the solutions compared with M , $N = 11 - 12$. Therefore, in the next section it was decided to use M , $N = 10$ to generate results with sufficient accuracy from an engineering viewpoint.

4.3. Numerical results and discussion

The developed Ritz formulation is applied in this section to obtain the natural frequencies of free vibration and modal shapes of general rhomboidal laminated plates as shown in Fig. 2d. The planform geometries of these plates are defined by means of the aspect ratio b/a , while boundaries having different combinations of in-plane and/or transverse constraints are analyzed. The non-dimensional frequency parameter is given by $\Omega_i = \omega_i a^2 \sqrt{\rho h / D_\beta}$, and results are presented for the representative high modulus

Table 3
Frequency parameters $\Omega = \omega a^2 \sqrt{\rho h / D_\beta}$ for cantilever (F₄F₄F₄C₁) trapezoidal laminated graphite/epoxy plates ($a/b = 1$, $\theta_1 = \theta_2$, $h/a = 0.001$)

c/b	Sources	Ω_1	Ω_2	Ω_3	Ω_4	Ω_5	Ω_6	Ω_7	Ω_8
(30°/–30°)									
0.25	Present (10 × 10)	2.0616	8.5508	9.7945	22.189	23.319	25.712	40.875	44.525
	Present (11 × 11)	2.0616	8.5507	9.7943	22.188	23.318	25.711	40.872	44.524
	Ref. [8]	2.0616	8.5506	9.7942	22.187	23.318	25.710	40.870	44.523
0.50	Present (10 × 10)	1.7646	6.1301	9.2878	16.288	17.378	25.533	32.120	33.098
	Present (11 × 11)	1.7646	6.1298	9.2873	16.288	17.377	25.529	32.118	33.098
	Ref. [8]	1.7645	6.1297	9.2871	16.288	17.376	25.528	32.118	33.097
0.75	Present (10 × 10)	1.5955	4.6155	8.8041	11.516	14.701	22.791	24.550	26.828
	Present (11 × 11)	1.5954	4.6151	8.8018	11.515	14.700	22.791	24.539	26.826
	Ref. [8]	1.5954	4.6153	8.8028	11.516	14.701	22.791	24.543	26.827
1.0	Present (10 × 10)	1.4803	3.6343	7.4610	9.5804	12.898	15.907	21.513	25.532
	Present (11 × 11)	1.4800	3.6339	7.4574	9.5792	12.897	15.907	21.507	25.520
	Ref. [8]	1.4803	3.6351	7.4611	9.5805	12.901	15.907	21.513	25.524
(30°/–30°/30°/–30°)									
0.25	Present (10 × 10)	3.2045	13.123	14.897	33.815	33.821	39.900	62.762	64.676
	Present (11 × 11)	3.2044	13.123	14.896	33.815	33.819	39.897	62.757	64.676
	Ref. [8]	3.2044	13.123	14.896	33.815	33.819	39.896	62.754	64.673
0.50	Present (10 × 10)	2.7513	9.4506	14.142	23.701	26.434	39.829	46.550	48.559
	Present (11 × 11)	2.7512	9.4503	14.141	23.701	26.432	39.816	46.550	48.551
	Ref. [8]	2.7512	9.4502	14.141	23.701	26.432	39.815	46.549	48.551
0.75	Present (10 × 10)	2.4896	7.1546	13.200	17.319	22.197	31.929	37.994	41.241
	Present (11 × 11)	2.4895	7.1541	13.192	17.318	22.195	31.929	37.962	41.229
	Ref. [8]	2.4895	7.1545	13.195	17.319	22.196	31.929	37.971	41.229
1.0	Present (10 × 10)	2.3083	5.6722	10.961	14.882	18.916	23.060	33.168	36.314
	Present (11 × 11)	2.3077	5.6716	10.950	14.878	18.914	23.059	33.150	36.269
	Ref. [8]	2.3083	5.6740	10.959	14.881	18.922	23.060	33.161	36.274

Table 4
Frequency parameters $\hat{\Omega} = \omega a^2 / (\pi^2 h) \sqrt{\rho / E_T}$ for fully clamped (C₁C₁C₁C₁) skew angle-ply and cross-ply graphite/epoxy plates ($a/b = 1$, $h/a = 0.001$)

α	Sources	$\hat{\Omega}_1$	$\hat{\Omega}_2$	$\hat{\Omega}_3$	$\hat{\Omega}_4$	$\hat{\Omega}_5$	$\hat{\Omega}_6$	$\hat{\Omega}_7$	$\hat{\Omega}_8$
(45°/–45°/45°/–45°)									
30°	Present (10 × 10)	4.8793	8.3836	11.123	12.563	16.341	17.469	20.082	22.503
	Present (11 × 11)	4.8793	8.3836	11.123	12.563	16.341	17.469	20.081	22.502
	Ref. [10]	4.8889	8.4053	11.1461	12.5901	16.3995	17.5057	20.1206	22.5803
45°	Present (10 × 10)	6.9426	11.052	15.615	16.637	20.828	22.808	26.845	29.697
	Present (11 × 11)	6.9424	11.052	15.615	16.636	20.828	22.807	26.084	29.686
	Ref. [10]	6.9564	11.0782	15.6482	16.6786	20.8790	22.9147	26.9540	30.0230
(90°/0°/90°/0°)									
30°	Present (10 × 10)	4.8604	8.6618	11.102	12.512	17.091	17.290	20.270	22.656
	Present (11 × 11)	4.8604	8.6618	11.102	12.511	17.091	17.290	20.270	22.656
	Ref. [10]	4.8701	8.6801	11.1241	12.5388	17.1424	17.3294	20.2977	22.7235
45°	Present (10 × 10)	6.9429	11.051	15.615	16.640	20.827	22.806	26.844	29.697
	Present (11 × 11)	6.9427	11.051	15.615	16.638	20.827	22.805	26.842	29.686
	Ref. [10]	6.9564	11.0782	15.6482	16.6786	20.8790	22.9147	26.9540	30.0230

graphite/epoxy material. To have better insight about the effect of different fibre orientation angle (β) and number of layers (N_c) on the dynamic properties of these general rhomboidal ($b/a = 1$) antisymmetric angle-ply laminated plates, the variations of the first two free vibration coefficients (Ω_1 and Ω_2) are plotted in Figs. 3 and 4. Two boundary conditions have been included, S₃S₃S₃S₃ in Fig. 3 and

