

1975

## Program bova - listing, March 1975.

W. S. Peterson

C. N. Kostem

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16. Abstract <p>The report contains the listing of Program BOVA (Bridge Overload Analysis). The program is written in FORTRAN IV and was tested in the Control Data Corporation 6400 Computer. The program is capable of predicting the full history of stresses, strains, deformations and damages that may occur to simple span beam-slab bridge superstructures with reinforced or prestressed concrete I-beams when they are subjected to overload vehicles. The analysis scheme employs the finite element displacement method and permits the inclusion of material nonlinearities. The pertinent theoretical developments, comparisons, and the information for the use of the program have been presented in other reports.</p>					
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Project 71-12: Overloading Behavior of Beam-Slab  
Type Highway Bridges

PROGRAM BOVA-LISTING

(Bridge Overloading Analysis)

by

William S. Peterson

Celal N. Kostem

This work was sponsored by the Pennsylvania Department of Transportation and the U. S. Department of Transportation, Federal Highway Administration.

The contents of this report reflect the view of the authors who are responsible for the facts and the accuracy of the data presented herein. The contents do not necessarily reflect the official views or policies of the Pennsylvania Department of Transportation or the Federal Highway Administration. This report does not constitute a standard, specification, or regulation.

LEHIGH UNIVERSITY

Office of Research

Bethlehem, Pennsylvania

March 1975

Fritz Engineering Laboratory Report No. 378B.6B

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## ABSTRACT

The report contains the listing of Program BOVA (Bridge Overload Analysis). The program is written in FORTRAN IV and was tested in the Control Data Corporation 6400 Computer. The program is capable of predicting the full history of stresses, strains, deformations and damages that may occur to simple span beam-slab bridge superstructures with reinforced or prestressed concrete I-beams when they are subjected to overload vehicles. The analysis scheme employs the finite element displacement method and permits the inclusion of material nonlinearities. The pertinent theoretical developments, comparisons, and the information for the use of the program have been presented in other reports.

## 1. Introduction

This report contains the listing of Program BOVA (Bridge Overload Analysis). The program was developed within the framework of the research project The Overloading Behavior of Beam-Slab Type Highway Bridges. The program is capable of predicting the elastic and inelastic response of simple span beam-slab type highway bridge superstructures with reinforced concrete deck and reinforced or prestressed concrete rectangular, T- or I-beams. The inelastic response of the bridge superstructure is assumed to occur due to the traverse of the overload vehicles.

## 2. Scope of the Investigation

The modeling of the bridge superstructure and the analysis of the developed analytical model are accomplished through the use of the finite element displacement method. The analysis is based on the first order geometry, i.e. the equations of equilibrium and compatibility are formulated on undeformed geometry. All material nonlinearities for concrete, reinforcing steel and the prestressing strands have been included in the formulation. The loading is assumed to be static in nature, and due to the nonlinearity of the overall problem, an incremental loading scheme is used. That is, if the total load of the overload vehicle is "P" small increments of "P" are applied and the structure is analyzed, then another fraction of "P" is applied, etc. until the attainment of the desired load level or the failure of the bridge superstructure. The superstructure failure is defined through the monitoring of certain

response characteristics, such as the strain, stress, crack width, etc.

### 3. Analytical Modeling of the Superstructure

The analytical modeling of the bridge superstructure is undertaken by discretizing the system to plate bending finite elements and beam elements. Their aggregation corresponds to the actual superstructure. In order to account for the penetration of the material nonlinearities as well as the damage, these elements are further divided into layers through the depth of the beams and the slab. This subdivision technique permits the assessment of the damage, and also the penetration of the damage for various locations in the superstructure for various load levels. The program provides the stresses in the superstructure at various locations, according to the finite element discretization employed, as tabulated printouts and as printer generated plots which can be used by engineers for quick assessment of the damage to the bridge for a given load level and loading location.

In the development of the overall modeling and analysis scheme, the research was compartmentalized and different phases were reported in different reports.

Report 1: The development of the finite element analysis scheme for eccentrically stiffened plates, i.e. bridges where the superstructure is assumed to remain elastic.

("Finite Element Analysis of Plates and Eccentrically Stiffened Plates," by A. W. Wegmuller and C. N. Kostem, Fritz Engineering Laboratory Report No. 378A.3, Lehigh

University, February 1973.)

- Report 2: The development of the basic model and solution scheme for the eccentrically stiffened plates, i.e. bridge superstructure, including material nonlinearities following the perfect plasticity relations. ("Finite Element Analysis of Elastic-Plastic Plates and Eccentrically Stiffened Plates," by A. W. Wegmuller and C. N. Kostem, Fritz Engineering Laboratory Report No. 378A.4, Lehigh University, February 1973.)
- Report 3: The modeling of reinforced and prestressed concrete beams, and the developments of analytical expressions for the inelastic response of concrete and steel subjected to uniaxial stress field. ("The Inelastic Analysis of Reinforced and Prestressed Concrete Beams," by J. M. Kulicki and C. N. Kostem, Fritz Engineering Laboratory Report No. 378B.1, Lehigh University, November 1972.)
- Report 4: The finalization of the computer program used to predict the inelastic response of bridge beams. ("User's Manual for Program Beam," by J. M. Kulicki and C. N. Kostem, Fritz Engineering Laboratory Report No. 378B.2, Lehigh University, February 1973.)
- Report 5: Pilot studies on the inelastic response of bridges, assuming that the beams will have the actual inelastic



response as reported in the above reports (Repts. 3 and 4) and the deck slab will exhibit perfect plasticity. ("The Inelastic Analysis of Prestressed and Reinforced Concrete Bridge Beams by the Finite Element Method," by J. M. Kulicki and C. N. Kostem, Fritz Engineering Laboratory Report No. 378A.6, Lehigh University, September 1973.)

Report 6: Inelastic analysis of reinforced concrete deck slabs, exhibiting all forms of material nonlinearities. ("The Inelastic Analysis of Reinforced Concrete Slabs," by W. S. Peterson, C. N. Kostem and J. M. Kulicki, Fritz Engineering Laboratory Report No. 378B.3, Lehigh University, May 1974.)

Report 7: Inelastic analysis of beam-slab bridge superstructures interfacing the analysis techniques developed and reported in Report 3 and Report 6 to predict the overload response of the bridges. ("The Inelastic Analysis of Beam-Slab Highway Bridge Superstructures," by W. S. Peterson and C. N. Kostem, Fritz Engineering Laboratory Report No. 378B.5, Lehigh University, March 1975.)

Report 8: The sensitivity of the analytical scheme presented in Report 7 is demonstrated through case studies. The report also contains summaries of Reports 6 and 7. ("The Inelastic Analysis of Beam-Slab Bridges," by W. S. Peterson and C. N. Kostem, Fritz Engineering Laboratory Report No. 400.20, Lehigh University, July 1975.)

Report 9: The formulations (Reports 1 through 8) employed in the prediction of the inelastic behavior of beam, slab and bridge superstructure are essentially based on the flexural response of these components. The pilot studies indicated that the stresses perpendicular to the plane of the slab do not cause any deviations in the prediction of the overload response via Program BOVA. ("Shear Punching of Bridge Decks," by C. N. Kostem, Fritz Engineering Laboratory Report No. 378B.4, Lehigh University, January 1977.)

Report 10: Application of Program BOVA to a wide range of bridge configurations resulted in a parametric study. ("Overloading of Highway Bridges - A Parametric Study," by C. N. Kostem, Fritz Engineering Laboratory Report No. 378B.7, Lehigh University, August 1976.)

Report 11: Description of the input and the output of the computer program to be employed, whose application was presented in Report 7, to predict the overload response of beam-slab type highway bridges. ("User's Manual for Program BOVA," by W. S. Peterson and C. N. Kostem, Fritz Engineering Laboratory Report No. 378B.6A, Lehigh University, March 1975.)

It should be noted that Reports 1, 2, 3 and 6 contain the fundamental research leading to the development of the analysis scheme for the overloading of the components of bridge superstructures. Reports 3, 5, 6, 7, and 8 contain the essential formulation and pertinent numerical

comparisons for the formulation of the prediction of the overloading of the types of bridges in question. Reports 7, 8 and 10 contain detailed applications of Program BOVA to bridge superstructures.

Prior to making major changes in Program BOVA it is essential that the implications of these changes be recognized through the mastery of Reports 3, 6, 7 and 11. Minor changes in Program BOVA, which will not effect the overall analysis scheme, should be undertaken with appropriate modifications, if needed, in Report 11.

Reports 3, 4, 6, 7, 9, 10 and 11 were prepared as a partial fulfillment of the requirements of Pennsylvania Department of Transportation's Research Project 71-12, Overloading Behavior of Beam-Slab Type Highway Bridges. The distribution copies of these reports have been submitted to the Pennsylvania Department of Transportation.

#### 4. Computer Environment

The program listed in this report was developed at Lehigh University's Computing Center using Control Data Corporation's 6400 digital computer. Every effort was made to comply with standard FORTRAN IV; however, due to the complexity of the analysis scheme and consequently that of the computer program, certain "commands and functions" were used that are peculiar to CDC 6400 computers. The implementation of this program at other installations will require changing some of the statements that are machine dependent and substituting them with those that the other computer configurations will have.

It should also be noted that the listed program has been extensively tested and appears to be totally debugged, but perfection is an elusive goal in computer program development so it is possible that some bugs may have gone undetected.

## 5. Implementation

The program presented herein can be used to predict the overload response of beam-slab bridges. The use of the program has been described in detail in Report 11 referred to in Section 3. The program can be used not only for the overload analysis of the bridges, if the load levels in question are in excess of the design load levels, but if the bridge has deteriorated it can also be used to find the loading which will not induce any damage. That is, the program can be used for the rating of bridges as well. In the use of the program it is strongly advisable that the "User's Manual for Program BOVA" always be referred to in order to have correct input and not to make systematic errors in the input or in the interpretation of the output.

The changes that may be made to this program should be undertaken with great care. It's complexity may be undetected by the "average programmer," thus intended improvements may in turn destroy the integrity of the program and may result in incorrect answers.