F₄F₄F₄C₁ in Fig. 4. Many others aspect ratios and boundary condition have been studied, but the corresponding results are not included for brevity purposes. In general, it is observed that for all analyzed boundary conditions and aspect ratios, the first two frequency coefficients are minimums for the number of layers (N_c) equal to 2. When $N_c \geq 4$, the Ω_i ($i = 1, 2$) values are considerably different

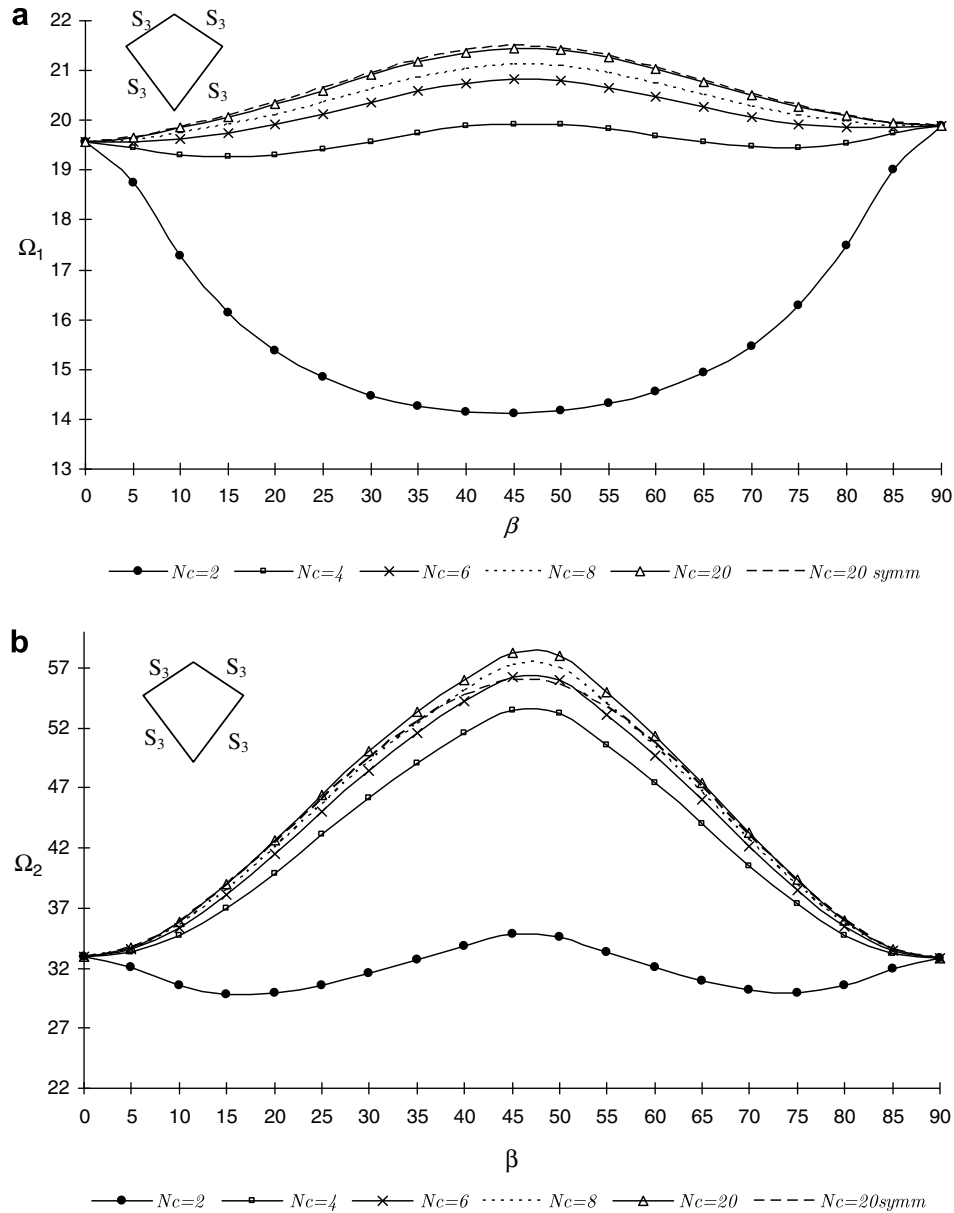


Fig. 3. Effect of fibre orientation and lamination sequence on the first two vibration frequency coefficients for $S_3S_3S_3S_3$ graphite/epoxy rhomboidal plates with $b/a = 1$. (a) $\Omega_1 = \omega_1 a^2 \sqrt{\rho h / D_\beta}$ versus β , (b) $\Omega_2 = \omega_2 a^2 \sqrt{\rho h / D_\beta}$ versus β .

from those of $N_c = 2$. By keeping the total thickness constant, if the number of layers is increased, the frequency coefficients increase. This is due to the reduction in the coupling stiffness terms B_{16} and B_{26} . Results for $N_c = 20$ symmetric lamination were also included in Figs. 3 and 4 and it is observed that, even for many layers, the influence of the stacking sequences is meaningful. Furthermore, it is important to point out that the way of variation of the fundamental frequency is rather different from the way in which varies the frequency that corresponds to the second mode of vibration. This situation occurs for all analyzed boundary conditions and should be specially kept in mind when specific requirements of design that involve at the first or the second vibration modes exist.

Finally, Figs. 5 and 6 show the first five nondimensional free vibration frequencies $\omega a^2 \sqrt{\rho h / D_\beta}$ of arbitrarily laminated graphite/epoxy rhomboidal plates and their corresponding nodal patterns ($b/a = 0.5$ in Fig. 5, $b/a = 1$ in Fig. 6). Three boundary conditions i.e. $S_1S_1S_1S_1$, $C_1F_4F_4F_4$, $C_2S_2C_2S_2$ and three stacking sequences i.e. $(45^\circ/-45^\circ)$, $(30^\circ/-30^\circ)$, $(0^\circ/45^\circ)$ are considered in each figure. It is shown that the variation in the frequency coefficients is noticeable for the different stacking sequences, especially between $(0^\circ/45^\circ)$ laminated plates and the anti-symmetric angle-ply ones. In arbitrarily laminated plates all the stretching–bending couplings $B_{ij}(i, j = 1, 2, 6)$ are different from zero and it is observed that the frequency parameters are higher in these cases.

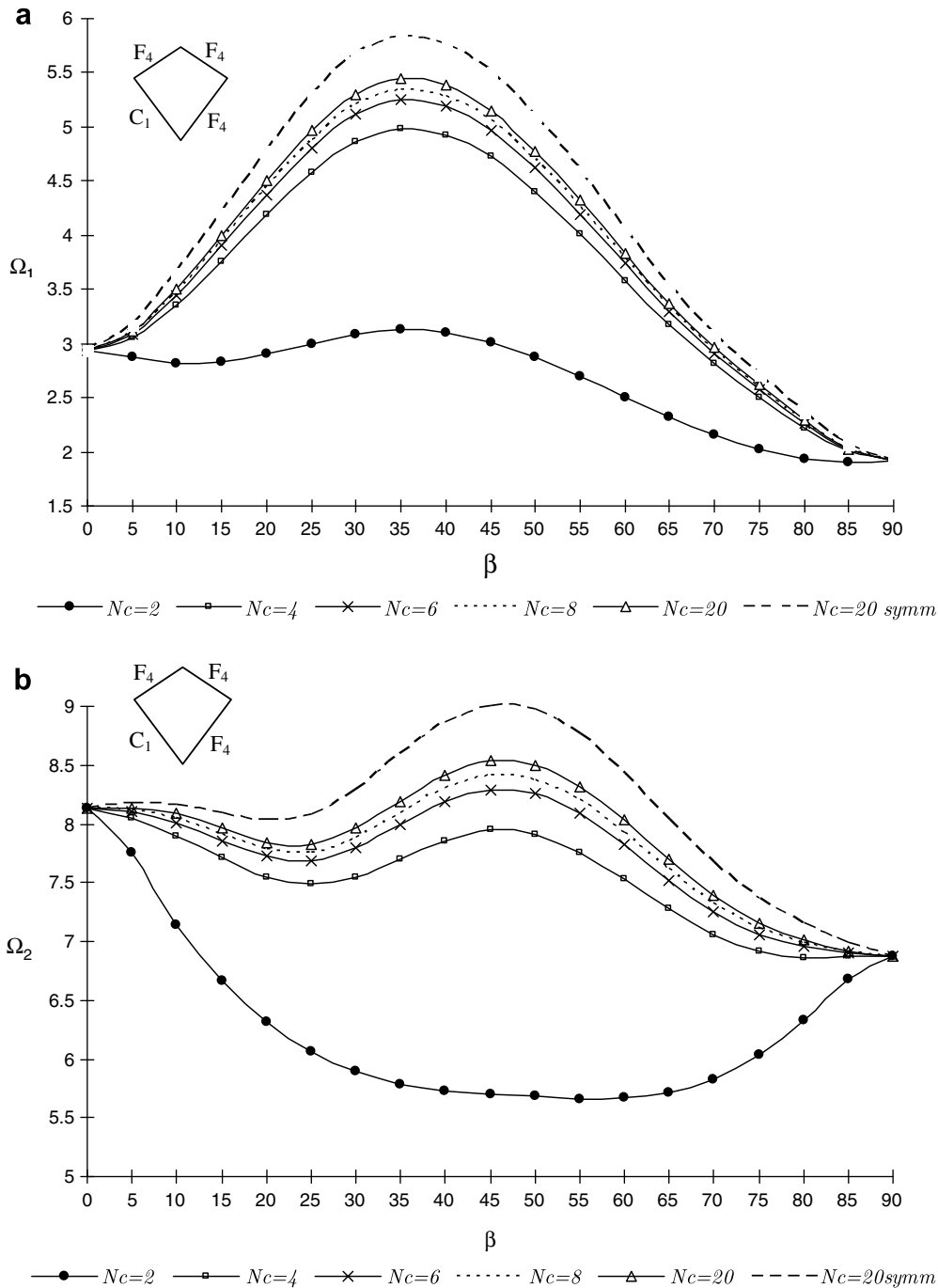


Fig. 4. Effect of fibre orientation and lamination sequence on the first two vibration frequency coefficients for $F_4F_4F_4C_1$ graphite/epoxy rhomboidal plates with $b/a = 1$. (a) $\Omega_1 = \omega_1^2 \sqrt{\rho h / D_\beta}$ versus β , (b) $\Omega_2 = \omega_2^2 \sqrt{\rho h / D_\beta}$ versus β .

5. Concluding remarks

A simple, computationally efficient and accurate formulation has been developed for the study of the free vibration of arbitrarily laminated composite plates. The methodology is based on the Ritz method and on the classical laminated plate theory, and uses natural coordinates to express the geometry of different laminates in a simple form. The transverse deflection and the in-plane stretching deformation of the plate are approximated by sets of char-

acteristic orthogonal polynomials generated using the Gram–Schmidt procedure. The algorithm developed is very general and allows taking into account a great variety of geometrical shapes, material properties and combinations of boundary conditions.

From the convergence studies and the comparisons with results available in the literature it is observed that the approach presented is reliable and accurate. Sets of numerical results are given in tabular and graphical form illustrating the influence of different number of layers, fibre