6. Listing of Program BOVA  
(Bridge Overload Analysis)

```

OVERLAY (ECCP,0,0) BOVA 2
PROGRAM BOVA (TAPE3=100,TAPE6=100,TAPE11=100,TAPE13=100,TAPE14=100,BOVA 3
1TAPE12=100,TAPE27=0,TAPE16=100,TAPE17=100,TAPE18=100,TAPE20=100,TA9OVA 4
2PE22=100,TAPE24=100,TAPE25=100,TAPE28=100,TAPE29=100,TAPE30=100,TA BOVA 5
3PE31=100,TAPE33=100,INPUT=100,TAPE1=INPUT,OUTPUT=100,TAPE2=OUTPUT,BOVA 6
4STARTC=100,TAPE7=STARTC,PLOTG=100,TAPE8=PLOTG,STARTI=100,TAPE9=STABOVA 7
5RTI,JUM4=100,TAPE4=JUM4,JUM5=100,TAPE5=JUM5,J19=100,TAPE19=J19,J23ROVA 8
6=100,TAPE23=J23,J15=100,TAPE15=J15,J21=100,TAPE21=J21,J32=100,TAPEBOVA 9
732=J32) BOVA 10

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C * * * * * BOVA 11
C BOVA 12
C-----BRIDGE OVERLOAD ANALYSIS-----BOVA 13

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C BOVA 14
C FINITE ELEMENT PROGRAM FOR PRESTRESSED AND REINFORCED BOVA 15
C BEAM-SLAB TYPE HIGHWAY BRIDGE SUPERSTRUCTURES BOVA 16

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C WITH REINFORCED CONCRETE DECKS BOVA 17
C BOVA 18
C BY BOVA 19

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C CELAL N. KOSTEM BOVA 20
C WILLIAM S. PETERSON BOVA 21
C BOVA 22

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C FRITZ ENGINEERING LABORATORY BOVA 23
C LEHIGH UNIVERSITY BOVA 24
C BETHLEHEM, PENNSYLVANIA BOVA 25

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C BOVA 26
C BOVA 27
C COMPILER - FORTRAN 3.0, LEVEL G BOVA 28

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C CONFIGURATION- CDC 6400 BOVA 29
C LEHIGH UNIVERSITY, BETHLEHEM, PA. BOVA 30
C FEE. 1975 BOVA 31

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C BOVA 32
C THEORETICAL DEVELOPMENT IS CONTAINED IN THE FOLLOWING REPORTS BOVA 33
C -THE INELASTIC ANALYSIS OF REINFORCED AND PRESTRESSED CONCRETE BOVA 34

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C BEAMS- BOVA 35
C FRITZ ENGINEERING LABORATORY REPORT NO. 378B.1 BOVA 36
C BY J. M. KULICKI AND C. N. KOSTEM BOVA 37

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C BOVA 38
C -THE INELASTIC ANALYSIS OF REINFORCED CONCRETE SLABS- BOVA 39
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C BY W. S. PETERSON, C. N. KOSTEM, AND J. M. KULICKI BOVA 41
C BOVA 42

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C -THE INELASTIC ANALYSIS OF BEAM-SLAB HIGHWAY BRIDGE BOVA 43
C SUPERSTRUCTURES- BOVA 44
C FRITZ ENGINEERING LABCRATORY REPORT NO. 378B.5 BOVA 45
C BY W. S. PETERSON AND C. N. KOSTEM BOVA 46

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C BOVA 47
C -THE INELASTIC ANALYSIS OF BEAM-SLAB BRIDGES BOVA 48
C FRITZ ENGINEERING LABORATORY REPORT NO. 400.20 BOVA 49

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C BY W. S. PETERSON AND C. N. KOSTEM BOVA 50
C BOVA 51

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C -USER S MANUAL FOR PROGRAM BOVA BOVA 52
C FRITZ ENGINEERING LABORATORY REPORT NO. 378B.6A BOVA 53
C BY W. S. PETERSON AND C. N. KOSTEM BOVA 54

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C BOVA 55
C BOVA 56
C***** BOVA 57
C BOVA 58

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C TAPES UED FOR DATA TRANSFER ARE AS FOLLOWS BOVA 59

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C	DESCRIPTION IS TAPE NUMBER - VARIABLES - COMMENT, NAME	BOVA	60
C		BOVA	61
C	FORMATED TAPES	BOVA	62
C		BOVA	63
C	TAPE 1 - INPUT FROM CARDS, NAME= INPUT	BOVA	64
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C	TAPE 12- DT, ELLA, ELLB - STIFFNESS MATRIX	BOVA	80
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C	TAPE 14- XLNGTH , GKEIX, ITYPE, ASXLR, AILR, ZCRD, TSHEAR	BOVA	82
C	-STIFFNESS MATRIX	BOVA	83
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C	TAPE 22- SXXOT, SYYOT, SXYOT -STIFFNESS MATRIX	BOVA	91
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C	TAPE 27- XLNGTH , GKEIX, NPIX, NPKX, ITYPE, ASXLR, AILR, ZCRD, TSHEAR	BOVA	95
C	TAPE 28- DT, ELLA, ELLB, NPI, NPJ, NPK, NPL, XSEG, YSEG	BOVA	96
C	TAPE 29- FORCE, LILA, FORCE( DEAD LOAD)	BOVA	97
C	TAPE 30- DISP	BOVA	98
C	TAPE 31- AS1, AF, AS	BOVA	99
C	TAPE 32- AFT, AST, NODE, X, Y -PRINTER PLOTS AND STRAIN POSITION	BOVA	100
C	TAPE 33- ESX	BOVA	101
C		BOVA	102
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	COMMON /CB01/ NUMEL, NANA, NUMNP, NETA, NA1, NULAY, NSLAYR, NCNS	BOVA	105
	COMMON /CB2/ NBEAMX, NIX, NULAYB, NULAYR, NULAYE, NR, NRE	BOVA	106
	COMMON /CB3/ LWIDTH, NITA, NPSEGM	BOVA	107
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	COMMON /CBS301/ COUPLE	BOVA	109
	COMMON /PROPC/ V(11)	BOVA	110
	COMMON /ITAPE/ ITN, ITO, I12, I22, I32	BOVA	111
	COMMON /PROPCB/ ZA, EZA, ZB, EZE	BOVA	112
	COMMON /EDOWN/ DUMMY5(2)	BOVA	113
	COMMON /SIGNIF/ NSIGNI(2)	BOVA	114
	COMMON /TFAIL/ TOLF	BOVA	115
	COMMON /TU00/ TU0(8,12)	BOVA	116
	COMMON /SKEW/ PHI, AU(2,2), AO(3,3), DS(3,3)	BOVA	117

COMMON /PFACT/ PFC	BOVA 118
COMMON /PLTCD/ PLTCRD	BOVA 119
COMMON /COMPLT/ ICMLT	BOVA 120
COMMON /SCALD/ ISCALE,IFN	BOVA 121
COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	BOVA 122
COMMON /SEARCH/ ISRCH,ISRT	BOVA 123
COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	BOVA 124
COMMON /CB305/ PCTT,PCTC	BOVA 125
COMMON /CB309/ FTCL,CTOL	BOVA 126
COMMON /CB310/ NELX,NSMAT,NELY	BOVA 127
COMMON /CB311/ PRINTB,PRINTS,DGX,DGY	BOVA 128
COMMON /OVER1/ A(17)	BOVA 129
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COMMON /OVER3/ C(11)	BOVA 131
COMMON /OVER4/ D(27)	BOVA 132
COMMON /OVER5/ E(53)	BOVA 133
COMMON /OVER10/ G(14)	BOVA 134
COMMON /OVER8/ F(30)	BOVA 135
COMMON /ICORES/ ICORE(10)	BOVA 136
COMMON /ICOREL/ L6,L7	BOVA 137
COMMON /SAVE/ AA(39)	BOVA 138
COMMON /FLOW/ IRS,ICUTS	BOVA 139
COMMON /BDIM/ RON(48)	BOVA 140
COMMON /PDIM/ PDT(70)	BOVA 141
COMMON /ICARD/ ICARDS	BOVA 142
COMMON /SMX/ OLOAD,PMAX,SHEARM	BOVA 143
COMMON /BTCHK/ STRAMX(30)	BOVA 144
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COMMON /STCHK/ STRAMS(30)	BOVA 146
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COMMON /CRWB/ CRB1,ICRB2,CRB3,CRB4,ICRB5,ICRB6,ICRB7,CRB8,CRB9,CRB	BOVA 149
110	BOVA 150
COMMON /PREDN/ PRED,IDX,IDY	BOVA 151
COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	BOVA 152
COMMON /PBEAM/ NPBEAM(20)	BOVA 153
COMMON /PRECK/ PRECRK	BOVA 154
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INTEGER G	BOVA 156
DIMENSION NFROB(16)	BOVA 157
C	BOVA 158
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C*****	BOVA 161
C	BOVA 162
C	BOVA 163
IN=1	BOVA 164
IO=2	BOVA 165
C	BOVA 166
C PRINT THE BANNER PAGE	BOVA 167
C	BOVA 168
WRITE(IO,100)	BOVA 169
WRITE(IO,101)	BOVA 170
WRITE(IO,102)	BOVA 171
100 FORMAT(1H1,/////,	BOVA 172
140X,52H* * * * * ,/,	BOVA 173
240X,52H*	BOVA 174
340X,52H*	BOVA 175



440X,52H*		* ,/,	BOVA 176
540X,52H*	B O V A	* ,/,	BOVA 177
640X,52H*		* ,/,	BOVA 178
740X,52H*	( BRIDGE OVERLOAD ANALYSIS )	* ,/,	BOVA 179
840X,52H*		* ,/,	BOVA 180
940X,52H*		* )	BOVA 181
101 FORMAT(			BOVA 182
\$40X,52H*		* ,/,	BOVA 183
140X,52H*	COMPUTER PROGRAM	* ,/,	BOVA 184
240X,52H*	TO PREDICT THE OVERLOAD RESPONSE	* ,/,	BOVA 185
340X,52H*	OF	* ,/,	BOVA 186
440X,52H*	SIMPLE SPAN BEAM-SLAB BRIDGES	* ,/,	BOVA 187
540X,52H*	WITH P/S OR R.C. I-BEAMS	* ,/,	BOVA 188
640X,52H*	AND R.C. DECK SLAB	* ,/,	BOVA 189
740X,52H*		* ,/,	BOVA 190
840X,52H*		* ,/,	BOVA 191
940X,52H*	BY	* )	BOVA 192
102 FORMAT(			BOVA 193
\$40X,52H*	CELAL N. KOSTEM	* ,/,	BOVA 194
140X,52H*	WILLIAM S. PETERSON	* ,/,	BOVA 195
240X,52H*		* ,/,	BOVA 196
340X,52H*	FRITZ ENGINEERING LABORATORY	* ,/,	BOVA 197
440X,52H*	LEHIGH UNIVERSITY	* ,/,	BOVA 198
540X,52H*	BETHLEHEM, PENNSYLVANIA	* ,/,	BOVA 199
640X,52H*	1975	* ,/,	BOVA 200
740X,52H*		* ,/,	BOVA 201
840X,52H*		* ,/,	BOVA 202
940X,52H*	***** )		BOVA 203
CALL REQMEM(700008)			BOVA 204
C			BOVA 205
C	INITIALIZE CORE REQUIREMENT CODE		BOVA 206
C			BOVA 207
	DO 110 I=1,10		BOVA 208
	ICORE(I)=0		BOVA 209
	110 CONTINUE		BOVA 210
C			BOVA 211
C	READ THE PROBLEM ID		BOVA 212
C			BOVA 213
	READ (IN,120) NPROB		BOVA 214
	120 FORMAT (16A5)		BOVA 215
	WRITE (IO,130) NPROB		BOVA 216
	130 FORMAT (1H1,///,6X,20(1H-),16A5,20(1H-))		BOVA 217
C			BOVA 218
	READ (IN,223) NELX,NELY		BOVA 219
	READ (IN,220) NULAY,NSLAYR		BOVA 220
	READ (IN,220) NOBM		BOVA 221
C			BOVA 222
C	CHECK FOR PREDISCRETIZATION		BOVA 223
C			BOVA 224
	PRED=3H NO		BOVA 225
	IF (NELX.LT.0) PRED=3HYES		BOVA 226
	IF (PRED.NE.3HYES) GO TO 150		BOVA 227
C			BOVA 228
	WRITE (IO,140) PRED,NELX,NELY		BOVA 229
	140 FORMAT (1H0,///,6X,*PREDISCRETIZATION OPTION WILL BE USED= *,A3,///BOVA		230
	1,9X,*LONGITUDINAL MESH GENERATION CODE = *,I4,///,9X,*TRANSVERSE MEBOVA		231
	2SH GENERATION CODE = *,I4)		BOVA 232
C			BOVA 233

C	DEFINE NELX AND NELY	BOVA 234
C		BOVA 235
	IDX=NELX	BOVA 236
	IDY=NELY	BOVA 237
	NELX=0	BOVA 238
	NELY=0	BOVA 239
C		BOVA 240
	IF (IDX.EQ.-1) NELX=8	BOVA 241
	IF (IDX.EQ.-2) NELX=8	BOVA 242
	IF (IDX.EQ.-3) NELX=6	BOVA 243
C		BOVA 244
	IF (IDY.LT.0) NELY=-(NOBM-1)*IDY	BOVA 245
	IF (IDY.GT.0) NELY=(NOBM-1)*IDY+IDY	BOVA 246
	IF (IDY.EQ.-100) NELY=6	BOVA 247
	IF (IDY.EQ.-200) NELY=8	BOVA 248
C		BOVA 249
	150 CONTINUE	BOVA 250
	WRITE (IO,160) NELX,NELY	BOVA 251
	160 FORMAT (//,5X,*NUMBER OF SLAB ELEMENTS ALONG THE X-AXIS        =*,I8,	BOVA 252
	1//,5X,*NUMBER OF SLAB ELEMENTS ALONG THE Y-AXIS        =*,I8,1X)	BOVA 253
C		BOVA 254
	WRITE (IO,170) NULAY,NSLAYR	BOVA 255
	170 FORMAT (//,5X,*NUMBER OF CONCRETE LAYERS IN SLAB, NULAY       =*,I8,	BOVA 256
	1//,5X,*NUMBER OF STEEL LAYERS IN SLAB, NSLAYR        =*,I8,1X)	BOVA 257
C		BOVA 258
	NBEAMX=NOBM*NELX	BOVA 259
	WRITE (IO,180) NOBM,NBEAMX	BOVA 260
	180 FORMAT (//,5X,*NUMBER OF BEAMS        =*,I8,	BOVA 261
	1//,5X,*TOTAL NUMBER OF BEAM ELEMENTS, NBEAMX        =*,I8,1X)	BOVA 262
C		BOVA 263
	IF (NOBM.GT.20) WRITE (IO,190)	BOVA 264
	190 FORMAT (1HD,//,6X,*MAXIMUM NUMBER OF BEAMS HAVE BEEN EXCEEDED*,//)	BOVA 265
	IF (NOBM.GT.20) STOP 0	BOVA 266
C		BOVA 267
	READ (IN,220) NULAYR,NULAYE,NPS	BOVA 268
	NULAYB=NULAYR+NULAYE+NPS	BOVA 269
	NRE=NULAYR+NULAYE	BOVA 270
	WRITE (IO,200) NULAYR,NULAYE,NPS	BOVA 271
	200 FORMAT (//,5X,*NUMBER OF ACTUAL LAYERS FOR BEAM        =*,I8,	BOVA 272
	1//,5X,*NUMBER OF EMBEDDED LAYERS IN BEAM        =*,I8,//,5X,*NBOVA	BOVA 273
	NUMBER OF PRESTRESS STRAND GROUPS        =*,I8,1X)	BOVA 274
C		BOVA 275
	READ (IN,220) NR,NRS	BOVA 276
	WRITE (IO,210) NR,NRS	BOVA 277
	210 FORMAT (//,5X,*MAX NUMBER OF BEAM PROPERTY SECTIONS       =*,I8,	BOVA 278
	1//,5X,*MAX NUMBER OF POINTS FOR PRESTRESS PROFILE       =*,I8,1X)	BOVA 279
C		BOVA 280
C	FIND MAXIMUM FOR DIMENSIONING ARRAYS	BOVA 281
C		BOVA 282
	IF (NRS.GT.NR) NR=NRS	BOVA 283
C		BOVA 284
	READ (IN,220) NPSEGM	BOVA 285
	220 FORMAT (10I5)	BOVA 286
	IF (NPSEGM.EQ.0) NPSEGM=1	BOVA 287
	WRITE (IO,230) NPSEGM	BOVA 288
	230 FORMAT (//,5X,*NUMBER OF SEGMENTS        =*,I8)	BOVA 289
	NPSEGM=(NELX+1)*(NELY+1)/NPSEGM	BOVA 290
C		BOVA 291

IF (NULAY.GT.15) WRITE (IO,240)	BOVA 292
240 FORMAT (1H0,/,/,6X,*MAXIMUM NUMBER OF SLAB CONCRETE LAYERS HAS BEEN	BOVA 293
1 EXCEEDED*,/)	BOVA 294
IF (NULAY.GT.15) STOP 0	BOVA 295
IF (NSLAYR.GT.6) WRITE (IO,250)	BOVA 296
250 FORMAT (1H0,/,/,6X,*MAXIMUM NUMBER OF SLAB STEEL LAYERS HAS BEEN EX	BOVA 297
1 CEDED*,/)	BOVA 298
IF (NSLAYR.GT.6) STOP 0	BOVA 299
C	BOVA 300
C*****	BOVA 301
C DATA INPUT AND INITIAL SOLUTION PROCEDURES	BOVA 302
C*****	BOVA 303
C	BOVA 304
C	BOVA 305
C SET UP COMMON BLOCKS OVER1, ETC WHICH CONTAIN ARRAY LOCATIONS	BOVA 306
C	BOVA 307
C CALL OVERLAY (4LECCP,9,0)	BOVA 308
C	BOVA 309
C FIND CORE REQUIERMENTS	BOVA 310
C	BOVA 311
CALL OVERLAY (4LECCP,1,0)	BOVA 312
CALL OVERLAY (4LECCP,2,0)	BOVA 313
CALL OVERLAY (4LECCP,3,0)	BOVA 314
CALL OVERLAY (4LECCP,4,0)	BOVA 315
CALL OVERLAY (4LECCP,5,0)	BOVA 316
I12=NCNS	BOVA 317
I22=NCNS	BOVA 318
I32=NCNS	BOVA 319
CALL OVERLAY (4LECCP,6,0)	BOVA 320
CALL OVERLAY (4LECCP,7,0)	BOVA 321
CALL OVERLAY (4LECCP,8,0)	BOVA 322
CALL OVERLAY (4LECCP,10,0)	BOVA 323
C	BOVA 324
NRFL=ICORE(1)+A(17)	BOVA 325
I=1	BOVA 326
WRITE (IO,260) I,ICORE(I),NRFL	BOVA 327
260 FORMAT (1H ,/,/,5X,*OVERLAY *,I2,* CM NEEDED TO LOAD OVERLAY	BOVA 328
1 = *,08,/,23X,*CM WITH DATA ARRAY STORAGE = *,08)	BOVA 329
ICORE(I)=NRFL	BOVA 330
C	BOVA 331
NRFL=ICORE(2)+B(22)	BOVA 332
I=2	BOVA 333
WRITE (IO,260) I,ICORE(I),NRFL	BOVA 334
ICORE(I)=NRFL	BOVA 335
C	BOVA 336
NRFL=ICORE(3)+C(11)	BOVA 337
I=3	BOVA 338
WRITE (IO,260) I,ICORE(I),NRFL	BOVA 339
ICORE(I)=NRFL	BOVA 340
C	BOVA 341
NRFL=ICORE(4)+D(27)	BOVA 342
I=4	BOVA 343
WRITE (IO,260) I,ICORE(I),NRFL	BOVA 344
ICORE(I)=NRFL	BOVA 345
C	BOVA 346
NRFL=ICORE(5)+E(53)	BOVA 347
I=5	BOVA 348
WRITE (IO,260) I,ICORE(I),NRFL	BOVA 349

	ICORE(I)=NRFL	BOVA 350
C		BOVA 351
	NRFL=ICORE(6)+L6	BOVA 352
	I=6	BOVA 353
	WRITE (IO,260) I,ICORE(I),NRFL	BOVA 354
	ICORE(I)=NRFL	BOVA 355
C		BOVA 356
	NRFL=ICORE(7)+L7	BOVA 357
	I=7	BOVA 358
	WRITE (IO,260) I,ICORE(I),NRFL	BOVA 359
	ICORE(I)=NRFL	BOVA 360
C		BOVA 361
	NRFL=ICORE(8)+F(30)	BOVA 362
	I=8	BOVA 363
	WRITE (IO,260) I,ICORE(I),NRFL	BOVA 364
	ICORE(I)=NRFL	BOVA 365
C		BOVA 366
	NRFL=ICORE(9)	BOVA 367
	I=9	BOVA 368
	WRITE (IO,260) I,ICORE(I),NRFL	BOVA 369
C		BOVA 370
	NRFL=ICORE(10)+G(14)	BOVA 371
	I=10	BOVA 372
	WRITE (IO,260) I,ICORE(I),NRFL	BOVA 373
	ICORE(I)=NRFL	BOVA 374
C		BOVA 375
C		BOVA 376
C	READ AND INITIALIZE INFORMATION	BOVA 377
C		BOVA 378
	IRS=1	BOVA 379
	CALL REQMEM(ICORE(5))	BOVA 380
	CALL OVERLAY (4LECCP,5,0)	BOVA 381
C		BOVA 382
C	READ DATA FOR SLAE	BOVA 383
C		BOVA 384
	CALL REQMEM(ICORE(1))	BOVA 385
	CALL OVERLAY (4LECCP,1,0)	BOVA 386
C		BOVA 387
C	SET UP TAPES FOR STIFFNESS MATRIX FORMULATION	BOVA 388
C		BOVA 389
	I12=NCNS	BOVA 390
	I22=NCNS	BOVA 391
	I32=NCNS	BOVA 392
	ITN=19	BOVA 393
	ITO=20	BOVA 394
	CALL REQMEM(ICORE(6))	BOVA 395
	CALL OVERLAY (4LECCP,6,0)	BOVA 396
	ITN=21	BOVA 397
	ITO=22	BOVA 398
	CALL REQMEM(ICORE(6))	BOVA 399
	CALL OVERLAY (4LECCP,6,0)	BOVA 400
C		BOVA 401
	IF (NBEAMX.EQ.0) GO TO 270	BOVA 402
C		BOVA 403
C	READ DATA FOR BEAM	BOVA 404
C		BOVA 405
	CALL REQMEM(ICORE(8))	BOVA 406
	CALL OVERLAY (4LECCP,8,0)	BOVA 407

C		BOVA 408
C	SET UP TAPES FOR STIFFNESS MATRIX FORMULATION	BOVA 409
C		BOVA 410
	ITN=23	BOVA 411
	ITO=24	BOVA 412
	CALL REQMEM(ICORE(7))	BOVA 413
	CALL OVERLAY (4LECCP,7,0)	BOVA 414
	ITN=15	BOVA 415
	ITO=16	BOVA 416
	CALL REQMEM(ICORE(7))	BOVA 417
	CALL OVERLAY (4LECCP,7,0)	BOVA 418
C		BOVA 419
C	IS THERE A PRESTRESS OR DEAD LOAD SOLUTION FOR BEAMS	BOVA 420
C		BOVA 421
	IF (KPS.EQ.0) GO TO 270	BOVA 422
C		BOVA 423
C	IS THIS A RESTART RUN	BOVA 424
C		BOVA 425
	IF (ICARDS.EQ.3HYES) GO TO 270	BOVA 426
C		BOVA 427
C	OBTAIN SOLUTIONS FOR INDIVIDUAL BEAMS	BOVA 428
C		BOVA 429
	CALL REQMEM(ICORE(4))	BOVA 430
	CALL OVERLAY (4LECCP,4,0)	BOVA 431
C		BOVA 432
C	WRITE TO TAPE USED FOR STIFFNESS MATRIX FORMULATION	BOVA 433
C		BOVA 434
	ITN=15	BOVA 435
	ITO=16	BOVA 436
	CALL REQMEM(ICORE(7))	BOVA 437
	CALL OVERLAY (4LECCP,7,0)	BOVA 438
	270 CONTINUE	BOVA 439
C		BOVA 440
C	CHECK IF THIS IS A RESTART RUN	BOVA 441
C		BOVA 442
	IF (ICARDS.EQ.3HYES) ISRCH=3HNO	BOVA 443
	IF (ICARDS.EQ.3HYES) IDEAD=3H NO	BOVA 444
	IF (ICARDS.EQ.3HYES) ISCALE=3H NO	BOVA 445
	IF (ICARDS.EQ.3HYES) GO TO 280	BOVA 446
C		BOVA 447
C	OBTAIN SOLUTION TO DETERMINE STRAIN POSITION	BOVA 448
C	WITHIN ELEMENTS	BOVA 449
C		BOVA 450
	CALL REQMEM(ICORE(2))	BOVA 451
	IF (ISRCH.EQ.3HYES) CALL OVERLAY (4LECCP,2,0)	BOVA 452
	CALL REQMEM(ICORE(3))	BOVA 453
	IF (ISRCH.EQ.3HYES) CALL OVERLAY (4LECCP,3,0)	BOVA 454
	IRS=7	BOVA 455
	CALL REQMEM(ICORE(5))	BOVA 456
	CALL OVERLAY (4LECCP,5,0)	BOVA 457
C		BOVA 458
C	SET A RETURN CODE	BOVA 459
C		BOVA 460
	IRS=3	BOVA 461
C		BOVA 462
C	CHECK FOR DEAD LOAD	BOVA 463
C		BOVA 464
	IF (IDEAD.EQ.3HYES) IRS=6	BOVA 465

	IF (IDEAD.EQ.3HYES) GC TO 310	BOVA 466
C		BOVA 467
C	CHECK FOR SCALING	BOVA 468
C		BOVA 469
	IF (ISCALE.NE.3HYES) GO TO 280	BOVA 470
	GO TO 310	BOVA 471
C		BOVA 472
C	*****	BOVA 473
C	OVERLOAD SOLUTION PROCEDURE	BOVA 474
C	*****	BOVA 475
C		BOVA 476
	280 CONTINUE	BOVA 477
	CALL REQMEM(ICORE(5))	BOVA 478
	CALL OVERLAY (4LECCP,5,0)	BOVA 479
	GO TO (310,300,290,320), IOUTS	BOVA 480
	290 CONTINUE	BOVA 481
C		BOVA 482
C	SET UP BEAM TAPE	BOVA 483
C		BOVA 484
	ITN=15	BOVA 485
	ITO=16	BOVA 486
	CALL REQMEM(ICORE(7))	BOVA 487
	IF (NBEAMX.NE.0) CALL OVERLAY (4LECCP,7,0)	BOVA 488
	300 CONTINUE	BOVA 489
C		BOVA 490
C	SET UP SLAB TAPE	BOVA 491
C		BOVA 492
	ITN=21	BOVA 493
	ITO=22	BOVA 494
	CALL REQMEM(ICORE(6))	BOVA 495
	CALL OVERLAY (4LECCP,6,0)	BOVA 496
	310 CONTINUE	BOVA 497
C		BOVA 498
C	SET UP SLAB TAPE	BOVA 499
C		BOVA 500
	ITN=19	BOVA 501
	ITO=20	BOVA 502
	CALL REQMEM(ICORE(6))	BOVA 503
	CALL OVERLAY (4LECCP,6,0)	BOVA 504
C		BOVA 505
C	SET UP BEAM TAPE	BOVA 506
C		BOVA 507
	ITN=23	BOVA 508
	ITO=24	BOVA 509
	CALL REQMEM(ICORE(7))	BOVA 510
	IF (NBEAMX.NE.0) CALL OVERLAY (4LECCP,7,0)	BOVA 511
C		BOVA 512
C	SET UP STIFFNESS MATRIX AND SOLVE	BOVA 513
C		BOVA 514
	CALL REQMEM(ICORE(2))	BOVA 515
	CALL OVERLAY (4LECCP,2,0)	BOVA 516
	CALL REQMEM(ICORE(3))	BOVA 517
	CALL OVERLAY (4LECCP,3,0)	BOVA 518
	GO TO 280	BOVA 519
	320 CONTINUE	BOVA 520
C		BOVA 521
C	DO PRINTER PLOTS	BOVA 522
C		BOVA 523

	CALL REQMEM(ICORE(10))	BOVA 524
	CALL OVERLAY (4LECCP,10,0)	BOVA 525
	GO TO 280	BOVA 526
C	STOP	BOVA 527
	END	BOVA 528
	SUBROUTINE BAXIAL (S1,S2,AL,SP,EP,DIRECT,E1B)	BOVA 529
C		BAXIAL 2
		BAXIAL 3
C	BIAXIAL STRESS-STRAIN RELATION FOR THE SLAB CONCRETE LAYER	BAXIAL 4
C		BAXIAL 5
C	STRESS STATE MAY FALL INTO ONE OF FOUR REGIONS	BAXIAL 6
C	REFER TO FRITZ LAB REPORT 378B.3 FOR DETAILS	BAXIAL 7
C		BAXIAL 8
	COMMON /PROPCB/ ZA,EZA,ZB,EZB	BAXIAL 9
	COMMON /PROPC/ V,EC,FC,ECOMP,FT,ET,PCOMP,ALMM,SALMM,EALMM,ALICC	BAXIAL10
	COMMON /TYPENN/ TYPE	BAXIAL11
C		BAXIAL12
C	INITIALIZE	BAXIAL13
C		BAXIAL14
	RV=1.0/V	BAXIAL15
	SIGNR=1.0	BAXIAL16
	TYPE=5HNONLR	BAXIAL17
	EPEAK=0.0	BAXIAL18
	DIRECT=5HNONE	BAXIAL19
	IF (S1.EQ.0.0) GO TO 110	BAXIAL20
C		BAXIAL21
C	SET REGION CODE	BAXIAL22
C		BAXIAL23
	AL=S2/S1	BAXIAL24
	IF (ABS(1.0-AL*V).LT.1.0E-012) AL=AL-AL*0.000001	BAXIAL25
	IA=2	BAXIAL26
	IF (S1.GT.0.0) IA=0	BAXIAL27
	IB=2	BAXIAL28
	IF (S2.GT.0.0) IB=1	BAXIAL29
	ICODE=IA+IB	BAXIAL30
C		BAXIAL31
C	TRANSFERE TO STRESS SPACE REGION	BAXIAL32
C		BAXIAL33
	GO TO (10,50,70,90), ICODE	BAXIAL34
C		BAXIAL35
10	CONTINUE	BAXIAL36
C		BAXIAL37
C	COMP.-COMP. REGION	BAXIAL38
C		BAXIAL39
	IF (AL.GE.0.0.AND.AL.LT.V) GO TO 20	BAXIAL40
	IF (AL.GE.V.AND.AL.LE.1.0) GO TO 30	BAXIAL41
	IF (AL.GT.1.0.AND.AL.LE.RV) GO TO 40	BAXIAL42
	SP=1.2*FC/(1.2*AL-1.0)	BAXIAL43
	EP=V*ECOMP*(SP/(1.2*V*FC)-1.0)*1.0E-06	BAXIAL44
	ES=SP/EP	BAXIAL45
	SIGNR=-1.0	BAXIAL46
	CALL ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E1B)	BAXIAL47
	RETURN	BAXIAL48
C		BAXIAL49
20	CONTINUE	BAXIAL50
	SP=(1.2*FC)/(1.2-AL)	BAXIAL51
	EP=ECOMP*1.0E-06	BAXIAL52
	ES=SP/EP	BAXIAL53

CALL ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E18)  
DIRECT=5HCRUSH  
RETURN

BAXIAL54  
BAXIAL55  
BAXIAL56

C  
30 CONTINUE  
SP=1.2\*FC

BAXIAL57  
BAXIAL58  
BAXIAL59

EP=ECOMP\*1.0E-06  
ES=SP/EP  
DIRECT=5HCRUSH

BAXIAL60  
BAXIAL61  
BAXIAL62

CALL ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E18)  
RETURN

BAXIAL63  
BAXIAL64  
BAXIAL65

C  
40 CONTINUE  
SP=1.2\*FC/AL  
EP=(ECOMP/(1.2\*(1.0-V)))\*(SP/FC-1.2\*V)\*1.0E-06  
ES=SP/EP

BAXIAL66  
BAXIAL67  
BAXIAL68

CALL ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E18)  
RETURN

BAXIAL69  
BAXIAL70  
BAXIAL71

C  
50 CONTINUE

BAXIAL72  
BAXIAL73

C  
COMP.- TENSION REGION

BAXIAL74  
BAXIAL75

C  
ALIM=1.0/ALMM

BAXIAL76  
BAXIAL77

C  
REGION 20  
SP=(ALIM-AL)/FT+1.0/(SALMM\*FC)  
SP=1.0/SP

BAXIAL78  
BAXIAL79  
BAXIAL80

C  
REGION 21  
IF (AL.GE.ALIM) SP=FC/(1.0+AL\*(1.0-SALMM)/(ALIM\*SALMM))

BAXIAL81  
BAXIAL82

C  
REGION 20  
EP=((PCOMP-V\*ET)\*SP/(EALMM\*FC)+V\*ET)\*1.0E-06

BAXIAL83  
BAXIAL84

C  
REGION 21  
IF (SP.GE.EALMM\*FC) EP=((ECOMP-PCOMP)\*(SP/FC-1.0)/(1.0-EALMM)+ECOMP

BAXIAL85  
BAXIAL86

1P)\*1.0E-06  
ES=SP/EP  
IF (AL.GE.1.0/ALICC) DIRECT=5HCRUSH

BAXIAL87  
BAXIAL88  
BAXIAL89

ECP=EZA+(EZA-EZB)\*(AL-ZA)/(ZA-ZB)  
IF (ECP.GE.1.0) GO TO 60  
IF (ECP.LT.0.0) ECP=0.0

BAXIAL90  
BAXIAL91  
BAXIAL92

EPEAK=ECP\*EC/(1.0-V\*AL)  
IF (EPEAK.GE.ES) GO TO 60  
IF (EPEAK.NE.0.0) TYPE=5HLINRR

BAXIAL93  
BAXIAL94  
BAXIAL95

CALL ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E18)  
RETURN

BAXIAL96  
BAXIAL97

60 CONTINUE

BAXIAL98

E18=ES  
TYPE=5HLINER  
RETURN

BAXIAL99  
BAXIA100  
BAXIA101

C  
70 CONTINUE

BAXIA102  
BAXIA103

C  
TENSION - COMP. REGION

BAXIA104  
BAXIA105

C  
REGION 30  
SP=ALMM\*(1.0/SALMM-1.0)+AL  
SP=FC/SP

BAXIA106  
BAXIA107  
BAXIA108  
BAXIA109

C  
REGION 31  
IF (AL.GT.ALMM) SP=FT/(AL\*(1.0/ALMM+FT/(SALMM\*FC))-1.0)

BAXIA110  
BAXIA111



C	REGION 30	BAXIA112
C	REGION 31	BAXIA113
	$EP = ((ET - V * ECOMP) * (SP / FT + 1.0) - ET) * 1.0E-06$	BAXIA114
	ES = SP / EP	BAXIA115
	IF (AL.GT.ALICC) DIRECT=5HCRACK	BAXIA116
	$ECP = EZA + (EZA - EZB) * (1.0 / AL - ZA) / (ZA - ZB)$	BAXIA117
	IF (ECP.GE.1.0) GO TO 80	BAXIA118
	IF (ECP.LT.0.0) ECP=0.0	BAXIA119
	$EPEAK = ECP * EC / (1.0 - V * AL)$	BAXIA120
	IF (EPEAK.GE.ES) GO TO 80	BAXIA121
	IF (EPEAK.NE.0.0) TYPE=5HLINRR	BAXIA122
	SIGNR=-1.0	BAXIA123
	CALL ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E1B)	BAXIA124
	RETURN	BAXIA125
80	CONTINUE	BAXIA126
	E1B=ES	BAXIA127
	TYPE=5HLINER	BAXIA128
	RETURN	BAXIA129
C		BAXIA130
90	CONTINUE	BAXIA131
C		BAXIA132
C	TENSION -TENSION REGION	BAXIA133
C		BAXIA134
	IF (AL.LE.1.0) GO TO 100	BAXIA135
	SP=-FT/AL	BAXIA136
	$ES = EC / (1.0 - V * AL)$	BAXIA137
	EP=SP/ES	BAXIA138
	TYPE=5HLINER	BAXIA139
	E1B=ES	BAXIA140
	RETURN	BAXIA141
C		BAXIA142
100	CONTINUE	BAXIA143
	SP=-FT	BAXIA144
	$ES = EC / (1.0 - V * AL)$	BAXIA145
	EP=SP/ES	BAXIA146
	TYPE=5HLINER	BAXIA147
	E1B=ES	BAXIA148
	DIRECT=5HCRACK	BAXIA149
	RETURN	BAXIA150
C		BAXIA151
110	CONTINUE	BAXIA152
	AL=0.0	BAXIA153
	IF (S2.GT.0.0) GO TO 120	BAXIA154
	IF (S2.LT.0.0) GO TO 130	BAXIA155
C		BAXIA156
C	FOR S2 EQUAL TO ZERO	BAXIA157
C	ZERO STRESS IN TWO PRINCIPAL DIRECTIONS	BAXIA158
C		BAXIA159
	SP=FC	BAXIA160
	$EP = ECOMP * 1.0E-06$	BAXIA161
	E1B=EC	BAXIA162
	ES=EC	BAXIA163
	RETURN	BAXIA164
C		BAXIA165
120	CONTINUE	BAXIA166
C		BAXIA167
C	FOR TENSILE STRAINS IN DIRECTION OF INTREST	BAXIA168
C		BAXIA169

EP=-V*ECOMP*1.0E-06	BAXIA170
SP=0.0	BAXIA171
ES=0.0	BAXIA172
E1B=0.0	BAXIA173
RETURN	BAXIA174
C	BAXIA175
130 CONTINUE	BAXIA176
C	BAXIA177
C FOR COMPRESSIVE STRAINS IN DIRECTION OF INTREST	BAXIA178
C	BAXIA179
EP=V*ET*1.0E-06	BAXIA180
SP=0.0	BAXIA181
ES=0.0	BAXIA182
DIRECT=5HCRACK	BAXIA183
E1B=ES	BAXIA184
RETURN	BAXIA185
END	BAXIA186
SUBROUTINE ESLOPE (SP,EP,ES,AL,EPEAK,SIGNR,S1,E1B)	ESLOPE 2
C	ESLOPE 3
C FIND THE INSTANTANEOUS SLOPE OF THE STRESS-STRAIN CURVE	ESLOPE 4
C	ESLOPE 5
COMMON /PROPC/ V,EC,FC,ECOMP,FT,ET,PCOMP,ALMM,SALMM,EALMM,ALICC	ESLOPE 6
E1B=0.0	ESLOPE 7
IF (ABS(S1).GE.ABS(SP)) RETURN	ESLOPE 8
F=EC/(1.0-V*AL)	ESLOPE 9
D=(1.0-EPEAK*F/(ES*ES))/(EP*EP)	ESLOPE10
C=F/(ES*EP)-1.0/EP-D*EP	ESLOPE11
G=(F/S1-C)**2-4.0*D	ESLOPE12
IF (G.LT.0.0) G=0.0	ESLOPE13
E=(F/S1-C-SQRT(G)*SIGNR)/(2.0*D)	ESLOPE14
E1B=S1*S1*(1.0/(F*E*E)-D/F)	ESLOPE15
RETURN	ESLOPE16
END	ESLOPE17
SUBROUTINE PRNCIP (SXX,SYY,SXY,S1,S2,THETA)	PRNCIP 2
C	PRNCIP 3
C FIND PRINCIPAL STRESSES	PRNCIP 4
C	PRNCIP 5
A=(SXX+SYY)/2.	PRNCIP 6
C=(SXX-SYY)/2.0	PRNCIP 7
B=SQRT(C*C+SXY*SXY)	PRNCIP 8
IF (B.LT.1.0E-12) GO TO 10	PRNCIP 9
IF (SXX.LT.A) B=-B	PRNCIP10
S1=A+B	PRNCIP11
S2=A-B	PRNCIP12
IF (ABS(C).LT.1.00E-12) GO TO 20	PRNCIP13
THETA=0.5*(ATAN(SXY/C))*57.29578	PRNCIP14
GO TO 30	PRNCIP15
10 THETA=0.0	PRNCIP16
S1=A	PRNCIP17
S2=A	PRNCIP18
GO TO 30	PRNCIP19
20 THETA=45.0	PRNCIP20
IF ((SXY*B).LT.0.) THETA=-45.	PRNCIP21
30 RETURN	PRNCIP22
END	PRNCIP23
SUBROUTINE TRANS (DD,THETA,DDD)	TRANS 2
C	TRANS 3
C ROTATE THE ELASTICITY MATRIX TO A DIFFERENT COORDINATE SYSTEM	TRANS 4

C		TRANS	5
	DIMENSION DD(3,3), DDD(3,3), A(3,3), T(3,3), TT(3,3)	TRANS	6
C		TRANS	7
C	TRANSFORMATION MATRIX T	TRANS	8
C		TRANS	9
	RAD=THE TA/57.29578	TRANS	10
	C=COS(RAD)	TRANS	11
	S=SIN(RAD)	TRANS	12
	T(1,1)=T(2,2)=C*C	TRANS	13
	T(1,2)=T(2,1)=S*S	TRANS	14
	T(1,3)=-2.0*C*S	TRANS	15
	T(2,3)=-T(1,3)	TRANS	16
	T(3,1)=-T(1,3)/2.0	TRANS	17
	T(3,2)=-T(3,1)	TRANS	18
	T(3,3)=T(1,1)-T(1,2)	TRANS	19
C		TRANS	20
C	TRANSPOSE T TO GET TT	TRANS	21
C		TRANS	22
	DO 10 I=1,3	TRANS	23
	DO 10 J=1,3	TRANS	24
	10 TT(I,J)=T(J,I)	TRANS	25
C		TRANS	26
C	FORM TRANSFORMED ELASTICITY MATRIX DDD      A = DD*TT	TRANS	27
C		TRANS	28
	DO 30 J=1,3	TRANS	29
	DO 30 I=1,3	TRANS	30
	SUM=0.0	TRANS	31
	DO 20 K=1,3	TRANS	32
	20 SUM=SUM+DD(I,K)*TT(K,J)	TRANS	33
	A(I,J)=SUM	TRANS	34
	30 CONTINUE	TRANS	35
C		TRANS	36
C	FORM TRANSFORMED ELASTICITY MATRIX DDD      DDD = T*DD*TT	TRANS	37
C		TRANS	38
	DO 50 J=1,3	TRANS	39
	DO 50 I=1,3	TRANS	40
	SUM=0.0	TRANS	41
	DO 40 K=1,3	TRANS	42
	40 SUM=SUM+T(I,K)*A(K,J)	TRANS	43
	DDD(I,J)=SUM	TRANS	44
	50 CONTINUE	TRANS	45
	RETURN	TRANS	46
	END	TRANS	47
	SUBROUTINE TRANG (SX,SY,SXY,ANG,S1,S2,S3)	TRANG	2
C		TRANG	3
C	TRANSFORMATION OF STRESS AND STRAIN TO CRACKED AXIS	TRANG	4
C	EXY = 0.5*ENGINEERING STRAIN	TRANG	5
C	E3 = 0.5*ENGINEERING STRAIN	TRANG	6
C		TRANG	7
	DIMENSION SS(3), T(3,3), SP(3)	TRANG	8
C		TRANG	9
	SS(1)=SX	TRANG	10
	SS(2)=SY	TRANG	11
	SS(3)=SXY	TRANG	12
	RAD=ANG/57.29578	TRANG	13
	C=COS(RAD)	TRANG	14
	S=SIN(RAD)	TRANG	15
	T(1,1)=C*C	TRANG	16

T(1,2)=S*S	TRANG 17
T(1,3)=2.0*S*C	TRANG 18
T(2,1)=T(1,2)	TRANG 19
T(2,2)=T(1,1)	TRANG 20
T(2,3)=-T(1,3)	TRANG 21
T(3,1)=-C*S	TRANG 22
T(3,2)=-T(3,1)	TRANG 23
T(3,3)=T(1,1)-T(1,2)	TRANG 24
<b>C</b>	TRANG 25
DO 20 I=1,3	TRANG 26
SUMS=0.0	TRANG 27
DO 10 J=1,3	TRANG 28
A=T(I,J)	TRANG 29
10 SUMS=A*SS(J)+SUMS	TRANG 30
20 SP(I)=SUMS	TRANG 31
<b>C</b>	TRANG 32
S1=SP(1)	TRANG 33
S2=SP(2)	TRANG 34
S3=SP(3)	TRANG 35
RETURN	TRANG 36
END	TRANG 37
OVERLAY (EGCP,5,0)	OVERL5 2
PROGRAM OVERL5	OVERL5 3
<b>C</b>	OVERL5 4
<b>C</b> THIS SETS UP SPACE FOR BLANK COMMON	OVERL5 5
<b>C</b> AND CALLS SUBROUTINE CONTROL	OVERL5 6
<b>C</b>	OVERL5 7
COMMON /CBS301/ COUPLE	OVERL5 8
COMMON /PROPC/ V(11)	OVERL5 9
COMMON /ITAPE/ ITA,ITO,I12,I22,I32	OVERL510
COMMON /PROPCB/ ZA,EZA,ZB,EZB	OVERL511
COMMON /EDOWN/ DUMMY5(2)	OVERL512
COMMON /SIGNIF/ NSIGNI	OVERL513
COMMON /TFAIL/ TOLF	OVERL514
COMMON /TU00/ TU0(8,12)	OVERL515
COMMON /PFACT/ PFC	OVERL516
COMMON /PLTCD/ PLTCRD	OVERL517
COMMON /COMPLT/ ICMLT	OVERL518
COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NCBM,NNN	OVERL519
COMMON /CB305/ PCTT,PCTC	OVERL520
COMMON /CB309/ FTCL,CTOL	OVERL521
COMMON /CB310/ NELX,NSMAT,NELY	OVERL522
COMMON /CB311/ PRINTB,PRINTS,DGX,DGY	OVERL523
COMMON /SCALED/ ISCALE,IFN	OVERL524
COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	OVERL525
COMMON /SEARCH/ ISRCH	OVERL526
COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	OVERL527
COMMON /CB2/ NBEAMX,NIX,NULAYB	OVERL528
COMMON /CB3/ LWIDTH,NITA,NPSEGM	OVERL529
COMMON /CB4/ IN,IC	OVERL530
COMMON /BDIM/ RON(48)	OVERL531
COMMON /PDIM/ PDT(70)	OVERL532
COMMON /ICARD/ ICARDS	OVERL533
COMMON /SMX/ OLOAD,PMAX	OVERL534
COMMON /BTCHK/ STRAMX(30)	OVERL535
COMMON /HISTB/ NVALB(30),IVALB(60)	OVERL536
COMMON /STCHK/ STRAMS(30)	OVERL537
COMMON /HISTS/ NVALS(30),IVALS(60)	OVERL538

COMMON /CRWS/ CRS(10)	OVERL539
COMMON /CRWB/ CRB(10)	OVERL540
COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	OVERL541
COMMON /SAVE/ AA(39)	OVERL542
COMMON /PRECK/ PRECRK	OVERL543
COMMON /ICORES/ ICR(4),ICCRE	OVERL544
COMMON /FLOW/ IRS,ICUTS	OVERL545
COMMON /OVER5/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13,N14,N15,OVERL546	OVERL546
1N16,N17,N18,N19,N20,N21,N22,N23,N24,N25,N26,N27,N28,N29,N30,N31,N30OVERL547	OVERL547
22,N33,N34,N35,N36,N37,N38,N39,N40,N41,N42,N43,N44,N45,N46,N47,N48,OVERL548	OVERL548
3N49,N50,N51,N52,N53	OVERL549
COMMON A(1)	OVERL550
C	OVERL551
IF (ICORE.NE.0) GO TO 10	OVERL552
ICORE=LOCF(A(1))+1	OVERL553
GO TO 20	OVERL554
10 CONTINUE	OVERL555
CALL CONTRL (NA1,NUMEL,NCNS,NULAY,NPERBM,NULAYB,NPEAMX,A(N1),A(N2)OVERL556	OVERL556
1,A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),A(N9),A(N10),A(N11),A(N12),A(1OVERL557	OVERL557
2N13),A(N14),A(N15),A(N16),A(N17),A(N18),A(N19),A(N20),A(N21),A(N22)OVERL558	OVERL558
3),A(N23),A(N24),A(N25),A(N26),A(N27),A(N28),A(N29),A(N30),A(N31),A(1OVERL559	OVERL559
4(N32),A(N33),A(N34),A(N35),A(N36),A(N37),A(N38),A(N39),A(N40),A(N4)OVERL560	OVERL560
51),A(N42),A(N43),A(N44),A(N45),A(N46),A(N47),A(N48),A(N49),A(N50),OVERL561	OVERL561
6A(N51),A(N52))	OVERL562
20 CONTINUE	OVERL563
END	OVERL564
SUBROUTINE CONTRL (P,C,D,E,F,G,H,FORCE,LILA,FORCEA,DISPLA,DISP,DISCONTRL 2	CONTRL 2
1PO,FORCEB,FCADD,FCSADD,CTXX,CTYY,CTXY,ETXX,ETYY,ETXY,CUXX,CUYX,CUXCONTRL 3	CONTRL 3
2Y,EIXX,EIYY,EIXY,AS1,AF,AS,SIGXXO,SIGYYO,SIGXYO,SXXOT,SYOT,SYOT,CONTRL 4	CONTRL 4
3EXTRA,EXTRA1,EXTRA2,EXTRA3,EXTRA4,EXTRA5,EXTRA6,EXTRA7,EIB,CUB,DUMCONTRL 5	CONTRL 5
4,EPSPS,SIBXXA,SIBXXC,ASXLR,AILR,ZCRD,TSHEAR,ITYPE,ESX,AFT,AST)	CONTRL 6
C	CONTRL 7
C THIS SUBROUTINE CONTROLS THE PROGRAM FLOW FOR DATA INPUT,	CONTRL 8
C RESTART, DEAD LOAD SOLUTION FOR THE WHOLE STRUCTURE,	CONTRL 9
C SCALING, AND INCREMENTAL SOLUTION UP TO FAILURE	CONTRL10
C	CONTRL11
COMMON /CBS301/ COUPLE	CONTRL12
COMMON /PROPC/ V(11)	CONTRL13
COMMON /ITAPE/ ITN,IT0,I12,I22,I32	CONTRL14
COMMON /PROPCB/ ZA,EZA,ZB,EZE	CONTRL15
COMMON /EDOWN/ DUMMY5(2)	CONTRL16
COMMON /SIGNIF/ NSIGNI(2)	CONTRL17
COMMON /TFAIL/ TOLF	CONTRL18
COMMON /TU00/ TU0(8,12)	CONTRL19
COMMON /PFACT/ PFC	CONTRL20
COMMON /SMX/ OLOAD,PMAX	CONTRL 21
COMMON /HISTB/ NVALB(6,5),IVALB(6,2,5)	CONTRL22
COMMON /HISTS/ NVALS(6,5),IVAL(6,2,5)	CONTRL23
COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	CONTRL24
COMMON /PLTCD/ PLTCRD	CONTRL25
COMMON /COMFLT/ ICMLT	CONTRL26
COMMON /PRECK/ PRECRK	CONTRL27
COMMON /SCALED/ ISCALE,IFN	CONTRL28
COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	CONTRL29
COMMON /SEARCH/ ISRCH,ISRT	CONTRL30
COMMON /CRWS/ CRS1,CRS2,CRS3,CRS4,CRS5,CRS6,CRS7,ICRS8,CRS9,ICRS10CONTRL31	CONTRL31
COMMON /CRWB/ CRB1,ICRB2,CRB3,CRB4,ICRB5,ICRB6,ICRB7,CRB8,CRB9,CRB)CONTRL32	CONTRL32
110	CONTRL33

	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNM	CONTRL34
	COMMON /CB305/ PCTT,PCTC	CONTRL35
	COMMON /CB309/ FTCL,CTOL	CONTRL36
	COMMON /CB310/ NELX,NSMAT,NELY	CONTRL37
	COMMON /CB311/ PRINTB,PRINTS,DGX,DGY	CONTRL38
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	CONTRL39
	COMMON /CB2/ NBEAMX,NIX,NULAYB	CONTRL40
	COMMON /CB3/ LWIDTH,NITA,NPSEGM	CONTRL41
	COMMON /CB4/ IN,IO	CONTRL42
	COMMON /PDIM/ PDT(15),PZC(15),SEMOD(4),SIGMAP(4),SPRON(4),SPROM(4)	CONTRL43
	1,NSTYPE(6),SDT(6),SZC(6),SPHI(6)	CONTRL44
	COMMON /BDIM/ RON(6),ROM(6),EDOWN(6),STRAN(6),SIGC(6),FT(6),RONT(3	CONTRL45
	1),ROMT(3),EDOWNT(3),EDOWNR(3)	CONTRL46
	COMMON /ICARD/ ICARDS	CONTRL47
	COMMON /SAVE/ AA,COEA,COET,DPFC,DSPMAX,ECARDS,ETIME,HDISP,HFORCE	CONTRL48
	1,CQAPA,QAPA,RQAPA,QAPAC,ICHECK,NCHECK,ICON,ICYCLE,LCYCLE,INOTEN,IO	CONTRL49
	2NE,IOUT,ITER,NITER,LITER,ITRIAL,NTRIAL,LTRIAL,ITP,RATIO,RA	CONTRL50
	3TIO,RFORCE,ROLD,TOLDSP,RDPR,RFPR,RDFPR,ROLD,RFOLD	CONTRL51
	COMMON /FLOW/ IRS,ICUTS	CONTRL52
	INTEGER P,C,D,E,F,G,H	CONTRL53
C	P = NA1, C = NUMEL, D = NCNS, E = NULAY, F = NPEREM, G = NULAYB	CONTRL54
C	H = NBEAMX	CONTRL55
C		CONTRL56
	DIMENSION FORCE(P), LILA(P), FORCEA(P), DISPLA(P), DISP(P), DISPO(	CONTRL57
	1P), FCADD(P), FCSADD(P), FORCEB(P), CTXX(C), CTYY(C), CTXY(C), CUX	CONTRL58
	2X(C), CUY(C), CUXY(C), ETXX(C), ETYY(C), ETXY(C), EIXX(C), EIYY(	CONTRL59
	3), EIXY(C), SIGXX(D), SIGYY(D), SIGXY(D), SXXOT(D), SYOT(D), S	CONTRL60
	4XYOT(D), AS1(C,D), AF(C,E), AS(C,E), AFT(D), AST(D), EXTRA(3), EXT	CONTRL61
	5RA1(G,3), EXTRA2(G,3), EXTRA3(G), EXTRA4(G,3), EXTRA5(G,3), EXTRA	CONTRL62
	6(G,3), EXTRA7(G), EIB(H), CUB(H), DUM(G), EPSPS(G), SIBXXA(G), SIB	CONTRL63
	7XXC(G), ESX(G,H), ASXLR(G), AILR(G), ZCRD(G), TSHEAR(G), ITYPE(G),	CONTRL64
	8 DUMMY(1)	CONTRL65
C		CONTRL66
C	THIS CONTROLS PROGRAM FLOW USING CODE IRS	CONTRL67
C		CONTRL68
	GO TO (10,100,380,1150,1530,160,30), IRS	CONTRL69
	10 CONTINUE	CONTRL70
C		CONTRL71
C	*****	CONTRL72
C		CONTRL73
C	READ INFORMATION AND INITIALIZE	CONTRL74
C		CONTRL75
C	*****	CONTRL76
C		CONTRL77
	CALL READCN (NUMEL,NUMNP,LWIDTH,ISCALE,IDEAD,ICMPLT,PRECRK,PLTORD,	CONTRL78
	1ICARDS,ECARDS,COEA,COET,PRINTB,PRINTS,DGX,DGY,ETIME,NSIGNI,DSPMA	CONTRL79
	2X,TOLDSP,RDPR,RFPR,NTRIAL,NITER,LITER,LTRIAL,PCTT,PCTC,LCYCLE,RRAT	CONTRL80
	3IO,TOLF,QAPA,RQAPA,CQAPA,NITERD,INOTEN,OLOAD,PMAX,NCHECK,DPFC,NREA	CONTRL81
	4DB,KIN,KPS,KC,KMAT,FTCL,CTOL)	CONTRL82
C		CONTRL83
	WRITE (IO,20)	CONTRL84
	WRITE (IO,20)	CONTRL85
	20 FORMAT (1H0)	CONTRL86
C		CONTRL87
	IOUT=3HNO	CONTRL88
	ICON=3HNO	CONTRL89
	ISTAGE=1	CONTRL90
	ICHECK=-1	CONTRL91

	ITRIAL=0	CONTRL92
	ITER=0	CONTRL93
	QAPAC=1.0	CONTRPL94
	KSIG=0	CONTRL95
	RDFPR=3HYES	CONTRL96
	RDOLD=0.0	CONTRL97
	RFOLD=0.0	CONTRL98
	IFN=3H NO	CONTRL99
	ITRALD=0	CONTR100
	ITERD=0	CONTR101
	HDISP=1.0	CONTR102
	HFORGE=1.0	CONTR103
	PFY=1.0	CONTR104
	IF (PLTCRD.EQ.3HYES) REWIND 8	CONTR105
	IONE=3HNO	CONTR106
	IF (LTRIAL.EQ.1.AND.LITER.EQ.1) IONE=3HYES	CONTR107
C		CONTR108
C	INITIALIZE PLATE INFORMATION	CONTR109
C		CONTR110
	CALL INITIA (NUMEL,NCNS,NULAY,NA1,ETXX,SXXOT,AS1,AF,AS,AFT,DISPLA)	CONTR111
C		CONTR112
C	INITIALIZE BEAM INFORMATION	CONTR113
C		CONTR114
	IF (NBEAMX.NE.0) CALL INITIB (NBEAMX,NULAYB,NNN,SI8XXA)	CONTR115
C		CONTR116
C		CONTR117
	IRS=2	CONTR118
	RETURN	CONTR119
C		CONTR120
C	READ IN PLATE INFORMATION	CONTR121
C		CONTR122
	30 CONTINUE	CONTR123
	IF (ISRT.EQ.3HAVE) RETURN	CONTR124
C		CONTR125
C	DETERMINE WHICH NODE POINT OF THE ELEMENTS WILL BE	CONTR126
C	USED TO COMPUTE THE STRAINS AND STRESSES	CONTR127
C	IF ISRCH = NO THEN THE CENTER OF THE ELEMENT WILL BE USED	CONTR128
C		CONTR129
C		CONTR130
	REWIND 30	CONTR131
	READ (30) DISP	CONTR132
C		CONTR133
	IF (ISRCH.EQ.3HYES) CALL FSTRAN (NUMEL,NA1,NANA,NULAY,CUXX,CUYY,CU	CONTR134
	1XY,EIXX,EIYY,EIXY,DISP,PDT,PZC,SZC,AFT,AST,NSLAYR)	CONTR135
C		CONTR136
	WRITE (IO,40)	CONTR137
40	FORMAT (1H1,////)	CONTR138
	WRITE (IO,50)	CONTR139
50	FORMAT (1H0,//,6X,*SLA3                    NODE POINT AT WHICH ELEMENT*,//,6X,CONTR140	
	1*ELEMENT                    STRESSES ARE COMPUTED*,//,18X,* ( 0 = CENTER OF ELEMENCONTR141	
	2T)*,//)	CONTR142
C		CONTR143
	REWIND 32	CONTR144
	DO 60 I=1,NUMEL	CONTR145
60	READ (32) AFT,AST	CONTR146
C		CONTR147
	DO 70 I=1,NUMEL	CONTR148
	READ (32) J,M,N	CONTR149

70 WRITE (10,80) I,J	CONTR150
80 FORMAT (1H ,8X,I3,26X,I3)	CONTR151
C	CONTR152
ISRCH=3HNO	CONTR153
C	CONTR154
DO 90 I=1,NA1	CONTR155
90 DISP(I)=0.	CONTR156
C	CONTR157
REWIND 30	CONTR158
WRITE (30) DISP	CONTR159
C	CONTR160
RETURN	CONTR161
C	CONTR162
C	CONTR163
100 CONTINUE	CONTR164
C	CONTR165
C	CONTR166
C*****	CONTR167
C RESTART USING INITIAL START CARDS ON FILE STARTI	CONTR168
C	CONTR169
C*****	CONTR170
C	CONTR171
IN=9	CONTR172
REWIND 19	CONTR173
REWIND 21	CONTR174
REWIND 32	CONTR175
REWIND IN	CONTR176
READ (IN) ICYCLE,ITRIAL,ITER,QAPAC,PFC,HDISP,HFORCE,PFY	CONTR177
READ (IN) FVAL(1),FVAL(2),FVAL(3),FVAL(4),FVAL(5),FVAL(6)	CONTR178
C	CONTR179
DO 110 I=1,NUMEL	CONTR180
READ (IN) SXXOT,SYYOT,SXYOT	CONTR181
READ (IN) SIGXX0,SIGYY0,SIGXY0	CONTR182
READ (IN) AFT,AST	CONTR183
WRITE (21) SXXOT,SYYOT,SXYOT	CONTR184
WRITE (19) SIGXX0,SIGYY0,SIGXY0	CONTR185
WRITE (32) AFT,AST	CONTR186
110 CONTINUE	CONTR187
C	CONTR188
DO 120 I=1,NUMEL	CONTR189
READ (IN) J,M,N	CONTR190
120 WRITE (32) J,M,N	CONTR191
C	CONTR192
READ (IN) FORCE,LILA	CONTR193
READ (IN) DISPO,FORCEB,FCADD	CONTR194
READ (IN) AS1,AF,AS	CONTR195
READ (IN) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONTR196
C	CONTR197
REWIND 17	CONTR198
WRITE (17) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONTR199
REWIND 31	CONTR200
WRITE (31) AS1,AF,AS	CONTR201
REWIND 18	CONTR202
WRITE (18) DISPO,FORCEB,FCADD	CONTR203
REWIND 29	CONTR204
WRITE (29) FORCE,LILA	CONTR205
C	CONTR206
IN=1	CONTR207



C	WRITE (10,20)	CONTR208
	WRITE (10,20)	CONTR209
	WRITE (10,880) ICYCLE	CONTR210
	WRITE (10,890) QAFAC,PFC	CONTR211
	WRITE (10,20)	CONTR212
C		CONTR213
	IOUTS=2	CONTR214
	IF (NBEAMX.EQ.0) GO TO 140	CONTR215
	IN=9	CONTR216
	REWIND 15	CONTR217
	REWIND 23	CONTR218
	REWIND 25	CONTR219
	REWIND 33	CONTR220
	DO 130 I=1,NBEAMX	CONTR221
	READ (IN) SIBXXA	CONTR222
	READ (IN) SIBXXC	CONTR223
	READ (IN) EPSPS	CONTR224
	WRITE (15) SIBXXA	CONTR225
	WRITE (23) SIBXXC	CONTR226
	WRITE (25) EPSPS	CONTR227
	130 CONTINUE	CONTR228
	READ (IN) ESX	CONTR229
	WRITE (33) ESX	CONTR230
	IN=1	CONTR231
	IOUTS=3	CONTR232
	140 CONTINUE	CONTR233
	IRS=4	CONTR234
	RETURN	CONTR235
C		CONTR236
	150 CONTINUE	CONTR237
C		CONTR238
	C*****	CONTR239
C		CONTR240
C	DEAD LOAD SOLUTION FOR WHOLE STRUCTURE	CONTR241
C		CONTR242
C	C*****	CONTR243
C		CONTR244
	IRS=6	CONTR245
	IOUTS=1	CONTR246
	IF (ITP.EQ.3HYES) ICUTS=3	CONTR247
C		CONTR248
C	SOLVE SYSTEM FOR DEAD LOAD SOLUTION	CONTR249
C		CONTR250
	RETURN	CONTR251
C		CONTR252
	160 CONTINUE	CONTR253
	IF (ITERD.EQ.0.AND.ITRALD.EQ.0) WRITE (10,40)	CONTR254
	WRITE (10,170)	CONTR255
	170 FORMAT (/,1H0,4X,*DEAD LOAD SOLUTION*)	CONTR256
	ITERD=ITERD+1	CONTR257
	WRITE (10,400) ITRALD	CONTR258
	WRITE (10,410) ITERD	CONTR259
	REWIND 18	CONTR260
	REWIND 30	CONTR261
	REWIND 31	CONTR262
	REWIND 33	CONTR263
	READ (18) DISPO,FORCEB,FCADD	CONTR264
		CONTR265

	READ (30) DISP	CONTR266
	READ (31) AS1,AF,AS	CONTR267
	READ (33) ESX	CONTR268
C		CONTR269
C	FIND SLAB STRAINS	CONTR270
C		CONTR271
	CALL PSTRAN (NUMEL,NA1,NANA,NULAY,CUXX,CUYX,CUXY,EIXX,EIYY,EIXY,DI 1SP,PDT,PZC,SZC,AFT,AST,NSLAYR)	CONTR272 CONTR273 CONTR274
C		CONTR275
C	FIND BEAM STRAINS	CONTR276
	IF (NBEAMX.NE.0) CALL BSTRAN (NIX,NULAYB,NA1,NBEAMX,EIB,CUB,DUM,DI 1SP)	CONTR277 CONTR278 CONTR279
C		CONTR280
C	FIND SLAB STRESSES	CONTR281
C		CONTR282
	PF=1.0 CALL PSTRSS (ISTAGE,PF,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYC 1,SIGXYO,CUXX,CUYX,CUXY,EIXX,EIYY,EIXY,PZC,PDT,DT,SZC,SXXOT,SYYOT,SG 2XYOT,AS1,AF,AS,SEMOD,SIGMAP,SPROM,SPRON,NSTYPE,SPHI)	CONTR283 CONTR284 CONTR285 CONTR286
C		CONTR287
C	FIND BEAM STRESSES	CONTR288
	IF (NBEAMX.NE.0) CALL BSTRSS (PF,DUM,ZCRD,EIB,CUB,KC,NULAYB,NBEAMX 1,KMAT,SIBXXA,SIBXXC,ESX,RCN,ROM,SIGO,FT,RONT,RONT,ITYPE)	CONTR289 CONTR290 CONTR291
C		CONTR292
C	CHECK CONVERGENCE	CONTR293
C		CONTR294
	DO 180 II=1,NA1 I=II A=DISP(I) B=DISP(I) IF (ABS(B).LE.1.0E-09) GO TO 180 IF (ABS((B-A)/B).GT.TOLDSP) GO TO 190	CONTR295 CONTR296 CONTR297 CONTR298 CONTR299
	180 CONTINUE ICON=3HYES GO TO 230	CONTR300 CONTR301 CONTR302
	190 CONTINUE ICON=3HNO	CONTR303 CONTR304 CONTR305
C		CONTR306
	200 DO 210 I=1,NA1 210 DISP(I)=DISP(I)	CONTR307 CONTR308 CONTR309
C		CONTR310
	IF (ITRALD.NE.0) GO TO 220 IF (ITERD.NE.1) GO TO 220	CONTR311 CONTR312
C		CONTR313
C	FIRST TIME THROUGH READ THE DEAD LOAD FORCE VECTOR	CONTR314
C		CONTR315
	REWIND 29 READ (29) FORCE,LILA	CONTR316 CONTR317
	READ (29) FORCEB 220 CONTINUE REWIND 18	CONTR318 CONTR319
	WRITE (18) DISP,FORCEB,FCADD	CONTR320
C		CONTR321
C	SET TRIAL AND ITERATION COUNTERS	CONTR322
C		CONTR323

	IF (ICON.EQ.3HYES.OR.ITERD.GE.NITERD) ITRALD=ITRALD+1	CONTR324
	IF (ICON.EQ.3HYES.OR.ITERD.GE.NITERD) ITERD=0	CONTR325
C		CONTR326
	IF (ISCALE.NE.3HYES) GO TO 230	CONTR327
	IF (IFN.EG.3HYES) GO TO 230	CONTR328
C		CONTR329
C	FIND SLAB SCALE FACTOR	CONTR330
C		CONTR331
	CALL PSCALE (RATIOB,IFN,ISCALE,SXXOT,SYOT,SXYOT,PCTT,PCTC,IO,NCNS	CONTR332
	1,NUMEL,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO,SIGXYO,NSTYPE,SPHI,SIGMAP)	CONTR333
C		CONTR334
C	FIND BEAM SCALE FACTOR	CONTR335
C		CONTR336
	IF (NBEAMX.NE.0) CALL BSCALE (RATIOB,IFN,ISCALE,KC,DUM,NBEAMX,NULAYB,	CONTR337
	1YB,KMAT,PCTT,PCTC,SIBXXA,SIBXXC,ITYPE,SIGO,FT)	CONTR338
	230 CONTINUE	CONTR339
C		CONTR340
	PFC=1.0	CONTR341
	IF (ITRALD.EQ.0) PFC=0.0	CONTR342
	IF (ITRALD.EQ.2) PFC=0.0	CONTR343
C		CONTR344
C	CHECK SLAB LAYER FAILURE	CONTR345
C		CONTR346
	REWIND 17	CONTR347
	READ (17) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONTR348
	CALL PCHECK (IO,NA1,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,NANA,PZC,PDT,SZCCONTR349	CONTR349
	1,SDT,DT,ELLA,ELLB,NPI,NPJ,NPK,NPL,FCADD,CTXX,CTYY,CTXY,CUXX,CUYX,	CONTR350
	2UXY,ETXX,ETYY,ETXY,EIXX,EIYY,EIXY,SXXOT,SYOT,SXYOT,SIGXXO,SIGYYO,	CONTR351
	3SIGXYO,AS1,AF,AS,AFT,AST,NSTYPE,SPHI,SIGMAP)	CONTR352
C		CONTR353
C	CHECK BEAM LAYER FAILURE, MODULUS, UNLOADING	CONTR354
C		CONTR355
	IF (NBEAMX.NE.0) CALL BCHECK (KC,NIX,NBEAMX,NULAYB,KMAT,NA1,IO,CUBCONTR356	CONTR356
	1,EIB,DUM,NPIX,NPKX,SIBXXC,SIBXXA,ITYPE,FT,SIGO,ESX,EDOWN,EDOWN,DICONTR357	CONTR357
	2SP,DISPLA,ZGRD,XLNGTH,ASXLR,EPSPS,STRAN,FCADD,IOUT)	CONTR358
C		CONTR359
	REWIND 33	CONTR360
	WRITE (33) ESX	CONTR361
C		CONTR362
	IF (ITRALD.EQ.2) PFC=1.0	CONTR363
	IF (PFC.NE.0.0) GO TO 240	CONTR364
	ITP=3HNO	CONTR365
	GO TO 150	CONTR366
	240 CONTINUE	CONTR367
C		CONTR368
	ITP=3HYES	CONTR369
C		CONTR370
	DO 250 I=1,NA1	CONTR371
	FORCE(I)=0.0	CONTR372
	250 FORCEA(I)=0.0	CONTR373
C		CONTR374
C	ACCUMULATE FIELD QUANTITIES AT END OF DEAD LOAD SOLUTION	CONTR375
C		CONTR376
	CALL ACCUMU (NUMEL,NA1,NBEAMX,NULAYB,NCNS,NULAY,CTXX,CTYY,CTXY,CUXCONTR377	CONTR377
	1X,CUYX,CUXY,ETXX,ETYY,ETXY,EIXX,EIYY,EIXY,SXXOT,SYOT,SXYOT,SIGXXCONTR378	CONTR378
	2,SIGYYO,SIGXYO,DISPLA,DISP,FORCE,FORCEA,ESX,ITYPE,ZGRD,DUM,EPSPS,ACONTR379	CONTR379
	3S1,AFT,AST,SIBXXC,SIBXXA,AF,AS)	CONTR380
C		CONTR381

	REWIND 17	CONTR382
	WRITE (17) FORCEA,DISPLA,FORCEA,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONTR383
	IF (ITRALD.EQ.2) GO TO 260	CONTR384
C		CONTR385
C	WRITE FICTITIOUS FORGE VECTOR TO TAPE	CONTR386
C		CONTR387
	REWIND 29	CONTR388
	READ (29) FORCE,LILA	CONTR389
	REWIND 29	CONTR390
	WRITE (29) FORCE,LILA	CONTR391
	WRITE (29) FCADD	CONTR392
	260 CONTINUE	CONTR393
C		CONTR394
C	FIND BEAM STRAINS	CONTR395
C		CONTR396
	IF (NBEAMX.EQ.0) GO TO 300	CONTR397
	REWIND 5	CONTR398
	REWIND 25	CONTR399
	REWIND 27	CONTR400
	DO 280 I=1,NBEAMX	CONTR401
	READ (27) A,A,A,A,SIBXXA,SIBXXA,SIBXXA,SIBXXA,SIBXXC	CONTR402
	READ (25) SIBXXC	CONTR403
	DO 270 J=1,NULAYB	CONTR404
	SIBXXC(J)=SIBXXC(J)+EIB(I)+CUB(I)*SIBXXA(J)	CONTR405
	270 CONTINUE	CONTR406
	WRITE (5) SIBXXC	CONTR407
	280 CONTINUE	CONTR408
	REWIND 5	CONTR409
	REWIND 25	CONTR410
	DO 290 I=1,NBEAMX	CONTR411
	READ (5) SIBXXC	CONTR412
	WRITE (25) SIBXXC	CONTR413
	290 CONTINUE	CONTR414
	300 CONTINUE	CONTR415
C		CONTR416
C	IF ITRALD = 1 THEN ITERATE SO AS TO REDISTRIBUTE FICTITIOUS FORCES	CONTR417
C	IF ITRALD = 2 THEN FINISH DEAD LOAD SOLUTION	CONTR418
C		CONTR419
	IF (ITRALD.EQ.2) GO TO 310	CONTR420
	GO TO 150	CONTR421
	310 CONTINUE	CONTR422
C		CONTR423
C	ENDING DEAD LOAD SOLUTION	CONTR424
C		CONTR425
	IDEAD=3HNO	CONTR426
	REWIND 17	CONTR427
	REWIND 18	CONTR428
	READ (17) FORCEA,DISPLA,FORCEA,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONTR429
	READ (18) DISPO,FORCEB,FCADD	CONTR430
	DO 320 I=1,NA1	CONTR431
	320 FORCEA(I)=FORCEB(I)	CONTR432
	WRITE (10,330)	CONTR433
	330 FORMAT (//,1H0,10X,*SOLUTION FOR DEAD LOAD ON THE STRUCTURE*,//)	CONTR434
	PFC=PLTCRD	CONTR435
	PLTCRD=3HNO	CONTR436
C		CONTR437
C	PRINT FORCE AND DISPLACEMENT VECTORS	CONTR438
C		CONTR439

	CALL QPRINT (HDISP,HFORCE,RDISP,RFORCE,RDFPR,NSLAYR,NULAY,NCNS,NUMCONTR440 1EL,AFT,AST,AS1,NA1,NUMNP,IO,FORCEA,DISPLA) PLTCRD=PFC	CONTR441 CONTR442
C	FIND SLAB INTERNAL FORCES	CONTR443 CONTR444 CONTR445
	CALL PFORCE (IO,NUMEL,NCNS,NULAY,NSLAYR,RDFPR,21,PZC,POT,DT,SDT,SZCONTR446 1G,SXXOT,SYOT,SXYCT)	CONTR447 CONTR448
C	FIND BEAM INTERNAL FORCES	CONTR449 CONTR450
	IF (NBEAMX.NE.0) CALL BFORCE (NULAYB,NBEAMX,IO,DUM,RDFPR,15,ASXLR,CONTR451 1SIBXXA,ZCRD)	CONTR452 CONTR453
C	PRINT SLAB STRESSES	CONTR454
	WRITE (IO,1390) WRITE (IO,470)	CONTR455 CONTR456 CONTR457
	CALL PRISTR (SXXOT,SYOT,SXYOT,1.0,21,NULAY,NUMEL,NCNS,NSLAYR,SPHI 1,IO)	CONTR458 CONTR459
C	WRITE (IO,1400) WRITE (IO,480)	CONTR460 CONTR461 CONTR462
C	PRINT BEAM STRESSES	CONTR463
	IF (NBEAMX.NE.0) CALL PRISRB (SIBXXA,15,NULAYB,NBEAMX,IO)	CONTR464 CONTR465 CONTR466
C	PRINT FICTITIOUS FORCES	CONTR467 CONTR468 CONTR469
	IF (CODEA.NE.3HYES) GO TO 350 REWIND 29 READ (29) FORCE,LILA	CONTR470 CONTR471 CONTR472
	READ (29) FCADD WRITE (IO,340) 340 FORMAT (1H0,10X,*LEFT OVER FICTITIOUS FORCES*,//)	CONTR473 CONTR474 CONTR475
	WRITE (IO,1510) WRITE (IO,440) (M,FCADD(5*M-4),FCADD(5*M-3),FCADD(5*M-2),FCADD(5*M 1-1),FCADD(5*M),M=1,NUMNP)	CONTR476 CONTR477 CONTR478
	350 CONTINUE	CONTR479
C	DO 360 I=1,NA1	CONTR480
	360 FORCEA(I)=0. REWIND 17 REWIND 18	CONTR481 CONTR482 CONTR483 CONTR484
	REWIND 30 WRITE (17) FORCEA,FORCEA,FORCEA,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY WRITE (18) FORCEA,FORCEA,FORCEA WRITE (30) FORCEA IF (ISCALE.NE.3HYES) GO TO 380 IOUTS=3	CONTR485 CONTR486 CONTR487 CONTR488 CONTR489 CONTR490
	IRS=3 RETURN	CONTR491 CONTR492 CONTR493
C	SOLVE EQUATIONS	CONTR494
	370 CONTINUE	CONTR495 CONTR496
C		CONTR497

C*****	CONTR498
C	CONTR499
C SCALE UP THE INITIAL SOLUTION TO WITHIN THE PCTT OR PCTC LIMITS	CONTR500
C	CONTR501
C*****	CONTR502
C	CONTR503
IOUTS=1	CONTR504
IRS=3	CONTR505
RETURN	CONTR506
C	CONTR507
C SOLVE SYSTEM	CONTR508
C	CONTR509
380 CONTINUE	CONTR510
C	CONTR511
C READ DATA FROM TAFES	CONTR512
C	CONTR513
REWIND 29	CONTR514
READ (29) FORCE,LILA	CONTR515
REWIND 18	CONTR516
READ (18) DISPO,FORCEB,FCADD	CONTR517
REWIND 30	CONTR518
READ (30) DISP	CONTR519
REWIND 31	CONTR520
REWIND 33	CONTR521
READ (31) AS1,AF,AS	CONTR522
C	CONTR523
READ (33) ESX	CONTR524
C	CONTR525
C FIND SLAB STRAINS	CONTR526
C	CONTR527
CALL PSTRAN (NUMEL,NA1,NANA,NULAY,CUXX,CUYX,CUXY,EIXX,EIYY,EIXY,DI	CONTR528
1SP,PDT,PZC,SZC,AFT,AST,NSLAYR)	CONTR529
C	CONTR530
C FIND BEAM STRAINS	CONTR531
C	CONTR532
IF (NBEAMX.NE.0) CALL BSTRAN (NIX,NULAYB,NA1,NBEAMX,EIB,CUB,DUM,DI	CONTR533
1SP)	CONTR534
IF (ISCALE.NE.3HYES) GO TO 840	CONTR535
C	CONTR536
C ROUTINES FOR CONVERGENCE AND NO CONVERGENCE	CONTR537
C	CONTR538
ITER=ITER+1	CONTR539
PF=1.0	CONTR540
C	CONTR541
C FIND SLAB STRESSES	CONTR542
C	CONTR543
CALL PSTRSS (ISTAGE,PF,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SIGXX0,SIGYY0	CONTR544
1,SIGXY0,CUXX,CUYX,CLXY,EIXX,EIYY,EIXY,PZC,PDT,DT,SZC,SXXOT,SYYOT,SC	CONTR545
2XYOT,AS1,AF,AS,SEMOE,SIGMAP,SPROM,SPRON,NSTYPE,SPHI)	CONTR546
C	CONTR547
C FIND BEAM STRESSES	CONTR548
C	CONTR549
IF (NBEAMX.NE.0) CALL BSTRSS (PF,DUM,ZCRD,EIB,CUB,KG,NULAYB,NBEAMX	CONTR550
1,KMAT,SIBXXA,SIBXXC,ESX,RON,ROM,SIGO,FT,RONT,ROMT,ITYPE)	CONTR551
C	CONTR552
IF (ITRIAL.EQ.0.AND.ITER.EQ.1) WRITE (IO,40)	CONTR553
WRITE (IO,390)	CONTR554
390 FORMAT (/,'1HD,4X,*SCALING PROCEDURE*')	CONTR555

	WRITE (IO,400) ITRIAL	CONTR556
	400 FORMAT (/ ,1H ,5X,*NUMBER OF TRIALS = *,I5)	CONTR557
	WRITE (IO,410) ITER	CONTR558
C	410 FORMAT (/ ,1H ,5X,*NUMBER OF ITERATIONS = *,I5)	CONTR559
	IF (CODEA.NE.3HYES) GO TO 490	CONTR560
C		CONTR561
	WRITE (IO,420)	CONTR562
	420 FORMAT (/5X,*DISPLACEMENT INCREMENT*,/)	CONTR563
	WRITE (IO,430)	CONTR564
	430 FORMAT (1H0,5X,*NODAL POINTS*,8X,*U-DISP*,11X,*V-DISP*,12X,*W-DISP*	CONTR565
	1*,12X,*MX-DISP*,11X,*MY-DISP*,//)	CONTR566
	WRITE (IO,440) (M,DISP(5*M-4),DISP(5*M-3),DISP(5*M-2),DISP(5*M-1),	CONTR567
	1DISP(5*M),M=1,NUMNP)	CONTR568
	440 FORMAT (6X,I5,2X,5F18.5)	CONTR569
C		CONTR570
	WRITE (IO,450)	CONTR571
	450 FORMAT (///9X,*NODAL POINT FORCE INCREMENT INCLUDING FICTIOUS FOR	CONTR572
	1 CES*,/1X)	CONTR573
	WRITE (IO,4510)	CONTR574
	WRITE (IO,440) (M,FORCE(5*M-4),FORCE(5*M-3),FORCE(5*M-2),FORCE(5*M	CONTR575
	1-1),FORCE(5*M),M=1,NUMNP)	CONTR576
	WRITE (IO,20)	CONTR577
C		CONTR578
C		CONTR579
	WRITE (IO,460)	CONTR580
	460 FORMAT (/5X,*STRESS INCREMENT*,/)	CONTR581
	WRITE (IO,470)	CONTR582
	470 FORMAT (/ ,1H0,4X,*EL LAYER*,6X,*SXX*,11X,*SYY*,11X,*SXY*,10X,2HS1,	CONTR583
	112X,2HS2,10X,6HTHETA1)	CONTR584
C		CONTR585
	CALL PRISTR (SIGXX0,SIGYY0,SIGXY0,1.0,19,NULAY,NUMEL,NCNS,NSLAYR,S	CONTR586
	1PHI,IO)	CONTR587
C		CONTR588
C		CONTR589
	WRITE (IO,460)	CONTR590
	WRITE (IO,480)	CONTR591
	480 FORMAT (1H )	CONTR592
C		CONTR593
	IF (NBEAMX.NE.0) CALL PRISRB (SIBXXC,23,NULAYB,NEEAMX,IO)	CONTR594
C		CONTR595
C		CONTR596
	490 CONTINUE	CONTR597
	IF (IFN.EG.3HYES) GO TO 840	CONTR598
C		CONTR599
C	CHECK FOR CONVERGENCE OF FIRST TRIAL LOAD	CONTR600
C		CONTR601
	DO 500 II=1,NA1	CONTR602
	I=II	CONTR603
	A=DISP(I)	CONTR604
	B=DISP(I)	CONTR605
	IF (ABS(B).LE.1.0E-09) GO TO 500	CONTR606
	IF (ABS((B-A)/B).GT.TOLDSP) GO TO 510	CONTR607
	500 CONTINUE	CONTR608
	ICON=3HYES	CONTR609
	GO TO 530	CONTR610
	510 CONTINUE	CONTR611
C		CONTR612