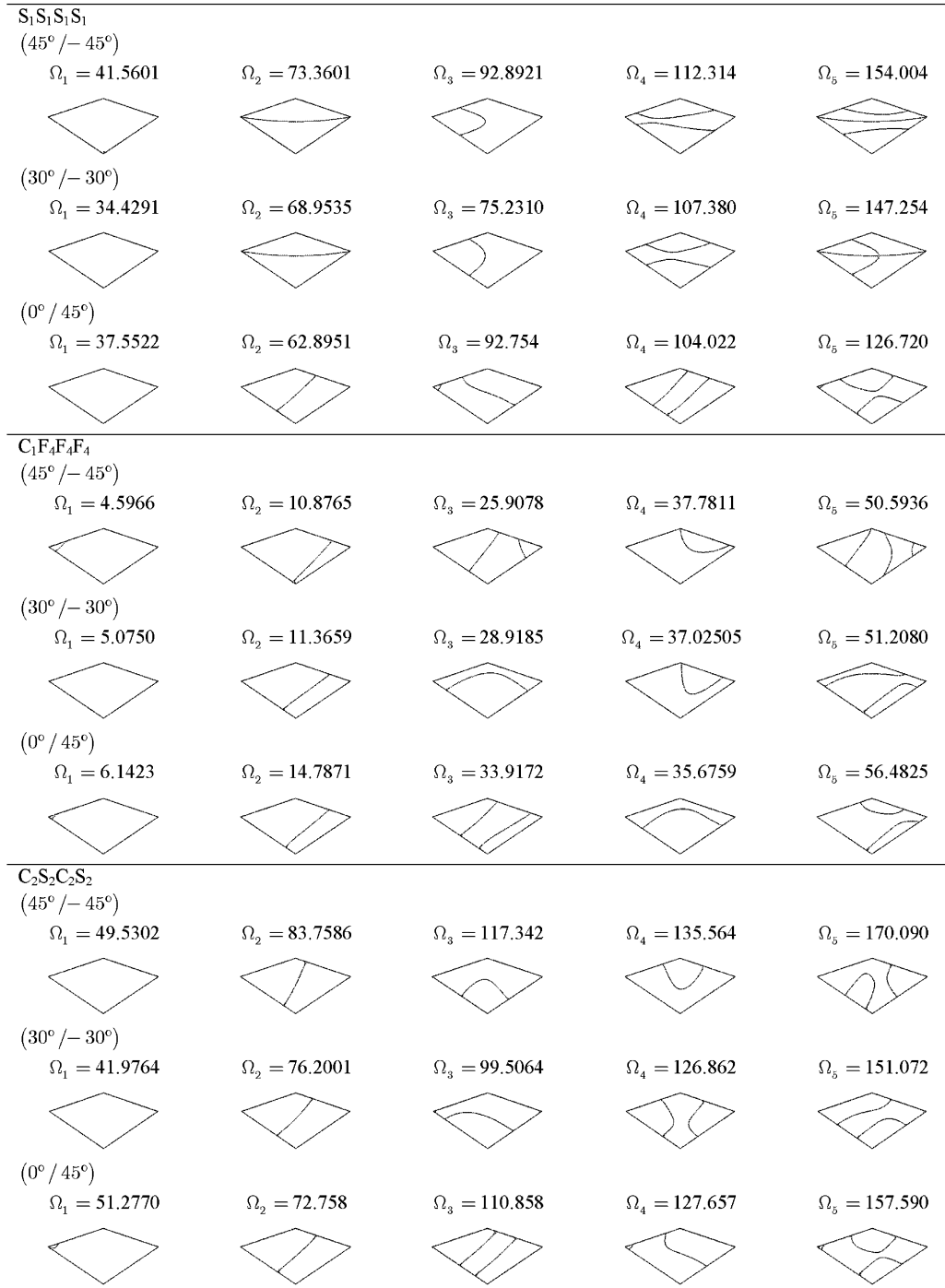


Fig. 5. First five non-dimensional frequency parameters $\Omega_i = \omega_i a^2 \sqrt{\rho h / D_{\beta}}$ ($i = 1, \dots, 5$), modal shapes and nodal patterns for general rhomboidal two-layered plates with $b/a = 0.5$.

stacking sequences and edge conditions on the natural frequencies and nodal patterns of a selection of laminated plates. Besides, all applications demonstrate that the present technique is accurate and efficient. Its flexibility offers the clear possibility of varying the parameters involved in the problem in a relatively simple way. Consequently it constitutes an efficient tool for the determination of natural frequencies so much in design problems as in optimization problems. Finally, it is important to point out that the

method presented can be easily modified to be applied to static deflection problems.

Appendix A

Matrices included in the second derivatives of displacement field

The matrices $[Op^{(1)}]$ and $[Op^{(2)}]$ that appear in Eq. (6) are as follow:

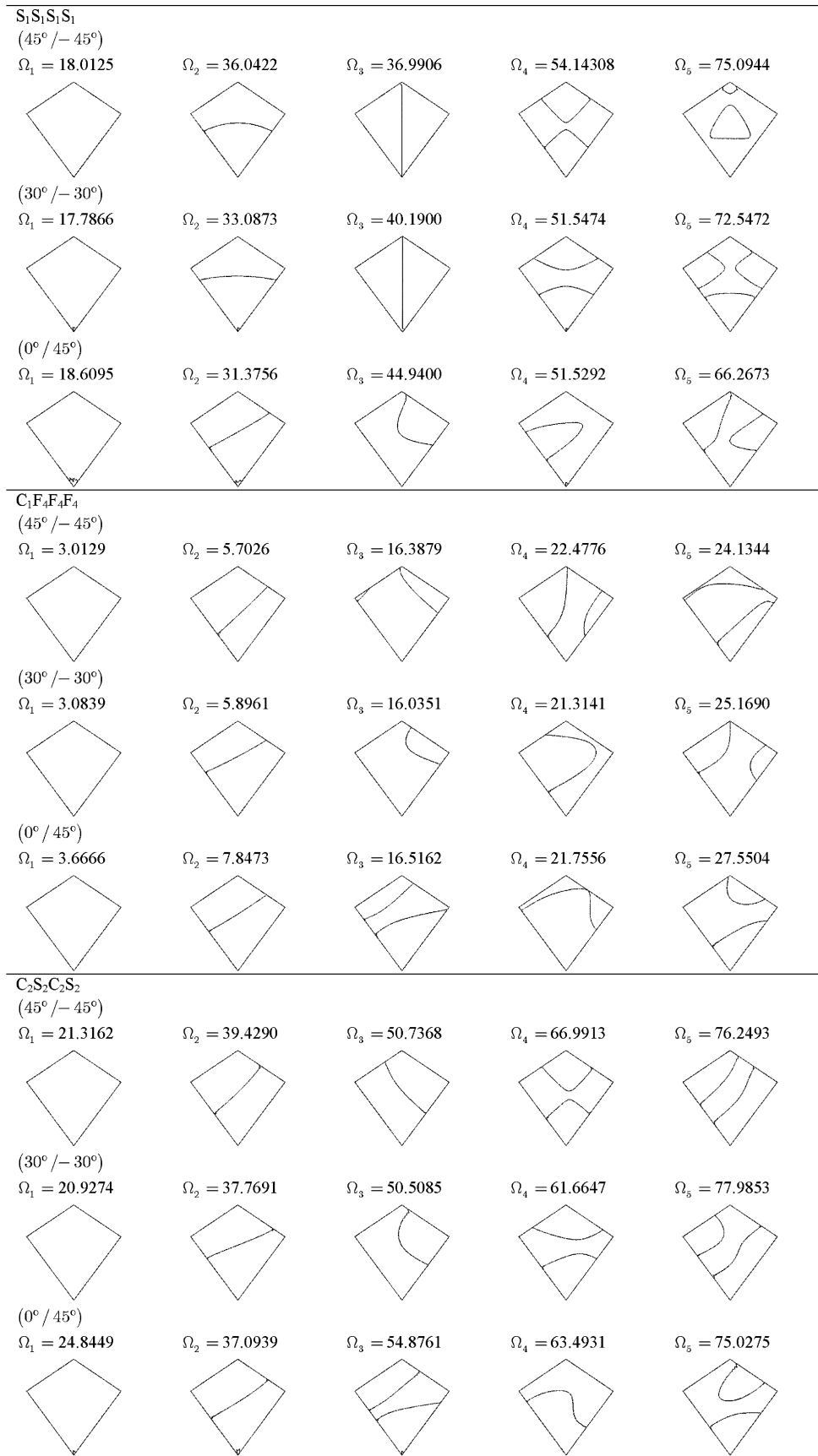


Fig. 6. First five non-dimensional frequency parameters $\Omega_i = \omega_i a^2 \sqrt{\rho h / D_\beta}$ ($i = 1, \dots, 5$), modal shapes and nodal patterns for general rhomboidal two-layered plates with $b/a = 1$.

$$[Op^{(1)}] = \begin{bmatrix} a_1 & a_2 & -a_3 \\ b_1 & b_2 & -b_3 \\ -c_1 & -c_2 & c_3 \end{bmatrix},$$

$$[Op^{(2)}] = \begin{bmatrix} \sum_{i=1}^3 a_i \alpha_i & \sum_{i=1}^3 a_i \beta_i \\ \sum_{i=1}^3 b_i \alpha_i & \sum_{i=1}^3 b_i \beta_i \\ -\sum_{i=1}^3 c_i \alpha_i & -\sum_{i=1}^3 c_i \beta_i \end{bmatrix},$$

where

$$a_1 = J_{22}^2, \quad a_2 = J_{12}^2, \quad a_3 = 2J_{12}J_{22}$$

$$b_1 = J_{21}^2, \quad b_2 = J_{11}^2, \quad b_3 = 2J_{11}J_{21}$$

$$c_1 = J_{21}J_{22}, \quad c_2 = J_{11}J_{12}, \quad c_3 = J_{11}J_{22} + J_{12}J_{21}$$

$$\alpha_1 = -J_{11,\xi}J_{22} + J_{12,\xi}J_{21} \quad \alpha_2 = -J_{21,\eta}J_{22} + J_{22,\eta}J_{21},$$

$$\alpha_3 = J_{11,\eta}J_{22} - J_{22,\xi}J_{21}$$

$$\beta_1 = J_{11,\xi}J_{12} - J_{12,\xi}J_{11}, \quad \beta_2 = J_{21,\eta}J_{12} - J_{22,\eta}J_{11},$$

$$\beta_3 = -J_{11,\eta}J_{12} + J_{22,\xi}J_{11}$$

Appendix B

Functions included in the strain energy

After substitution of Eq. (6) into Eq. (3) one obtains the maximum strain energy as a function of the derivatives of the displacements U , V and W with respect to the natural coordinates ξ , η . The factors of these derivatives depend on the geometrical and mechanical characteristics of the plates, and are given by

$$A_1^* = A_{11}a_1 - 2A_{16}c_1 + A_{66}b_1$$

$$A_2^* = A_{11}a_2 - 2A_{16}c_2 + A_{66}b_2$$

$$A_3^* = -A_{11}a_3 + 2A_{16}c_3 - A_{66}b_3$$

$$A_4^* = 2(A_{12}\sqrt{a_1b_2} + A_{66}\sqrt{a_2b_1}) - A_{16}a_3 - A_{26}b_3$$

$$A_5^* = -A_{12}a_3 + 2(A_{16}a_1 + A_{26}b_1 - A_{66}c_1)$$

$$A_6^* = -2c_2(A_{12} + A_{66}) + 2A_{16}a_2 + 2A_{26}b_2$$

$$A_7^* = 2(A_{12}\sqrt{a_2b_1} + A_{66}\sqrt{a_1b_2}) - A_{16}a_3 - A_{26}b_3$$

$$A_8^* = A_{22}b_1 - 2A_{26}c_1 + A_{66}a_1$$

$$A_9^* = A_{22}b_2 - 2A_{26}c_2 + A_{66}a_2$$

$$A_{10}^* = -A_{22}2\sqrt{b_1b_2} + 2A_{26}c_3 - A_{66}a_3$$

$$B_1^* = 2(\sqrt{a_1}(B_{11}a_1 + B_{12}b_1 - 2B_{16}c_1)$$

$$+ \sqrt{b_1}(-B_{16}a_1 - B_{26}b_1 + 2B_{66}c_1))$$

$$B_2^* = 2(\sqrt{a_1}(B_{11}a_2 + B_{12}b_2 - 2B_{16}c_2)$$

$$+ \sqrt{b_1}(-B_{16}a_2 - 2B_{26}b_2 + 2B_{66}c_2))$$

$$B_3^* = 2(\sqrt{a_2}(-B_{11}a_1 - B_{12}b_1 + 2B_{16}c_1)$$

$$+ \sqrt{b_2}(B_{16}a_1 + B_{26}b_1 - 2B_{66}c_1))$$

$$B_4^* = 2(\sqrt{a_1}(-B_{11}a_3 - B_{12}b_3 + 2B_{16}c_3) + \sqrt{b_1}(B_{16}a_3 + B_{26}b_3$$

$$- 2B_{66}c_3))$$

$$B_5^* = 2(\sqrt{a_2}(-B_{11}a_2 - B_{12}b_2 + 2B_{16}c_2) + \sqrt{b_2}(B_{16}a_2 + B_{26}b_2$$

$$- 2B_{66}c_2))$$

$$B_6^* = 2(\sqrt{a_2}(B_{11}a_3 + B_{12}b_3 - 2B_{16}c_3) + \sqrt{b_2}(-B_{16}a_3 - B_{26}b_3$$

$$+ 2B_{66}c_3))$$

$$B_7^* = 2(\sqrt{a_1}(B_{11}\sum_{i=1}^3 a_i \alpha_i + B_{12}\sum_{i=1}^3 b_i \alpha_i - 2B_{16}\sum_{i=1}^3 c_i \alpha_i)$$