C	ROUTINE FOR NO CONVERGENCE	CONTR613

C	ICON=3HNO	CONTR614
	DO 520 I=1,NA1	CONTR615
		CONTR616
	520 DISPO(I)=DISP(I)	CONTR617
	REWIND 18	CONTR618
	WRITE (18) DISPO,FORCEB,FCA00	CONTR619
C		CONTR620
	530 CONTINUE	CONTR621
C		CONTR622
C	TEST FOR MAXIMUM NUMBER OF ITERATIONS	CONTR623
C		CONTR624
	IF (ITER.NE.1) GO TO 540	CONTR625
	IF (ITRIAL.NE.0) GO TO 540	CONTR626
C		CONTR627
C	FIND IF PCTT OR PCTC LIMITS ARE EXCEEDED	CONTR628
C		CONTR629
C	FOR SLAB	CONTR630
C		CONTR631
	CALL PSCALE (RATIOB,IFN,ISCALE,SXXOT,SYOT,SXYOT,PCTT,PCTC,IO,NCNS	CONTR632
	1,NUMEL,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO,SIGXYO,NSTYPE,SPHI,SIGMAP)	CONTR633
	IF (IFN.EQ.3HYES) GO TO 840	CONTR634
C		CONTR635
C	FOR BEAM	CONTR636
C		CONTR637
	IF (NBEAMX.NE.0) CALL BSCALE (RATIOB,IFN,ISCALE,KC,DUM,NBEAMX,NULAY	CONTR638
	1YB,KMAT,PCTT,PCTC,SIBXXA,SIBXXC,ITYPE,SIGO,FT)	CONTR639
	IF (IFN.EQ.3HYES) GO TO 840	CONTR640
	540 CONTINUE	CONTR641
C		CONTR642
C	CHECK ITERATIONS, CONVERGENCE, AND TRIALS	CONTR643
C		CONTR644
	IF (ITER.GT.NITER) GO TO 640	CONTR645
C		CONTR646
C	TEST FOR CONVERGENCE	CONTR647
C		CONTR648
	IF (ICON.EQ.3HNO ) GO TO 370	CONTR649
C		CONTR650
C	DID IT CONVERGE BEFORE OR AFTER SCALING	CONTR651
C		CONTR652
	IF (ITRIAL.NE.0) GO TO 710	CONTR653
C		CONTR654
C	CONVERGENCE BEFORE SCALING	CONTR655
C		CONTR656
	550 CONTINUE	CONTR657
	ITER=0	CONTR658
C		CONTR659
C	FIND SLAB SCALE FACTOR	CONTR660
C		CONTR661
	CALL PSCALE (RATIOB,IFN,ISCALE,SXXOT,SYOT,SXYOT,PCTT,PCTC,IO,NCNS	CONTR662
	1,NUMEL,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO,SIGXYO,NSTYPE,SPHI,SIGMAP)	CONTR663
	IF (IFN.EQ.3HYES) GO TO 840	CONTR664
C		CONTR665
C	FIND BEAM SCALE FACTOR	CONTR666
C		CONTR667
	IF (NBEAMX.EQ.0) GO TO 560	CONTR668
C		CONTR669
	CALL BSCALE (RATIOB,IFN,ISCALE,KC,DUM,NBEAMX,NULAYB,KMAT,PCTT,PCTC	CONTR670
	1,SIBXXA,SIBXXC,ITYPE,SIGO,FT)	CONTR671



	IF (IFN.EQ.3HYES) GO TO 840	CONTR672
C		CONTR673
	IF (RATIO3.GT.RATIOP) RATIOP=RATIOB	CONTR674
C		CONTR675
C	RATIOY IS YIELD RATIO	CONTR676
C	RATIOP IS CURRENT LOAD RATIO	CONTR677
C		CONTR678
	560 CONTINUE	CONTR679
	IF (RATIOP.GT.1.0.OR.RATIOP.EQ.0.0) GO TO 1600	CONTR680
	RATIOP=1.0/RATIOP	CONTR681
	RATIOY=RATIOP	CONTR682
	WRITE (IO,570) RATIOY	CONTR683
	570 FORMAT (/ ,1H ,5X,*INITIAL SCALING RATIO = *,E11.4)	CONTR684
C		CONTR685
C	SCALE TO YIELD	CONTR686
C		CONTR687
	DO 580 I=1,NA1	CONTR688
	DISPO(I)=DISP(I)*RATIOP	CONTR689
	FORCE(I)=FORCE(I)*RATIOP	CONTR690
580	FORCEB(I)=FORCE(I)	CONTR691
	REWIND 29	CONTR692
	WRITE (29) FORCE,LILA	CONTR693
	REWIND 18	CONTR694
	WRITE (18) DISPO,FORCEB,FGADD	CONTR695
C		CONTR696
	REWIND 5	CONTR697
	REWIND 19	CONTR698
	DO 600 N=1,NUMEL	CONTR699
	READ (19) SIGXXO,SIGYYO,SIGXYO	CONTR700
	DO 590 ML=1,NCNS	CONTR701
	SIGXXO(ML)=SIGXXO(ML)*RATIOP	CONTR702
	SIGYYO(ML)=SIGYYO(ML)*RATIOP	CONTR703
	SIGXYO(ML)=SIGXYO(ML)*RATIOP	CONTR704
590	CONTINUE	CONTR705
	WRITE (5) SIGXXO,SIGYYO,SIGXYO	CONTR706
600	CONTINUE	CONTR707
	REWIND 5	CONTR708
	REWIND 19	CONTR709
	CALL FTNCOPY (4LJUM5,3LJ19,0)	CONTR710
C		CONTR711
	IF (NBEAMX.EQ.0) GO TO 630	CONTR712
	REWIND 5	CONTR713
	REWIND 23	CONTR714
	DO 620 N=1,NBEAMX	CONTR715
	READ (23) SIBXXC	CONTR716
	DO 610 ML=1,NULAYE	CONTR717
	SIBXXC(ML)=SIBXXC(ML)*RATIOP	CONTR718
610	CONTINUE	CONTR719
	WRITE (5) SIBXXC	CONTR720
620	CONTINUE	CONTR721
	REWIND 5	CONTR722
	REWIND 23	CONTR723
	CALL FTNCOPY (4LJUM5,3LJ23,0)	CONTR724
C		CONTR725
	630 CONTINUE	CONTR726
C		CONTR727
C	DO FOR FIRST TRIAL	CONTR728
C		CONTR729

	AA=1.0	CONTR730
	ITRIAL=ITRIAL+1	CONTR731
	GO TO 370	CONTR732
	640 CONTINUE	CONTR733
C		CONTR734
C	NO CONVERGENCE	CONTR735
C	CHECK IF FIRST TRIAL	CONTR736
C		CONTR737
	IF (ITRIAL.EQ.0) GO TO 550	CONTR738
C		CONTR739
C	SCALE DOWN AND TRY AGAIN TO GET CONVERGENCE	CONTR740
C		CONTR741
	ITRIAL=ITRIAL+1	CONTR742
	IF (ITRIAL.GT.NTRIAL) GO TO 710	CONTR743
	AA=AA*RRATIO	CONTR744
	DO 650 I=1,NA1	CONTR745
	FORCE(I)=FORCEB(I)*AA	CONTR746
	DISPO(I)=DISP(I)*RRATIO	CONTR747
	650 CONTINUE	CONTR748
	REWIND 18	CONTR749
	WRITE (18) DISPO,FORCEB,FCADD	CONTR750
	REWIND 29	CONTR751
	WRITE (29) FORCE,LILA	CONTR752
C		CONTR753
	REWIND 5	CONTR754
	REWIND 19	CONTR755
	DO 670 N=1,NUMEL	CONTR756
	READ (19) SIGXXO,SIGYYO,SIGXYO	CONTR757
	DO 660 ML=1,NCNS	CONTR758
	SIGXXO(ML)=SIGXXO(ML)*RRATIO	CONTR759
	SIGYYO(ML)=SIGYYO(ML)*RRATIO	CONTR760
	SIGXYO(ML)=SIGXYO(ML)*RRATIO	CONTR761
	660 CONTINUE	CONTR762
	WRITE (5) SIGXXO,SIGYYO,SIGXYO	CONTR763
	670 CONTINUE	CONTR764
	REWIND 5	CONTR765
	REWIND 19	CONTR766
	CALL FTNCOPY (4LJUM5,3LJ19,0)	CONTR767
C		CONTR768
	IF (NBEAMX.EQ.0) GO TO 700	CONTR769
	REWIND 5	CONTR770
	REWIND 23	CONTR771
	DO 690 N=1,NBEAMX	CONTR772
	READ (23) SIBXXC	CONTR773
	DO 680 ML=1,NULAYB	CONTR774
	SIBXXC(ML)=SIBXXC(ML)*RRATIO	CONTR775
	680 CONTINUE	CONTR776
	WRITE (5) SIBXXC	CONTR777
	690 CONTINUE	CONTR778
	REWIND 5	CONTR779
	REWIND 23	CONTR780
	CALL FTNCOPY (4LJUM5,3LJ23,0)	CONTR781
C		CONTR782
	700 CONTINUE	CONTR783
	RATIOP=AA*RATIOY	CONTR784
	ITER=0	CONTR785
	GO TO 370	CONTR786
	710 CONTINUE	CONTR787

C		CONTR788
C	CONVERGENCE AFTER SCALING	CONTR789
C		CONTR790
C	GET FORCE VECTOR	CONTR791
C		CONTR792
	DO 720 I=1,NA1	CONTR793
	720 FORCE(I)=FORCEB(I)*AA	CONTR794
C		CONTR795
C	*****	CONTR796
C		CONTR797
C	OBTAIN FORCE VECTOR FOR USE IN INCREMENTAL SOLUTION SCHEME	CONTR798
C		CONTR799
C	*****	CONTR800
C		CONTR801
	730 CONTINUE	CONTR802
C		CONTR803
	ICYCLE=0	CONTR804
	PFC=1.0	CONTR805
C		CONTR806
C	GET SCALE FACTOR	CONTR807
C		CONTR808
	IFN=3HYES	CONTR809
C		CONTR810
	CALL PSCALE (RATIOB,IFN,ISCALE,SXXOT,SYTOT,SXYOT,PCTT,PCTC,IO,NCNS	CONTR811
	1,NUMEL,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO,SIGXYO,NSTYPE,SPHI,SIGMAP)	CONTR812
	IF (NBEAMX.EQ.0) GO TO 740	CONTR813
C		CONTR814
	CALL BSCALE (RATIOB,IFN,ISCALE,KC,DUM,NBEAMX,NULAYB,KMAT,PCTT,PCTC	CONTR815
	1,SIBXXA,SIBXXC,ITYPE,SIGO,FT)	CONTR816
	IF (RATIOB.GT.RATIOB) RATIOB=RATIOB	CONTR817
C		CONTR818
	740 CONTINUE	CONTR819
	WRITE (IO,750)	CONTR820
	750 FORMAT (/,1H0,4X,*FIND THE FORCE INCREMENT*)	CONTR821
	RATIOB=1.0/RATIOB	CONTR822
C		CONTR823
C	GET REFERENCE FORCE AND DISPLACEMENT	CONTR824
C		CONTR825
	LL=NSIGNI(2)	CONTR826
	HDISP=DISP(LL)*RATIOB	CONTR827
	LL=NSIGNI(1)	CONTR828
	HFORCE=FORCE(LL)*RATIOB	CONTR829
	IF (ABS(HFORCE).LE.1.0E-06) HFORCE=1.	CONTR830
	IF (ABS(HDISP).LE.1.0E-06) HDISP=1.	CONTR831
	J=NSIGNI(2)	CONTR832
	II=(J-1)/5+1	CONTR833
	ML=J-((J-1)/5)*5	CONTR834
	J=NSIGNI(1)	CONTR835
	N=(J-1)/5+1	CONTR836
	M=J-((J-1)/5)*5	CONTR837
	IF (ML.EQ.1) ML=3H U	CONTR838
	IF (ML.EQ.2) ML=3H V	CONTR839
	IF (ML.EQ.3) ML=3H W	CONTR840
	IF (ML.EQ.4) ML=3H MX	CONTR841
	IF (ML.EQ.5) ML=3H MY	CONTR842
	IF (M.EQ.1) M=3H U	CONTR843
	IF (M.EQ.2) M=3H V	CONTR844
	IF (M.EQ.3) M=3H W	CONTR845

IF (M.EQ.4) M=3H MX	CONTR846
IF (M.EQ.5) M=3H MY	CONTR847
WRITE (IO,760) HDISP,II,ML,HFORCE,N,M	CONTR848
760 FORMAT (/ ,1H ,5X,*REFERENCE DISPLACEMENT = *,E12.5,* NODE= *,I3,A3	CONTR849
1,*DISP*,/,1H ,5X,*REFERENCE FORCE = *,E12.5,* NODE= *,I3,A3	CONTR850
2,*FORCE*)	CONTR851
C	CONTR852
C	CONTR853
C SET FORCE INCREMENT	CONTR854
C	CONTR855
DO 770 I=1,NA1	CONTR856
770 FORCEB(I)=FORCE(I)*RATIOF	CONTR857
REWIND 18	CONTR858
WRITE (18) DISPO,FORCEB,FOADD	CONTR859
C	CONTR860
IF (AA.NE.3HYES) GO TO 1310	CONTR861
C	CONTR862
C	CONTR863
C SET INCREMENTS EQUAL TO ZERO	CONTR864
C	CONTR865
DO 780 I=1,NA1	CONTR866
DISP(I)=0.0	CONTR867
780 FORCE(I)=0.0	CONTR868
C	CONTR869
REWIND 19	CONTR870
DO 790 ML=1,NCNS	CONTR871
790 SIGXXO(ML)=0.0	CONTR872
DO 800 N=1,NUMEL	CONTR873
EIXX(N)=0.0	CONTR874
EIYY(N)=0.0	CONTR875
EIXY(N)=0.0	CONTR876
CUXX(N)=0.0	CONTR877
CUYX(N)=0.0	CONTR878
CUXY(N)=0.0	CONTR879
WRITE (19) SIGXXO,SIGXXO,SIGXXO	CONTR880
800 CONTINUE	CONTR881
C	CONTR882
IF (NBEAMX.EQ.0) GO TO 830	CONTR883
REWIND 23	CONTR884
DO 810 ML=1,NULAYB	CONTR885
810 SIBXXO(ML)=0.0	CONTR886
DO 820 N=1,NBEAMX	CONTR887
EIB(N)=0.0	CONTR888
CUB(N)=0.0	CONTR889
WRITE (23) SIBXXO	CONTR890
820 CONTINUE	CONTR891
830 CONTINUE	CONTR892
C	CONTR893
GO TO 1310	CONTR894
C	CONTR895
C DO IF NO SCALING CR HAVE TERMINATED SCALING PREMATURELY	CONTR896
C	CONTR897
840 AA=3HYES	CONTR898
IF (ISCALE.EQ.3HYES) GO TO 730	CONTR899
AA=1.0	CONTR900
PFC=1.0	CONTR901
ICYCLE=0	CONTR902
DO 850 I=1,NA1	CONTR903

FORCEB(I)=FORCE(I)	CONTR904
FORCE(I)=0.0	CONTR905
850 CONTINUE	CONTR906
REWIND 18	CONTR907
WRITE (18) DISPO,FORCEB,FCADD	CONTR908
GO TO 1310	CONTR909
C	CONTR910
C	CONTR911
C*****	CONTR912
C INCREMENTAL SOLUTION SCHEME UP TO FAILURE	CONTR913
C	CONTR914
C*****	CONTR915
C	CONTR916
860 CONTINUE	CONTR917
C	CONTR918
C START ITERATION LOOP	CONTR919
C	CONTR920
C START OF NEW CYCLE	CONTR921
C INITIALIZE	CONTR922
C	CONTR923
QAPAC=QAPA	CONTR924
PFC=1.0	CONTR925
ITER=0	CONTR926
ITRIAL=1	CONTR927
IF (LITER.NE.1.OR.LTRIAL.NE.1) IONE=3HNO	CONTR928
IF (ICHECK.EQ.NCHECK) QAPAC=QAPAC*RQAPA	CONTR929
IF (ICHECK.EQ.NCHECK) IONE=3HYES	CONTR930
C	CONTR931
C SET FORCE INCREMENT	CONTR932
C	CONTR933
DO 870 I=1,NA1	CONTR934
FORCE(I)=FORCEB(I)*QAPAC+FCADD(I)	CONTR935
870 CONTINUE	CONTR936
C	CONTR937
C INCREASE CYCLE NUMBER	CONTR938
C	CONTR939
ICYCLE=ICYCLE+1	CONTR940
IF (ICYCLE.EQ.1) RDFPR=3HYES	CONTR941
C	CONTR942
WRITE (IO,880) ICYCLE	CONTR943
880 FORMAT (//,1H ,5X,*LOAD CYCLE = *,I8)	CONTR944
WRITE (IO,890) QAPAC,PFC	CONTR945
890 FORMAT (/,1H ,5X,*----LOAD RATIO AT START OF CYCLE = *,E16.9,/ 1,6X,*----PRESCAN FACTOR AT START OF CYCLE = *,E16.9)	CONTR946
C	CONTR947
C START OF ITERATION WITHIN CYCLE	CONTR948
C	CONTR949
900 CONTINUE	CONTR950
C	CONTR951
C	CONTR952
REWIND 19	CONTR953
DO 910 ML=1,NCNS	CONTR954
SXXOT(ML)=0.0	CONTR955
910 CONTINUE	CONTR956
DO 920 N=1,NUMEL	CONTR957
WRITE (19) SXXOT,SXXOT,SXXOT	CONTR958
920 CONTINUE	CONTR959
C	CONTR960
IF (NBEAMX.EQ.0) GO TO 950	CONTR961

REWIND 23	CONTR962
DO 930 ML=1,NULAYE	CONTR963
SIBXXC(ML)=0.0	CONTR964
930 CONTINUE	CONTR965
DO 940 N=1,NBEAMX	CONTR966
WRITE (23) SIBXXC	CONTR967
940 CONTINUE	CONTR968
950 CONTINUE	CONTR969
DO 960 I=1,NA1	CONTR970
DISPO(I)=0.0	CONTR971
960 CONTINUE	CONTR972
C	CONTR973
C	CONTR974
C TERMINATION CHECK	CONTR975
C	CONTR976
IF (IOUT.NE.3HYES.AND.ICYCLE.LT.LCYCLE) GO TO 1090	CONTR977
IF (ECARDS.EQ.3HYES) GO TO 1100	CONTR978
C	CONTR979
C END OF OVERLOAD SOLUTION PROCEDURE	CONTR980
C	CONTR981
970 CONTINUE	CONTR982
C	CONTR983
C PRINT OUT HISTOGRAM	CONTR984
C	CONTR985
WRITE (IO,980) FVAL(1),FVAL(2),FVAL(3)	CONTR986
980 FORMAT (1H0,/,11X,*SLAB -FIRST CRACK LOAD = *,F11.4,* KIPS*,/,1CONTR987	
11X,*SLAB -FIRST CRUSH LOAD = *,F11.4,* KIPS*,/,11X,*SLAB -FIRSTCONTR988	
2 YIELD LOAD = *,F11.4,* KIPS*,/))	CONTR989
C	CONTR990
IF (NBEAMX.NE.0) WRITE (IO,990) FVAL(4),FVAL(5),FVAL(6)	CONTR991
990 FORMAT (1H0,/,11X,*BEAM -FIRST CRACK LOAD = *,F11.4,* KIPS*,/,1CONTR992	
11X,*BEAM -FIRST CRUSH LOAD = *,F11.4,* KIPS*,/,11X,*BEAM -FIRSTCONTR993	
2 YIELD LOAD = *,F11.4,* KIPS*,/))	CONTR994
C	CONTR995
I=(NSIGNI(2)-3)/5+1	CONTR996
WRITE (IO,1000) NVAL(1),I,PMAX,NVAL(4),FVAL(7)	CONTR997
1000 FORMAT (1H0,/,11X*MAXIMUM DISPLACEMENT HAS BEEN EXCEEDED (NODE) CONT998	
1=* ,5X,A3,* (* ,I3,* )*,/,11X,*SPECIFIED OVERLOAD RATIO OF *,E10.3,*CONTR999	
2 EXCEEDED=* ,5X,A3,/,11X,*RATIO OF TOTAL LOAD TO THE SPECIFIED OVECONT1000	
3RLOAD =*,F8.3,/))	CONT1001
IF (NBEAMX.NE.0) WRITE (IO,1010) NVAL(2),IVAL(2),IVAL(3)	CONT1002
1010 FORMAT (1H0,/,11X,*MAX SHEAR HAS BEEN EXCEEDED(BEAM ELEMENT,LAYERCONT1003	
1)* ,5X,A3,* (* ,I3,* ,* ,I2,* )*,/))	CONT1004
C	CONT1005
M=ICRS8	CONT1006
I=ICRS10	CONT1007
WRITE (IO,1020) NVAL(3),CRS7,M,CRS9,I	CONT1008
1020 FORMAT (1H0,/,11X,*MAXIMUM SLAB CRACK WIDTH HAS BEEN EXCEEDED =*,5CONT1009	
1X,A3,/,16X,*CRACK WIDTH*,9X,* = *,F8.5,* (IN)*,/,16X,*ELEMENT NUMBECONT1010	
2R = *,I8,/,16X,*CRACK ANGLE*,9X,* = *,F8.3,* (DEGREES)*,/,16X,CONT1011	
3*CRACK LOCATION = *,4X,A4,/))	CONT1012
I=ICRB5	CONT1013
M=ICRB2	CONT1014
IF (NBEAMX.NE.0.AND.M.GT.0) WRITE (IO,1030) NVAL(5),CRB10,I	CONT1015
1030 FORMAT (1H0,/,11X,*MAXIMUM BEAM CRACK WIDTH HAS BEEN EXCEEDED =*,5CONT1016	
1X,A3,/,16X,*CRACK WIDTH*,9X,* = *,F8.5,* (IN)*,/,16X,*ELEMENT NUMBECONT1017	
2R = *,I8,/))	CONT1018
II=1	CONT1019

	IF (NBEAMX.EQ.0) II=2	CONT1020
	DO 1080 N=II,2	CONT1021
C		CONT1022
C	PRINT OUT THE BEAM CHECKS IF N=1	CONT1023
C	PRINT OUT THE SLAB CHECKS IF N=2	CONT1024
C		CONT1025
C		CONT1026
	IF (N.EQ.1) WRITE (IO,1040) (J,J=1,KMAT)	CONT1027
	1040 FORMAT (1H0,/,11X,*BEAM TERMINATION CHECKS / MATERIAL TYPE =*,5	CONT1028
	1X,6(I1,10X))	CONT1029
C		CONT1030
	IF ( N.EQ.2)	CONT1031
	*WRITE(IO,153) (J,J=1,NSMAT)	CONT1032
	153 FORMAT(1H0,/,11X,*SLAB TERMINATION CHECKS / MATERIAL TYPE =*,	CONT1033
	** CONCRETE/STEEL-*,6(I1,10X))	CONT1034
C		CONT1035
	WRITE (IO,1050)	CONT1036
	1050 FORMAT (1H ,10X,*FOR EXCEEDING LIMITS ON*)	CONT1037
C		CONT1038
	ML=NSMAT+1	CONT1039
	DO 1080 I=1,5	CONT1040
C		CONT1041
C	CHOOSE TITLE	CONT1042
C		CONT1043
	NSTYPE(1)=10H	CONT1044
	NSTYPE(2)=10H	CONT1045
	NSTYPE(3)=10H	CONT1046
	IF (I.EQ.1) NSTYPE(1)=10HSTRAIN	CONT1047
	IF (I.EQ.2) NSTYPE(1)=10HTENSILE ST	CONT1048
	IF (I.EQ.2) NSTYPE(2)=10HRESS	CONT1049
	IF (I.EQ.3) NSTYPE(1)=10HCOMPRESSIV	CONT1050
	IF (I.EQ.3) NSTYPE(2)=10HE STRESS	CONT1051
	IF (I.EQ.4) NSTYPE(1)=10HCRACKED OR	CONT1052
	IF (I.EQ.4) NSTYPE(2)=10H YIELDED L	CONT1053
	IF (I.EQ.4) NSTYPE(3)=10HAYERS	CONT1054
	IF (I.EQ.5) NSTYPE(1)=10HCRUSHED LA	CONT1055
	IF (I.EQ.5) NSTYPE(2)=10HYERS	CONT1056
C		CONT1057
C	WRITE VALUES	CONT1058
C		CONT1059
C		CONT1060
	IF (N.EQ.1) WRITE (IO,1060) (NSTYPE(J),J=1,3),(NVALB(J,I),J=1,KMAT	CONT1061
	1)	CONT1062
	1060 FORMAT (1H ,/,16X,3A10,7X,*=*,4X,6(A3,8X))	CONT1063
	IF (N.EQ.1) WRITE (IO,1070) ((IVALB(J,K,I),K=1,2),J=1,KMAT)	CONT1064
	1070 FORMAT (1H ,19X,* ELEMENT LAYER *,19X,6(1X,I3,1X,I2,4X))	CONT1065
C		CONT1066
	IF (N.EQ.2) WRITE (IO,1060) (NSTYPE(J),J=1,3),(NVALS(J,I),J=1,ML)	CONT1067
	IF (N.EQ.2) WRITE (IO,1070) ((IVALS(J,K,I),K=1,2),J=1,ML)	CONT1068
C		CONT1069
	1080 CONTINUE	CONT1070
C		CONT1071
	WRITE (IO,1104)	CONT1072
	1104 FORMAT(1H0,5X,	CONT1073
	1#THIS ANALYTIC MODEL CONSIDERS THE FLEXURAL AND INPLANE#,	CONT1074
	2/,8X,	CONT1075
	3#BEHAVIOR OF THE DECK SLAB AND THE FLEXURAL AND AXIAL #,	CONT1076
	4/,8X,	CONT1077

	5#DEFORMATIONS OF THE BEAMS. TRANSVERSE SHEAR DEFORMATION #,	CONT1078
	6/,8X,	CONT1079
	7#NORMAL TO THE PLANE OF THE DECK SLAB IS NOT CONSIDERED.#)	CONT1080
C		CONT1081
C		CONT1082
	WRITE(IO,1081)	CONT1083
	1081 FORMAT(////,1H0,5X,	CONT1084
	1#THE PREDICTED OVERLOADING OF 6 BRIDGE SUPERSTRUCTURES TO#,	CONT1085
	2/,8X,	CONT1086
	3#FAILURE HAVE INDICATED THAT (1) AT THE EARLY PHASES OF#,	CONT1087
	4/,8X,	CONT1088
	5#THE OVERLOADING, I.E. CRACKING OF THE SLAB AND/OR BEAMS,#)	CONT1089
C		CONT1090
	WRITE(IO,1082)	CONT1091
	1082 FORMAT(8X,	CONT1092
	1#THE RESULTS MAY BE OFF BY 20 PERCENT, (2) AT FIRST YIELDING#,	CONT1093
	2/,8X,	CONT1094
	3#OR A DECREASE IN STIFFNESS THE RESULTS MAY BE OFF BY 20#,	CONT1095
	4/,8X,	CONT1096
	5#PERCENT, AND (3) AT THE LATER STAGES OF OVERLOADING#)	CONT1097
C		CONT1098
	WRITE(IO,1102)	CONT1099
	1102 FORMAT(8X,	CONT1100
	1#,I.E. CRUSHING OF THE CONCRETE AND/OR FRACTURING OF THE STEEL#,	CONT1101
	2/,8X,	CONT1102
	3#THE RESULTS MAY BE OFF BY 10 PERCENT. THESE CONCLUSIONS#,	CONT1103
	4/,8X,	CONT1104
	5#DEPEND ON THE PARTICULAR DISCRETIZATION, METHOD OF STRESS #)	CONT1105
C		CONT1106
	WRITE(IO,1103)	CONT1107
	1103 FORMAT(8X,	CONT1108
	1#COMPUTATION, AND ITERATION SCHEME CHOSEN. THUS THE CONCLUSIONS#,	CONT1109
	2/,8X,	CONT1110
	3#ARE ONLY APPLICABLE TO THE AFOREMENTIONED EXAMPLES. ALSO IT #,	CONT1111
	4/,8X,	CONT1112
	5#SHOULD BE NOTED THAT THE ABOVE PERCENTAGES ARE THE MAXIMUM#,	CONT1113
	6/,8X,	CONT1114
	7#OBSERVED DEVIATIONS.#)	CONT1115
C		CONT1116
C		CONT1117
	WRITE(IO,1083)	CONT1118
	1083 FORMAT(1H0,5X,	CONT1119
	1#THE ACCURACY OF THE RESULTS DEPENDS UPON THE ACCURACY OF THE#,	CONT1120
	2/,8X,	CONT1121
	3#INPUT OF THE MATERIAL PROPERTIES, DEFINITION OF THE BRIDGE#,	CONT1122
	4/,8X,	CONT1123
	5#DESIGN PARAMETERS, AND THE CORRECT SIMULATION OF THE OVERLOAD#)	CONT1124
C		CONT1125
	WRITE(IO,1084)	CONT1126
	1084 FORMAT(8X,	CONT1127
	1#CONFIGURATION. SUBSTANTIAL DEVIATIONS FROM ANY ONE OF THESE#,	CONT1128
	2/,8X,	CONT1129
	3#VALUES WILL RESULT IN A SOLUTION THAT MAY NOT BE#,	CONT1130
	4/,8X,	CONT1131
	5#REPRESENTATIVE OF THE ACTUAL BRIDGE BEHAVIOR.#)	CONT1132
C		CONT1133
C		CONT1134
	WRITE(IO,1085)	CONT1135



1085	FORMAT(1H0,5X, 1#THE AGE OF THE CONCRETE AND THE DETERIORATION OF THE BRIDGE#, 2/,8X, 3#SUPERSTRUCTURE MUST BE CONSIDERED BY THE USER. THE USER MUST#, 4/,8X, 5#EXERT JUDGEMENT IN THE CORRECT ASSESMENT OF THESE PARAMETERS.#)	CONT1136 CONT1137 CONT1138 CONT1139 CONT1140 CONT1141
C	WRITE(IO,1086) 1086	CONT1142 CONT1143 CONT1144
	FORMAT(8X, 1#ANY SUBSTANTIAL DEVIATION IN THE INFUT FROM THE ACTUAL STATE#, 4/,8X, 3#OF THE SUPERSTRUCTURE MAY RESULT IN AN INCORRECT SIMULATION#, 4/,8X, 5#OF THE OVERLOAD RESPONSE OF THE BRIDGE.#)	CONT1145 CONT1146 CONT1147 CONT1148 CONT1149 CONT1150
C	WRITE(IO,1087) 1087	CONT1151 CONT1152 CONT1153
	FORMAT(1H0,5X, 1#CRACK WIDTHS ARE COMPUTED FROM FORMULAE THAT ARE BASED ON#, 2/,8X, 3#EMPIRICALLY DERIVED RELATIONSHIPS AND ARE THEREFORE CONSIDERED#, 4/,8X, 5#TO BE APPRCXIMATE.#)	CONT1154 CONT1155 CONT1156 CONT1157 CONT1158 CONT1159
C	WRITE(IO,1088) 1088	CONT1160 CONT1161 CONT1162
	FORMAT(1H0,5X, 1#THE PROGRAM DOES NOT INCLUDE FATIGUE PROVISIONS FOR THE#, 2/,8X, 3#SUPERSTRUCTURE OR ITS CONSTITUTIVE MATERIALS. IF THE FATIGUE#, 4/,8X, 5#IS BELIEVED TO BE IMPORTANT, THEN IT MAY BE ADVISABLE TO#)	CONT1163 CONT1164 CONT1165 CONT1166 CONT1167 CONT1168
C	WRITE(IO,1089) 1089	CONT1169 CONT1170 CONT1171
	FORMAT(8X, 1#PERFORM A NEW ANALYSIS WITH REDUCED CONCRETE AND/OR STEEL#, 2/,8X, 3#STRENGTHS.#)	CONT1172 CONT1173 CONT1174
C	WRITE(IO,1091) 1091	CONT1175 CONT1176 CONT1177
	FORMAT(1H0,5X, 1#THE ANALYSIS IS CARRIED OUT FOR THE LANE LOCATION OF THE#, 2/,8X, 3#OVERLOAD VEHICLE THAT IS DEFINED BY THE USER OF THE PROGRAM.#, 4/,8X, 5#ANY DEVIATION FROM THIS LANE LOADING IN THE ACTUAL OVERLOADING#)	CONT1178 CONT1179 CONT1180 CONT1181 CONT1182 CONT1183
C	WRITE(IO,1092) 1092	CONT1184 CONT1185 CONT1186
	FORMAT(8X, 1#OF THE BRIDGE MAY RESULT IN A RESPONSE DIFFERENT FROM THAT#, 2/,8X, 3#PREDICTED BY THE PROGRAM.#)	CONT1187 CONT1188 CONT1189
C	WRITE(IO,1093) 1093	CONT1190 CONT1191 CONT1192
	FORMAT(1H0,5X, 1#THE ANALYSIS IS CARRIED OUT FOR THE GIVEN LOADING SPECIFIED BY#,	CONT1193

	2/,8X,	CONT1194
	3#THE USER. IF DURING THE ACTUAL LOADING OF THE BRIDGE OTHER#,	CONT1195
	4/,8X,	CONT1196
C	5#VEHICLES OF QUESTIONABLE WEIGHT ARE PRESENT IN ADDITION TO THE#)	CONT1197
	WRITE(IO,1094)	CONT1198
	1094 FORMAT(8X,	CONT1199
	1#GIVEN LOADING, THEN THE ACTUAL BRIDGE RESPONSE MAY BE DIFFERENT#,	CONT1200
	2/,8X,	CONT1201
	3#AS COMPARED TO THAT PREDICTED BY THE PROGRAM.#)	CONT1202
C		CONT1203
C		CONT1204
	WRITE(IO,1095)	CONT1205
	1095 FCRMAT(1H0,5X,	CONT1206
	1#THE COMPUTER PROGRAM ASSUMES THAT DYNAMIC EFFECTS ARE NOT#,	CONT1207
	2/,8X,	CONT1208
	3#PRESENT. THUS THE VEHICULAR SPEED SHOULD BE REDUCED TO CRAWL#,	CONT1209
	4/,8X,	CONT1210
	5#SPEED, I.E. A MAXIMUM OF 5 MPH, DURING THE TRAVEL ACROSS#)	CONT1211
C		CONT1212
	WRITE(IO,1096)	CONT1213
	1096 FORMAT(8X,	CONT1214
	1#THE BRIDGE. IF THE VEHICLE IS TO TRAVEL THE BRIDGE AT NORMAL#,	CONT1215
	2/,8X,	CONT1216
	3#TRAFFIC SPEED, THEN THE ACTUAL VEHICULAR LOAD SHOULD BE#,	CONT1217
	4/,8X,	CONT1218
	5#INCREASED BY SOME IMPACT FACTOR FOR ANALYSIS PURPOSES.#)	CONT1219
C		CONT1220
C		CONT1221
C		CONT1222
	WRITE(IO,1097)	CONT1223
	1097 FORMAT(1H0,5X,	CONT1224
	1#IF THE OVERLOAD ANALYSIS IS CARRIED OUT FOR THE MAXIMUM#,	CONT1225
	2/,8X,	CONT1226
	3#FLEXURAL RESPONSE, THEN THE USER SHOULD ALSO CONSIDER THE#,	CONT1227
	4/,8X,	CONT1228
	5#NECESSITY OF AN ANALYSIS FOR A LOAD CONFIGURATION/POSITION#)	CONT1229
C		CONT1230
	WRITE(IO,1098)	CONT1231
	1098 FORMAT(8X,	CONT1232
	1#WHICH WILL PRODUCE A MAXIMUM HORIZONTAL SHEAR CONDITION #,	CONT1233
	2/,8X,	CONT1234
	3#IN THE BEAMS NEAR THE SUPPORTS.#)	CONT1235
C		CONT1236
C		CONT1237
	WRITE(IO,1099)	CONT1238
	1099 FCRMAT(1H0,5X,	CONT1239
	1#IT IS ADVISABLE THAT THE USER CONSIDERS THE MAXIMUM DEFLECTION#,	CONT1240
	2/,8X,	CONT1241
	3#OF THE BRIDGE SUPERSTRUCTURE AS COMPARED TO THE DESIGN#,	CONT1242
	4/,8X,	CONT1243
	5#DEFLECTION. THE DESIGN DEFLECTION IS TO BE OBTAINED BY THE USER.#)	CONT1244
	6)	CONT1245
C		CONT1246
C		CONT1247
	WRITE(IO,1101)	CONT1248
	1101 FORMAT(1H0,5X,	CONT1249
	1#NO SPECIAL STRESS CHECKS HAVE BEEN UNDERTAKEN FOR THE BEAMS#,	CONT1250
	2/,8X,	CONT1251

3#WITH DRAPED STRANDS AT THE DRAPE POINTS. THE USER SHOULD#,  
4/,8X,  
5#INVESTIGATE THIS MANUALLY IF IT IS PERTINENT.#)

CONT1252

CONT1253

CONT1254

C  
WRITE (10,40)

CONT1255

CONT1256

CONT1257

STOP 5

CONT1258

CONT1259

C  
1090 CONTINUE

CONT1260

C  
START OF NEW ITERATION OR TRIAL

CONT1261

CONT1262

CONT1263

C  
TIME CHECK

CONT1264

CONT1265

CONT1266

CALL SECOND (TIME)  
IF (TIME.LT.ETIME) GO TO 1140

CONT1267

CONT1268

CONT1269

C  
CHECK FOR RESTART CARD GENERATION

CONT1270

CONT1271

IF (ECARDS.NE.3HYES) GO TO 970

CONT1272

C  
1100 CONTINUE

CONT1273

CONT1274

CONT1275

C  
WRITE END CARDS

CONT1276

CONT1277

C  
REWIND 19

CONT1278

REWIND 21

CONT1279

REWIND 32

CONT1280

IN=7

CONT1281

REWIND IN

CONT1282

WRITE (IN) ICYCLE,ITRIAL,ITER,GAPAC,PFC,HDISP,HFORCE,PFY

CONT1283

CONT1284

C  
WRITE (IN) FVAL(1),FVAL(2),FVAL(3),FVAL(4),FVAL(5),FVAL(6)

CONT1285

DO 1110 I=1,NUMEL

CONT1286

READ (21) SXXOT,SYYOT,SXYOT

CONT1287

READ (19) SIGXXO,SIGYYO,SIGXYO

CONT1288

READ (32) AFT,AST

CONT1289

WRITE (IN) SXXOT,SYYOT,SXYOT

CONT1290

WRITE (IN) SIGXXO,SIGYYO,SIGXYO

CONT1291

WRITE (IN) AFT,AST

CONT1292

1110 CONTINUE

CONT1293

C  
DO 1120 I=1,NUMEL

CONT1294

READ (32) J,M,N

CONT1295

1120 WRITE (IN) J,M,N

CONT1297

C  
WRITE (IN) FORCE,LILA

CONT1298

WRITE (IN) DISPO,FORCEB,FCADD

CONT1300

WRITE (IN) AS1,AF,AS

CONT1301

REWIND 17

CONT1302

READ (17) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY

CONT1303

WRITE (IN) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY

CONT1304

CONT1305

C  
IN=1

CONT1306

ECARDS=3HNO

CONT1307

IOUT=3HYES

CONT1308

C  
CONT1309

	IF (NBEAMX.EQ.0) GO TO 970	CONT1310
	REWIND 15	CONT1311
	REWIND 23	CONT1312
	REWIND 25	CONT1313
	REWIND 33	CONT1314
	IN=7	CONT1315
	DO 1130 I=1,NBEAMX	CONT1316
	READ (15) SIBXXA	CONT1317
	READ (23) SIBXXC	CONT1318
	READ (25) EPSPS	CONT1319
	WRITE (IN) SIBXXA	CONT1320
	WRITE (IN) SIBXXC	CONT1321
1130	WRITE (IN) EPSPS	CONT1322
	CONTINUE	CONT1323
	READ (33) ESX	CONT1324
	WRITE (IN) ESX	CONT1325
	IN=1	CONT1326
	GO TO 970	CONT1327
C		CONT1328
	1140 CONTINUE	CONT1329
C		CONT1330
C	SOLVE SYSTEM	CONT1331
C		CONT1332
	REWIND 18	CONT1333
	WRITE (18) DISPO,FORCEB,FCADD	CONT1334
	REWIND 29	CONT1335
	WRITE (29) FORCE,LILA	CONT1336
C		CONT1337
	IRS=4	CONT1338
	IOUTS=1	CONT1339
	IF (ITP.EQ.3HYES) ICUTS=3	CONT1340
	RETURN	CONT1341
C		CONT1342
C	SOLVE EQUATIONS	CONT1343
C		CONT1344
	1150 CONTINUE	CONT1345
C		CONT1346
	REWIND 31	CONT1347
	REWIND 33	CONT1348
	REWIND 18	CONT1349
	REWIND 29	CONT1350
	REWIND 30	CONT1351
	READ (31) AS1,AF,AS	CONT1352
	READ (33) ESX	CONT1353
	READ (18) DISPO,FORCEB,FCADD	CONT1354
	READ (29) FORCE,LILA	CONT1355
	READ (30) DISP	CONT1356
C		CONT1357
C	FIND SLAB STRAINS	CONT1358
C		CONT1359
	CALL PSTRAN (NUMEL,NA1,NANA,NULAY,CUXX,CUYX,CUXY,EIXX,EIYY,EIXY,DI	CONT1360
	1SP,PDT,PZC,SZC,AFT,AST,NSLAYR)	CONT1361
C		CONT1362
C	FIND BEAM STRAINS	CONT1363
C		CONT1364
	IF (NBEAMX.NE.0) CALL BSTRAN (NIX,NULAYB,NA1,NBEAMX,EIB,CUB,DUM,DI	CONT1365
	1SP)	CONT1366
C		CONT1367

C	ROUTINES FOR CONVERGENCE AND NO CONVERGENCE	CONT1368
C		CONT1369
	PF=1.0	CONT1370
	ITER=ITER+1	CONT1371
	ISTAGE=1	CONT1372
C		CONT1373
C	FIND SLAB STRESSES	CONT1374
C		CONT1375
	CALL PSTRSS (ISTAGE,PF,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO	CONT1376
	1,SIGXYO,CUXX,CUYX,CUXY,EIXX,EIYY,EIXY,PZC,PDT,DT,SZC,SXXOT,SYOT,SC	CONT1377
	2XYOT,AS1,AF,AS,SEMOD,SIGMAP,SPROM,SPRON,NSTYPE,SPHI)	CONT1378
C		CONT1379
	ISTAGE=ISTAGE+1	CONT1380
	PF=1.0	CONT1381
C		CONT1382
C	FIND PRESCAN FACTOR FOR SLAB	CONT1383
C		CONT1384
	CALL PSTRSS (ISTAGE,PF,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO	CONT1385
	1,SIGXYO,CUXX,CUYX,CUXY,EIXX,EIYY,EIXY,PZC,PDT,DT,SZC,SXXOT,SYOT,SC	CONT1386
	2XYOT,AS1,AF,AS,SEMOD,SIGMAP,SPROM,SPRON,NSTYPE,SPHI)	CONT1387
C		CONT1388
C	FIND BEAM STRESSES	CONT1389
C		CONT1390
	IF (NBEAMX.NE.0) CALL BSTRSS (PF,DUM,ZCRD,ETB,CUB,KC,NULAYB,NBEAMX	CONT1391
	1,KMAT,SIBXXA,SIBXXC,ESX,RON,ROM,SIGO,FT,RONT,ROMT,ITYPE)	CONT1392
	IF (PF.EQ.0.0) PF=DPFC	CONT1393
C		CONT1394
	WRITE (IO,400) ITRIAL	CONT1395
	WRITE (IO,410) ITER	CONT1396
C		CONT1397
C	CHECK PRINT CODES	CONT1398
C		CONT1399
	IF (RDFPR.NE.3HYES) GO TO 1160	CONT1400
	IF (CODEA.NE.3HYES) GO TO 1160	CONT1401
C		CONT1402
	WRITE (IO,420)	CONT1403
	WRITE (IO,430)	CONT1404
	WRITE (IO,440) (M,DISP(5*M-4),DISP(5*M-3),DISP(5*M-2),DISP(5*M-1),	CONT1405
	1DISP(5*M),M=1,NUMNP)	CONT1406
C		CONT1407
	WRITE (IO,450)	CONT1408
	WRITE (IO,1510)	CONT1409
	WRITE (IO,440) (M,FORCE(5*M-4),FORCE(5*M-3),FORCE(5*M-2),FORCE(5*M	CONT1410
	1-1),FORCE(5*M),M=1,NUMNP)	CONT1411
	WRITE (IO,20)	CONT1412
C		CONT1413
C		CONT1414
	WRITE (IO,460)	CONT1415
	WRITE (IO,470)	CONT1416
	CALL PRISTR (SIGXXO,SIGYYO,SIGXYO,1.0,19,NULAY,NUMEL,NCNS,NSLAYR,SC	CONT1417
	1PHI,IO)	CONT1418
C		CONT1419
	WRITE (IO,460)	CONT1420
	WRITE (IO,480)	CONT1421
	IF (NBEAMX.NE.0) CALL PRISRB (SIBXXC,23,NULAYB,NBEAMX,IO)	CONT1422
C		CONT1423
C		CONT1424
	1160 CONTINUE	CONT1425

C		CONT1426
C	INCREMENTAL MODE AND PRESCAN CHECKS	CONT1427
C		CONT1428
	IF (IONE.EQ.3HYES.AND.PF.NE.1.0) GO TO 1300	CONT1429
	IF (IONE.EQ.3HYES) GO TO 1280	CONT1430
C		CONT1431
C	CHECK CONVERGENCE	CONT1432
C		CONT1433
C	CHECK DISPLACEMENT CONVERGENCE	CONT1434
C		CONT1435
	DO 1170 II=1,NA1	CONT143E
	I=II	CONT1437
	A=DISPO(I)	CONT1438
	B=DISP(I)	CONT1439
	IF (ABS(B).LE.1.0E-09) GO TO 1170	CONT1440
	IF (ABS((B-A)/B).GT.TOLDSP) GO TO 1180	CONT1441
	1170 CONTINUE	CONT1442
C		CONT1443
C	ROUTINE FOR CONVERGENCE	CONT1444
C		CONT1445
	ICON=4HYES1	CONT1446
	GO TO 1200	CONT1447
	1180 CONTINUE	CONT1448
C		CONT1449
C	ROUTINE FOR NO CONVERGENCE	CONT1450
C		CONT1451
	ICON=4HNO	CONT1452
	DO 1190 I=1,NA1	CONT1453
	1190 DISPO(I)=DISP(I)	CONT1454
	1200 CONTINUE	CONT1455
C		CONT1456
	IF (ICON.EQ.4HYES1) GO TO 1280	CONT1457
C		CONT1458
C	CHECK ITERATIONS	CONT1459
C		CONT1460
	IF (ITER.EQ.LITER) GO TO 1210	CONT1461
	IF (ITER.GT.(LITER+1)) GO TO 1280	CONT1462
G		CONT1463
	IF (PF.NE.1.0) GO TO 1250	CONT1464
C		CONT1465
C		CONT1466
	GO TO 1090	CONT1467
	1210 CONTINUE	CONT1468
C		CONT1469
C	ITERATIONS EXCEDED LIMIT LITER	CONT1470
C		CONT1471
	ITER=0	CONT1472
	IF (ITRIAL.EQ.LTRIAL) GO TO 1240	CONT1473
	ITRIAL=ITRIAL+1	CONT1474
C		CONT1475
C	REDUCE LOAD INCREMENT	CONT1476
C		CONT1477
	QAPAC=RQAPA*QAPAC	CONT1478
	PFC=1.0	CONT1479
	DO 1220 I=1,NA1	CONT1480
	1220 FORCE(I)=FORCEB(I)*QAPAC+FCADD(I)	CONT1481
C		CONT1482
	WRITE (10,1230) RQAPA,QAPAC	CONT1483

1230	FORMAT (/ ,1H ,5X,*----LOAD STEP WILL BE REDUCED-----*,/,10X,*L	CONT1484
	LOAD REDUCTION FACTOR = *,E16.9,/,10X,*NEW LOAD RATIO	CONT1485
2	= *,E16.9)	CONT1486
C		CONT1487
	GO TO 930	CONT1488
1240	CONTINUE	CONT1489
C		CONT1490
C	TRIALS EXCEEDED LIMIT LTRIAL	CONT1491
C		CONT1492
	IF (PF.EQ.1.0) GO TO 1280	CONT1493
	PFC=PFC*DPFC	CONT1494
	ITER=LITER+1	CONT1495
1250	CONTINUE	CONT1496
C		CONT1497
C	REDUCE LOAD INCREMENT SINCE PRESCAN WAS NOT SATISFIED	CONT1498
C		CONT1499
	PFC=PF*PFC	CONT1500
	A=QAPAC*PFC	CONT1501
	DO 1260 I=1,NA1	CONT1502
	FORCE(I)=FORCEB(I)*QAPAC*PFC+FGADD(I)	CONT1503
1260	CONTINUE	CONT1504
C		CONT1505
	WRITE (IO,1270) PF,A	CONT1506
1270	FORMAT (/ ,1H ,5X,*----LOAD STEP WILL BE REDUCED-----*,/,10X,*P	CONT1507
	1RESCAN REDUCTION FACTOR = *,E16.9,/,10X,*NEW LOAD RATIO	CONT1508
2	= *,E16.9)	CONT1509
C		CONT1510
	GO TO 900	CONT1511
1280	CONTINUE	CONT1512
C		CONT1513
C	CONVERGENCE	CONT1514
C		CONT1515
	DO 1290 I=1,NA1	CONT1516
	FORCE(I)=FORCEB(I)*QAFAC*PFC	CONT1517
1290	CONTINUE	CONT1518
C		CONT1519
C		CONT1520
	GO TO 1310	CONT1521
1300	CONTINUE	CONT1522
	IF (PRECRK.NE.3HYES) GO TO 1280	CONT1523
C		CONT1524
C	PRECRACK LAYERS SINCE PRESCAN FACTOR SHOWS LAYERS WILL FAIL WITH	CONT1525
C	LITTLE OR NO ADDITIONAL LOAD APPLIED	CONT1526
C		CONT1527
	PFC=0.0	CONT1528
1310	CONTINUE	CONT1529
C		CONT1530
C	START PROCEDURE FOR END OF CYCLE	CONT1531
C		CONT1532
	REWIND 17	CONT1533
	READ (17) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONT1534
C		CONT1535
	IF (ICHECK.EQ.NCHECK) ICHECK=-1	CONT1536
	IF (PFC.EQ.0.0) GO TO 1360	CONT1537
C		CONT1538
C	FIND FINAL SLAB AND BEAM STRESSES	CONT1539
C		CONT1540
	REWIND 6	CONT1541

	REWIND 19	CONT1542
	REWIND 21	CONT1543
	DO 1330 I=1,NUMEL	CONT1544
	READ (21) SXXOT,SYYOT,SXYOT	CONT1545
	READ (19) SIGXXO,SIGYYO,SIGXYO	CONT1546
	DO 1320 J=1,NCNS	CONT1547
	SXXOT(J)=SXXOT(J)+SIGXXO(J)	CONT1548
	SYYOT(J)=SYYOT(J)+SIGYYO(J)	CONT1549
	SXYOT(J)=SXYOT(J)+SIGXYO(J)	CONT1550
1320	CONTINUE	CONT1551
	WRITE (6) SXXOT,SYYOT,SXYOT	CONT1552
1330	CONTINUE	CONT1553
	IF (NBEAMX.EQ.0) GO TO 1360	CONT1554
	REWIND 3	CONT1555
	REWIND 15	CONT1556
	REWIND 23	CONT1557
	DO 1350 I=1,NBEAMX	CONT1558
	READ (23) SIBXXC	CONT1559
	READ (15) SIBXXA	CONT1560
	DO 1340 J=1,NULAYB	CONT1561
	SIBXXA(J)=SIBXXA(J)+SIBXXC(J)	CONT1562
1340	CONTINUE	CONT1563
	WRITE (3) SIBXXA	CONT1564
1350	CONTINUE	CONT1565
1360	CONTINUE	CONT1566
C		CONT1567
C	CHECK LAYERS FOR FAILURE AND FIND FICTITIOUS FORCE VECTOR IF	CONT1568
C	PFC DOES NOT EQUAL ZERO FOR BOTH SLAB AND BEAM	CONT1569
C		CONT1570
	CALL PCHECK (IO,NA1,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,NANA,PZC,PDT,SZC	CONT1571
	1,SDT,DT,ELLA,ELLB,NPI,NPJ,NPK,NPL,FCADD,CTXX,CTYY,CTXY,CUXX,CUY	CONT1572
	2,UXY,ETXX,ETYY,ETXY,EIXX,EIYY,EIXY,SXXOT,SYYOT,SXYOT,SIGXXO,SIGYYO,	CONT1573
	3,SIGXYO,AS1,AF,AS,AFT,AST,NSTYPE,SPHI,SIGMAP)	CONT1574
C		CONT1575
	IF (NBEAMX.NE.0) CALL BCHECK (KC,NIX,NBEAMX,NULAYB,KMAT,NA1,IO,CUB	CONT1576
	1,EIB,DUM,NPIX,NPKX,SIBXXC,SIBXXA,ITYPE,FT,SIGO,ESX,EDOWNT,EDOWN,DIC	CONT1577
	2,SP,DISPLA,ZCRD,XLNGTH,ASXLR,EPSPS,STRAN,FCADD,IOUT)	CONT1578
C		CONT1579
	REWIND 33	CONT1580
	WRITE (33) ESX	CONT1581
C		CONT1582
	IF (PFC.NE.0.0) GO TO 1370	CONT1583
C		CONT1584
	ITP=3HNO	CONT1585
	GO TO 860	CONT1586
1370	CONTINUE	CONT1587
	ITP=3HYES	CONT1588
C		CONT1589
	IF (PLTCRD.EQ.3HYES) WRITE (8,1380) ICYCLE	CONT1590
1380	FORMAT (I5)	CONT1591
C		CONT1592
C	CHECK BEAM CRACK WIDTH	CONT1593
C		CONT1594
	I=ICRB2	CONT1595
	IF (I.NE.0.AND.NBEAMX.NE.0) CALL BEAMC (NBEAMX,NULAYB,NA1,EIB,CUB,	CONT1596
	1,SIBXXA,SIBXXC,EPSPS,ITYPE,ASXLR,AIR,KC,ZCRD,TSHEAR,ESX,DISPLA)	CONT1597
C		CONT1598
C	ACCUMULATE FIELD QUANTITIES	CONT1599



C	CALL ACCUMU (NUMEL,NA1,NBEAMX,NULAYB,NCNS,NULAY,CTXX,CTYY,CTXY,CUXCONT1600 1X,CUY,CUXY,ETXX,ETYY,ETXY,EIXX,EIYY,EIXY,SXXOT,SYOT,SXYOT,SIGXXOCONT1601 2,SIGYYO,SIGXYO,DISPLA,DISP,FORCE,FORCEA,ESX,ITYPE,ZGRD,DUM,EPSPS,ACONT1602 3S1,AFT,AST,SIBXXC,SIBXXA,AF,AS)	CONT1603
C	REWIND 17 WRITE (17) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,GTXX,CTYY,CTXY REWIND 18	CONT1604 CONT1605 CONT1606 CONT1607 CONT1608
C	WRITE (18) DISPO,FORCEB,FCADD	CONT1609
C	QAPAC=QAPAC*PFC	CONT1610 CONT1611
C	PRINT FORCE AND DISPLACEMENT VECTORS	CONT1612 CONT1613 CONT1614
C	CALL QPRINT (HDISP,HFORCE,RDISP,RFORCE,RDFPR,NSLAYR,NULAY,NCNS,NUMCONT1615 1EL,AFT,AST,AS1,NA1,NUMNP,IO,FORCEA,DISPLA)	CONT1616 CONT1617
C	PRINT SLAB INTERNAL FORCES	CONT1618 CONT1619
C	CALL PFORCE (IO,NUMEL,NCNS,NULAY,NSLAYR,RDFPR,6,PZC,PDT,DT,SDT,SZCCONT1620 1,SXXOT,SYOT,SXYOT)	CONT1621
C	PRINT BEAM INTERNAL FORCES	CONT1622 CONT1623 CONT1624
C	IF (NBEAMX.NE.0) CALL BFORCE (NULAYB,NBEAMX,IO,DUM,RDFPR,3,ASXLR,SCONT1625 1I8XXA,ZCRD)	CONT1626
C	IF (PLTCRD.EQ.3HYES) END FILE 8	CONT1627 CONT1628
C	CHECK PRINT CODE	CONT1629 CONT1630 CONT1631
C	IF (RDFPR.NE.3HYES) GO TO 1420	CONT1632
C	WRITE (IO,1390) 1390 FORMAT (//,10X,*TOTAL ACCUMULATED STRESSES IN SLAB LAYERS*,/,10X,*CONT1634 1(STRESS IS IN KSI, ANGLES ARE IN DEGREES*,/,10X,* + = TENSION, - =CONT1635 2 COMPRESSION)*,/) WRITE (IO,470)	CONT1636 CONT1637 CONT1638
C	PRINT SLAB STRESSES	CONT1639 CONT1640
C	CALL PRISTR (SXXOT,SYOT,SXYOT,1.0,6,NULAY,NUMEL,NCNS,NSLAYR,SPHI,CONT1641 1IO)	CONT1642 CONT1643
C	WRITE (IO,1400) 1400 FORMAT (//,10X,*TOTAL ACCUMULATED STRESSES IN BEAM LAYERS*,/,10X,*CONT1645 1(STRESS IS IN KSI, + = TENSION, - = COMPRESSION)*,/) WRITE (IO,480)	CONT1646 CONT1647 CONT1648 CONT1649
C	PRINT BEAM STRESSES	CONT1650
C	IF (NBEAMX.NE.0) CALL PRISRB (SIBXXA,3,NULAYB,NBEAMX,IO)	CONT1651 CONT1652 CONT1653
C	WRITE (IO,1410) 1410 FORMAT (//10X,*TOTAL NCRMAL, SHEAR, AND PRINCIPAL STRESSES IN BEAMCONT1654 1*/,10X,*(STRESS IS IN KSI, ANGLES ARE IN DEGREES*,/,10X,* + = TENSCONT1655 2ION, - = COMPRESSION)*,/) WRITE (IO,480)	CONT1656 CONT1657

C		CONT1658
	1420 CONTINUE	CONT1659
C		CONT1660
C	FIND SHEAR STRESSES AND PRINCIPAL STRESSES FOR THE BEAM	CONT1661
C		CONT1662
	IF (NBEAMX.NE.0) CALL SHEAR (NULAYB,NPERBY,XLNGTH,SIBXXA,ASXLR,TSHCONT1663	
	1EAR,ZGRD,ITYPE,KC,NOBM,IO,DUM,RDFPR,EXTRA,EXTRA1,EXTRA2,EXTRA3,EXTCONT1664	
	2RA4,EXTRA5,EXTRA6,EXTRA7,NBEAMX)	CONT1665
C		CONT1666
C	CHECK PRINT CODE	CONT1667
C		CONT1668
	IF (RDFPR.NE.3HYES) GO TO 1540	CONT1669
	WRITE (IO,20)	CONT1670
C		CONT1671
C	PRINT STRAINS	CONT1672
C		CONT1673
	IF (CODET.EQ.3HYES) CALL STOTL (IC,NCNS,NULAY,NUMEL,NSLAYR,PZC,PDTCONT1674	
	1,DT,SZC,SPHI,CTXX,CTYY,CTXY,ETXX,ETYY,ETXY)	CONT1675
C		CONT1676
C	PRINT CRACK/CRUSH/YIELD ANGLE IN SLAB LAYERS	CONT1677
C		CONT1678
	WRITE (IO,1430)	CONT1679
	1430 FORMAT (1H0,5X,*ANGLES SHOWING DIRECTION OF PRINCIPAL AXIS, FIRST CONT1680	
	1CRACK*,/,5X,* AND SECOND CRACK (ANGLE = 999.0 IF NO CRACK)*,/,5X,CONT1681	
	2* ANGLES ARE IN DEGREES*)	CONT1682
C		CONT1683
	WRITE (IO,1440)	CONT1684
	1440 FORMAT (1H0,5X,* EL LAYER PRINCIPAL*,8X,*FIRST*,18X,*SECOND*)CONT1685	
	REWIND 32	CONT1686
	I=NULAY+1	CONT1687
	DO 1490 N=1,NUMEL	CONT1688
	READ (32) AFT,AST	CONT1689
	DO 1460 M=1,NULAY	CONT1690
	IF (AS1(N,M).EQ.999.0) GO TO 1460	CONT1691
	WRITE (IO,1450) N,M,AS1(N,M),AF(N,M),AFT(M),AS(N,M),AST(M)	CONT1692
	1450 FORMAT (1H ,5X,I5,2X,I5,F12.6,2X,F12.6,1H-,A5,2X,F12.6,1H-,A5)	CONT1693
	1460 CONTINUE	CONT1694
	IF (NSLAYR.EQ.0) GO TO 1490	CONT1695
	DO 1480 M=I,NCNS	CONT1696
	IF (AS1(N,M).EQ.999.0) GO TO 1480	CONT1697
	WRITE (IO,1470) N,M,AS1(N,M),AFT(M)	CONT1698
	1470 FORMAT (1H ,5X,I5,2X,I5,F12.6,14X,1H-,A5)	CONT1699
	1480 CONTINUE	CONT1700
	1490 CONTINUE	CONT1701
C		CONT1702
C	PRINT FICTITIOUS FORCE VECTOR	CONT1703
C		CONT1704
	IF (CODEA.NE.3HYES) GO TO 1520	CONT1705
	WRITE (IO,20)	CONT1706
	WRITE (IO,1500)	CONT1707
	1500 FORMAT (///9X,*FICTIOUS NODAL POINT FORCE VECTOR*)	CONT1708
	WRITE (IO,1510)	CONT1709
	1510 FORMAT (1H0,5X,*NODAL POINTS*,8X,*U-LOAD*,11X,*V-LOAD*,12X,*W-LOADCONT1710	
	1*,12X,*MX-LOAD*,11X,*MY-LOAD*,//)	CONT1711
	WRITE (IO,440) (M,FCADD(5*M-4),FCADD(5*M-3),FCADD(5*M-2),FCADD(5*MCNT1712	
	1-1),FCADD(5*M),M=1,NUMNP)	CONT1713
	1520 CONTINUE	CONT1714
C		CONT1715

C	DO PRINTER PLOTS FOR BEAM/SLAB	CONT1716
C		CONT1717
	IF (PRINTB.NE.3HYES.AND.PRINTS.NE.3HYES) GO TO 1540	CONT1718
	IRS=5	CONT1719
	IOUTS=4	CONT1720
	RETURN	CONT1721
C		CONT1722
	1530 CONTINUE	CONT1723
	REWIND 17	CONT1724
	REWIND 18	CONT1725
	REWIND 29	CONT1726
	REWIND 31	CONT1727
	READ (17) FORCEA,DISPLA,FCSADD,ETXX,ETYY,ETXY,CTXX,CTYY,CTXY	CONT1728
	READ (18) DISPO,FORCEB,FCAOD	CONT1729
	READ (29) FORCE,LILA	CONT1730
	READ (31) AS1,AF,AS	CONT1731
C		CONT1732
C	SET UP DISPLACEMENT AND FORCE PRINT CONTROLS	CONT1733
C		CONT1734
	1540 CONTINUE	CONT1735
	I=NSIGNI(2)	CONT1736
	RDISP=DISPLA(I)/HDISP	CONT1737
	I=NSIGNI(1)	CONT1738
	RFORCE=FORCEA(I)/HFORCE	CONT1739
	RDFPR=3HNO	CONT1740
	IF (ABS(RDOLD-RDISP).GE.RDPR-0.0001) GO TO 1550	CONT1741
	IF (ABS(RFOLD-RFORCE).GE.RFPR-.0001) GO TO 1550	CONT1742
	GO TO 1560	CONT1743
	1550 CONTINUE	CONT1744
	RFOLD=RFORCE	CONT1745
	RDOLD=RDISP	CONT1746
	RDFPR=3HYES	CONT1747
	1560 CONTINUE	CONT1748
C		CONT1749
C	PERFORM TERMINATION CHECKS	CONT1750
C		CONT1751
	IF (ICHECK.EQ.NOCHECK) ICHECK=-1	CONT1752
	IF (QAPAC.GE.(QAPA+CQAPA)) ICHECK=-1	CONT1753
	ICHECK=ICHECK+1	CONT1754
	I=NSIGNI(2)	CONT1755
	IF (ABS(DISPLA(I)).GE.DSPMAX) IOUT=3HYES	CONT1756
	IF (ABS(DISPLA(I)).GE.DSPMAX) NVAL(1)=3HYES	CONT1757
	IF (NVAL(4).EQ.3HYES) IOUT=3HYES	CONT1758
	IF (NVAL(2).EQ.3HYES) IOUT=3HYES	CONT1759
	M=ICRS10	CONT1760
	A=CRS5	CONT1761
	IF (M.EQ.4HTOP ) A=CRS6	CONT1762
	IF (CRS7.GE.A) NVAL(3)=3HYES	CONT1763
	IF (NVAL(3).EQ.3HYES) IOUT=3HYES	CONT1764
C		CONT1765
	DO 1590 I=1,5	CONT1766
C		CONT1767
C	CHECK VALUES FOR SLAB	CONT1768
C		CONT1769
	ML=NSMAT+1	CONT1770
	DO 1570 J=1,ML	CONT1771
	IF (NVALS(J,I).EQ.3HYES) IOUT=3HYES	CONT1772
	1570 CONTINUE	CONT1773

C		CONT1774
C	CHECK VALUES FOR BEAM	CONT1775
C		CONT1776
	IF (N3EAMX.EQ.0) GO TO 1590	CONT1777
	DO 1580 J=1,KMAT	CONT1778
	IF (NVALB(J,I).EQ.3HYES) IOUT=3HYES	CONT1779
	1580 CONTINUE	CONT1780
C		CONT1781
	M=ICRB2	CONT1782
	IF (M.GT.0.AND.CR810.GE.CR89) NVAL(5)=3HYES	CONT1783
	IF (NVAL(5).EQ.3HYES) IOUT=3HYES	CONT1784
	1590 CONTINUE	CONT1785
C		CONT1786
	GO TO 860	CONT1787
G		CONT1788
C	AN EXIT PROCEDURE	CONT1789
C		CONT1790
	1600 CONTINUE	CONT1791
	WRITE (IO,1610)	CONT1792
	1610 FORMAT (1H0,*SCALING RATIO IS LESS THAN 1.0 OR IS INFINITE-----	CONT1793
	1--EXECUTION IS TERMINATED*)	CONT1794
	STOP 5	CONT1795
	RETURN	CONT1796
	END	CONT1797
	SUBROUTINE READCN (NUMEL,NUMNP,LWIDTH,ISCALE,IDEAD,ICMPLT,PRECRK,FREADCN 2	READCN 2
	1LTCRD,ICARDS,ECARDS,COEA,COEET,PRINTB,PRINTS,DGX,DGY,ETIME,NSIGNIREADCN 3	READCN 3
	2,DSPMAX,TOLDSP,RDPR,RFPR,NTRIAL,NITER,LITER,LTRIAL,PCTT,PCTG,LCYCLREADCN 4	READCN 4
	3E,RRATIO,TOLF,QAPA,RQAPA,CQAPA,NITERD,INOTEN,oload,PMAX,NCHECK,DPFREADCN 5	READCN 5
	4C,NREADB,KIN,KPS,KC,KMAT,FTOL,CTOL)	READCN 6
C		READCN 7
C	THIS SUBROUTINE READS INPUT DATA AND INITIALIZES	READCN 8
C		READCN 9
	COMMON /CB4/ IN,IC	READCN10
	COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)	READCN11
	COMMON /SEARCH/ ISRCH,ISRT	READCN12
C		READCN13
	DIMENSION NSIGNI(2)	READCN14
C		READCN15
	WRITE (IO,10)	READCN16
	10 FORMAT (1H0)	READCN17
C		READCN18
	WRITE (IO,20) NUMEL,NUMNP	READCN19
	20 FORMAT (//,5X,*NUMBER OF SLAB ELEMENTS, NUMEL =*,I8,READCN20	READCN20
	1//,5X,*NUMBER OF NODAL POINTS, NUMNP =*,I8,1X)	READCN21
C		READCN22
	WRITE (IO,30) LWIDTH	READCN23
	30 FORMAT (//,5X,*HALF BANDWIDTH, LWIDTH =*,I8,READCN24	READCN24
	11X)	READCN25
C		READCN26
	WRITE (IO,10)	READCN27
	WRITE (IO,10)	READCN28
C		READCN29
	READ (IN,40) ISCALE,IDEAD,MODES	READCN30
	40 FORMAT (2X,A3,2X,A3,2X,A3)	READCN31
	WRITE (IO,50) ISCALE,IDEAD,MODES	READCN32
	50 FORMAT (//,5X,*SCALE INITIAL SOLUTION AND PRESCRIBED FORCES =*,5X,READCN33	READCN33
	1A3,//,5X,*IS THERE A DEAD LOAD ON THE STRUCTURE, IDEAD =*,5X,A3,//READCN34	READCN34
	2,5X,*SOLUTION MODE IS ITERATIVE, MODES =*,5X,A3,1X)	READCN35

C	READ (IN,40) ICARDS,ECARDS	READCN36
	WRITE (IO,60) ICARDS,ECARDS	READCN37
	60 FORMAT (//,5X,*ARE THERE INITIAL START CARDS, ICARDS	READCN38
	1A3,//,5X,*WILL THERE BE END CARDS, ECARDS	READCN39
	2)	READCN40
C		READCN41
C		READCN42
	READ (IN,40) PRINTB,PRINTS	READCN43
	WRITE (IO,70) PRINTB,PRINTS	READCN44
	70 FORMAT (//,5X,*PRINTER PLOT FOR THE BEAMS, PRINTB	READCN45
	1A3,//,5X,*PRINTER PLOT FOR THE SLAB, PRINTS	READCN46
	2)	READCN47
		READCN48
C		READCN49
	READ (IN,90) DGX,DGY	READCN50
	WRITE (IO,80) DGX,DGY	READCN51
	80 FORMAT (//,5X,*GRAPH LENGTH IN X DIRECTION ACROSS THE PAGE	READCN52
	12,* (IN)*,//,5X,*GRAPH LENGTH IN Y DIRECTION DOWN THE PAGE	READCN53
	28.2,* (IN)*,1X)	READCN54
		READCN55
C		READCN56
C		READCN57
	READ (IN,90) ETIME	READCN58
	90 FORMAT (8F10.0)	READCN59
	WRITE (IO,100) ETIME	READCN60
	100 FORMAT (//,5X,*MAXIMUM RUN TIME (SECONDS), ETIME	READCN61
	12,1X)	READCN62
C		READCN63
	READ (IN,110) NSIGNI(1),NSIGNI(2)	READCN64
	110 FORMAT (16I5)	READCN65
	WRITE (IO,120) NSIGNI(1),NSIGNI(2)	READCN66
	120 FORMAT (//,5X,*FORCE (W-DIRECTION) NODAL POINT	READCN67
	1//,5X,*DISPLACEMENT (W-DIRECTION) NODAL POINT	READCN68
		READCN69
C		READCN70
	READ (IN,90) DSPMAX,oload,PMAX	READCN71
	WRITE (IO,130) DSPMAX,oload,PMAX	READCN72
	130 FORMAT (//,5X,*MAXIMUM DISPLACEMENT FOR SIGNIFICANT POINT	READCN73
	14,* (IN)*,//,5X,*SPECIFIED OVERLOAD	READCN74
	28.3,* (KIPS)*,//,5X,*MAXIMUM RATIO OF APPLIED LOAD TO OVERLOAD	READCN75
	3=*,F8.3,/,1X)	READCN76
		READCN77
C		READCN78
	IF (DSPMAX.EQ.0.0) DSPMAX=1.0E+20	READCN79
	IF (oload.EQ.0.0) oload=1.0	READCN80
	IF (PMAX.EQ.0.0) PMAX=1.0E+20	READCN81
C		READCN82
	READ (IN,90) QAPA	READCN83
	WRITE (IO,140) QAPA	READCN84
	140 FORMAT (//,5X,*FORCE RATIO INCREMENT ,QAPA	READCN85
	13,1X)	READCN86
		READCN87
C		READCN88
	READ (IN,90) RDPR,RFPR	READCN89
	WRITE (IO,150) RDPR,RFPR	READCN90
	150 FORMAT (//,5X,*DISPLACEMENT RATIO INCREMENT FOR PRINTING	READCN91
	14,//,5X,*FORCE RATIO INCREMENT FOR PRINTING	READCN92
		READCN93
C		READCN94
	READ (IN,110) LCYCLE	READCN95
	WRITE (IO,160) LCYCLE	READCN96
	160 FORMAT (//,5X,*NUMBER OF LOAD CYCLES, LCYCLE	READCN97

	11X)	READCN94
C	WRITE (IO,10)	READCN95
	WRITE (IO,10)	READCN96
C	READ (IN,110) NREADB,KIN,KPS	READCN97
		READCN98
C		READCN99
	WRITE (IO,170) NREADB,KIN,KPS	READC100
	170 FORMAT (//,5X,*NUMBER OF BEAMS TO BE READ IN, NREADB	READC101
	1//,5X,*READ ONLY FIRST ELEMENT IF KIN = 0, KIN	READC102
	2EAD LOAD OR PRESTRESS FOR BEAMS (0=NO), KPS =*,I8,1X)	READC103
C		READC104
	READ (IN,180) ISRT,ISRCH	READC105
	180 FCRMAT(A3,A4)	READC106
	WRITE (IO,190) ISRT,ISRCH	READC107
	190 FORMAT (//,5X,*COMPUTE ELEMENT STRESS USING	READC108
	1A4,//)	READC109
C		READC110
	ISRCH=3H NO	READC111
	IF (ISRT.EQ.3HNOD) ISRCH=3HYES	READC112
C		READC113
	THIS PROGRAM CAN BE EXTENDED TO CONSIDER SKEWED BRIDGES BY-	READC114
C		READC115
C	1. INPUTTING THE CORRECT SKEW ANGLE	READC116
C		READC117
C	2. IF APPROPRIATE, SPECIFYING FIXED BC CONDITIONS IN THE SKEW	READC118
C	COORDINATE SYSTEM, I.E. IVALUE = 2 RATHER THAN 1	READC119
C		READC120
C	3. SPECIFYING COORDINATE LOCATIONS AND AREAS FOR	READC121
C	DISTRIBUTED LOADS, PATCH LOADS, AND CONCENTRATED	READC122
C	LOADS IN THE SKEW COORDINATE SYSTEM	READC123
C		READC124
C	4. SPECIFYING THE SLAB ELEMENT DIMENSIONS IN THE SKEW	READC125
C	COORDINATE SYSTEM, I.E. YSEG IN SUBROUTINE PINF	READC126
C		READC127
C	SEE F.E.L. REPORT NO. 400.20 FOR COMMENTS ON SKEW BRIDGES	READC128
C		READC129
	PHI=90.0	READC130
	WRITE (IO,200) PHI	READC131
	200 FORMAT (1H0,//,5X,*SKEW ANGLE (DEGREES)	READC132
	1,F8.3)	READC133
C		READC134
	CARTESIAN FORCE = AU*SKEW FORCE.. INPLANE	READC135
C		READC136
	RPHI=PHI*4.*ATAN(1.)/180.	READC137
	AU(1,1)=1.	READC138
	AU(1,2)=COS(RPHI)	READC139
	AU(2,1)=0.	READC140
	AU(2,2)=SIN(RPHI)	READC141
C		READC142
	CARTESIAN FORCE = AO*SKEW FORCE.. BENDING	READC143
C		READC144
	AO(1,1)=1.	READC145
	AO(2,1)=0.	READC146
	AO(1,2)=0.	READC147
	AO(2,3)=0.	READC148
	AO(1,3)=0.	READC149
	AO(3,1)=0.	READC150
		READC151

	A0(2,2)=SIN(RPHI)	READC152
	A0(3,2)=-COS(RPHI)	READC153
	A0(3,3)=1.	READC154
C		READC155
C	CARTESIAN STRAIN = DS* SKEW STRAIN	READC156
C		READC157
	DS(1,1)=1.	READC158
	DS(1,2)=0.	READC159
	DS(1,3)=0.	READC160
	DS(3,2)=0.	READC161
	DS(2,1)=1./(TAN(RPHI)*TAN(RPHI))	READC162
	DS(2,2)=1./(SIN(RPHI)*SIN(RPHI))	READC163
	DS(2,3)=-COS(RPHI)/(SIN(RPHI)*SIN(RPHI))	READC164
	DS(3,1)=-2./TAN(RPHI)	READC165
	DS(3,3)=1./SIN(RPHI)	READC166
C		READC167
C		READC168
	NITERD=5	READC169
C		READC170
	RRATIO=.5	READC171
	RQAPA=.2	READC172
	CGAPA=.1	READC173
	NCHECK=3	READC174
	DPFG=.1	READC175
C		READC176
	TOLF=.1	READC177
	TOLDSP=.20	READC178
C		READC179
	FTOL=.10	READC180
	GTOL=.1	READC181
C		READC182
	PCTT=.90	READC183
	PCTC=.60	READC184
C		READC185
	NTRIAL=1	READC186
	NITER=4	READC187
	LITER=1	READC188
	LTRIAL=1	READC189
	IF (MODES.EQ.3HYES) LITER=3	READC190
	I=3	READC191
	J=3	READC192
	IF (NSIGNI(1).NE.0) NSIGNI(1)=(NSIGNI(1)-1)*5+J	READC193
	IF (NSIGNI(2).NE.0) NSIGNI(2)=(NSIGNI(2)-1)*5+J	READC194
C		READC195
	INOTEN=3HNO	READC196
	PLTCRD=3HYES	READC197
	ICMPLT=3HNO	READC198
	PREGRK=3HNO	READC199
	CODEA=3HNO	READC200
	CODET=3HNO	READC201
C		READC202
	RETURN	READC203
	END	READC204
	SUBROUTINE INITIA (NUMEL,NGNS,NULAY,NA1,ETXX,SXXOT,AS1,AF,AS,AFT,DINITIA 2	
	1ISPLA)	INITIA 3
C		INITIA 4
C	THIS SUBROUTINE INITIALIZES ARRAYS/TAPES ASSOCIATED WITH THE SLAB	INITIA 5
C		INITIA 6

DIMENSION SXXOT(NCNS), AS1(NUMEL,NCNS), AF(NUMEL,NULAY), AS(NUMEL,INITIA 7  
 1NULAY), AFT(NCNS), DISPLA(NA1), ETXX(NUMEL), DUMMY(1) INITIA 8  
 INITIA 9

C	COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	INITIA10
	COMMON /SEARCH/ ISRCH	INITIA11
	COMMON /PDIM/ PDT(70)	INITIA12
	COMMON /STCHK/ STRAMS(30)	INITIA13
	COMMON /HISTS/ NVALS(30),IVAL(10)	INITIA14
	COMMON /CRWB/ CRB(10)	INITIA15
C	COMMON /CRWS/ CRS(10)	INITIA16
	REWIND 19	INITIA17
	REWIND 21	INITIA18
	REWIND 31	INITIA19
	REWIND 32	INITIA20
C	DO 10 I=1,10	INITIA21
	NVAL(I)=3H NO	INITIA22
	IVAL(I)=0	INITIA23
	FVAL(I)=-999.	INITIA24
	10 CONTINUE	INITIA25
C	DO 30 N=1,NUMEL	INITIA26
	ETXX(N)=0.0	INITIA27
	DO 20 ML=1,NCNS	INITIA28
	AS1(N,ML)=999.0	INITIA29
	20 CONTINUE	INITIA30
	DO 30 ML=1,NULAY	INITIA31
	AF(N,ML)=999.0	INITIA32
	AS(N,ML)=999.0	INITIA33
	30 CONTINUE	INITIA34
	DO 40 ML=1,NCNS	INITIA35
	SXXOT(ML)=0.0	INITIA36
	AFT(ML)=5H	INITIA37
	40 CONTINUE	INITIA38
	DO 50 I=1,NA1	INITIA39
	DISPLA(I)=0.0	INITIA40
	50 CONTINUE	INITIA41
	DO 60 I=1,NUMEL	INITIA42
	WRITE (19) SXXOT,SXXOT,SXXOT	INITIA43
	WRITE (21) SXXOT,SXXOT,SXXOT	INITIA44
	WRITE (32) AFT,AFT	INITIA45
	60 CONTINUE	INITIA46
C	N=0	INITIA47
	DO 70 I=1,NUMEL	INITIA48
	70 WRITE (32) N,N,N	INITIA49
C	WRITE (31) AS1,AF,AS	INITIA50
C	REWIND 18	INITIA51
	WRITE (18) DISPLA,DISPLA,CISPLA	INITIA52
	REWIND 17	INITIA53
	WRITE (17) DISPLA,DISFLA,DISPLA,ETXX,ETXX,ETXX,ETXX,ETXX,ETXX	INITIA54
	REWIND 30	INITIA55
	WRITE (30) DISPLA	INITIA56
C	DO 80 I=1,70	INITIA57
		INITIA58
		INITIA59
		INITIA60
		INITIA61
		INITIA62
		INITIA63
		INITIA64



	80 PDT(I)=-1.E+20	INITIA65
C		INITIA66
	DO 90 I=1,30	INITIA67
	90 STRAMS(I)=-1.0	INITIA68
C		INITIA69
	DO 100 I=1,60	INITIA70
	100 IVALS(I)=0	INITIA71
C		INITIA72
	DO 110 I=1,30	INITIA73
	110 NVALS(I)=3H NO	INITIA74
C		INITIA75
	DO 120 I=1,10	INITIA76
	120 CRS(I)=0.	INITIA77
	DO 130 I=1,10	INITIA78
	130 CRB(I)=0.	INITIA79
	RETURN	INITIA80
	END	INITIA81
	SUBROUTINE INITIB (NBEAMX,NULAYB,NNN,SIBXXA)	INITIB 2
C		INITIB 3
C	THIS SUBROUTINE INITIALIZES ARRAYS ASSOCIATED WITH BEAM ELEMENTS.	INITIB 4
C		INITIB 5
	COMMON /BDIM/ RON(48)	INITIB 6
	COMMON /BTCHK/ STRAMX(30)	INITIB 7
	COMMON /HISTB/ NVALB(30),IVALB(60)	INITIB 8
	DIMENSION SIBXXA(NULAYB), DUMMY(1)	INITIB 9
	REWIND 23	INITIB10
	REWIND 25	INITIB11
	DO 10 ML=1,NULAYB	INITIB12
	SIBXXA(ML)=0.0	INITIB13
	10 CONTINUE	INITIB14
	DO 20 I=1,NBEAMX	INITIB15
	WRITE (23) SIBXXA	INITIB16
	WRITE (25) SIBXXA	INITIB17
	20 CONTINUE	INITIB18
C		INITIB19
	DO 30 I=1,30	INITIB20
	30 NVALB(I)=3H NO	INITIB21
C		INITIB22
	DO 40 I=1,60	INITIB23
	40 IVALB(I)=0	INITIB24
C		INITIB25
	DO 50 I=1,48	INITIB26
	50 RON(I)=-1.E+20	INITIB27
C		INITIB28
	DO 60 I=1,30	INITIB29
	60 STRAMX(I)=-1.	INITIB30
C		INITIB31
	RETURN	INITIB32
	END	INITIB33
	SUBROUTINE BSCALE (RB,IFN,ISCALE,KC,DUM,NBEAMX,NULAYB,KMAT,PCTT,PCBSCALE	BSCALE 2
	1TC,SIBXXA,SIBXXC,ITYPE,SIGO,FT)	BSCALE 3
C		BSCALE 4
C	THIS SUBROUTINE FINDS THE BEAM SCALE FACTOR	BSCALE 5
C		BSCALE 6
C	COMMON BLOCKS	BSCALE 7
C		BSCALE 8
	COMMON /CB4/ IN,IC	BSCALE 9
	DIMENSION SIGO(KMAT), FT(KMAT), ITYPE(NULAYB), SIBXXC(NULAYB), SIBSCALE10	

	1XXA(NULAY9), DUM(NULAY8), DUMMY(1)	BSCALE11
C		BSCALE12
	10 CONTINUE	BSCALE13
	RB=0.	BSCALE14
C		BSCALE15
	REWIND 15	BSCALE16
	REWIND 23	BSCALE17
	REWIND 27	BSCALE18
C		BSCALE19
	DO 50 N=1,N3EAMX	BSCALE20
	READ (15) SIBXXA	BSCALE21
	READ (23) SIBXXC	BSCALE22
	READ (27) S,S,S,S,ITYFE,DUM,DUM,DUM,DUM	BSCALE23
	DO 50 ML=1,NULAY8	BSCALE24
	SIBA=SIBXXA(ML)	BSCALE25
	S=SIBXXC(ML)	BSCALE26
	K=ITYFE(ML)	BSCALE27
C		BSCALE28
	IF (IFN.EQ.3HYES) GO TO 40	BSCALE29
C		BSCALE30
C	CONSIDER INITIAL STRESS WHEN SCALING	BSCALE31
C		BSCALE32
	IF (S.GT.0.0) GO TO 20	BSCALE33
	RR=PCTC	BSCALE34
	IF (K.GT.KC) RR=PCTT	BSCALE35
	D=-SIGO(K)*RR-SIBA	BSCALE36
	IF (D.GE.S) GO TO 60	BSCALE37
	RATIO=S/D	BSCALE38
	GO TO 30	BSCALE39
	20 CONTINUE	BSCALE40
	D=FT(K)*PCTT-SIBA	BSCALE41
	IF (D.LE.S) GO TO 60	BSCALE42
	RATIO=S/D	BSCALE43
	30 IF (RATIO.GT.RB) RB=RATIO	BSCALE44
	GO TO 50	BSCALE45
C		BSCALE46