$$+ \sqrt{b_1}(-B_{16}\sum_{i=1}^3 a_i \alpha_i - B_{26}\sum_{i=1}^3 b_i \alpha_i + 2B_{66}\sum_{i=1}^3 c_i \alpha_i))$$

$$B_8^* = 2(\sqrt{a_1}(B_{11}\sum_{i=1}^3 a_i \beta_i + 2B_{12}\sum_{i=1}^3 b_i \beta_i - 2B_{16}\sum_{i=1}^3 c_i \beta_i)$$

$$+ \sqrt{b_1}(-B_{16}\sum_{i=1}^3 a_i \beta_i - B_{26}\sum_{i=1}^3 b_i \beta_i + 2B_{66}\sum_{i=1}^3 c_i \beta_i))$$

$$B_9^* = 2(\sqrt{a_2}(-B_{11}\sum_{i=1}^3 a_i \alpha_i - B_{12}\sum_{i=1}^3 b_i \alpha_i + 2B_{16}\sum_{i=1}^3 c_i \alpha_i)$$

$$+ \sqrt{b_2}(B_{16}\sum_{i=1}^3 a_i \alpha_i + B_{26}\sum_{i=1}^3 b_i \alpha_i - 2B_{66}\sum_{i=1}^3 c_i \alpha_i))$$

$$B_{10}^* = 2(\sqrt{a_2}(-B_{11}\sum_{i=1}^3 a_i \beta_i - B_{12}\sum_{i=1}^3 b_i \beta_i + 2B_{16}\sum_{i=1}^3 c_i \beta_i)$$

$$+ \sqrt{b_2}(B_{16}\sum_{i=1}^3 a_i \beta_i + B_{26}\sum_{i=1}^3 b_i \beta_i - 2B_{66}\sum_{i=1}^3 c_i \beta_i))$$

$$B_{11}^* = 2(\sqrt{a_1}(B_{16}a_1 + B_{26}b_1 - 2B_{66}c_1) + \sqrt{b_1}(-B_{12}a_1 - B_{22}b_1$$

$$+ 2B_{26}c_1))$$

$$B_{12}^* = 2(\sqrt{a_1}(B_{16}a_2 + B_{26}b_2 - 2B_{66}c_2) + \sqrt{b_1}(-B_{12}a_2 - B_{22}b_2$$

$$+ 2B_{26}c_2))$$

$$B_{13}^* = 2(\sqrt{a_2}(-B_{16}a_1 - B_{26}b_1 + 2B_{66}c_1) + \sqrt{b_2}(B_{12}a_1 + B_{22}b_1$$

$$- 2B_{26}c_1))$$

$$B_{14}^* = 2(\sqrt{a_1}(-B_{16}a_3 - B_{26}b_3 + 2B_{66}c_3) + \sqrt{b_1}(B_{12}a_3 + B_{22}b_3$$

$$- 2B_{26}c_3))$$

$$B_{15}^* = 2(\sqrt{a_2}(-B_{16}a_2 - B_{26}b_2 + 2B_{66}c_2) + \sqrt{b_2}(B_{12}a_2 + B_{22}b_2$$

$$- 2B_{26}c_2))$$

$$B_{16}^* = 2\left(\sqrt{a_2}(B_{16}a_3 + B_{26}b_3 - 2B_{66}c_3) + \sqrt{b_2}(-B_{12}a_3 - B_{22}b_3 + 2B_{26}c_3)\right)$$

$$B_{17}^* = 2\left(\sqrt{a_1}\left(B_{16}\sum_{i=1}^3 a_i\alpha_i + B_{26}\sum_{i=1}^3 b_i\alpha_i - 2B_{66}\sum_{i=1}^3 c_i\alpha_i\right) + \sqrt{b_1}\left(-B_{12}\sum_{i=1}^3 a_i\alpha_i - B_{22}\sum_{i=1}^3 b_i\alpha_i + 2B_{26}\sum_{i=1}^3 c_i\alpha_i\right)\right)$$

$$B_{18}^* = 2\left(\sqrt{a_1}\left(B_{16}\sum_{i=1}^3 a_i\beta_i + B_{26}\sum_{i=1}^3 b_i\beta_i - 2B_{66}\sum_{i=1}^3 c_i\beta_i\right) + \sqrt{b_1}\left(-B_{12}\sum_{i=1}^3 a_i\beta_i - B_{22}\sum_{i=1}^3 b_i\beta_i + 2B_{26}\sum_{i=1}^3 c_i\beta_i\right)\right)$$

$$B_{19}^* = 2\left(\sqrt{a_2}\left(-B_{16}\sum_{i=1}^3 a_i\alpha_i - B_{26}\sum_{i=1}^3 b_i\alpha_i + 2B_{66}\sum_{i=1}^3 c_i\alpha_i\right) + \sqrt{b_2}\left(B_{12}\sum_{i=1}^3 a_i\alpha_i + B_{22}\sum_{i=1}^3 b_i\alpha_i - 2B_{26}\sum_{i=1}^3 c_i\alpha_i\right)\right)$$

$$B_{20}^* = 2\left(\sqrt{a_2}\left(-B_{16}\sum_{i=1}^3 a_i\beta_i - B_{26}\sum_{i=1}^3 b_i\beta_i + 2B_{66}\sum_{i=1}^3 c_i\beta_i\right) + \sqrt{b_2}\left(B_{12}\sum_{i=1}^3 a_i\beta_i + B_{22}\sum_{i=1}^3 b_i\beta_i - 2B_{26}\sum_{i=1}^3 c_i\beta_i\right)\right)$$

$$D_1^* = D_{11}a_1^2 + D_{22}b_1^2 + 2D_{12}a_1b_1 - 4D_{16}a_1c_1 - 4D_{26}b_1c_1 + 4D_{66}c_1^2$$

$$D_2^* = D_{11}a_2^2 + D_{22}b_2^2 + 2D_{12}a_2b_2 - 4D_{16}a_2c_2 - 4D_{26}b_2c_2 + 4D_{66}c_2^2$$

$$D_3^* = 2(D_{11}a_1a_2 + D_{22}b_1b_2 + D_{12}(b_1a_2 + b_2a_1) - 2D_{16}(c_2a_1 + c_1a_2) - 2D_{26}(b_2c_1 + b_1c_2) + 4D_{66}c_1c_2)$$

$$D_4^* = D_{11}a_3^2 + D_{22}b_3^2 + 2D_{12}a_3b_3 - 4D_{16}a_3c_3 - 4D_{26}b_3c_3 + 4D_{66}c_3^2$$

$$D_5^* = 2(-D_{11}a_3a_1 - D_{22}b_3b_1 - D_{12}(b_1a_3 + b_3a_1) + 2D_{16}(a_1c_3 + c_1a_3) + 2D_{26}(b_3c_1 + b_1c_3) - 4D_{66}c_3c_1)$$

$$D_6^* = 2(-D_{11}a_2a_3 - D_{22}b_3b_2 - D_{12}(b_3a_2 + b_2a_3) + 2D_{16}(c_3a_2 + c_2a_3) + 2D_{26}(b_3c_2 + b_2c_3) - 4D_{66}c_3c_2)$$

$$D_7^* = 2\left(D_{11}a_1\sum_{i=1}^3 a_i\alpha_i + D_{22}b_1\sum_{i=1}^3 b_i\alpha_i + D_{12}\left(a_1\sum_{i=1}^3 b_i\alpha_i + b_1\sum_{i=1}^3 a_i\alpha_i\right) - 2D_{16}\left(a_1\sum_{i=1}^3 c_i\alpha_i + c_1\sum_{i=1}^3 a_i\alpha_i\right) - 2D_{26}\left(b_1\sum_{i=1}^3 c_i\alpha_i + c_1\sum_{i=1}^3 b_i\alpha_i\right) + 4D_{66}c_1\sum_{i=1}^3 c_i\alpha_i\right)$$

$$D_8^* = 2\left(D_{11}a_2\sum_{i=1}^3 a_i\beta_i + D_{22}b_2\sum_{i=1}^3 b_i\beta_i + D_{12}\left(a_2\sum_{i=1}^3 b_i\beta_i + b_2\sum_{i=1}^3 a_i\beta_i\right) - 2D_{16}\left(a_2\sum_{i=1}^3 c_i\beta_i + c_2\sum_{i=1}^3 a_i\beta_i\right) - 2D_{26}\left(c_2\sum_{i=1}^3 b_i\beta_i + b_2\sum_{i=1}^3 c_i\beta_i\right) + 4D_{66}c_2\sum_{i=1}^3 c_i\beta_i\right)$$

$$D_9^* = 2\left(D_{11}a_1\sum_{i=1}^3 a_i\beta_i + D_{22}b_1\sum_{i=1}^3 b_i\beta_i + D_{12}\left(a_1\sum_{i=1}^3 b_i\beta_i + b_1\sum_{i=1}^3 a_i\beta_i\right) - 2D_{16}\left(a_1\sum_{i=1}^3 c_i\beta_i + c_1\sum_{i=1}^3 a_i\beta_i\right) - 2D_{26}\left(c_1\sum_{i=1}^3 b_i\beta_i + b_1\sum_{i=1}^3 c_i\beta_i\right) + 4D_{66}c_1\sum_{i=1}^3 c_i\beta_i\right)$$

$$D_{10}^* = 2\left(D_{11}a_2\sum_{i=1}^3 a_i\alpha_i + D_{22}b_2\sum_{i=1}^3 b_i\alpha_i + D_{12}\left(a_2\sum_{i=1}^3 b_i\alpha_i + b_2\sum_{i=1}^3 a_i\alpha_i\right) - 2D_{16}\left(a_2\sum_{i=1}^3 c_i\alpha_i + c_2\sum_{i=1}^3 a_i\alpha_i\right) - 2D_{26}\left(c_2\sum_{i=1}^3 b_i\alpha_i + b_2\sum_{i=1}^3 c_i\alpha_i\right) + 4D_{66}c_2\sum_{i=1}^3 c_i\alpha_i\right)$$

$$D_{11}^* = 2\left(-D_{11}a_3\sum_{i=1}^3 a_i\alpha_i - D_{22}b_3\sum_{i=1}^3 b_i\alpha_i - D_{12}\left(a_3\sum_{i=1}^3 b_i\alpha_i + b_3\sum_{i=1}^3 a_i\alpha_i\right) + 2D_{16}\left(a_3\sum_{i=1}^3 c_i\alpha_i + c_3\sum_{i=1}^3 a_i\alpha_i\right) + 2D_{26}\left(c_3\sum_{i=1}^3 b_i\alpha_i + b_3\sum_{i=1}^3 c_i\alpha_i\right) - 4D_{66}c_3\sum_{i=1}^3 c_i\alpha_i\right)$$

$$D_{12}^* = 2\left(-D_{11}a_3\sum_{i=1}^3 a_i\beta_i - D_{22}b_3\sum_{i=1}^3 b_i\beta_i - D_{12}\left(a_3\sum_{i=1}^3 b_i\beta_i + b_3\sum_{i=1}^3 a_i\beta_i\right) + 2D_{16}\left(a_3\sum_{i=1}^3 c_i\beta_i + c_3\sum_{i=1}^3 a_i\beta_i\right) + 2D_{26}\left(c_3\sum_{i=1}^3 b_i\beta_i + b_3\sum_{i=1}^3 c_i\beta_i\right) - 4D_{66}c_3\sum_{i=1}^3 c_i\beta_i\right)$$