	40 CONTINUE	BSCALE47
C		BSCALE48
C	SCALING IS FINISHED/ USED FOR FINDING WHAT FORCE VECTOR TO APPLY	BSCALE49
C		BSCALE50
	RR=PCTC*SIGO(K)	BSCALE51
	IF (S.GT.0.0.AND.K.LE.KC) RR=PCTT*FT(K)	BSCALE52
	IF (K.GT.KC) RR=PCTT*SIGO(K)	BSCALE53
	RATIO=ABS(S/RR)	BSCALE54
	IF (RATIO.GT.RB) RB=RATIO	BSCALE55
	50 CONTINUE	BSCALE56
	IF (RB.EQ.0.0) RB=1.0E-09	BSCALE57
	RETURN	BSCALE58
	60 CONTINUE	BSCALE59
C		BSCALE60
	IFN=3HYES	BSCALE61
	WRITE (IO,70)	BSCALE62
	70 FORMAT (1H0,/,/,6X,*PCTC OR PCTT LIMITS EXCEEDED IN BSCALE*,/)	BSCALE63
	GO TO 10	BSCALE64
C		BSCALE65
	END	BSCALE66
	SUBROUTINE PSCALE (RATIO,IFN,ISCALE,SXXOT,SYOT,SXYOT,PCTT,PCTC,IOPSCALE	2
	1,NCNS,NUMEL,NULAY,NSLAYR,NSMAT,SIGXXO,SIGYYO,SIGXYO,NSTYPE,SPHI,SIPSCALE	3

	2GMAP)	PSCALE 4
C		PSCALE 5
C	THIS SUBROUTINE FINDS THE PLATE SCALE FACTOR	PSCALE 6
C		PSCALE 7
	DIMENSION SIGXX0(NCNS), SIGYY0(NCNS), SIGXY0(NCNS), SXXOT(NCNS), SPSCALE 8 1YYOT(NCNS), SXYOT(NCNS), SPHI(NSLAYR), NSTYPE(NSLAYR), SIGMAP(NSMAPSCALE 9 2T), DUMMY(1)	PSCALE 10
C		PSCALE 11
	COMMON /TYPENN/ TYPE	PSCALE 12
	COMMON /PROPC/ V(11)	PSCALE 13
C		PSCALE 14
	10 CONTINUE	PSCALE 15
	REWIND 19	PSCALE 16
	REWIND 21	PSCALE 17
	FG=V(3)	PSCALE 18
	FT=V(5)	PSCALE 19
C		PSCALE 20
	RATIO=0.0	PSCALE 21
	DO 60 N=1,NUMEL	PSCALE 22
C		PSCALE 23
	READ (19) SIGXX0,SIGYY0,SIGXY0	PSCALE 24
	READ (21) SXXOT,SYOT,SXYOT	PSCALE 25
C		PSCALE 26
	DO 60 ML=1,NCNS	PSCALE 27
	SX=SIGXX0(ML)	PSCALE 28
	SY=SIGYY0(ML)	PSCALE 29
	SXY=SIGXY0(ML)	PSCALE 30
	SXT=SXXOT(ML)	PSCALE 31
	SYT=SYOT(ML)	PSCALE 32
	SXYT=SXYOT(ML)	PSCALE 33
	IF (ML.GT.NULAY) GO TO 20	PSCALE 34
	CALL PRNCIP (SX,SY,SXY,S1,S2,THETA)	PSCALE 35
	CALL PRNCIP (SXT,SYT,SXYT,S1T,S2T,THETA)	PSCALE 36
	S1T=-S1T	PSCALE 37
	S2T=-S2T	PSCALE 38
C		PSCALE 39
C	SET INITIAL STRESS IN SLAE TO ZERO IF SCALING IS FINISHED, USED TO	PSCALE 40
C	FIND THE FORCE VECTOR FOR THE INCREMENTAL SOLUTION.	PSCALE 41
C		PSCALE 42
	IF (IFN.EQ.3HYES) S1T=0.0	PSCALE 43
	IF (IFN.EQ.3HYES) S2T=0.0	PSCALE 44
C		PSCALE 45
	S1=-S1	PSCALE 46
	S2=-S2	PSCALE 47
	GO TO 30	PSCALE 48
	20 CONTINUE	PSCALE 49
	ANG=SPHI(ML-NULAY)	PSCALE 50
	CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	PSCALE 51
	CALL TRANG (SXT,SYT,SXYT,ANG,S1T,S2T,S3T)	PSCALE 52
	M=NSTYPE(ML-NULAY)	PSCALE 53
	SP1=SIGMAP(M)	PSCALE 54
	IF (S1.LT.0.0) SP1=-SP1	PSCALE 55
	D=SP1*PCTT-S1T	PSCALE 56
	IF (IFN.NE.3HYES.AND.ABS(D).LE.ABS(S1)) GO TO 70	PSCALE 57
	TEMP1=S1/D	PSCALE 58
	GO TO 50	PSCALE 59
	30 CONTINUE	PSCALE 60
C		PSCALE 61

C	ESTIMATE FINAL STRESS STATE AFTER SCALING	PSCALE62
C		PSCALE63
	X=S1	PSCALE64
	Y=S2	PSCALE65
	IF (S2.EQ.0.0) Y=1.0E-09	PSCALE66
	IF (S1.EQ.0.0) X=1.0E-09	PSCALE67
C		PSCALE68
	S1X=FC	PSCALE69
	S2X=FC	PSCALE70
	IF (S1.LT.0.0) S1X=-FT	PSCALE71
	IF (S2.LT.0.0) S2X=-FT	PSCALE72
C		PSCALE73
	SA=S1X	PSCALE74
	SB=Y*(SA-S1T)/X+S2T	PSCALE75
	IF (ABS(SB).LE.(ABS(S2X)+.001)) GO TO 40	PSCALE76
C		PSCALE77
	SB=S2X	PSCALE78
	SA=X*(SB-S2T)/Y+S1T	PSCALE79
C		PSCALE80
	40 CONTINUE	PSCALE81
C		PSCALE82
C	SCALE FACTOR IS BASED ON ESTIMATED STRESS PATH	PSCALE83
C		PSCALE84
	CALL BAXIAL (SB,SA,ALPH2,SP2,EP2,DIRT2,EC2)	PSCALE85
	RR=PCTC	PSCALE86
	IF (TYPE.EQ.5HLINER) RR=PCTT	PSCALE87
	D=SP2*RR-S2T	PSCALE88
	IF (IFN.NE.3HYES.AND.ABS(D).LE.ABS(S2)) GO TO 70	PSCALE89
	TEMP2=S2/D	PSCALE90
	IF (TEMP2.GT.RATIO) RATIO=TEMP2	PSCALE91
	CALL BAXIAL (SA,SB,ALPH1,SP1,EP1,DIRT1,EC1)	PSCALE92
	RR=PCTC	PSCALE93
	IF (TYPE.EQ.5HLINER) RR=PCTT	PSCALE94
	D=SP1*RR-S1T	PSCALE95
	IF (IFN.NE.3HYES.AND.ABS(D).LE.ABS(S1)) GO TO 70	PSCALE96
	TEMP1=S1/D	PSCALE97
	50 IF (TEMP1.GT.RATIO) RATIO=TEMP1	PSCALE98
C		PSCALE99
	60 CONTINUE	PSCALE100
	IF (RATIO.EQ.0.0) RATIO=1.0E-09	PSCALE101
	RETURN	PSCALE102
	70 CONTINUE	PSCALE103
C		PSCALE104
	IFN=3HYES	PSCALE105
	D=-D	PSCALE106
	WRITE (IO,80)	PSCALE107
	80 FORMAT (1H0,/,6X,*PCTC OR PCTT LIMITS EXCEEDED IN PSCALE*,/)	PSCALE108
	WRITE (IO,90) N,ML,D	PSCALE109
	90 FORMAT (1H ,5X,*ELEMENT *,I3,* LAYER *,I3,* ALLOWABLE STRESS	PSCALE110
	1CHANGE OF *,E16.9,* HAS BEEN EXCEEDED*,/)	PSCALE111
	GO TO 10	PSCALE112
C		PSCALE113
	END	PSCALE114
	SUBROUTINE BSTRAN (NIX,NULAY8,NA1,NBEAMX,EIB,CUB,DUM,DISP)	BSTRAN 2
C		BSTRAN 3
C	THIS SUBROUTINE COMPUTES THE INPLANE STRAIN AND THE BENDING	BSTRAN 4
C	CURVATURE INCREMENTS FOR THE BEAM ELEMENTS	BSTRAN 5
C		BSTRAN 6

	DIMENSION DISP(NA1), EIB(NBEAMX), CUB(NBEAMX), DUM(NULAYB), DUMMY(BSTRAN 7 11)	BSTRAN 8 BSTRAN 9
C	REWIND 27	BSTRAN10
C	DO 10 LSE=1,NBEAMX	BSTRAN11 BSTRAN12
C	READ (27) XLNGTH,GKEI,NPIX,NPKX,DUM,DUM,DUM,DUM,DUM	BSTRAN13 BSTRAN14
C	FIND LOCATION OF NODE POINTS	BSTRAN15
C	I=NPIX	BSTRAN16 BSTRAN17
C	K=NPKX	BSTRAN18
C	I=5*I	BSTRAN19
C	K=5*K	BSTRAN20 BSTRAN21
C	COMPUTE INPLANE STRAINS AND CURVATURES	BSTRAN22
C	EIB(LSE)=(DISP(K-4)-DISP(I-4))/XLNGTH	BSTRAN23 BSTRAN24
C	CUB(LSE)=(DISP(K)-DISP(I))/XLNGTH	BSTRAN25
C	10 CONTINUE	BSTRAN26 BSTRAN27
C	RETURN	BSTRAN28
C	END	BSTRAN29
C	SUBROUTINE BSTRSS (PF,DUM,ZCRD,EIB,CUB,KC,NULAYB,NBEAMX,KMAT,SIBXXC,BSTRSS 2 1A,SIBXXC,ESX,RON,ROM,SIGO,FT,RONT,ROMT,ITYPE)	BSTRSS 3
C	THIS SUBROUTINE COMPUTES THE STRESS INCREMENTS	BSTRSS 4
C	AND THE PRESCAN FACTOR	BSTRSS 5
C		BSTRSS 6
C		BSTRSS 7
C		BSTRSS 8
C	DIMENSION ESX(NULAYB,NBEAMX), RON(KMAT), ROM(KMAT), SIGO(KMAT), FT,BSTRSS 9 1(KMAT), RONT(KC), ROMT(KC), SIBXXA(NULAYB), SIBXXC(NULAYB), ITYPE(BSTRSS10 2NULAYB), ZCRD(NULAYB), EIB(NBEAMX), CUB(NBEAMX), DUM(NULAYB), DUMBSTRSS11	BSTRSS12
C	3Y(1)	BSTRSS13
C	COMMON /CB309/ FTCL,CTOL	BSTRSS14
C	REWIND 5	BSTRSS15
C	REWIND 15	BSTRSS16
C	REWIND 23	BSTRSS17
C	REWIND 27	BSTRSS18 BSTRSS19
C	DO 40 N=1,NBEAMX	BSTRSS20
C	READ (15) SIBXXA	BSTRSS21
C	READ (23) SIBXXC	BSTRSS22
C	READ (27) XLNGTH,GKEI,NPIX,NPKX,ITYPE,DUM,DUM,ZCRD,DUM	BSTRSS23 BSTRSS24
C	DO 30 ML=1,NULAYB	BSTRSS25 BSTRSS26
C		BSTRSS27
C		BSTRSS28
C		BSTRSS29
C	ES=ESX(ML,N)	BSTRSS30
C	SIBA=SIBXXA(ML)	BSTRSS31
C	SIBC=SIBXXC(ML)	BSTRSS32
C	ZCR=ZCRD(ML)	BSTRSS33
C	EPSXBT=EIB(N)+CUB(N)*ZCR	BSTRSS34
C	S=SIBA+SIBC	BSTRSS35
C	K=ITYPE(ML)	BSTRSS36

C		BSTRSS37
C	FIND MODULUS	BSTRSS38
C		BSTRSS39
C	IF (ES.EQ.0.0) GO TO 30	BSTRSS40
C	ZESX=ES/(1.0+RON(K)*(1.0-ROM(K))/ROM(K)*((ABS(S)/SIGO(K))**(RON(K)	BSTRSS41
	1-1.)))	BSTRSS42
C		BSTRSS43
C	IF (S.GT.0.0.AND.K.LE.KC) ZESX=ES/(1.+RONT(K)*(1.-ROMT(K))/ROMT(K)	BSTRSS44
	1*((ABS(S)/FT(K))**(RONT(K)-1.)))	BSTRSS45
C		BSTRSS46
C	IF (K.GT.KC) GO TO 10	BSTRSS47
		BSTRSS48
C		BSTRSS49
C	CHECK LAYER FAILURE	BSTRSS50
C		BSTRSS51
	SP=-SIGO(K)	BSTRSS52
	IF (S.GT.0.) SP=FT(K)	BSTRSS53
	IF (ABS(S).GT.ABS(SP)) ZESX=0.0	BSTRSS54
	10 CONTINUE	BSTRSS55
C		BSTRSS56
C	FIND PRESCAN FACTOR	BSTRSS57
C		BSTRSS58
	SIBC=EPSXBT*ZESX	BSTRSS59
	SIBXXC(ML)=SIBC	BSTRSS60
	S=SIBC+SIBA	BSTRSS61
	SP=-SIGO(K)	BSTRSS62
	IF (S.GT.0.) SP=FT(K)	BSTRSS63
	IF (ABS(S).LT.ABS(SP)) GO TO 30	BSTRSS64
	IF (K.GT.KC) GO TO 30	BSTRSS65
	IF (ABS(S-SIBA).LT.1.0E-06) GO TO 20	BSTRSS66
	ELOW=-SIGO(K)*(1.-CTOL/2.)	BSTRSS67
	IF (S.GT.0.) ELOW=FT(K)*(1.-FTOL/2.)	BSTRSS68
	TEMP=ABS((ELOW-SIBA)/(S-SIBA))	BSTRSS69
	IF (TEMP.LT.PF) PF=TEMP	BSTRSS70
	GO TO 30	BSTRSS71
	20 CONTINUE	BSTRSS72
	PF=0.0	BSTRSS73
	30 CONTINUE	BSTRSS74
	WRITE (5) SIBXXC	BSTRSS75
	40 CONTINUE	BSTRSS76
	REWIND 23	BSTRSS77
	REWIND 5	BSTRSS78
	CALL FTNCOPY (4LJUM5,3LJ23,0)	BSTRSS79
	RETURN	BSTRSS80
	END	BSTRSS81
C	SUBROUTINE PSTRAN (NUMEL,NA1,NANA,NULAY,CUXX,CUYX,CUY, EIXX,FIYY,EPSTRAN	PSTRAN 2
	1IXY,DISP,PDT,PZC,SZC,AFT,AST,NSLAYR)	PSTRAN 3
C		PSTRAN 4
C	THIS SUBROUTINE COMPUTES THE INPLANE STRAINS AND CURVATURES	PSTRAN 5
C	FOR THE SLAB	PSTRAN 6
C		PSTRAN 7
	DIMENSION CUXX(NUMEL), CUYX(NUMEL), CUY(NUMEL), EIXX(NUMEL), EIYY(PSTRAN	PSTRAN 8
	1(NUMEL), EIXY(NUMEL), DISP(NA1), PDT(NULAY), PZC(NULAY), CURV(3), PSTRAN	PSTRAN 9
	2BB(3,12), DPV(12)	PSTRAN10
	DIMENSION EUT(3), DPUT(8), CURVT(3), DPVT(12)	PSTRAN11
	DIMENSION AST(NULAY), AFT(NULAY), SZC(NSLAYR)	PSTRAN12
	DIMENSION BU(3,8), EE(3), ECUR(3), IXP(5), IYP(5), NODE(5), DPU(8)	PSTRAN13
C		PSTRAN14

COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)  
COMMON /SEARCH/ ISRCH,ISRT

PSTRAN15  
PSTRAN16  
PSTRAN17

C

DATA (IXP(I),I=1,5)/-1,-1,1,1,0/  
DATA (IYP(I),I=1,5)/1,-1,1,-1,0/

PSTRAN18  
PSTRAN19  
PSTRAN20

C

C POSITION TAPE

PSTRAN21

C

REWIND 32

PSTRAN22

PSTRAN23

DO 10 I=1,NUMEL

PSTRAN24

10 READ (32) AFT,AST

PSTRAN25

C

REWIND 28

PSTRAN26

PSTRAN27

C

DO 310 N=1,NUMEL

PSTRAN28

PSTRAN29

READ (28) DT,ELLA,ELLE,NPI,NPJ,NFK,NPL

PSTRAN30

C

NODE(1)=NPI

PSTRAN31

NODE(2)=NPJ

PSTRAN32

NODE(3)=NPK

PSTRAN33

NODE(4)=NPL

PSTRAN34

NODE(5)=0

PSTRAN35

PSTRAN36

C

C GET THE BENDING DISP

PSTRAN37

PSTRAN38

C

I=NPI\*5-3

PSTRAN39

J=NPJ\*5-3

PSTRAN40

PSTRAN41

K=NPK\*5-3

PSTRAN42

L=NPL\*5-3

PSTRAN43

DO 20 II=1,3

PSTRAN44

DPV(II)=DISP(I+II)

PSTRAN45

DPV(II+3)=DISP(J+II)

PSTRAN46

DPV(II+6)=DISP(K+II)

PSTRAN47

20 DPV(II+9)=DISP(L+II)

PSTRAN48

C

C GET THE INPLANE DISP

PSTRAN49

PSTRAN50

C

I=I-2

PSTRAN51

J=J-2

PSTRAN52

K=K-2

PSTRAN53

L=L-2

PSTRAN54

PSTRAN55

C

DO 30 II=1,2

PSTRAN56

DPU(II)=DISP(I+II)

PSTRAN57

DPU(II+2)=DISP(J+II)

PSTRAN58

DPU(II+4)=DISP(K+II)

PSTRAN59

DPU(II+6)=DISP(L+II)

PSTRAN60

30 CONTINUE

PSTRAN61

PSTRAN62

C

IF (ABS(PHI).EQ.9(.)) GO TO 100

PSTRAN63

PSTRAN64

C

C GET THE SKEW DISPLACEMENTS

PSTRAN65

C

DO 50 II=1,12,3

PSTRAN66

DO 50 I=1,3

PSTRAN67

SUM=0.

PSTRAN68

PSTRAN69

C

C SKEW DISP = AO\*\*T \*CARTESIAN DISP

PSTRAN70

PSTRAN71

C

PSTRAN72

C	DO 40 L=1,3	PSTRAN73
	40 SUM=SUM+AO(L,I)*DFV(II+L-1)	PSTRAN74
C		PSTRAN75
C	50 DPVT(II+I-1)=SUM	PSTRAN76
C		PSTRAN77
	DO 60 I=1,12	PSTRAN78
	60 DPV(I)=DPVT(I)	PSTRAN79
C		PSTRAN80
C		PSTRAN81
C	GET THE SKEW DISPLACEMENTS	PSTRAN82
C		PSTRAN83
C		PSTRAN84
	DO 80 II=1,8,2	PSTRAN85
	DO 80 I=1,2	PSTRAN86
	SUM=0.	PSTRAN87
C		PSTRAN88
C	SKEW DISP = AU**T *CARTESIAN DISP	PSTRAN89
C		PSTRAN90
	DO 70 L=1,2	PSTRAN91
	70 SUM=SUM+AU(L,I)*DPU(II+L-1)	PSTRAN92
C		PSTRAN93
C	80 DPUT(II+I-1)=SUM	PSTRAN94
C		PSTRAN95
	DO 90 I=1,8	PSTRAN96
	90 DPU(I)=DPUT(I)	PSTRAN97
	100 CONTINUE	PSTRAN98
	EMAXT=-999.0	PSTRAN99
	INODET=0	PSTRA100
	III=1	PSTRA101
	IF (ISRCH.EQ.3HYES) III=5	PSTRA102
C		PSTRA103
C	LOOK AT ALL NODES OF ELEMENT N	PSTRA104
C		PSTRA105
	DO 290 II=1,III	PSTRA106
	IF (ISRT.EQ.3HAVE) GO TO 110	PSTRA107
C		PSTRA108
C	FIND NODE POSITION	PSTRA109
C		PSTRA110
	IX=IXP(II)	PSTRA111
	IY=IYP(II)	PSTRA112
	IF (ISRCH.NE.3HYES) READ (32) ML,IX,IY	PSTRA113
	X=IX	PSTRA114
	Y=IY	PSTRA115
C		PSTRA116
C	SET UP BB MATRIX TO FIND CURVATURES	PSTRA117
C		PSTRA118
C		PSTRA119
	A=6.*X	PSTRA120
	B=6.*Y*X	PSTRA121
	C=2.*Y	PSTRA122
	AA=ELLA*ELLA	PSTRA123
	BB(1,1)=(-A-B)/AA	PSTRA124
	BB(1,2)=0.	PSTRA125
	BB(1,3)=(A-2.*C+B)/ELLA	PSTRA126
	BB(1,4)=(-A+B)/AA	PSTRA127
	BB(1,5)=0.	PSTRA128
	BB(1,6)=(A-2.*C-B)/ELLA	PSTRA129
	BB(1,7)=(A+B)/AA	PSTRA130



	BB(1,8)=0.	PSTRA131
	BB(1,9)=(A+2.*C+B)/ELLA	PSTRA132
	BB(1,10)=(A-B)/AA	PSTRA133
	BB(1,11)=0.	PSTRA134
	BB(1,12)=(A+2.*C-B)/ELLA	PSTRA135
C		PSTRA136
	A=2.*X	PSTRA137
	B=6.*Y	PSTRA138
	C=6.*X*Y	PSTRA139
	AA=ELLB*ELLB	PSTRA140
	BB(2,1)=(B-C)/AA	PSTRA141
	BB(2,2)=(-2.*A-B+C)/ELLB	PSTRA142
	BB(2,3)=0.	PSTRA143
	BB(2,4)=(-B+C)/AA	PSTRA144
	BB(2,5)=(2.*A-B+C)/ELLB	PSTRA145
	BB(2,6)=0.	PSTRA146
	BB(2,7)=(B+C)/AA	PSTRA147
	BB(2,8)=(-2.*A-B-C)/ELLB	PSTRA148
	BB(2,9)=0.	PSTRA149
	BB(2,10)=(-B-C)/AA	PSTRA150
	BB(2,11)=(2.*A-B-C)/ELLB	PSTRA151
	BB(2,12)=0.	PSTRA152
C		PSTRA153
	A=4.*X	PSTRA154
	B=4.*Y	PSTRA155
	C=6.*X*X	PSTRA156
	D=6.*Y*Y	PSTRA157
	AA=ELLA*ELLB	PSTRA158
	BB(3,1)=(8.*C-D)/AA	PSTRA159
	BB(3,2)=(-2.*B+D)/ELLA	PSTRA160
	BB(3,3)=(-2.*A+C)/ELLB	PSTRA161
	BB(3,4)=(-8.*C+D)/AA	PSTRA162
	BB(3,5)=(-2.*B+D)/ELLA	PSTRA163
	BB(3,6)=(2.*A-C)/ELLB	PSTRA164
	BB(3,7)=(-8.*C+D)/AA	PSTRA165
	BB(3,8)=(2.*B-D)/ELLA	PSTRA166
	BB(3,9)=(-2.*A+C)/ELLB	PSTRA167
	BB(3,10)=(8.*C-D)/AA	PSTRA168
	BB(3,11)=(2.*B-D)/ELLA	PSTRA169
	BB(3,12)=(2.*A-C)/ELLB	PSTRA170
C		PSTRA171
C		PSTRA172
C	SET UP BU MATRIX TO FIND INPLANE STRAINS	PSTRA173
C		PSTRA174
	X=ELLA*X	PSTRA175
	Y=ELLB*Y	PSTRA176
C		PSTRA177
	A=ELLB+Y	PSTRA178
	B=ELLB-Y	PSTRA179
	BU(1,1)=-A	PSTRA180
	BU(1,2)=0.	PSTRA181
	BU(1,3)=-B	PSTRA182
	BU(1,4)=0.	PSTRA183
	BU(1,5)=A	PSTRA184
	BU(1,6)=0.	PSTRA185
	BU(1,7)=B	PSTRA186
	BU(1,8)=0.	PSTRA187
C		PSTRA188

	C=ELLA+X	PSTRA189
	D=ELLA-X	PSTRA190
	BU(2,1)=0.	PSTRA191
	BU(2,2)=D	PSTRA192
	BU(2,3)=0.	PSTRA193
	BU(2,4)=-D	PSTRA194
	BU(2,5)=0.	PSTRA195
	BU(2,6)=C	PSTRA196
	BU(2,7)=0.	PSTRA197
	BU(2,8)=-C	PSTRA198
C		PSTRA199
	BU(3,1)=D	PSTRA200
	BU(3,2)=-A	PSTRA201
	BU(3,3)=-D	PSTRA202
	BU(3,4)=-B	PSTRA203
	BU(3,5)=C	PSTRA204
	BU(3,6)=A	PSTRA205
	BU(3,7)=-C	PSTRA206
	BU(3,8)=B	PSTRA207
C		PSTRA208
	GO TO 130	PSTRA209
C		PSTRA210
	110 CONTINUE	PSTRA211
C		PSTRA212
C	COMPUTE STRAIN BASED ON INTEGRATED AVERAGE	PSTRA213
C		PSTRA214
C	SET UP BB MATRIX TO FIND CURVATURES	PSTRA215
C		PSTRA216
	A=2./ELLA	PSTRA217
	B=2./ELLB	PSTRA218
	C=4./(ELLA*ELLB)	PSTRA219
C		PSTRA220
	DO 120 I=1,3	PSTRA221
	DO 120 J=1,12	PSTRA222
120	BB(I,J)=0.	PSTRA223
	BB(1,3)=-A	PSTRA224
	BB(1,6)=-A	PSTRA225
	BB(1,9)=A	PSTRA226
	BB(1,12)=A	PSTRA227
	BB(2,2)=-B	PSTRA228
	BB(2,8)=-B	PSTRA229
	BB(2,5)=B	PSTRA230
	BB(2,11)=B	PSTRA231
	BB(3,1)=C	PSTRA232
	BB(3,10)=C	PSTRA233
	BB(3,4)=-C	PSTRA234
	BB(3,7)=-C	PSTRA235
C		PSTRA236
C	SET UP BU MATRIX TO FIND INPLANE STRAINS	PSTRA237
C		PSTRA238
	A=ELLA	PSTRA239
	B=ELLB	PSTRA240
	BU(1,1)=-B	PSTRA241
	BU(1,3)=-B	PSTRA242
	BU(3,2)=-B	PSTRA243
	BU(3,4)=-B	PSTRA244
	BU(1,5)=B	PSTRA245
	BU(1,7)=B	PSTRA246

BU(3,6)=B	PSTRA247
BU(3,8)=B	PSTRA248
BU(2,2)=A	PSTRA249
BU(2,6)=A	PSTRA250
BU(3,1)=A	PSTRA251
BU(3,5)=A	PSTRA252
BU(2,4)=-A	PSTRA253
BU(2,8)=-A	PSTRA254
BU(3,3)=-A	PSTRA255
BU(3,7)=-A	PSTRA256
BU(1,2)=0.	PSTRA257
BU(1,4)=0.	PSTRA258
BU(1,6)=0.	PSTRA259
BU(1,8)=0.	PSTRA260
BU(2,1)=0.	PSTRA261
BU(2,3)=0.	PSTRA262
BU(2,5)=0.	PSTRA263
BU(2,7)=0.	PSTRA264
C	PSTRA265
130 CONTINUE	PSTRA266
C	PSTRA267
DO 150 I=1,3	PSTRA268
DO 140 J=1,12	PSTRA269
140 BB(I,J)=8B(I,J)/8.	PSTRA270
C	PSTRA271
DO 150 J=1,8	PSTRA272
150 BU(I,J)=BU(I,J)/(4.*ELLA*ELLB)	PSTRA273
C	PSTRA274
C	PSTRA275
DO 170 I=1,3	PSTRA276
SUM=0.0	PSTRA277
DO 160 K=1,12	PSTRA278
160 SUM=SUM+BB(I,K)*DPV(K)	PSTRA279
170 CURV(I)=SUM	PSTRA280
C	PSTRA281
C COMPUTE THE INPLANE STRAINS	PSTRA282
C	PSTRA283
ABAR=(PZC(1)-PDT(1)/2.0+PZC(NULAY)+PDT(NULAY)/2.0)	PSTRA284
ABAR=ABAR*DT/2.0	PSTRA285
C	PSTRA286
DO 190 I=1,3	PSTRA287
SUM=0.	PSTRA288
DO 180 K=1,8	PSTRA289
180 SUM=SUM+BU(I,K)*DPU(K)	PSTRA290
190 EE(I)=SUM	PSTRA291
C	PSTRA292
IF (ABS(ABAR).LE.1.0E-03) GO TO 220	PSTRA293
DO 210 I=1,3	PSTRA294
SUM=0.	PSTRA295
XJ=0.	PSTRA296
DO 200 K=1,8	PSTRA297
J=XJ	PSTRA298
SUM=SUM+BU(I,K)*(-1.0)**(K+1)*ABAR*DPV(3*K-(K/2)*4+J)	PSTRA299
XJ=XJ+.5	PSTRA300
200 CONTINUE	PSTRA301
210 EE(I)=EE(I)+SUM	PSTRA302
220 CONTINUE	PSTRA303
C	PSTRA304

C	TRANSFORM STRAINS AND CURVATURES TO CARTESIAN	PSTRA305
C		PSTRA306
	IF (ABS(PHI).EQ.90.) GO TO 260	PSTRA307
	DO 240 I=1,3	PSTRA308
	SUM=0.	PSTRA309
	SUMC=0.	PSTRA310
	DO 230 L=1,3	PSTRA311
	SUMC=SUMC+DS(I,L)*EE(L)	PSTRA312
230	SUM=SUM+DS(I,L)*CURV(L)	PSTRA313
	EUT(I)=SUMC	PSTRA314
240	CURVT(I)=SUM	PSTRA315
	DO 250 I=1,3	PSTRA316
	EE(I)=EUT(I)	PSTRA317
250	CURV(I)=CURVT(I)	PSTRA318
260	CONTINUE	PSTRA319
C		PSTRA320
C		PSTRA321
	IF (ISRCH.NE.3HYES) GO TO 290	PSTRA322
C		PSTRA323
C	LOOK AT TOP AND BOTTOM CONCRETE LAYERS	PSTRA324
C		PSTRA325
	DO 280 JJ=1,2	PSTRA326
	ML=NULAY	PSTRA327
	IF (JJ.EQ.1) ML=1	PSTRA328
	CALL STRANO (N,ML,CURV,ECUR,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	PSTRA329
C		PSTRA330
C	GET TOTAL STRAIN	PSTRA331
C		PSTRA332
	DO 270 I=1,3	PSTRA333
270	ECUR(I)=EE(I)+ECUR(I)	PSTRA334
	ECUR(3)=ECUR(3)/2.	PSTRA335
C		PSTRA336
	CALL PRNCIP (ECUR(1),ECUR(2),ECUR(3),E1,E2,THETA)	PSTRA337
C		PSTRA338
C	GET MAX + STRAIN	PSTRA339
C		PSTRA340
	IF (E2.GT.E1) E1=E2	PSTRA341
	IF (E1.LT.EMAXT) GO TO 280	PSTRA342
	EMAXT=E1	PSTRA343
	INODET=II	PSTRA344
C		PSTRA345
C	GO TO ANOTHER LAYER	PSTRA346
C		PSTRA347
	280 CONTINUE	PSTRA348
C		PSTRA349
C	GO TO ANOTHER NODE	PSTRA350
C		PSTRA351
	290 CONTINUE	PSTRA352
C		PSTRA353
	IF (ISRCH.NE.3HYES) GO TO 300	PSTRA354
	WRITE (32) NODE(INODET),IXP(INODET),IYP(INODET)	PSTRA355
	GO TO 310	PSTRA356
C		PSTRA357
	300 CONTINUE	PSTRA358
C		PSTRA359
C	ASSIGN ELEMENT STRAINS	PSTRA360
C		PSTRA361
	CUXX(N)=CURV(1)	PSTRA362

	CUYY(N)=CURV(2)	PSTRA363
	CUXY(N)=CURV(3)	PSTRA364
C		PSTRA365
	EIXX(N)=EE(1)	PSTRA366
	EIYY(N)=EE(2)	PSTRA367
	EIXY(N)=EE(3)	PSTRA368
C		PSTRA369
C	GO TO ANOTHER ELEMENT	PSTRA370
C		PSTRA371
	310 CONTINUE	PSTRA372
	RETURN	PSTRA373
	END	PSTRA374
	SUBROUTINE PSTRSS (ICON,PF,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SIGXXO,SIPSTRSS 2 1GYYO,SIGXYO,CUXX,CUYY,CUXY,EIXX,EIYY,EIXY,PZC,PDT,DT,SZC,SXXOT,SYYPSTRSS 3 2OT,SXYOT,AS1,AF,AS,SEMOD,SIGMAP,SPROM,SPRON,NSTYPE,SPHI)	PSTRSS 4
C		PSTRSS 5
C	THIS SUBROUTINE COMPUTES THE SLAB LAYER STRESS INCREMENTS	PSTRSS 6
C	AND CALLS PDMAT TO GET THE PRESCAN FACTOR	PSTRSS 7
C	IF ICON = 2	PSTRSS 8
C		PSTRSS 9
	DIMENSION SIGXXO(NCNS), SIGYYO(NCNS), SIGXYO(NCNS), SXXOT(NCNS), SPSTRSS10 1YYOT(NCNS), SXYOT(NCNS), CUXX(NUMEL), CUYY(NUMEL), CUXY(NUMEL), EIPSTRSS11 2XX(NUMEL), EIYY(NUMEL), EIXY(NUMEL), EE(3), SS(3), DDD(3,3), CT(3)PSTRSS12	PSTRSS13
C		PSTRSS14
C	IF (ICON.NE.2) REWIND 28	PSTRSS15
	REWIND 19	PSTRSS16
	REWIND 21	PSTRSS17
	IF (ICON.NE.2) REWIND 5	PSTRSS18
C		PSTRSS19
C		PSTRSS20
	DO 70 N=1,NUMEL	PSTRSS21
	IF (ICON.NE.2) READ (28) DT,A,A,A,A,A,A	PSTRSS22
	READ (19) SIGXXO,SIGYYO,SIGXYO	PSTRSS23
	READ (21) SXXOT,SYYOT,SXYOT	PSTRSS24
	IF (ICON.EQ.2) GO TO 10	PSTRSS25
	CT(1)=CUXX(N)	PSTRSS26
	CT(2)=CUYY(N)	PSTRSS27
	CT(3)=CUXY(N)	PSTRSS28
	10 CONTINUE	PSTRSS29
	DO 50 ML=1,NCNS	PSTRSS30
	IF (ICON.EQ.2) GO TO 20	PSTRSS31
	CALL STRANO (N,ML,CT,EE,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	PSTRSS32
	EE(1)=EE(1)+EIXX(N)	PSTRSS33
	EE(2)=EE(2)+EIYY(N)	PSTRSS34
	EE(3)=EE(3)+EIXY(N)	PSTRSS35
	20 CONTINUE	PSTRSS36
	XXOT=SXXOT(ML)	PSTRSS37
	YYOT=SYYOT(ML)	PSTRSS38
	XYOT=SXYOT(ML)	PSTRSS39
	SIGXX=SIGXXO(ML)	PSTRSS40
	SIGYY=SIGYYO(ML)	PSTRSS41
	SIGXY=SIGXYO(ML)	PSTRSS42
C		PSTRSS43
C	FIND ELASTICITY RELATIONSHIP / PRESCAN FACTOR	PSTRSS44
C		PSTRSS45
	CALL PDMAT (N,ML,DDD,ICON,PF,NUMEL,NCNS,NSMAT,NULAY,NSLAYR,XXOT,YYYPSTRSS46 1OT,XYOT,SIGXX,SIGYY,SIGXY,AS1,AF,AS,SEMOD,SIGMAP,SPROM,SPRON,NSTYPSTRSS47	PSTRSS47

	2E,SPHI)	PSTRSS48
	IF (ICON.EQ.2) GO TO 50	PSTRSS49
C		PSTRSS50
C	FIND STRESS INCREMENT	PSTRSS51
C		PSTRSS52
	DO 40 I=1,3	PSTRSS53
	SUM=0.	PSTRSS54
	DO 30 K=1,3	PSTRSS55
	30 SUM=SUM+DDD(I,K)*EE(K)	PSTRSS56
	40 SS(I)=SUM	PSTRSS57
	SIGXXO(ML)=SS(1)	PSTRSS58
	SIGYYO(ML)=SS(2)	PSTRSS59
	SIGXYO(ML)=SS(3)	PSTRSS60
C		PSTRSS61
	50 CONTINUE	PSTRSS62
	IF (ICON.EQ.2) GO TO 60	PSTRSS63
	WRITE (5) SIGXXO,SIGYYO,SIGXYO	PSTRSS64
	60 CONTINUE	PSTRSS65
	70 CONTINUE	PSTRSS66
	IF (ICON.EQ.2) RETURN	PSTRSS67
	REWIND 5	PSTRSS68
	REWIND 19	PSTRSS69
	CALL FTNCOPY (4LJUM5,3LJ19,0)	PSTRSS70
	RETURN	PSTRSS71
	END	PSTRSS72
	SUBROUTINE PDMAT (N,ML,DDD,ICON,PF,NUMEL,NCNS,NSMAT,NULAY,NSLAYR,SPOMAT	2
	1XXOT,SYYOT,SXYOT,SIGXXO,SIGYYO,SIGXYO,AS1,AF,AS,SEM0D,SIGMAP,SPROMPDMAT	3
	2,SPRON,NSTYPE,SPHI)	PDMAT 4
C		PDMAT 5
C	THIS SUBROUTINE FINDS THE CONSTITUTIVE STRESS-STRAIN RELATION	PDMAT 6
C	OR PRESCAN FACTOR FOR THE SLAB LAYERS	PDMAT 7
C		PDMAT 8
	DIMENSION AS1(NUMEL,NCNS), AF(NUMEL,NULAY), AS(NUMEL,NULAY), NSTYPPDMAT	9
	1E(NSLAYR), SPHI(NSLAYR), SEMOD(NSMAT), SIGMAP(NSMAT), SPRON(NSMAT)	PDMAT 10
	2, SPROM(NSMAT), DD(3,3), DDD(3,3)	PDMAT 11
	COMMON /PROPC/ V,EC,FC,ECOMP,FT,ET,PCOMP,ALMM,SALMM,EALMM,ALICC	PDMAT 12
	COMMON /TFAIL/ TOLF	PDMAT 13
	COMMON /COMPLT/ ICPLT	PDMAT 14
	DO 10 I=1,3	PDMAT 15
	DO 10 J=1,3	PDMAT 16
	DDD(I,J)=0.	PDMAT 17
	10 DD(I,J)=0.0	PDMAT 18
	ALPH1 = 0.	PDMAT 19
	ALPH2 = 0.	PDMAT 20
	EC1=0.	PDMAT 21
	EC2=0.	PDMAT 22
C		PDMAT 23
C	CURRENT TOTAL STRESS LEVEL	PDMAT 24
C		PDMAT 25
	SX=SXXOT+SIGXXO	PDMAT 26
	SY=SYYOT+SIGYYO	PDMAT 27
	SXY=SXYOT+SIGXYO	PDMAT 28
	IF (ML.GT.NULAY) GO TO 120	PDMAT 29
C		PDMAT 30
C	CHECK FOR LAYER FAILURE	PDMAT 31
	IF (AS1(N,ML).NE.999.0) GO TO 20	PDMAT 32
	GO TO 70	PDMAT 33
	20 CONTINUE	PDMAT 34

C		PDMAT 35
C	A LAYER HAS CRACKED OR CRUSHED	PDMAT 36
C		PDMAT 37
C	CHECK IF THERE ARE TWO CRACKED - CRUSHED LINES	PDMAT 38
C		PDMAT 39
	IF (AS(N,ML).NE.999.0) GO TO 170	PDMAT 40
C		PDMAT 41
C	FIND TRANSFORMATION ANGLE	PDMAT 42
C		PDMAT 43
	ANG=AS1(N,ML)	PDMAT 44
	ANGF=AF(N,ML)	PDMAT 45
	THETAS=ANG	PDMAT 46
C		PDMAT 47
C	TRANSFORM STRESSES AND STRAINS TO CRACKED AXIS	PDMAT 48
C		PDMAT 49
	CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	PDMAT 50
	CALL TRANG (SXXOT,SYYOT,SXYOT,ANG,SOT1,SOT2,SOT3)	PDMAT 51
C		PDMAT 52
C	CHANGE SIGN	PDMAT 53
C	CONSIDER 2 DIRECTION AS DIRECTION OF INTEREST	PDMAT 54
C		PDMAT 55
	S1=-S1	PDMAT 56
	S2=-S2	PDMAT 57
	SOT1=-SOT1	PDMAT 58
	SOT2=-SOT2	PDMAT 59
C		PDMAT 60
	IF (ANG.EQ.ANGF) GO TO 30	PDMAT 61
C		PDMAT 62
	CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	PDMAT 63
	SOT=SOT2	PDMAT 64
	SP=SP2	PDMAT 65
	SCT=S2	PDMAT 66
	IF (ABS(S2).GE.ABS(SP)) EC2=0.0	PDMAT 67
	DIRECT=DIRT2	PDMAT 68
	GO TO 40	PDMAT 69
C		PDMAT 70
	30 CONTINUE	PDMAT 71
C		PDMAT 72
C	CONSIDER 1 DIRECTION AS DIRECTION OF INTEREST	PDMAT 73
C		PDMAT 74
	CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	PDMAT 75
	SOT=SOT1	PDMAT 76
	SP=SP1	PDMAT 77
	SCT=S1	PDMAT 78
	IF (ABS(S1).GE.ABS(SP)) EC1=0.0	PDMAT 79
	DIRECT=DIRT1	PDMAT 80
C		PDMAT 81
	40 CONTINUE	PDMAT 82
C		PDMAT 83
C	CHECK EFFECTIVE LAYER FOR FAILURE	PDMAT 84
C		PDMAT 85
	IF (DIRECT.EQ.5HNONE ) GO TO 130	PDMAT 86
C		PDMAT 87
C	CHECK EFFECTIVE DIRECTION FOR TOLERANCE	PDMAT 88
C		PDMAT 89
	IF (ABS(SCT).GE.ABS(SP)) GO TO 50	PDMAT 90
	GO TO 130	PDMAT 91
C		PDMAT 92

50 CONTINUE	PDMAT 93
C	PDMAT 94
C FIND PRESCAN FACTOR	PDMAT 95
G	PDMAT 96
IF (ABS(SCT-SOT).LT.1.0E-(6) GO TO 60	PDMAT 97
ELOW=SP-SP*TOLF/2.0	PDMAT 98
TEMP=ABS((ELOW-SOT)/(SCT-SOT))	PDMAT 99
IF (TEMP.LT.PF) PF=TEMP	PDMAT100
GO TO 130	PDMAT101
60 CONTINUE	PDMAT102
PF=0.0	PDMAT103
GO TO 130	PDMAT104
C	PDMAT105
70 CONTINUE	PDMAT106
C	PDMAT107
C NO CRACKS IN ELEMENT	PDMAT108
C	PDMAT109
C	PDMAT110
C FIND PRINCIPAL DIRECTIONS AND TRANSFORM ALL STRESS-STATE TO	PDMAT111
C CURRENT PRINCIPAL AXIS	PDMAT112
CALL PRNCIP (SX,SY,SXY,S1,S2,THETAS)	PDMAT113
CALL TRANG (SXXOT,SYYOT,SXYOT,THETAS,SOT1,SOT2,SOT3)	PDMAT114
SOT2=-SOT2	PDMAT115
SOT1=-SOT1	PDMAT116
S1=-S1	PDMAT117
S2=-S2	PDMAT118
C	PDMAT119
C CONSIDER 1 DIRECTION AS DIRECTION OF INTREST	PDMAT120
C	PDMAT121
CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	PDMAT122
IF (ABS(S1).GE.ABS(SP1)) EC1=0.0	PDMAT123
IF (DIRT1.EQ.5HCRUSH.AND.EC1.EQ.0.0.AND.ICMPLT.EQ.3HYES) EC2=0.0	PDMAT124
C	PDMAT125
C CONSIDER 2 DIRECTION AS DIRECTION OF INTREST	PDMAT126
C	PDMAT127
CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	PDMAT128
IF (ABS(S2).GT.ABS(SP2)) EC2=0.0	PDMAT129
IF (DIRT2.EQ.5HCRUSH.AND.EC2.EQ.0.0.AND.ICMPLT.EQ.3HYES) EC1=0.0	PDMAT130
C	PDMAT131
C CHECK FAILURE STRESS DIRECTION	PDMAT132
C	PDMAT133
IF (DIRT1.EQ.5HNONE ) GO TO 80	PDMAT134
C	PDMAT135
C S1 WILL CAUSE CRACK	PDMAT136
C S2 IS EFFECTIVE	PDMAT137
C	PDMAT138
SOT=SOT1	PDMAT139
SP=SP1	PDMAT140
SCT=S1	PDMAT141
GO TO 90	PDMAT142
80 CONTINUE	PDMAT143
C	PDMAT144
C S2 WILL CAUSE CRACK	PDMAT145
C S1 IS EFFECTIVE	PDMAT146
C	PDMAT147
SOT=SOT2	PDMAT148
SP=SP2	PDMAT149
SCT=S2	PDMAT150



90	CONTINUE	PDMAT151
	IF (ABS(SCT).GE.ABS(SP)) GO TO 100	PDMAT152
	GO TO 130	PDMAT153
C		PDMAT154
C	FIND PRESCAN FACTOR	PDMAT155
C		PDMAT156
100	CONTINUE	PDMAT157
	IF (ABS(SCT-SOT).LT.1.0E-06) GO TO 110	PDMAT158
	ELOW=SP-SP*TOLF/2.0	PDMAT159
	TEMP=ABS((ELOW-SOT)/(SCT-SOT))	PDMAT160
	IF (TEMP.LT.PF) PF=TEMP	PDMAT161
	GO TO 130	PDMAT162
110	CONTINUE	PDMAT163
	PF=0.0	PDMAT164
	GO TO 130	PDMAT165
120	CONTINUE	PDMAT166
C		PDMAT167
C	STEEL LAYERS	PDMAT168
C		PDMAT169
	ANG=SPHI(ML-NULAY)	PDMAT170
	M=NSTYPE(ML-NULAY)	PDMAT171
	THETAS=ANG	PDMAT172
	RN=SPRON(M)	PDMAT173
	RM=SPROM(M)	PDMAT174
	ES=SEMOD(M)	PDMAT175
	SIGMAY=SIGMAP(M)	PDMAT176
	CALL TRANG (SX,SY,S)Y,ANG,S1,S2,S3)	PDMAT177
	EC1=ES/(1.0+RN*(1.0-RM)/RM*((ABS(S1)/SIGMAY)**(RN-1.0)))	PDMAT178
130	CONTINUE	PDMAT179
C		PDMAT180
C	FIND V1 AND V2	PDMAT181
C	FIND ELASTICITY RELATION	PDMAT182
C		PDMAT183
	VA=V	PDMAT184
	VB=V	PDMAT185
	IF (EC1.EQ.0..AND.EC2.EQ.0.0) GO TO 170	PDMAT186
	IF (EC1.EQ.0..OR.EC2.EQ.0.0) GO TO 140	PDMAT187
	V1=V	PDMAT188
	V2=(EC1*(1.0-V1*ALPH1)/(EC2*V1))+ALPH2	PDMAT189
	V2=1.0/V2	PDMAT190
	IF (V1.GE.0.0.AND.V2.GE.0.0) VB=V2	PDMAT191
	IF (V1.GE.0.0.AND.V2.GE.0.0) VA=V1	PDMAT192
	IF (V2.LE.V1.AND.V2.GE.0.0) GO TO 150	PDMAT193
	V2=V	PDMAT194
	V1=(EC2*(1.0-V2*ALPH2)/(EC1*V2))+ALPH1	PDMAT195
	V1=1.0/V1	PDMAT196
	IF (V1.GE.0.0.AND.V2.GE.0.0) VB=V2	PDMAT197
	IF (V1.GE.0.0.AND.V2.GE.0.0) VA=V1	PDMAT198
	IF (V1.LE.V2.AND.V1.GE.0.0) GO TO 150	PDMAT199
	V1=VA	PDMAT200
	V2=VB	PDMAT201
	GO TO 150	PDMAT202
140	CONTINUE	PDMAT203
	V1=0.	PDMAT204
	V2=0.	PDMAT205
	EC1=EC1*(1.0-V*ALPH1)	PDMAT206
	EC2=EC2*(1.0-V*ALPH2)	PDMAT207
	GO TO 160	PDMAT208

150	CONTINUE	PDMAT209
	EC1=EC1*(1.0-V1*ALPH1)	PDMAT210
	EC2=EG2*(1.0-V2*ALPH2)	PDMAT211
160	CONTINUE	PDMAT212
C		PDMAT213
C	COMPUTE COUPLED DD MATRIX	PDMAT214
C		PDMAT215
	A=1.0-V1*V2	PDMAT216
	DD(1,1)=EC1/A	PDMAT217
	DD(1,2)=EC1*V2/A	PDMAT218
	DD(2,1)=DD(1,2)	PDMAT219
	DD(2,2)=EC2/A	PDMAT220
	DD(3,3)=EC1*EC2/(EC1+EC2+2.0*EG2*V1)	PDMAT221
C		PDMAT222
C	TRANSFORM TO X-Y COORDINATES	PDMAT223
C		PDMAT224
	CALL TRANS (DD,THETAS,DDD)	PDMAT225
170	CONTINUE	PDMAT226
	RETURN	PDMAT227
	END	PDMAT228
	SUBROUTINE STRANO (N,ML,GT,EE,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	STRANO 2
C		STRANO 3
C	THIS SUBROUTINE COMPUTES THE OUT-OF-PLANE STRAIN INCREMENTS	STRANO 4
C	IN PLATE LAYERS.	STRANO 5
C		STRANO 6
	DIMENSION PDT(NULAY), PZC(NULAY), SZC(NSLAYR), EE(3), CT(3)	STRANO 7
	ABAR=(PZC(1)-PDT(1)/2.0+PZC(NULAY)+PDT(NULAY)/2.0)	STRANO 8
	ABAR=ABAR*DT/2.0	STRANO 9
	IF (ML.GT.NULAY) GO TO 10	STRANO10
	DTSTAR=PZC(ML)*DT-ABAR	STRANO11
	GO TO 20	STRANO12
10	CONTINUE	STRANO13
	DTSTAR=SZC(ML-NULAY)-ABAR	STRANO14
20	CONTINUE	STRANO15
	EE(1)=DTSTAR*CT(1)	STRANO16
	EE(2)=DTSTAR*CT(2)	STRANO17
	EE(3)=DTSTAR*CT(3)	STRANO18
	RETURN	STRANO19
	END	STRANO20
	SUBROUTINE BCHECK (KC,NIX,NBEAMX,NULAYB,KMAT,NA1,IO,CUB,EIB,DUM,NPBCHECK	2
	1IX,NPKX,SIBXXC,SIBXXA,ITYPE,FT,SIGO,ESX,EDOWNT,EDOWN,DISP,DISPLA,ZBCHECK	3
	2CRD,XLNGTH,ASXLR,EPSPS,STRAN,FCADD,IOUT)	BCHECK 4
C		BCHECK 5
C	THIS SUBROUTINE ASSIGNS LAYER FAILURES AND COMPUTES	BCHECK 6
C	THE FICTITIOUS FORCES	BCHECK 7
C		BCHECK 8
	DIMENSION CUB(NBEAMX), EIB(NBEAMX), DUM(NULAYB), SIBXXA(NULAYB), SBCHECK	9
	1IBXXC(NULAYB), ITYPE(NULAYB), ZCRD(NULAYB), ASXLR(NULAYB), EPSPS(NBCHECK	10
	2ULAYB), ESX(NULAYB,NBEAMX), SIGO(KMAT), FT(KMAT), EDOWN(KMAT), STRBCHECK	11
	3AN(KMAT), EDOWNT(KC), FCADD(NA1), DISP(NA1), DISPLA(NA1), DUMMY(1)BCHECK	12
	COMMON /PFACT/ PFC	BCHECK13
	COMMON /CB309/ FT(L,CTOL	BCHECK14
	COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	BCHECK15
C		BCHECK16
C		BCHECK17
	IF (PFC.NE.0.0) REWIND 5	BCHECK18
	REWIND 15	BCHECK19
	REWIND 23	BCHECK20

REWIND 27  
REWIND 25

BCHECK21  
BCHECK22  
BCHECK23

C

WRITE (IO,10)  
10 FORMAT (/,1H0,10X,\*BEAM - NEWLY CRACKED, CRUSHED OR YIELDED LAYERS  
1\*)

BCHECK24  
BCHECK25  
BCHECK26

WRITE (IO,20)  
20. FORMAT (1H0,5X,\*ELEMENT LAYER\*)

BCHECK27  
BCHECK28  
BCHECK29

C

DO 120 J=1,NBEAMX  
READ (23) SIBXXC  
READ (15) SIBXXA  
READ (27) XLNGTH,GKEI,NPIX,NPKX,ITYPE,ASXLR,DUM,ZCRD,DUM  
READ (25) EPSXS  
II=5\*NPIX

BCHECK30  
BCHECK31  
BCHECK32  
BCHECK33  
BCHECK34  
BCHECK35

KK=5\*NPKX  
DZCR = 0.  
DASX=0.

BCHECK36  
BCHECK37  
BCHECK38

ICALL=3HNO

BCHECK39

C

DO 100 I=1,NULAYB

BCHECK40  
BCHECK41

C

SIBC=SIBXXC(I)  
SIBA=SIBXXA(I)

BCHECK42  
BCHECK43  
BCHECK44

K=ITYPE(I)  
ASX=ASXLR(I)  
ZCR=ZCRD(I)

BCHECK45  
BCHECK46  
BCHECK47

ES=ESX(I,J)  
EPSXBT=EIB(J)+ZCR\*CUB(J)  
SIG=SIBA+SIBC

BCHECK48  
BCHECK49  
BCHECK50

IF (ES.EQ.0.0.AND.ABS(SIG).LE.1.E-06) GO TO 100

BCHECK51

C

CHECK FOR LAYER FAILURE

BCHECK52

C

SET UP HISTOGRAM VALUES

BCHECK53

C

IF (K.GT.KC) GO TO 60

BCHECK54

C

IF (ES.EQ.0.0) GO TO 40

BCHECK55

C

FTM=FT(K)\*(1.-FTOL)

BCHECK56

C

SIGM=FTM

BCHECK57

IF (SIG.LT.0.) SIGM=-(SIGO(K)\*(1.0-OTOL))

BCHECK58

IF (ABS(SIG).LT.ABS(SIGM)) GO TO 100

BCHECK59

C

IF (FVAL(4).EQ.-999.0.AND.SIG.GE.0.) FVAL(4)=999.0

BCHECK60

IF (FVAL(5).EQ.-999.0.AND.SIG.LT.0.0) FVAL(5)=999.0

BCHECK61

C

IF (FVAL(4).EQ.-999.0.AND.SIG.GE.0.) FVAL(4)=999.0

BCHECK62

IF (FVAL(5).EQ.-999.0.AND.SIG.LT.0.0) FVAL(5)=999.0

BCHECK63

C

IF (ES.NE.0.0) WRITE (IO,30) J,I

BCHECK64

30 FORMAT (1H ,9X,I3,4X,I3)

BCHECK65

C

ESX(I,J)=0.

BCHECK66

ES=0.0

BCHECK67

C

GO TO 100

BCHECK68

40 CONTINUE

BCHECK69

IF (PFC.EQ.0.0) GO TO 100

BCHECK70

IF (SIG.LT.0.) GO TO 50

BCHECK71

C

IF (EPSXBT.LT.0.0) GO TO 100

BCHECK72

SIGF=EDOWNT(K)\*EPSXBT

BCHECK73

C

IF (EPSXBT.LT.0.0) GO TO 100

BCHECK74

SIGF=EDOWNT(K)\*EPSXBT

BCHECK75

SIGF=EDOWNT(K)\*EPSXBT

BCHECK76

SIGF=EDOWNT(K)\*EPSXBT

BCHECK77

SIGF=EDOWNT(K)\*EPSXBT

BCHECK78

	GO TO 70	BCHECK79
C		BCHECK80
C		BCHECK81
	50 CONTINUE	BCHECK82
	IF (EPSXBT.GT.0.0) GO TO 100	BCHECK83
	60 CONTINUE	BCHECK84
C		BCHECK85
C	FIND STRAINS FOR UNLOADING	BCHECK86
C		BCHECK87
	UI=DISPLA(II-4)+DISP(II-4)	BCHECK88
	UK=DISPLA(KK-4)+DISP(KK-4)	BCHECK89
	THETA I=DISPLA(II)+DISP(II)	BCHECK90
	THETA K=DISPLA(KK)+DISP(KK)	BCHECK91
	EPS=(UK-UI+(THETA K-THETA I)*ZCR)/XLNGTH	BCHECK92
	EPS=EPS+EPSPS(I)	BCHECK93
	IF (K.GT.KC) GO TO 90	BCHECK94
	IF (EPS.GE.STRAN(K)) GO TO 100	BCHECK95
C		BCHECK96
C	FIND STRESS TO UNLOAD	BCHECK97
C		BCHECK98
	EPS=EPS-STRAN(K)	BCHECK99
	IF (EPS.LT.EPSXBT) SIGF=EDOWN(K)*EPSXBT	BCHECK100
	IF (EPS.GT.EPSXBT) SIGF=EDOWN(K)*EPS	BCHECK101
C		BCHECK102
	70 CONTINUE	BCHECK103
	IF (ABS(SIGF).GE.ABS(SIG)) SIGF=SIG*(1.-1.E-10)	BCHECK104
	IF (PFC.EQ.0.0) GO TO 80	BCHECK105
	SIBXXA(I)=SIBA-SIGF	BCHECK106
	80 CONTINUE	BCHECK107
C		BCHECK108
C	COMPUTE FICTITIOUS FORCES	BCHECK109
C		BCHECK110
	ICALL=3HYES	BCHECK111
	DASX=SIGF*ASX+DASX	BCHECK112
	DZCR=SIGF*ASX*ZCR+DZCR	BCHECK113
C		BCHECK114
	GO TO 100	BCHECK115
C		BCHECK116
C	SET UP HISTOGRAM	BCHECK117
C		BCHECK118
	90 CONTINUE	BCHECK119
	IF (ABS(SIG).GT.SIGO(K).AND.FVAL(6).EQ.-999.0) FVAL(6)=999.0	BCHECK120
C		BCHECK121
	100 CONTINUE	BCHECK122
C		BCHECK123
	IF (ICALL.NE.3HYES) GO TO 110	BCHECK124
	IF (PFC.EQ.0.0) GO TO 110	BCHECK125
C		BCHECK126
C	PLACE FICTITIOUS FORCES INTO THE FORCE VECTOR	BCHECK127
C		BCHECK128
	FCADD(II-4)=FCADD(II-4)-DASX	BCHECK129
	FCADD(KK-4)=FCADD(KK-4)+DASX	BCHECK130
	FCADD(II)=FCADD(II)-DZCR	BCHECK131
	FCADD(KK)=FCADD(KK)+DZCR	BCHECK132
	110 CONTINUE	BCHECK133
C		BCHECK134
	IF (PFC.EQ.0.0) GO TO 120	BCHECK135
	WRITE (5) SIBXXA	BCHECK136

120	CONTINUE	BCHEC137
	IF (PFC.EQ.0.0) RETURN	BCHEC138
	REWIND 5	BCHEC139
	REWIND 15	BCHEC140
	CALL FTNCOPY (4LJUM5,3LJ15,0)	BCHEC141
	RETURN	BCHEC142
	END	BCHEC143
	SUBROUTINE PCHECK (IO,NA1,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,NANA,PZC,PPCHECK 2	PCHECK 2
	1DT,SZC,SOT,DT,ELLA,ELLB,NPI,NPJ,NPK,NPL,FCADD,CTXX,CTYY,CTXY,CUXX,PCHECK 3	PCHECK 3
	2CUIY,CUIY,ETXX,ETYY,ETXY,EIXX,EIYY,EIXY,SXXOT,SYOT,SXYOT,SIGXXO,SPCHECK 4	PCHECK 4
	3IGIYO,SIGXYO,AS1,AF,AS,AFT,AST,NSTYPE,SPHI,SIGMAP)	PCHECK 5
C		PCHECK 6
C	THIS SUBROUTINE SETS THE ANGLE CODES FOR CRACKING AND CRUSHING	PCHECK 7
C	AND COMPUTES THE FICTIOUS FORCE VECTOR	PCHECK 8
C		PCHECK 9
	DIMENSION FCADD(NA1), CTXX(NUMEL), CTYY(NUMEL), CTXY(NUMEL), CUXX(PCHECK10	PCHECK10
	1NUMEL), CUIY(NUMEL), CUIY(NUMEL), SXXOT(NCNS), SYOT(NCNS), SXYOT(PCHECK11	PCHECK11
	2NCNS), SIGXXO(NCNS), SIGIYO(NCNS), SIGXYO(NCNS), AS1(NUMEL,NCNS), PCHECK12	PCHECK12
	3AF(NUMEL,NULAY), AS(NUMEL,NULAY), ETXX(NUMEL), ETTY(NUMEL), ETXY(INPCHECK13	PCHECK13
	4UMEL), EIXX(NUMEL), EIYY(NUMEL), EIXY(NUMEL), SIGMAP(NSMAT), SPHI(PCHECK14	PCHECK14
	5NSLAYR), NSTYPE(NSLAYR), AFT(NCNS), AST(NCNS), CT(3), EE(3), CTOT(PCHECK15	PCHECK15
	63), ETOT(3), DD(3,3), DDD(3,3)	PCHECK16
	COMMON /EDOWN/ EDOWNG,EDOWNT	PCHECK17
	COMMON /PROPC/ V,EC,FC,ECOMP,FT,ET,PCOMP,ALMM,SALMM,EALMM,ALICC	PCHECK18
	COMMON /TFAIL/ TOLF	PCHECK19
	COMMON /PFACT/ PFC	PCHECK20
	COMMON /COMPLT/ ICOMPLT	PCHECK21
C		PCHECK22
	IF (PFC.NE.0.0) REWIND 5	PCHECK23
	REWIND 4	PCHECK24
	REWIND 32	PCHECK25
	REWIND 19	PCHECK26
	REWIND 21	PCHECK27
	REWIND 28	PCHECK28
	REWIND 31	PCHECK29
C		PCHECK30
	IF (PFC.EQ.0.0) GO TO 20	PCHECK31
	DO 10 I=1,NA1	PCHECK32
	10 FCADD(I)=0.0	PCHECK33
	20 CONTINUE	PCHECK34
	WRITE (IO,30)	PCHECK35
	30 FORMAT (/,1H0,10X,*SLAB - NEWLY CRACKED, CRUSHED OR YIELDED LAYERS PCHECK36	PCHECK36
	1*)	PCHECK37
	WRITE (IO,40)	PCHECK38
	40 FORMAT (1H0,5X,*ELEMENT LAYER ANGLE*)	PCHECK39
C		PCHECK40
	DO 190 N=1,NUMEL	PCHECK41
	READ (32) AFT,AST	PCHECK42
	READ (28) DT,ELLA,ELLB,NPI,NPJ,NPK,NPL	PCHECK43
	READ (19) SIGXXO,SIGIYO,SIGXYO	PCHECK44
	READ (21) SXXOT,SYOT,SXYOT	PCHECK45
	CTOT(1)=CTXX(N)	PCHECK46
	CTOT(2)=CTYY(N)	PCHECK47
	CTOT(3)=CTXY(N)	PCHECK48
	CT(1)=CUXX(N)	PCHECK49
	CT(2)=CUIY(N)	PCHECK50
	CT(3)=CUIY(N)	PCHECK51
	ICALL=3HNO	PCHECK52

	ISTART=3HYES	PCHECK53
	IEND=3HNO	PCHECK54
	DO 180 ML=1,NCNS	PCHECK55
C		PCHECK56
C	CURRENT TOTAL STRESS LEVEL	PCHECK57
C		PCHECK58
	SX=SXXOT(ML)+SIGXXO(ML)	PCHECK59
	SY=SYYOT(ML)+SIGYYO(ML)	PCHECK60
	SXY=SXYOT(ML)+SIGXYO(ML)	PCHECK61
C		PCHECK62
C	CHECK IF A LAYER HAS CRACKED OR CRUSHED	PCHECK63
C		PCHECK64
	IF (ML.GT.NULAY) GO TO 170	PCHECK65
	IF (AS1(N,ML).NE.999.0) GO TO 50	PCHECK66
	GO TO 130	PCHECK67
	50 CONTINUE	PCHECK68
C		PCHECK69
C	CRACKED- CRUSHED PROCEEDURE	PCHECK70
C		PCHECK71
	ANG=AS1(N,ML)	PCHECK72
	ANGF=AF(N,ML)	PCHECK73
	ANGDS=-ANG	PCHECK74
	CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	PCHECK75
	S1=-S1	PCHECK76
	S2=-S2	PCHECK77
C		PCHECK78
C	CURRENT TOTAL STRAIN LEVEL	PCHECK79
C		PCHECK80
	CALL STRANO (N,ML,CTOT,ETOT,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	PCHECK81
	CALL STRANO (N,ML,CT,EE,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	PCHECK82
	EX=ETXX(N)+EIXX(N)+EE(1)+ETOT(1)	PCHECK83
	EY=ETYY(N)+EIYY(N)+ETOT(2)+EE(2)	PCHECK84
	EXY=ETXY(N)+EIXY(N)+EE(3)+ETOT(3)	PCHECK85
	EOTXX=ETXX(N)+ETOT(1)	PCHECK86
	EOTYY=ETYY(N)+ETOT(2)	PCHECK87
	EOTXY=ETXY(N)+ETOT(3)	PCHECK88
	EOTXY=0.5*EOTXY	PCHECK89
	EXY=0.5*EXY	PCHECK90
	SD1=0.	PCHECK91
	SD2=0.	PCHECK92
	SD3=0.	PCHECK93
C		PCHECK94
C	TRANSFORM STRESSES AND STRAINS TO CRACKED AXIS	PCHECK95
C		PCHECK96
	CALL TRANG (EX,EY,EXY,ANG,E1,E2,E3)	PCHECK97
	CALL TRANG (EOTXX,EOTYY,EOTXY,ANG,EOT1,EOT2,EOT3)	PCHECK98
C		PCHECK99
C	CHANGE SIGN DUE TO DIFFERENT CONVENTIONS USED	PCHECK100
C		PCHECK101
	E1=-E1	PCHECK102
	E2=-E2	PCHECK103
	EOT1=-EOT1	PCHECK104
	EOT2=-EOT2	PCHECK105
C		PCHECK106
C	GET STRESS STRAIN RELATIONSHIP	PCHECK107
C	GET LIMITS	PCHECK108
C		PCHECK109
C		PCHECK110

C	CHECK IF SECOND CRACK HAS DEVELOPED	PCHEC111
G		PCHEC112
	IF (AS(N,ML).NE.999.0) GO TO 100	PCHEC113
C		PCHEC114
C	CHECK DIRECTION OF CRACK	PCHEC115
G		PCHEC116
	IF (ANG.EQ.ANGF) GO TO 80	PCHEC117
C		PCHEC118
C	S2 IS STILL EFFECTIVE	PCHEC119
C	CRACK IS IN THE DIRECTION OF S2	PCHEC120
C	CHECK IF EFFECTIVE DIRECTION IS CRACKED	PCHEC121
C		PCHEC122
	CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	PCHEC123
	IF (DIRT2.EQ.5HNONE ) GO TO 70	PCHEC124
	ELOW=SP2-SP2*TOLF	PCHEC125
	IF (ABS(S2).LT.ABS(ELOW)) GO TO 70	PCHEC126
	AST(ML)=DIRT2	PCHEC127
	AS(N,ML)=ANGF+90.	PCHEC128
	WRITE (IO,60) N,ML,AS(N,ML)	PCHEC129
60	FORMAT (1H0,9X,I3,4X,I3,2X,F9.4)	PCHEC130
70	CONTINUE	PCHEC131
	IF (PFC.EQ.J.0) GO TO 180	PCHEC132
C		PCHEC133
C	UNLOAD IN S1 DIRECTION	PCHEC134
C		PCHEC135
	IF (ABS(S1).LE.1.0E-03) GO TO 120	PCHEC136
	IF (ABS(E1).LT.ABS(EOT1)) GO TO 120	PCHEC137
C		PCHEC138
C	UNLOAD	PCHEC139
C	SET DOWNWARD MODULUS FOR DETERMINING FICTITIOUS FORCES	PCHEC140
C		PCHEC141
	ISE=0	PCHEC142
	IS=0	PCHEC143
	DD(1,1)=+EDOWNC	PCHEC144
	IF (S1.LT.0.0) DD(1,1)=-EDOWNT	PCHEC145
	DE=E1-EOT1	PCHEC146
	IF (DE.GT.0.0) DD(1,1)=-DD(1,1)	PCHEC147
	DS=(E1-EOT1)*DD(1,1)	PCHEC148
	IF (S1.GE.0.0) IS=1	PCHEC149
	IF ((DS+S1).GE.0.0) ISE=1	PCHEC150
	IF (IS.NE.ISE) DS=-S1	PCHEC151
	SD1=DS	PCHEC152
	GO TO 120	PCHEC153
C		PCHEC154
80	CONTINUE	PCHEC155
C		PCHEC156
C	S1 IS STILL EFFECTIVE	PCHEC157
C	CRACK IS IN THE DIRECTION OF S1	PCHEC158
C	CHECK IF EFFECTIVE DIRECTION IS CRACKED	PCHEC159
C		PCHEC160
	CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	PCHEC161
	IF (DIRT1.EQ.5HNONE ) GO TO 90	PCHEC162
	ELOW=SP1-TOLF*SP1	PCHEC163
	IF (ABS(S1).LT.ABS(ELOW)) GO TO 90	PCHEC164
	AST(ML)=DIRT1	PCHEC165
	AS(N,ML)=ANGF+90.	PCHEC166
	WRITE (IO,60) N,ML,AS(N,ML)	PCHEC167
90	CONTINUE	PCHEC168

	IF (PFC.EQ.0.0) GO TO 180	PCHEC169
C		PGHEC170
C	UNLOAD IN S2 DIRECTION	PCHEC171
C		PCHEC172
	IF (ABS(S2).LE.1.0E-03) GO TO 120	PCHEC173
	IF (ABS(E2).LT.ABS(EOT2)) GO TO 120	PGHEC174
C		PCHEC175
C	UNLOAD	PCHEC176
C	SET DOWNWARD MODULUS FOR DETERMINING FICTITIOUS FORCES	PCHEC177
C		PGHEC178
	ISE=0	PCHEC179
	IS=0	PGHEC180
	DD(2,2)=+EDOWNC	PCHEC181
	IF (S2.LT.0.0) DD(2,2)=-EDOWNT	PCHEC182
	DE=E2-EOT2	PCHEC183
	IF (DE.GT.0.0) DD(2,2)=-DD(2,2)	PCHEC184
	DS=(E2-EOT2)*DD(2,2)	PCHEC185
	IF (S2.GE.0.0) IS=1	PCHEC186
	IF ((DS+S2).GE.0.0) ISE=1	PCHEC187
	IF (IS.NE.ISE) DS=-S2	PCHEC188
	SD2=DS	PCHEC189
	GO TO 120	PCHEC190
C		PCHEC191
	100 CONTINUE	PCHEC192
	IF (PFC.EQ.0.0) GO TO 180	PCHEC193
C		PCHEC194
C	BOTH PRINCIPAL DIRECTIONS ARE CRACKED	PCHEC195
C		PCHEC196
	IF (ABS(S1).LE.1.0E-03) GO TO 110	PCHEC197
	IF (ABS(E1).LT.ABS(EOT1)) GO TO 110	PCHEC198
	ISE=0	PCHEC199
	IS=0	PCHEC200
	DD(1,1)=+EDOWNC	PCHEC201
	IF (S1.LT.0.0) DD(1,1)=-EDOWNT	PCHEC202
	DE=E1-EOT1	PCHEC203
	IF (DE.GT.0.0) DD(1,1)=-DD(1,1)	PCHEC204
	DS=(E1-EOT1)*DD(1,1)	PCHEC205
	IF (S1.GE.0.0) IS=1	PCHEC206
	IF ((DS+S1).GE.0.0) ISE=1	PCHEC207
	IF (IS.NE.ISE) DS=-S1	PCHEC208
	SD1=DS	PCHEC209
	110 CONTINUE	PCHEC210
	IF (ABS(S2).LE.1.0E-03) GO TO 120	PCHEC211
	IF (ABS(E2).LT.ABS(EOT2)) GO TO 120	PCHEC212
	ISE=0	PCHEC213
	IS=0	PCHEC214
	DD(2,2)=+EDOWNC	PCHEC215
	IF (S2.LT.0.0) DD(2,2)=-EDOWNT	PCHEC216
	DE=E2-EOT2	PCHEC217
	IF (DE.GT.0.0) DD(2,2)=-DD(2,2)	PCHEC218
	DS=(E2-EOT2)*DD(2,2)	PCHEC219
	IF (S2.GE.0.0) IS=1	PCHEC220
	IF ((DS+S2).GE.0.0) ISE=1	PCHEC221
	IF (IS.NE.ISE) DS=-S2	PCHEC222
	SD2=DS	PCHEC223
	120 CONTINUE	PCHEC224
	IF (SD1.EQ.0.0.AND.SD2.EQ.0.0) GO TO 180	PCHEC225
C		PCHEC226



C	TRANSFORM FROM PRINCIPAL TO X-Y DIRECTION	PCHEC227
C	CALL TRANG (SD1,SD2,SD3,ANGDS,SDX,SDY,SDXY)	PCHEC228
C		PCHEC229
C	ADD RESIDUAL STRESS VECTOR TO OLD TOTAL STRESSES	PCHEC230
C		PCHEC231
C		PCHEC232
	SXXOT(ML)=SXXOT(ML)-SDX	PCHEC233
	SYYOT(ML)=SYYOT(ML)-SDY	PCHEC234
	SXYOT(ML)=SXYOT(ML)-SDXY	PCHEC235
C		PCHEC236
C		PCHEC237
C	DETERMINE FICTIOUS FORCE VECTOR	PCHEC238
C		PCHEC239
	ICALL=3HYES	PCHEC240
	CALL FLOADP (N,ML,SDX,SDY,SDXY,NUMEL,NULAY,NA1,NSLAYR,NANA,ISTART,	PCHEC241
	1IEND,NPI,NPJ,NPK,NPL,FZC,PDT,DT,SZC,SDT,ELLA,ELLB,FCADD)	PCHEC242
	ISTART=3HNO	PCHEC243
	GO TO 180	PCHEC244
	130 CONTINUE	PCHEC245
C		PCHEC246
C	NO CRACKING OR CRUSHING	PCHEC247
C		PCHEC248
	CALL PRNCIP (SX,SY,SXY,S1,S2,THETAS)	PCHEC249
	S1=-S1	PCHEC250
	S2=-S2	PCHEC251
C		PCHEC252
C	CONSIDER 1 DIRECTION AS DIRECTION OF INTREST	PCHEC253
C		PCHEC254
	CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	PCHEC255
C		PCHEC256
C	CONSIDER 2 DIRECTION AS DIRECTION OF INTREST	PCHEC257
C		PCHEC258
	CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	PCHEC259
C		PCHEC260
C	CHECK DIRECTION OF POSSIBLE CRACK	PCHEC261
C		PCHEC262
	IF (DIRT1.EQ.5HNONE ) GO TO 150	PCHEC263
C		PCHEC264
C	CRACK IS IN DIRECTION OF S2 AND CAUSED BY S1	PCHEC265
C	S2 IS STILL EFFECTIVE	PCHEC266
C		PCHEC267
	ELOW=SP1-TOLF*SP1	PCHEC268
	IF (ABS(S1).GE.ABS(ELOW)) GO TO 140	PCHEC269
	GO TO 180	PCHEC270
	140 CONTINUE	PCHEC271
	AFT(ML)=DIRT1	PCHEC272
	AS1(N,ML)=THETAS	PCHEC273
	AF(N,ML)=THETAS-90.0	PCHEC274
	IF (ICMPLT.EQ.3HYES.AND.DIRT1.EQ.5HCRUSH) AS(N,ML)=THETAS	PCHEC275
	IF (AS(N,ML).EQ.THETAS) AST(ML)=DIRT1	PCHEC276
	WRITE (IO,60) N,ML,AF(N,ML)	PCHEC277
	GO TO 180	PCHEC278
C		PCHEC279
	150 CONTINUE	PCHEC280
	IF (DIRT2.EQ.5HNONE ) GO TO 180	PCHEC281
C		PCHEC282
C	CRACK IS IN DIRECTION OF S1 AND CAUSED BY S2	PCHEC283
C	S1 IS STILL EFFECTIVE	PCHEC284

C	ELOW=SP2-TOLF*SP2	PCHEC285
	IF (ABS(S2).GE.ABS(ELOW)) GO TO 160	PCHEC286
	GO TO 180	PCHEC287
160	CONTINUE	PCHEC288
	AS1(N,ML)=THETAS	PCHEC289
	AF(N,ML)=THETAS	PCHEC290
	AFT(ML)=DIRT2	PCHEC291
	IF (ICMPLT.EQ.3HYES.AND.DIRT2.EQ.5HGRUSH) AS(N,ML)=THETAS+90.	PCHEC292
	IF (AS(N,ML).EQ.(THETAS+90.)) AST(ML)=DIRT2	PCHEC293
	WRITE (IO,60) N,ML,AF(N,ML)	PCHEC294
	GO TO 180	PCHEC295
C		PCHEC296
C	FOR STEEL	PCHEC297
C		PCHEC298
		PCHEC299
170	CONTINUE	PCHEC300
	IF (AS1(N,ML).NE.999.0) GO TO 180	PCHEC301
	ANG=SPHI(ML-NULAY)	PCHEC302
	M=NSTYPE(ML-NULAY)	PCHEC303
	CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	PCHEC304
	IF (ABS(S1).LT.SIGMAP(M)) GO TO 180	PCHEC305
	AFT(ML)=5HYIELD	PCHEC306
	AS1(N,ML)=ANG	PCHEC307
	WRITE (IO,60) N,ML,AS1(N,ML)	PCHEC308
C		PCHEC309
C	SET UP FICTITIOUS FORCE VECTOR	PCHEC310
C		PCHEC311
180	CONTINUE	PCHEC312
	IEND=3HYES	PCHEC313
	WRITE (4) AFT,AST	PCHEC314
	IF (PFC.EQ.0.0) GO TO 190	PCHEC315
	IF (ICALL.EQ.3HYES) CALL FLOADP (N,ML,SDX,SDY,SDXY,NUMEL,NULAY,NA1,	PCHEC316
	1,NSLAYR,NANA,ISTART,IEND,NPI,NPJ,NPK,NPL,PZC,PDT,DT,SZC,SDT,ELLA,EPGHEC317	PCHEC317
	2LLB,FCADD)	PCHEC318
	WRITE (5) SXXOT,SYOT,SXYOT	PCHEC319
C		PCHEC320
C	END ROUTINE	PCHEC321
C		PCHEC322
190	CONTINUE	PCHEC323
C		PCHEC324
	DO 200 I=1,NUMEL	PCHEC325
	READ (32) N,ML,M	PCHEC326
200	WRITE (4) N,ML,M	PCHEC327
	REWIND 31	PCHEC328
	WRITE (31) AS1,AF,AS	PCHEC329
	REWIND 4	PCHEC330
	REWIND 32	PCHEC331
	CALL FTNCOPY (4LJUM4,3LJ32,0)	PCHEC332
	IF (PFC.EQ.0.0) RETURN	PCHEC333
	REWIND 5	PCHEC334
	REWIND 21	PCHEC335
	CALL FTNCOPY (4LJUM5,3LJ21,0)	PCHEC336
	RETURN	PCHEC337
	END	PCHEC338
	SUBROUTINE FLOADP (N,ML,SDX,SDY,SDXY,NUMEL,NULAY,NA1,NSLAYR,NANA,I	FLOADP 2
	1START,IEND,NPI,NPJ,NPK,NPL,PZC,PDT,DT,SZC,SDT,ELLA,ELLB,FCADD)	FLOADP 3
C		FLOADP 4
C	THIS SUBROUTINE COMPUTES FICTITIOUS FORCES	FLOADP 5

C		FLOADP 6
C		FLOADP 7
	COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)	FLOADP 8
C	DIMENSION SB(3), SBT(3), SUT(3), SU(3)	FLOADP 9
	DIMENSION FUT(8), FBT(12)	FLOADP10
	DIMENSION PDT(NULAY), PZC(NULAY), SDT(NSLAYR), SZC(NSLAYR), FCADE(1NA1), FU(8), FB(12)	FLOADP12
		FLOADP13
C		FLOADP14
C	CHECK FOR IEND	FLOADP15
C		FLOADP16
	IF (IEND.EQ.3HYES) GO TO 40	FLOADP17
	IF (ISTART.EQ.3HNO ) GO TO 10	FLOADP18
C		FLOADP19
C	INITIALIZE	FLOADP20
C		FLOADP21
	ABAR=(PZC(1)-PDT(1)/2.0+PZC(NULAY)+PDT(NULAY)/2.0)	FLOADP22
	ABAR=ABAR*DT/2.0	FLOADP23
	SDXB = 0.	FLOADP24
	SDYB = 0.	FLOADP25
	SDXYB=0.	FLOADP26
	SDXU=0.	FLOADP27
	SDYU=0.	FLOADP28
	SDXYU=0.	FLOADP29
	10 CONTINUE	FLOADP30
C		FLOADP31
C	COMPUTE INPLANE FORCE VECTOR	FLOADP32
C		FLOADP33
	IF (ML.GT.NULAY) GO TO 20	FLOADP34
	C1=(PZC(ML)-PDT(ML)/2.0)*DT	FLOADP35
	C2=(PZC(ML)+PDT(ML)/2.0)*DT	FLOADP36
	GO TO 30	FLOADP37
	20 CONTINUE	FLOADP38
	C2=SZC(ML-NULAY)+SDT(ML-NULAY)/2.0	FLOADP39
	C1=SZC(ML-NULAY)-SDT(ML-NULAY)/2.0	FLOADP40
	30 CONTINUE	FLOADP41
	CONST=C2-C1	FLOADP42
	SDXU=SDX*CONST+SDXU	FLOADP43
	SDYU=SDY*CONST+SDYU	FLOADP44
	SDXYU=SDXY*CONST+SDXYU	FLOADP45
C		FLOADP46
C	OUT OF PLANE FORCE VECTOR	FLOADP47
C		FLOADP48
	CONST=(C2+C1-2.0*ABAR)*(C2-C1)/2.0	FLOADP49
	SDXB=SDXB+SDX*CONST	FLOADP50
	SDYB=SDYB+SDY*CONST	FLOADP51
	SDXYB=SDXYB+SDXY*CONST	FLOADP52
	RETURN	FLOADP53
C		FLOADP54
C	END ROUTINE	FLOADP55
C		FLOADP56
	40 CONTINUE	FLOADP57
	IF (ABS(PHI).EQ.90.) GO TO 70	FLOADP58
	SB(1)=SDXB	FLOADP59
	SB(2)=SDYB	FLOADP60
	SB(3)=SDXYB	FLOADP61
	SU(1)=SDXU	FLOADP62
	SU(2)=SDYU	FLOADP63

	SU(3)=SDXYU	FLOADP64
C		FLOADP65
C	TRANSFORM TO SKEW	FLOADP66
	DO 60 I=1,3	FLOADP67
	SUMB=0.	FLOADP68
	SUMU=0.	FLOADP69
	DO 50 L=1,3	FLOADP70
	SUME=SUMB+DS(L,I)*SB(L)	FLOADP71
50	SUMU=SUMU+DS(L,I)*SU(L)	FLOADP72
C		FLOADP73
	SBT(I)=SUMB	FLOADP74
60	SUT(I)=SUMU	FLOADP75
C		FLOADP76
	SDXB=SBT(1)	FLOADP77
	SDYB=SBT(2)	FLOADP78
	SDXYB=SBT(3)	FLOADP79
	SDXU=SUT(1)	FLOADP80
	SDYU=SUT(2)	FLOADP81
	SDXYU=SUT(3)	FLOADP82
70	CONTINUE	FLOADP83
C		FLOADP84
	TA=ELLB	FLOADP85
	TB=ELLA	FLOADP86
	TAB=2.0	FLOADP87
C		FLOADP88
C	OUT OF PLANE	FLOADP89
C		FLOADP90
	FB(1)=2.0*SDXYB	FLOADP91
	FB(2)=-TB*SDYB	FLOADP92
	FB(3)=-TA*SDXB	FLOADP93
	FB(4)=-2.0*SDXYB	FLOADP94
	FB(5)=+TB*SDYB	FLOADP95
	FB(6)=-TA*SDXB	FLOADP96
	FB(7)=-2.0*SDXYB	FLOADP97
	FB(8)=-TB*SDYB	FLOADP98
	FB(9)=+TA*SDXB	FLOADP99
	FB(10)=+2.0*SDXYB	FLOAD100
	FB(11)=+TB*SDYB	FLOAD101
	FB(12)=+TA*SDXB	FLOAD102
C		FLOAD103
C	INPLANE	FLOAD104
C		FLOAD105
	FU(1)=-TA*SDXU+TB*SDXYU	FLOAD106
	FU(3)=-TA*SDXU-TB*SDXYU	FLOAD107
	FU(5)=+TA*SDXU+TB*SDXYU	FLOAD108
	FU(7)=+TA*SDXU-TB*SDXYU	FLOAD109
	FU(2)=-TA*SDXYU+TB*SDYU	FLOAD110
	FU(4)=-TA*SDXYU-TB*SDYU	FLOAD111
	FU(6)=+TA*SDXYU+TB*SDYU	FLOAD112
	FU(8)=+TA*SDXYU-TB*SDYU	FLOAD113
	IF (ABS(PHI).EQ.90.) GO TO 140	FLOAD114
C		FLOAD115
C	TRANSFORM BACK TO CARTESIAN	FLOAD116
C		FLOAD117
	DO 90 II=1,8,2	FLOAD118
	DO 90 I=1,2	FLOAD119
	SUM=0.	FLOAD120
	DO 80 L=1,2	FLOAD121

80	SUM=SUM+AU(I,L)*FU(II+L-1)	FLOAD122
90	FUT(II+I-1)=SUM	FLOAD123
C		FLOAD124
	DO 110 II=1,12,3	FLOAD125
	DO 110 I=1,3	FLOAD126
	SUM=0.	FLOAD127
	DO 100 L=1,3	FLOAD128
100	SUM=SUM+AO(I,L)*FB(II+L-1)	FLOAD129
110	FBT(II+I-1)=SUM	FLOAD130
C		FLOAD131
	SINA=PHI*ATAN(1.0)/45.	FLOAD132
	SINA=SIN(SINA)	FLOAD133
	DO 120 I=1,8	FLOAD134
120	FU(I)=FUT(I)*SINA	FLOAD135
	DO 130 I=1,12	FLOAD136
130	FB(I)=FBT(I)*SINA	FLOAD137
C		FLOAD138
140	CONTINUE	FLOAD139
C		FLOAD140
C		FLOAD141
C	PLACE ELEMENT FORCE VECTORS INTO GLOBAL FORCE VECTOR	FLOAD142
C		FLOAD143
	I=NPI*5-3	FLOAD144
	J=NPJ*5-3	FLOAD145
	K=NPK*5-3	FLOAD146
	L=NPL*5-3	FLOAD147
	DO 150 II=1,3	FLOAD148
	FCADD(I+II)=FCADD(I+II)+FB(II)	FLOAD149
	FCADD(J+II)=FCADD(J+II)+FB(II+3)	FLOAD150
	FCADD(K+II)=FCADD(K+II)+FB(II+6)	FLOAD151
150	FCADD(L+II)=FCADD(L+II)+FB(II+9)	FLOAD152
C		FLOAD153
	I=I-2	FLOAD154
	J=J-2	FLOAD155
	K=K-2	FLOAD156
	L=L-2	FLOAD157
	DO 160 II=1,2	FLOAD158
	FCADD(I+II)=FCADD(I+II)+FU(II)	FLOAD159
	FCADD(J+II)=FCADD(J+II)+FU(II+2)	FLOAD160
	FCADD(K+II)=FCADD(K+II)+FU(II+4)	FLOAD161
	FCADD(L+II)=FCADD(L+II)+FU(II+6)	FLOAD162
	IF (ABS(ABAR).LT.0.001) GO TO 160	FLOAD163
	FCADD(I+6-II)=FCADD(I+6-II)+FU(II)*ABAR*(-1.0)**(II+1)	FLOAD164
	FCADD(J+6-II)=FCADD(J+6-II)+FU(II+2)*ABAR*(-1.0)**(II+1)	FLOAD165
	FCADD(K+6-II)=FCADD(K+6-II)+FU(II+4)*ABAR*(-1.0)**(II+1)	FLOAD166
	FCADD(L+6-II)=FCADD(L+6-II)+FU(II+6)*ABAR*(-1.0)**(II+1)	FLOAD167
160	CONTINUE	FLOAD168
	RETURN	FLOAD169
	END	FLOAD170
	SUBROUTINE ACCUMU (NUMEL,NA1,NBEAMX,NULAY8,NCNS,NULAY,CTXX,CTYY,CTACCUMU 2	
	1XY,CUXX,CUYX,CUXY,ETXX,ETYY,ETXY,EIXX,EIYY,EIXY,SXXOT,SYOT,SXYOT,ACCUMU 3	
	2SIGXX0,SIGYY0,SIGXY0,DISPLA,DISP,FORCE,FORCEA,ESX,ITYPE,ZCRD,DUM,EACCUMU 4	
	3PSPS,AS1,AFT,AST,SIBXXC,SIBXXA,AF,AS)	ACCUMU 5
C		ACCUMU 6
C	THIS SUBROUTINE FINDS ACCUMULATED VALUES OF NEEDED FIELD	ACCUMU 7
C	QUANTITIES.	ACCUMU 8
C	ALSO TERMINATION CHECKS ARE MADE.	ACCUMU 9
C		ACCUMU10

	DIMENSION CTXX(NUMEL), CTYY(NUMEL), CTXY(NUMEL), CUXX(NUMEL), CUYXACCUMU11	
	1(NUMEL), CUXY(NUMEL), ETXX(NUMEL), ETTY(NUMEL), ETXY(NUMEL), EIXX(ACCUMU12	
	2NUMEL), EIYY(NUMEL), EIXY(NUMEL), SXXOT(NCNS), SYOT(NCNS), SXYOT(ACCUMU13	
	3NCNS), SIGXXO(NCNS), SIGYYO(NCNS), SIGXYO(NCNS), SIBXXA(NULAYB), SACCUMU14	
	4IBXXG(NULAYB), ESX(NULAYB,NEEAMX), ITYPE(NULAYB), ZCRG(NULAYB), DUACCUMU15	
	5M(NULAYB), EPSPS(NULAYB), AS1(NUMEL,NCNS), AFT(NCNS), AST(NCNS), DACCUMU15	
	6ISPLA(NA1), DISP(NA1), FORCEA(NA1), FORCE(NA1), DUMMY(1)	ACCUMU17
	DIMENSION NLH(6), NLK(6), CTOT(3), ETOT(3)	ACCUMU18
	DIMENSION AS(NUMEL,NULAY), AF(NUMEL,NULAY), EM(2,2), ANC(2)	ACCUMU19
C		ACCUMU20
	COMMON /CB3J0/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	ACCUMU21
	COMMON /BDIM/ RON(6),ROM(6),EDOWN(6),STRAN(6),SIGO(6),FT(6),RONT(3ACCUMU22	
	1),ROMT(3),EDOWNT(3),EDOWNR(3)	ACCUMU23
	COMMON /BTCHK/ STRAMX(6),STEMX(6),STCEMX(6),NK(6),NH(6)	ACCUMU24
	COMMON /HISTB/ NVALB(6,5),IVALB(6,2,5)	ACCUMU25
	COMMON /STCHK/ STRAMS(6),STEMS(6),STGEMS(6),NKS(6),NHS(6)	ACCUMU26
	COMMON /HISTS/ NVALS(6,5),IVALS(6,2,5)	ACCUMU27
	COMMON /PDIM/ PDT(15),PZC(15),SEMOD(4),SIGMAP(4),SPRON(4),SPROM(4)ACCUMU28	
	1,NSTYPE(6),SDT(6),SZC(6),SPHI(6)	ACCUMU29
	COMMON /CB31G/ NELX,NSMAT,NELY	ACCUMU30
	COMMON /GRWS/ ACR(2),CMIN(2),WGMAX(2),W,NW,DC,SURC	ACCUMU31
C		ACCUMU32
	REWIND 5	ACCUMU33
	REWIND 19	ACCUMU34
	REWIND 21	ACCUMU35
	REWIND 15	ACCUMU36
	REWIND 23	ACCUMU37
	REWIND 25	ACCUMU38
	REWIND 27	ACCUMU39
	REWIND 28	ACCUMU40
	REWIND 32	ACCUMU41
C		ACCUMU42
C	INITIALIZE CRACK WIDTH DATA	ACCUMU43
C		ACCUMU44
	W=0.	ACCUMU45
	NW=0	ACCUMU46
	DC=0.	ACCUMU47
C		ACCUMU48
	WM=0.	ACCUMU49
	NWM=0	ACCUMU50
	DCM=0.	ACCUMU51
	SURCM=4HNONE	ACCUMU52
C		ACCUMU53
	SURC=4HNONE	ACCUMU54
C		ACCUMU55
	DO 220 N=1,NUMEL	ACCUMU56
	READ (19) SIGXXO,SIGYYO,SIGXYO	ACCUMU57
	READ (21) SXXOT,SYOT,SXYOT	ACCUMU58
	CTXX(N)=CTXX(N)+CUXX(N)	ACCUMU59
	CTYY(N)=CTYY(N)+CUIYY(N)	ACCUMU60
	CTXY(N)=CTXY(N)+CUXY(N)	ACCUMU61
	ETXX(N)=ETXX(N)+EIXX(N)	ACCUMU62
	ETYY(N)=ETYY(N)+EIYY(N)	ACCUMU63
	ETXY(N)=ETXY(N)+EIXY(N)	ACCUMU64
C		ACCUMU65
C	READ INFORMATION FOR TERMINATION CHECKS	ACCUMU66
C		ACCUMU67
	READ (28) DT,A,A,A,A,A,A	ACCUMU68

	READ (32) AFT,AST	ACCUMU69
	CTOT(1)=CTXX(N)	ACCUMU70
	CTOT(2)=CTYY(N)	ACCUMU71
	CTOT(3)=CTXY(N)	ACCUMU72
C	DO 10 I=1,6	ACCUMU73
	NLH(I)=0	ACCUMU74
10	NLK(I)=0	ACCUMU75
C		ACCUMU76
	DO 100 ML=1,NCNS	ACCUMU77
	SXXOT(ML)=SXXOT(ML)+SIGXXO(ML)	ACCUMU78
	SYYOT(ML)=SYYOT(ML)+SIGYYO(ML)	ACCUMU79
	SXYOT(ML)=SXYOT(ML)+SIGXYO(ML)	ACCUMU80
C		ACCUMU81
C	FIND TOTAL STRAIN	ACCUMU82
C		ACCUMU83
	CALL STRANO (N,ML,CTOT,ETOT,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	ACCUMU84
	EX=ETOT(1)+ETXX(N)	ACCUMU85
	EY=ETOT(2)+ETYY(N)	ACCUMU86
	EXY=ETOT(3)+ETXY(N)	ACCUMU87
	EXY=0.5*EXY	ACCUMU88
C		ACCUMU89
C	FIND STRESSES	ACCUMU90
C		ACCUMU91
	SX=SXXOT(ML)	ACCUMU92
	SY=SYYOT(ML)	ACCUMU93
	SXY=SXYOT(ML)	ACCUMU94
C		ACCUMU95
C	CHECK FOR STEEL LAYER	ACCUMU96
C		ACCUMU97
	IF (ML.GT.NULAY) GO TO 50	ACCUMU98
C		ACCUMU99
C	FOR CONCRETE	ACCUM100
C		ACCUM101
	K=1	ACCUM102
C		ACCUM103
C	CHECK FOR CRACKED OR CRUSHED LAYER	ACCUM104
C		ACCUM105
	IF (AS1(N,ML).NE.999.0) GO TO 20	ACCUM106
C		ACCUM107
C	FOR NO CRACKING OR CRUSHING	ACCUM108
C		ACCUM109
	CALL PRNCIP (SX,SY,SXY,S1,S2,ANG)	ACCUM110
	CALL PRNCIP (EX,EY,EXY,E1,E2,ANG)	ACCUM111
	GO TO 30	ACCUM112
20	CONTINUE	ACCUM113
C		ACCUM114
C	FOR CRACKING OR CRUSHING	ACCUM115
C		ACCUM116
	ANG=AS1(N,ML)	ACCUM117
	CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	ACCUM118
	CALL TRANG (EX,EY,EXY,ANG,E1,E2,E3)	ACCUM119
30	CONTINUE	ACCUM120
C		ACCUM121
C	FIND FAILURE STRESS	ACCUM122
C		ACCUM123
	S1=-S1	ACCUM124
	S2=-S2	ACCUM125
		ACCUM126

	CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	ACCUM127
	CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	ACCUM128
C		ACCUM129
C	SET UP TOP AND BOTTOM CONCRETE LAYER STRAINS	ACCUM130
C		ACCUM131
	IF (ML.EQ.1) EM(1,2)=E1	ACCUM132
	IF (ML.EQ.1) EM(2,2)=E2	ACCUM133
	IF (ML.EQ.NULAY) EM(1,1)=E1	ACCUM134
	IF (ML.EQ.NULAY) EM(2,1)=E2	ACCUM135
C		ACCUM136
C		ACCUM137
C	FIND CRITICAL VALUES	ACCUM138
C		ACCUM139
	S=0.	ACCUM140
	E=0.	ACCUM141
	SP=1.0E+10	ACCUM142
	EP=1.0E+10	ACCUM143
	DIRT=5HNONE	ACCUM144
	IF (DIRT1.EQ.5HNONE ) GO TO 40	ACCUM145
	DIRT=DIRT1	ACCUM146
	S=-S1	ACCUM147
	E=E1	ACCUM148
	SP=-SP1	ACCUM149
	EP=-EP1	ACCUM150
	GO TO 60	ACCUM151
	40 IF (DIRT2.EQ.5HNONE ) GO TO 60	ACCUM152
	DIRT=DIRT2	ACCUM153
	S=-S2	ACCUM154
	E=E2	ACCUM155
	SP=-SP2	ACCUM156
	EP=-EP2	ACCUM157
	GO TO 60	ACCUM158
	50 CONTINUE	ACCUM159
C		ACCUM160
C	FOR STEEL	ACCUM161
C		ACCUM162
	M=NSTYPE(ML-NULAY)	ACCUM163
	K=M+1	ACCUM164
	ANG=SPHI(ML-NULAY)	ACCUM165
	CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	ACCUM166
	CALL TRANG (EX,EY,EXY,ANG,E1,E2,E3)	ACCUM167
	DIRT=5HCRUSH	ACCUM168
	IF (S1.GE.0.0) DIRT=5HCRACK	ACCUM169
	S=S1	ACCUM170
	E=E1	ACCUM171
	SP=1.0	ACCUM172
	EP=1.0	ACCUM173
	60 CONTINUE	ACCUM174
C		ACCUM175
C	TERMINATION CHECKS	ACCUM176
C		ACCUM177
C		ACCUM178
C	CHECK STRAIN	ACCUM179
C		ACCUM180
	A=STRAMS(K)	ACCUM181
	IF (ABS(E).LT.ABS(A*EP)) GO TO 70	ACCUM182
	NVALS(K,1)=3HYES	ACCUM183
	IVALS(K,1,1)=N	ACCUM184



	IVAL(S(K,2,1)=ML	ACCUM185
	70 IF (DIRT.NE.5HCRACK) GO TO 80	ACCUM186
C		ACCUM187
C	CHECK TENSILE STRESS	ACCUM188
C		ACCUM189
	A=STEMS(K)	ACCUM190
	IF (ABS(S).LT.ABS(A*SP)) GO TO 90	ACCUM191
	NVALS(K,2)=3HYES	ACCUM192
	IVAL(S(K,1,2)=N	ACCUM193
	IVAL(S(K,2,2)=ML	ACCUM194
	GO TO 90	ACCUM195
	80 IF (DIRT.NE.5HCRUSH) GO TO 90	ACCUM196
C		ACCUM197
C	CHECK COMPRESSIVE STRESS	ACCUM198
C		ACCUM199
	A=STEMS(K)	ACCUM200
	IF (ABS(S).LT.ABS(A*SP)) GO TO 90	ACCUM201
	NVALS(K,3)=3HYES	ACCUM202
	IVAL(S(K,1,3)=N	ACCUM203
	IVAL(S(K,2,3)=ML	ACCUM204
	90 CONTINUE	ACCUM205
C		ACCUM206
C	SUM CRACKED, CRUSHED AND YIELDED LAYERS	ACCUM207
C		ACCUM208
	IF (AFT(ML).EQ.5HCRACK.OR.AST(ML).EQ.5HCRACK) NLK(K)=NLK(K)+1	ACCUM209
	IF (AFT(ML).EQ.5HCRUSH.OR.AST(ML).EQ.5HCRUSH) NLH(K)=NLH(K)+1	ACCUM210
	IF (AFT(ML).EQ.5HYIELD) NLK(K)=NLK(K)+1	ACCUM211
C		ACCUM212
	100 CONTINUE	ACCUM213
	WRITE (5) SXXOT,SYOT,SXYOT	ACCUM214
C		ACCUM215
C	CHECK NUMBER OF CRACKED,CRUSHED OR YIELDED LAYERS	ACCUM216
C		ACCUM217
	I=NSMAT+1	ACCUM218
	DO 120 K=1,I	ACCUM219
	IF (K.GT.1) GO TO 110	ACCUM220
C		ACCUM221
C	CHECK CRUSHED LAYERS	ACCUM222
C		ACCUM223
	IF (NLH(K).LT.NHS(K)) GO TO 110	ACCUM224
	NVALS(K,5)=3HYES	ACCUM225
	IVAL(S(K,1,5)=N	ACCUM226
C		ACCUM227
C	CHECK CRACKED OR YIELDED LAYERS	ACCUM228
C		ACCUM229
	110 IF (NLK(K).LT.NKS(K)) GO TO 120	ACCUM230
	NVALS(K,4)=3HYES	ACCUM231
	IVAL(S(K,1,4)=N	ACCUM232
	120 CONTINUE	ACCUM233
C		ACCUM234
	DO 200 K=1,2	ACCUM235
	IF (ACR(K).EQ.0.) GO TO 200	ACCUM236
C		ACCUM237
C	CHOOSE TOP OR BOTTOM CONCRETE LAYER	ACCUM238
C		ACCUM239
	I=1	ACCUM240
	LP=0	ACCUM241
	IF (K.EQ.1) LP=NULAY+1	ACCUM242

	ML=I*(-1)**K+LP	ACCUM243
C		ACCUM244
C	CHECK FOR CRACKS	ACCUM245
C	AND ASSIGN ANGLES	ACCUM246
C		ACCUM247
	ANC(1)=999.0	ACCUM248
	ANC(2)=999.0	ACCUM249
	ANG=999.	ACCUM250
G		ACCUM251
	IF (AFT(ML).NE.5HCRACK) GO TO 130	ACCUM252
	ANC(1)=AF(N,ML)	ACCUM253
	ANG=AS1(N,ML)	ACCUM254
	130 CONTINUE	ACCUM255
	IF (AST(ML).NE.5HCRACK) GO TO 140	ACCUM256
	ANC(2)=AS(N,ML)	ACCUM257
	ANG=AS1(N,ML)	ACCUM258
	140 CONTINUE	ACCUM259
C		ACCUM260
C	GO TO ANOTHER LAYER IF NO CRACKS	ACCUM261
C		ACCUM262
	IF (ANG.EQ.999.) GO TO 200	ACCUM263
C		ACCUM264
C	THERE IS A CRACK	ACCUM265
C		ACCUM266
	SUR=4HTOP	ACCUM267
	IF (K.EQ.1) SUR=4H8OT.	ACCUM268
	IF (ANG.LT.0.) ANG=ANG+180.	ACCUM269
C		ACCUM270
C	CHOOSE DIRECTION 1 OR 2	ACCUM271
C		ACCUM272
	DO 190 ID=1,2	ACCUM273
	NC=0	ACCUM274
	ANGT=ANC(ID)	ACCUM275
	IF (ANGT.EQ.999.0) GO TO 190	ACCUM276
	IF (ANGT.LT.0.) ANGT=ANGT+180.	ACCUM277
C		ACCUM278
C	FIND CRACK DEPTH	ACCUM279
C		ACCUM280
	DO 160 I=1,NULAY	ACCUM281
	ML=I*(-1)**(K)+LP	ACCUM282
	NCC=NC	ACCUM283
G		ACCUM284
	IF (AFT(ML).NE.5HCRACK) GO TO 150	ACCUM285
	TEST=AF(N,ML)	ACCUM286
	IF (TEST.LT.0.0) TEST=TEST+180.	ACCUM287
	IF (TEST.LE.(ANGT+45.0).AND.TEST.GE.(ANGT-45.0)) NC=NC+1	ACCUM288
	IF (NCC.EQ.NC) GO TO 150	ACCUM289
	GO TO 160	ACCUM290
C		ACCUM291
	150 IF (AST(ML).NE.5HCRACK) GO TO 170	ACCUM292
	TEST=AS(N,ML)	ACCUM293
	IF (TEST.LT.0.0) TEST=TEST+180.	ACCUM294
	IF (TEST.LE.(ANGT+45.0).AND.TEST.GE.(ANGT-45.0)) NC=NC+1	ACCUM295
	IF (NCC.EQ.NC) GO TO 170	ACCUM296
C		ACCUM297
C	GO TO ANOTHER CONCRETE LAYER AND CHECK FOR CRACK	ACCUM298
C		ACCUM299
	160 CONTINUE	ACCUM300

	170 CONTINUE	ACCUM301
C		ACCUM302
C	END OF CRACK	ACCUM303
C		ACCUM304
C	FIND CRACK DIRECTION WITH RESPECT TO PRINCIPAL ANGLE	ACCUM305
C		ACCUM306
	M=2	ACCUM307
	IF (ANGT.NE.ANG) M=1	ACCUM308
C		ACCUM309
C	COMPUTE CRACK DEPTH	ACCUM310
C		ACCUM311
	A=NULAY	ACCUM312
	HC=NC	ACCUM313
	HC=HC*DT/A	ACCUM314
C		ACCUM315
C	COMPUTE CRACK WIDTH	ACCUM316
C		ACCUM317
	A=2.+4.25*(ACR(K)-CMIN(K))/HC	ACCUM318
	A=ACR(K)*9.0*EM(M,K)/A	ACCUM319
C		ACCUM320
C	CHECK ALLOWABLE CRACK WIDTH	ACCUM321
	IF (A.LT.W.OR.A.LT.WCMAX(K)) GO TO 180	ACCUM322
	IF (A.LT.W) GO TO 190	ACCUM323
	W=A	ACCUM324
	NW=N	ACCUM325
	SURC=SUR	ACCUM326
	DC=ANGT	ACCUM327
C		ACCUM328
	180 IF (A.LT.WM) GO TO 190	ACCUM329
	WM=A	ACCUM330
	NWM=N	ACCUM331
	SURCM=SUR	ACCUM332
	DCM=ANGT	ACCUM333
C		ACCUM334
C	GO TO NEW DIRECTION	ACCUM335
C		ACCUM336
	190 CONTINUE	ACCUM337
C		ACCUM338
C	GO TO NEW SUFACE	ACCUM339
C		ACCUM340
	200 CONTINUE	ACCUM341
C		ACCUM342
	IF (W.NE.0.0) GO TO 210	ACCUM343
	W=WM	ACCUM344
	NW=NWM	ACCUM345
	DC=DCM	ACCUM346
	SURC=SURCM	ACCUM347
	210 CONTINUE	ACCUM348
C		ACCUM349
	220 CONTINUE	ACCUM350
	REWIND 5	ACCUM351
	REWIND 21	ACCUM352
	CALL FTNCOPY (4LJUM5,3LJ21,0)	ACCUM353
	DO 230 I=1,NA1	ACCUM354
	DISPLA(I)=DISPLA(I)+DISP(I)	ACCUM355
	FORCEA(I)=FORCEA(I)+FORCE(I)	ACCUM356
	230 CONTINUE	ACCUM357
C		ACCUM358

	IF (NBEAMX.EQ.0) RETURN	ACCUM359
	REWIND 5	ACCUM360
	DO 310 N=1,NBEAMX	ACCUM361
	READ (15) SIBXXA	ACCUM362
	READ (23) SIBXXC	ACCUM363
C		ACCUM364
C	READ INFORMATION USED IN TERMINATION CHECKS	ACCUM365
C		ACCUM366
	READ (25) EPSPS	ACCUM367
C	READ (27) XLNGTH,GKEI,NPIX,NPKX,ITYPE,DUM,DUM,ZCRD,DUM	ACCUM368
		ACCUM369
	DO 240 I=1,KMAT	ACCUM370
	NLK(I)=0	ACCUM371
240	NLH(I)=0	ACCUM372
C		ACCUM373
	II=5*NPIX	ACCUM374
	KK=5*NPKX	ACCUM375
C		ACCUM376
	DO 290 ML=1,NULAYE	ACCUM377
	SIBXXA(ML)=SIBXXA(ML)+SIBXXC(ML)	ACCUM378
C		ACCUM379
	SIG=SIBXXA(ML)	ACCUM380
	K=ITYPE(ML)	ACCUM381
	ES=ESX(ML,N)	ACCUM382
	ZCR=ZCRD(ML)	ACCUM383
C		ACCUM384
	UI=DISPLA(II-4)+DISP(II-4)	ACCUM385
	UK=DISPLA(KK-4)+DISP(KK-4)	ACCUM386
	THETAI=DISPLA(II)+DISP(II)	ACCUM387
	THETAK=DISPLA(KK)+DISP(KK)	ACCUM388
	EPS=(UK-UI+(THETAK-THETAI)*ZCR)/XLNGTH	ACCUM389
	EPS=EPS+EPSPS(ML)	ACCUM390
C		ACCUM391
C	CHECK STRAIN	ACCUM392
C		ACCUM393
	IF (ABS(EPS).LT.STRAMX(K)) GO TO 250	ACCUM394
	NVALB(K,1)=3HYES	ACCUM395
	IVALB(K,1,1)=N	ACCUM396
	IVALB(K,2,1)=ML	ACCUM397
C		ACCUM398
250	IF (SIG.LT.0.0) GO TO 260	ACCUM399
C		ACCUM400
C	CHECK TENSILE STRESS	ACCUM401
C		ACCUM402
	IF (SIG.LT.STTEMX(K)) GO TO 270	ACCUM403
	NVALB(K,2)=3HYES	ACCUM404
	IVALB(K,1,2)=N	ACCUM405
	IVALB(K,2,2)=ML	ACCUM406
C		ACCUM407
C	CHECK COMPRESSIVE STRESS	ACCUM408
C		ACCUM409
260	IF (ABS(SIG).LT.STCEMX(K)) GO TO 270	ACCUM410
	NVALB(K,3)=3HYES	ACCUM411
	IVALB(K,1,3)=N	ACCUM412
	IVALB(K,2,3)=ML	ACCUM413
270	CONTINUE	ACCUM414
C		ACCUM415
C	SUM CRACKED, CRUSHED AND YIELDED LAYERS	ACCUM415

C	IF (K.GT.KC) GO TO 280	ACCUM417
	IF (ES.EQ.0.0.AND.SIG.LT.0.0) NLH(K)=NLH(K)+1	ACCUM418
	IF (ES.EQ.0.0.AND.SIG.GE.0.0) NLK(K)=NLK(K)+1	ACCUM419
	GO TO 290	ACCUM420
	280 IF (ABS(SIG).GE.FT(K).OR.ABS(SIG).GE.SIGO(K)) NLK(K)=NLK(K)+1	ACCUM421
C		ACCUM422
C		ACCUM423
	290 CONTINUE	ACCUM424
	WRITE (5) SIBXXA	ACCUM425
C		ACCUM426
C		ACCUM427
	DO 310 I=1,KMAT	ACCUM428
	IF (I.GT.KC) GO TO 300	ACCUM429
C		ACCUM430
C	CHECK NUMBER OF CRUSHED LAYERS	ACCUM431
C		ACCUM432
	IF (NLH(I).LT.NH(I)) GO TO 300	ACCUM433
	NVALB(K,5)=3HYES	ACCUM434
	IVALB(K,1,5)=N	ACCUM435
C		ACCUM436
C	CHECK NUMBER OF CRACKED LAYERS OR YIELDED LAYERS	ACCUM437
C		ACCUM438
	300 IF (NLK(I).LT.NK(I)) GO TO 310	ACCUM439
	NVALB(K,4)=3HYES	ACCUM440
	IVALB(K,1,4)=N	ACCUM441
C		ACCUM442
	310 CONTINUE	ACCUM443
	REWIND 5	ACCUM444
	REWIND 15	ACCUM445
	CALL FTNGOPY (4LJUM5,3LJ15,0)	ACCUM446
	RETURN	ACCUM447
	END	ACCUM448
		ACCUM449
	SUBROUTINE BEAMC (NBEAMX,NULAYB,NA1,EIB,CUB,SIBXXA,SIBXXC,EPSPS,IT	BEAMC 2
	1TYPE,ASXLR,AILR,KC,ZCRD,TSHEAR,ESX,DISPLA)	BEAMC 3
C		BEAMC 4
C	COMPUTE CRACK WIDTH ON BOTTOM SUFACE OF CRITICAL BEAM ELEMENT	BEAMC 5
C		BEAMC 6
	COMMON /CRWB/ TC,M,TS,AE,NEL,ML,MLS,FSO,WBMAX,WC	BEAMC 7
C		BEAMC 8
	DIMENSION CUB(NBEAMX), EIB(NBEAMX), SIBXXA(NULAYB), SIBXXC(NULAYB)	BEAMC 9
	1, EPSPS(NULAYB), ITYPE(NULAYB), ASXLR(NULAYB), AILR(NULAYB), ZCRD(	BEAMC 10
	2NULAYB), TSHEAR(NULAYB), ESX(NULAYB,NBEAMX), DISPLA(NA1)	BEAMC 11
C		BEAMC 12
C	INITIALIZE	BEAMC 13
C		BEAMC 14
	WC=0.	BEAMC 15
C		BEAMC 16
C	CHECK TO SEE IF INITIAL STEEL STRESS IS TO BE FOUND	BEAMC 17
C		BEAMC 18
	IF (FSO.NE.-999.0) GO TO 130	BEAMC 19
C		BEAMC 20
C	FIND CRITICAL BEAM ELEMENT, NEL	BEAMC 21
C		BEAMC 22
	NEL=0	BEAMC 23
	A=0.	BEAMC 24
	DO 10 I=1,NBEAMX	BEAMC 25
	IF (A.GT.CUB(I)) GO TO 10	BEAMC 26

	NEL=I	BEAMC 27
	A=CUB(I)	BEAMC 28
	10 CONTINUE	BEAMC 29
	IF (NEL.EQ.0) RETURN	BEAMC 30
C		BEAMC 31
C	REWIND DATA TAPES	BEAMC 32
C		BEAMC 33
	REWIND 15	BEAMC 34
	REWIND 23	BEAMC 35
	REWIND 25	BEAMC 36
	REWIND 27	BEAMC 37
C		BEAMC 38
C	SET UP DATA VALUES FOR ELEMENT NEL	BEAMC 39
C		BEAMC 40
	DO 20 I=1,NEL	BEAMC 41
	READ (15) SI0XXA	BEAMC 42
	READ (23) SI0XXC	BEAMC 43
	READ (25) EPSPS	BEAMC 44
	READ (27) XLNGTH,GKEIX,NPIX,NPKX,ITYPE,ASXLR,AILR,ZCRD,TSHEAR	BEAMC 45
	20 CONTINUE	BEAMC 46
C		BEAMC 47
C	FIND THE CRITICAL CONCRETE LAYER, ML	BEAMC 48
C		BEAMC 49
	ML=0	BEAMC 50
	A=-999.	BEAMC 51
	DO 30 I=1,NULAYB	BEAMC 52
	IF (ITYPE(I).GT.KC) GO TO 30	BEAMC 53
	IF (A.GE.ZCRD(I)) GO TO 30	BEAMC 54
	A=ZCRD(I)	BEAMC 55
	ML=I	BEAMC 56
	30 CONTINUE	BEAMC 57
	IF (ML.NE.0) GO TO 40	BEAMC 58
	M=0	BEAMC 59
	RETURN	BEAMC 60
	40 CONTINUE	BEAMC 61
C		BEAMC 62
C	CHECK IF CONCRETE STRAIN IS TENSILE	BEAMC 63
C		BEAMC 64
	I=5*NPIX	BEAMC 65
	K=5*NPKX	BEAMC 66
	EB=DISPLA(K-4)-DISPLA(I-4)+(DISPLA(K)-DISPLA(I))*ZCRD(ML)	BEAMC 67
	EB=EB/XLNGTH+EPSPS(ML)	BEAMC 68
	EA=EIB(NEL)+CUB(NEL)*ZCRD(ML)	BEAMC 69
	IF (EA.EQ.0.) RETURN	BEAMC 70
	IF ((EA+EB).LT.0.) RETURN	BEAMC 71
C		BEAMC 72
C	FIND CRITICAL STEEL LAYER, MLS	BEAMC 73
C		BEAMC 74
	A=-999.0	BEAMC 75
	MLS=0	BEAMC 76
	DO 50 I=1,NULAYB	BEAMC 77
	IF (ITYPE(I).LE.KC) GO TO 50	BEAMC 78
	IF (A.GE.ZCRD(I)) GO TO 50	BEAMC 79
	A=ZCRD(I)	BEAMC 80
	MLS=I	BEAMC 81
	50 CONTINUE	BEAMC 82
	IF (MLS.NE.0) GO TO 60	BEAMC 83
	M=0	BEAMC 84

	RETURN	BEAMC 85
	60 CONTINUE	BEAMC 86
C		BEAMC 87
C	DEFINE STEEL CENTROID LOCATION, TS , MEASURED FROM BOTTOM	BEAMC 88
C		BEAMC 89
	A=0.	BEAMC 90
	B=0.0	BEAMC 91
	DO 70 I=1,NULAYB	BEAMC 92
	IF (ITYPE(I).LE.KC) GO TO 70	BEAMC 93
	A=A+ZCRD(I)*ASXLR(I)*ESX(I,NEL)	BEAMC 94
	B=B+ASXLR(I)*ESX(I,NEL)	BEAMC 95
	70 CONTINUE	BEAMC 96
	IF (B.EQ.0.) GO TO 80	BEAMC 97
	TSZ=A/B	BEAMC 98
	TS=ZCRD(ML)+ASXLR(ML)/TSHEAR(ML)/2.-TSZ	BEAMC 99
	IF (TS.GE.0.) GO TO 90	BEAMC 100
	80 CONTINUE	BEAMC 101
	M=0	BEAMC 102
	RETURN	BEAMC 103
	90 CONTINUE	BEAMC 104
C		BEAMC 105
C	DEFINE THE CONCRETE LAYER WIDTH, TW, AT THE STEEL CENTROID	BEAMC 106
C		BEAMC 107
	A=0.	BEAMC 108
	B=0.	BEAMC 109
	DO 110 I=1,NULAYB	BEAMC 110
C		BEAMC 111
C	CHECK IF LAYER IS STEEL	BEAMC 112
C		BEAMC 113
	IF (ITYPE(I).LE.KC) GO TO 110	BEAMC 114
C		BEAMC 115
C	FIND CONCRETE LAYER WIDTH AT STEEL LAYER CENTROID	BEAMC 116
C		BEAMC 117
	DO 100 K=1,NULAYB	BEAMC 118
	IF (ITYPE(K).GT.KC) GO TO 100	BEAMC 119
	TL=ASXLR(K)/TSHEAR(K)	BEAMC 120
	IF ((ZCRD(K)+TL/2.).LT.ZCRD(I)) GO TO 100	BEAMC 121
	IF ((ZCRD(K)-TL/2.).GE.ZCRD(I)) GO TO 100	BEAMC 122
	B=B+ASXLR(I)*ESX(I,NEL)	BEAMC 123
	A=A+TSHEAR(K)*ASXLR(I)*ESX(I,NEL)	BEAMC 124
	GO TO 110	BEAMC 125
	100 CONTINUE	BEAMC 126
C		BEAMC 127
C	FIND NEW STEEL LAYER	BEAMC 128
C		BEAMC 129
	110 CONTINUE	BEAMC 130
	IF (B.GT.0.) GO TO 120	BEAMC 131
	M=0	BEAMC 132
	RETURN	BEAMC 133
	120 CONTINUE	BEAMC 134
C		BEAMC 135
C	COMPUTE EFFECTIVE CONCRETE WIDTH	BEAMC 136
C		BEAMC 137
	TW=A/B	BEAMC 138
C		BEAMC 139
C	COMPUTE EFFECTIVE CONCRETE AREA SYMMETRICAL ABOUT STEEL C.G.	BEAMC 140
C		BEAMC 141
	AE=2.*TW*TS	BEAMC 142

C		BEAMC143
C	FIND THE STEEL STRAIN AT ZERO CONCRETE STRAIN	BEAMC144
C		BEAMC145
	I=5*NPIX	BEAMC146
	K=5*NPKX	BEAMC147
	B=EIB(NEL)+CUB(NEL)*ZCRD(MLS)	BEAMC148
	A=DISPLA(K-4)-DISPLA(I-4)+(DISPLA(K)-DISPLA(I))*ZCRD(MLS)	BEAMC149
	A=A/XLNGTH+EPSPS(ML)	BEAMC150
	A=A+B	BEAMC151
	FSO=A-(EA+EB)*B/EA	BEAMC152
C		BEAMC153
C		BEAMC154
C	CHECK BEAM CRACK WIDTH FOR ELEMENT NEL	BEAMC155
C		BEAMC156
	130 CONTINUE	BEAMC157
	IF (ESX(ML,NEL).NE.0.) RETURN	BEAMC158
C		BEAMC159
C	FIND DATA VALUES FOR CRITICAL ELEMENT	BEAMC160
C		BEAMC161
	REWIND 15	BEAMC162
	REWIND 23	BEAMC163
	REWIND 25	BEAMC164
	REWIND 27	BEAMC165
	DO 140 I=1,NEL	BEAMC166
	READ (15) SIBXXA	BEAMC167
	READ (23) SIBXXC	BEAMC168
	READ (25) EPSPS	BEAMC169
	READ (27) XLNGTH,GKEIX,NPIX,NPKX,ITYPE,ASXLR,AILR,ZCRD,TSHEAR	BEAMC170
	140 CONTINUE	BEAMC171
C		BEAMC172
C	CHECK FOR TENSILE STRAIN	BEAMC173
C		BEAMC174
	I=5*NPIX	BEAMC175
	K=5*NPKX	BEAMC176
	A=DISPLA(K-4)-DISPLA(I-4)	BEAMC177
	B=DISPLA(K)-DISPLA(I)	BEAMC178
C		BEAMC179
	EA=(A+B*ZCRD(MLS))/XLNGTH+EPSPS(MLS)	BEAMC180
	EA=EA+EIB(NEL)+CUB(NEL)*ZCRD(MLS)	BEAMC181
	IF ((EA-FSO).LT.0.0) RETURN	BEAMC182
C		BEAMC183
	EA=(A+B*ZCRD(ML))/XLNGTH+EPSPS(ML)	BEAMC184
	EA=EA+EIB(NEL)+CUB(NEL)*ZCRD(ML)	BEAMC185
	IF (EA.LT.0.) RETURN	BEAMC186
C		BEAMC187
C	FIND NEUTRAL AXIS	BEAMC188
C		BEAMC189
	I=0	BEAMC190
	150 I=I+1	BEAMC191
	IF (I.GT.NULAYB) M=0	BEAMC192
	IF (I.GT.NULAYB) RETURN	BEAMC193
	IF (I.EQ.ML.OR.ITYPE(I).GT.KC.OR.ASXLR(I).EQ.0.0) GO TO 150	BEAMC194
C		BEAMC195
	EB=(A+B*ZCRD(I))/XLNGTH+EPSPS(I)	BEAMC196
	EB=EB+EIB(NEL)+CUB(NEL)*ZCRD(I)	BEAMC197
	IF (EA.EQ.EB) EA=EA+EA*1.0E-06	BEAMC198
	ZNA=ZCRD(ML)-EA*(ZCRD(ML)-ZCRD(I))/(EA-EB)	BEAMC199
C		BEAMC200



C	COMPUTE CRACK WIDTH	BEAMC201
C	H2=ZCRD(ML)+ASXLR(ML)/TSHEAR(ML)/2.	BEAMC202
	H2=H2-ZNA	BEAMC203
	IF (H2.EQ.TS) H2=H2+H2*1.E-06	BEAMC204
	R=1.+TS/(H2-TS)	BEAMC205
	RM=M	BEAMC206
	EA=(A+B*ZCRD(MLS))/XLNGTH+EPSPS(MLS)	BEAMC207
	EA=EA+EIB(NEL)+GUB(NEL)*ZCRD(MLS)	BEAMC208
	B=(EA-FSO)*ESX(MLS,NEL)	BEAMC209
	WC=0.091*1.6*R*(TG*AE/RM)**(1.0/3.0)*B/1000.	BEAMC210
C	RETURN	BEAMC211
	END	BEAMC212
	SUBROUTINE QPRINT (HDISP,HFORC,RDISP,RFORC,RDFPR,NSLAYR,NULAY,NCNS	QPRINT 2
	1,NUMEL,AFT,AST,AS1,NA1,NUMNP,IO,FORCEA,DISPLA)	QPRINT 3
	DIMENSION FORCEA(NA1), DISPLA(NA1)	QPRINT 4
	DIMENSION AFT(NCNS), AST(NCNS), AS1(NUMEL,NCNS)	QPRINT 5
	COMMON /SMX/ OLOAD,PMAX	QPRINT 6
C	COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	QPRINT 7
	COMMON /PLTCD/ PLTCRD	QPRINT 8
	COMMON /SIGNIF/ NSIGNI(2)	QPRINT 9
C	IF (RDFPR.NE.3HYES) GO TO 70	QPRINT 10
	WRITE (IO,10)	QPRINT 11
	10 FORMAT (//,1H0,10X,*APPLIED NODAL POINT FORCES IN KIPS AND IN-KIPS	QPRINT 12
	1*,//)	QPRINT 13
	WRITE (IO,20)	QPRINT 14
	20 FORMAT (1H0,5X,*NODAL POINTS*,8X,*U-LOAD*,11X,*V-LOAD*,12X,*W-LOAD	QPRINT 15
	1*,12X,*MX-LOAD*,11X,*MY-LOAD*,//)	QPRINT 16
	WRITE (IO,30) (M,FORCEA(5*M-4),FORCEA(5*M-3),FORCEA(5*M-2),FORCEA(	QPRINT 17
	15*M-1),FORCEA(5*M),M=1,NUMNP)	QPRINT 18
	30 FORMAT (6X,I5,2X,5F18.5)	QPRINT 19
C	WRITE (IO,40)	QPRINT 20
	40 FORMAT (//,1H0,10X,*NODAL POINT DISPLACEMENTS IN INCHES AND RADIAN	QPRINT 21
	1S*,//)	QPRINT 22
	WRITE (IO,50)	QPRINT 23
	50 FORMAT (1H0,5X,*NODAL POINTS*,8X,*U-DISP*,11X,*V-DISP*,12X,*W-DISP	QPRINT 24
	1*,12X,*MX-DISP*,11X,*MY-DISP*,//)	QPRINT 25
	WRITE (IO,60) (M,DISPLA(5*M-4),DISPLA(5*M-3),DISPLA(5*M-2),DISPLA(	QPRINT 26
	15*M-1),DISPLA(5*M),M=1,NUMNP)	QPRINT 27
	60 FORMAT (6X,I5,2X,5F18.5)	QPRINT 28
C	70 CONTINUE	QPRINT 29
C	PRINT REFERENCE VALUES	QPRINT 30
C	LL=NSIGNI(2)	QPRINT 31
	A=DISPLA(LL)/HDISP	QPRINT 32
	WRITE (IO,80) A	QPRINT 33
	80 FORMAT (1H0,//,20X,*DISPLACEMENT/REFERENCE DISPLACEMENT =*,F11.5)	QPRINT 34
C	LL=NSIGNI(1)	QPRINT 35
	A=FORCEA(LL)/HFORC	QPRINT 36
	WRITE (IO,90) A	QPRINT 37
	90 FORMAT (1H0,19X,*FORCE/REFERENCE FORCE =*,F11.5)	QPRINT 38
		QPRINT 39
		QPRINT 40
		QPRINT 41
		QPRINT 42
		QPRINT 43
		QPRINT 44
		QPRINT 45

C		QPRINT46
C	FIND SUM OF VERTICAL NODAL POINT LOADS	QPRINT47
C		QPRINT48
	A=0.	QPRINT49
	DO 100 I=1,NUMNP	QPRINT50
	J=I*5-2	QPRINT51
	A=FORCEA(J)+A	QPRINT52
100	CONTINUE	QPRINT53
	WRITE (IO,110) A	QPRINT54
	110 FORMAT (1H0,19X,*SUM OF NODAL POINT W-LOADS                    =*,F11.4)	QPRINT55
C		QPRINT56
C	SET UP HISTOGRAM	QPRINT57
C		QPRINT58
	IF (ABS(A/OLOAD).GT.PMAX) NVAL(4)=3HYES	QPRINT59
	FVAL(7)=A/OLOAD	QPRINT60
C		QPRINT61
	REWIND 32	QPRINT62
	DO 150 N=1,NUMEL	QPRINT63
	READ (32) AFT,AST	QPRINT64
C		QPRINT65
	DO 130 M=1,NULAY	QPRINT66
	IF (AS1(N,M).EQ.999.0) GO TO 130	QPRINT67
	IF (FVAL(1).NE.-999.0) GO TO 120	QPRINT68
	IF (AFT(M).EQ.5HCRACK.OR.AST(M).EQ.5HCRACK) FVAL(1)=A	QPRINT69
120	IF (FVAL(2).NE.-999.0) GO TO 130	QPRINT70
	IF (AFT(M).EQ.5HCRUSH.OR.AST(M).EQ.5HCRUSH) FVAL(2)=A	QPRINT71
130	CONTINUE	QPRINT72
C		QPRINT73
	IF (NSLAYR.EQ.0) GO TO 150	QPRINT74
	I=NULAY+1	QPRINT75
	DO 140 M=I,NCNS	QPRINT76
	IF (AS1(N,M).EQ.999.0) GO TO 140	QPRINT77
	IF (FVAL(3).NE.-999.0) GO TO 140	QPRINT78
	IF (AFT(M).EQ.5HYIELD) FVAL(3)=A	QPRINT79
140	CONTINUE	QPRINT80
C		QPRINT81
	150 CONTINUE	QPRINT82
C		QPRINT83
C		QPRINT84
	IF (FVAL(4).EQ.+999.0) FVAL(4)=A	QPRINT85
	IF (FVAL(5).EQ.+999.0) FVAL(5)=A	QPRINT86
	IF (FVAL(6).EQ.+999.0) FVAL(6)=A	QPRINT87
C		QPRINT88
C	WRITE PLOT CARDS	QPRINT89
C		QPRINT90
	IF (PLTCRD.NE.3HYES) RETURN	QPRINT91
	WRITE (8,160) (M,FORCEA(5*M-2),DISPLA(5*M-2),M=1,NUMNP)	QPRINT92
160	FORMAT (3(I2,E12.5,E12.5),2X)	QPRINT93
C		QPRINT94
	RETURN	QPRINT95
	END	QPRINT96
	SUBROUTINE PRISR8 (SIEXXC,IN,NULAY8,NBEAMX,IO)	PRISR8 2
C		PRISR8 3
C	THIS SUBROUTINE PRINTS BEAM STRESSES	PRISR8 4
C		PRISR8 5
	DIMENSION SIBXXC(NULAY8)	PRISR8 6
C		PRISR8 7
	REWIND IN	PRISR8 8

C	WRITE (IO,10) (ML,ML=1,NULAYE)	PRISRB 9
	10 FORMAT (1H0,5X,*BEAM*,/,1H ,5X,*ELEMENT/ LAYER =*,10(I6,5X),(/,22X	PRISRB10
	1,10(I6,5X)))	PRISRB12
	DO 40 N=1,N3EAMX	PRISRB13
	WRITE (IO,20)	PRISRB14
	20 FORMAT (1H )	PRISRB15
C	READ (IN) SIBXXC	PRISRB16
	WRITE (IO,30) N,(SIBXXC(ML),ML=1,NULAYB)	PRISRE17
	30 FORMAT (1H0,7X,I3,11X,10F11.3,(/,22X,10F11.3))	PRISRB18
	40 CONTINUE	PRISRB19
	WRITE (IO,20)	PRISRB20
	RETURN	PRISRB21
	END	PRISRB22
	SUBROUTINE STOTL (IC,NCNS,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC,SPHI,CSTOTL	STOTL 2
	1TXX,CTYY,CTXY,ETXX,ETYY,ETXY)	STOTL 3
C		STOTL 4
C		STOTL 5
C	THIS SUBROUTINE PRINTS X,Y AND PRINCIPAL STRAINS IN PLATE	STOTL 6
C		STOTL 7
C		STOTL 8
	DIMENSION CTXX(NUMEL), CTYY(NUMEL), CTXY(NUMEL), ETXX(NUMEL), ETYY	STOTL 9
	1(NUMEL), ETXY(NUMEL), SPHI(NSLAYR), CT(3), EE(3)	STOTL 10
C		STOTL 11
	REWIND 28	STOTL 12
C		STOTL 13
	WRITE (IO,10)	STOTL 14
	10 FORMAT (//)	STOTL 15
	WRITE (IO,20)	STOTL 16
	20 FORMAT (//10X,*TOTAL ACCUMULATED STRAIN IN PLATE LAYERS*,/)	STOTL 17
	WRITE (IO,10)	STOTL 18
	WRITE (IO,30)	STOTL 19
	30 FORMAT (//6X,4HN ML,9X,5HEXXOT,9X,5HEYYOT,7X,8H EXYOT,10X,2HE1,1	STOTL 20
	12X,2HE2,10X,6HTHETA1)	STOTL 21
	DO 70 N=1,NUMEL	STOTL 22
C		STOTL 23
	READ (28) DT,EX,EX,EX,EX,EX,EX	STOTL 24
C		STOTL 25
	CT(1)=CTXX(N)	STOTL 26
	CT(2)=CTYY(N)	STOTL 27
	CT(3)=CTXY(N)	STOTL 28
	WRITE (IO,10)	STOTL 29
	DO 70 ML=1,NCNS	STOTL 30
	CALL STRANO (N,ML,CT,EE,NULAY,NUMEL,NSLAYR,PZC,PDT,DT,SZC)	STOTL 31
	EX=ETXX(N)+EE(1)	STOTL 32
	EY=ETYY(N)+EE(2)	STOTL 33
	EXY=ETXY(N)+EE(3)	STOTL 34
	EXY=.5*EXY	STOTL 35
	IF (ML.GT.NULAY) GO TO 40	STOTL 36
	CALL PRNCIP (EX,EY,EXY,E1,E2,THETA)	STOTL 37
	GO TO 50	STOTL 38
	40 CONTINUE	STOTL 39
	THETA=SPHI(ML-NULAY)	STOTL 40
	CALL TRANG (EX,EY,EXY,THETA,E1,E2,E3)	STOTL 41
	50 CONTINUE	STOTL 42
	EXY=2.0*EXY	STOTL 43
	WRITE (IO,60) N,ML,EX,EY,EXY,E1,E2,THETA	STOTL 44

60	FORMAT (1H ,3X,I3,1X,I2,2X,6(E12.5,2X))	STOTL 45
70	CONTINUE	STOTL 46
	RETURN	STOTL 47
	END	STOTL 48
	SUBROUTINE PRISTR (SXXOT,SYOT,SXYOT,A,IN,NULAY,NUMEL,NCNS,NSLAYR,PRISTR,1SPHI,IO)	PRISTR 2
		PRISTR 3
C		PRISTR 4
C	PRINT SLAB STRESSES	PRISTR 5
C		PRISTR 6
	DIMENSION SXXOT(NCNS), SYOT(NCNS), SXYOT(NCNS), SPHI(NSLAYR), DUMPRISTR 7	PRISTR 8
	1MY(1)	PRISTR 9
C		PRISTR 10
	REWIND IN	PRISTR 11
C		PRISTR 12
	DO 50 N=1,NUMEL	PRISTR 13
	READ (IN) SXXOT,SYOT,SXYOT	PRISTR 14
	WRITE (IO,10)	PRISTR 15
10	FORMAT (//)	PRISTR 16
	DO 50 ML=1,NCNS	PRISTR 17
	SX=SXXOT(ML)	PRISTR 18
	SY=SYOT(ML)	PRISTR 19
	SXY=SXYOT(ML)*A	PRISTR 20
	IF (ML.GT.NULAY) GO TO 20	PRISTR 21
	CALL PRNCIP (SX,SY,SXY,S1,S2,THETA1)	PRISTR 22
	GO TO 30	PRISTR 23
20	CONTINUE	PRISTR 24
	THETA1=SPHI(ML-NULAY)	PRISTR 25
	CALL TRANG (SX,SY,SXY,THETA1,S1,S2,S3)	PRISTR 26
30	CONTINUE	PRISTR 27
	WRITE (IO,40) N,ML,SX,SY,SXY,S1,S2,THETA1	PRISTR 28
40	FORMAT (1H ,3X,I3,1X,I2,2X,6(F12.5,2X))	PRISTR 29
50	CONTINUE	PRISTR 30
	RETURN	PRISTR 31
	END	PRISTA 2
	SUBROUTINE PRISTA (EIXX,EIYY,EIXY,IO,NUMEL,NULAY,NSLAYR,SPHI)	PRISTA 3
C		PRISTA 4
C	PRINT CURVATURES	PRISTA 5
C	PRINT INPLANE STRAIN	PRISTA 6
C		PRISTA 7
	DIMENSION EIXX(NUMEL), EIYY(NUMEL), EIXY(NUMEL), SPHI(NSLAYR), DUMPRISTA 8	PRISTA 9
	1MY(1)	PRISTA 10
	WRITE (IO,10)	PRISTA 11
10	FORMAT (1H )	PRISTA 12
	DO 40 N=1,NUMEL	PRISTA 13
	WRITE (IO,10)	PRISTA 14
	EX=EIXX(N)	PRISTA 15
	EY=EIYY(N)	PRISTA 16
	EXY=EIXY(N)	PRISTA 17
	EXY=0.5*EXY	PRISTA 18
	ML=0	PRISTA 19
	CALL PRNCIP (EX,EY,EXY,E1,E2,THETA)	PRISTA 20
	WRITE (IO,20) N,ML,EX,EY,EXY,E1,E2,THETA	PRISTA 21
20	FORMAT (1H ,3X,I3,1X,I2,2X,6(E12.5,2X))	PRISTA 22
	IF (NSLAYR.EQ.0) GO TO 40	PRISTA 23
	DO 30 I=1,NSLAYR	PRISTA 24
	THETA=SPHI(ML-NULAY)	PRISTA 25
	CALL TRANG (EX,EY,EXY,THETA,E1,E2,E3)	
	ML=NULAY+I	

	WRITE (IO,20) N,ML,EX,EY,EXY,E1,E2,THETA	PRISTA26
	30 CONTINUE	PRISTA27
	40 CONTINUE	PRISTA28
	RETURN	PRISTA29
	END	PRISTA30
	SUBROUTINE BFORCE (NULAYB,NBEAMX,IO,DDUM,RDFPR,IST,ASXLR,SIBXXA,ZCRD,DDUM,1RD)	BFORCE 2
C		BFORCE 3
C	THIS SUBROUTINE COMPUTES MOMENTS IN BEAM ELEMENTS AND IN-	BFORCE 4
C	PLANE FORCES.	BFORCE 5
C		BFORCE 6
	DIMENSION ASXLR(NULAYB), SIBXXA(NULAYB), ZCRD(NULAYB), DDUM(NULAYB	BFORCE 7
	1), DUMMY(1)	BFORCE 8
	COMMON /PLTCO/ PLTCRD	BFORCE 9
C		BFORCE10
		BFORCE11
C		BFORCE12
	REWIND 27	BFORCE13
	REWIND IST	BFORCE14
C		BFORCE15
	IF (RDFPR.EQ.3HYES) WRITE (IO,10)	BFORCE16
	10 FORMAT (1H0,/,9X,*INTERNAL MOMENTS (IN-KIPS) AND NORMAL FORCES (KB	BFORCE17
	1IPS) IN BEAM ELEMENTS*,/,1H ,7X,*EL*,9X,*MB*,17X,*NB*,/)	BFORCE18
	DO 30 N=1,NBEAMX	BFORCE19
	SUM1=0.	BFORCE20
	SUM2=0.	BFORCE21
C		BFORCE22
	READ (IST) SIBXXA	BFORCE23
C		BFORCE24
	READ (27) XLNGTH,GKEIX,NPIX,NPKX,DDUM,ASXLR,DDUM,ZCRD,DDUM	BFORCE25
		BFORCE26
	DO 20 ML=1,NULAYB	BFORCE27
	ASX=ASXLR(ML)	BFORCE28
	ZCR=ZCRD(ML)	BFORCE29
	SIB=SIBXXA(ML)	BFORCE30
	SUM1=SUM1+ASX*SIB*ZCR	BFORCE31
	20 SUM2=SUM2+ASX*SIB	BFORCE32
	IF (RDFPR.EQ.3HYES) WRITE (IO,40) N,SUM1,SUM2	BFORCE33
	30 CONTINUE	BFORCE34
	40 FORMAT (5X,I5,F15.4,3X,F15.4)	BFORCE35
	RETURN	BFORCE36
	END	PFORCE 2
	SUBROUTINE PFORCE (IO,NUMEL,NCNS,NULAY,NSLAYR,RDFPR,IST,PZC,PDT,DT	PFORCE 3
	1,SDT,SZC,SXXOT,SYOT,SXYOT)	PFORCE 4
C		PFORCE 5
C	THIS SUBROUTINE COMPUTES INTERNAL MOMENTS AND NORMAL FORCES IN	PFORCE 6
C	THE SLAB ELEMENTS	PFORCE 7
C		PFORCE 8
	DIMENSION PDT(NULAY), PZC(NULAY), SDT(NSLAYR), SZC(NSLAYR), SXXOT(N	PFORCE 9
	1NCNS), SYOT(NCNS), SXYOT(NCNS), DUMMY(1)	PFORCE10
	COMMON /PLTCO/ PLTCRD	PFORCE11
C		PFORCE12
	REWIND IST	PFORCE13
	REWIND 28	PFORCE14
	IF (RDFPR.EQ.3HYES) WRITE (IO,10)	PFORCE15
	10 FORMAT (1H0,/,10X,*INTERNAL MOMENTS (IN-KIPS/IN) AND NORMAL FORCE	PFORCE16
	1S (KIPS/IN) IN THE SLAB*,/,1H ,7X,*EL*,14X,*MY*,13X,*MX *,13X,*MX	PFORCE17
	2Y*,13X,*NX *,13X,*NY *,13X,*NXY*)	PFORCE18
	DO 70 N=1,NUMEL	PFORCE19
	READ (28) DT,ELLA,ELLB,A,A,A,A	

READ (IST) SXXOT,SYYOT,SXYOT	PFORCE20
SUM1=0.	PFORCE21
SUM2=0.	PFORCE22
SUM3=0.	PFORCE23
SUM4=0.0	PFORCE24
SUM5=0.0	PFORCE25
SUM6=0.0	PFORCE26
ABAR=(PZC(1)-PDT(1)/2.0+PZC(NULAY)+PDT(NULAY)/2.0)	PFORCE27
ABAR=ABAR*DT/2.0	PFORCE28
DO 40 ML=1,NGNS	PFORCE29
IF (ML.GT.NULAY) GO TO 20	PFORCE30
A=PDT(ML)*DT*(PZC(ML)*DT-ABAR)	PFORCE31
AA=DT*PDT(ML)	PFORCE32
GO TO 30	PFORCE33
20 CONTINUE	PFORCE34
A=SDT(ML-NULAY)*(SZC(ML-NULAY)-ABAR)	PFORCE35
AA=SDT(ML-NULAY)	PFORCE36
30 CONTINUE	PFORCE37
SUM1=SUM1+SXXOT(ML)*A	PFORCE38
SUM2=SUM2+SYYCT(ML)*A	PFORCE39
SUM3=SUM3+SXYOT(ML)*A	PFORCE40
SUM4=SUM4+SXXOT(ML)*AA	PFORCE41
SUM5=SUM5+SYYOT(ML)*AA	PFORCE42
SUM6=SUM6+SXYOT(ML)*AA	PFORCE43
40 CONTINUE	PFORCE44
C	PFORCE45
C	PFORCE46
IF (RDFPR.NE.3HYES) GO TO 60	PFORCE47
WRITE (IO,50) N,SUM1,SUM2,SUM3,SUM4,SUM5,SUM6	PFORCE48
50 FORMAT (1H,4X,I5,6(5X,F11.3))	PFORCE49
60 CONTINUE	PFORCE50
70 CONTINUE	PFORCE51
RETURN	PFORCE52
END	PFORCE53
SUBROUTINE SHEAR (NLAY,NBMS,XLNGTH,SIBXXA,ASXLR,TSHEAR,ZCRD,ITYPE,SHEAR	SHEAR 2
1KC,NOBM,IO,AILR,RDFPR,XLEN,SIG,A,QQ,ITYP,TSHR,ZCR,Q,NBEAMX)	SHEAR 3
C	SHEAR 4
C THIS SUBROUTINE COMPUTES SHEARS AND PRINCIPAL STRESSES IN	SHEAR 5
C BEAM LAYERS	SHEAR 6
C	SHEAR 7
DIMENSION XLEN(3), SIG(NLAY,3), A(NLAY,3), ITYP(NLAY,3), TSHR(NLAYSHEAR	SHEAR 8
1,3), ZCR(NLAY,3), QQ(NLAY), C(NLAY), SIBXXA(NLAY), ASXLR(NLAY), TSSHEAR	SHEAR 9
2HEAR(NLAY), ZCRD(NLAY), ITYPE(NLAY), AILR(NLAY), DUMMY(1)	SHEAR 10
C	SHEAR 11
C	SHEAR 12
COMMON /HIST/ NVAL(10),IVAL(10),FVAL(10)	SHEAR 13
COMMON /SMX/ OLOAD,PMAX,SHEARM	SHEAR 14
REWIND 3	SHEAR 15
REWIND 27	SHEAR 16
REWIND 4	SHEAR 17
C	SHEAR 18
IF (RDFPR.EQ.3HYES) WRITE (IO,10)	SHEAR 19
10 FORMAT (/,1H0,5X,*BEAM ELEMENT SHEAR STRESS (KSI) AT TOP*)	SHEAR 20
C	SHEAR 21
DO 190 JOE=1,NOBM	SHEAR 22
C	SHEAR 23
C STRESS POSITIONS IN THE BEAM	SHEAR 24
C ELEMENT OF INTEREST	SHEAR 25

C		*						SHEAR	26
C		*						SHEAR	27
C	*	*	*	*	*	*		SHEAR	28
C	*	EL-L	*	EL-M	*	EL-R	*	SHEAR	29
C	*****							SHEAR	30
C	*	+	*	+	*	+	*	SHEAR	31
C	*	+	*	+	*	+	*	SHEAR	32
C	*	+	*	+	*	+	*	SHEAR	33
C	*	+SLB*SA	+	SB*SRA	+	*		SHEAR	34
							LAYER OF INTEREST		
C	*	+	*	+	*	+	*	SHEAR	35
C	*	+	*	+	*	+	*	SHEAR	36
C	*****							SHEAR	37
C								SHEAR	38
	DO	190	JJ=1,NBMS					SHEAR	39
		QE=0.						SHEAR	40
	QS=0.							SHEAR	41
	ZIE=0.							SHEAR	42
	ZIS=0.							SHEAR	43
	ZUE=0.							SHEAR	44
C								SHEAR	45
	IS=1							SHEAR	46
	IE=2							SHEAR	47
C								SHEAR	48
	IF (JJ.EQ.1) GO TO 30							SHEAR	49
C								SHEAR	50
	IS=2							SHEAR	51
	IE=1							SHEAR	52
C								SHEAR	53
C	MOVE	LAYER	PROPERTIES	FRM	MIDDLE	ELEMENT	TO	SHEAR	54
C	MOVE	LAYER	PROPERTIES	FRM	RIGHT	ELEMENT	TO	SHEAR	55
C								SHEAR	56
	DO	20	KK=1,2					SHEAR	57
		K=KK+1						SHEAR	58
	XLEN(KK)=XLEN(K)							SHEAR	59
	DO	20	I=1,NLAY					SHEAR	60
	A(I, KK)=A(I, K)							SHEAR	61
	SIG(I, KK)=SIG(I, K)							SHEAR	62
	TSHR(I, KK)=TSHR(I, K)							SHEAR	63
	ZCR(I, KK)=ZCR(I, K)							SHEAR	64
	ITYP(I, KK)=ITYP(I, K)							SHEAR	65
	20	CONTINUE						SHEAR	66
	30	CONTINUE						SHEAR	67
C								SHEAR	68
C	SET	UP	MIDDLE	ELEMENT	AND	RIGHT	ELEMENT	SHEAR	69
C	DO	NOT	READ	IF	LAST	BEAM	ELEMENT	SHEAR	70
C	OTHERWISE	SET	UP	RIGHT	ELEMENT			SHEAR	71
C								SHEAR	72
	IF (JJ.EQ.NBMS) GO TO 50							SHEAR	73
	DO	40	KK=1,IE					SHEAR	74
	READ (27)	XLNGTH,GKEIX,NPIX,NPKX,ITYPE,ASXLR,AILR,ZCRD,TSHEAR						SHEAR	75
	READ (3)	SIBXXA						SHEAR	76
	K=KK+IS							SHEAR	77
	XLEN(K)=XLNGTH							SHEAR	78
	DO	40	I=1,NLAY					SHEAR	79
	A(I, K)=ASXLR(I)							SHEAR	80
	SIG(I, K)=SIBXXA(I)							SHEAR	81
	TSHR(I, K)=TSHEAR(I)							SHEAR	82
	ZCR(I, K)=ZCRD(I)							SHEAR	83

	II=-1	SHEAR 84
	IF (AILR(I).EQ.0.0) II=KC+1	SHEAR 85
	ITYP(I,K)=II	SHEAR 86
C	40 CONTINUE	SHEAR 87
		SHEAR 88
C	50 CONTINUE	SHEAR 89
		SHEAR 90
C	DO 60 I=1,NLAY	SHEAR 91
	Q(I)=SIG(I,2)	SHEAR 92
C	60 CONTINUE	SHEAR 93
		SHEAR 94
C	DO 160 II=1,NLAY	SHEAR 95
		SHEAR 96
C	SHEAR IS COMPUTED AT CENTROID OF LAYER DEFINED BY ZCR MATRIX	SHEAR 97
C	SELECT CONCRETE LAYER I FROM ELEMENT JJ	SHEAR 98
		SHEAR 99
C	I=NLAY+1-II	SHEAR100
	J=2	SHEAR101
		SHEAR102
C	DOT NOT COUNT EMBEDDED LAYERS OR PRESTRESS LAYERS (IE AILR = 0.0)	SHEAR103
		SHEAR104
C	IF (ITYP(I,J).GT.KC) GO TO 150	SHEAR105
		SHEAR106
C	ZI=ZCR(I,J)	SHEAR107
	ZIS=ZIE	SHEAR108
	ZIE=ZI	SHEAR109
	KSTEEL=0	SHEAR110
	SA=0.	SHEAR111
	SB=0.	SHEAR112
	XM=XLEN(J)	SHEAR113
		SHEAR114
C	DO 80 KK=1,NLAY	SHEAR115
	K=NLAY+1-KK	SHEAR116
		SHEAR117
C	SEARCH ELEMENT OF INTEREST	SHEAR118
C	GET Z OF LAYER	SHEAR119
		SHEAR120
C	ZLY=ZCR(K,J)	SHEAR121
	AS=A(K,J)	SHEAR122
		SHEAR123
C	CHECK FOR EMBEDDED LAYER	SHEAR124
		SHEAR125
C	IF (ITYP(K,J).GT.KC) GO TO 70	SHEAR126
		SHEAR127
C	FIND UPPER DIST OF LAYER	SHEAR128
		SHEAR129
C	T=AS/TSHR(K,J)	SHEAR130
	ZU=ZLY+T/2.	SHEAR131
	ZUE=ZLY-T/2.	SHEAR132
		SHEAR133
C	IS ZU TOO SMALL	SHEAR134
		SHEAR135
C	IF (ZU.LE.ZI) GO TO 80	SHEAR136
		SHEAR137
C	FIND LOWER DIST OF LAYER	SHEAR138
		SHEAR139
C	ZL=ZLY-T/2.	SHEAR140
		SHEAR141



C	DO WE CONSIDER A PORTION OF CONCRETE LAYER K OF ELEMENT J	SHEAR142
C	IF (ZL.LT.ZI) AS=(ZU-ZI)*TSHR(K,J)	SHEAR143
	SA=SA+SIG(K,J)*AS	SHEAR144
	SB=SA	SHEAR145
	GO TO 80	SHEAR146
	70 CONTINUE	SHEAR147
C		SHEAR148
C	EMBEDDED LAYER	SHEAR149
C		SHEAR150
C	IF (ZLY.LT.ZI) GO TO 80	SHEAR151
	KSTEEL=KSTEEL+1	SHEAR152
	SA=SA+SIG(K,J)*AS	SHEAR153
	SB=SA	SHEAR154
	80 CONTINUE	SHEAR155
		SHEAR156
C		SHEAR157
C	MOVE TO LEFT ELEMENT	SHEAR158
C		SHEAR159
	XL=0.	SHEAR160
	SLB=0.	SHEAR161
	IF (JJ.EQ.1) GO TO 110	SHEAR162
	J=1	SHEAR163
	XL=XLEN(J)	SHEAR164
	ISTEEL=0	SHEAR165
C		SHEAR166
	DO 100 KK=1,NLAY	SHEAR167
	K=NLAY+1-KK	SHEAR168
	ZLY=ZGR(K,J)	SHEAR169
	AS=A(K,J)	SHEAR170
C		SHEAR171
C	CHECK FOR EMBEDDED LAYER	SHEAR172
C		SHEAR173
	IF (ITYP(K,J).GT.KC) GO TO 90	SHEAR174
C		SHEAR175
C	FIND UPPER DIST OF LAYER	SHEAR176
C		SHEAR177
	T=AS/TSHR(K,J)	SHEAR178
	ZU=ZLY+T/2.	SHEAR179
C		SHEAR180
C	IS ZU TOO SMALL	SHEAR181
C		SHEAR182
	IF (ZU.LE.ZI) GO TO 100	SHEAR183
C		SHEAR184
C	FIND LOWER DIST OF LAYER	SHEAR185
C		SHEAR186
	ZL=ZLY-T/2.	SHEAR187
C		SHEAR188
C	DO WE CONSIDER A PORTION OF CONCRETE LAYER K OF ELEMENT J	SHEAR189
C		SHEAR190
	IF (ZL.LT.ZI) AS=(ZU-ZI)*TSHR(K,J)	SHEAR191
	SLB=SLB+SIG(K,J)*AS	SHEAR192
	GO TO 100	SHEAR193
	90 CONTINUE	SHEAR194
C		SHEAR195
C	EMBEDDED LAYER	SHEAR196
C		SHEAR197
	ISTEEL=ISTEEL+1	SHEAR198
	IF (ISTEEL.GT.KSTEEL) GO TO 100	SHEAR199

	SLB=SLB+SIG(K,J)*AS	SHEAR200
	100 CONTINUE	SHEAR201
	110 CONTINUE	SHEAR202
C		SHEAR203
C	MOVE TO THE RIGHT	SHEAR204
C		SHEAR205
	XR=0.	SHEAR206
	SRA=0.	SHEAR207
	IF (JJ.EQ.NBMS) GO TO 140	SHEAR208
	J=3	SHEAR209
	XR=XLEN(J)	SHEAR210
	ISTEEL=0	SHEAR211
C		SHEAR212
	DO 130 KK=1,NLAY	SHEAR213
	K=NLAY+1-KK	SHEAR214
	ZLY=ZCR(K,J)	SHEAR215
	AS=A(K,J)	SHEAR216
C		SHEAR217
C	CHECK FOR EMBEDDED LAYER	SHEAR218
C		SHEAR219
	IF (ITYP(K,J).GT.KC) GO TO 120	SHEAR220
C		SHEAR221
C	FIND UPPER DIST OF LAYER	SHEAR222
C		SHEAR223
	T=AS/TSHR(K,J)	SHEAR224
	ZU=ZLY+T/2.	SHEAR225
C		SHEAR226
C	IS ZU TOO SMALL	SHEAR227
C		SHEAR228
	IF (ZU.LE.ZI) GO TO 130	SHEAR229
C		SHEAR230
C	FIND LOWER DIST OF LAYER	SHEAR231
C		SHEAR232
	ZL=ZLY-T/2.	SHEAR233
C		SHEAR234
C	DO WE CONSIDER A PORTION OF CONCRETE LAYER K OF ELEMENT J	SHEAR235
C		SHEAR236
	IF (ZL.LT.ZI) AS=(ZU-ZI)*TSHR(K,J)	SHEAR237
	SRA=SRA+SIG(K,J)*AS	SHEAR238
	GO TO 130	SHEAR239
	120 CONTINUE	SHEAR240
C		SHEAR241
C	EMBEDDED LAYER	SHEAR242
C		SHEAR243
	ISTEEL=ISTEEL+1	SHEAR244
	IF (ISTEEL.GT.KSTEEL) GO TO 130	SHEAR245
	SRA=SRA+SIG(K,J)*AS	SHEAR246
	130 CONTINUE	SHEAR247
	140 CONTINUE	SHEAR248
C		SHEAR249
C	CHEK IF ENDS OF BEAM	SHEAR250
C		SHEAR251
	ENDF=1.	SHEAR252
	IF (JJ.EQ.1 .OR. JJ.EQ.NBMS) ENDF = 2.	SHEAR253
	IF (JJ.EQ.1) SA=0.	SHEAR254
	IF (JJ.EQ.NBMS) SB=0.	SHEAR255
C		SHEAR256
C	COMPUTE LAYER SHEAR	SHEAR257

	C	AS=(SLB-SA)/(XL+XM)+(SB-SRA)/(XM+XR)	SHEAR258
		AS=AS*ENDF	SHEAR259
		QQ(I)=AS/TSHR(I,2)	SHEAR260
		QS=QE	SHEAR261
		QE=QQ(I)	SHEAR262
		GO TO 160	SHEAR263
	150	CONTINUE	SHEAR264
		QQ(I)=0.0	SHEAR265
	160	CONTINUE	SHEAR266
	C	SA=0.	SHEAR267
		WRITE (4) Q,(SA,K=1,NLAY),QQ	SHEAR268
		IF (RDFPR.NE.3HYES) GO TO 190	SHEAR269
	C		SHEAR270
	C	COMPUTE INTERFACE SHEAR BETWEEN BEAM AND SLAB	SHEAR271
	C		SHEAR272
		QE=(ZUE-ZIS)*(QE-QS)/(ZIE-ZIS)+QS	SHEAR273
		KK=JJ+NBMS*(JOE-1)	SHEAR274
		IF (ABS(QE).LT.SHEARM) GO TO 170	SHEAR275
		NVAL(2)=3HYES	SHEAR276
		IVAL(2)=KK	SHEAR277
		IVAL(3)=0	SHEAR278
	170	CONTINUE	SHEAR279
		WRITE (IO,180) KK,QE	SHEAR280
	180	FORMAT (1H ,14X,I3,9X,F9.4)	SHEAR281
	C		SHEAR282
	190	CONTINUE	SHEAR283
	C		SHEAR284
		IF (RDFPR.NE.3HYES) GO TO 210	SHEAR285
		WRITE (IO,200)	SHEAR286
	200	FORMAT (//,6X,4HN ML,9X,5HSXX ,9X,5HSZZ ,9X,5HSXZ ,10X,2HS1,13XSHEAR287	SHEAR287
		1,2HS2,10X,6HTheta1)	SHEAR288
	210	CONTINUE	SHEAR289
		CALL PRISTR (Q,SIBXXA,QQ,1.0,4,NLAY,NBEAMX,NLAY,0,SPHI,IO)	SHEAR290
		RETURN	SHEAR291
		END	SHEAR292
		OVERLAY (ECCP,1,0)	SHEAR293
		PROGRAM OVERL1	SHEAR294
	C		OVERL1 2
	C	THIS OVERLAY SETS UP SLAB DATA/TAPES AND COMPUTES FORCE VECTORS	OVERL1 3
	C		OVERL1 4
		COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	OVERL1 5
		COMMON /CB4/ IN,IO	OVERL1 6
		COMMON /CBS301/ COUPLE	OVERL1 7
		COMMON /CB310/ NELX,NSMAT,NELY	OVERL1 8
		COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	OVERL1 9
		COMMON /CB2/ NBEAMX,NIX(6)	OVERL1 10
		COMMON /PBEAM/ NPBEAM(20)	OVERL1 11
		COMMON /TU00/ TUO(8,12)	OVERL1 12
		COMMON /PDIM/ PDT(70)	OVERL1 13
		COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	OVERL1 14
		COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)	OVERL1 15
		COMMON /ICORES/ ICORE	OVERL1 16
		COMMON /STCHK/ STRAMS(30)	OVERL1 17
		COMMON /CRWS/ CRS(10)	OVERL1 18
		COMMON /PREDN/ PRED,IDX,IDY	OVERL1 19
		COMMON /SIGNIF/ NSIGNI(2)	OVERL1 20

	COMMON /OVER1/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13,N14,N15,OVERL123	OVERL123
	1N16,N17	OVERL124
	COMMON A(1)	OVERL125
C		OVERL126
C		OVERL127
	IF (ICORE.NE.0) GO TO 10	OVERL128
	ICORE=LOCF(A(1))+1	OVERL129
	GO TO 20	OVERL130
	10 CONTINUE	OVERL131
	CALL PINF (NANA,NUMEL,NUMNP,NA1,NULAY,NSLAYR,NELX,NELY,NSMAT,A(N1)	OVERL132
	1,A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),A(N9),A(N10),A(N11),A(N12),	OVERL133
	A(N13),A(N14),A(N15),A(N16))	OVERL134
	20 CONTINUE	OVERL135
	END	OVERL136
	SUBROUTINE PINF (NANA,NUMEL,NUMNP,NA1,NULAY,NSLAYR,NELX,NELY,NSMAT,PINF	2
	1,NPI,NPJ,NPK,NPL,ELLA,ELLB,NEQ1,NEQ2,NEQ3,NEQ4,DT,Q,FORCE,LILA,XSE	PINF 3
	2G,YSEG)	PINF 4
C		PINF 5
C	THIS SUBROUTINE READS AND COMPUTES DATA FOR SLAB	PINF 6
C		PINF 7
	COMMON /PDIM/ PDT(15),PZC(15),SEMOD(4),SIGMAP(4),SPRON(4),SPROM(4)	PINF 8
	1,NSTYPE(6),SDT(6),SZC(6),SPHI(6)	PINF 9
	COMMON /TU00/ TUO(8,12)	PINF 10
	COMMON /STCHK/ STRAMS(6),STTEMS(6),STCEMS(6),NKS(6),NHS(6)	PINF 11
	COMMON /CRWS/ ACR(2),CMIN(2),WCMAX(2),W(4)	PINF 12
	COMMON /PREDN/ PRED,IDX,IOY	PINF 13
	COMMON /SIGNIF/ NSIGNI(2)	PINF 14
	COMMON /PROPC/ V,EC,FC,ECOMP,FT,ET,PCOMP,ALMM,SALMM,EALMM,ALICC	PINF 15
	COMMON /PROPCB/ ZA,EZA,ZB,EZB	PINF 16
	COMMON /EDOWN/ EDOWNC,EDOWNT	PINF 17
	COMMON /CBS301/ COUPLE	PINF 18
	COMMON /CB4/ IN,IO	PINF 19
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NORM,NNN	PINF 20
	COMMON /CB2/ NBEAMX,NIX(6)	PINF 21
	COMMON /PBEAM/ NPBEAM(20)	PINF 22
	COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	PINF 23
	DIMENSION NPI(NANA),NPJ(NANA),NPK(NANA),NPL(NANA),ELLA(NUMEL),	PINF 24
	1 ELLB(NUMEL),DT(NUMEL),Q(NUMEL),NEQ1(NUMNP),NEQ2(NUMNP),NEQ3(PINF	PINF 25
	2NUMNP),NEQ4(NUMNP),FORCE(NA1),LILA(NA1),XSEG(NELX),YSEG(NELY)	PINF 26
	3,NZ(4)	PINF 27
C		PINF 28
	DIMENSION TIN(10)	PINF 29
	DIMENSION FTFIND(18)	PINF 30
	DIMENSION BARSZ(11)	PINF 31
	DATA (FTFIND(I),I=1,18)/0.,1.,2.,3.,4.,5.,6.,7.,1000.,.11,.11,.1,	PINF 32
	109,.09,.08,.08,.07,.07/	PINF 33
	DATA (BARSZ(I),I=1,11)/0.,.05,.11,.2,.31,.44,.6,.79,1.,1.27,1.56/	PINF 34
C		PINF 35
C	COUPLING BETWEEN THE INPLANE AND BENDING BEHAVIOR IS CONSIDERED	PINF 36
C		PINF 37
	COUPLE=3HYES	PINF 38
C		PINF 39
C	INITIALIZE	PINF 40
C		PINF 41
	DO 10 I=1,20	PINF 42
	10 NPBEAM(I)=0	PINF 43
C		PINF 44
	DO 20 I=1,8	PINF 45

DO 20 J=1,12	PINF	46
20 TUO(I,J)=0.	PINF	47
TUO(1,3)=1.0	PINF	48
TUO(3,6)=1.0	PINF	49
TUO(5,9)=1.0	PINF	50
TUO(7,12)=1.0	PINF	51
TUO(2,2)=-1.0	PINF	52
TUO(4,5)=-1.0	PINF	53
TUO(6,8)=-1.0	PINF	54
TUO(8,11)=-1.0	PINF	55
WRITE (IO,30)	PINF	56
30 FORMAT (1H1,////)	PINF	57
C	PINF	58
C CHECK FOR PREDISCRETIZATION	PINF	59
C	PINF	60
IF (PRED.NE.3HYES) GO TO 140	PINF	61
C	PINF	62
C FOR PREDISCRETIZATION	PINF	63
C	PINF	64
READ (IN,150) A	PINF	65
READ (IN,150) B	PINF	66
C	PINF	67
WRITE (IO,40) A,B	PINF	68
40 FORMAT (1H0,/,/6X,*LENGTH = *,F8.2,* (IN) WIDTH = *,F8.2,* (IN)*	PINF	69
1)	PINF	70
C	PINF	71
C CHECK TRANSVERSE MESH GENERATION CODE	PINF	72
C	PINF	73
IF (IDY.EQ.-100) GO TO 90	PINF	74
IF (IDY.EQ.-200) GO TO 100	PINF	75
C	PINF	76
N=1	PINF	77
K=0	PINF	78
J=NOBM-1	PINF	79
C=J	PINF	80
IF (IDY.GT.0) C=C+.5	PINF	81
RIDY=IABS(IDY)	PINF	82
NS=IABS(IDY)	PINF	83
IF (J.EQ.0) GO TO 70	PINF	84
C	PINF	85
DO 60 M=1,J	PINF	86
C	PINF	87
C SET STARTING NODE FOR BEAMS	PINF	88
C	PINF	89
NPBEAM(M)=(N-1)*(NELX+1)+1	PINF	90
C	PINF	91
DO 50 L=1,NS	PINF	92
C	PINF	93
C SET ELEMENT DIMENSIONS IN TRANSVERSE DIRECTION	PINF	94
C	PINF	95
K=K+1	PINF	96
YSEG(K)=B/C/RIDY	PINF	97
50 CONTINUE	PINF	98
C	PINF	99
N=N+NS	PINF	100
60 CONTINUE	PINF	101
C	PINF	102
70 CONTINUE	PINF	103

C		PINF 104
C	SET LAST BEAM	PINF 105
C		PINF 106
	NPBEAM(M)=(N-1)*(NELX+1)+1	PINF 107
	IF (IDY.LT.0) GO TO 110	PINF 108
C		PINF 109
C	SET IDY ADDITIONAL ROWS OF ELEMENTS	PINF 110
C		PINF 111
	DO 80 L=1,NS	PINF 112
	K=K+1	PINF 113
	YSEG(K)=B/C/RIDY/2.	PINF 114
	80 CONTINUE	PINF 115
	GO TO 110	PINF 116
	90 CONTINUE	PINF 117
C		PINF 118
C	IDY = -100	PINF 119
C		PINF 120
	YSEG(1)=0.3*B	PINF 121
	YSEG(2)=0.3*B	PINF 122
	YSEG(3)=0.20*B	PINF 123
	YSEG(4)=0.10*B	PINF 124
	YSEG(5)=0.05*B	PINF 125
	YSEG(6)=0.05*B	PINF 126
	GO TO 110	PINF 127
C		PINF 128
	100 CONTINUE	PINF 129
C		PINF 130
C	IDY = -200	PINF 131
C		PINF 132
	YSEG(1)=0.2*B	PINF 133
	YSEG(2)=0.15*B	PINF 134
	YSEG(3)=0.10*B	PINF 135
	YSEG(4)=0.05*B	PINF 136
	YSEG(5)=0.05*B	PINF 137
	YSEG(6)=0.10*B	PINF 138
	YSEG(7)=0.15*B	PINF 139
	YSEG(8)=0.20*B	PINF 140
	110 CONTINUE	PINF 141
C		PINF 142
C	SET ELEMENT DIMENSIONS IN LONGITUDINAL DIRECTION	PINF 143
C		PINF 144
	IF (IDX.EQ.-1) GO TO 120	PINF 145
	IF (IDX.EQ.-2) GO TO 130	PINF 146
C		PINF 147
C	IDX = -3	PINF 148
C		PINF 149
	XSEG(1)=0.3*A	PINF 150
	XSEG(2)=0.3*A	PINF 151
	XSEG(3)=0.2*A	PINF 152
	XSEG(4)=0.1*A	PINF 153
	XSEG(5)=0.05*A	PINF 154
	XSEG(6)=0.05*A	PINF 155
	GO TO 160	PINF 156
C		PINF 157
	120 CONTINUE	PINF 158
C		PINF 159
C	IDX = -1	PINF 160
C		PINF 161

XSEG(1)=0.1*A	PINF 162
XSEG(2)=0.1*A	PINF 163
XSEG(3)=0.05*A	PINF 164
XSEG(4)=0.05*A	PINF 165
XSEG(5)=0.1*A	PINF 166
XSEG(6)=0.1*A	PINF 167
XSEG(7)=0.2*A	PINF 168
XSEG(8)=0.3*A	PINF 169
GO TO 160	PINF 170
C 130 CONTINUE	PINF 171
C	PINF 172
C	PINF 173
C	PINF 174
C	PINF 175
C	PINF 176
XSEG(1)=0.2*A	PINF 177
XSEG(2)=0.15*A	PINF 178
XSEG(3)=0.1*A	PINF 179
XSEG(4)=0.05*A	PINF 180
XSEG(5)=0.05*A	PINF 181
XSEG(6)=0.1*A	PINF 182
XSEG(7)=0.15*A	PINF 183
XSEG(8)=0.2*A	PINF 184
GO TO 160	PINF 185
C	PINF 186
C	PINF 187
C	PINF 188
C	PINF 189
C	PINF 190
C	PINF 191
150 FORMAT (8F10.0)	PINF 192
C	PINF 193
C	PINF 194
C	PINF 195
C	PINF 196
C	PINF 197
170 FORMAT (/,1H0,5X,*ELEMENT LENGTHS IN THE X DIRECTION ARE*,* (IN)*, 1(/5X,8(3X,F10.4)))	PINF 198
WRITE (IO,180) (YSEG(N),N=1,NELY)	PINF 199
180 FORMAT (/,1H0,5X,*ELEMENT LENGTHS IN THE Y DIRECTION ARE*,* (IN)*, 1(/5X,8(3X,F10.4)))	PINF 200
C	PINF 201
C	PINF 202
C	PINF 203
C	PINF 204
C	PINF 205
C	PINF 206
DO 190 I=1,NUMNP	PINF 207
NEQ1(I)=NUMEL+1	PINF 208
NEQ2(I)=NUMEL+1	PINF 209
NEQ3(I)=NUMEL+1	PINF 210
NEQ4(I)=NUMEL+1	PINF 211
190 CONTINUE	PINF 212
DO 200 J=1,NELY	PINF 213
DO 200 I=1,NELX	PINF 214
M=I+(J-1)*NELX	PINF 215
ELLA(M)=XSEG(I)/2.0	PINF 216
ELLB(M)=YSEG(J)/2.0	PINF 217
NPI(M)=I+(J-1)*(NELX+1)	PINF 218
NPK(M)=NPI(M)+1	PINF 219
NPJ(M)=NPI(M)+NELX+1	PINF 219

	NPL(M)=NPI(M)+NELX+2	PINF 220
	NEQ1(NPI(M))=M	PINF 221
	NEQ2(NPK(M))=M	PINF 222
	NEQ3(NPL(M))=M	PINF 223
	NEQ4(NPJ(M))=M	PINF 224
200	CONTINUE	PINF 225
	J=NUMNP+1	PINF 226
	I=NUMEL+1	PINF 227
	NPI(I)=J	PINF 228
	NPJ(I)=J	PINF 229
	NPK(I)=J	PINF 230
	NPL(I)=J	PINF 231
C		PINF 232
C		PINF 233
	DO 210 I=1,NA1	PINF 234
210	LILA(I)=0	PINF 235
C		PINF 236
C	READ ELEMENT THICKNESS	PINF 237
C		PINF 238
	WRITE (IO,220)	PINF 239
220	FORMAT (1H )	PINF 240
230	READ (IN,240) NS,VALUE,NE,INC	PINF 241
240	FORMAT (I5,F10.0,I5,I5)	PINF 242
C		PINF 243
	IF (NE.EQ.0) NE=NS	PINF 244
	IF (INC.EQ.0) INC=1	PINF 245
	IF (NS.NE.0) GO TO 250	PINF 246
	NS=1	PINF 247
	NE=NUMEL	PINF 248
250	CONTINUE	PINF 249
	WRITE (IO,260) NS,VALUE,NE,INC	PINF 250
260	FORMAT (1H0,5X,*ELEMENT *,I3,* THICKNESS = *,F8.3,*(IN)*,* END ELE	PINF 251
	1MENT IS *,I3,* INCREMENT IS *,I3)	PINF 252
C		PINF 253
	IF (NE.GT.NUMEL) STOP 1	PINF 254
	IF (NS.GT.NUMEL.OR.NS.LE.0) STOP 1	PINF 255
C		PINF 256
	DO 270 I=NS,NE,INC	PINF 257
270	DT(I)=VALUE	PINF 258
	IF (NE.LT.NUMEL) GO TO 230	PINF 259
C		PINF 260
C		PINF 261
	REWIND 28	PINF 262
	DO 280 I=1,NUMEL	PINF 263
	WRITE (28) DT(I),ELLA(I),ELLB(I),NPI(I),NPJ(I),NPK(I),NPL(I)	PINF 264
280	CONTINUE	PINF 265
	WRITE (28) XSEG,YSEG	PINF 266
C		PINF 267
	REWIND 12	PINF 268
	REWIND 11	PINF 269
	DO 300 M=1,NUMNP	PINF 270
	N1=NEQ1(M)	PINF 271
	NZ(1) = NEQ1(M)	PINF 272
	N2=NEQ2(M)	PINF 273
	NZ(2) = NEQ2(M)	PINF 274
	N3=NEQ3(M)	PINF 275
	NZ(3) = NEQ3(M)	PINF 276
	N4=NEQ4(M)	PINF 277



NZ(4) = NEQ4(M)	PINF 278
WRITE (11) N1,N2,N3,N4,NPJ(N1),NPK(N1),NPL(N1),NPI(N2),NPJ(N2),NPL	PINF 279
1(N2),NPI(N3),NPJ(N3),NPK(N3),NPI(N4),NPK(N4),NPL(N4)	PINF 280
DO 290 NR=1,4	PINF 281
N=NZ(NR)	PINF 282
IF (N.EQ.NANA) GO TO 290	PINF 283
A=ELLA(N)	PINF 284
B=ELLB(N)	PINF 285
C=DT(N)	PINF 286
WRITE (12) C,A,B	PINF 287
290 CONTINUE	PINF 288
300 CONTINUE	PINF 289
C	PINF 290
C SET UP COUNTER USED IN DEAD LOAD FORCE VECTOR INPUT	PINF 291
C	PINF 292
IDD=0	PINF 293
310 IDD=IDD+1	PINF 294
C	PINF 295
WRITE (IO,220)	PINF 296
C	PINF 297
DO 320 I=1,NA1	PINF 298
320 FORCE(I)=0.	PINF 299
C	PINF 300
DO 330 I=1,NUMEL	PINF 301
330 Q(I)=0.0	PINF 302
C	PINF 303
340 CONTINUE	PINF 304
WRITE (IO,220)	PINF 305
C	PINF 306
C THIS ROUTINE IS USED TO READ LIVE LOAD FORCE VECTOR,	PINF 307
C BOUNDARY CONDITIONS, AND DEAD LOAD FORCE VECTOR	PINF 308
C	PINF 309
READ (IN,350) (TIN(I),I=1,10),NC	PINF 310
350 FORMAT (10A1,I5)	PINF 311
WRITE (IO,360) (TIN(I),I=1,10),NC	PINF 312
360 FORMAT (1H0,5X,*TITLE = *,10A1,* NUMBER OF CARDS = *,I3)	PINF 313
C	PINF 314
ILOC=5	PINF 315
DO 370 N=1,10	PINF 316
IF (TIN(N).EQ.1HE) GO TO 560	PINF 317
IF (TIN(N).EQ.1HU) ILOC=4	PINF 318
IF (TIN(N).EQ.1HV) ILOC=3	PINF 319
IF (TIN(N).EQ.1HW) ILOC=2	PINF 320
IF (TIN(N).EQ.1HX) ILOC=1	PINF 321
IF (TIN(N).EQ.1HY) ILOC=0	PINF 322
IF (TIN(N).EQ.1HQ) ILOC=-1	PINF 323
IF (TIN(N).EQ.1HA) ILOC=-3	PINF 324
IF (TIN(N).EQ.1HL) ILOC=-2	PINF 325
IF (TIN(N).EQ.1HT) ILOC=-4	PINF 326
I=N	PINF 327
IF (ILOC.LT.5) GO TO 390	PINF 328
370 CONTINUE	PINF 329
C	PINF 330
WRITE (IO,380)	PINF 331
380 FORMAT (1H0,5X,*NO TRANSLATION*,//)	PINF 332
STOP	PINF 333
C	PINF 334
390 CONTINUE	PINF 335

	IF (ILOC.LT.0) GO TO 410	PINF 336
	M=0	PINF 337
	DO 400 N=I,10	PINF 338
	IF (TIN(N).EQ.1HD) M=1	PINF 339
	IF (TIN(N).EQ.1HF) M=2	PINF 340
	IF (M.GT.0) GO TO 490	PINF 341
	400 CONTINUE	PINF 342
C		PINF 343
	WRITE (IO,380)	PINF 344
	STOP	PINF 345
C		PINF 346
C		PINF 347
	410 CONTINUE	PINF 348
	IF (ILOC.EQ.-3) GO TO 450	PINF 349
	IF (ILOC.EQ.-4) GO TO 430	PINF 350
C		PINF 351
C	LONGITUDINAL LINE OF SYMMETRY	PINF 352
C		PINF 353
	M=NELX+1	PINF 354
	N=NELY*M	PINF 355
	DO 420 J=1,M	PINF 356
	I=J+N	PINF 357
	LILA(I*5-3)=1	PINF 358
	LILA(I*5-1)=1	PINF 359
	420 CONTINUE	PINF 360
	IF (ILOC.EQ.-2) GO TO 450	PINF 361
	430 CONTINUE	PINF 362
C		PINF 363
C	TRANSVERSE LINE OF SYMMETRY	PINF 364
C		PINF 365
	N=NELX+1	PINF 366
	M=NELY+1	PINF 367
	DO 440 J=1,M	PINF 368
	I=J*N	PINF 369
	LILA(I*5-4)=1	PINF 370
	LILA(I*5)=1	PINF 371
	440 CONTINUE	PINF 372
	GO TO 470	PINF 373
	450 CONTINUE	PINF 374
C		PINF 375
C	SIMPLE SUPPORTS AT NODES OF FAR END	PINF 376
C		PINF 377
	N=NELX+1	PINF 378
	M=NELY+1	PINF 379
	DO 460 J=1,M	PINF 380
	I=J*N	PINF 381
	LILA(I*5-4)=1	PINF 382
	LILA(I*5-2)=1	PINF 383
	LILA(I*5-1)=1	PINF 384
	460 CONTINUE	PINF 385
	470 CONTINUE	PINF 386
C		PINF 387
C	SIMPLE SUPPORTS AT NODES OF NEAR END	PINF 388
C		PINF 389
	N=NELX+1	PINF 390
	M=NELY+1	PINF 391
	DO 480 J=1,M	PINF 392
	I=NELX*(J-1)+J	PINF 393

	LILA(I*5-2)=1	PINF 394
	LILA(I*5-1)=1	PINF 395
480	CONTINUE	PINF 396
	I=1+NELY*(NELX+1)	PINF 397
	LILA(I*5-3)=1	PINF 398
	GO TO 340	PINF 399
C		PINF 400
490	CONTINUE	PINF 401
	IF (M.EQ.2) GO TO 530	PINF 402
C		PINF 403
C	READ IN DISPLACEMENT BOUNDARY CONDITIONS	PINF 404
C	1 = FIXED, 0 = FREE	PINF 405
C		PINF 406
	IF (NC.EQ.0) GO TO 340	PINF 407
	DO 520 N=1,NC	PINF 408
	IF (NC.EQ.0) GO TO 340	PINF 409
	READ (IN,500) NS,IVALUE,NE,INC	PINF 410
500	FORMAT (4I5)	PINF 411
C		PINF 412
	IF (NE.EQ.0) NE=NS	PINF 413
	IF (INC.EQ.0) INC=1	PINF 414
C		PINF 415
	WRITE (IO,510) NS,IVALUE,NE,INC	PINF 416
510	FORMAT (1H0,5X,*NODE = *,I3,* VALUE = *,I3,* END NODE = *,I3,*	PINF 417
	1 INCREMENTED BY*,I3)	PINF 418
C		PINF 419
	IF (NS.GT.NUMNP.OR.NS.LE.0) STOP 1	PINF 420
	IF (NE.GT.NUMNP) STOP 1	PINF 421
C		PINF 422
C		PINF 423
	DO 520 I=NS,NE,INC	PINF 424
520	LILA(5*I-ILOC)=IVALUE	PINF 425
C		PINF 426
	GO TO 340	PINF 427
C		PINF 428
C	READ IN NODAL POINT FORCES	PINF 429
C	FORCES ARE IN KIPS AND IN-KIPS	PINF 430
C		PINF 431
530	CONTINUE	PINF 432
	IF (NC.EQ.0) GO TO 340	PINF 433
	DO 550 N=1,NC	PINF 434
	READ (IN,240) NS,VALUE,NE,INC	PINF 435
C		PINF 436
	IF (NE.EQ.0) NE=NS	PINF 437
	IF (INC.EQ.0) INC=1	PINF 438
C		PINF 439
	WRITE (IO,540) NS,VALUE,NE,INC	PINF 440
540	FORMAT (1H0,5X,*NODE = *,I3,* VALUE = *,F9.3,* END NODE = *,I3,*	PINF 441
	1 INCREMENTED BY *,I3)	PINF 442
C		PINF 443
	IF (NE.GT.NUMNP) STOP 1	PINF 444
	IF (NS.GT.NUMNP.OR.NS.LE.0) STOP 1	PINF 445
C		PINF 446
C		PINF 447
	DO 550 I=NS,NE,INC	PINF 448
550	FORCE(I*5-ILOC)=VALUE	PINF 449
C		PINF 450
	GO TO 340	PINF 451

C	560 CONTINUE	PINF 452
C		PINF 453
C		PINF 454
C	READ UNIFORM LINE LOAD (KIPS/IN)	PINF 455
C	WRITE (IO,220)	PINF 456
	WRITE (IO,570)	PINF 458
	570 FORMAT (1H0,5X,*UNIFORM LINE LOADS (KIPS/IN) ALONG X-DIRECTION NOD	PINF 459
	1AL LINES*)	PINF 460
	READ (IN,240) NC	PINF 461
	WRITE (IO,580) NC	PINF 462