$$\begin{aligned}
D_{13}^* &= D_{11} \left(\sum_{i=1}^3 a_i \alpha_i \right)^2 + D_{22} \left(\sum_{i=1}^3 b_i \alpha_i \right)^2 + 2D_{12} \sum_{i=1}^3 b_i \alpha_i \\
&\quad \times \sum_{i=1}^3 a_i \alpha_i - 4D_{16} \sum_{i=1}^3 c_i \alpha_i \sum_{i=1}^3 a_i \alpha_i - 4D_{26} \sum_{i=1}^3 b_i \alpha_i' \\
&\quad \times \sum_{i=1}^3 c_i \alpha_i' + 4D_{66} \left(\sum_{i=1}^3 c_i \alpha_i' \right)^2 \\
D_{14}^* &= D_{11} \left(\sum_{i=1}^3 a_i \beta_i \right)^2 + D_{22} \left(\sum_{i=1}^3 b_i \beta_i \right)^2 + 2D_{12} \sum_{i=1}^3 b_i \beta_i \\
&\quad \times \sum_{i=1}^3 a_i \beta_i - 4D_{16} \sum_{i=1}^3 c_i \beta_i \sum_{i=1}^3 a_i \beta_i - 4D_{26} \sum_{i=1}^3 b_i \beta_i \\
&\quad \times \sum_{i=1}^3 c_i \beta_i + 4D_{66} \left(\sum_{i=1}^3 c_i \beta_i \right)^2 \\
D_{15}^* &= 2 \left(D_{11} \sum_{i=1}^3 a_i \alpha_i \sum_{i=1}^3 a_i \beta_i + D_{22} \sum_{i=1}^3 b_i \alpha_i \sum_{i=1}^3 b_i \beta_i \right. \\
&\quad + D_{12} \left(\sum_{i=1}^3 b_i \beta_i \sum_{i=1}^3 a_i \alpha_i + \sum_{i=1}^3 b_i \alpha_i \sum_{i=1}^3 a_i \beta_i \right) \\
&\quad - 2D_{16} \left(\sum_{i=1}^3 c_i \beta_i \sum_{i=1}^3 a_i \alpha_i + \sum_{i=1}^3 c_i \alpha_i \sum_{i=1}^3 a_i \beta_i \right) \\
&\quad - 2D_{26} \left(\sum_{i=1}^3 b_i \beta_i \sum_{i=1}^3 c_i \alpha_i + \sum_{i=1}^3 b_i \alpha_i \sum_{i=1}^3 c_i \beta_i \right) \\
&\quad \left. + 4D_{66} \sum_{i=1}^3 c_i \alpha_i \sum_{i=1}^3 c_i \beta_i \right)
\end{aligned}$$

where A_{ij} , B_{ij} , D_{ij} ($i, j = 1, 2, 6$) are the conventional laminate stiffness coefficients and a_i , b_i , c_i , α_i , β_i , ($i = 1, \dots, 3$) are defined in Appendix A.

References

- [1] Reddy JN. Mechanics of laminated anisotropic plates: theory and analysis. Boca Raton, FL: CRC Press; 1997.
- [2] Baharlou B, Leissa AW. Vibration and buckling of generally laminated composite plates with arbitrary edge conditions. *Int J Mech Sci* 1987;29:545–55.
- [3] Farsa J, Kukreti AR, Bert CW. Fundamental frequency analysis of laminated rectangular plates by differential quadrature method. *Int J Numer Methods Eng* 1993;36:2341–56.
- [4] Barton O, Reiss R. Vibration of antisymmetric laminated plates using eigensensitivity analysis. *Comput Struct* 1997;64:425–39.
- [5] Kabir HRH, Al-Khalefi AM, Chaudhuri RA. Free vibration analysis of thin arbitrarily laminated anisotropic plates using boundary-continuous displacement Fourier approach. *Compos Struct* 2001;53:469–76.
- [6] Kabir HRH. On free vibration response and mode shapes of arbitrarily laminated rectangular plates. *Compos Struct* 2004;65:13–27.
- [7] Cheung YK, Zhou D. Free vibrations of rectangular unsymmetrically laminated plates with internal line supports. *Comput Struct* 2001;79:1923–32.
- [8] Liew KM, Lim CW. Vibratory characteristics of general laminates, I: symmetric trapezoids. *J Sound Vibr* 1995;183:615–42.
- [9] Lim CW, Liew KM, Kitipornchai S. Vibration of arbitrarily laminated plates of general trapezoidal planform. *J Acoust Soc Am* 1996;100:3674–85.
- [10] Wang S. Vibration of thin skew fibre reinforced composite laminates. *J Sound Vibr* 1997;201:335–52.
- [11] Krishna Reddy AR, Palaninathan R. Free vibration of skew laminates. *Comput Struct* 1999;70:415–23.
- [12] Nallim LG, Oller S, Grossi RO. Statical and dynamical behaviour of thin fibre reinforced composite laminates with different shapes. *Comput Methods Appl Mech Eng* 2005;194:1797–822.
- [13] Whitney JM. Structural analysis of laminated anisotropic plates. Pennsylvania, USA: Technomic Publishing Co. Inc.; 1987.
- [14] Li WY, Cheung YK, Tham LG. Spline finite strip analysis of general plates. *J Eng Mech* 1986;112:43–54.
- [15] Cheung YK, Tham LG, Li WY. Free vibration and static analysis of general plate by spline finite strip method. *Comput Mech* 1988;3:187–97.
- [16] Geannakakes GN. Vibration analysis of arbitrarily shaped plates using beam characteristic orthogonal polynomials in semi-analytical finite strip method. *J Sound Vibr* 1990;137:283–303.
- [17] Wang G, Cheng-Tzu TH. Static and dynamic analysis of arbitrary quadrilateral flexural plates by B3-spline functions. *Int J of Solids Struct* 1994;31:657–67.
- [18] Bert CW, Malik M. The differential quadrature method for irregular domains and application to plate vibration. *Int J Mech Sci* 1996;38:589–606.
- [19] Saadatpour MM, Azhari M, Bradford MA. Vibration analysis of simply supported plates of general shape with internal point and line supports using the Galerkin method. *Eng Struct* 2000;22:1180–8.
- [20] Zienkiewicz OC, Taylor RL. The finite element method. 4th ed. New York: McGraw-Hill; 1991.
- [21] Jones RM. Mechanics of composite materials. second ed. Philadelphia: Taylor and Francis; 1999.
- [22] Rektorys K. Variational methods in mathematics. Dordrecht: Science and Engineering. Reidel Co.; 1980.
- [23] Kantorovich L, Krylov V. Approximate methods of higher analysis. Interscience Publishers; 1964.
- [24] Mikhlin S. Variational methods of mathematical physics. New York: Mac Millan Co.; 1964.
- [25] Nallim LG, Grossi RO. On the use of orthogonal polynomials in the study of anisotropic plates. *J Sound Vibr* 2003;264:1201–7.
- [26] Nallim LG, Luccioni BM, Grossi RO. Vibration of general triangular composite plates with elastically restrained edges. *Thin Walled Struct* 2005;43:1711–45.
- [27] Bhat RB. Plate deflection using orthogonal polynomials. *J Eng Mech ASCE* 1985;101:1301–9.
- [28] Bhat RB. Natural frequencies of rectangular plates using characteristic orthogonal polynomials in Rayleigh–Ritz method. *J Sound Vibr* 1985;102:493–9.