	580 FORMAT (1H0,5X,*NUMBER OF UNIFORM LINE LOAD CARDS =*,I5)	PINF 463
C		PINF 464
	IF (NC.EQ.0) GO TO 610	PINF 465
	DO 600 K=1,NC	PINF 466
C		PINF 467
	READ (IN,240) NB,A	PINF 468
	WRITE (IO,590) NB,A	PINF 469
	590 FORMAT (1H0,5X,*STARTING NODE POINT IS *,I3,* LOAD = *,F9.4)	PINF 470
	IF (NB.GT.NUMNP) STOP 1	PINF 471
C		PINF 472
C	ADD TO FORCE VECTOR	PINF 473
C		PINF 474
	DO 600 M=1,NELX	PINF 475
	I=NB+M-1	PINF 476
	J=I+1	PINF 477
	B=XSEG(M)	PINF 478
	C=A*B/2.0	PINF 479
	FORCE(5*I-2)=FORCE(5*I-2)+C	PINF 480
	FORCE(5*J-2)=FORCE(5*J-2)+C	PINF 481
	C=A*B*B/12.0	PINF 482
	FORCE(5*I)=FORCE(5*I)-C	PINF 483
	FORCE(5*J)=FORCE(5*J)+C	PINF 484
	600 CONTINUE	PINF 485
	610 CONTINUE	PINF 486
C		PINF 487
C	READ IN THE DISTRIBUTED FORCES	PINF 488
C		PINF 489
	WRITE (IO,220)	PINF 490
	READ (IN,240) NC	PINF 491
	WRITE (IO,620) NC	PINF 492
	620 FORMAT (1H0,5X,*DISTRIBUTED FORCES (KIPS/SQ.IN) *,/,6X,*NUMBER OF	PINF 493
	1 LOAD CARDS*,14X,*=*,I5)	PINF 494
	IF (NC.EQ.0) GO TO 650	PINF 495
C		PINF 496
	DO 640 M=1,NC	PINF 497
	READ (IN,240) NS,A,NE,INC	PINF 498
C		PINF 499
	IF (NS.EQ.0) NE=NUMEL	PINF 500
	IF (NS.EQ.0) NS=1	PINF 501
	IF (NE.EQ.0) NE=NS	PINF 502
	IF (INC.EQ.0) INC=1	PINF 503
C		PINF 504
	WRITE (IO,630) NS,A,NE,INC	PINF 505
	630 FORMAT (1H0,5X,*ELEMENT = *,I3,* LOAD = *,F9.4,* END ELEMENT = *,	PINF 506
	1I3,* INCREMENTED BY *,I3)	PINF 507
C		PINF 508
	IF (NE.GT.NUMEL) STOP 1	PINF 509

	IF (NS.GT.NUMEL.OR.NS.LE.0) STOP 1	PINF 510
C		PINF 511
C		PINF 512
	DO 640 I=NS,NE,INC	PINF 513
	640 Q(I)=A	PINF 514
C		PINF 515
	650 CONTINUE	PINF 516
C		PINF 517
C	ADD DISTRIBUTED LOADS TO FORCE VECTOR	PINF 518
C		PINF 519
	CALL GELOAD (Q,NANA,NUMNP,NA1,NUMEL,NPI,NPJ,NPK,NPL,NEQ1,NEQ2,NEQ3	PINF 520
	1,NEQ4,ELLA,ELL3,FORCE)	PINF 521
C		PINF 522
C	READ CONCENTRATED LOADS LOCATED WITHIN ELEMENTS	PINF 523
C	COMPUTE NODAL POINT FORCES AND ADD TO FORCE VECTOR	PINF 524
C		PINF 525
C		PINF 526
	CALL COLOAD (NELX,NELY,NA1,NANA,IN,IO,XSEG,YSEG,FORCE,NPI,NPJ,NPK,	PINF 527
	1NPL)	PINF 528
C		PINF 529
C	PRINT FORCE VECTOR	PINF 530
C		PINF 531
	WRITE (IO,660)	PINF 532
	660 FORMAT (//,1HJ,20X,*NODAL POINT FORCES WITH SPECIFIED UNIFORM AND	PINF 533
	1CONCENTRATED LOADS*,//)	PINF 534
	WRITE (IO,670)	PINF 535
	670 FORMAT (1H0,20X,*CONCENTRATED NODAL POINT FORCES (KIPS AND IN-KIPSPINF	PINF 536
	1)*,///,6X,*NODAL POINT*,8X,*U-LOAD*,12X,*V-LOAD*,12X,*W-LOAD*,11X,PINF	PINF 537
	2*MX-LOAD*,11X,*MY-LOAD*,//)	PINF 538
C		PINF 539
	WRITE (IO,680) (M,FORCE(5*M-4),FORCE(5*M-3),FORCE(5*M-2),FORCE(5*M	PINF 540
	1-1),FORCE(5*M),M=1,NUMNP)	PINF 541
	680 FORMAT (1H ,6X,I5,1X,5F18.5)	PINF 542
C		PINF 543
	A=0.	PINF 544
	DO 690 M=1,NUMNP	PINF 545
	A=A+FORCE(5*M-2)	PINF 546
	690 CONTINUE	PINF 547
	WRITE (IO,700) A	PINF 548
	700 FORMAT (1H0,19X,*SUM OF NODAL POINT W-LOADS                    =*,F11.4)	PINF 549
C		PINF 550
	IF (IDD.EQ.2) GO TO 1110	PINF 551
C		PINF 552
C	PRINT ELEMENT TOPOLOGY AND DIMENSIONS	PINF 553
C		PINF 554
	WRITE (IO,220)	PINF 555
C		PINF 556
C		PINF 557
	WRITE (IO,710)	PINF 558
	710 FORMAT (1H0,//,20X,*ELEMENT TOPOLOGY *,//,6X,*NODAL POINT*,8X,*NEQ	PINF 559
	11*,8X,*NEQ2*,8X,*NEQ3*,8X,*NEQ4*,//)	PINF 560
	WRITE (IO,720) (M,NEQ1(M),NEQ2(M),NEQ3(M),NEQ4(M),M=1,NUMNP)	PINF 561
	720 FORMAT (1H ,6X,I5,5X,4I12)	PINF 562
C		PINF 563
	WRITE (IO,730)	PINF 564
	730 FORMAT (1H0,//,20X,*ELEMENT ARRAY*,///6X,14HELEMENT NUMBER,7X,1HI,	PINF 565
	114X,1HJ,14X,1HK,14X,1HL,//)	PINF 566
C		PINF 567

C	WRITE (IO,740) (N,NPI(N),NPJ(N),NPK(N),NPL(N),N=1,NUMEL)	PINF 568
	740 FORMAT (6X,I5,10X,I5,10X,I5,10X,I5,10X,I5)	PINF 569
	WRITE (IO,750)	PINF 570
	750 FORMAT (1H0,/,20X,*SLAB- ELEMENT GEOMETRY*,//)	PINF 571
	WRITE (IO,760)	PINF 572
	760 FORMAT (1H0,5X,*ELEMENT LENGTH A LENGTH B DT (DIMENSION	PINF 573
	1S ARE IN INCHES)*)	PINF 574
C		PINF 575
	WRITE (IO,770) (N,ELLA(N),ELLB(N),DT(N),N=1,NUMEL)	PINF 576
	770 FORMAT (1H0,4X,I5,2X,F10.4,2X,F10.4,2X,F10.4)	PINF 577
C		PINF 578
C		PINF 579
C	READ INFORMATION ABOUT PLATE LAYERS	PINF 580
C		PINF 581
C		PINF 582
	A=NULAY	PINF 583
	A=1./A	PINF 584
	B=-.5	PINF 585
	DO 780 I=1,NULAY	PINF 586
	PDT(I)=A	PINF 587
	B=B+A/2.	PINF 588
	PZC(I)=3	PINF 589
	B=B+A/2.	PINF 590
	780 CONTINUE	PINF 591
	WRITE (IO,790)	PINF 592
	790 FORMAT (1H0,/,20X,*SLAB- LAYER GEOMETRY*,//,6X,*LAYER THICKNES	PINF 593
	1S/DT*,5X,*CENTROID/DT*,//)	PINF 594
	WRITE (IO,800) (I,PDT(I),PZC(I),I=1,NULAY)	PINF 595
	800 FORMAT (1H0,6X,I3,6X,F10.6,7X,F10.6)	PINF 596
C		PINF 597
C	READ THE CONCRETE MATERIAL PROPERTIES	PINF 598
C		PINF 599
	WRITE (IO,810)	PINF 600
	810 FORMAT (1H0,//,20X,*SLAB- CONCRETE MATERIAL PROPERTIES (KSI)*)	PINF 601
	READ (IN,150) FC,FT,EC,EDOWNC,EDOWNT	PINF 602
	IF (FT.NE.0.) GO TO 840	PINF 603
	DO 820 I=1,9	PINF 604
	IF (FTFIND(I).GT.FC) GO TO 830	PINF 605
	820 CONTINUE	PINF 606
	830 FT=(FTFIND(9+I)-FTFIND(8+I))*(FC-FTFIND(I-1))*FC	PINF 607
	FT=FT/(FTFIND(I)-FTFIND(I-1))+FTFIND(8+I)*FC	PINF 608
	840 CONTINUE	PINF 609
	IF (EC.EQ.0.) EC=33.*(145.)*1.5*SQRT(FC*1000.)/1000.	PINF 610
	IF (EDOWNC.EQ.0.) EDOWNC=1000.	PINF 611
	IF (EDOWNT.EQ.0.) EDOWNT=800.	PINF 612
C		PINF 613
	WRITE (IO,850) FC,FT,EC,EDOWNC,EDOWNT	PINF 614
	850 FORMAT (//,8X,*UNIAXIAL CONCRETE COMPRESSIVE STRENGTH, FC =*,F9.	PINF 615
	14,//,8X,*DIRECT TENSILE STRENGTH, FT =*,F9.4,//,8X,*UNL	PINF 616
	2X,*INITIAL MODULUS, EC =*,F9.2,//,8X,*UNL	PINF 617
	30ADING MODULUS IN COMPRESSION =*,F9.2,//,8X,*UNLOADING	PINF 618
	4 MODULUS IN TENSION =*,F9.2,1X)	PINF 619
C		PINF 620
	V=.2	PINF 621
	ALICC=-15.	PINF 622
	ALMM=-19.2	PINF 623
	SALMM=.85	PINF 624
C		PINF 625

	ECOMP=2500.	PINF 626
	ET=FT/EC*1.0E+06	PINF 627
	EALMM=.80	PINF 628
C	PCOMP=1150.	PINF 629
	ZA=-.052	PINF 630
	EZA=.125	PINF 631
	ZB=-.203	PINF 632
	EZB=1.0	PINF 633
C	NSMAT=0	PINF 634
	IF (NSLAYR.EQ.0) GO TO 980	PINF 635
C		PINF 636
C	READ INFORMATION ABOUT STEEL LAYERS IN SLAB	PINF 637
C	READ NUMBER OF DIFFERENT MATERIALS	PINF 638
C		PINF 639
C	READ (IN,860) NSMAT	PINF 640
	860 FORMAT (7I5)	PINF 641
C		PINF 642
C	IF (NSMAT.GT.4) WRITE (IO,870)	PINF 643
	870 FORMAT (1H0,/,11X,*MAXIMUM NUMBER OF SLAB STEEL MATERIALS HAS BEEN	PINF 644
	1N EXCEEDED*,//)	PINF 645
	IF (NSMAT.GT.4) STOP 1	PINF 646
C		PINF 647
C	N=NSMAT	PINF 648
C		PINF 649
C	READ MATERIAL PROPERTIES	PINF 650
C		PINF 651
C	READ (IN,880) (I,SIGMAP(I),SEMOD(I),SPROM(I),SPRON(I),J=1,N)	PINF 652
	880 FORMAT (I5,4F10.0)	PINF 653
C		PINF 654
C	DO 890 I=1,NSMAT	PINF 655
	IF (SEMOD(I).EQ.0.) SEMOD(I)=29000.	PINF 656
	IF (SPROM(I).EQ.0.) SPROM(I)=.7	PINF 657
	IF (SPRON(I).EQ.0.) SPRON(I)=100.	PINF 658
	890 CONTINUE	PINF 659
C		PINF 660
C	WRITE INFORMATION ABOUT STEEL LAYERS	PINF 661
C		PINF 662
C	WRITE (IO,900)	PINF 663
	900 FORMAT (1H0,/,20X,*SLAB - STEEL LAYER MATERIAL PROPERTIES*)	PINF 664
	WRITE (IO,910)	PINF 665
	910 FORMAT (//7X,*MAT. TYPE      YOUNGS MODULUS      YIELD STRENGTH	PINF 666
	1    RAMBERG-M    RAMBERG-N*)	PINF 667
	WRITE (IO,920) (I,SEMOD(I),SIGMAP(I),SPROM(I),SPRON(I),I=1,N)	PINF 668
	920 FORMAT (11X,I1,10X,F8.2,* KSI*,9X,F8.2,* KSI*,9X,F5.2,5X,F5.1)	PINF 669
	WRITE (IO,930)	PINF 670
	930 FORMAT (1H0,/,20X,*SLAB - STEEL LAYER GEOMETRY*)	PINF 671
C		PINF 672
C	WRITE (IO,940)	PINF 673
	940 FORMAT (1H0,11X,*LAYER MAT. TYPE    BAR NO.    SPACING (IN)    THICKNES	PINF 674
	1S (IN)    CENTROID (IN)    ANGLE (DEGREES)*,//)	PINF 675
C		PINF 676
C	READ LAYER GEOMETRY	PINF 677
C		PINF 678
C	DO 970 I=1,NSLAYR	PINF 679
	READ (IN,950) J,NSTYPE(J),ISIZE,SPACE,SZC(J),SPHI(J)	PINF 680
	950 FORMAT (3I5,3F10.0)	PINF 681
		PINF 682
		PINF 683

	SDT(J)=SPACE	PINF 684
	IF (ISIZE.GT.C) SDT(J)=BARSZ(ISIZE)/SPACE	PINF 685
	IF (ISIZE.LE.0) SPACE=0.0	PINF 686
	WRITE (IO,960) J,NSTYPE(J),ISIZE,SPACE,SDT(J),SZC(J),SPHI(J)	PINF 687
960	FORMAT (1H ,11X,I5,3X,I5,5X,I5,6X,F9.4,6X,F9.5,7X,F9.5,6X,F8.3)	PINF 688
970	CONTINUE	PINF 689
980	CONTINUE	PINF 690
C		PINF 691
C	READ IN THE TERMINATION CHECKS	PINF 692
C		PINF 693
C	READ IN THE NUMBER OF CARDS TO SPECIFY THE CHECKS	PINF 694
C		PINF 695
	READ (IN,880) NC	PINF 696
	WRITE (IO,990) NC	PINF 697
990	FORMAT (1H0,/,6X,*NUMBER OF CARDS TO SPECIFY TERMINATION CHECKS =	PINF 698
	1 *,I2,/,)	PINF 699
	IF (NC.GT.(NSMAT+1)) STOP 1	PINF 700
C		PINF 701
	WRITE (IO,1000)	PINF 702
1000	FORMAT (1H0,/,21X,*TERMINATION CHECKS FOR THE SLAB LAYERS*,/,21X,*	PINF 703
	1(STRESS IN KSI, STRAIN IN PERCENT)*,/,6X,*MAT. NO. MAX STRAIN	PINF 704
	2AX TENSILE MAX COMP NUMBER OF CRACKED NUMBER OF*,/,31X,*STRES	PINF 705
	3S STRESS OR YIELDED LAYERS CRUSHED LAYERS*,/,)	PINF 706
C		PINF 707
	IF (NC.EQ.0) GO TO 1020	PINF 708
C		PINF 709
	READ (IN,1010) (J,STRAMS(J+1),STTEMS(J+1),STCEMS(J+1),NKS(J+1),NHS	PINF 710
	1(J+1),I=1,NC)	PINF 711
1010	FORMAT (I5,3F10.0,2I5)	PINF 712
C		PINF 713
1020	CONTINUE	PINF 714
C		PINF 715
C	PRINT OUT CHECKS AND SET UP DEFAULT VALUES	PINF 716
C		PINF 717
	N=NSMAT+1	PINF 718
	DO 1050 J=1,N	PINF 719
C		PINF 720
	L=J-1	PINF 721
	A=.0025	PINF 722
	B=6.0*SQRT(FC*1000.0)/1000.0	PINF 723
	C=0.8*FC	PINF 724
	I = 1	PINF 725
	K=1	PINF 726
	IF (J.EQ.1) GO TO 1030	PINF 727
C		PINF 728
	A=SIGMAP(L)/(SPRON(L)*SEMOD(L))	PINF 729
	B=SIGMAP(L)	PINF 730
	C=SIGMAP(L)	PINF 731
	I=1	PINF 732
	K=0	PINF 733
C		PINF 734
1030	CONTINUE	PINF 735
	A=A*100.	PINF 736
	IF (STRAMS(J).LT.0.0) STRAMS(J)=A	PINF 737
	IF (STTEMS(J).LT.0.0) STTEMS(J)=B	PINF 738
	IF (STCEMS(J).LT.0.0) STCEMS(J)=C	PINF 739
	IF (NKS(J).LT.0) NKS(J)=I	PINF 740
	IF (NHS(J).LT.0) NHS(J)=K	PINF 741



C	WRITE (IO,1040) L,STRAMS(J),STTEMS(J),STCEMS(J),NKS(J),NHS(J)	PINF 742
	1040 FORMAT (1H ,7X,I5,4X,F8.3,4X,F8.3,4X,F8.3,8X,I5,11X,I5)	PINF 743
C	STRAMS(J)=STRAMS(J)/100.	PINF 744
	1050 CONTINUE	PINF 745
C		PINF 746
C		PINF 747
C	SET UP RATIOS USED FOR THE BIAXIALLY STRESSED CONCRETE	PINF 748
C		PINF 749
C	STRAMS(1)=STRAMS(1)/0.0025	PINF 750
	STTEMS(1)=STTEMS(1)/FT	PINF 751
	STCEMS(1)=STCEMS(1)/FC	PINF 752
C		PINF 753
C		PINF 754
C	READ CRACK WIDTH DATA	PINF 755
C		PINF 756
C	READ FOR TOP SURFACE	PINF 757
C		PINF 758
C	READ (IN,150) ACR(2),CMIN(2),WCMAX(2)	PINF 759
	IF (WCMAX(2).LE.0.0) WCMAX(2)=0.004	PINF 760
	A=3HTOP	PINF 761
	B=3H	PINF 762
	WRITE (IO,1060) A,B,ACR(2),CMIN(2),WCMAX(2)	PINF 763
	1060 FORMAT (//,6X,A3,A3,* SURFACE OF SLAB*,* (IN)*,//,13X,*BAR SPACING	PINF 764
	1*,15X,*= *,F8.3,//,13X,*CONCRETE COVER*,12X,*= *,F8.3,//,13X,*MAX	PINF 765
	2ALLOWABLE CRACK WIDTH = *,F8.5,//)	PINF 766
C		PINF 767
C	READ FOR BOTTOM SURFACE	PINF 768
C		PINF 769
C	READ (IN,150) ACR(1),CMIN(1),WCMAX(1)	PINF 770
	IF (WCMAX(1).LE.0.0) WCMAX(1)=0.004	PINF 771
	A=3HBOT	PINF 772
	B=3HTOM	PINF 773
	WRITE (IO,1060) A,B,ACR(1),CMIN(1),WCMAX(1)	PINF 774
	ACR(2)=SQRT (ACR(2)**2/4.+CMIN(2)**2)	PINF 775
	ACR(1)=SQRT (ACR(1)**2/4.+CMIN(1)**2)	PINF 776
C		PINF 777
C	FIND REF NODE	PINF 778
C		PINF 779
C	IF (NSIGNI(1).NE.0.AND.NSIGNI(2).NE.0) GO TO 1090	PINF 780
	DO 1070 M=1,5	PINF 781
	DO 1070 I=1,NUMNF	PINF 782
	J=(I-1)*5+M	PINF 783
	IF (FORCE(J).NE.0.0.AND.LILA(J).EQ.0) GO TO 1080	PINF 784
	1070 CONTINUE	PINF 785
	J=3	PINF 786
	1080 NSIGNI(1)=J	PINF 787
	NSIGNI(2)=J	PINF 788
	1090 CONTINUE	PINF 789
C		PINF 790
	REWIND 29	PINF 791
	WRITE (29) FORCE,LILA	PINF 792
	REWIND 5	PINF 793
	WRITE (5) XSEG,YSEG	PINF 794
	IF (IDEAD.EQ.3HYES) WRITE (IO,1100)	PINF 795
	1100 FORMAT (1H0,//,20X,*DEAD LOAD FORCES*)	PINF 796
	IF (IDEAD.EQ.3HYES) GO TO 310	PINF 797
		PINF 798
		PINF 799

	GO TO 1120	PINF 800
1110	CONTINUE	PINF 801
	WRITE (29) FORCE	PINF 802
C		PINF 803
1120	CONTINUE	PINF 804
C		PINF 805
C	PRINT SUMMARY OF BOUNDARY CONDITIONS	PINF 806
C		PINF 807
	WRITE (10,1130)	PINF 808
1130	FORMAT (1H0, //, 20X, *BOUNDARY CONDITIONS*, //)	PINF 809
C		PINF 810
	I=0	PINF 811
	J=0	PINF 812
	DO 1140 M=1, NUMNP	PINF 813
	L=5*M-4	PINF 814
	IF (LILA(L).NE.0) I=I+1	PINF 815
	IF (LILA(L).NE.0) NEQ1(I)=M	PINF 816
	L=L+1	PINF 817
	IF (LILA(L).NE.0) J=J+1	PINF 818
	IF (LILA(L).NE.0) NEQ2(J)=M	PINF 819
1140	CONTINUE	PINF 820
	IF (I.EQ.0) NEQ1(1)=0	PINF 821
	IF (J.EQ.0) NEQ2(1)=0	PINF 822
	IF (I.EQ.0) I=1	PINF 823
	IF (J.EQ.0) J=1	PINF 824
C		PINF 825
C		PINF 826
	WRITE (10,1150) (NEQ1(M), M=1, I)	PINF 827
1150	FORMAT (1H0, 5X, * U CONSTRAINED NODES ARE *, 10(2X, I3), (/, 5X, 26X, 10(12X, I3)))	PINF 828
C		PINF 829
	WRITE (10,220)	PINF 830
	WRITE (10,1160) (NEQ2(M), M=1, J)	PINF 831
1160	FORMAT (1H0, 5X, * V CONSTRAINED NODES ARE *, 10(2X, I3), (/, 5X, 26X, 10(12X, I3)))	PINF 833
C		PINF 834
	I=0	PINF 836
	J=0	PINF 837
	N=0	PINF 838
	DO 1170 M=1, NUMNP	PINF 839
	L=5*M-2	PINF 840
	IF (LILA(L).NE.0) I=I+1	PINF 841
	IF (LILA(L).NE.0) NEQ1(I)=M	PINF 842
	L=L+1	PINF 843
	IF (LILA(L).NE.0) J=J+1	PINF 844
	IF (LILA(L).NE.0) NEQ2(J)=M	PINF 845
	L=L+1	PINF 846
	IF (LILA(L).NE.0) N=N+1	PINF 847
	IF (LILA(L).NE.0) NEQ3(N)=M	PINF 848
1170	CONTINUE	PINF 849
	IF (I.EQ.0) NEQ1(1)=0	PINF 850
	IF (J.EQ.0) NEQ2(1)=0	PINF 851
	IF (N.EQ.0) NEQ3(1)=0	PINF 852
	IF (I.EQ.0) I=1	PINF 853
	IF (J.EQ.0) J=1	PINF 854
	IF (N.EQ.0) N=1	PINF 855
C		PINF 856
C		PINF 857

	WRITE (IO,220)	PINF 858
	WRITE (IO,1180) (NEQ1(M),M=1,I)	PINF 859
	1180 FORMAT (1H0,5X,* W CONSTRAINED NODES ARE *,10(2X,I3),(/,5X,26X,10(12X,I3)))	PINF 860
C		PINF 861
	WRITE (IO,220)	PINF 862
	WRITE (IO,1190) (NEQ2(M),M=1,J)	PINF 864
	1190 FORMAT (1H0,5X,*MX CONSTRAINED NODES ARE *,10(2X,I3),(/,5X,26X,10(12X,I3)))	PINF 865
		PINF 866
C		PINF 867
	WRITE (IO,220)	PINF 868
	WRITE (IO,1200) (NEQ3(M),M=1,N)	PINF 869
	1200 FORMAT (1H0,5X,*MY CONSTRAINED NODES ARE *,10(2X,I3),(/,5X,26X,10(12X,I3)))	PINF 870
		PINF 871
C		PINF 872
	IF (NBEAMX.EQ.0) RETURN	PINF 873
	WRITE (IO,1210)	PINF 874
	1210 FORMAT (1H1,////)	PINF 875
C		PINF 876
C	READ IN THE NUMBER OF BEAMS AND STARTING NODE POINTS	PINF 877
C		PINF 878
	WRITE (IO,220)	PINF 879
	WRITE (IO,1220) NOBM	PINF 880
	1220 FORMAT (1H0,5X,*NUMBER OF BEAMS =*,I3)	PINF 881
C		PINF 882
	WRITE (IO,220)	PINF 883
	NB=NELX+1	PINF 884
	DO 1240 I=1,NOBM	PINF 885
C		PINF 886
	N=NPBEAM(I)	PINF 887
	IF (PRED.NE.3HYES) READ (IN,240) N	PINF 888
	WRITE (IO,1230) I,N	PINF 889
	1230 FORMAT (1H0,5X,*BEAM NUMBER =*,I3,* STARTING NODE POINT =*,I3)	PINF 890
C		PINF 891
C	WRITE BOUNDARY CONDITIONS FOR BEAMS TO TAPE	PINF 892
C		PINF 893
	NPBEAM(I)=N	PINF 894
	WRITE (5) N	PINF 895
	DO 1240 J=1,NB	PINF 896
	M=N+J-1	PINF 897
	1240 WRITE (5) LILA(5*M-4),LILA(5*M-2),LILA(5*M)	PINF 898
C		PINF 899
C		PINF 900
	RETURN	PINF 901
	END	PINF 902
	SUBROUTINE GELOAD (Q,NANA,NUMNP,NA1,NUMEL,NPI,NPJ,NPK,NPL,NEQ1,NEQ2,NEQ3,NEQ4,ELLA,ELLB,FORCE)	GELOAD 2
C		GELOAD 3
		GELOAD 4
C	COMPUTE NODAL POINT FORCES FROM DISTRIBUTED LOADS	GELOAD 5
C	ADD RESULTING NODAL POINT FORCES TO THE FORCE VECTOR	GELOAD 6
C		GELOAD 7
C		GELOAD 8
	DIMENSION NPI(NANA), NPJ(NANA), NPK(NANA), NPL(NANA), NEQ1(NUMNP), NEQ2(NUMNP), NEQ3(NUMNP), NEQ4(NUMNP), Q(NUMEL), ELLA(NUMEL), ELLB(NUMEL), FORCE(NA1), NZ(4)	GELOAD 9
	1 NEQ2(NUMNP), NEQ3(NUMNP), NEQ4(NUMNP), Q(NUMEL), ELLA(NUMEL), ELLB(NUMEL), FORCE(NA1), NZ(4)	GELOAD 10
	DIMENSION FF(3), FFT(3)	GELOAD 11
		GELOAD 12
C		GELOAD 13
	COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)	GELOAD 14

C	SINA=PHI*ATAN(1.0)/45. SINA=SIN(SINA)	GEL0AD15 GEL0AD16 GEL0AD17
C	SET UP LOCATIONS AND INITIALIZE	GEL0AD18 GEL0AD19 GEL0AD20
C	DO 100 M=1,NUMNP NZ(1)=NEQ1(M) NZ(2)=NEQ2(M)	GEL0AD21 GEL0AD22 GEL0AD23
C	NZ(3)=NEQ3(M) NZ(4)=NEQ4(M) N1=NZ(1) N2=NZ(2) N3=NZ(3) N4=NZ(4)	GEL0AD24 GEL0AD25 GEL0AD26 GEL0AD27 GEL0AD28 GEL0AD29
C	Z1 = 0. Z2 = 0. Z3 = 0. Z4 = 0. X1 = 0. X2 = 0.	GEL0AD30 GEL0AD31 GEL0AD32 GEL0AD33 GEL0AD34 GEL0AD35
C	X3 = 0. X4 = 0. Y1 = 0. Y2 = 0. Y3 = 0. Y4 = 0.	GEL0AD36 GEL0AD37 GEL0AD38 GEL0AD39 GEL0AD40 GEL0AD41
C	ELEMENT NEQ1 NODE I CONTRIBUTES	GEL0AD42 GEL0AD43 GEL0AD44
C	N=N1 IF (N.EQ.NANA) GO TO 10 FAC=Q(N)*ELLA(N)*ELLB(N)*4.0	GEL0AD45 GEL0AD46 GEL0AD47
C	Z1=FAC/4.0 X1=-FAC*ELLB(N)/12.0 Y1=-FAC*ELLA(N)/12.0	GEL0AD48 GEL0AD49 GEL0AD50
C	10 CONTINUE	GEL0AD51 GEL0AD52
C	ELEMENT NEQ4 NODE J CONTRIBUTES	GEL0AD53 GEL0AD54
C	N=N4 IF (N.EQ.NANA) GO TO 20 FAC=Q(N)*ELLA(N)*ELLB(N)*4.0 Z4=FAC/4.0 X4=+FAC*ELLB(N)/12.0 Y4=-FAC*ELLA(N)/12.0	GEL0AD55 GEL0AD56 GEL0AD57 GEL0AD58 GEL0AD59 GEL0AD60
C	20 CONTINUE	GEL0AD61 GEL0AD62
C	ELEMENT NEQ2 NODE K CONTRIBUTES	GEL0AD63 GEL0AD64
C	N=N2 IF (N.EQ.NANA) GO TO 30 FAC=Q(N)*ELLA(N)*ELLB(N)*4.0 Z2=FAC/4.0 X2=-FAC*ELLB(N)/12.0 Y2=+FAC*ELLA(N)/12.0	GEL0AD65 GEL0AD66 GEL0AD67 GEL0AD68 GEL0AD69 GEL0AD70
C	30 CONTINUE	GEL0AD71 GEL0AD72

C	ELEMENT NEQ3 NODE L CONTRIBUTES	GELOAD73
C		GELOAD74
	N=N3	GELOAD75
	IF (N.EQ.NANA) GO TO 40	GELOAD76
	FAC=Q(N)*ELLA(N)*ELLB(N)*4.0	GELOAD77
	Z3=FAC/4.0	GELOAD78
	X3=+FAC*ELLB(N)/12.0	GELOAD79
	Y3=+FAC*ELLA(N)/12.0	GELOAD80
	40 CONTINUE	GELOAD81
C		GELOAD82
C	ADD TO FORCE VECTOR	GELOAD83
C		GELOAD84
	FF(1)=Z1+Z2+Z3+Z4	GELOAD85
	FF(2)=X1+X2+X3+X4	GELOAD86
	FF(3)=Y1+Y2+Y3+Y4	GELOAD87
C		GELOAD88
C		GELOAD89
	IF (ABS(PHI).EQ.90.) GO TO 80	GELOAD90
C		GELOAD91
C	TRANSFORM TO CARTESIAN	GELOAD92
C		GELOAD93
C		GELOAD94
	DO 60 I=1,3	GELOAD95
	SUM=0.	GELOAD96
	DO 50 J=1,3	GELOAD97
	50 SUM=SUM+A0(I,J)*FF(J)	GELOAD98
	60 FFT(I)=SUM*SINA	GELOAD99
C		GELOA100
	DO 70 I=1,3	GELOA101
	70 FF(I)=FFT(I)	GELOA102
C		GELOA103
	80 CONTINUE	GELOA104
C		GELOA105
C		GELOA106
	J=5*M-3	GELOA107
	DO 90 I=1,3	GELOA108
	90 FORCE(J+I)=FORCE(J+I)+FF(I)	GELOA109
C		GELOA110
	100 CONTINUE	GELOA111
	RETURN	GELOA112
	END	GELOA113
	SUBROUTINE FOVEC	GELOA114
	RETURN	GELOA115
	END	GELOA116
	SUBROUTINE COLOAD (NELX,NELY,NA1,NANA,IN,IO,XSEG,YSEG,FORCE,NPI,NPCOLOAD	2
	1J,NPK,NPL)	COLOAD 3
C		COLOAD 4
C	READ THE CONCENTRATED LOADS WITHIN THE ELEMENT, COMPUTE	COLOAD 5
C	THE RESULTING NODAL POINT LOADS, AND ADD TO	COLOAD 6
C	THE FORCE VECTOR	COLOAD 7
C		COLOAD 8
	DIMENSION XSEG(NELX), YSEG(NELY), FORCE(NA1), NPI(NANA), NPJ(NANA)	COLOAD 9
	1, NPK(NANA), NPL(NANA)	COLOAD10
	DIMENSION FF(12), FFT(12)	COLOAD11
C		COLOAD12
	COMMON /SKEW/ PHI,AU(2,2),A0(3,3),DS(3,3)	COLOAD13
C		COLOAD14
C		COLOAD15

	DIMENSION CB(12,12), CGB(12,12), PQ(12), NZ(4)	COLOAD16
C		COLOAD17
	EQUIVALENCE (ZI,PQ(1)), (XMI,PQ(2)), (YMI,PQ(3))	COLOAD18
	EQUIVALENCE (ZJ,PQ(4)), (XMJ,PQ(5)), (YMJ,PQ(6))	COLOAD19
	EQUIVALENCE (ZK,PQ(7)), (XMK,PQ(8)), (YMK,PQ(9))	COLOAD20
	EQUIVALENCE (ZL,PQ(10)), (XML,PQ(11)), (YML,PQ(12))	COLOAD21
	EQUIVALENCE (PQ,FF)	COLOAD22
C		COLOAD23
	DATA ((CB(I,J),I=1,12),J=1,12)/.250,-.375,.375,0.000,-.500,0.000,.500,0.000,-.375,.375,0.000,	COLOAD24
	1125,0.000,0.000,-.125,.125,.125,-.125,.125,-.125,0.000,.125,.125,0.000,	COLOAD25
	2.000,0.000,-.125,.125,0.000,-.125,-.125,.125,-.125,.125,.125,0.000	COLOAD26
	3,-.125,.125,0.000,0.000,-.125,0.000,.250,-.375,-.375,0.000,.500,0.000,	COLOAD27
	4000,.125,0.000,0.000,.125,-.125,-.125,.125,-.125,-.125,0.000,.125,	COLOAD28
	5,-.125,0.000,0.000,.125,.125,0.000,-.125,-.125,.125,.125,.125,-.125	COLOAD29
	6,0.000,-.125,-.125,0.000,0.000,.125,0.000,.250,.375,.375,0.000,.500	COLOAD30
	70,0.000,-.125,0.000,0.000,-.125,-.125,-.125,-.125,-.125,-.125,0.000	COLOAD31
	80,-.125,.125,0.000,0.000,.125,.125,0.000,.125,.125,.125,.125,-.125	COLOAD32
	9,.125,0.000,-.125,-.125,0.000,0.000,-.125,0.000,.250,.375,-.375,0.000	COLOAD33
	\$000,-.500,0.000,-.125,0.000,0.000,.125,.125,.125,.125,.125,-.125,	COLOAD34
	\$,000,-.125,-.125,0.000,0.000,-.125,.125,0.000,.125,.125,.125,-.125	COLOAD35
	\$,-.125,-.125,0.000,-.125,.125,0.000,0.000,.125,0.000/	COLOAD36
C		COLOAD37
C		COLOAD38
C	READ NUMBER OF CONCENTRATED LOAD CARDS	COLOAD39
C		COLOAD40
	WRITE (IO,10)	COLOAD41
	10 FORMAT (1H )	COLOAD42
	READ (IN,20) NC	COLOAD43
	20 FORMAT (I5)	COLOAD44
	WRITE (IO,30) NC	COLOAD45
	30 FORMAT (1H0,5X,*NUMBER OF CONCENTRATED LOAD CARDS =*,I5)	COLOAD46
	IF (NC.EQ.0) GO TO 180	COLOAD47
C		COLOAD48
	WRITE (IO,40)	COLOAD49
	40 FORMAT (1H0,5X,*LOAD CARD LOAD (KIPS) X-POSITION (IN) Y-POSITION (IN)*,//)	COLOAD50
		COLOAD51
C		COLOAD52
C	READ LOAD AND LOCATION	COLOAD53
C		COLOAD54
	DO 170 I=1,NC	COLOAD55
C		COLOAD56
	READ (IN,50) P,X,Y	COLOAD57
	50 FORMAT (8F10.0)	COLOAD58
	WRITE (IO,60) I,P,X,Y	COLOAD59
	60 FORMAT (1H ,11X,I3,5X,F9.4,5X,F8.2,10X,F8.2,5X,F8.2,5X,F8.2)	COLOAD60
C		COLOAD61
C	LOCATE ELEMENT	COLOAD62
C		COLOAD63
	DISTX=0.	COLOAD64
	DO 70 J=1,NELX	COLOAD65
	XL=XSEG(J)	COLOAD66
	DISTX=DISTX+XL	COLOAD67
	IF ((DISTX+.001).GE.X) GO TO 90	COLOAD68
	70 CONTINUE	COLOAD69
	WRITE (IO,80)	COLOAD70
	80 FORMAT (1H0,5X,20(1H*),*INCORRECT LOAD POSITION-ABORT FROM SUBROUTINE	COLOAD71
	LINE COLOAD*,//)	COLOAD72
	STOP 5	COLOAD73

90	CONTINUE	COLOAD74
	DISTY=0.	COLOAD75
	DO 100 K=1,NELY	COLOAD76
	YL=YSEG(K)	COLOAD77
	DISTY=DISTY+YL	COLOAD78
	IF ((DISTY+.001).GE.Y) GO TO 110	COLOAD79
100	CONTINUE	COLOAD80
	WRITE (IO,80)	COLOAD81
	STOP 5	COLOAD82
110	CONTINUE	COLOAD83
C		COLOAD84
C	FIND FIXED END FORCES	COLOAD85
C		COLOAD86
	A=XL*YL	COLOAD87
	XB=DISTX-X	COLOAD88
	YB=DISTY-Y	COLOAD89
	XA=XL-XB	COLOAD90
	YA=YL-YB	COLOAD91
C		COLOAD92
C	COMPUTE VERTICAL FORCE	COLOAD93
C		COLOAD94
	ZI=P*XB*YB/A	COLOAD95
	ZJ=P*XB*YA/A	COLOAD96
	ZK=P*XA*YB/A	COLOAD97
	ZL=P*XA*YA/A	COLOAD98
C		COLOAD99
C	COMPUTE MOMENTS AROUND X-AXIS	COLOA100
C		COLOA101
	XMI=-P*YB*YB*YA*XB/A/YL	COLOA102
	XMJ=+P*YA*YA*YB*XB/A/YL	COLOA103
	XMK=-P*YB*YB*YA*XA/A/YL	COLOA104
	XML=+P*YA*YA*YB*XA/A/YL	COLOA105
C		COLOA106
C	COMPUTE MOMENTS AROUND Y-AXIS	COLOA107
C		COLOA108
	YMI=-P*XB*XB*XA*YB/A/XL	COLOA109
	YMJ=-P*XB*XB*XA*YA/A/XL	COLOA110
	YMK=+P*XA*XA*XB*YB/A/XL	COLOA111
	YML=+P*XA*XA*XB*YA/A/XL	COLOA112
C		COLOA113
	FF(1)=ZI	COLOA114
	FF(2)=XMI	COLOA115
	FF(3)=YMI	COLOA116
	FF(4)=ZJ	COLOA117
	FF(5)=XMJ	COLOA118
	FF(6)=YMJ	COLOA119
	FF(7)=ZK	COLOA120
	FF(8)=XMK	COLOA121
	FF(9)=YMK	COLOA122
	FF(10)=ZL	COLOA123
	FF(11)=XML	COLOA124
	FF(12)=YML	COLOA125
C		COLOA126
C	ADD TO FORCE VECTOR	COLOA127
C		COLOA128
	M=J+NELX*(K-1)	COLOA129
	NZ(1)=NPI(M)	COLOA130
	NZ(2)=NPJ(M)	COLOA131

	NZ(3)=NPK(M)	COLOA132
	NZ(4)=NPL(M)	COLOA133
C		COLOA134
	IF (ABS(PHI).EQ.90.) GO TO 150	COLCA135
C		COLOA136
C	TRANSFORM TO CARTESIAN	COLOA137
C		COLOA138
	DO 130 K=1,12,3	COLOA139
	DO 130 J=1,3	COLOA140
	SUM=0.	COLOA141
	DO 120 L=1,3	COLOA142
	120 SUM=SUM+A0(J,L)*FF(K+L-1)	COLOA143
	130 FFT(K+J-1)=SUM	COLOA144
C		COLOA145
	DO 140 J=1,12	COLOA146
	140 FF(J)=FFT(J)	COLOA147
C		COLOA148
	150 CONTINUE	COLOA149
C		COLOA150
	DO 160 J=1,4	COLCA151
	II=NZ(J)*5-3	COLOA152
	DO 160 K=1,3	COLOA153
	JJ=(J-1)*3+K	COLCA154
	KK=II+K	COLOA155
	160 FORCE(KK)=FORCE(KK)+PQ(JJ)	COLOA156
C		COLOA157
	170 CONTINUE	COLOA158
C		COLOA159
	180 CONTINUE	COLOA160
C		COLOA161
C	READ THE NUMBER OF PATCH LOAD CARDS	COLOA162
C		COLOA163
	READ (IN,20) NC	COLOA164
	WRITE (IO,190) NC	COLOA165
	190 FORMAT (1H0,5X,*NUMBER OF PATCH LOAD CARDS*,8X,*=*,I5)	COLOA166
C		COLOA167
	IF (NC.EQ.0) RETURN	COLOA168
	WRITE (IO,200)	COLOA169
	200 FORMAT (1H0,5X,*LOAD CARD      LOAD (KSI)      X OF CENTER(IN)      Y OF	COLOA170
	1 ENTER(IN)      LENGTH(IN)      WIDTH(IN)*)	COLOA171
C		COLOA172
	SINA=PHI*ATAN(1.0)/45.	COLOA173
	SINA=SIN(SINA)	COLOA174
	DO 300 NI=1,NC	COLOA175
C		COLOA176
	READ (IN,50) Q,X,Y,XLL,YLL	COLOA177
	WRITE (IO,60) NI,Q,X,Y,XLL,YLL	COLCA178
C		COLOA179
	DISTY2=0.	COLOA180
	DO 290 K=1,NELY	COLOA181
	YL=YSEG(K)	COLOA182
	DISTY1=DISTY2	COLOA183
	DISTY2=DISTY2+YL	COLOA184
C		COLOA185
	DISTX2=0.	COLOA186
	DO 290 J=1,NELX	COLOA187
	XL=XSEG(J)	COLOA188
	DISTX1=DISTX2	COLOA189



	DISTX2=DISTX2+XL	COL0A190
C		COL0A191
	IF ((X-XLL/2.).GE.DISTX2.OR.(X+XLL/2.).LE.DISTX1) GO TO 290	COL0A192
	IF ((Y-YLL/2.).GE.DISTY2.OR.(Y+YLL/2.).LE.DISTY1) GO TO 290	COL0A193
C		COL0A194
	A1=X-XLL/2.-DISTX1-XL/2.	COL0A195
	A2=X+XLL/2.-DISTX1-XL/2.	COL0A196
	B1=DISTY1+YL/2.-Y-YLL/2.	COL0A197
	B2=DISTY1+YL/2.-Y+YLL/2.	COL0A198
C		COL0A199
	IF (A1.EQ.0.0) A1=1.0E-06	COL0A200
	IF (A2.EQ.0.0) A2=1.0E-06	COL0A201
	IF (B1.EQ.0.0) B1=1.0E-06	COL0A202
	IF (B2.EQ.0.0) B2=1.0E-06	COL0A203
C		COL0A204
	IF (A1.LT.-XL/2.) A1=-XL/2.	COL0A205
	IF (A2.GT.XL/2.) A2=XL/2.	COL0A206
	IF (B1.LT.-YL/2.) B1=-YL/2.	COL0A207
	IF (B2.GT.YL/2.) B2=YL/2.	COL0A208
C		COL0A209
	A=XL/2.	COL0A210
	B=YL/2.	COL0A211
	A2=A2/A	COL0A212
	A1=A1/A	COL0A213
	B2=B2/B	COL0A214
	B1=B1/B	COL0A215
	CALL PATGHL (PQ,A2,A1,B2,B1,A,B)	COL0A216
C		COL0A217
	DO 210 I=1,12	COL0A218
	DO 210 L=1,12,3	COL0A219
	CCB(I,L)=CB(I,L)	COL0A220
	CCB(I,L+1)=CB(I,L+1)*B	COL0A221
	210 CCB(I,L+2)=CB(I,L+2)*A	COL0A222
C		COL0A223
	DO 230 I=1,12	COL0A224
	SUM=0.	COL0A225
	DO 220 L=1,12	COL0A226
	220 SUM=SUM+CCB(L,I)*PQ(L)	COL0A227
	230 CCB(1,I)=SUM	COL0A228
C		COL0A229
	M=(K-1)*NELX+J	COL0A230
	NZ(1)=NPI(M)	COL0A231
	NZ(2)=NPJ(M)	COL0A232
	NZ(3)=NPK(M)	COL0A233
	NZ(4)=NPL(M)	COL0A234
C		COL0A235
C		COL0A236
	IF (ABS(PHI).EQ.90.) GO TO 270	COL0A237
C		COL0A238
C	TRANSFORM TO CARTESIAN	COL0A239
C		COL0A240
	DO 250 KKK=1,12,3	COL0A241
	DO 250 JJJ=1,3	COL0A242
	SUM=0.	COL0A243
C		COL0A244
	DO 240 L=1,3	COL0A245
	240 SUM=SUM+A0(JJJ,L)*CCB(1,KKK+L-1)	COL0A246
C		COL0A247

250	FFT(KKK+JJJ-1)=SUM	COL0A249
	DO 260 JJJ=1,12	COL0A249
260	CCB(1, JJJ)=FFT(JJJ)*SINA	COL0A250
C		COL0A251
270	CONTINUE	COL0A252
C		COL0A253
	DO 280 I=1,4	COL0A254
	II=NZ(I)*5-3	COL0A255
	DO 280 L=1,3	COL0A256
	JJ=(I-1)*3+L	COL0A257
	KK=II+L	COL0A258
280	FORCE(KK)=FORCE(KK)+CCB(1, JJ)*Q	COL0A259
C		COL0A260
290	CONTINUE	COL0A261
300	CONTINUE	COL0A262
C		COL0A263
C		COL0A264
	RETURN	COL0A265
	END	COL0A266
	SUBROUTINE PATCHL (P, A2, A1, B2, B1, A, B)	PATCHL 2
C		PATCHL 3
C	COMPUTE INTEGRATION COEFFICIENTS FOR PATCH LOAD	PATCHL 4
C		PATCHL 5
	DIMENSION P(12)	PATCHL 6
	REAL N, NN, NNN, N4	PATCHL 7
C		PATCHL 8
	E=A2-A1	PATCHL 9
	EE=A2*A2-A1*A1	PATCHL10
	EEE=A2**3-A1**3	PATCHL11
	E4=A2**4-A1**4	PATCHL12
C		PATCHL13
	N=B2-B1	PATCHL14
	NN=B2*B2-B1*B1	PATCHL15
	NNN=B2**3-B1**3	PATCHL16
	N4=B2**4-B1**4	PATCHL17
C		PATCHL18
	P(1)=E*N	PATCHL19
	P(2)=EE*N/2.	PATCHL20
	P(3)=E*NN/2.	PATCHL21
	P(4)=EEE*N/3.	PATCHL22
	P(5)=EE*NN/4.	PATCHL23
	P(6)=E*NNN/3.	PATCHL24
	P(7)=E4*N/4.	PATCHL25
	P(8)=EEE*NN/6.	PATCHL26
	P(9)=EE*NNN/6.	PATCHL27
	P(10)=E*N4/4.	PATCHL28
	P(11)=E4*NN/8.	PATCHL29
	P(12)=EE*N4/8.	PATCHL30
C		PATCHL31
	DO 10 I=1,12	PATCHL32
10	P(I)=P(I)*A*B	PATCHL33
C		PATCHL34
	RETURN	PATCHL35
	END	PATCHL36
	OVERLAY (ECCP,10,0)	OVERL8 2
	PROGRAM OVERL8	OVERL8 3
C		OVERL8 4
C	THIS OVERLAY READS BEAM DATA AND ASSIGNS ELEMENT PROPERTIES	OVERL8 5

C	COMMON /OVER8/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13,N14,N15,N16,N17,N18,N19,N20,N21,N22,N23,N24,N25,N26,N27,N28,N29,N30	OVERL 8 6 OVERL 8 7 OVERL 8 8
	COMMON /BDIM/ RON(48)	OVERL 8 9
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	OVERL 810
	COMMON /ICORES/ ICR(7),ICCRE	OVERL 811
	COMMON /CB310/ NELX,NSMAT,NELY	OVERL 812
	COMMON /CB2/ NBEAMX,NIX,NULAYB,NULAYR,NULAYE,NR,NRE	OVERL 813
	COMMON /CB4/ IN,IO	OVERL 814
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	OVERL 815
	COMMON /SMX/ OLOAD,PMAX,SHEARM	OVERL 816
	COMMON /BTCHK/ STRAMX(30)	OVERL 817
	COMMON /CRWB/ CRB(10)	OVERL 818
	COMMON A(1)	OVFRL 819 OVERL 820
C		OVERL 821
G	IF (ICORE.NE.0) GO TO 10	OVERL 822
	ICORE=LOC(A(1))+1	OVERL 823
	GO TO 20	OVERL 824
10	CONTINUE	OVERL 825
	CALL BINF (NBEAMX,NIX,NULAYB,NUMNP,NR,NRE,NULAYR,NULAYE,NELX,NELY,1NPERBM,NREADB,KIN,KC,KMAT,NOBM,A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),A(N9),A(N10),A(N11),A(N12),A(N13),A(N14),A(N15),A(N16),A(N17),A(N18),A(N19),A(N20),A(N21),A(N22),A(N23),A(N24),A(N25),A(N26),A(N27),A(N28),A(N29))	OVERL 826 OVERL 827 OVERL 828 OVERL 829 OVERL 830
20	CONTINUE	OVERL 831
	END	OVERL 832
	SUBROUTINE BINF (NBEAMX,NIX,NULAYB,NUMNP,NR,NRE,NULAYR,NULAYE,NELX,NELY,NPERBM,NREADB,KIN,KC,KMAT,NOBM,NPARX,NXL,NXR,GKEIX,NPIX,NPKXBINF,1,2,SIBXXA,PA,PB,XSEG,YSEG,B,T,ITR,SIR,D,CGTB,PGTE,BE,TE,SE,ITE,GGTE,BINF,3ASXLR,AIR,ZCRD,TSHEAR,ESX,ITYPE)	BINF 2 BINF 3 BINF 4 BINF 5
	COMMON /BDIM/ RON(6),ROM(6),EDOWN(6),STRAN(6),SIGO(6),FT(6),ROMT(3),ROMT(3),EDOWNT(3),EDOWNR(3)	BINF 6 BINF 7
	COMMON /BTCHK/ STRAMX(6),STEMX(6),STEMX(6),NK(6),NH(6)	BINF 8
	COMMON /CRWB/ TCC,MBAR,TS,AE,NEL,ML,MLS,FSO,WBMAX,WC	BINF 9
	COMMON /SMX/ OLOAD,PMAX,SHEARM	BINF 10
	COMMON /CB4/ IN,IO	BINF 11
	DIMENSION NPARX(NUMNP),NXL(NUMNP),NXR(NUMNP),NPIX(NIX),NPKX(NIBINF,1X),SIBXXA(NULAYB,NPERBM),ASXLR(NULAYB,NPERBM),AIR(NULAYB,NPERB,2M),ZCRD(NULAYB,NPERBM),TSHEAR(NULAYB,NPERBM),ITYPE(NULAYB,NPERB,3M),ESX(NULAYB,NBEAMX),D(NR),CGTB(NR),CGTE(NPERBM),PGTE(NPERB,4),BE(NRE,NPERBM),TE(NRE,NPERBM),SE(NRE,NPERBM),ITE(NRE,NPERB,5,B(NRE,NR),T(NRE,NR),ITR(NRE,NR),SIR(NRE,NR),PA(NRE),PB(NRE),6,GKEIX(NPERBM),XSEG(NELX),YSEG(NELY),NZ(2)	BINF 12 BINF 13 BINF 14 BINF 15 BINF 16 BINF 17 BINF 18
	DIMENSION ESXX(6)	BINF 19
	DIMENSION ETFIND(12),FTFIND(18),STFIND(10)	BINF 20
	DIMENSION RY2(12),RY1(12),SR(12)	BINF 21
	REAL ITE,ITR	BINF 22
	DATA (ETFIND(I),I=1,12)/0.,3.,3.9,4.75,5.6,1000.,700.,700.,1250.,1800.,3000.,3000./	BINF 23 BINF 24
	DATA (STFIND(I),I=1,10)/0.,3.,3.9,4.75,1000.,.0024,.0024,.0023,.00122,.0022/	BINF 25 BINF 26
	DATA (FTFIND(I),I=1,18)/0.,1.,2.,3.,4.,5.,6.,7.,1000.,.11,.11,.1,109,.09,.08,.08,.07,.07/	BINF 27 BINF 28
	DATA (RY1(I),I=1,12)/0.,0.5,0.6,0.7,0.8,1.0,0.25,0.25,1.05,2.15,3.155,3.55/	BINF 29 BINF 30
	DATA (RY2(I),I=1,12)/0.,0.5,0.6,0.7,0.8,1.0,1.4,1.4,2.73,4.45,6.57,15,6.575/	BINF 31 BINF 32

	DATA (SR(I),I=1,12)/0.,0.5,0.6,0.7,0.8,1.0,42.,42.,46.,53.,62.,62.	BINF	33
	1/	BINF	34
C		BINF	35
C	IF (NBEAMX.EQ.0) GO TO 1050	BINF	36
C		BINF	37
	REWIND 5	BINF	38
	READ (5) XSEG,YSEG	BINF	39
	REWIND 4	BINF	40
	REWIND 13	BINF	41
	REWIND 14	BINF	42
	REWIND 15	BINF	43
	REWIND 27	BINF	44
C		BINF	45
C	SET UP BEAM ELEMENT TOPOLOGY	BINF	46
C		BINF	47
	DO 10 I=1,NUMNP	BINF	48
	NXL(I)=NBEAMX+1	BINF	49
	NXR(I)=NBEAMX+1	BINF	50
	NPARX(I)=0	BINF	51
	10 CONTINUE	BINF	52
C		BINF	53
	20 FORMAT (16I5)	BINF	54
	DO 30 I=1,NOBM	BINF	55
	READ (5) NSTART	BINF	56
	READ (5) LB1,LB2,LB3	BINF	57
C		BINF	58
	DO 30 J=1,NELX	BINF	59
	M=J+(I-1)*NELX	BINF	60
	READ (5) LB1,LB2,LB3	BINF	61
	NPIX(M)=NSTART+J-1	BINF	62
	NPKX(M)=NSTART+J	BINF	63
	NPARX(NSTART+J)=1	BINF	64
	NPARX(NSTART+J-1)=1	BINF	65
	NXR(NPIX(M))=M	BINF	66
	NXL(NPKX(M))=M	BINF	67
	30 CONTINUE	BINF	68
	I=NBEAMX+1	BINF	69
	NPIX(I)=NUMNP+1	BINF	70
	NPKX(I)=NUMNP+1	BINF	71
	WRITE (IO,40)	BINF	72
	40 FORMAT (1H0,/,20X,*BEAM ELEMENT ARRAY // X-AXIS*,/,5X,*ELEMENT NUMBER*,7X,1HI,14X,1HK,11X,*LENGTH (IN)*,/,)	BINF	73
C		BINF	74
	DO 50 I=1,NOBM	BINF	75
	DO 50 J=1,NPERBM	BINF	76
	N=J+(I-1)*NPERBM	BINF	77
	50 WRITE (IO,60) N,NPIX(N),NPKX(N),XSEG(J)	BINF	78
	60 FORMAT (6X,I5,10X,I5,10X,I5,5X,F10.2)	BINF	79
C		BINF	80
C		BINF	81
	WRITE (IO,70)	BINF	82
	WRITE (IO,80) (M,NPARX(M),NXL(M),NXR(M),M=1,NUMNP)	BINF	83
	70 FORMAT (///20X,30HBEAM ELEMENT TOPOLOGY //X-AXIS,///4X,11HNODAL POINTS	BINF	84
	1INT,8X,5HNPARX,11X,3HNXL,12X,3HNXR///)	BINF	85
	80 FORMAT (6X,I5,10X,I5,10X,I5,10X,I5)	BINF	86
C		BINF	87
C	READ MATERIAL PROPERTIES	BINF	88
C		BINF	89
		BINF	90

C	READ (IN,20) KC,KMAT	BINF	91
	WRITE (IO,90) KC,KMAT	BINF	92
	90 FORMAT (//,5X,*NUMBER OF CONCRETE MATERIALS, KC	BINF	94
	1//,5X,*NUMBER OF CONCRETE PLUS STEEL MATERIALS, KMAT=*,I8,1X)	BINF	95
C		BINF	96
	IF (KC.GT.3) WRITE (IO,100)	BINF	97
	100 FORMAT (1H0,//,11X,*MAX NUMBER OF CONCRETE MATERIALS HAS BEEN EXCE	BINF	98
	1EDED*,//)	BINF	99
	IF (KMAT.GT.5) WRITE (IO,110)	BINF	100
	110 FORMAT (1H0,//,11X,*MAX NUMBER OF BEAM MATERIALS HAS BEEN EXCEEDED	BINF	101
	1*,//)	BINF	102
	IF (KC.GT.3) STOP 10	BINF	103
	IF (KMAT.GT.6) STOP 10	BINF	104
C		BINF	105
	WRITE (IO,120)	BINF	106
	120 FORMAT (1H0,//,20X,*BEAM- MATERIAL PROPERTIES*,//,6X,*MAT. NO. STR	BINF	107
	1GY (KSI) FT (KSI) MODULUS (KSI) ROM RON EDOWN	BINF	108
	2(KSI) STRAN (PERCENT)*,/) )	BINF	109
C		BINF	110
C		BINF	111
	DO 180 J=1,KMAT	BINF	112
	READ (IN,130) I,SIGO(I),FT(I),ESXX(I),ROM(I),RON(I),EDOWN(I),STRAN	BINF	113
	1(I)	BINF	114
	130 FORMAT (I5,7F10.0)	BINF	115
	STRAN(I)=STRAN(I)/100.	BINF	116
	IF (I.GT.KC) GO TO 140	BINF	117
	IF (ESXX(I).EQ.0.) ESXX(I)=33.*(145.)*1.5*SQRT(SIGO(I)*1000.)/100	BINF	118
	10.	BINF	119
	IF (FT(I).EQ.0.) CALL FIND (FT(I),FTFIND,18,SIGO(I),SIGO(I))	BINF	120
	IF (STRAN(I).EQ.0.) CALL FIND (STRAN(I),STFIND,10,SIGO(I),1.0)	BINF	121
	IF (EDOWN(I).EQ.0.0) CALL FIND (EDOWN(I),ETFIND,12,SIGO(I),1.0)	BINF	122
	IF (RON(I).EQ.0.0) RON(I)=9.0	BINF	123
	IF (ROM(I).EQ.0.0) ROM(I)=SIGO(I)/(0.002*ESXX(I))	BINF	124
	STRAN(I)=STRAN(I)*100.	BINF	125
	GO TO 160	BINF	126
	140 CONTINUE	BINF	127
	IF (FT(I).EQ.0.) FT(I)=SIGO(I)	BINF	128
	IF (ESXX(I).EQ.0.) ESXX(I)=27000.	BINF	129
	IF (ROM(I).EQ.0.) ROM(I)=.67	BINF	130
	IF (RON(I).EQ.0.) RON(I)=25.	BINF	131
	IF (STRAN(I).EQ.0.) STRAN(I)=1.	BINF	132
	STRAN(I)=STRAN(I)*100.	BINF	133
	150 FORMAT (8F10.0)	BINF	134
	160 WRITE (IO,170) I,SIGO(I),FT(I),ESXX(I),ROM(I),RON(I),EDOWN(I),STRAN	BINF	135
	1N(I)	BINF	136
	170 FORMAT (1H0,7X,I5,2X,F10.4,4X,F10.4,2X,F8.2,7X,F9.4,2X,F8.3,3X,F8.	BINF	137
	12,6X,F9.5)	BINF	138
C		BINF	139
	STRAN(I)=-STRAN(I)	BINF	140
	STRAN(I)=STRAN(I)/100.	BINF	141
	180 CONTINUE	BINF	142
C		BINF	143
C	READ CONCRETE TENSILE PROPERTIES	BINF	144
C		BINF	145
	IF (KC.EQ.0) GO TO 230	BINF	146
	WRITE (IO,190)	BINF	147
	190 FORMAT (1H0,//,20X,*BEAM- CONCRETE TENSILE PROPERTIES*,//,6X,*MAT.	BINF	148

	1 NO.	ROMT	RONT	EDOWNT (KSI)*, //)	BINF	
C					149	BINF
					150	BINF
					151	BINF
					152	BINF
C					153	BINF
					154	BINF
					155	BINF
					156	BINF
					157	BINF
					158	BINF
	200				159	BINF
					160	BINF
	210				161	BINF
					162	BINF
					163	BINF
	220				164	BINF
	230				165	BINF
C					166	BINF
C					167	BINF
C					168	BINF
					169	BINF
					170	BINF
	240				171	BINF
					172	BINF
					173	BINF
	250				174	BINF
					175	BINF
C					176	BINF
					177	BINF
	260				178	BINF
					179	BINF
					180	BINF
					181	BINF
					182	BINF
C					183	BINF
					184	BINF
	270				185	BINF
	280				186	BINF
C					187	BINF
C					188	BINF
C					189	BINF
					190	BINF
C					191	BINF
					192	BINF
					193	BINF
C					194	BINF
					195	BINF
					196	BINF
	1				197	BINF
C					198	BINF
					199	BINF
					200	BINF
C					201	BINF
					202	BINF
					203	BINF
C					204	BINF
					205	BINF
					206	BINF

C	STRAMX(J)=STRAMX(J)*100.	BINF 207
	WRITE (IO,290) J,STRAMX(J),STTEMX(J),STCEMX(J),NK(J),NH(J)	BINF 208
	290 FORMAT (1H ,7X,I5,4X,F8.3,4X,F8.3,4X,F8.3,8X,I5,11X,I5)	BINF 209
C	STRAMX(J)=STRAMX(J)/100.	BINF 210
	300 CONTINUE	BINF 211
C		BINF 212
C	READ IN SHEAR CHECK	BINF 213
C		BINF 214
C	READ (IN,270) I,SHEARM	BINF 215
	IF (SHEARM.LT.0.0) SHEARM=2.0*0.85*SQRT(SIG0(I)*1000.)/1000.	BINF 216
	WRITE (IO,310) I,SHEARM	BINF 217
	310 FORMAT (1H0,/,6X,* MATERIAL TYPE USED IF SHEAR IS TO BE COMPUTED	BINF 218
	1 = *,I8,/,6X,*MAX ALLOWABLE FLEXURAL SHEAR FOR THE BEAM	BINF 219
	2 (KSI) = *,F8.3,/) )	BINF 220
C		BINF 221
C	CRACK WIDTH DATA	BINF 222
C		BINF 223
C	READ (IN,130) MBAR,TCC,WBMAX	BINF 224
	IF (WBMAX.LE.0.0) WEMAX=0.004	BINF 225
	WRITE (IO,320) MBAR,TCC,WBMAX	BINF 226
	320 FORMAT (1H0,/,6X,*NUMBER OF RE-BARS AND STRANDS = *,5X,I3,/,6X,*	BINF 227
	1CONCRETE COVER*,16X,*= *,F8.3,* (IN)*,/,6X,*MAX ALLOWABLE CRACK	BINF 228
	2WIDTH*,5X,*= *,F8.5,* (IN)*,/) )	BINF 229
	FS0=-999.0	BINF 230
C		BINF 231
C	READ PROPERTY INPUT CODE	BINF 232
C		BINF 233
C	READ (IN,700) NAME1,NAME2	BINF 234
	WRITE (IO,330) NAME1,NAME2	BINF 235
	330 FORMAT (1H0,/,6X,*EEM INPUT IS BY *,A5,A5)	BINF 236
C		BINF 237
C	READ ELEMENT LENGTHS	BINF 238
C		BINF 239
C	REWIND 5	BINF 240
	READ (5) XSEG,YSEG	BINF 241
C		BINF 242
C	READ ALL BEAMS TO BE INPUTTED	BINF 243
C		BINF 244
C		BINF 245
C	NT=0	BINF 246
	NL=1	BINF 247
C		BINF 248
C	DO 1020 IREADS=1,NREADS	BINF 249
C		BINF 250
C	READ (IN,20) NB	BINF 251
	WRITE (4) NB	BINF 252
C		BINF 253
C	NT=NT+NB	BINF 254
C		BINF 255
C	WRITE (IO,340) IREADS,NB,NL,NT	BINF 256
	340 FORMAT (1H0,/,6X,*READ NUMBER = *,I5,/,6X,*NUMBER OF DUPLICATE BE	BINF 257
	1AMS = *,I5,/,6X,*STARTING BEAM =*,I5,* END BEAM =*,I5,/) )	BINF 258
C		BINF 259
C	IF (NT.GT.NOBM) WRITE (IO,350) NT,NOBM	BINF 260
	350 FORMAT (1H0,/,6X,*TOTAL NUMBER OF BEAMS TO BE GENERATED-*,I5,*-	BINF 261
	1EXCEEDS THE NUMBER OF BEAMS INITIALLY COMPUTED-*,I5,/) )	BINF 262
		BINF 263
		BINF 264

C	IF (NT.GT.NOBM) STOP	BINF 265
		BINF 266
C		RINF 267
	IF (NAMEI.EQ.5HSECTI) GO TO 420	BINF 268
C		BINF 269
C	INITIALIZE	BINF 270
C	READ IN PROPERTIES FOR BEAM ELEMENTS	BINF 271
C		9INF 272
	DO 360 I=1,NULAYB	BINF 273
	DO 360 J=1,NPERBM	BINF 274
	SIBXXA(I,J)=0.	BINF 275
	ITYPE(I,J)=0.	BINF 276
	ASXLR(I,J)=0.	BINF 277
	AILR(I,J)=0.	BINF 278
	ZCRD(I,J)=0.0	BINF 279
	TSHEAR(I,J)=0.	BINF 280
	360 CONTINUE	BINF 281
C		BINF 282
C	READ ELEMENT PROPERTY CODE	RINF 283
C		BINF 284
	WRITE (IO,460)	BINF 285
		BINF 286
C	ASSIGN READ CODE	BINF 287
C		BINF 288
	370 CONTINUE	BINF 289
	READ (IN,380) NAME,NAME2,NAME3	BINF 290
	380 FORMAT (A2,A5,A3)	BINF 291
	WRITE (IO,390) NAME,NAME2,NAME3	BINF 292
	390 FORMAT (1H0,5X,* CODE LETTERS = *,A2,A5,A3)	BINF 293
C		BINF 294
	IF (NAME.EQ.2HEN) GO TO 420	BINF 295
	M=0	BINF 296
	IF (NAME.EQ.2HST) M=1	BINF 297
	IF (NAME.EQ.2HTY) M=2	BINF 298
	IF (NAME.EQ.2HAR) M=3	BINF 299
	IF (NAME.EQ.2HIN) M=4	BINF 300
	IF (NAME.EQ.2HZC) M=5	BINF 301
	IF (NAME.EQ.2HWI) M=6	BINF 302
	IF (M.NE.0) GO TO 410	BINF 303
C		BINF 304
	WRITE (IO,460)	BINF 305
	WRITE (IO,400)	BINF 306
	400 FORMAT (1H0,5X,*NO TRANSLATION*,//)	BINF 307
	STOP	BINF 308
C		BINF 309
	410 CONTINUE	BINF 310
C		BINF 311
C	READ SPECIFIC PROPERTY ARRAY	BINF 312
C		BINF 313
	I=NULAYB	BINF 314
	J=NPERBM	RINF 315
	IF (M.EQ.1) CALL READF (I,J,1,I,1,J,IN,KIN,SIBXXA)	BINF 316
	IF (M.EQ.2) CALL READI (I,J,1,I,1,J,IN,KIN,ITYPE)	BINF 317
	IF (M.EQ.3) CALL READF (I,J,1,I,1,J,IN,KIN,ASXLR)	BINF 318
	IF (M.EQ.4) CALL READF (I,J,1,I,1,J,IN,KIN,AILR)	BINF 319
	IF (M.EQ.5) CALL READF (I,J,1,I,1,J,IN,KIN,ZCRD)	BINF 320
	IF (M.EQ.6) CALL READF (I,J,1,I,1,J,IN,KIN,TSHEAR)	BINF 321
	GO TO 370	BINF 322



C	420 CONTINUE	BINF 323
C		BINF 324
C		BINF 325
C	WRITE BEAM END POINT BOUNDARY CONDITIONS TO TAPE	BINF 326
C	FOR BOTH ELEMENTS AND SECTIONS INPUT OPTIONS	BINF 327
C		BINF 328
	N=PPERBM+1	BINF 329
	DO 430 J=NL,NT	BINF 330
	READ (5) M	BINF 331
	DO 430 I=1,N	BINF 332
	READ (5) LB1,LB2,LB3	BINF 333
	IF (J.EQ.NT) WRITE (4) LB1,LB2,LB3	BINF 334
	430 CONTINUE	BINF 335
	NPS=0	BINF 336
	IF (NAMEI.NE.5HSECTI) WRITE (4) NPS	BINF 337
	IF (NAMEI.NE.5HSECTI) GO TO 910	BINF 338
C		BINF 339
C	ROUTINE FOR SECTIONS OPTION	BINF 340
C	INITIALIZE	BINF 341
C		BINF 342
	DO 440 J=1,NR	BINF 343
	CGTB(J)=0.	BINF 344
	DO 440 I=1,NRE	BINF 345
	B(I,J)=0.	BINF 346
	T(I,J)=0.	BINF 347
	ITR(I,J)=0.	BINF 348
	SIR(I,J)=0.	BINF 349
	440 CONTINUE	BINF 350
C		BINF 351
C	READ NUMBER OF SECTIONS AT WHICH LAYER PROPERTIES WILL	BINF 352
C	BE DEFINED	BINF 353
C		BINF 354
C		BINF 355
	READ (IN,20) NX	BINF 356
	WRITE (IO,450) NX	BINF 357
	450 FORMAT (1H0,/,6X,*NUMBER OF BEAM PROPERTY SECTIONS =*,I5)	BINF 358
C		BINF 359
C	READ X-LOCATION OF SECTIONS	BINF 360
C		BINF 361
	WRITE (IO,460)	BINF 362
	460 FORMAT (1H )	BINF 363
	READ (IN,150) (D(J),J=1,NX)	BINF 364
	WRITE (IO,470)	BINF 365
	470 FORMAT (1H0,5X,*X-COORDINATE DISTANCES FROM START OF BEAM (IN)*)	BINF 366
	WRITE (IO,480) (D(J),J=1,NX)	BINF 367
	480 FORMAT ((1H0,5X,10(F8.3,2X,)))	BINF 368
C		BINF 369
	WRITE (IO,460)	BINF 370
	READ (IN,150) (CGTB(J),J=1,NX)	BINF 371
	WRITE (IO,490)	BINF 372
	490 FORMAT (1H0,5X,*Z-DISTANCES FROM REF. PLANE TO TOP OF BEAM (IN)*)	BINF 373
	WRITE (IO,480) (CGTB(J),J=1,NX)	BINF 374
C		BINF 375
C	READ CONCRETE LAYER PROPERTIES	BINF 376
C		BINF 377
	WRITE (IO,460)	BINF 378
	500 CONTINUE	BINF 379
C		BINF 380

C	READ PROPERTY CODE	BINF 381
C		BINF 382
	READ (IN,380) NAME,NAME2,NAME3	BINF 383
	WRITE (IO,390) NAME,NAME2,NAME3	BINF 384
C		BINF 385
C	ASSIGN READ CODE	BINF 386
C		BINF 387
	IF (NAME.EQ.2HEN) GO TO 520	BINF 388
	M=5	BINF 389
	IF (NAME.EQ.2HST) M=1	BINF 390
	IF (NAME.EQ.2HTY) M=2	BINF 391
	IF (NAME.EQ.2HWI) M=3	BINF 392
	IF (NAME.EQ.2HTH) M=4	BINF 393
	IF (M.NE.0) GO TO 510	BINF 394
C		BINF 395
	WRITE (IO,460)	BINF 396
	WRITE (IO,400)	BINF 397
	STOP	BINF 398
C		BINF 399
	510 CONTINUE	BINF 400
C		BINF 401
C	READ SPECIFIED LAYER PROPERTY ARRAY FOR THE SECTION	BINF 402
C		BINF 403
	IF (M.EQ.1) CALL READF (NRE,NR,1,NULAYR,1,NX,IN,1,SIR)	BINF 404
	IF (M.EQ.2) CALL READF (NRE,NR,1,NULAYR,1,NX,IN,1,ITR)	BINF 405
	IF (M.EQ.3) CALL READF (NRE,NR,1,NULAYR,1,NX,IN,1,B)	BINF 406
	IF (M.EQ.4) CALL READF (NRE,NR,1,NULAYR,1,NX,IN,1,T)	BINF 407
	IF (M.EQ.5) CALL BEAMT (NRE,NR,1,NULAYR,1,NX,IN,IO,1,NAME2,SIR,ITR	BINF 408
	1,B,T,NAME3)	BINF 409
	GO TO 500	BINF 410
C		BINF 411
	520 CONTINUE	BINF 412
C		BINF 413
C	PRINT SECTION PROPERTIES	BINF 414
C		BINF 415
	WRITE (IO,530)	BINF 416
	530 FORMAT (1H0,/,6X,*SECTION PROPERTIES (SECTIONS ARE HORIZNTAL, LA	BINF 417
	1YERS ARE VERTICAL)*)	BINF 418
C		BINF 419
	WRITE (IO,460)	BINF 420
	WRITE (IO,540)	BINF 421
	540 FORMAT (1H0,5X,*ACTUAL LAYERS*,/,6X,*SECTION- WIDTH (IN) OF LAYER	BINF 422
	1S*)	BINF 423
	CALL WRITEF (NRE,NR,1,NULAYR,1,NX,IO,9)	BINF 424
C		BINF 425
	WRITE (IO,550)	BINF 426
	550 FORMAT (1H0,5X,*SECTION- THICKNESS (IN) OF LAYERS*)	BINF 427
	CALL WRITEF (NRE,NR,1,NULAYR,1,NX,IO,T)	BINF 428
C		BINF 429
	WRITE (IO,560)	BINF 430
	560 FORMAT (1H0,5X,*SECTION- INITIAL STRESS (KSI) OF LAYERS*)	BINF 431
	CALL WRITEF (NRE,NR,1,NULAYR,1,NX,IO,SIR)	BINF 432
C		BINF 433
	WRITE (IO,570)	BINF 434
	570 FORMAT (1H0,5X,*SECTION- MATERIAL TYPE FOR LAYERS*)	BINF 435
	CALL WRITEF (NRE,NR,1,NULAYR,1,NX,IO,ITR)	BINF 436
C		BINF 437
C	READ EMBEDDED LAYER PROPERTIES	BINF 438

C	IF (NULAYE.EQ.0) GO TO 650	BINF 439
	IS=NULAYR+1	BINF 440
	IE=NULAYR+NULAYE	BINF 441
C	580 CONTINUE	BINF 442
C	READ PROPERTY CODE	BINF 443
C		BINF 444
	READ (IN,380) NAME,NAME2,NAME3	BINF 445
	WRITE (IO,390) NAME,NAME2,NAME3	BINF 446
C		BINF 447
C	ASSIGN READ CODE	BINF 448
C		BINF 449
	IF (NAME.EQ.2HEN) GO TO 600	BINF 450
	M=0	BINF 451
	IF (NAME.EQ.2HAR) M=1	BINF 452
	IF (NAME.EQ.2HZC) M=2	BINF 453
	IF (NAME.EQ.2HST) M=3	BINF 454
	IF (NAME.EQ.2HTY) M=4	BINF 455
	IF (M.NE.0) GO TO 590	BINF 456
C		BINF 457
	WRITE (IO,400)	BINF 458
	STOP	BINF 459
C		BINF 460
	590 CONTINUE	BINF 461
C		BINF 462
C	READ SPECIFIED ARRAY	BINF 463
C		BINF 464
	IF (M.EQ.1) CALL READF (NRE,NR,IS,IE,1,NX,IN,1,B)	BINF 465
	IF (M.EQ.2) CALL READF (NRE,NR,IS,IE,1,NX,IN,1,T)	BINF 466
	IF (M.EQ.3) CALL READF (NRE,NR,IS,IE,1,NX,IN,1,SIR)	BINF 467
	IF (M.EQ.4) CALL READF (NRE,NR,IS,IE,1,NX,IN,1,ITR)	BINF 468
	GO TO 580	BINF 469
C		BINF 470
	600 CONTINUE	BINF 471
C		BINF 472
C	PRINT ARRAYS	BINF 473
C		BINF 474
	WRITE (IO,460)	BINF 475
	WRITE (IO,610)	BINF 476
	610 FORMAT (1H0,5X,*EMBEDDED LAYERS*,//,6X,*SECTION- AREAS (SQ.IN) OF	BINF 477
	1LAYERS*)	BINF 478
	CALL WRITEF (NRE,NR,IS,IE,1,NX,IO,B)	BINF 479
C		BINF 480
	WRITE (IO,620)	BINF 481
	620 FORMAT (1H0,5X,*SECTION- CENTROIDAL DISTANCE (IN) OF LAYERS FROM T	BINF 482
	10P OF BEAM*)	BINF 483
	CALL WRITEF (NRE,NR,IS,IE,1,NX,IO,T)	BINF 484
C		BINF 485
	WRITE (IO,630)	BINF 486
	630 FORMAT (1H0,5X,*SECTION- INITIAL STRESS (KSI) OF LAYERS*)	BINF 487
	CALL WRITEF (NRE,NR,IS,IE,1,NX,IO,SIR)	BINF 488
C		BINF 489
	WRITE (IO,640)	BINF 490
	640 FORMAT (1H0,5X,*SECTION- MATERIAL TYPE FOR LAYERS*)	BINF 491
	CALL WRITEF (NRE,NR,IS,IE,1,NX,IO,ITR)	BINF 492
C		BINF 493
		BINF 494
		BINF 495
		BINF 496

650 CONTINUE	BINF 497
C	BINF 498
C FIND ELEMENT PROPERTIES BY AVERAGING SECTION PROPERTIES ABOUT	BINF 499
C THE CENTER OF THE ELEMENT.	BINF 500
C	BINF 501
CALL AVGER (B,BE,D,NRE,NR,NRE,NPERBM,XSEG,NX,PA,PB)	BINF 502
CALL AVGER (T,TE,D,NRE,NR,NRE,NPERBM,XSEG,NX,PA,PB)	BINF 503
CALL AVGER (SIR,SE,D,NRE,NR,NRE,NPERBM,XSEG,NX,PA,PB)	BINF 504
CALL AVGER (ITR,ITE,D,NRE,NR,NRE,NPERBM,XSEG,NX,PA,PB)	BINF 505
CALL AVGER (CGTB,CGTE,D,1,NR,1,NPERBM,XSEG,NX,PA,PB)	BINF 506
C	BINF 507
C COMPUTE LAYER PROPERTIES	BINF 508
C	BINF 509
DO 660 J=1,NPERBM	BINF 510
ZCR=CGTE(J)	BINF 511
DO 660 I=1,NULAYR	BINF 512
TA=TE(I,J)	BINF 513
BA=BE(I,J)	BINF 514
ZCR=ZCR+TA/2.0	BINF 515
ASXLR(I,J)=TA*BA	BINF 516
ZCRD(I,J)=ZCR	BINF 517
AILR(I,J)=BA*TA*TA*TA/12.0	BINF 518
SIBXXA(I,J)=SE(I,J)	BINF 519
TSHEAR(I,J)=BA	BINF 520
ZCR=ZCR+TA/2.0	BINF 521
ITYPE(I,J)=ITE(I,J)+.5	BINF 522
660 CONTINUE	BINF 523
C	BINF 524
IF (NULAYE.EQ.0) GO TO 680	BINF 525
IS=NULAYR+1	BINF 526
IE=NULAYR+NULAYE	BINF 527
C	BINF 528
DO 670 J=1,NPERBM	BINF 529
ZCR=CGTE(J)	BINF 530
DO 670 I=IS,IE	BINF 531
AILR(I,J)=0.	BINF 532
SIBXXA(I,J)=SE(I,J)	BINF 533
TSHEAR(I,J)=1.0	BINF 534
ZCRD(I,J)=TE(I,J)+ZCR	BINF 535
ITYPE(I,J)=ITE(I,J)+.5	BINF 536
ASXLR(I,J)=BE(I,J)	BINF 537
670 CONTINUE	BINF 538
C	BINF 539
680 CONTINUE	BINF 540
C	BINF 541
C PRESTRESS INFORMATION	BINF 542
C	BINF 543
READ (IN,20) NPS	BINF 544
WRITE (IO,690) NPS	BINF 545
690 FORMAT (1H0,/,6X,*NUMBER OF PRESTRESS STRAND GROUPS = *,I3)	BINF 546
IR=4	BINF 547
WRITE (IR) NPS	BINF 548
IF (NPS.EQ.0) GO TO 910	BINF 549
C	BINF 550
C INITIALIZE END FORCES CAUSED BY PRESTRESS	BINF 551
C	BINF 552
VS=0.	BINF 553
AXS=0.	BINF 554

	AMS=0.	BINF 555
	VE=0.	BINF 556
	AXE=0.	BINF 557
	AME=0.	BINF 558
C		BINF 559
C	READ INFORMATION FOR ALL PRESTRESS GROUPS	BINF 560
C		BINF 561
	DO 900 JJ=1,NPS	BINF 562
C		BINF 563
	READ (IN,700) NAME,NAME2,APS,SPS,ITYPS	BINF 564
	700 FORMAT (A5,A5,2F10.0,I5)	BINF 565
	WRITE (IO,710) JJ,NAME,NAME2,APS,SPS,ITYPS	BINF 566
	710 FORMAT (1H0,5X,*PRESTRESS STRAND GROUP =*,I5,/,11X,*CLASS =*,BINF 567	
	1A5,A5,/,11X,*AREA =*,F8.4,* SQ.IN*,/,11X,*STRESS =*,F8.3BINF 568	
	2,* KSI*,/,11X,*MAT. TYPE =*,I8,/) BINF 569	
		BINF 570
C	READ (IN,720) CODEL,CODET	BINF 571
	720 FORMAT (2X,A3,A5)	BINF 572
	WRITE (IO,730) CODEL,CODET	BINF 573
	730 FORMAT (1H0,5X,*COMPUTE PRESTRESS LOSS- *,A3,/,6X,*USING	
	1 - *,A5)	BINF 574
		BINF 575
C		BINF 576
	EL=0.0	BINF 577
C		BINF 578
C	CHECK IF PRESTRESS LOSS IS TO BE COMPUTED	BINF 579
C		BINF 580
	IF (CODEL.NE.3HYES) GO TO 780	BINF 581
C		BINF 582
C	READ DATA TO COMPUTE THE LOSS	BINF 583
C		BINF 584
	READ (IN,150) AG,RIG,EG,DLMG,RNI	BINF 585
	WRITE (IO,740) AG,RIG,EG,DLMG,RNI	BINF 586
	740 FORMAT (1H0,5X,*PROPERTIES FOR BEAM CROSS-SECTION*,/,11X,*AREA	
	1 =*,F8.2,* SQ. IN*,/,11X,*INERTIA BINF 588	
	2 =*,F8.1,* IN4*,/,11X,*STRAND ECCENTRICITY FROM C.G. =BINF 589	
	3 *,F8.4,* IN*,/,11X,*DEAD LOAD MOMENT ON SECTION =*,F8.2,* IN-KBINF 590	
	4IPS*,/,11X,*MODULAR RATIO OF STEEL/CONC. =*,F8.4,/,1X) BINF 591	
		BINF 592
C		BINF 593
	READ (IN,150) RIC,EC,DLMC	BINF 594
	WRITE (IO,750) RIC,EC,DLMC	BINF 595
	750 FORMAT (1H0,5X,*PROPERTIES FOR COMPOSITE CROSS-SECTION*,/,11X,*INEBINF 596	
	1RTIA =*,F8.1,* IN4*,/,11X,*STRAND ECCENTRICBINF 597	
	2ITY FROM C.G. =*,F8.4,* IN*,/,11X,*DEAD LOAD MOMENT ON SECTION BINF 598	
	3=*,F8.2,* IN-KIPS*,/,1X) BINF 599	
		BINF 600
C		BINF 601
	READ (IN,150) FPU,TK1,TC	BINF 602
	WRITE (IO,760) FPU,TK1,TC	BINF 603
	760 FORMAT (1H0,5X,*ULTIMATE STRENGTH =*,F8.3,* KSI*,/6X,*TIME INTERVBINF 604	
	1AL FROM TENSIONING OF STEEL TO TRANSFER =*,F8.1,* DAYS*,/,6X,*TIMBINF 605	
	2E FROM TRANSFER =*,F8.1,* DAYS*,/,1X) BINF 606	
		BINF 607
C		BINF 608
	BETA=APS*(1.0/AG+EG*EG/RIG)	BINF 609
	BETA=1.0/BETA	BINF 610
	FCL=DLMG*EG/RIG	BINF 611
C		BINF 612
C	CHECK WHICH FORMULA TO USE	BINF 613
C		BINF 614
	IF (CODET.NE.5HRD201) GO TO 770	BINF 615

C		BINF 613
C	USE METHOD PRESENTED IN PENNSYLVANIA DEPARTMENT OF TRANSPORTATION	BINF 614
C	DESIGN STANDARDS BD201	PINF 615
C		BINF 616
	FC3=(SPS-BETA*FCL)/BETA	BINF 617
	EL=5.0*FC3	BINF 618
	TL=6.0+16.*FC3+0.08*SPS	BINF 619
	SPS=SPS-TL+EL	BINF 620
	GO TO 780	BINF 621
C		BINF 622
C		BINF 623
C	USE METHOD PRESENTED IN - PRESTRESS LOSSES IN PRETENSIONED CONCRET	BINF 624
C	STRUCTURAL MEMBERS - FRITZ LAB REPORT NO. 339.9 BY TI HUANG	BINF 625
C		BINF 626
	770 CONTINUE	BINF 627
	IF (RIC.NE.0.0) FCL=FCL+DLMC*EC/RI	BINF 628
	R=SPS/FPU	BINF 629
	CALL FIND (RYB,RY2,12,R,1.0)	BINF 630
	CALL FIND (RYA,RY1,12,R,1.0)	PINF 631
	CALL FIND (SRL,SR,12,R,1.0)	BINF 632
	REL=(RYB-RYA)*ALOG10(TK1)/ALOG10(20.)+RYA	BINF 633
	REL=FPU*REL/100.	BINF 634
	FC3=(SPS-REL)/(BETA+RNI-1.0)	BINF 635
	EL=RNI*FC3	BINF 636
	RIL=REL+EL	BINF 637
	RLD=2.*RNI*BETA*FCL/(BETA+RNI-1.0)	BINF 638
	ECR=2.2*RNI*FC3	BINF 639
	TL=SRL+ECR-RLD	BINF 640
	PL=RIL+0.22*(TL-RIL)*ALOG10(TC)	BINF 641
	SPS=SPS-PL+EL	BINF 642
C		BINF 643
	780 CONTINUE	BINF 644
	WRITE (IO,790) SPS,EL	BINF 645
	790 FORMAT (1H0,/,6X,*PRESTRESS AFTER LOSSES (EXCEPT ELASTIC LOSS) =	BINF 646
	1*F8.3,* KSI*,/,6X,*ESTIMATED ELASTIC LOSS =	BINF 647
	2*,F8.3,* KSI*,/,1X)	BINF 648
C		BINF 649
C	CHECK SHAPE OF STRAND	BINF 650
C		BINF 651
	IF (NAME.EQ.5HPAPAB) GO TO 850	BINF 652
C		BINF 653
C	FOR STRAIGHT STRAND (STRAND MADE UP OF STRAIGHT LINE SEGMENTS)	BINF 654
C		BINF 655
	WRITE (IO,460)	BINF 656
	READ (IN,20) NX	BINF 657
	WRITE (IO,800) NX	BINF 658
	800 FORMAT (1H0,5X,*NUMBER OF (X,Z) COORDINATE POINTS ARE *,I3)	BINF 659
C		BINF 660
	WRITE (IO,460)	BINF 661
	READ (IN,150) (D(J),J=1,NX)	BINF 662
	WRITE (IO,810)	BINF 663
	810 FORMAT (1H0,5X,*X - COORDINATES FROM START OF BEAM (IN)*)	BINF 664
	WRITE (IO,480) (D(J),J=1,NX)	BINF 665
C		BINF 666
	WRITE (IO,460)	BINF 667
	READ (IN,150) (CGTB(J),J=1,NX)	BINF 668
	WRITE (IO,820)	BINF 669
	820 FORMAT (1H0,5X,*Z - COORDINATES FROM TOP OF BEAM (IN)*)	BINF 670

	WRITE (IO,480) (CGTB(J),J=1,NX)	BINF 671
C		BINF 672
	CALL AVGER (CGTB,PGTE,D,1,NR,1,NPERBM,XSEG,NX,PA,PB)	BINF 673
C		BINF 674
C	ASSIGN LAYER PROPERTIES	BINF 675
C		BINF 676
	J=NULAYR+NULAYE+JJ	BINF 677
	DO 830 I=1,NPERBM	BINF 678
	ZCRD(J,I)=PGTE(I)+CGTE(I)	BINF 679
	ASXLR(J,I)=APS	BINF 680
	AILR(J,I)=0.0	BINF 681
	SIBXXA(J,I)=SPS	BINF 682
	TSHEAR(J,I)=1.0	BINF 683
	ITYPE(J,I)=ITYPS	BINF 684
	830 CONTINUE	BINF 685
C		BINF 686
C	COMPUTE PRESTRESS FORCES ON BEAM	BINF 687
C		BINF 688
C		BINF 689
C	CONCENTRATED END FORCES	BINF 690
C		BINF 691
	THETA=ATAN((CGTB(2)-CGTB(1))/D(2))	BINF 692
	VS=+APS*SPS*SIN(THETA)+VS	BINF 693
	AX=+APS*SPS*COS(THETA)	BINF 694
	AXS=AXS+AX	BINF 695
	AMS=+AX*(CGTB(1)+CGTE(1))+AMS	BINF 696
C		BINF 697
	THETA=ATAN((CGTB(NX)-CGTB(NX-1))/(D(NX)-D(NX-1)))	BINF 698
	VE=VE-APS*SPS*SIN(THETA)	BINF 699
	AX=APS*SPS*COS(THETA)	BINF 700
	AXE=AXE-AX	BINF 701
	AME=AME-AX*(CGTB(NX)+CGTE(NPERBM))	BINF 702
C		BINF 703
C	VERTICAL FORCES ALONG BEAM	BINF 704
C		BINF 705
	N=NX-2	BINF 706
	WRITE (IR) NAME,N	BINF 707
	IF (NX.LE.2) GO TO 900	BINF 708
C		BINF 709
	N=NX-1	BINF 710
	DO 840 I=2,N	BINF 711
	THETA1=ATAN((CGTB(I)-CGTB(I-1))/(D(I)-D(I-1)))	BINF 712
	THETA2=ATAN((CGTB(I+1)-CGTB(I))/(D(I+1)-D(I)))	BINF 713
	THETA=THETA1-THETA2	BINF 714
	V=-APS*SPS*SIN(THETA)	BINF 715
C		BINF 716
	WRITE (IR) D(I),V	BINF 717
	840 CONTINUE	BINF 718
	GO TO 900	BINF 719
C		BINF 720
C	FOR PARABOLIC DRAPPED STRAND	BINF 721
C		BINF 722
	850 CONTINUE	BINF 723
	READ (IN,150) XA,YA,XB,YB,THETA	BINF 724
	IF (THETA.NE.0) WRITE (IO,860)	BINF 725
	860 FORMAT (1H0,/,5X,*INCORRECT DATA FOR PARABOLIC STRAND- SLOPE SHOULD	BINF 726
	1LD BE ZERO AT SECOND COORDINATE POINT*)	BINF 727
	IF (THETA.NE.0) STOP 13	BINF 728

C	BP=(XA-XB)*(XA-XB)	BINF 729
	AP=(YA-YB)/BP	BINF 730
	BP=-2.0*AP*XB	BINF 731
	CP=AP*X3*X8+YB	BINF 732
C		BINF 733
	WRITE (IO,460)	BINF 734
	WRITE (IO,870) XA,YA,XB,YB,THETA,AP,BP,CP	BINF 735
	870 FORMAT (1H0,5X,*FIRST COORDINATE (X,Z) = (*,F8.3,*,*,F8.3,*) INCH	BINF 736
	1ES*,/,6X,*SECOND COORDINATE (X,Z) = (*,F8.3,*,*,F8.3,*) INCHES*,/6	BINF 737
	2X,*SLOPE AT SECOND POINT = *,F8.3,/,6X,*PARABOLIC EQUATION IS Z	BINF 738
	3= (*,E16.9,*) XX + (*,E16.9,*) X + (*,E16.9,*)*)	BINF 739
C		BINF 740
C	END FORCES AND UNIFORM VERTICAL LOAD ALONG BEAM	BINF 741
C		BINF 742
	XL=0.	BINF 743
	DO 880 I=1,NPERBM	BINF 744
	880 XL=XL+XSEG(I)	BINF 745
C		BINF 746
	THETA=ATAN(BP)	BINF 747
	VS=VS+APS*SPS*SIN(THETA)	BINF 748
	AX=APS*SPS*COS(THETA)	BINF 749
	AXS=AXS+AX	BINF 750
	AMS=AMS+AX*(CP+CGTE(1))	BINF 751
C		BINF 752
	THETA=ATAN(2.0*AP*XL+BP)	BINF 753
	VE=VE-APS*SPS*SIN(THETA)	BINF 754
	AX=APS*SPS*COS(THETA)	BINF 755
	AXE=AXE-AX	BINF 756
	AME=AME-AX*(AP*XL*XL+BP*XL+CP+CGTE(NPERBM))	BINF 757
C		BINF 758
	W=APS*SPS*2.0*AP	BINF 759
	I=3	BINF 760
	WRITE (IR) NAME,I	BINF 761
	WRITE (IR) W	BINF 762
C		BINF 763
C	ASSIGN LAYER PROPERTIES	BINF 764
C		BINF 765
	XB=0.	BINF 766
	XA=0.	BINF 767
	J=NULAYR+NULAYE+JJ	BINF 768
	DO 890 I=1,NPERBM	BINF 769
	XB=XB+XSEG(I)	BINF 770
	ASXLR(J,I)=APS	BINF 771
	SIBXXA(J,I)=SPS	BINF 772
	ATLR(J,I)=0.0	BINF 773
	TSHEAR(J,I)=1.0	BINF 774
	ITYPE(J,I)=ITYPS	BINF 775
	ZCRD(J,I)=CGTE(I)+(XB**3-XA**3)*AP/3./(XB-XA)+(XB+XA)*BP/2.+CP	BINF 776
	XA=XB	BINF 777
	890 CONTINUE	BINF 778
C		BINF 779
C	GO TO A NEW PRESTRESS GROUP	BINF 780
C		BINF 781
	900 CONTINUE	BINF 782
C		BINF 783
C	WRITE END FORCES TO TAPE	BINF 784
C		BINF 785
		BINF 786



	NAME=5HEND	BINF 787
	I=3	BINF 788
	WRITE (IR) NAME,I	BINF 789
	WRITE (IR) VS,AXS,AMS	BINF 790
	WRITE (IR) VE,AXE,AME	BINF 791
	910 CONTINUE	BINF 792
C		BINF 793
C	PRINT ELEMENT PROPERTY ARRAYS	BINF 794
C		BINF 795
	IS=PPERBM	BINF 796
	IE=NULAYB	BINF 797
C		BINF 798
	WRITE (IO,460)	BINF 799
	WRITE (IO,920) IREADS	BINF 800
	920 FORMAT (1H0,5X,*READ NUMBER = *,I3,/,6X,*BEAM ELEMENT/LAYER PROPE	BINF 801
	1RTIES (LAYERS VERTICAL, ELEMENTS HORIZONTAL)*)	BINF 802
C		BINF 803
	WRITE (IO,460)	BINF 804
	WRITE (IO,930)	BINF 805
	930 FORMAT (1H0,5X,*ELEMENTS- INITIAL STRESS (KSI) IN LAYERS*)	BINF 806
	CALL WRITEF (IE,IS,1,IE,1,IS,IO,SIBXXA)	BINF 807
C		BINF 808
	WRITE (IO,940)	BINF 809
	940 FORMAT (1H0,5X,*ELEMENTS- MATERIAL TYPE FOR LAYERS*)	BINF 810
	CALL WRITEI (IE,IS,1,IE,1,IS,IO,ITYPE)	BINF 811
C		BINF 812
	WRITE (IO,950)	BINF 813
	950 FORMAT (1H0,5X,*ELEMENTS- AREAS (SQ.IN) OF LAYERS*)	BINF 814
	CALL WRITEF (IE,IS,1,IE,1,IS,IO,ASXLR)	BINF 815
C		BINF 816
C		BINF 817
	WRITE (IO,960)	BINF 818
	960 FORMAT (1H0,5X,*ELEMENTS- MOMENT OF INERTIA OF LAYERS*)	BINF 819
	CALL WRITEF (IE,IS,1,IE,1,IS,IO,AILR)	BINF 820
C		BINF 821
	WRITE (IO,970)	BINF 822
	970 FORMAT (1H0,5X,*ELEMENTS- CENTROIDAL DISTANCE (IN) OF LAYERS FROM	BINF 823
	1REFERENCE PLANE*)	BINF 824
	CALL WRITEF (IE,IS,1,IE,1,IS,IO,ZCRD)	BINF 825
C		BINF 826
	WRITE (IO,980)	BINF 827
	980 FORMAT (1H0,5X,*ELEMENTS- SHEAR THICKNESS (IN) OF LAYERS*)	BINF 828
	CALL WRITEF (IE,IS,1,IE,1,IS,IO,TSHEAR)	BINF 829
C		BINF 830
C	WRITE ELEMENT PROPERTY ARRAYS TO TAPE	BINF 831
C		BINF 832
	DO 1010 III=NL,NT	BINF 833
C		BINF 834
	DO 1000 II=1,PPERBM	BINF 835
	J=II+(III-1)*PPERBM	BINF 836
	I=II	BINF 837
C		BINF 838
	DO 990 N=1,NULAYB	BINF 839
	990 ESX(N,J)=ESXX(ITYPE(N,I))	BINF 840
C		BINF 841
	GKEIX(I)=0.	BINF 842
	WRITE (27) XSEG(I),GKEIX(I),NPIX(J),NPKX(J),(ITYPE(N,I),N=1,NULAYB	BINF 843
	1),(ASXLR(N,I),N=1,NULAYB),(AILR(N,I),N=1,NULAYB),(ZCRD(N,I),N=1,NUBINF	BINF 844

	2LAYB),(TSHEAR(N,I),N=1,NULAYB)	BINF 845
C	WRITE (15) (SIBXXA(N,I),N=1,NULAYB)	BINF 846
		BINF 847
C	DO 1000 JJ=1,2	BINF 848
	I=II+JJ-2	BINF 849
	IF (I.EQ.0.OR.I.GT.NPERBM) GO TO 1000	BINF 850
	WRITE (14) XSEG(I),GKEIX(I),(ITYPE(N,I),N=1,NULAYE),(ASXLR(N,I),N=	BINF 851
	11,NULAYB),(AILR(N,I),N=1,NULAYB),(ZCRD(N,I),N=1,NULAYB),(TSHEAR(N,	BINF 852
	2I),N=1,NULAYB)	BINF 853
	1000 CONTINUE	BINF 854
C		BINF 855
		BINF 856
	I=NPERBM	BINF 857
	WRITE (14) XSEG(I),GKEIX(I),(ITYPE(N,I),N=1,NULAYB),(ASXLR(N,I),N=	BINF 858
	11,NULAYB),(AILR(N,I),N=1,NULAYE),(ZCRD(N,I),N=1,NULAYB),(TSHEAR(N,	BINF 859
	2I),N=1,NULAYB)	BINF 860
C		BINF 861
	1010 CONTINUE	BINF 862
	IF (NT.GE.NCBM) GO TO 1030	BINF 863
	NL=NT+1	BINF 864
C		BINF 865
C	GO TO ANOTHER GROUP OF BEAMS	BINF 866
C		BINF 867
	1020 CONTINUE	BINF 868
C		BINF 869
	1030 CONTINUE	BINF 870
C		BINF 871
	DO 1040 M=1,NUMNP	BINF 872
	N1=NXL(M)	BINF 873
	N2=NXR(M)	BINF 874
	WRITE (13) NPARX(M),N1,N2,NPIX(N1),NPKX(N2)	BINF 875
	1040 CONTINUE	BINF 876
C		BINF 877
	REWIND 33	BINF 878
	WRITE (33) ESX	BINF 879
C		BINF 880
	1050 CONTINUE	BINF 881
	RETURN	BINF 882
	END	BINF 883
	SUBROUTINE BEAMT (NULAYB,NPERBM,IRS,IRE,ICS,ICE,IN,IO,KIN,NAME,S,KBEAMT	2
	1,B,T,NAME3)	BEAMT 3
C		BEAMT 4
C	BEAM TABLES	BEAMT 5
C		BEAMT 6
	REAL K	BEAMT 7
	DIMENSION S(NULAYB,NPERBM), K(NULAYB,NPERBM), B(NULAYB,NPERBM), T(BEAMT	8
	1NULAYB,NPERBM)	BEAMT 9
C		BEAMT 10
	WRITE (IO,10)	BEAMT 11
	10 FORMAT (1H ,20X,*BEAM LAYERING WILL BE GENERATED*)	BEAMT 12
	ILOC=0	BEAMT 13
	IF (NAME.EQ.5HSHO-1) ILOC=1	BEAMT 14
	IF (NAME.EQ.5HSHO-2) ILOC=2	BEAMT 15
	IF (NAME.EQ.5HSHO-3) ILOC=3	BEAMT 16
	IF (NAME.EQ.5HSHO-4) ILOC=4	BEAMT 17
	IF (NAME.EQ.5HSHO-5) ILOC=5	BEAMT 18
	IF (NAME.EQ.5HSHO-6) ILOC=6	BEAMT 19
	IF (NAME.EQ.5H18/30) ILOC=7	BEAMT 20

IF (NAME.EQ.5H20/30) ILOC=8	BEAMT 21
IF (NAME.EQ.5H18/33) ILOC=9	BEAMT 22
IF (NAME.EQ.5H20/33) ILOC=10	BEAMT 23
IF (NAME.EQ.5H24/33) ILOC=11	BEAMT 24
IF (NAME.EQ.5H26/33) ILOC=12	BEAMT 25
IF (NAME.EQ.5H18/36) ILOC=13	BEAMT 26
IF (NAME.EQ.5H20/36) ILOC=14	BEAMT 27
IF (NAME.EQ.5H24/36) ILOC=15	BEAMT 28
IF (NAME.EQ.5H26/36) ILOC=16	BEAMT 29
IF (NAME.EQ.5H20/39) ILOC=17	BEAMT 30
IF (NAME.EQ.5H24/42) ILOC=18	BEAMT 31
IF (NAME.EQ.5H24/45) ILOC=19	BEAMT 32
IF (NAME.EQ.5H24/48) ILOC=20	BEAMT 33
IF (NAME.EQ.5H24/51) ILOC=21	BEAMT 34
IF (NAME.EQ.5H24/54) ILOC=22	BEAMT 35
IF (NAME.EQ.5H24/60) ILOC=23	BEAMT 36
IF (NAME.EQ.5H26/60) ILOC=24	BEAMT 37
IF (NAME.EQ.5H26/63) ILOC=25	BEAMT 38
C	BEAMT 39
IF (ILOC.GT.0) GO TO 40	BEAMT 40
C	BEAMT 41
WRITE (IO,20)	BEAMT 42
20 FORMAT (1H0)	BEAMT 43
WRITE (IO,30)	BEAMT 44
30 FORMAT (1H0,5X,*NO TRANSLATION*,//)	BEAMT 45
STOP	BEAMT 46
C	BEAMT 47
40 CONTINUE	BEAMT 48
C	BEAMT 49
IF (IRE.EQ.10) GO TO 60	BEAMT 50
C	BEAMT 51
WRITE (IO,50) IRE	BEAMT 52
50 FORMAT (//,1H0,5X,*NUMBER OF CONCRETE LAYERS MUST EQUAL 10 INSTEAD	BEAMT 53
1 OF *,I3,//)	BEAMT 54
STOP	BEAMT 55
C	BEAMT 56
60 CONTINUE	BEAMT 57
DO 70 I=IRS,IRE	BEAMT 58
DO 70 J=ICS,ICE	BEAMT 59
S(I,J)=0.	BEAMT 60
70 K(I,J)=1.	BEAMT 61
CALL BA (ILOC,B,T,IRS,IRE,ICS,ICE,NULAYB,NPERBM)	BEAMT 62
C	BEAMT 63
IF (NAME3.NE.3HH ) RETURN	BEAMT 64
DO 80 I=IRS,IRE	BEAMT 65
DO 80 J=ICS,ICE	BEAMT 66
80 B(I,J)=B(I,J)/2.	BEAMT 67
C	BEAMT 68
RETURN	BEAMT 69
END	BEAMT 70
SUBROUTINE BA (ILOC,B,T,IRS,IRE,ICS,ICE,NULAYB,NPERBM)	BA 2
C	BA 3
C THIS SUBROUTINE SELECTS BEAM DIMENSIONS	BA 4
C THEN LAYER WIDTHS AND THICKNESSES	BA 5
C	BA 6
DIMENSION B(NULAYB,NPERBM), T(NULAYB,NPERBM)	BA 7
C	BA 8
GO TO (10,20,30,40,50,60,70,80,90,100,110,120,130,140,150,160,170,BA	9

1180,190,200,210,220,230,240,250), ILOC

C		BA	10
		BA	11
	10 CONTINUE	BA	12
C	TYPE 1	BA	13
C		BA	14
	D1=4.	BA	15
	D2=3.	BA	16
	D3=11.	BA	17
	D4=5.	BA	18
	D5=5.	BA	19
	B1=12.	BA	20
	B2=16.	BA	21
	B3=6.	BA	22
	GO TO 260	BA	23
C		BA	24
	20 CONTINUE	BA	25
C	TYPE 2	BA	26
C		BA	27
	D1=6.	BA	28
	D2=3.	BA	29
	D3=15.	BA	30
	D4=6.	BA	31
	D5=6.	BA	32
	B1=12.	BA	33
	B2=18.	BA	34
	B3=6.	BA	35
	GO TO 260	BA	36
C		BA	37
	30 CONTINUE	BA	38
C	TYPE 3	BA	39
C		BA	40
	D1=7.	BA	41
	D2=4.5	BA	42
	D3=19.	BA	43
	D4=7.5	BA	44
	D5=7.	BA	45
	B1=16.	BA	46
	B2=22.	BA	47
	B3=7.	BA	48
	GO TO 260	BA	49
C		BA	50
	40 CONTINUE	BA	51
C	TYPE 4	BA	52
C		BA	53
	D1=8.	BA	54
	D2=6.	BA	55
	D3=23.	BA	56
	D4=9.	BA	57
	D5=8.	BA	58
	B1=20.	BA	59
	B2=26.	BA	60
	B3=8.	BA	61
	GO TO 260	BA	62
C		BA	63
	50 CONTINUE	BA	64
C	TYPE 5	BA	65
C		BA	66
	D1=5.	BA	67

	D2=3.	BA	68
	D3=37.	BA	69
	D4=10.	BA	70
	D5=8.	BA	71
	B1=42.	PA	72
	B2=28.	BA	73
	B3=8.	BA	74
	GO TO 260	BA	75
C		BA	76
	60 CONTINUE	BA	77
C	TYPE 6	BA	78
C		BA	79
	D1=5.	BA	80
	D2=3.	BA	81
	D3=46.	BA	82
	D4=10.	BA	83
	D5=8.	BA	84
	B1=42.	PA	85
	B2=28.	BA	86
	B3=8.	BA	87
	GO TO 260	BA	88
C		BA	89
	70 CONTINUE	BA	90
C	18/30	BA	91
C		BA	92
	D1=3.	BA	93
	D2=3.	PA	94
	D3=12.	BA	95
	D4=8.	BA	96
	D5=4.	BA	97
	B1=12.	BA	98
	B2=18.	BA	99
	B3=6.	BA	100
	GO TO 260	BA	101
C		BA	102
	80 CONTINUE	BA	103
C	20/30	BA	104
C		BA	105
	D1=3.	PA	106
	D2=3.	BA	107
	D3=12.	BA	108
	D4=8.	BA	109
	D5=4.	PA	110
	B1=14.	BA	111
	B2=20.	BA	112
	B3=8.	BA	113
	GO TO 260	BA	114
C		BA	115
	90 CONTINUE	BA	116
C	18/33	BA	117
C		BA	118
	D1=4.	PA	119
	D2=3.	BA	120
	D3=12.	BA	121
	D4=8.	BA	122
	D5=6.	PA	123
	B1=12.	BA	124
	B2=18.	BA	125

	B3=6.	BA	126
	GO TO 260	BA	127
C		BA	128
C	100 CONTINUE	BA	129
C	20/33	BA	130
C		BA	131
	D1=4.	BA	132
	D2=3.	BA	133
	D3=12.	BA	134
	D4=8.	BA	135
	D5=6.	BA	136
	B1=14.	BA	137
	B2=20.	BA	138
	B3=8.	BA	139
	GO TO 260	BA	140
C		BA	141
C	110 CONTINUE	BA	142
C	24/33	BA	143
C		BA	144
	D1=4.	BA	145
	D2=3.	BA	146
	D3=12.	BA	147
	D4=8.	BA	148
	D5=6.	BA	149
	B1=18.	BA	150
	B2=24.	BA	151
	B3=12.	BA	152
	GO TO 260	BA	153
C		BA	154
C	120 CONTINUE	BA	155
C	26/33	BA	156
C		BA	157
	D1=4.	BA	158
	D2=3.	BA	159
	D3=12.	BA	160
	D4=8.	BA	161
	D5=6.	BA	162
	B1=20.	BA	163
	B2=26.	BA	164
	B3=14.	BA	165
	GO TO 260	BA	166
C		BA	167
C	130 CONTINUE	BA	168
C	18/36	BA	169
C		BA	170
	D1=5.	BA	171
	D2=3.	BA	172
	D3=12.	BA	173
	D4=8.	BA	174
	D5=8.	BA	175
	B1=12.	BA	176
	B2=18.	BA	177
	B3=6.	BA	178
	GO TO 260	BA	179
C		BA	180
C	140 CONTINUE	BA	181
C	20/36	BA	182
C		BA	183

	D1=5.	BA	184
	D2=3.	BA	185
	D3=12.	BA	186
	D4=8.	BA	187
	D5=8.	BA	188
	31=14.	BA	189
	B2=20.	BA	190
	B3=8.	BA	191
	GO TO 260	BA	192
C	150 CONTINUE	BA	193
C	24/36	BA	194
C		BA	195
	D1=5.	BA	196
	D2=3.	BA	197
	D3=12.	BA	198
	D4=8.	BA	199
	D5=8.	BA	200
	B1=18.	BA	201
	B2=24.	BA	202
	B3=12.	BA	203
	GO TO 260	BA	204
C	160 CONTINUE	BA	205
C	26/36	BA	206
C		BA	207
	D1=5.	BA	208
	D2=3.	BA	209
	D3=12.	BA	210
	D4=8.	BA	211
	D5=8.	BA	212
	B1=20.	BA	213
	B2=26.	BA	214
	B3=14.	BA	215
	GO TO 260	BA	216
C	170 CONTINUE	BA	217
C	20/39	BA	218
C		BA	219
	D1=8.	BA	220
	D2=3.	BA	221
	D3=12.	BA	222
	D4=8.	BA	223
	D5=8.	BA	224
	B1=14.	BA	225
	B2=20.	BA	226
	B3=8.	BA	227
	GO TO 260	BA	228
C	180 CONTINUE	BA	229
C	24/42	BA	230
C		BA	231
	D1=4.	BA	232
	D2=4.	BA	233
	D3=17.	BA	234
	D4=10.	BA	235
	D5=7.	BA	236
	B1=18.	BA	237
		BA	238
		BA	239
		BA	240
		BA	241

B2=24.  
B3=8.  
GO TO 260

BA 242  
BA 243  
BA 244

C  
190 CONTINUE  
C 24/45

BA 245  
BA 246  
BA 247

C  
D1=7.  
D2=4.

BA 248  
BA 249  
BA 250

D3=17.  
D4=10.  
D5=7.

BA 251  
BA 252  
BA 253

B1=18.  
B2=24.  
B3=8.

BA 254  
BA 255  
BA 256

GO TO 260

BA 257

C  
200 CONTINUE

BA 258  
BA 259

C 24/48  
C

BA 260  
BA 261

D1=8.

BA 262

D2=4.

BA 263

D3=17.

BA 264

D4=10.

BA 265

D5=9.

BA 266

B1=18.

BA 267

B2=24.

BA 268

B3=8.

BA 269

GO TO 260

BA 270

C  
210 CONTINUE

BA 271  
BA 272

C 24/51  
C

BA 273  
BA 274

D1=11.

BA 275

D2=4.

BA 276

D3=17.

BA 277

D4=10.

BA 278

D5=9.

BA 279

B1=18.

BA 280

B2=24.

BA 281

B3=8.

BA 282

GO TO 260

BA 283

C  
220 CONTINUE

BA 284  
BA 285

C 24/54

BA 286

C

BA 287

D1=14.

BA 288

D2=4.

BA 289

D3=17.

BA 290

D4=10.

BA 291

D5=9.

BA 292

B1=18.

BA 293

B2=24.

BA 294

B3=8.

BA 295

GO TO 260

BA 296

C  
230 CONTINUE

BA 297  
BA 298

C 24/60

BA 299



		BA	300
	D1=6.	BA	301
	D2=6.	BA	302
	D3=29.	BA	303
	D4=10.	BA	304
	D5=9.	BA	305
	B1=24.	BA	306
	B2=24.	BA	307
	B3=8.	BA	308
	GO TO 260	BA	309
C		BA	310
	240 CONTINUE	BA	311
C		BA	312
	26/60	BA	313
C		BA	314
	D1=6.	BA	315
	D2=6.	BA	316
	D3=29.	BA	317
	D4=10.	BA	318
	D5=9.	BA	319
	B1=26.	BA	320
	B2=26.	BA	321
	B3=10.	BA	322
	GO TO 260	BA	323
C		BA	324
	250 CONTINUE	BA	325
C		BA	326
	26/63	BA	327
C		BA	328
	D1=9.	BA	329
	D2=3.	BA	330
	D2=6.	BA	331
	D3=29.	BA	332
	D4=10.	BA	333
	D5=9.	BA	334
	B1=26.	BA	335
	B2=26.	BA	336
	B3=10.	BA	337
C		BA	338
	260 CONTINUE	BA	339
	I=IRS	BA	340
	B(I,1)=B1	BA	341
	I=I+1	BA	342
	B(I,1) = B1	BA	343
	I=I+1	BA	344
	B(I,1) = (B1+B3) / 2.	BA	345
	I=I+1	BA	346
	B(I,1)=B3	BA	347
	I=I+1	BA	348
	B(I,1)=B2/4.+0.75*B3	BA	349
	I=I+1	BA	350
	B(I,1)=B3/4.+0.75*B2	BA	351
	I=I+1	BA	352
	B(I,1)=B2	BA	353
	I=I+1	BA	354
	B(I,1)=B2	BA	355
	I=I+1	BA	356
	B(I,1) = B2	BA	357

C	I=IRS	BA	358
	T(I,1)=2.0	BA	359
	I=I+1	BA	360
	T(I,1) = D1-2.0	BA	361
	I = I+1	BA	362
	T(I,1)=D2	BA	363
	I=I+1	BA	364
	T(I,1)=D3/2.	BA	365
	I=I+1	BA	366
	T(I,1)=D3/2.	BA	367
	I=I+1	BA	368
	T(I,1)=D4/2.	BA	369
	I=I+1	BA	370
	T(I,1)=D4/2.	BA	371
	I=I+1	BA	372
	T(I,1)=D5/2.0-1.	BA	373
	I=I+1	BA	374
	T(I,1)=D5/2.0-1.	BA	375
	I = I+1	BA	376
	T(I,1) = 2.	BA	377
C		BA	378
	DO 270 I=IRS,IRE	BA	379
	DO 270 J=ICS,ICE	BA	380
	B(I,J)=B(I,1)	BA	381
270	T(I,J)=T(I,1)	BA	382
C		BA	383
	RETURN	BA	384
	END	BA	385
	SUBROUTINE FIND (A,E,NB,C,D)	BA	386
	COMMON /CB4/ IN,IO	FIND	2
	DIMENSION B(NB)	FIND	3
C		FIND	4
	TABLE LOOK UP AND INTERPOLATION	FIND	5
C	C IS INDEPENDENT DATA	FIND	6
C	B IS TABLE	FIND	7
C	D IS MULTIPLICATION FACTOR	FIND	8
C	A IS RESULT	FIND	9
C		FIND	10
	N=NB/2.	FIND	11
	DO 10 I=1,N	FIND	12
	IF (B(I).GT.C) GO TO 20	FIND	13
10	CONTINUE	FIND	14
20	CONTINUE	FIND	15
	A=(B(N+I)-B(N+I-1))/(B(I)-B(I-1))*(C-B(I-1))+B(N+I-1)*D	FIND	16
	RETURN	FIND	17
	END	FIND	18
	SUBROUTINE WRITEI (NULAYB,NPERBM,IRS,IRE,ICS,ICE,IO,A)	FIND	19
C		WRITEI	2
	INTEGER A	WRITEI	3
	DIMENSION A (NULAYB,NPERBM)	WRITEI	4
C		WRITEI	5
	WRITE (IO,10)	WRITEI	6
10	FORMAT (1H )	WRITEI	7
	DO 20 I=IRS,IRE	WRITEI	8
20	WRITE (IO,30) (A(I,J),J=ICS,ICE)	WRITEI	9
30	FORMAT (1H0,5X,10I5,(/,11X,9I5))	WRITEI	10
	WRITE (IO,10)	WRITEI	11
		WRITEI	12

C	RETURN	WRITEI13
	END	WRITEI14
		WRITEI15
C	SUBROUTINE WRITEF (NULAYB,NPERBM,IRS,IRE,ICS,ICE,IO,A)	WRITEF 2
		WRITEF 3
	DIMENSION A(NULAYB,NPERBM)	WRITEF 4
C		WRITEF 5
	WRITE (IO,10)	WRITEF 6
	10 FORMAT (1H )	WRITEF 7
	DO 20 I=IRS,IRE	WRITEF 8
	20 WRITE (IO,30) (A(I,J),J=ICS,ICE)	WRITEF 9
	30 FORMAT (1H0,5X,10F12.4,(/,18X,9F12.4))	WRITEF10
	WRITE (IO,10)	WRITEF11
C		WRITEF12
	RETURN	WRITEF13
	END	WRITEF14
C	SUBROUTINE SUBST (PA,PD,NA,NR,L)	SUBST 2
		SUBST 3
	DIMENSION PA(NA), PD(NA,NR)	SUBST 4
C		SUBST 5
	DO 10 I=1,NA	SUBST 6
	10 PA(I)=PD(I,L)	SUBST 7
C		SUBST 8
	RETURN	SUBST 9
	END	SUBST 10
C	SUBROUTINE AVGER (PD,PE,D,NRE,NR,NEX,NPERBM,XSEG,NX,PA,PB)	AVGER 2
		AVGER 3
C		AVGER 4
	THIS SUBROUTINE AVERAGES SECTION PROPERTIES TO GET ELEMENT	AVGER 5
C	PROPERTY	AVGER 6
		AVGER 7
	DIMENSION PD(NRE,NR), PE(NRE,NPERBM), D(NR), PA(NRE), PB(NRE), XSEG(NRE,NR), XSEG(NRE,NR), XSEG(NRE,NR), XSEG(NRE,NR)	AVGER 8
C		AVGER 9
	NODE I PROPERTIES IN VECTOR PA	AVGER 10
C		AVGER 11
	NODE J PROPERTIES IN VECTOR PB	AVGER 12
C		AVGER 13
	FIRST ELEMENT- NODE I	AVGER 14
C		AVGER 15
	XA=0.0	AVGER 16
	XB=0.0	AVGER 17
	IF (XA.LE.D(1)) CALL SUBST (PA,PD,NRE,NR,1)	AVGER 18
	IF (XA.GT.D(NX)) CALL SUBST (PA,PD,NRE,NR,NX)	AVGER 19
C		AVGER 20
	DO 150 II=1,NPERBM	AVGER 21
C		AVGER 22
	FIND PROP OF NODE J FOR ELEMENT II	AVGER 23
C		AVGER 24
	XB=XB+XSEG(II)	AVGER 25
	IF (XB.LE.D(1)) CALL SUBST (PB,PD,NRE,NR,1)	AVGER 26
	IF (XB.GE.D(NX)) CALL SUBST (PB,PD,NRE,NR,NX)	AVGER 27
	IF (NX.EQ.1) GO TO 40	AVGER 28
C		AVGER 29
	CHECK IF NODE IS BETWEEN SPECIFIED PROP	AVGER 30
C		AVGER 31
	NN=NX-1	AVGER 32
	DO 10 J=1,NN	AVGER 33
	IF (XB.GE.D(J).AND.XB.LE.D(J+1)) GO TO 20	AVGER 34
	10 CONTINUE	AVGER 34

C	GO TO 40	AVGER 35
C		AVGER 36
C		AVGER 37
C	20 CONTINUE	AVGER 38
C	NODE IS BETWEEN J AND J+1 SECTIONS	AVGER 39
C	FIND AVERAGE PROP	AVGER 40
C		AVGER 41
C		AVGER 42
C	$A = (XB - D(J)) / (D(J) - D(J+1))$	AVGER 43
C	DO 30 I=1,NRE	AVGER 44
C	30 $PB(I) = A * (PD(I, J) - PD(I, J+1)) + PD(I, J)$	AVGER 45
C		AVGER 46
C	40 CONTINUE	AVGER 47
C	IE=0	AVGER 48
C	IS=0	AVGER 49
C		AVGER 50
C	FIND SECTIONS THAT ARE IN BETWEEN NODES I AND J OF ELEMENT II	AVGER 51
C		AVGER 52
C	DO 60 J=1,NX	AVGER 53
C	IF (IS.NE.0) GO TO 50	AVGER 54
C	A=D(J)	AVGER 55
C	IF (A.GT.XA.AND.A.LT.XB) IS=J	AVGER 56
C	50 CONTINUE	AVGER 57
C	IF (IE.NE.0) GO TO 60	AVGER 58
C	A=D(NX-J+1)	AVGER 59
C	IF (A.GT.XA.AND.A.LT.XB) IE=NX-J+1	AVGER 60
C	60 CONTINUE	AVGER 61
C		AVGER 62
C	IF (IS.NE.0) GO TO 80	AVGER 63
C		AVGER 64
C	THERE ARE NO SECTIONS BETWEEN NODES	AVGER 65
C		AVGER 66
C	DO 70 I=1,NRE	AVGER 67
C	70 $PE(I, II) = (PA(I) + PB(I)) / 2.0$	AVGER 68
C	GO TO 130	AVGER 69
C		AVGER 70
C	80 CONTINUE	AVGER 71
C		AVGER 72
C	INITIALIZE ELEMENT PROPERTY MATRIX	AVGER 73
C		AVGER 74
C	DO 90 I=1,NRE	AVGER 75
C	90 $PE(I, II) = 0.$	AVGER 76
C		AVGER 77
C	IF (IS.EQ.IE) GO TO 110	AVGER 78
C		AVGER 79
C	STARTING SECTION IS IS	AVGER 80
C	END SECTION IS IE	AVGER 81
C	AVERAGE SECTIONS BETWEEN NODE POINTS	AVGER 82
C		AVGER 83
C	IE=IE-1	AVGER 84
C	DO 100 J=IS,IE	AVGER 85
C	$A = (D(J+1) - D(J)) / 2.0$	AVGER 86
C	DO 100 I=1,NRE	AVGER 87
C	100 $PE(I, II) = A * (PD(I, J+1) + PD(I, J)) + PE(I, II)$	AVGER 88
C	IE=IE+1	AVGER 89
C		AVGER 90
C	ADD IN THE END PORTIONS	AVGER 91
C		AVGER 92

110	CONTINUE	AVGER 93
	C=XSEG(II)	AVGER 94
	A=(D(IS)-XA)/2.0/C	AVGER 95
	B=(XB-D(IE))/2.0/C	AVGER 96
C		AVGER 97
	DO 120 I=1,NRE	AVGER 98
	PE(I,II)=A*(PD(I,IS)+PA(I))+B*(PD(I,IE)+PB(I))+PE(I,II)/C	AVGER 99
120	CONTINUE	AVGER100
C		AVGER101
	130 CONTINUE	AVGER102
C		AVGER103
C	SET NODE I EQUAL TO NODE J	AVGER104
C		AVGER105
	XA=XB	AVGER106
	DO 140 I=1,NRE	AVGER107
	140 PA(I)=PB(I)	AVGER108
C		AVGER109
C	GO TO NEXT ELEMENT	AVGER110
C		AVGER111
	150 CONTINUE	AVGER112
	RETURN	AVGER113
	END	AVGER114
	SUBROUTINE READF (NULAYB,NPERBM,IRS,IRE,ICS,ICE,IN,KIN,A)	READF 2
C		READF 3
	DIMENSION A(NULAYB,NPERBM)	READF 4
C		READF 5
	DO 30 I=IRS,IRE	READF 6
	NC=ICS	READF 7
	IF (KIN.EQ.1) NC=ICE	READF 8
	READ (IN,10) (A(I,J),J=ICS,NC)	READF 9
	10 FORMAT (8F10.0)	READF 10
	IF (KIN.EQ.1) GO TO 30	READF 11
C		READF 12
	DO 20 J=ICS,ICE	READF 13
	20 A(I,J)=A(I,ICS)	READF 14
C		READF 15
	30 CONTINUE	READF 16
	RETURN	READF 17
	END	READF 18
	SUBROUTINE READI (NULAYB,NPERBM,IRS,IRE,ICS,ICE,IN,KIN,A)	READI 2
	INTEGER A	READI 3
	DIMENSION A(NULAYB,NPERBM)	READI 4
C		READI 5
	DO 30 I=IRS,IRE	READI 6
	NC=ICS	READI 7
	IF (KIN.EQ.1) NC=ICE	READI 8
	READ (IN,10) (A(I,J),J=ICS,NC)	READI 9
	10 FORMAT (16I5)	READI 10
	IF (KIN.EQ.1) GO TO 30	READI 11
C		READI 12
	DO 20 J=ICS,ICE	READI 13
	20 A(I,J)=A(I,ICS)	READI 14
C		READI 15
	30 CONTINUE	READI 16
	RETURN	READI 17
	END	READI 18
	OVERLAY (ECCP,2,0)	OVERL2 2
	PROGRAM OVERL2	OVERL2 3

C		OVERL2 4
C	THIS OVERLAY FORMULATES STIFFNESS MATRIX	OVERL2 5
C		OVERL2 6
	COMMON /CBS301/ COUPLE	OVERL2 7
	COMMON /PROPC/ V(11)	OVERL2 8
	COMMON /COMPLT/ ICMFLT	OVERL2 9
	COMMON /TU00/ TU0(8,12)	OVERL210
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	OVERL211
	COMMON /CB2/ NBEAMX,NIX,NULAYB	OVERL212
	COMMON /CB3/ LWIDTH,NITA,NPSEGM	OVERL213
	COMMON /CB309/ FTCL,CTOL	OVERL214
	COMMON /CB310/ NELX,NSMAT	OVERL215
	COMMON /BDIM/ RON(48)	OVERL216
	COMMON /PDIM/ PDT(70)	OVERL217
	COMMON /CB4/ IN,IO	OVERL218
	COMMON /CB300/ ISPACE(4),KC,KMAT	OVERL219
	COMMON /ICORES/ ICR(1),ICCRE	OVERL220
	COMMON /OVER2/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13,N14,N15,	OVERL221
	1N16,N17,N18,N19,N20,N21,N22	OVERL222
	COMMON A(1)	OVERL223
C		OVERL224
C	IF (ICORE.NE.0) GO TO 10	OVERL225
	ICORE=LOGF(A(1))+1	OVERL226
	GO TO 20	OVERL227
	10 CONTINUE	OVERL228
	NA5=NA1+LWIDTH-1	OVERL229
	LAMA=5*NPSEGM	OVERL230
	CALL PBS (NUMEL,NUMNP,NA1,NULAY,NSLAYR,NCNS,LAMA,LWIDTH,NA5,NULAYB,	OVERL231
	1,NBEAMX,NANA,NPSEGM,NIX,KC,KMAT,A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),	OVERL232
	2),A(N7),A(N8),A(N9),A(N10),A(N11),A(N12),A(N13),A(N14),A(N15),A(N16),	OVERL233
	36),A(N17),A(N18),A(N19),A(N20),A(N21))	OVERL234
	20 CONTINUE	OVERL235
	END	OVERL236
	SUBROUTINE PBS (NUMEL,NUMNP,NA1,NULAY,NSLAYR,NCNS,LAMA,LWIDTH,NA5,PBS	OVERL237
	1NULAYB,NBEAMX,NANA,NPSEGM,NIX,KC,KMAT,FORCE,LILA,AS1,AF,AS,SIGXX0,PBS	2
	2SIGYY0,SIGXY0,SXX0T,SYOT,SXYOT,ALL,OVK,SIBXXA,SIBXXC,ASXLR,AILR,ZP	3
	3CRD,TSHEAR,ITYPE,ESX)	4
		PBS 5
		PBS 6
C		PBS 7
C	THIS SUBROUTINE FORMULATES GLOBAL STIFFNESS MATRIX	PBS 8
C		PES 9
	COMMON /PDIM/ PDT(15),PZC(15),SEMOD(4),SIGMAP(4),SPRON(4),SPROM(4)	PBS 10
	1,NSTYPE(6),SDT(6),SZC(6),SPHI(6)	PBS 11
	COMMON /BDIM/ RON(6),ROM(6),EDOWN(6),STRAN(6),SIGO(6),FT(6),RONT(3	PBS 12
	1),ROMT(3),EDOWNT(3),EDOWNR(3)	PBS 13
	COMMON /CBS301/ COUPLE	PBS 14
	COMMON /CB309/ FTCL,CTOL	PBS 15
	COMMON /CB310/ NELX,NSMAT	PBS 16
	COMMON /TU00/ TU0(8,12)	PBS 17
	COMMON /CB09/ S1,S2,S3,S4	PBS 18
	COMMON /CB19/ BBS1,BBS2	PBS 19
	DIMENSION FORCE(NA1),LILA(NA1),AS1(NUMEL,NCNS),AF(NUMEL,NULAY),	PBS 20
	1 AS(NUMEL,NULAY),SIGXX0(NCNS),SIGYY0(NCNS),SIGXY0(NCNS),SXX0T(PBS	PBS 21
	2NCNS),SYOT(NCNS),SXYOT(NCNS),ALL(LWIDTH,LAMA),OVK(5,NA5),SIBP	PBS 22
	3XXA(NULAYB),SIBXXC(NULAYB),ASXLR(NULAYB),AILR(NULAYB),ZCRD(NULP	PBS 23
	4AYB),TSHEAR(NULAYB),ITYPE(NULAYB),ESX(NULAYB,NBEAMX),NZ(4)	PBS 24
	DIMENSION S1(4,5,5),S2(4,5,5),S3(4,5,5),S4(4,5,5),BBS1(2,5,5),	PBS 25
	1 BBS2(2,5,5),NL(2)	PBS 25

		PBS	
	DIMENSION MM(4,4)	26	
C		27	
C		28	
	REWIND 11	29	
	REWIND 12	30	
	REWIND 20	31	
	REWIND 22	32	
	REWIND 13	33	
	REWIND 14	34	
	REWIND 16	35	
	REWIND 24	36	
	REWIND 29	37	
	REWIND 31	38	
	REWIND 33	39	
	READ (29) FORCE,LILA	40	
	READ (31) AS1,AF,AS	41	
	READ (33) ESX	42	
C		43	
	COMET=1.0E30	44	
	K=0	45	
	REWIND 3	46	
	ISEGM=NUMNP/NPSEGM	47	
	KOUNT=G	48	
C		49	
C	FIND NUMBER OF NODE POINTS IN EQUATION BLOCK	50	
C		51	
	GO TO 20	52	
	10 IF (KLX2.EQ.NUMNP) GO TO 130	53	
	KLX1=1+KOUNT*NPSEGM	54	
	KLX2=NUMNP	55	
	GO TO 30	56	
	20 IF (KOUNT.GE.ISEGM) GO TO 10	57	
	KLX1=1+KOUNT*NPSEGM	58	
	KLX2=KLX1+NPSEGM-1	59	
C		60	
C	INITIALIZE	61	
C		62	
	30 DO 40 I=1,LWIDTH	63	
	DO 40 J=1,LAMA	64	
	40 ALL(I,J)=0.	65	
	KJ=0	66	
C		67	
C	FORMULATE A PORTION OF THE STIFFNESS MATRIX	68	
C		69	
	DO 120 M=KLX1,KLX2	70	
	KJ=KJ+5	71	
	K=K+5	72	
C		73	
C	INITIALIZE	74	
C		75	
	DO 50 I=1,5	76	
	DO 50 J=1,NA5	77	
	OVK(I,J)=0.	78	
	50 CONTINUE	79	
C		80	
C	CONTRIBUTION OF PLATE ELEMENTS.	81	
C		82	
	READ (11) NZ(1),NZ(2),NZ(3),NZ(4),JN1,KN1,LN1,IN2,JN2,LN2,IN3,JN3,PBS	83	

	1KN3,IN4,KN4,LN4	PBS	84
C		PBS	85
C	ASSIGN NODAL POINTS FOR THE 4 ELEMENTS SURROUNDING NODE M	PBS	86
C	I-J-K-L SEQUENCE	PBS	87
C		PBS	88
C	FOR NZ=1	PBS	89
	MM(1,1)=M	PBS	90
	MM(1,2)=JN1	PBS	91
	MM(1,3)=KN1	PBS	92
C	MM(1,4)=LN1	PBS	93
	FOR NZ=2	PBS	94
	MM(2,1)=IN2	PBS	95
	MM(2,2)=JN2	PBS	96
	MM(2,3)=M	PBS	97
	MM(2,4)=LN2	PBS	98
C	FOR NZ=3	PBS	99
	MM(3,1)=IN3	PBS	100
	MM(3,2)=JN3	PBS	101
	MM(3,3)=KN3	PBS	102
	MM(3,4)=M	PBS	103
C	FOR NZ=4	PBS	104
	MM(4,1)=IN4	PBS	105
	MM(4,2)=M	PBS	106
	MM(4,3)=KN4	PBS	107
	MM(4,4)=LN4	PBS	108
C		PBS	109
C		PBS	110
	CALL ELSTMA (NCNS,NANA,NULAY,NUMNF,NUMEL,NSLAYR,NSMAT,M,NZ,MM,LILAPBS	PBS	111
	1,NA1,PZC,PDT,SZC,SDT,SXXOT,SYOT,SXYOT,SIGXXO,SIGYYO,SIGXYO,AS1,AFPBS	PBS	112
	2,AS,SPHI,NSTYPE,SPROM,SPRON,SEMOD,SIGMAP)	PBS	113
C		PBS	114
C	FIND MATRIX LOCATIONS	PBS	115
C		PBS	116
	I=1	PBS	117
	IP=5*M-4	PBS	118
	JN1=5*JN1-4	PBS	119
	KN1=5*KN1-4	PBS	120
	LN1=5*LN1-4	PBS	121
	IN2=5*IN2-4	PBS	122
	JN2=5*JN2-4	PBS	123
	LN2=5*LN2-4	PBS	124
	IN3=5*IN3-4	PBS	125
	JN3=5*JN3-4	PBS	126
	KN3=5*KN3-4	PBS	127
	IN4=5*IN4-4	PBS	128
	KN4=5*KN4-4	PBS	129
	LN4=5*LN4-4	PBS	130
C		PBS	131
C	STORAGE IN AUXILIARY ARRAY OVK(I,J)	PBS	132
C		PBS	133
	DO 60 II=1,5	PBS	134
	DO 60 JJ=1,5	PBS	135
	KZ=II-1	PBS	136
	LZ=JJ-1	PBS	137
	OVK(I+KZ,IP+LZ)=S1(1,II, JJ)+S3(4,II, JJ)+S2(3,II, JJ)+S4(2,II, JJ)	PBS	138
	OVK(I+KZ,JN1+LZ)=OVK(I+KZ,JN1+LZ)+S1(2,II, JJ)	PBS	139
	OVK(I+KZ,KN1+LZ)=OVK(I+KZ,KN1+LZ)+S1(3,II, JJ)	PBS	140
	OVK(I+KZ,LN1+LZ)=OVK(I+KZ,LN1+LZ)+S1(4,II, JJ)	PBS	141



	OVK(I+KZ,IN2+LZ)=OVK(I+KZ,IN2+LZ)+S2(1,JJ,II)	PBS	142
	OVK(I+KZ,JN2+LZ)=OVK(I+KZ,JN2+LZ)+S2(2,JJ,II)	PBS	143
	OVK(I+KZ,LN2+LZ)=OVK(I+KZ,LN2+LZ)+S2(4,II,JJ)	PBS	144
	OVK(I+KZ,IN3+LZ)=OVK(I+KZ,IN3+LZ)+S3(1,JJ,II)	PBS	145
	OVK(I+KZ,JN3+LZ)=OVK(I+KZ,JN3+LZ)+S3(2,JJ,II)	PBS	146
	OVK(I+KZ,KN3+LZ)=OVK(I+KZ,KN3+LZ)+S3(3,JJ,II)	PBS	147
	OVK(I+KZ,IN4+LZ)=OVK(I+KZ,IN4+LZ)+S4(1,JJ,II)	PBS	148
	OVK(I+KZ,KN4+LZ)=OVK(I+KZ,KN4+LZ)+S4(3,II,JJ)	PBS	149
	OVK(I+KZ,LN4+LZ)=OVK(I+KZ,LN4+LZ)+S4(4,II,JJ)	PBS	150
	60 CONTINUE	PBS	151
C		PBS	152
C	CONTRIBUTION OF STIFFENER ELEMENTS //X-AXIS.	PBS	153
C		PBS	154
	IF (NBEAMX.EQ.0) GO TO 90	PBS	155
	READ (13) NPARX,NXL,NXR,NPIX,NPKX	PBS	156
	NL(1)=NXL	PBS	157
	NL(2)=NXR	PBS	158
	IF (NPARX.EQ.0) GO TO 90	PBS	159
	CALL BEASTX (M,NL,NUMNP,NIX,NBEAMX,NULAYB,KC,KMAT,ZCRD,ASXLR,AILR,	PBS	160
	1ITYPE,ESX,TSHEAR,RON,ROM,SIGO,RONT,ROMT,FT,SIBXXC,SIBXXA)	PBS	161
C		PBS	162
C	FIND MATRIX LOCATIONS	PBS	163
C		PBS	164
	IL=NPIX	PBS	165
	KR=NPKX	PBS	166
	I=1	PBS	167
	IL=5*IL-4	PBS	168
	KR=5*KR-4	PBS	169
C		PBS	170
C	STORAGE IN AUXILIARY ARRAY OVM(I,J)	PBS	171
C		PBS	172
	DO 80 II=1,5	PBS	173
C		PBS	174
	IF (LILA(IP+II-1).NE.2) GO TO 70	PBS	175
	BBS2(1,II,II)=BBS2(1,II,II)*COMET	PBS	176
	BBS1(2,II,II)=BBS1(2,II,II)*COMET	PBS	177
	70 CONTINUE	PBS	178
C		PBS	179
	DO 80 JJ=1,5	PBS	180
	KZ=II-1	PBS	181
	LZ=JJ-1	PBS	182
	OVK(I+KZ,IP+LZ)=OVK(I+KZ,IP+LZ)+8BBS2(1,II,II)+BBS1(2,II,II)	PBS	183
	OVK(I+KZ,IL+LZ)=OVK(I+KZ,IL+LZ)+BBS1(1,II,II)	PBS	184
	OVK(I+KZ,KR+LZ)=OVK(I+KZ,KR+LZ)+BBS2(2,II,II)	PBS	185
	80 CONTINUE	PBS	186
C		PBS	187
C	TRANSFER INTO ARRAY ALL(I,J)	PBS	188
C		PBS	189
	90 DO 100 IA=1,LWIDTH	PBS	190
	ALL(IA,KJ-4)=OVK(1,IA+K-5)	PBS	191
	ALL(IA,KJ-3)=OVK(2,IA+K-4)	PBS	192
	ALL(IA,KJ-2)=OVK(3,IA+K-3)	PBS	193
	ALL(IA,KJ-1)=OVK(4,IA+K-2)	PBS	194
	ALL(IA,KJ)=OVK(5,IA+K-1)	PBS	195
	100 CONTINUE	PBS	196
C		PBS	197
C	IMPOSE BOUNDARY CONDITIONS.	PBS	198
C		PBS	199

	DO 110 LK=1,5	PBS 200
	IF (LILA(IP+LK-1).EQ.0) GO TO 110	PBS 201
	IF (LILA(IP+LK-1).EQ.2) GO TO 110	PBS 202
	ALL(1,KJ+LK-5)=ALL(1,KJ+LK-5)*COMET	PBS 203
110	CONTINUE	PBS 204
120	CONTINUE	PBS 205
C		PBS 206
C	TRANSFER TO TAPE 3	PBS 207
C		PBS 208
	WRITE (3) ALL	PBS 209
	KOUNT=KOUNT+1	PBS 210
C		PBS 211
C	CHECK IF ALL BLOCKS HAVE BEEN OBTAINED	PBS 212
C		PBS 213
	GO TO 20	PBS 214
130	END FILE 3	PBS 215
	RETURN	PBS 216
	END	PBS 217
	SUBROUTINE BEASTX (M,NL,NUMNP,NIX,NBEAMX,NULAYB,KC,KMAT,ZCRD,ASXLR,1,AILR,ITYPE,ESX,TSHEAR,RON,ROM,SIGO,ROMT,FT,SIBXXC,SIBXXA)	BEASTX 2 BEASTX 3 BEASTX 4
C		BEASTX 5
C	THIS SUBROUTINE GENERATES THE BEAM-ELEMENT STIFFNESS BY ASSEMBLING	BEASTX 6
C	TOTAL STIFFNESS LAYER BY LAYER AND CONSIDERING THE PRESENT STATE	BEASTX 7
C	STRESS IN EACH LAYER.THE STIFFENER IS ASSUMED TO BE IN A UNIAXIAL	BEASTX 8
C	STATE OF STRESS AND THE INTERACTION OF THE TORSIONAL MOMENT ON	BEASTX 9
C	IS NEGLECTED IN THE PRESENT ANALYSIS	BEASTX10
C		BEASTX11
C	COMPUTE STIFFNESS MATRIX FOR BEAM ELEMENT	BEASTX12
C	NODE * 1 * 2 *	BEASTX13
C	*****	BEASTX14
C	* * *	BEASTX15
C	1 * BS1 * BS2 *	BEASTX16
C	* * *	BEASTX17
C	*****	BEASTX18
C	* * *	BEASTX19
C	2 * BS3 * BS4 *	BEASTX20
C	* * *	BEASTX21
C	*****	BEASTX22
C		BEASTX23
	DIMENSION BBS1(2,5,5), BBS2(2,5,5), NL(2)	BEASTX24
	DIMENSION ESX(NULAYB,NBEAMX), RON(KMAT), ROM(KMAT), SIBXXA(NULAYB)	BEASTX25
	1, SIBXXC(NULAYB), ASXLR(NULAYB), ZCRD(NULAYB), AILR(NULAYB), ITYPE	BEASTX26
	2(NULAYB), TSHEAR(NULAYB), ROMT(KC), ROMT(KC), SIGO(KMAT), FT(KMAT)	BEASTX27
	3, DUMMY(1)	BEASTX28
C		BEASTX29
	COMMON /CB19/ BBS1,BBS2	BEASTX30
	COMMON /CB309/ FTOL,GTOL	BEASTX31
	DO 70 NR=1,2	BEASTX32
	N=NL(NR)	BEASTX33
	IF (N.GE.NIX) GO TO 50	BEASTX34
C		BEASTX35
	READ (14) XLNGTH,GKEIX,ITYPE,ASXLR,AILR,ZCRD,TSHEAR	BEASTX36
	READ (15) SIBXXA	BEASTX37
	READ (24) SIBXXC	BEASTX38
C		BEASTX39
	BASX=0.0	BEASTX40
	BEIX=0.0	BEASTX41

	BSSX=0.0	BEASTX42
	A=XLNGTH	BEASTX43
	AA=A**2	BEASTX44
	DO 20 ML=1,NULAYB	BEASTX45
	S=SIBXXC(ML)+SIBXXA(ML)	BEASTX46
	ZASX=ASXLR(ML)	BEASTX47
C		BEASTX48
C	ADD CONTRIBUTION FROM ALL LAYERS	BEASTX49
C		BEASTX50
	ZSSX=ZASX*ZGRD(ML)	BEASTX51
	ZEIX=ZSSX*ZGRD(ML)+AILR(ML)	BEASTX52
	K=ITYPE(ML)	BEASTX53
C		BEASTX54
C	GET STRESS-STRAIN RELATION	BEASTX55
C		BEASTX56
	ZESX=ESX(ML,N)/(1.+RON(K)*(1.-ROM(K))/ROM(K)*((ABS(S)/SIGO(K))**((RBEASTX57 1ON(K)-1.)))	BEASTX58
C		BEASTX59
	IF (S.GT.0.0.AND.K.LE.KC) ZESX=ESX(ML,N)/(1.+RONT(K)*(1.-ROMT(K))/3EASTX60 1ROMT(K)*((ABS(S)/FT(K))**((RONT(K)-1.)))	BEASTX61
C		BEASTX62
	IF (K.GT.KC) GO TO 10	BEASTX63
	SP=-SIGO(K)	BEASTX64
	IF (S.GT.0.0) SP=FT(K)	BEASTX65
	IF (ABS(S).GT.ABS(SP)) ZESX=0.0	BEASTX66
C		BEASTX67
	10 CONTINUE	BEASTX68
C		BEASTX69
C	ADD LAYER STIFFNESS TERMS FOR INPLANE, COUPLING, AND BENDING	BEASTX70
C	ACTIONS	BEASTX71
C		BEASTX72
	BSSX=BSSX+ZSSX*ZESX	BEASTX73
	BEIX=BEIX+ZEIX*ZESX	BEASTX74
	BASX=BASX+ZASX*ZESX	BEASTX75
C		BEASTX76
	20 CONTINUE	BEASTX77
C		BEASTX78
	ZSSX=BSSX/A/A/A	BEASTX79
	ZEIX=BEIX/A/A/A	BEASTX80
	ZASX=BASX/A/A/A	BEASTX81
C		BEASTX82
C	BBS1(1) = BS3 OR BS2 TRANSPOSED	BEASTX83
C	BBS2(1) = BS4	BEASTX84
C	BBS1(2) = BS1	BEASTX85
C	BBS2(2) = BS2	BEASTX86
C		BEASTX87
C	GET ELEMENT STIFFNESS SUBMATRICES	BEASTX88
C		BEASTX89
	GO TO (30,40), NR	BEASTX90
	30 CONTINUE	BEASTX91
C		BEASTX92
	S53=+1.0	BEASTX93
	S55=+.50	BEASTX94
	S11=-1.0	BEASTX95
	S15=-1.0	BEASTX96
	S33=-1.0	BEASTX97
	S35=-1.0	BEASTX98
	S51=-1.0	BEASTX99

CALL SUBH (BBS1,NR,ZSSX,ZEIX,ZASX,A,AA,S11,S15,S33,S35,S51,S53,S55	BEAST100
1)	BEAST101
C	BEAST102
S11=+1.0	BEAST103
S15=+1.0	BEAST104
S33=+1.0	BEAST105
S35=+1.0	BEAST106
S51=+1.0	BEAST107
S53=+1.0	BEAST108
S55=+1.0	BEAST109
CALL SUBH (BBS2,NR,ZSSX,ZEIX,ZASX,A,AA,S11,S15,S33,S35,S51,S53,S55	BEAST110
1)	BEAST111
C	BEAST112
GO TO 70	BEAST113
40 CONTINUE	BEAST114
C	BEAST115
S11=+1.0	BEAST116
S15=+1.0	BEAST117
S33=+1.0	BEAST118
S51=+1.0	BEAST119
S55=+1.0	BEAST120
S35=-1.0	BEAST121
S53=-1.0	BEAST122
CALL SUBH (BBS1,NR,ZSSX,ZEIX,ZASX,A,AA,S11,S15,S33,S35,S51,S53,S55	BEAST123
1)	BEAST124
C	BEAST125
S55=+.50	BEAST126
S53=+1.0	BEAST127
S11=-1.0	BEAST128
S15=-1.0	BEAST129
S33=-1.0	BEAST130
S35=-1.0	BEAST131
S51=-1.0	BEAST132
CALL SUBH (BBS2,NR,ZSSX,ZEIX,ZASX,A,AA,S11,S15,S33,S35,S51,S53,S55	BEAST133
1)	BEAST134
C	BEAST135
GO TO 70	BEAST136
50 DO 60 I=1,5	BEAST137
DO 60 J=1,5	BEAST138
BBS1(NR,I,J)=0.	BEAST139
BBS2(NR,I,J)=0.	BEAST140
60 CONTINUE	BEAST141
C	BEAST142
C	BEAST143
C	BEAST144
70 CONTINUE	BEAST145
RETURN	BEAST146
END	BEAST147
SUBROUTINE ELSTMA (NCNS,NANA,NULAY,NUMNP,NUMEL,NSLAYR,NSMAT,M,NZ,MELSTMA	2
1M,LILA,NA1,PZC,PDT,SZC,SDT,SXXOT,SYOT,SXYOT,SIGXXO,SIGYYO,SIGXYO,ELSTMA	3
2AS1,AF,AS,SPHI,NSTYPE,SPROM,SPRON,SEMOD,SIGMAP)	ELSTMA 4
C	ELSTMA 5
C	ELSTMA 6
C	ELSTMA 7
C	ELSTMA 8
C	ELSTMA 9
C	ELSTMA 10
C	ELSTMA 11

*PDT(NULAY),PZC(NULAY),	ELSTMA12
*SXXOT(NCNS),SYYOT(NCNS),SXYOT(NCNS),	ELSTMA13
*SIGXXO(NCNS),SIGYYO(NCNS),SIGXYO(NCNS),	ELSTMA14
*SDT(NSLAYR ),SZC(NSLAYR ),	ELSTMA15
*DUMMY(1)	ELSTMA16
<b>C</b>	
DIMENSION IULOC(4,4),JULOC(4,4),KUU(8,8),AK3(8,8),K1(12,12),KELSTMA18	
12(12,12),K3(12,12),K4(12,12),K5(12,12),K6(12,12),CTU(8,8),AKELSTMA19	
21(12,12),AK2(12,12),NZ(4),DDO(3,3),CCB(12,12)	ELSTMA20
DIMENSION GB(12,12)	ELSTMA21
DIMENSION S1(4,5,5),S2(4,5,5),S3(4,5,5),S4(4,5,5)	ELSTMA22
DIMENSION ILOC(4,4),JLOC(4,4)	ELSTMA23
DIMENSION ICLOC(4,8),JCLOC(4,8)	ELSTMA24
DIMENSION CGU(8,8),KUO(8,12),AKU(8,12)	ELSTMA25
COMMON /CB09/ S1,S2,S3,S4	ELSTMA26
COMMON /GBS301/ COUPLE	ELSTMA27
COMMON /TU00/ TUO(8,12)	ELSTMA28
COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)	ELSTMA29
<b>C</b>	
REAL KT	ELSTMA30
REAL KUU	ELSTMA31
REAL KUO	ELSTMA32
REAL K1,K2,K3,K4,K5,K6	ELSTMA33
DATA ((ILOC(I,J),J=1,4),I=1,4)/0,0,0,0,0,0,3,6,6,0,3,6,9,0,3,3,3/	ELSTMA35
DATA ((JLOC(I,J),J=1,4),I=1,4)/0,3,6,9,6,6,6,9,9,9,9,3,3,6,9/	ELSTMA36
DATA ((ICLOC(I,J),J=1,8),I=1,4)/0,0,0,0,0,2,4,6,0,2,4,4,4,4,4,6,0,	ELSTMA37
12,4,6,6,6,6,6,0,2,2,2,2,2,4,6/	ELSTMA38
DATA ((JCLOC(I,J),J=1,8),I=1,4)/-2,1,4,7,-2,-2,-2,-2,4,4,4,7,-2,1,	ELSTMA39
14,4,7,7,7,7,-2,1,4,7,1,1,4,7,-2,1,1,1/	ELSTMA40
DATA ((IULOC(I,J),J=1,4),I=1,4)/0,0,0,0,0,2,4,4,0,2,4,6,0,2,2,2/	ELSTMA41
DATA ((JULOC(I,J),J=1,4),I=1,4)/0,2,4,6,4,4,4,6,6,6,6,6,2,2,4,6/	ELSTMA42
<b>C</b>	
<b>C</b>	
DATA ((K1(I,J),J=1,12),I=1,12)/0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA45
1,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA46
2.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,	ELSTMA47
30,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA48
40.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA49
50,0.0,0.0,0.0,0.0,45.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA50
6,5.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA51
7.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA52
8,0.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA53
90.0,0.0,0.0,0.0,0.0/	ELSTMA54
DATA ((K2(I,J),J=1,12),I=1,12)/0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA55
1,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA56
2.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,	ELSTMA57
30,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA58
40.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA59
5.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA60
60,0.0,0.0,0.0,15.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,0.0,	ELSTMA61
70,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA62
8,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,15.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA63
90.0,0.0,0.0,0.0,15.0,0.0/	ELSTMA64
DATA ((K3(I,J),J=1,12),I=1,12)/0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA65
1,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA66
2.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,-15.0,	ELSTMA67
3.0,0.0,0.0,0.0,0.0,0.0,-15.0,-15.0,0.0,0.0,0.0,-15.0,0.0,0.0,0.0,	ELSTMA68
4.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,0.0,	ELSTMA69



DO 10 J=1,5	ELSTM128
DO 10 I=1,5	ELSTM129
S1(NE,I,J)=0.0	ELSTM130
S2(NE,I,J)=0.0	ELSTM131
S3(NE,I,J)=0.0	ELSTM132
S4(NE,I,J)=0.0	ELSTM133
10 CONTINUE	ELSTM134
C	ELSTM135
C SELECT ELEMENTS AROUND NODE POINT	ELSTM136
C	ELSTM137
DO 480 NR=1,4	ELSTM138
N=NZ(NR)	ELSTM139
C	ELSTM140
C CHECK IF THERE IS AN ELEMENT	ELSTM141
C	ELSTM142
IF (N.EQ.NANA) GO TO 480	ELSTM143
C	ELSTM144
READ (12) DT,ELLA,ELLB	ELSTM145
READ (20) SIGXX0,SIGYY0,SIGXY0	ELSTM146
READ (22) SXXOT,SYOT,SXYOT	ELSTM147
C	ELSTM148
C INITIALIZE	ELSTM149
C	ELSTM150
CONST1=0.0	ELSTM151
CONST2=0.0	ELSTM152
CONST3=0.0	ELSTM153
CONST4=0.0	ELSTM154
CONST5=0.0	ELSTM155
CONST6=0.0	ELSTM156
RD11=0.0	ELSTM157
RD12=0.0	ELSTM158
RD13=0.0	ELSTM159
RD22=0.0	ELSTM160
RD23=0.0	ELSTM161
RD33=0.0	ELSTM162
SD11=0.0	ELSTM163
SD12=0.0	ELSTM164
SD13=0.0	ELSTM165
SD22=0.0	ELSTM166
SD23=0.0	ELSTM167
SD33=0.0	ELSTM168
C	ELSTM169
C ASSIGN DIMENSIONS	ELSTM170
C	ELSTM171
A=ELLA	ELSTM172
B=ELLB	ELSTM173
ABAR=(PZC(1)-PDT(1)/2.0+PZC(NULAY)+PDT(NULAY)/2.0)	ELSTM174
ABAR=ABAR*DT/2.0	ELSTM175
C	ELSTM176
C ADD CONTRIBUTIONS FROM ALL LAYERS	ELSTM177
C	ELSTM178
DO 40 ML=1,NCNS	ELSTM179
C	ELSTM180
C GET STRESS-STRAIN RELATIONSHIP FOR LAYER	ELSTM181
C	ELSTM182
CALL DEMATR (N,ML,DDD,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SXXOT,SYOT,SXELSTM183 1YOT,SIGXX0,SIGYY0,SIGXY0,AS1,AF,AS,SPHI,NSTYPE,SPRON,SPROM,SEMOD,SELSTM184 2IGMAP)	ELSTM185

C		ELSTM186
C	GET SKEW STRESS - STRAIN RELATIONSHIP	ELSTM187
C		ELSTM188
C	IF (ABS(PHI).NE.90.) CALL TRAT (DDD)	ELSTM189
	D11=DDD(1,1)	ELSTM190
	D12=DDD(1,2)	ELSTM191
	D13=DDD(1,3)	ELSTM192
	D22=DDD(2,2)	ELSTM193
	D23=DDD(2,3)	ELSTM194
	D33=DDD(3,3)	ELSTM195
C		ELSTM196
C	THIS IS TO TAKE CARE OF THE SIGN COMPATIBILITY BETWEEN INPLANE	ELSTM197
C	AND OUT OF PLANE ACTIONS	ELSTM198
C	THIS MODIFICATION ONLY FOR THE BENDING STIFFNESS MATRIX	ELSTM199
C		ELSTM200
	D13=-D13	ELSTM201
	D23=-D23	ELSTM202
	IF (ML.GT.NULAY) GO TO 20	ELSTM203
C		ELSTM204
C	CONCRETE LAYER GEOMETRY	ELSTM205
C		ELSTM206
	C1=(PZC(ML)-PDT(ML)/2.0)*DT	ELSTM207
	C2=(PZC(ML)+PDT(ML)/2.0)*DT	ELSTM208
	GO TO 30	ELSTM209
	20 CONTINUE	ELSTM210
C		ELSTM211
C	STEEL LAYER GEOMETRY	ELSTM212
C		ELSTM213
	C1=SZC(ML-NULAY)-SDT(ML-NULAY)/2.0	ELSTM214
	C2=SZC(ML-NULAY)+SDT(ML-NULAY)/2.0	ELSTM215
	30 CONTINUE	ELSTM216
C		ELSTM217
C	SUM UP BENDING STIFFNESS CHARACTERISTICS	ELSTM218
C		ELSTM219
	CCC=(C2*C2*C2-C1*C1*C1)/3.0+(C2-C1)*(ABAR*ABAR-ABAR*(C2+C1))	ELSTM220
	CONST1=CONST1+D11*CCC	ELSTM221
C		ELSTM222
C	SUM UP INPLANE STIFFNESS CHARACTERISTICS	ELSTM223
C		ELSTM224
	CONST2=CONST2+D12*CCC	ELSTM225
	CONST3=CONST3+D13*CCC	ELSTM226
	CONST4=CONST4+D22*CCC	ELSTM227
	CONST5=CONST5+D23*CCC	ELSTM228
	CONST6=CONST6+D33*CCC	ELSTM229
	D13=-D13	ELSTM230
	D23=-D23	ELSTM231
	ZFAC=C2-C1	ELSTM232
	SD11=SD11+D11*ZFAC	ELSTM233
	SD12=SD12+D12*ZFAC	ELSTM234
	SD13=SD13+D13*ZFAC	ELSTM235
	SD22=SD22+D22*ZFAC	ELSTM236
	SD23=SD23+D23*ZFAC	ELSTM237
	SD33=SD33+D33*ZFAC	ELSTM238
	IF (COUPLE.NE.3HYES) GO TO 40	ELSTM239
C		ELSTM240
C	SUM UP COUPLING STIFFNESS CHARACTERISTICS	ELSTM241
C		ELSTM242
		ELSTM243



	GC=(C2+C1-2.0*ABAR)*(C2-C1)/2.0	ELSTM244
	RD11=RD11+D11*CC	ELSTM245
	RD12=RD12+D12*CC	ELSTM246
	RD13=RD13+D13*CC	ELSTM247
	RD22=RD22+D22*CC	ELSTM248
	RD23=RD23+D23*CC	ELSTM249
	RD33=RD33+D33*CC	ELSTM250
C		ELSTM251
C	GO TO ANOTHER LAYER	ELSTM252
C		ELSTM253
	40 CONTINUE	ELSTM254
C		ELSTM255
C	FOR COUPLING ONLY	ELSTM256
C		ELSTM257
	IF (COUPLE.NE.3HYES) GO TO 60	ELSTM258
C		ELSTM259
C	INITIALIZE	ELSTM260
C		ELSTM261
	DO 50 I=1,8	ELSTM262
	DO 50 J=1,12	ELSTM263
	50 KUO(I,J)=0.0	ELSTM264
	D11=RD11	ELSTM265
	D12=RD12	ELSTM266
	D13=RD13	ELSTM267
	D22=RD22	ELSTM268
	D23=RD23	ELSTM269
	D33=RD33	ELSTM270
C		ELSTM271
C	D11 TERMS	ELSTM272
	KUO(2,4)=-8.0*B*D11/A	ELSTM273
	KUO(4,8)=KUO(2,4)*B/3.0	ELSTM274
C		ELSTM275
C	D22 TERMS	ELSTM276
	KUO(7,6)=-8.0*A*D22/B	ELSTM277
	KUO(8,9)=KUO(7,6)*A/3.0	ELSTM278
C		ELSTM279
C	D33 TERMS	ELSTM280
	KUO(3,5)=-8.0*D33	ELSTM281
	KUO(6,5)=KUO(3,5)	ELSTM282
	KUO(3,11)=KUO(3,5)	ELSTM283
	KUO(3,12)=KUO(3,5)	ELSTM284
	KUO(6,12)=KUO(3,5)	ELSTM285
	KUO(6,11)=KUO(3,5)	ELSTM286
	KUO(4,8)=KUO(4,8)-16.0*A*D33/3.0	ELSTM287
	KUO(8,9)=KUO(8,9)-16.*B*D33/3.0	ELSTM288
C		ELSTM289
C	D12 TERMS	ELSTM290
	KUO(2,6)=-8.0*A*D12/B	ELSTM291
	KUO(4,10)=KUO(2,6)*B	ELSTM292
	KUO(7,4)=-8.*B*D12/A	ELSTM293
	KUO(8,7)=KUO(7,4)*A	ELSTM294
C		ELSTM295
C	D13 TERMS	ELSTM296
	KUO(2,5)=-8.0*D13	ELSTM297
	KUO(2,11)=KUO(2,5)	ELSTM298
	KUO(2,12)=KUO(2,5)	ELSTM299
	KUO(3,4)=-8.0*B*D13/A	ELSTM300
	KUO(4,7)=KUO(3,4)*A	ELSTM301

	KUO(4,9)=-16.*B*D13/3.0	ELSTM302
	KUO(6,4)=KUO(3,4)	ELSTM303
	KUO(8,8)=-8.0*B*B*D13/(3.0*A)	ELSTM304
C		ELSTM305
C	D23 TERMS	ELSTM306
	KUO(3,6)=-8.*A*D23/8	ELSTM307
	KUO(4,9)=KUO(4,9)-8.0*A*A*D23/(3.*B)	ELSTM308
	KUO(6,6)=KUO(3,6)	ELSTM309
	KUO(7,5)=-8.*D23	ELSTM310
	KUO(7,11)=KUO(7,5)	ELSTM311
	KUO(7,12)=KUO(7,5)	ELSTM312
	KUO(8,8)=KUO(8,8)-16.*D23*A/3.0	ELSTM313
	KUO(8,10)=-8.*A*D23	ELSTM314
	60 CONTINUE	ELSTM315
C		ELSTM316
C	FOR INPLANE STIFFNESS	ELSTM317
C		ELSTM318
	D11=SD11	ELSTM319
	D12=SD12	ELSTM320
	D13=SD13	ELSTM321
	D22=SD22	ELSTM322
	D23=SD23	ELSTM323
	D33=SD33	ELSTM324
C		ELSTM325
C	IN PLANE	ELSTM326
	DO 70 I=1,8	ELSTM327
	DO 70 J=1,8	ELSTM328
	70 KUU(I,J)=0.	ELSTM329
	KUU(2,2)=D11	ELSTM330
	KUU(3,2)=D13	ELSTM331
	KUU(2,3)=D13	ELSTM332
	KUU(6,2)=D13	ELSTM333
	KUU(2,6)=D13	ELSTM334
	KUU(7,2)=D12	ELSTM335
	KUU(2,7)=D12	ELSTM336
	KUU(3,3)=D33	ELSTM337
	KUU(6,6)=D33	ELSTM338
	KUU(6,3)=D33	ELSTM339
	KUU(3,6)=D33	ELSTM340
	KUU(7,3)=D23	ELSTM341
	KUU(3,7)=D23	ELSTM342
	KUU(7,6)=D23	ELSTM343
	KUU(6,7)=D23	ELSTM344
	KUU(4,4)=D11*B*B/3.0+D33*A*A/3.0	ELSTM345
	KUU(8,4)=D23*A*A/3.0+D13*B*B/3.0	ELSTM346
	KUU(4,8)=D23*A*A/3.0+D13*B*B/3.0	ELSTM347
	KUU(7,7)=D22	ELSTM348
	KUU(8,8)=D22*A*A/3.0+D33*B*B/3.0	ELSTM349
	DO 80 I=1,8	ELSTM350
	DO 80 J=1,8	ELSTM351
	80 KUU(I,J)=KUU(I,J)*4.0*A*B	ELSTM352
C		ELSTM353
C		ELSTM354
C	FOR BENDING	ELSTM355
C		ELSTM356
	C=16.0/15.0	ELSTM357
C		ELSTM358
C	FORM BENDING AND INPLANE C MATRICES	ELSTM359

	CONST1=CONST1*C*B/(A*A*A)	ELSTM360
	CONST2=CONST2*C/(A*B)	ELSTM361
	CONST3=CONST3*C/(A*A)	ELSTM363
	CONST4=CONST4*C*A/(B*B*B)	ELSTM364
	CONST5=CONST5*C/(B*B)	ELSTM365
	CONST6=CONST6*C/(A*B)	ELSTM366
	DO 90 I=1,12	ELSTM367
	DO 90 J=1,12	ELSTM368
90	AK1(I,J)=K1(I,J)*CONST1+K2(I,J)*CONST2+K3(I,J)*CONST3+K4(I,J)*CONST4+K5(I,J)*CONST5+K6(I,J)*CONST6	ELSTM369
	DO 100 I=1,12	ELSTM370
	DO 100 J=1,12,3	ELSTM372
	CCB(I,J)=CB(I,J)	ELSTM373
	CCB(I,J+1)=CB(I,J+1)*E	ELSTM374
	CCB(I,J+2)=CB(I,J+2)*A	ELSTM375
100	CONTINUE	ELSTM376
	DO 110 I=1,8	ELSTM377
	DO 110 J=1,5,4	ELSTM378
	CCU(I,J)=CTU(I,J)	ELSTM379
	CCU(I,J+1)=CTU(I,J+1)/A	ELSTM380
	CCU(I,J+2)=CTU(I,J+2)/B	ELSTM381
	CCU(I,J+3)=CTU(I,J+3)/(A*B)	ELSTM382
110	CONTINUE	ELSTM383
C		ELSTM384
C	FORM IN PLANE STIFF	ELSTM385
C		ELSTM386
	DO 130 I=1,8	ELSTM387
	DO 130 J=1,8	ELSTM388
	SUM=0.	ELSTM389
	DO 120 K=1,8	ELSTM390
120	SUM=CCU(I,K)*KUU(K,J)+SUM	ELSTM391
130	AK3(I,J)=SUM	ELSTM392
C		ELSTM393
	DO 150 I=1,8	ELSTM394
	DO 150 J=1,8	ELSTM395
	SUM=0.	ELSTM396
	DO 140 K=1,8	ELSTM397
140	SUM=SUM+AK3(I,K)*CCU(J,K)	ELSTM398
150	KUU(I,J)=SUM	ELSTM399
C		ELSTM400
C		ELSTM401
C	FORM BENDING STIFFNESS	ELSTM402
C		ELSTM403
	DO 170 I=1,12	ELSTM404
	DO 170 J=1,12	ELSTM405
	SUM=0.	ELSTM406
	DO 160 KQ=1,12	ELSTM407
160	SUM=SUM+AK1(I,KQ)*CCB(KQ,J)	ELSTM408
	AK2(I,J)=SUM	ELSTM409
170	CONTINUE	ELSTM410
C		ELSTM411
	DO 190 I=1,12	ELSTM412
	DO 190 J=1,12	ELSTM413
	SUM=0.	ELSTM414
	DO 180 KQ=1,12	ELSTM415
180	SUM=SUM+CCB(KQ,I)*AK2(KQ,J)	ELSTM416
190	AK1(I,J)=SUM	ELSTM417

C	IF (COUPLE.NE.3HYES) GO TO 300	ELSTM418
C		ELSTM419
C		ELSTM420
C	FORM COUPLING STIFFNESS	ELSTM421
C		ELSTM422
	DO 210 I=1,8	ELSTM423
	DO 210 J=1,12	ELSTM424
	SUM=0.	ELSTM425
	DO 200 KQ=1,12	ELSTM426
	200 SUM=SUM+KUO(I,KQ)*CC8(KQ,J)	ELSTM427
	AKU(I,J)=SUM	ELSTM428
	210 CONTINUE	ELSTM429
	DO 230 I=1,8	ELSTM430
	DO 230 J=1,12	ELSTM431
	SUM=0.0	ELSTM432
	DO 220 KQ=1,8	ELSTM433
	220 SUM=SUM+CCU(I,KQ)*AKU(KQ,J)	ELSTM434
	KUO(I,J)=SUM	ELSTM435
	230 CONTINUE	ELSTM436
C		ELSTM437
	IF (ABS(ABAR).LT.0.001) GO TO 300	ELSTM438
C		ELSTM439
C	IF COUPLE = NO THEN ABAR MUST = 0.0	ELSTM440
C		ELSTM441
	DO 250 I=1,12	ELSTM442
	DO 250 J=1,12	ELSTM443
	SUM=0.	ELSTM444
	DO 240 K=1,8	ELSTM445
	240 SUM=SUM+ABAR*TUO(K,I)*KUO(K,J)	ELSTM446
	AK1(J,I)=AK1(J,I)+SUM	ELSTM447
	250 AK1(I,J)=AK1(I,J)+SUM	ELSTM448
C		ELSTM449
	DO 270 I=1,8	ELSTM450
	DO 270 J=1,12	ELSTM451
	SUM=0.	ELSTM452
	DO 260 K=1,8	ELSTM453
	260 SUM=ABAR*KUU(I,K)*TUO(K,J)+SUM	ELSTM454
	KUO(I,J)=KUO(I,J)+SUM	ELSTM455
	270 AKU(I,J)=SUM	ELSTM456
C		ELSTM457
C		ELSTM458
	DO 290 I=1,12	ELSTM459
	DO 290 J=1,12	ELSTM460
	SUM=0.	ELSTM461
	DO 280 K=1,8	ELSTM462
	280 SUM=SUM+ABAR*TUO(K,I)*AKU(K,J)	ELSTM463
	290 AK1(I,J)=AK1(I,J)+SUM	ELSTM464
C		ELSTM465
	300 CONTINUE	ELSTM466
C		ELSTM467
C	APPLY BOUNDARY CONDITIONS ON SKEW STIFFNESS MATRIX	ELSTM468
C		ELSTM469
C	SELECT I,J,K,OR L	ELSTM470
	DO 330 I=1,4	ELSTM471
C		ELSTM472
C	SELECT U OR V INPLANE DISP	ELSTM473
C		ELSTM474
	DO 330 J=1,2	ELSTM475

	K=5*MM(NR,I)-5+J	ELSTM476
	IF (LILA(K).NE.2) GO TO 330	ELSTM477
	NE=2*I-2+J	ELSTM478
310	DO 310 KQ=1,8	ELSTM479
	KUU(NE,KQ)=0.0	ELSTM480
	KUU(KQ,NE)=0.0	ELSTM481
320	DO 320 KQ=1,12	ELSTM482
	KUO(NE,KQ)=0.	ELSTM483
	KUU(NE,NE)=1.	ELSTM484
330	CONTINUE	ELSTM485
C		ELSTM486
C	SELECT I,J,K,OR L	ELSTM487
C		ELSTM488
	DO 360 I=1,4	ELSTM489
C		ELSTM490
C	SELECT BENDING W,FX,MY	ELSTM491
C		ELSTM492
	DO 360 J=1,3	ELSTM493
	K=5*MM(NR,I)-3+J	ELSTM494
	IF (LILA(K).NE.2) GO TO 360	ELSTM495
	NE=3*I-3+J	ELSTM496
	DO 340 KQ=1,12	ELSTM497
	AK1(NE,KQ)=0.	ELSTM498
340	AK1(KQ,NE)=0.	ELSTM499
	DO 350 KQ=1,8	ELSTM500
350	KUO(KQ,NE)=0.	ELSTM501
	AK1(NE,NE)=1.	ELSTM502
360	CONTINUE	ELSTM503
C		ELSTM504
C	TRANSFORM TO CARTESIAN	ELSTM506
C	WHERE CARTESIAN FORCE = A * SKEW FORCE	ELSTM507
C		ELSTM508
	IF (ABS(PHI).EQ.90.) GO TO 370	ELSTM509
	CALL TRA (KUU,KT,8,8,AU,2,AU,2)	ELSTM510
	CALL TRA (KUO,KT,8,12,AU,2,AO,3)	ELSTM511
	CALL TRA (AK1,KT,12,12,AO,3,AO,3)	ELSTM512
370	CONTINUE	ELSTM513
C		ELSTM514
C		ELSTM515
C	PUT BENDING AND INPLANE STIFFNESS TERMS INTO	ELSTM516
C	APPROPRIATE LOCATIONS.	ELSTM517
C		ELSTM518
	GO TO (380,390,400,410), NR	ELSTM519
380	CONTINUE	ELSTM520
	CALL SADD (S1,NR,AK1,ILOC,JLOC)	ELSTM521
	CALL SKADD (S1,NR,KUU,IULOC,JULOC)	ELSTM522
	GO TO 420	ELSTM523
390	CONTINUE	ELSTM524
	CALL SADD (S2,NR,AK1,ILOC,JLOC)	ELSTM525
	CALL SKADD (S2,NR,KUU,IULOC,JULOC)	ELSTM526
	GO TO 420	ELSTM527
400	CONTINUE	ELSTM528
	CALL SADD (S3,NR,AK1,ILOC,JLOC)	ELSTM529
	CALL SKADD (S3,NR,KUU,IULOC,JULOC)	ELSTM530
	GO TO 420	ELSTM531
410	CONTINUE	ELSTM532
	CALL SADD (S4,NR,AK1,ILOC,JLOC)	ELSTM533

	CALL SKADD (S4, NR, KUU, IULOC, JULOC)	ELSTM534
420	CONTINUE	ELSTM535
	IF (COUPLE.NE.3HYES) GO TO 470	ELSTM536
	GO TO (430, 440, 450, 460), NR	ELSTM537
C		ELSTM538
C	PUT COUPLING STIFFNESS TERMS INTO APPROPRIATE LOCATIONS	ELSTM539
C		ELSTM540
430	CONTINUE	ELSTM541
	CALL KUOADD (S1, NR, KUO, ICLOC, JCLOC)	ELSTM542
	GO TO 470	ELSTM543
440	CONTINUE	ELSTM544
	CALL KUOADD (S2, NR, KUO, ICLOC, JCLOC)	ELSTM545
	GO TO 470	ELSTM546
450	CONTINUE	ELSTM547
	CALL KUOADD (S3, NR, KUO, ICLOC, JCLOC)	ELSTM548
	GO TO 470	ELSTM549
460	CONTINUE	ELSTM550
	CALL KUOADD (S4, NR, KUO, ICLOC, JCLOC)	ELSTM551
	470 CONTINUE	ELSTM552
C		ELSTM553
C	GO TO ANOTHER ELEMENTT	ELSTM554
C		ELSTM555
480	CONTINUE	ELSTM556
	RETURN	ELSTM557
	END	ELSTM558
	SUBROUTINE TRA (K, KT, N, M, A, NA, B, NB)	TRA 2
C		TRA 3
C	THIS SUBROUTINE TRANSFORMS STIFFNESS FROM SKEW TO CARTESIAN SYS	TRA 4
C		TRA 5
	DIMENSION K(N, M), KT(NA, NB), A(NA, NA), B(NB, NB)	TRA 6
	REAL K, KT	TRA 7
C		TRA 8
C	TRANSFORM STIFFNESS MATRIX USING NA BY NB CELLS	TRA 9
C		TRA 10
C	K IS STIFFNESS IN SKEW SYSTEM	TRA 11
C	K = A*K*B-TRANPOSED	TRA 12
C	K IS STIFFNESS IN CARTESIAN SYSTEM	TRA 13
C		TRA 14
C		TRA 15
	DO 50 II=1, N, NA	TRA 16
	DO 50 JJ=1, M, NB	TRA 17
C		TRA 18
	DO 20 I=1, NA	TRA 19
	DO 20 J=1, NB	TRA 20
	SUM=0.	TRA 21
	DO 10 L=1, NA	TRA 22
10	SUM=SUM+A(I, L)*K(II+L-1, JJ+J-1)	TRA 23
20	KT(I, J)=SUM	TRA 24
C		TRA 25
C		TRA 26
	DO 40 I=1, NA	TRA 27
	DO 40 J=1, NB	TRA 28
	SUM=0.	TRA 29
	DO 30 L=1, NB	TRA 30
30	SUM=SUM+KT(I, L)*B(J, L)	TRA 31
40	K(II+I-1, JJ+J-1)=SUM	TRA 32
C		TRA 33
C		TRA 34

	50 CONTINUE	TRA	35
C	RETURN	TRA	36
	END	TRA	37
	SUBROUTINE TRAT (DDD)	TRA	78
C		TRAT	2
		TRAT	3
C	THIS SUBROUTINE GETS SKEW STRESS - STRAIN RELATIONSHIP	TRAT	4
C	WHERE CARTESIAN STRAINS = DS * SKEW STRAINS	TRAT	5
C		TRAT	6
	DIMENSION DDD(3,3), D(3,3)	TRAT	7
	COMMON /SKEW/ PHI,AU(2,2),AO(3,3),DS(3,3)	TRAT	8
C		TRAT	9
	SINA=PHI*ATAN(1.0)/45.	TRAT	10
	SINA=SIN(SINA)	TRAT	11
	DO 20 I=1,3	TRAT	12
	DO 20 J=1,3	TRAT	13
	SUM=0.	TRAT	14
	DO 10 L=1,3	TRAT	15
	10 SUM=SUM+DS(L,I)*DDD(L,J)	TRAT	16
	20 D(I,J)=SUM	TRAT	17
C		TRAT	18
	DO 40 I=1,3	TRAT	19
	DO 40 J=1,3	TRAT	20
	SUM=0.	TRAT	21
	DO 30 L=1,3	TRAT	22
	30 SUM=SUM+D(I,L)*DS(L,J)	TRAT	23
	40 DDD(I,J)=SUM*SINA	TRAT	24
	RETURN	TRAT	25
	END	TRAT	26
	SUBROUTINE SADD (S,NR,AK1,ILOC,JLOC)	SADD	2
C		SADD	3
C	ADD CONTRIBUTION OF BENDING STIFFNESS TO ELEMENT STIFFNESS MATRIX	SADD	4
C		SADD	5
	DIMENSION AK1(12,12)	SADD	6
	DIMENSION S(4,5,5)	SADD	7
	DIMENSION ILOC(4,4), JLOC(4,4)	SADD	8
	DO 10 N=1,4	SADD	9
	IA=ILOC(NR,N)	SADD	10
	JA=JLOC(NR,N)	SADD	11
	DO 10 I=1,3	SADD	12
	DO 10 J=1,3	SADD	13
	S(N,I+2,J+2)=S(N,I+2,J+2)+AK1(I+IA,J+JA)	SADD	14
	10 CONTINUE	SADD	15
	RETURN	SADD	16
	END	SADD	17
	SUBROUTINE SKADD (S,NR,KUU,IULOC,JULOC)	SKADD	2
C		SKADD	3
C	ADD INPLANE STIFFNESS TO ELEMENT MATRICES	SKADD	4
C		SKADD	5
	DIMENSION S(4,5,5)	SKADD	6
	DIMENSION KUU(8,8), IULOC(4,4), JULOC(4,4)	SKADD	7
	REAL KUU	SKADD	8
	DO 10 N=1,4	SKADD	9
	IADD=IULOC(NR,N)	SKADD	10
	JADD=JULOC(NR,N)	SKADD	11
	DO 10 I=1,2	SKADD	12
	DO 10 J=1,2	SKADD	13
	S(N,I,J)=S(N,I,J)+KUU(I+IADD,J+JADD)	SKADD	14

10	CONTINUE	SKADD 15
	RETURN	SKADD 16
	END	SKADD 17
C	SUBROUTINE KUOADD (S,NR,KUO,ICLOC,JCLOC)	KUOADD 2
C	ADD COUPLING STIFFNESS TO ELEMENT STIFFNESS MATRIX	KUOADD 3
C		KUOADD 4
	DIMENSION KUO(8,12)	KUOADD 5
	DIMENSION S(4,5,5)	KUOADD 6
		KUOADD 7
	DIMENSION ICLOC(4,8), JCLOC(4,8)	KUOADD 8
	REAL KUO	KUOADD 9
	DO 10 N=1,4	KUOADD10
	IAIJ=ICLOC(NR,N)	KUOADD11
	IAJI=ICLOC(NR,N+4)	KUOADD12
	JAIJ=JCLOC(NR,N)	KUOADD13
	JAJI=JCLOC(NR,N+4)	KUOADD14
	DO 10 I=1,2	KUOADD15
	DO 10 J=3,5	KUOADD16
	S(N,I,J)=S(N,I,J)+KUO(I+IAIJ,J+JAIJ)	KUOADD17
	S(N,J,I)=S(N,J,I)+KUO(I+IAJI,J+JAJI)	KUOADD18
10	CONTINUE	KUOADD19
	RETURN	KUOADD20
	END	KUOADD21
	SUBROUTINE DEMATR (N,ML,DDD,NUMEL,NCNS,NULAY,NSLAYR,NSMAT,SXXOT,SYDEMATR	2
	1YOT,SXYOT,SIGXXO,SIGYYO,SIGXYO,AS1,AF,AS,SPHI,NSTYPE,SPRON,SPROM,SDEMATR	3
	2EMOD,SIGMAP)	DEMATR 4
C		DEMATR 5
C	THIS SUBROUTINE COMPUTES STRESS-STRAIN RELATIONSHIP OF SLAB LAYER	DEMATR 6
C		DEMATR 7
	DIMENSION SXXOT(NCNS), SYYOT(NCNS), SXYOT(NCNS), SIGXXO(NCNS), SIGDEMATR	8
	1YYO(NCNS), SIGXYO(NCNS), AS1(NUMEL,NCNS), AF(NUMEL,NULAY), AS(NUMEL	DEMATR 9
	2L,NULAY), SPHI(NSLAYR), NSTYPE(NSLAYR), SEMOD(NSMAT), SIGMAP(NSMAT	DEMATR10
	3), SPRON(NSMAT), SPRON(NSMAT), DD(3,3), DDD(3,3)	DEMATR11
	COMMON /PROPC/ V,EC,FC,ECOMP,FT,ET,PCOMP,ALMM,SALMM,EALMM,ALICC	DEMATR12
	COMMON /COMPLT/ ICMPLT	DEMATR13
C		DEMATR14
C		DEMATR15
C	INITIALIZE	DEMATR15
	DO 10 I=1,3	DEMATR17
	DO 10 J=1,3	DEMATR18
	DDD(I,J)=0.	DEMATR19
10	DD(I,J)=0.0	DEMATR20
	EC1=0.	DEMATR21
	EC2=0.	DEMATR22
	ALPH1=0.0	DEMATR23
	ALPH2=0.0	DEMATR24
C		DEMATR25
C	CURRENT TOTAL STRESS LEVEL	DEMATR26
C		DEMATR27
	SX=SXXOT(ML)+SIGXXO(ML)	DEMATR28
	SY=SYYOT(ML)+SIGYYO(ML)	DEMATR29
	SXY=SXYOT(ML)+SIGXYO(ML)	DEMATR30
	IF (ML.GT.NULAY) GO TO 50	DEMATR31
C		DEMATR32
C	CHECK FOR LAYER FAILURE	DEMATR33
C		DEMATR34
	IF (AS1(N,ML).NE.999.0) GO TO 20	DEMATR35
	GO TO 40	DEMATR36



20 CONTINUE	DEMATR37
C	DEMATR38
C A LAYER HAS CRACKED OR CRUSHED	DEMATR39
C	DEMATR40
C CHECK IF THERE ARE TWO CRACKED - CRUSHED LINES	DEMATR41
C	DEMATR42
IF (AS(N,ML).NE.999.0) GO TO 100	DEMATR43
C	DEMATR44
C FIND TRANSFORMATION ANGLE	DEMATR45
C	DEMATR46
ANG=AS1(N,ML)	DEMATR47
ANGF=AF(N,ML)	DEMATR48
THETAS=ANG	DEMATR49
C	DEMATR50
C TRANSFORM STRESSES AND STRAINS TO CRACKED AXIS	DEMATR51
C	DEMATR52
CALL TRANG (SX,SY,SXY,ANG,S1,S2,S3)	DEMATR53
C	DEMATR54
C CHANGE SIGN	DEMATR55
C	DEMATR56
S1=-S1	DEMATR57
S2=-S2	DEMATR58
IF (ANG.EQ.ANGF) GO TO 30	DEMATR59
CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	DEMATR60
IF (ABS(S2).GE.ABS(SP2)) EC2=0.0	DEMATR61
GO TO 60	DEMATR62
30 CONTINUE	DEMATR63
CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	DEMATR64
IF (ABS(S1).GE.ABS(SP1)) EC1=0.0	DEMATR65
GO TO 60	DEMATR66
40 CONTINUE	DEMATR67
C	DEMATR68
C FOR NO FAILURES	DEMATR69
C	DEMATR70
CALL PRNCIP (SX,SY,SXY,S1,S2,THETAS)	DEMATR71
S1=-S1	DEMATR72
S2=-S2	DEMATR73
C	DEMATR74
C CONSIDER 1 DIRECTION AS DIRECTION OF INTREST	DEMATR75
C	DEMATR76
CALL BAXIAL (S1,S2,ALPH1,SP1,EP1,DIRT1,EC1)	DEMATR77
IF (ABS(S1).GE.ABS(SP1)) EC1=0.0	DEMATR78
IF (DIRT1.EQ.5HCRUSH.AND.EC1.EQ.0.0.AND.ICMPLT.EQ.3HYES) EC2=0.0	DEMATR79
C	DEMATR80
C CONSIDER 2 DIRECTION AS DIRECTION OF INTREST	DEMATR81
C	DEMATR82
CALL BAXIAL (S2,S1,ALPH2,SP2,EP2,DIRT2,EC2)	DEMATR83
IF (ABS(S2).GE.ABS(SP2)) EC2=0.0	DEMATR84
IF (DIRT2.EQ.5HCRUSH.AND.EC2.EQ.0.0.AND.ICMPLT.EQ.3HYES) EC1=0.0	DEMATR85
GO TO 60	DEMATR86
50 CONTINUE	DEMATR87
THETAS=SPHI(ML-NULAY)	DEMATR88
C	DEMATR89
C FOR STEEL LAYER	DEMATR90
C	DEMATR91
CALL TRANG (SX,SY,SXY,THETAS,S1,S2,S3)	DEMATR92
M=NSTYPE(ML-NULAY)	DEMATR93
RN=SPRON(M)	DEMATR94

	RM=SPROM(M)	DEMATR95
	ES=SEMOD(M)	DEMATR96
	SIGMAY=SIGMAP(M)	DEMATR97
	EC1=ES/(1.0+RN*(1.0-RM)/RM*((ABS(S1)/SIGMAY)**(RN-1.0)))	DEMATR98
60	CONTINUE	DEMATR99
C		DEMAT100
C	FIND V1 AND V2	DEMAT101
C	FIND THE ELASTICITY RELATIONSHIP	DEMAT102
C		DEMAT103
	VA=V	DEMAT104
	VB=V	DEMAT105
	IF (EC1.EQ.0..AND.EC2.EQ.0.0) GO TO 100	DEMAT106
	IF (EC1.EQ.0..OR.EC2.EQ.0.0) GO TO 70	DEMAT107
	V1=V	DEMAT108
	V2=(EC1*(1.0-V1*ALPH1)/(EC2*V1))+ALPH2	DEMAT109
	V2=1.0/V2	DEMAT110
	IF (V1.GE.0.0.AND.V2.GE.0.0) VB=V2	DEMAT111
	IF (V1.GE.0.0.AND.V2.GE.0.0) VA=V1	DEMAT112
	IF (V2.LE.V1.AND.V2.GE.0.0) GO TO 80	DEMAT113
	V2=V	DEMAT114
	V1=(EC2*(1.0-V2*ALPH2)/(EC1*V2))+ALPH1	DEMAT115
	V1=1.0/V1	DEMAT116
	IF (V1.GE.0.0.AND.V2.GE.0.0) VB=V2	DEMAT117
	IF (V1.GE.0.0.AND.V2.GE.0.0) VA=V1	DEMAT118
	IF (V1.LE.V2.AND.V1.GE.0.0) GO TO 80	DEMAT119
	V1=VA	DEMAT120
	V2=VB	DEMAT121
	GO TO 80	DEMAT122
70	CONTINUE	DEMAT123
	V1=0.	DEMAT124
	V2=0.	DEMAT125
	EC1=EC1*(1.0-V*ALPH1)	DEMAT126
	EC2=EC2*(1.0-V*ALPH2)	DEMAT127
	GO TO 90	DEMAT128
80	CONTINUE	DEMAT129
	EC1=EC1*(1.0-V1*ALPH1)	DEMAT130
	EC2=EC2*(1.0-V2*ALPH2)	DEMAT131
90	CONTINUE	DEMAT132
C		DEMAT133
C	COMPUTE COUPLED DD MATRIX	DEMAT134
C		DEMAT135
	A=1.0-V1*V2	DEMAT136
	DD(1,1)=EC1/A	DEMAT137
	DD(1,2)=EC1*V2/A	DEMAT138
	DD(2,1)=DD(1,2)	DEMAT139
	DD(2,2)=EC2/A	DEMAT140
	DD(3,3)=EC1*EC2/(EC1+EC2+2.0*EC2*V1)	DEMAT141
C		DEMAT142
C	TRANSFORM TO X-Y COORDINATES	DEMAT143
C		DEMAT144
	CALL TRANS (DD,THETAS,DDD)	DEMAT145
100	CONTINUE	DEMAT146
	RETURN	DEMAT147
	END	DEMAT148
	SUBROUTINE SUBH (BBS,NR,ZSSX,ZEIX,ZASX,A,AA,S11,S15,S33,S35,S51,S55	SUBH 2
	13,S55)	SUBH 3
C		SUBH 4
C	THIS WILL SELECT A 5X5 MATRIX FROM THE BEAM ELEMENT	SUBH 5

C	STIFFNESS MATRIX	SUBH	6
C	DIMENSION BBS(2,5,5), BS(5,5)	SUBH	7
C		SUBH	8
C	DO 10 I=1,5	SUBH	9
	DO 10 J=1,5	SUBH	10
	BS(I,J)=0.0	SUBH	11
10	CONTINUE	SUBH	12
C		SUBH	13
C	BS(1,1)=S11*ZASX*AA	SUBH	14
	BS(1,5)=S15*ZSSX*AA	SUBH	15
	BS(3,3)=S33*12.0*ZEIX	SUBH	16
	BS(3,5)=S35*6.0*ZEIX*A	SUBH	17
	BS(5,1)=S51*ZSSX*AA	SUBH	18
	BS(5,3)=S53*6.0*ZEIX*A	SUBH	19
	BS(5,5)=S55*4.0*ZEIX*AA	SUBH	20
C		SUBH	21
C	DO 20 I=1,5	SUBH	22
	DO 20 J=1,5	SUBH	23
	BBS(NR,I,J)=BS(I,J)	SUBH	24
20	CONTINUE	SUBH	25
C		SUBH	26
C	RETURN	SUBH	27
	END	SUBH	28
	OVERLAY (ECCP,3,0)	OVERL3	2
	PROGRAM OVERL3	OVERL3	3
C		OVERL3	4
C	SOLVE BASIC EQUATIONS,PRINT UNKNOWN DISPLACEMENTS AND REACTIONS.	OVERL3	5
C		OVERL3	6
	COMMON /OVER3/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11	OVERL3	7
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	OVERL3	8
	COMMON /CB3/ LWIDTH,NITA,NPSEGM	OVERL3	9
	COMMON /CB4/ IN,IO	OVERL3	10
	COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	OVERL3	11
	COMMON /SEARCH/ ISRCH	OVERL3	12
	COMMON /IGORES/ IGR(2),IGORE	OVERL3	13
	COMMON A(1)	OVERL3	14
C		OVERL3	15
C		OVERL3	16
	IF (IGORE.NE.0) GO TO 10	OVERL3	17
	IGORE=LOC(A(1))+1	OVERL3	18
	GO TO 20	OVERL3	19
10	CONTINUE	OVERL3	20
	NIETE=LWIDTH-1	OVERL3	21
	NA5=NA1+LWIDTH-1	OVERL3	22
	LAMA=5*NPSEGM	OVERL3	23
	CALL SOL3 (NUMNP,NPSEGM,NA1,NA5,LWIDTH,LAMA,NIETE,A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),A(N9),A(N10))	OVERL3	24
20	CONTINUE	OVERL3	25
	END	OVERL3	26
	SUBROUTINE SOL3 (NUMNP,NPSEGM,NA1,NA5,LWIDTH,LAMA,NIETE,C,X,Y,ALL,SOL3	OVERL3	27
	1AL,H,V,FORCE,LILA,DISP)	SOL3	3
C		SOL3	4
C	SOLUTION ROUTINE	SOL3	5
C		SOL3	6
	DIMENSION C(NA1), X(NA5), Y(NA5), ALL(LWIDTH,LAMA), AL(LWIDTH,NIETSOL3	SOL3	7
	1E), H(LWIDTH), V(LWIDTH), FORCE(NA1), LILA(NA1), DISP(NA1), DUMMY(SOL3	SOL3	8
	21)	SOL3	9

C	COMMON /DEADL/ IDEAD,ITERD,NITERD,ITRALD	SOL3	13
	COMMON /SKEW/ PHI	SOL3	11
	COMMON /SEARCH/ ISRCH	SOL3	12
C	READ FORCE VECTOR	SOL3	13
C		SOL3	14
C	REWIND 29	SOL3	15
	REWIND 30	SOL3	16
	READ (29) FORCE,LILA	SOL3	17
C	IF (IDEAD.EQ.3HYES.AND.ISRCH.NE.3HYES) READ (29) FORCE	SOL3	18
		SOL3	19
	ISEGM=NUMNP/NPSEGM	SOL3	20
C		SOL3	21
C	IMPOSE BOUNDARY CONDITIONS	SOL3	22
C		SOL3	23
C	APPLY BC TO FORCE VECTOR	SOL3	24
C		SOL3	25
	RPHI=PHI*ATAN(1.)/45.	SOL3	26
C		SOL3	27
C	0-FREE, 1-FIXED CATESIAN, 2-FIXED SKEW	SOL3	28
C	SELECT NODE	SOL3	29
C		SOL3	30
	DO 50 KK=1,NUMNP	SOL3	31
C		SOL3	32
C	INPLANE	SOL3	33
C		SOL3	34
	DO 20 N=1,2	SOL3	35
	I=KK*5-5+N	SOL3	36
	IF (LILA(I).EQ.0) GO TO 20	SOL3	37
	IF (LILA(I).NE.2) GO TO 10	SOL3	38
	IF (N.EQ.1) FORCE(I)=FORCE(I+1)/TAN(RPHI)	SOL3	39
	IF (N.EQ.2) FORCE(I-1)=FORCE(I-1)-FORCE(I)/TAN(RPHI)	SOL3	40
	IF (N.EQ.2) FORCE(I)=0.	SOL3	41
	GO TO 20	SOL3	42
	10 FORCE(I)=0.	SOL3	43
	20 CONTINUE	SOL3	44
C		SOL3	45
C	BENDING	SOL3	46
C		SOL3	47
	DO 40 N=1,3	SOL3	48
	I=5*KK-3+N	SOL3	49
	IF (LILA(I).EQ.0) GO TO 40	SOL3	50
	IF (LILA(I).NE.2) GO TO 30	SOL3	51
	IF (N.EQ.1) GO TO 30	SOL3	52
	IF (N.EQ.2) FORCE(I+1)=FORCE(I)/TAN(RPHI)+FORCE(I+1)	SOL3	53
	IF (N.EQ.2) FORCE(I)=0.	SOL3	54
	IF (N.EQ.3) FORCE(I)=-FORCE(I-1)/TAN(RPHI)	SOL3	55
	GO TO 40	SOL3	56
	30 FORCE(I)=0.	SOL3	57
	40 CONTINUE	SOL3	58
C		SOL3	59
C	GO TO ANOTHER NODE	SOL3	60
C		SOL3	61
	50 CONTINUE	SOL3	62
C		SOL3	63
	DO 60 I=1,NA1	SOL3	64
	C(I)=FORCE(I)	SOL3	65
		SOL3	66
		SOL3	67

60 CONTINUE	SOL 3	68
C	SOL 3	69
C SOLVE EQUATION SYSTEM	SOL 3	70
C	SOL 3	71
CALL SOLV1 (C,X,LAMA,NIETE,ISEGM,LWIDTH,NA1,NUMNP,NPSEGM,NA5,ALL,ASOL 3	SOL 3	72
1L,H,V,Y)	SOL 3	73
C	SOL 3	74
DO 70 I=1,NA1	SOL 3	75
DISP(I)=X(I)	SOL 3	76
70 CONTINUE	SOL 3	77
C	SOL 3	78
WRITE (30) DISP	SOL 3	79
C	SOL 3	80
RETURN	SOL 3	81
END	SOL 3	82
SUBROUTINE SOLV1 (C,X,LAMA,NIETE,ISEGM,LWIDTH,NA1,NUMNP,NPSEGM,NA5	SOLV1	2
1,ALL,AL,H,V,Y)	SOLV1	3
C	SOLV1	4
C	SOLV1	5
C THIS SUBROUTINE SOLVES THE FORGE DISPLACEMENT EQUATIONS	SOLV1	6
C BY USING THE CHOLESKI DECOMPOSITION METHOD	SOLV1	7
C AND WAS CODED BY A. W. WEGMULLER, FRITZ LAB REPORTS	SOLV1	8
C 378A.3 AND 378A.4	SOLV1	9
C	SOLV1	10
DIMENSION C(NA1), X(NA5), Y(NA5), ALL(LWIDTH,LAMA), AL(LWIDTH,NIET	SOLV1	11
1E), H(LWIDTH), V(LWIDTH), DUMMY(1)	SOLV1	12
L=LWIDTH	SOLV1	13
N=LAMA	SOLV1	14
REWIND 3	SOLV1	15
REWIND 4	SOLV1	16
DO 10 K=1,NIETE	SOLV1	17
10 Y(K)=0.	SOLV1	18
DO 20 J=1,NIETE	SOLV1	19
DO 20 I=1,L	SOLV1	20
AL(I,J)=0.	SOLV1	21
20 CONTINUE	SOLV1	22
KOUNT=0	SOLV1	23
GO TO 40	SOLV1	24
30 IF (KZ2.EQ.NA1) GO TO 160	SOLV1	25
KZ1=1+KOUNT*N	SOLV1	26
KZ2=NA1	SOLV1	27
GO TO 50	SOLV1	28
40 IF (KOUNT.GE.ISEGM) GO TO 30	SOLV1	29
KZ1=1+KOUNT*N	SOLV1	30
KZ2=KZ1+N-1	SOLV1	31
50 READ (3) ALL	SOLV1	32
KZ3=0	SOLV1	33
DO 150 KZ=KZ1,KZ2	SOLV1	34
KZ3=KZ3+1	SOLV1	35
DO 60 I=1,L	SOLV1	36
60 V(I)=ALL(I,KZ3)	SOLV1	37
SUM=0.	SOLV1	38
DO 70 K=2,L	SOLV1	39
SUM=SUM+AL(K,L-K+1)**2	SOLV1	40
70 CONTINUE	SOLV1	41
V(1)=1.0/SQRT(V(1)-SUM)	SOLV1	42
NN=L-1	SOLV1	43
DO 90 M=2,NN	SOLV1	44

	LM=M-1	SOLV1 45
	MM=L-M	SOLV1 46
	SUM=0.	SOLV1 47
	DO 80 K=1,MM	SOLV1 48
	SUM=SUM+AL(LM+K+1,L-K)*AL(K+1,L-K)	SOLV1 49
80	CONTINUE	SOLV1 50
	V(M)=(V(M)-SUM)*V(1)	SOLV1 51
90	CONTINUE	SOLV1 52
	V(L)=V(L)*V(1)	SOLV1 53
	DO 100 I=1,L	SOLV1 54
100	ALL(I,KZ3)=V(I)	SOLV1 55
	DO 110 I=1,NIETE	SOLV1 56
	K=L-I+1	SOLV1 57
110	H(I)=AL(K,I)	SOLV1 58
	SUM=0.	SOLV1 59
	DO 120 J=1,NIETE	SOLV1 60
120	SUM=SUM+H(J)*Y(J+KZ-1)	SOLV1 61
	Y(NIETE+KZ)=(C(KZ)-SUM)*V(1)	SOLV1 62
	NIETA=NIETE-1	SOLV1 63
	DO 130 J=1,NIETA	SOLV1 64
	DO 130 I=1,L	SOLV1 65
	AL(I,J)=AL(I,J+1)	SOLV1 66
130	CONTINUE	SOLV1 67
	DO 140 I=1,L	SOLV1 68
	AL(I,NIETE)=V(I)	SOLV1 69
140	CONTINUE	SOLV1 70
150	CONTINUE	SOLV1 71
	WRITE (4) ALL	SOLV1 72
	KOUNT=KOUNT+1	SOLV1 73
	GO TO 40	SOLV1 74
160	DO 170 K=1,NA1	SOLV1 75
170	Y(K)=Y(K+L-1)	SOLV1 76
C	GENERATION OF UNKNOWN VECTOR X.	SOLV1 77
	NA=NA1+1	SOLV1 78
	NB=L+NA1-1	SOLV1 79
	DO 180 K=NA,NB	SOLV1 80
180	X(K)=0.	SOLV1 81
	BACKSPACE 4	SOLV1 82
	KOUNT=0	SOLV1 83
	KZ2=0	SOLV1 84
	KWP=NA1	SOLV1 85
	IQUANT=ISEGM*NPSEGM-NUMNP	SOLV1 86
	IF (IQUANT) 190,200,200	SOLV1 87
190	ISEGM=ISEGM+1	SOLV1 88
	IQUANT=ISEGM*NPSEGM-NUMNP	SOLV1 89
	KZ1=1+5*IQUANT	SOLV1 90
	KZ2=N	SOLV1 91
	KZ3=5*IQUANT	SOLV1 92
	KWP=5*ISEGM*NPSEGM	SOLV1 93
	GO TO 210	SOLV1 94
200	IF (KZ2.GE.NA1) GO TO 250	SOLV1 95
	KZ1=1+KOUNT*N	SOLV1 96
	KZ2=KZ1+N-1	SOLV1 97
	KZ3=0	SOLV1 98
210	READ (4) ALL	SOLV1 99
	DO 240 KZ=KZ1,KZ2	SOLV1100
	DO 220 I=1,L	SOLV1101
220	H(I)=ALL(I,N-KZ3)	SOLV1102

	SUM=0.	SOLV1103
	DO 230 J=2,L	SOLV1104
230	SUM=SUM+H(J)*X(KWP+J-KZ)	SOLV1105
	X(KWP-KZ+1)=(Y(KWP-KZ+1)-SUM)*H(1)	SOLV1106
	KZ3=KZ3+1	SOLV1107
240	CONTINUE	SOLV1108
	BACKSPACE 4	SOLV1109
	BACKSPACE 4	SOLV1110
	KOUNT=KOUNT+1	SOLV1111
	GO TO 200	SOLV1112
250	RETURN	SOLV1113
	END	SOLV1114
	OVERLAY (ECCP,4,0)	OVERL4 2
	PROGRAM OVERL4	OVERL4 3
C		OVERL4 4
C	THIS OVERLAY COMPUTES SOLUTIONS FOR INDIVIDUAL BEAMS PRIOR TO	OVERL4 5
C	OVERLOADING OF SUPERSTRUCTURE	OVERL4 6
C		OVERL4 7
	COMMON /OVER4/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13,N14,N15,	OVERL4 8
	1N16,N17,N18,N19,N20,N21,N22,N23,N24,N25,N26,N27	OVERL4 9
	COMMON /BDIM/ RON(48)	OVERL4 10
	COMMON /CB4/ IN,IO	OVERL4 11
	COMMON /CB2/ NBEAMX,NIX,NULAYB	OVERL4 12
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	OVERL4 13
	COMMON /ICORES/ ICR(3),ICORE	OVERL4 14
	COMMON /SCALED/ ISCALE,IFN	OVERL4 15
	COMMON /CB305/ PCTT,PCTC	OVERL4 16
	COMMON A(1)	OVERL4 17
C		OVERL4 18
C		OVERL4 19
	IF (ICORE.NE.0) GO TO 10	OVERL4 20
	ICORE=LOCF(A(1))+1	OVERL4 21
	GO TO 20	OVERL4 22
10	CONTINUE	OVERL4 23
	NUMBP=NPERBM+1	OVERL4 24
	LWIDTH=6	OVERL4 25
	NA1=(NPERBM+1)*3	OVERL4 26
	NA5=NA1+5	OVERL4 27
	CALL FORT4 (NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN,NBEAMX,NIX,	OVERL4 28
	1,NULAYB,NUMBP,LWIDTH,NA1,NA5,A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),	OVERL4 29
	2(N7),A(N8),A(N9),A(N10),A(N11),A(N12),A(N13),A(N14),A(N15),A(N16),	OVERL4 30
	3A(N17),A(N18),A(N19),A(N20),A(N21),A(N22),A(N23),A(N24),A(N25),A(N	OVERL4 31
	426))	OVERL4 32
20	CONTINUE	OVERL4 33
	END	OVERL4 34
	SUBROUTINE FORT4 (NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN,NBEAMX,	FORT4 2
	1MX,NIX,NULAYB,NUMBP,LWIDTH,NA1,NA5,SIG,DSIG,ESX,ESXC,ZCRDC,ASXLR,	FORT4 3
	2EPSPSC,AILRC,ITYPC,SIBXXA,ZCRD,ASXLR,EPSPS,AILR,ITYPE,TSHEAR,XLEN,	FORT4 4
	3,LILB,FORCE,TEMP,B,AK,X,Y,C,ALL)	FORT4 5
C		FORT4 6
C	THIS ROUTINE OBTAINS THE BEAM SOLUTIONS	FORT4 7
C		FORT4 8
	COMMON /BDIM/ RON(6),ROM(6),EDCWN(6),STRAN(6),SIGO(6),FT(6),RONT(3	FORT4 9
	1),ROMT(3),EDOWNT(3),EDOWNR(3)	FORT4 10
	COMMON /CB4/ IN,IO	FORT4 11
	COMMON /CB305/ PCTT,PCTC	FORT4 12
	COMMON /SCALED/ ISCALE,IFN	FORT4 13
	DIMENSION ESX(NULAYE,NBEAMX), SIG(NULAYB,NPERBM), DSIG(NULAYB,NPER	FORT4 14

	1BM), ESXC(NULAYB,NPERBM), ZCRDC(NULAYB,NPERBM), ASXLRC(NULAYB,NPERFORT	15
	2BM), EPSPSC(NULAYB,NPERBM), AILRC(NULAYB,NPERBM), ITYPE(NULAYB,NPEFORT	16
	3RBM), SIBXXA(NULAYB), ZCRO(NULAYB), ASXLR(NULAYB), EPSPS(NULAYB), FORT	17
	4AILR(NULAYB), ITYPE(NULAYB), TSHEAR(NULAYB), XLENC(NPERBM), LILB(NFORT	18
	5A1), FORCE(NA1), TEMP(NA1), B(NA1), AK(NA1,NA1), X(NA5), Y(NA5), CFORT	19
	6(NA1), ALL(LWIDTH,NA1), DUMMY(1)	FORT 20
C		FORT 21
C		FORT 22
	REWIND 3	FORT 23
	REWIND 4	FORT 24
	REWIND 5	FORT 25
	REWIND 15	FORT 26
	REWIND 25	FORT 27
	REWIND 27	FORT 28
	REWIND 33	FORT 29
	READ (33) ESX	FORT 30
	WRITE (IO,10)	FORT 31
	10 FORMAT (1H1,///,20X,*DEAD LOAD AND/OR PRESTRESS SOLUTION FOR INDIFORT	32
	1VIDUAL BEAMS*,//)	FORT 33
C		FORT 34
C		FORT 35
	READ (IN,140) NSOLVB	FORT 36
C		FORT 37
	WRITE (IO,20) NSOLVB	FORT 38
	20 FORMAT (1H0,//,20X,*NUMBER OF SOLUTION GROUPS =*,I5,///)	FORT 39
	IF (NSOLVB.GT.NOBM) WRITE (IO,30)	FORT 40
	30 FORMAT (1H0,//,6X,*NSOLVB IS GREATER THAN THE NUMBER OF BEAMS*,///FORT	41
	1)	FORT 42
	IF (NSOLVB.GT.NOBM) STOP 4	FORT 43
C		FORT 44
C	COMPUTE SOLUTIONS FOR NSOLVB GROUPS	FORT 45
C		FORT 46
	NL=1	FORT 47
	NT=0	FORT 48
	NRB=0	FORT 49
C		FORT 50
	DO 520 ITIMES=1,NSOLVB	FORT 51
C		FORT 52
	READ (IN,140) NSOLB	FORT 53
	NT=NT+NSOLB	FORT 54
C		FORT 55
	WRITE (IO,40) ITIMES,NSOLB,NL,NT	FORT 56
	40 FORMAT (1H0,//,6X,*SOLUTION NUMBER = *,I5,/,6X,*NUMBER OF DUPLICATFORT	57
	1E SOLUTIONS IN THIS GROUP =*,I5,/,6X,*STARTING BEAM = *,I5,* END BFORT	58
	2EAM = *,I5,///)	FORT 59
C		FORT 60
	IF (NT.GT.NOBM) WRITE (IO,50) NT,NOBM	FORT 61
	50 FORMAT (1H0,//,6X,*END BEAM =*,I5,*- IS GREATER THAN TOTAL NUMBERFORT	62
	1 OF BEAMS =*,I5,///)	FORT 63
	IF (NT.GT.NOBM) STOP 4	FORT 64
C		FORT 65
	NSTART=NPERBM*(NT-1)+1	FORT 66
C		FORT 67
	IF (NRB.GE.NT) REWIND 3	FORT 68
	IF (NRB.GE.NT) GO TO 90	FORT 69
C		FORT 70
C	READ NUMBER OF BEAMS IN THE INPUT GROUP	FORT 71
C		FORT 72



	READ (4) NB	FORT4 73
	NTSOL=0	FORT4 74
C		FORT4 75
C	FIND TOTAL NUMBER OF BEAMS INPUTTED	FORT4 76
C		FORT4 77
	N=NRB+NB	FORT4 78
	IF (NT.GT.N) WRITE (IO,60) NT,N	FORT4 79
	60 FORMAT (1H0,/,6X,*TOTAL NUMBER OF BEAM SOLUTIONS DESIRED =*,I5,*	FORT4 80
	1IS GREATER THAN TOTAL NUMBER OF BEAMS INPUTTED AT THIS POINT =*,I5	FORT4 81
	2,///)	FORT4 82
	IF (NT.GT.N) WRITE (IO,70)	FORT4 83
	70 FORMAT (1H0,/,6X,*CHECK THE NUMBER OF DUPLICATE SOLUTIONS IN THE*	FORT4 84
	1,/,6X,*SOLUTION GROUPS AGAINST THE DUPLICATE BEAMS IN THE BEAM *,/	FORT4 85
	2,6X,*INPUT SECTION*,/))	FORT4 86
	IF (NT.GT.N) STOP 4	FORT4 87
C		FORT4 88
C	READ BOUNDARY CONDITIONS FOR THE BEAM GROUP	FORT4 89
C		FORT4 90
	N=PPERBM+1	FORT4 91
	DO 80 I=1,N	FORT4 92
	80 READ (4) LILB(I*3-2),LILB(I*3-1),LILB(I*3)	FORT4 93
C		FORT4 94
C		FORT4 95
	90 CONTINUE	FORT4 96
C		FORT4 97
	NTSOL=NTSOL+NSOLB	FORT4 98
	IF (NTSOL.GT.NB) WRITE (IO,100) NB,NTSOL	FORT4 99
	100 FORMAT (1H0,/,6X,*NUMBER OF DUPLICATE BEAMS =*,I5,* IS LESS THAN*	FORT4 100
	1,/,6X,*THE NUMBER OF SOLUTIONS FOR THESE BEAMS =*,I5,///)	FORT4 101
	IF (NTSOL.GT.NB) WRITE (IO,70)	FORT4 102
	IF (NTSOL.GT.NB) STOP 4	FORT4 103
C		FORT4 104
	N=PPERBM+1	FORT4 105
	WRITE (IO,120)	FORT4 106
	WRITE (IO,130) (I,LILB(I*3-2),LILB(I*3-1),LILB(I*3),I=1,N)	FORT4 107
C		FORT4 108
C		FORT4 109
C	ASSIGN LAYER PROPERTIES	FORT4 110
C		FORT4 111
	DO 110 II=1,NSOLB	FORT4 112
	DO 110 J=1,PPERBM	FORT4 113
	L=NSTART+J-1	FORT4 114
	READ (15) SIBXXA	FORT4 115
	READ (27) XLNGTH,GKEIX,NPIX,NPKX,ITYPE,ASXLR,AILR,ZCRD,TSHEAR	FORT4 116
	XLENC(J)=XLNGTH	FORT4 117
	DO 110 I=1,NULAYB	FORT4 118
	ZCRDC(I,J)=ZCRD(I)	FORT4 119
	ITYPC(I,J)=ITYPE(I)	FORT4 120
	ASXLR(I,J)=ASXLR(I)	FORT4 121
	AILRC(I,J)=AILR(I)	FORT4 122
	SIG(I,J)=SIBXXA(I)	FORT4 123
	EPSPSC(I,J)=0.0	FORT4 124
	IF (ESX(I,L).NE.0.0) EPSPSC(I,J)=SIG(I,J)/ESX(I,L)	FORT4 125
	110 ESXC(I,J)=ESX(I,L)	FORT4 126
C		FORT4 127
C		FORT4 128
	120 FORMAT (1H0,/,20X,*BOUNDARY CONDITIONS*,/,6X,*NODAL POINT	FORT4 129
	1ISP W-DISP MY-DISP*,/))	FORT4 130

130	FORMAT (5X,I5,7X,I5,7X,I5,7X,I5)	FORT4131
C		FORT4132
C	READ NUMBER OF FORCE CARDS	FORT4133
C		FORT4134
	READ (IN,140) NFORCE	FORT4135
140	FORMAT (I5)	FORT4136
C		FORT4137
C	INITIALIZE	FORT4138
C		FORT4139
	DO 150 I=1,NA1	FORT4140
150	FORCE(I)=0.	FORT4141
C		FORT4142
	WRITE (IO,160) NFORCE	FORT4143
160	FORMAT (1H0,/,5X,*NUMBER OF NODAL POINT FORCE CARDS = *,I3)	FORT4144
C		FORT4145
	IF (NFORCE.EQ.0) GO TO 220	FORT4146
C		FORT4147
C	READ FORCES	FORT4148
C		FORT4149
	DO 170 I=1,NFORCE	FORT4150
170	READ (IN,180) J,FORCE (3*J-2),FORCE (3*J-1),FORCE (3*J)	FORT4151
180	FORMAT (I5,5F10.0,25X)	FORT4152
	WRITE (IO,200)	FORT4153
	WRITE (IO,210)	FORT4154
	WRITE (IO,190) (M,FORCE (3*M-2),FORCE (3*M-1),FORCE (3*M),M=1,NUMBP)	FORT4155
190	FORMAT (1H ,5X,I5,2X,3F20.4)	FORT4156
200	FORMAT (1H0,/,20X,*INPUTTED NODAL POINT FORCES ON INDIVIDUAL BEAM	FORT4157
	1S (KIPS AND IN-KIPS)*,/,)	FORT4158
210	FORMAT (1H0,/,6X,*NODAL POINT*,10X,*U-LOAD*,14X,*W-LOAD*,13X,*MY-	FORT4159
	1LOAD*,/,)	FORT4160
C		FORT4161
C		FORT4162
	220 CONTINUE	FORT4163
C		FORT4164
C	READ THE UNIFORM AND CONCENTRATED LOADS	FORT4165
C		FORT4166
	CALL BLOAD (NA1,NPERBY,XLENC,FORCE,IN,IO,NRB,NT)	FORT4167
C		FORT4168
	WRITE (IO,230)	FORT4169
	WRITE (IO,230)	FORT4170
230	FORMAT (1H )	FORT4171
	WRITE (IO,240)	FORT4172
240	FORMAT (1H0,20X,*NODAL POINT FORCES INCLUDING SPECIFIED UNIFORM AN	FORT4173
	10 CONCENTRATED LOADS*)	FORT4174
	WRITE (IO,210)	FORT4175
C		FORT4176
	WRITE (IO,190) (M,FORCE (3*M-2),FORCE (3*M-1),FORCE (3*M),M=1,NUMBP)	FORT4177
	KOUNT2=0	FORT4178
	ICY=0	FORT4179
	JACK=0	FORT4180
	ICY=ICY+1	FORT4181
	DO 250 I=1,NA1	FORT4182
250	TEMP(I)=0.	FORT4183
260	KODE=1	FORT4184
	JACK=JACK+1	FORT4185
C		FORT4186
	WRITE (IO,230)	FORT4187
	WRITE (IO,270) JACK	FORT4188

270	FORMAT (1H0,5X,*ITERATION FOR BEAM SOLUTION = *,I3)	FORT4189
C		FORT4190
C	GET BEAM STIFFNESS	FORT4191
C		FORT4192
	CALL STIFF (NA1,NUMBP,AK,NULAYB,NPERBM,ASXLRC,ESXC,ZCRDC,AILRC,XLE	FORT4193
	1NC,KODE)	FORT4194
C		FORT4195
C	ENFORCE BOUNDARY CONDITIONS	FORT4196
C		FORT4197
	DO 290 KK=1,NA1	FORT4198
	IF (LILB(KK)) 280,290,280	FORT4199
280	FORCE(KK)=0.	FORT4200
	AK(KK,KK)=1.0E30	FORT4201
290	CONTINUE	FORT4202
C		FORT4203
C	SOLVE FORCE DISPLACEMENT EQUATIONS	FORT4204
C		FORT4205
	DO 300 I=1,NA1	FORT4206
300	B(I)=FORCE(I)	FORT4207
	CALL SOLV2 (AK,9,NA1,NA5,X,ALL,Y,C)	FORT4208
C		FORT4209
C	GET STRAINS AND STRESSES	FORT4210
C		FORT4211
	DO 310 I=1,NPERBM	FORT4212
	L=NSTART-1+I	FORT4213
	EPS1=B(3*I+1)-B(3*I-2)	FORT4214
	EPS2=B(3*I+3)-B(3*I)	FORT4215
	DO 310 J=1,NULAYB	FORT4216
310	DSIG(J,I)=((EPS1+EPS2*ZCRDC(J,I))*ESXC(J,I))/XLENC(I)	FORT4217
C		FORT4218
C	CHECK CONVERGENCE	FORT4219
C		FORT4220
	DO 320 I=1,NA1	FORT4221
	IF (ABS(B(I)).LE..000000001) GO TO 320	FORT4222
	IF (ABS((B(I)-TEMP(I))/B(I))-0.01) 320,330,330	FORT4223
320	CONTINUE	FORT4224
	GO TO 410	FORT4225
C		FORT4226
C	CHECK SCALING LIMITS AND LAYER FAILURE	FORT4227
C		FORT4228
330	DO 370 I=1,NPERBM	FORT4229
	L=NSTART+I-1	FORT4230
	DO 370 J=1,NULAYB	FORT4231
	K=ITYPC(J,I)	FORT4232
	A=SIG(J,I)+DSIG(J,I)	FORT4233
	IF (A.GT.0.J.AND.K.LE.KC) GO TO 340	FORT4234
	S=SIGO(K)	FORT4235
	RM=ROM(K)	FORT4236
	RN=RON(K)	FORT4237
	GO TO 350	FORT4238
340	CONTINUE	FORT4239
	S=FT(K)	FORT4240
	RM=ROMT(K)	FORT4241
	RN=RONT(K)	FORT4242
350	CONTINUE	FORT4243
	IF (K.LE.KC.AND.ABS(A).GT.ABS(S)) ESX(J,L)=0.	FORT4244
	E=ESX(J,L)	FORT4245
	ESXC(J,I)=E/(1.0+RN*(1.0-RM)/RM*((ABS(A)/S)**(RN-1.0)))	FORT4246

	RR=PCTC	FORT4247
	IF (A.GT.0.C) RR=PCTT	FORT4248
	IF (K.GT.KC) RR=PCTT	FORT4249
	IF (ABS(A).GT.ABS(RR*S).AND.IFN.NE.3HYES) WRITE (IO,360)	FORT4250
360	FORMAT (1H0,/,6X,*PCTC OR PCTT LIMITS EXCEEDED IN FORT4*,/)	FORT4251
	IF (ABS(A).GT.ABS(RR*S)) IFN=3HYES	FORT4252
370	CONTINUE	FORT4253
	IF (JACK.LE.15) GO TO 390	FORT4254
	WRITE (IO,380)	FORT4255
380	FORMAT (1H0,/,5X,*NO CONVERGENCE*,/,5X*MAX NUMBER OF ITERATIONS	FORT4256
	1ALLOWED = 15*)	FORT4257
C		FORT4258
	STOP	FORT4259
390	CONTINUE	FORT4260
	DO 400 I=1,NA1	FORT4261
400	TEMP(I)=B(I)	FORT4262
C		FORT4263
C	DO ANOTHER ITERATION	FORT4264
C		FORT4265
	GO TO 260	FORT4266
410	CONTINUE	FORT4267
C		FORT4268
C	END OF SOLUTION ROUTINE	FORT4269
C		FORT4270
	WRITE (IO,420)	FORT4271
420	FORMAT (1H0,/,20X,*NODAL POINT DISPLACEMENTS FOR THE BEAM*)	FORT4272
	WRITE (IO,430)	FORT4273
430	FORMAT (1H ,*-----	FORT4274
	1-----*)	FORT4275
	WRITE (IO,490)	FORT4276
	WRITE (IO,440) (B(I),I=1,NA1)	FORT4277
440	FORMAT (1H ,6X,F9.5,12X,F9.5,12X,F9.5)	FORT4278
	WRITE (IO,480)	FORT4279
	DO 460 I=1,NPERBM	FORT4280
	EPS1=B(3*I+1)-B(3*I-2)	FORT4281
	EPS2=B(3*I+3)-B(3*I)	FORT4282
	L=NSTART-1+I	FORT4283
	DO 450 J=1,NULAYB	FORT4284
	A=(EPS1+EPS2*ZCRDC(J,I))/XLENC(I)	FORT4285
	A=A+EPSPSC(J,I)	FORT4286
	EPSPS(J)=A	FORT4287
	EPSPSC(J,I)=A	FORT4288
	SIBXXA(J)=SIG(J,I)+DSIG(J,I)	FORT4289
	SIG(J,I)=SIBXXA(J)	FORT4290
450	CONTINUE	FORT4291
460	CONTINUE	FORT4292
	DO 470 I=1,NULAYB	FORT4293
470	WRITE (IO,500) (SIG(I,J),J=1,NPERBM)	FORT4294
C		FORT4295
480	FORMAT (1H0,/,5X,*STRESSES (KSI) - ELEMENTS HORIZONTAL, LAYERS VEFORT4296	
	1RTICAL*,/)	FORT4297
490	FORMAT (1H0,9X,*U-DISP (IN) W-DISP (IN) MY-DISP	FORT4298
	1(RADIANS) *,/)	FORT4299
500	FORMAT (1H ,5X,14F9.4)	FORT4300
C		FORT4301
C	SET REMAINING BEAMS EQUAL TO LAST BEAM SOLUTION	FORT4302
C		FORT4303
C		FORT4304

	DO 510 I=NL,NT	FORT4305
	DO 510 J=1,NPERBM	FORT4306
	WRITE (5) (SIG(L,J),L=1,NULAYB)	FORT4307
	WRITE (25) (EPSPSC(L,J),L=1,NULAYB)	FORT4308
	KK=NSTART+J-1	FORT4309
	K=(I-1)*NPERBM+J	FORT4310
	DO 510 L=1,NULAYB	FORT4311
	ESX(L,K)=ESX(L,KK)	FORT4312
	510 CONTINUE	FORT4313
C		FORT4314
C		FORT4315
	IF (NT.EQ.NOBM) GO TO 530	FORT4316
	NL=NT+1	FORT4317
	IF (NRB.LT.NT) NRB=NRB+NB	FORT4318
C		FORT4319
C	GO TO ANOTHER GROUP OF BEAMS	FORT4320
C		FORT4321
	520 CONTINUE	FORT4322
C		FORT4323
C		FORT4324
	530 CONTINUE	FORT4325
C		FORT4326
	REWIND 5	FORT4327
	REWIND 15	FORT4328
	CALL FTNCOPY (4LJUM5,3LJ15,0)	FORT4329
	REWIND 33	FORT4330
	WRITE (33) ESX	FORT4331
C		FORT4332
	WRITE (IO,540)	FORT4333
	540 FORMAT (1H0,/,9X,*INTERNAL MOMENTS (IN-KIPS) AND NORMAL FORCES (K	FORT4334
	1IPS) IN BEAM ELEMENTS*,/,1H ,7X,*EL*,9X,*MB*,17X,*N3*,/)	FORT4335
C		FORT4336
	REWIND 15	FORT4337
	REWIND 27	FORT4338
C		FORT4339
	DO 570 I=1,NBEAMX	FORT4340
	READ (15) SIBXXA	FORT4341
	READ (27) XLNGTH,GKEIX,NPIX,NPKX,ITYPE,ASXLR,AILR,ZCRD,TSHEAR	FORT4342
	A=0.	FORT4343
	S=0.	FORT4344
C		FORT4345
	DO 550 J=1,NULAYB	FORT4346
	A=A+SIBXXA(J)*ASXLR(J)*ZCRD(J)	FORT4347
	S=S+SIBXXA(J)*ASXLR(J)	FORT4348
	550 CONTINUE	FORT4349
C		FORT4350
	WRITE (IO,560) I,A,S	FORT4351
	560 FORMAT (5X,I5,F15.4,3X,F15.4)	FORT4352
	570 CONTINUE	FORT4353
C		FORT4354
	RETURN	FORT4355
	END	FORT4356
	SUBROUTINE BLOAD (NA1,NPERBM,XLENC,FORCE,IN,IO,NRB,NT)	BLOAD 2
C		BLOAD 3
C	THIS SUBROUTINE READS UNIFORM AND CONCENTRATED LOADS ON THE	BLOAD 4
C	BEAM	BLOAD 5
C		BLOAD 6
	DIMENSION FORCE(NA1), XLENC(NPERBM)	BLOAD 7

C		BLOAD 8
C	READ THE UNIFORM LOAD	BLOAD 9
C		BLOAD 10
	WRITE (IO,10)	BLOAD 11
10	FORMAT (1H )	BLOAD 12
	READ (IN,20) W	BLOAD 13
20	FORMAT (2F10.0)	BLOAD 14
	WRITE (IO,30) W	BLOAD 15
30	FORMAT (1H0,5X,*UNIFORM LOAD ON BEAM*,14X,*=*,F9.4,* KIPS/IN*)	BLOAD 16
	IF (W.EQ.0.) GO TO 40	BLOAD 17
C		BLOAD 18
C	GET NODAL POINT FORCES	BLOAD 19
C		BLOAD 20
	CALL UNIFORM (FORCE,XLENC,W,NPERBM,NA1)	BLOAD 21
40	CONTINUE	BLOAD 22
C		BLOAD 23
C	READ NUMBER OF CONCENTRATED LOAD CARDS	BLOAD 24
C		BLOAD 25
	WRITE (IO,10)	BLOAD 26
	READ (IN,50) NC	BLOAD 27
50	FORMAT (I5)	BLOAD 28
	WRITE (IO,60) NC	BLOAD 29
60	FORMAT (1H0,5X,*NUMBER OF CONCENTRATED LOAD CARDS =*,I5)	BLOAD 30
	IF (NC.EQ.0) GO TO 100	BLOAD 31
	WRITE (IO,10)	BLOAD 32
	WRITE (IO,70)	BLOAD 33
70	FORMAT (1H0,5X,*LOAD CARD LOAD (KIPS) POSITION (IN)*,//)	BLOAD 34
	DO 90 I=1,NC	BLOAD 35
	READ (IN,20) P,X	BLOAD 36
	WRITE (IO,80) I,P,X	BLOAD 37
80	FORMAT (1H ,11X,I3,5X,F9.4,5X,F8.2)	BLOAD 38
	CALL CONC (FORCE,XLENC,NPERBM,NA1,P,X,IO)	BLOAD 39
90	CONTINUE	BLOAD 40
C		BLOAD 41
C	ADD PRESTRESS FORCES	BLOAD 42
C		BLOAD 43
100	CONTINUE	BLOAD 44
C		BLOAD 45
C	GET NODAL POINT FORCES	BLOAD 46
C		BLOAD 47
	IR=4	BLOAD 48
	IF (NRB.GE.NT) IR=3	BLOAD 49
C		BLOAD 50
C	READ NUMBER OF PRESTRESS GROUPS (OR LAYERS)	BLOAD 51
C		BLOAD 52
	READ (IR) NPS	BLOAD 53
	IF (NRB.LT.NT) WRITE (3) NPS	BLOAD 54
	IF (NPS.EQ.0) GO TO 200	BLOAD 55
	WRITE (IO,110)	BLOAD 56
110	FORMAT (1H0,//,6X,*FORCES ON THE BEAM DUE TO PRESTRESS*)	BLOAD 57
	DO 160 JJ=1,NPS	BLOAD 58
	WRITE (IO,10)	BLOAD 59
	WRITE (IO,120) JJ	BLOAD 60
120	FORMAT (1H0,5X,*PRESTRESS GROUP*,I3)	BLOAD 61
C		BLOAD 62
C	IDENTIFY SHAPE OF STRAND	BLOAD 63
C		BLOAD 64
	READ (IR) NAME,NC	BLOAD 65

	IF (NRB.LT.NT) WRITE (3) NAME,NC	BLOAD 66
	IF (NAME.EQ.5HPARAB) GO TO 130	BLOAD 67
	IF (NAME.EQ.5HSTRAI) GO TO 140	BLOAD 68
	GO TO 200	BLOAD 69
C	130 CONTINUE	BLOAD 70
C	FOR PARABCLIC STRAND	BLOAD 71
C	READ (IR) W	BLOAD 72
	IF (NRB.LT.NT) WRITE (3) W	BLOAD 73
	WRITE (IO,30) W	BLOAD 74
	CALL UNIFRM (FORCE,XLENC,W,NPERBM,NA1)	BLOAD 75
	GO TO 160	BLOAD 76
	140 CONTINUE	BLOAD 77
C	FOR STRAIGHT LINE SEGMENTS	BLOAD 78
C	WRITE (IO,60) NC	BLOAD 79
	IF (NC.LE.0) GO TO 160	BLOAD 80
	WRITE (IO,70)	BLOAD 81
	DO 150 I=1,NC	BLOAD 82
	READ (IR) X,P	BLOAD 83
	IF (NRB.LT.NT) WRITE (3) X,P	BLOAD 84
	WRITE (IO,80) I,P,X	BLOAD 85
	CALL CONC (FORCE,XLENC,NPERBM,NA1,P,X,IO)	BLOAD 86
	150 CONTINUE	BLOAD 87
	160 CONTINUE	BLOAD 88
	READ (IR) NAME,NC	BLOAD 89
	IF (NRB.LT.NT) WRITE (3) NAME,NC	BLOAD 90
	IF (NAME.EQ.5HEND ) GO TO 170	BLOAD 91
	GO TO 200	BLOAD 92
	170 CONTINUE	BLOAD 93
C	END FORCES ON BEAM	BLOAD 94
C	WRITE (IO,10)	BLOAD 95
	WRITE (IO,180)	BLOAD 96
	180 FORMAT (1H0,5X,*END FORCES ON BEAM DUE TO PRESTRESS*,//,6X,*NODE	BLOAD 97
	1 AXIAL LOAD(KIPS) VERTICAL LOAD (KIPS) MOMENT (IN-KIPS)*)	BLOAD 98
	I=1	BLOAD 99
	READ (IR) P,W,X	BLOAD 100
	IF (NRB.LT.NT) WRITE (3) P,W,X	BLOAD 101
	WRITE (IO,190) I,W,P,X	BLOAD 102
	190 FORMAT (1H0,5X,I4,4X,F9.2,15X,F9.4,9X,F11.3)	BLOAD 103
	FORCE(I*3)=FORCE(I*3)+X	BLOAD 104
	FORCE(I*3-1)=FORCE(I*3-1)+P	BLOAD 105
	FORCE(I*3-2)=FORCE(I*3-2)+W	BLOAD 106
	I=NPERBM+1	BLOAD 107
	READ (IR) P,W,X	BLOAD 108
	IF (NRB.LT.NT) WRITE (3) P,W,X	BLOAD 109
	WRITE (IO,190) I,W,P,X	BLOAD 110
	FORCE(I*3)=FORCE(I*3)+X	BLOAD 111
	FORCE(I*3-1)=FORCE(I*3-1)+P	BLOAD 112
	FORCE(I*3-2)=FORCE(I*3-2)+W	BLOAD 113
	200 RETURN	BLOAD 114
	END	BLOAD 115
	SUBROUTINE CONC (FORCE,XLENC,NPERBM,NA1,P,X,IO)	BLOAD 116
C		CONC 2
		CONC 3

C	THIS SUBROUTINE COMPUTES THE NODAL POINT FORCES FROM CONCENTRATED	CONC	4
C	VERTICAL LOADS	CONC	5
C		CONC	6
C	DIMENSION FORCE(NA1), XLENC(NPERBM)	CONC	7
C	FIND LOADED ELEMENT	CONC	8
C		CONC	9
C	DIST=0.	CONC	10
C	DO 10 J=1,NPERBM	CONC	11
C	DIST=DIST+XLENC(J)	CONC	12
C	IF ((DIST+.001).GT.X) GO TO 30	CONC	13
C	10 CONTINUE	CONC	14
C	WRITE (IO,20)	CONC	15
C	20 FORMAT (1H0,5X,20(1H*),*INCORRECT LOAD POSITION- ABORT FROM SUBROU	CONC	16
C	1TINE CONG*,//)	CONC	17
C		CONC	18
C	STOP 4	CONC	19
C	30 CONTINUE	CONC	20
C		CONC	21
C	COMPUTE FIX END FORCES	CONC	22
C		CONC	23
C	XL=XLENC(J)	CONC	24
C	B=DIST-X	CONC	25
C	A=XL-B	CONC	26
C	AM=P*A*B*B/XL/XL	CONC	27
C	BM=P*B*A*A/XL/XL	CONC	28
C	AV=P*B*B*(2.*A+XL)/XL/XL/XL	CONC	29
C	BV=P*A*A*(2.0*B+XL)/XL/XL/XL	CONC	30
C	FORCE(J*3)=FORCE(J*3)-AM	CONC	31
C	FORCE(J*3+3)=FORCE(J*3+3)+BM	CONC	32
C	FORCE(J*3-1)=FORCE(J*3-1)+AV	CONC	33
C	FORCE(J*3+2)=FORCE(J*3+2)+BV	CONC	34
C	RETURN	CONC	35
C	END	CONC	36
C	SUBROUTINE UNIFRM (FORCE,XLENC,W,NPERBM,NA1)	UNIFRM	2
C		UNIFRM	3
C	THIS SUBROUTINE COMPUTES THE NODAL POINT FORCES FROM	UNIFRM	4
C	A UNIFORM LINE LOAD ON THE BEAM	UNIFRM	5
C		UNIFRM	6
C	DIMENSION FORCE(NA1), XLENC(NPEREM)	UNIFRM	7
C		UNIFRM	8
C	DO 10 I=1,NPERBM	UNIFRM	9
C	J=I+1	UNIFRM	10
C	XL=XLENC(I)	UNIFRM	11
C	FORCE(I*3)=FORCE(I*3)-W*XL*XL/12.	UNIFRM	12
C	FORCE(J*3)=FORCE(J*3)+W*XL*XL/12.	UNIFRM	13
C	FORCE(I*3-1)=FORCE(I*3-1)+W*XL/2.	UNIFRM	14
C	FORCE(J*3-1)=FORCE(J*3-1)+W*XL/2.	UNIFRM	15
C	10 CONTINUE	UNIFRM	16
C		UNIFRM	17
C	RETURN	UNIFRM	18
C	END	UNIFRM	19
C	SUBROUTINE STIFF (NA1,NUMNP,AK,NLAY,NPERBM,ASXLR,ESX,ZCRD,AILR,XLESTIFF	STIFF	2
C	1N,KODE)	STIFF	3
C		STIFF	4
C	THIS SUBROUTINE COMPUTES THE STIFFNESS COEFFICIENTS FOR THE BEAM	STIFF	5
C		STIFF	6
C	DIMENSION AK(NA1,NA1), XLEN(NPERBM), ASXLR(NLAY,NPERBM), ESX(NLAY,STIFF	STIFF	7
C	1NPERBM), ZCRD(NLAY,NPERBM), AILR(NLAY,NPERBM), ABAR(2), SBAR(2), ASTIFF	STIFF	8



	2IEAR(2)	STIFF 9
	10 FORMAT (1H ,12F10.3)	STIFF 10
	KODE=1	STIFF 11
C		STIFF 12
C	INITIALIZE STIFFNESS MATRIX.	STIFF 13
C		STIFF 14
	DO 20 I=1,NA1	STIFF 15
	DO 20 J=1,NA1	STIFF 16
	20 AK(I,J)=0.	STIFF 17
G		STIFF 18
C	SET UP STIFFNESS COEFFICIENTS OF A NODE POINT BY CONSIDERING THAT	STIFF 19
C	NODE POINT AND THE TWO ADJACENT NODES WITH SPECIAL PROVISION FOR	STIFF 20
C	FIRST AND LAST NODES.	STIFF 21
G		STIFF 22
	DO 170 J=1,NUMNP	STIFF 23
	L=3*J	STIFF 24
C		STIFF 25
C	INITIALIZE A SET OF LAYER PROPERTIES.	STIFF 26
C		STIFF 27
	DO 30 JJ=1,2	STIFF 28
	ABAR(JJ)=0.	STIFF 29
	SBAR(JJ)=0.	STIFF 30
	30 AIBAR(JJ)=0.	STIFF 31
	JOE=1	STIFF 32
	40 IF (JOE-3) 50,100,50	STIFF 33
	50 CONTINUE	STIFF 34
	IF (J-1) 70,60,70	STIFF 35
	60 JOE=2	STIFF 36
	70 KK=J-2+JOE	STIFF 37
G		STIFF 38
C	COMPUTE LAYER PROPERTIES..	STIFF 39
G		STIFF 40
	DO 80 I=1,NLAY	STIFF 41
	ABAR(JOE)=ABAR(JOE)+ASXLR(I, KK)*ESX(I, KK)	STIFF 42
	SBAR(JOE)=SBAR(JOE)+ASXLR(I, KK)*ZCRD(I, KK)*ESX(I, KK)	STIFF 43
	80 AIBAR(JOE)=AIBAR(JOE)+ASXLR(I, KK)*ZCRD(I, KK)**2*ESX(I, KK)+AILR(I, K	STIFF 44
	1K)*ESX(I, KK)	STIFF 45
	ABAR(JOE)=ABAR(JOE)/XLEN(KK)	STIFF 46
	SBAR(JOE)=SBAR(JOE)/XLEN(KK)	STIFF 47
	AIBAR(JOE)=AIBAR(JOE)/XLEN(KK)	STIFF 48
	IF (J-NUMNP) 90,110,90	STIFF 49
	90 JOE=JOE+1	STIFF 50
	GO TO 40	STIFF 51
	100 CONTINUE	STIFF 52
	110 CONTINUE	STIFF 53
	IF (J-1) 130,120,130	STIFF 54
	120 GO TO 160	STIFF 55
	130 IF (J-NUMNP) 150,140,150	STIFF 56
	140 KODE=2	STIFF 57
C		STIFF 58
C	COMPUTE STIFFNESS COEFFICIENTS OF LAST NODE AND SKIP TO NO. 142,	STIFF 59
C	IF LAST NODE SKIP PART AFTER NUMBER 142.	STIFF 60
C		STIFF 61
	150 AK(L-2,L-5)=-ABAR(1)	STIFF 62
	AK(L-2,L-3)=-SBAR(1)	STIFF 63
	AK(L-2,L-2)=ABAR(1)	STIFF 64
	AK(L-2,L)=SBAR(1)	STIFF 65
	AK(L-1,L-4)=12.*AIBAR(1)/XLEN(J-1)/XLEN(J-1)*(-1.)	STIFF 66

	AK(L-1,L-3)=6.*AIBAR(1)/XLEN(J-1)	STIFF 67
	AK(L-1,L-1)=-AK(L-1,L-4)	STIFF 68
	AK(L-1,L)=AK(L-1,L-3)	STIFF 69
	AK(L,L-5)=-SBAR(1)	STIFF 70
	AK(L,L-4)=-6.*AIBAR(1)/XLEN(J-1)	STIFF 71
	AK(L,L-3)=2.*AIBAR(1)	STIFF 72
	AK(L,L-2)=SBAR(1)	STIFF 73
	AK(L,L-1)=-AK(L,L-4)	STIFF 74
	AK(L,L)=4.*AIBAR(1)	STIFF 75
	GO TO (160,170), KOCE	STIFF 76
160	AK(L-2,L-2)=AK(L-2,L-2)+ABAR(2)	STIFF 77
	AK(L-2,L)=AK(L-2,L)+SBAR(2)	STIFF 78
	AK(L-2,L+1)=-ABAR(2)	STIFF 79
	AK(L-2,L+3)=-SBAR(2)	STIFF 80
	AK(L-1,L-1)=AK(L-1,L-1)+12.*AIBAR(2)/(XLEN(J)**2)	STIFF 81
	AK(L-1,L)=AK(L-1,L)-6.*AIBAR(2)/XLEN(J)	STIFF 82
	AK(L-1,L+2)=-12.*AIBAR(2)/(XLEN(J)**2)	STIFF 83
	AK(L-1,L+3)=-6.*AIBAR(2)/XLEN(J)	STIFF 84
	AK(L,L-2)=AK(L,L-2)+SBAR(2)	STIFF 85
	AK(L,L-1)=AK(L,L-1)-6.*AIBAR(2)/XLEN(J)	STIFF 86
	AK(L,L)=AK(L,L)+4.*AIBAR(2)	STIFF 87
	AK(L,L+1)=-SBAR(2)	STIFF 88
	AK(L,L+2)=6.*AIBAR(2)/XLEN(J)	STIFF 89
	AK(L,L+3)=2.*AIBAR(2)	STIFF 90
170	CONTINUE	STIFF 91
	RETURN	STIFF 92
	END	STIFF 93
	SUBROUTINE SOLV2 (AK,B,NA1,NA5,X,ALL,Y,C)	SOLV2 2
C		SOLV2 3
C	SOLVE FORCE DISPLACEMENT EQUATIONS USING CHOLESKI DECOMPOSITION	SOLV2 4
C		SOLV2 5
	DIMENSION AK(NA1,NA1), B(NA1), X(NA5), Y(NA5), C(NA1), ALL(6,NA1),	SOLV2 6
	1 H(6), V(6), AL(6,5)	SOLV2 7
	NUMNP=NA1/3	SOLV2 8
	NPSEGM=NUMNP	SOLV2 9
	NIETE=5	SOLV2 10
	ISEGM=1	SOLV2 11
	LWIDTH=6	SOLV2 12
	LAMA=NA1	SOLV2 13
	DO 20 JA=1,NA1	SOLV2 14
	DO 20 IA=1,LWIDTH	SOLV2 15
	ALL(IA,JA)=0.	SOLV2 16
	IF (JA+IA-1-NA1) 10,10,20	SOLV2 17
10	ALL(IA,JA)=(AK(JA,JA+IA-1)+AK(JA+IA-1,JA))/2.	SOLV2 18
20	CONTINUE	SOLV2 19
	DO 30 I=1,NA1	SOLV2 20
30	C(I)=B(I)	SOLV2 21
C		SOLV2 22
C	ORIGINAL SOLV1 STARTS HERE W/O TAPES	SOLV2 23
C	DECOMPOSITION INTO LOWER AND UPPER TRIANG.MATRIX	SOLV2 24
C	AND GENERATION OF AUXILIARY VECTOR Y	SOLV2 25
C		SOLV2 26
	L=LWIDTH	SOLV2 27
	N=LAMA	SOLV2 28
	DO 40 K=1,NIETE	SOLV2 29
40	Y(K)=0.	SOLV2 30
	DO 50 J=1,NIETE	SOLV2 31
	DO 50 I=1,L	SOLV2 32

	AL(I,J)=0.	SOLV2 33
50	CONTINUE	SOLV2 34
	KOUNT=0	SOLV2 35
	GO TO 70	SOLV2 36
60	IF (KZ2.EQ.NA1) GO TO 190	SOLV2 37
	KZ1=1+KOUNT*N	SOLV2 38
	KZ2=NA1	SOLV2 39
	GO TO 80	SOLV2 40
70	IF (KOUNT.GE.ISEGN) GO TO 60	SOLV2 41
	KZ1=1+KOUNT*N	SOLV2 42
	KZ2=KZ1+N-1	SOLV2 43
80	CONTINUE	SOLV2 44
	KZ3=0	SOLV2 45
	DO 180 KZ=KZ1,KZ2	SOLV2 46
	KZ3=KZ3+1	SOLV2 47
	DO 90 I=1,L	SOLV2 48
90	V(I)=ALL(I,KZ3)	SOLV2 49
	SUM=0.	SOLV2 50
	DO 100 K=2,L	SOLV2 51
	SUM=SUM+AL(K,L-K+1)**2	SOLV2 52
100	CONTINUE	SOLV2 53
	V(1)=1.0/SQRT(V(1)-SUM)	SOLV2 54
	NN=L-1	SOLV2 55
	DO 120 M=2,NN	SOLV2 56
	LM=M-1	SOLV2 57
	MM=L-M	SOLV2 58
	SUM=0.	SOLV2 59
	DO 110 K=1,MM	SOLV2 60
	SUM=SUM+AL(LM+K+1,L-K)*AL(K+1,L-K)	SOLV2 61
110	CONTINUE	SOLV2 62
	V(M)=(V(M)-SUM)*V(1)	SOLV2 63
120	CONTINUE	SOLV2 64
	V(L)=V(L)*V(1)	SOLV2 65
	DO 130 I=1,L	SOLV2 66
130	ALL(I,KZ3)=V(I)	SOLV2 67
	DO 140 I=1,NIETE	SOLV2 68
	K=L-I+1	SOLV2 69
140	H(I)=AL(K,I)	SOLV2 70
	SUM=0.	SOLV2 71
	DO 150 J=1,NIETE	SOLV2 72
150	SUM=SUM+H(J)*Y(J+KZ-1)	SOLV2 73
	Y(NIETE+KZ)=(G(KZ)-SUM)*V(1)	SOLV2 74
	NIETA=NIETE-1	SOLV2 75
	DO 160 J=1,NIETA	SOLV2 76
	DO 160 I=1,L	SOLV2 77
	AL(I,J)=AL(I,J+1)	SOLV2 78
160	CONTINUE	SOLV2 79
	DO 170 I=1,L	SOLV2 80
	AL(I,NIETE)=V(I)	SOLV2 81
170	CONTINUE	SOLV2 82
180	CONTINUE	SOLV2 83
	KOUNT=KOUNT+1	SOLV2 84
	GO TO 70	SOLV2 85
190	DO 200 K=1,NA1	SOLV2 86
200	Y(K)=Y(K+L-1)	SOLV2 87
C		SOLV2 88
C	GENERATION OF UNKNOWN VECTOR X.	SOLV2 89
C		SOLV2 90

	NA=NA1+1	SOLV2 91
	NB=L+NA1-1	SOLV2 92
	DO 210 K=NA,NB	SOLV2 93
210	X(K)=0.	SOLV2 94
	KOUNT=0	SOLV2 95
	KZ2=0	SOLV2 96
	KWP=NA1	SOLV2 97
	IQUANT=ISEGM*NPSEGM-NUMNP	SOLV2 98
	IF (IQUANT) 220,230,230	SOLV2 99
220	ISEGM=ISEGM+1	SOLV2100
	IQUANT=ISEGM*NPSEGM-NUMNP	SOLV2101
	KZ1=1+3*IQUANT	SOLV2102
	KZ2=N	SOLV2103
	KZ3=3*IQUANT	SOLV2104
	KWP=3*ISEGM*NPSEGM	SOLV2105
	GO TO 240	SOLV2106
230	IF (KZ2,GE,NA1) GO TO 280	SOLV2107
	KZ1=1+KOUNT*N	SOLV2108
	KZ2=KZ1+N-1	SOLV2109
	KZ3=0	SOLV2110
240	CONTINUE	SOLV2111
	DO 270 KZ=KZ1,KZ2	SOLV2112
	DO 250 I=1,L	SOLV2113
250	H(I)=ALL(I,N-KZ3)	SOLV2114
	SUM=0.	SOLV2115
	DO 260 J=2,L	SOLV2116
260	SUM=SUM+H(J)*X(KWP+J-KZ)	SOLV2117
	X(KWP-KZ+1)=(Y(KWP-KZ+1)-SUM)*H(1)	SOLV2118
	KZ3=KZ3+1	SOLV2119
270	CONTINUE	SOLV2120
	KOUNT=KOUNT+1	SOLV2121
	GO TO 230	SOLV2122
280	CONTINUE	SOLV2123
C		SOLV2124
C	ORIGINAL SOLV1 ENDS HERE	SOLV2125
C		SOLV2126
	DO 290 I=1,NA1	SOLV2127
290	B(I)=X(I)	SOLV2128
	RETURN	SOLV2129
	END	SOLV2130
	OVERLAY (EGCP,6,0)	OVERL6 2
	PROGRAM OVERL6	OVERL6 3
C		OVERL6 4
C	THIS OVERLAY SETS UP DATA IN CORRECT ORDER FOR	OVERL6 5
C	STIFFNESS MATRIX FORMULATION OF SLAB	OVERL6 6
C		OVERL6 7
	COMMON /ITAPE/ IN,I0,I12,I22,I32	OVERL6 8
	COMMON /CB01/ NUMEL,NANA,NUMNP	OVERL6 9
	COMMON /ICORES/ ICR(5),ICCRE	OVERL610
	COMMON /CB4/ INN,I00	OVERL611
	COMMON /ICOREL/ N7	OVERL612
	COMMON A(1)	OVERL613
	N1=1	OVERL614
	N2=N1+NUMEL*I12	OVERL615
	N3=N2+NUMEL*I22	OVERL616
	N4=N3+NUMEL*I32	OVERL617
	N5=N4+I12	OVERL618
	N6=N5+I22	OVERL619

	N7=N6+I32	OVERL620
C	IF (ICORE.NE.0) GO TO 10	OVERL621
	ICORE=LOCF(A(1))+1	OVERL622
	GO TO 20	OVERL623
	10 CONTINUE	OVERL624
	CALL TWRITE (IN,IO,NUMEL,NUMNP,NANA,I12,I22,I32,A(N1),A(N2),A(N3),	OVERL626
	1A(N4),A(N5),A(N6))	OVERL627
	20 CONTINUE	OVERL628
	END	OVERL629
	SUBROUTINE TWRITE (IN,IO,NUMEL,NUMNP,NANA,I12,I22,I32,SXX,SYY,SXY,	TWRITE 2
	1SXXOT,SYYOT,SXYOT)	TWRITE 3
C		TWRITE 4
C	THIS SUBROUTINE SETS UP DATA IN CORRECT ORDER FOR	TWRITE 5
C	STIFFNESS MATRIX FORMULATION OF SLAB	TWRITE 6
C		TWRITE 7
	DIMENSION SXX(NUMEL,I12), SYY(NUMEL,I22), SXY(NUMEL,I32), SXXOT(I1	TWRITE 8
	12), SYYOT(I22), SXYOT(I32), NZ(4), DUM(1)	TWRITE 9
C		TWRITE10
C	READ ARRAYS	TWRITE11
C		TWRITE12
	REWIND IN	TWRITE13
	REWIND IO	TWRITE14
	DO 40 I=1,NUMEL	TWRITE15
	READ (IN) SXXOT,SYYOT,SXYOT	TWRITE16
	DO 10 J=1,I12	TWRITE17
	SXX(I,J)=SXXOT(J)	TWRITE18
	10 CONTINUE	TWRITE19
	DO 20 J=1,I22	TWRITE20
	SYY(I,J)=SYYOT(J)	TWRITE21
	20 CONTINUE	TWRITE22
	DO 30 J=1,I32	TWRITE23
	SXY(I,J)=SXYOT(J)	TWRITE24
	30 CONTINUE	TWRITE25
	40 CONTINUE	TWRITE26
C		TWRITE27
C	REORDER AND WRITE TO TAPE	TWRITE28
C		TWRITE29
	REWIND 11	TWRITE30
	DO 80 I=1,NUMNP	TWRITE31
	READ (11) NZ(1),NZ(2),NZ(3),NZ(4),DUM,DUM,DUM,DUM,DUM,DUM,DUM,DUM,	TWRITE32
	1DUM,DUM,DUM,DUM	TWRITE33
	DO 80 NR=1,4	TWRITE34
	N=NZ(NR)	TWRITE35
	IF (N.EQ.NANA) GO TO 80	TWRITE36
	DO 50 J=1,I12	TWRITE37
	SXXOT(J)=SXX(N,J)	TWRITE38
	50 CONTINUE	TWRITE39
	DO 60 J=1,I22	TWRITE40
	SYYOT(J)=SYY(N,J)	TWRITE41
	60 CONTINUE	TWRITE42
	DO 70 J=1,I32	TWRITE43
	SXYOT(J)=SXY(N,J)	TWRITE44
	70 CONTINUE	TWRITE45
	WRITE (IO) SXXOT,SYYOT,SXYOT	TWRITE46
	80 CONTINUE	TWRITE47
C		TWRITE48
	RETURN	TWRITE49

END  
OVERLAY (ECCP,7,0)  
PROGRAM OVERL7

TWRITE50  
OVERL7 2  
OVERL7 3

C  
C THIS OVERLAY SETS UP DATA IN CORRECT ORDER FOR  
C STIFFNESS MATRIX FORMULATION OF BEAM

OVERL7 4  
OVERL7 5  
OVERL7 6

C  
COMMON /CB2/ NBEAMX,NIX,NULAYB  
COMMON /ITAPE/ IN,IC

OVERL7 7  
OVERL7 8  
OVERL7 9

COMMON /CB01/ NUMEL,NANA,NUMNP  
COMMON /CB4/ INN,IOO  
COMMON /ICORES/ ICR(6),IGCRE  
COMMON /ICOREL/ N7,N3  
COMMON A(1)

OVERL710  
OVERL711  
OVERL712  
OVERL713  
OVERL714

N1=1  
N2=N1+NULAYB\*NBEAMX  
N3=N2+NULAYB

OVERL715  
OVERL716  
OVERL717  
OVERL718

C  
IF (ICORE.NE.0) GO TO 10  
ICORE=LOC(A(1))+1  
GO TO 20

OVERL719  
OVERL720  
OVERL721

10 CONTINUE  
CALL TWRB (IN,IO,NULAYB,NBEAMX,NIX,NUMNP,A(N1),A(N2))

OVERL722  
OVERL723  
OVERL724

C  
20 CONTINUE

OVERL725  
OVERL726

C  
END

OVERL727

SUBROUTINE TWRB (IN,IO,NULAYB,NBEAMX,NIX,NUMNP,SXX,SIBXXA)

TWRB 2

C  
C THIS SUBROUTINE SETS UP DATA IN CORRECT ORDER FOR  
C STIFFNESS FORMULATION OF BEAM

TWRB 3  
TWRB 4  
TWRB 5

C  
DIMENSION SXX(NULAYB,NBEAMX), SIBXXA(NULAYB), NZ(2)

TWRB 6  
TWRB 7

REWIND IN  
REWIND IO  
REWIND 13

TWRB 8  
TWRB 9  
TWRB 10

DO 10 J=1,NBEAMX  
READ (IN) SIBXXA  
DO 10 I=1,NULAYB

TWRB 11  
TWRB 12  
TWRB 13

SXX(I,J)=SIBXXA(I)  
10 CONTINUE  
DO 30 I=1,NUMNP

TWRB 14  
TWRB 15  
TWRB 16

READ (13) NPARX,NZ(1),NZ(2),NPIX,NPKX  
DO 30 NR=1,2  
IF (NPARX.EQ.0) GO TO 30

TWRB 17  
TWRB 18  
TWRB 19

N=NZ(NR)  
IF (N.EQ.NIX) GO TO 30  
DO 20 J=1,NULAYB

TWRB 20  
TWRB 21  
TWRB 22

SIBXXA(J)=SXX(J,N)  
20 CONTINUE  
WRITE (IO) SIBXXA

TWRB 23  
TWRB 24  
TWRB 25

30 CONTINUE  
RETURN  
END

TWRB 26  
TWRB 27  
TWRB 28

OVERLAY (ECCP,11,0)  
PROGRAM OVERL9

OVERL9 2  
OVERL9 3

C  
C THIS OVERLAY SETS UP LOCATIONS FOR DYNAMIC STORAGE

OVERL9 4  
OVERL9 5

C	REQUIREMENTS	OVERL
C	COMMON /OV1/ AA(16)	9 6
	COMMON /OV2/ BB(21)	9 7
	COMMON /OV3/ CC(10)	9 8
	COMMON /OV4/ DD(26)	9 9
	COMMON /OV5/ EE(52)	910
	COMMON /OV10/ GG(13)	911
	COMMON /OV8/ FF(29)	912
	COMMON /OVER1/ A(17)	913
	COMMON /OVER2/ B(22)	914
	COMMON /OVER3/ C(11)	915
	COMMON /OVER4/ D(27)	916
	COMMON /OVER5/ E(53)	917
	COMMON /OVER10/ G(14)	918
	COMMON /OVER8/ F(30)	919
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	920
	COMMON /CB2/ NBEAMX,NIX,NULAYB	921
	COMMON /CB3/ LWIDTH,NITA,NPSEGM	922
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NCBM,NNN	923
	COMMON /CB310/ NELX,NSMAT,NELY	924
	COMMON /ICORES/ ICR(8),ICCRE	925
	COMMON // BLANK(1)	926
	INTEGER A,B,C,D,E,F	927
	INTEGER AA,BB,CC,DD,EE,FF	928
	INTEGER G,GG	929
C	ICORE=LOC(ELANK(1))+1	930
C	NA=16	931
	NB=21	932
	NC=10	933
	ND=26	934
	NE=52	935
	NF=29	936
	NG=13	937
	NPA=NA+1	938
	NPB=NB+1	939
	NPC=NC+1	940
	NPD=ND+1	941
	NPE=NE+1	942
	NPF=NF+1	943
	NPG=NG+1	944
	NUMEL=NELX*NELY	945
	NITA=0	946
	NCNS=NULAY+NSLAYR	947
	NPERBM=NELX	948
	NANA=NUMEL+1	949
	NUMNP=(NELX+1)*(NELY+1)	950
	NETA=NUMNP+1	951
	NA1=5*NUMNP	952
	NIX=NBEAMX+1	953
	LWIDTH=(NELX+3)*5	954
	LAMA=NPSEGM*5	955
	NIETE=LWIDTH-1	956
	NA5=NA1+LWIDTH-1	957
	NUMBP=NELX+1	958
	LWIDTHB=6	959
		960
		961
		962
		963

	NA1B=3*(NELX+1)	OVERL964
	NA5B=NA1B+5	OVFRL965
	NOBM=NBEAMX/NELX	OVERL966
	NNN=NOBM*(NELX+1)*3	OVERL967
C		OVERL968
C	FIND ARRAY LENGTHS FOR EACH OVERLAY	OVFRL969
C		OVERL970
	CALL VV1 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	OVERL971
	CALL VV2 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	OVERL972
	CALL VV3 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	OVERL973
	CALL VV4 (NA1B,NA5B,LWDTHB)	OVERL974
	CALL VV5 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	OVERL975
	CALL VV8	OVERL976
	CALL VV10	OVERL977
C		OVERL978
C	SET UP ARRAY LOCATIONS	OVERL979
C		OVERL980
	CALL DIM (AA,NA,A,NPA)	OVERL981
	CALL DIM (BB,NB,B,NPB)	OVERL982
	CALL DIM (CC,NC,C,NPC)	OVERL983
	CALL DIM (DD,ND,D,NPD)	OVERL984
	CALL DIM (EE,NE,E,NPE)	OVERL985
	CALL DIM (FF,NF,F,NPF)	OVERL986
	CALL DIM (GG,NG,G,NPG)	OVERL987
	END	OVERL988
	SUBROUTINE DIM (AA,NA,A,NPA)	DIM 2
	INTEGER A,AA	DIM 3
	DIMENSION AA(NA), A(NPA)	DIM 4
	A(1)=1	DIM 5
	DO 10 I=2,NPA	DIM 6
	A(I)=A(I-1)+AA(I-1)	DIM 7
10	CONTINUE	DIM 8
	RETURN	DIM 9
	END	DIM 10
	SUBROUTINE VV5 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	VV5 2
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	VV5 3
	COMMON /CB2/ NBEAMX,NIX,NULAYB	VV5 4
	COMMON /CB310/ NELX,NSMAT,NELY	VV5 5
	COMMON /OV5/ FORCE,LILA,FORCEA,DISPLA,DISP,DISPO,FORCEB,FCADD,FCSAVV5	VV5 6
	1DD,CTXX,CTYY,CTXY,ETXX,ETYY,ETXY,CUXX,CUYY,CUXY,EIXX,EIYY,EIXY,AS1VV5	VV5 7
	2,AF,AS,SIGXX0,SIGYY0,SIGXY0,SXXOT,SYOT,SXYOT,EXTRA,EXTRA1,EXTRA2,VV5	VV5 8
	3EXTRA3,EXTRA4,EXTRA5,EXTRA6,EXTRA7,EIB,CUB,DUM,EPSPS,SIBXXA,SIBXXCVV5	VV5 9
	4,ASXLR,AILR,ZCRD,TSHEAR,ITYPE,ESX,AFT,AST	VV5 10
	INTEGER FORCE,LILA,FORCEA,DISPLA,DISP,DISPO,FORCEB,FCADD,FCSADD,CTVV5	VV5 11
	1XX,CTYY,CTXY,ETXX,ETYY,ETXY,CUXX,CUYY,CUXY,EIXX,EIYY,EIXY,AS1,AF,AVV5	VV5 12
	2S,SIGXX0,SIGYY0,SIGXY0,SXXOT,SYOT,SXYOT,EXTRA,EXTRA1,EXTRA2,EXTRA	VV5 13
	33,EXTRA4,EXTRA5,EXTRA6,EXTRA7,EIB,CUB,DUM,EPSPS,SIBXXA,SIBXXC,ASXLVV5	VV5 14
	4R,A ILR,ZCRD,TSHEAR,ITYPE,ESX,AFT,AST	VV5 15
C		VV5 16
	FORCE=NA1	VV5 17
	LILA=NA1	VV5 18
	FORCEA=NA1	VV5 19
	DISPLA=NA1	VV5 20
	DISP=NA1	VV5 21
	DISPO=NA1	VV5 22
	FORCEB=NA1	VV5 23
	FCADD=NA1	VV5 24
	FCSADD=NA1	VV5 25



CTXX=NUMEL	VV5	26
CTYY=NUMEL	VV5	27
CTXY=NUMEL	VV5	28
ETXX=NUMEL	VV5	29
ETYY=NUMEL	VV5	30
ETXY=NUMEL	VV5	31
CUXX=NUMEL	VV5	32
CUYY=NUMEL	VV5	33
CUXY=NUMEL	VV5	34
EIXX=NUMEL	VV5	35
EIYY=NUMEL	VV5	36
EIXY=NUMEL	VV5	37
AS1=NUMEL*NCNS	VV5	38
AF=NUMEL*NULAY	VV5	39
AS=AF	VV5	40
SIGXX0=NCNS	VV5	41
SIGYY0=NCNS	VV5	42
SIGXY0=NCNS	VV5	43
SXXOT=NCNS	VV5	44
SYYOT=NCNS	VV5	45
SXYOT=NCNS	VV5	46
NE3=3	VV5	47
EXTRA=NE3	VV5	48
EXTRA1=NE3*NULAYB	VV5	49
EXTRA2=EXTRA1	VV5	50
EXTRA4=EXTRA1	VV5	51
EXTRA5=EXTRA1	VV5	52
EXTRA6=EXTRA1	VV5	53
EXTRA3=NULAYB	VV5	54
EXTRA7=NULAYB	VV5	55
EIB=NBEAMX	VV5	56
CUB=NBEAMX	VV5	57
EPSPS=NULAYB	VV5	58
DUM=NULAYB	VV5	59
SIBXXA=NULAYB	VV5	60
SIBXXC=NULAYB	VV5	61
ASXLR=NULAYB	VV5	62
AILR=NULAYB	VV5	63
ZCRD=NULAYB	VV5	64
TSHEAR=NULAYB	VV5	65
ITYPE=NULAYB	VV5	66
ESX=NULAYB*NBEAMX	VV5	67
AFT=NCNS	VV5	68
AST=NCNS	VV5	69
<b>C</b>	VV5	70
RETURN	VV5	71
END	VV5	72
SUBROUTINE VV1 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	VV1	2
COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	VV1	3
COMMON /CB310/ NELX,NSMAT,NELY	VV1	4
COMMON /OV1/ NPI,NPJ,NPK,NPL,ELLA,ELLB,NEQ1,NEQ2,NEQ3,NEQ4,DT,Q,FOVV1	VV1	5
1RCE,LILA,XSEG,YSEG	VV1	6
INTEGER NPI,NPJ,NPK,NPL,ELLA,ELLB,NEQ1,NEQ2,NEQ3,NEQ4,DT,Q,FORCE,LVV1	VV1	7
1ILA,XSEG,YSEG	VV1	8
<b>C</b>	VV1	9
NPI=NANA	VV1	10
NPJ=NANA	VV1	11
NPK=NANA	VV1	12

NPL=NANA	VV1	13
ELLA=NUMEL	VV1	14
ELLB=NUMEL	VV1	15
NEQ1=NUMNP	VV1	16
NEQ2=NUMNP	VV1	17
NEQ3=NUMNP	VV1	18
NEQ4=NUMNP	VV1	19
DT=NUMEL	VV1	20
Q=NUMEL	VV1	21
FORCE=NA1	VV1	22
LILA=NA1	VV1	23
XSEG=NELX	VV1	24
YSEG=NELY	VV1	25
C	VV1	26
RETURN	VV1	27
END	VV1	28
SUBROUTINE VV8	VV8	2
COMMON /C801/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	VV8	3
COMMON /C82/ NBEAMX,NIX,NULAYB,NULAYR,NULAYE,NR,NRE	VV8	4
COMMON /C8310/ NELX,NSMAT,NELY	VV8	5
COMMON /C8300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NCBM,NNN	VV8	6
COMMON /OV8/ NPARX,NXL,NXR,GKEIX,NPIX,NPKX,SIBXXA,PA,PB,XSEG,YSEG,VV8	VV8	7
1B,T,ITR,SIR,D,CGTB,PGTE,BE,TE,SE,ITE,CGTE,ASXLR,AILR,ZCRD,TSHEAR,EVV8	VV8	8
2SX,ITYPE	VV8	9
INTEGER NPARX,NXL,NXR,GKEIX,NPIX,NPKX,SIBXXA,PA,PR,XSEG,YSEG,B,T,IVV8	VV8	10
1TR,SIR,D,CGTB,PGTE,BE,TE,SE,ITE,CGTE,ASXLR,AILR,ZCRD,TSHEAR,ESX,ITVV8	VV8	11
2YPE	VV8	12
C	VV8	13
NPARX=NUMNP	VV8	14
NXL=NUMNP	VV8	15
NXR=NUMNP	VV8	16
NPIX=NIX	VV8	17
NPKX=NIX	VV8	18
ESX=NULAYB*NBEAMX	VV8	19
PA=NRE	VV8	20
PB=NRE	VV8	21
GKEIX=NPERBM	VV8	22
SIBXXA=NULAYB*NPERBM	VV8	23
ASXLR=SIBXXA	VV8	24
AILR=SIBXXA	VV8	25
ZCRD=SIBXXA	VV8	26
TSHEAR=SIBXXA	VV8	27
ITYPE=SIBXXA	VV8	28
B=NRE*NR	VV8	29
T=B	VV8	30
ITR=B	VV8	31
SIR=B	VV8	32
D=NR	VV8	33
CGTB=NR	VV8	34
PGTE=NPERBM	VV8	35
CGTE=NPERBM	VV8	36
TE=NPERBM*NRE	VV8	37
BE=TE	VV8	38
SE=TE	VV8	39
ITE=TE	VV8	40
XSEG=NELX	VV8	41
YSEG=NELY	VV8	42
C	VV8	43

	RETURN	VV8	44
	END	VV8	45
	SUBROUTINE VV2 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	VV2	2
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	VV2	3
	COMMON /CB2/ NBEAMX,NIX,NULAYB	VV2	4
	COMMON /CB3/ LWIDTH,NITA,NPSEGM	VV2	5
	COMMON /OV2/ FORCE,LILA,AS1,AF,AS,SIGXXO,SIGYYO,SIGXYO,SXXOT,SYOT	VV2	6
	1,SXYOT,ALL,OVK,SIBXXA,SIBXXC,ASXLR,AILR,ZCRD,TSHEAR,ITYPE,ESX	VV2	7
	INTEGER FORCE,LILA,AS1,AF,AS,SIGXXO,SIGYYO,SIGXYC,SXXOT,SYOT,SXYO	VV2	8
	1T,ALL,OVK,SIBXXA,SIBXXC,ASXLR,AILR,ZCRD,TSHEAR,ITYPE,ESX	VV2	9
C		VV2	10
	FORCE=NA1	VV2	11
	LILA=NA1	VV2	12
	AS1=NUMEL*NCNS	VV2	13
	AF=NUMEL*NULAY	VV2	14
	AS=AF	VV2	15
	SIGXXO=NCNS	VV2	16
	SIGYYO=NCNS	VV2	17
	SIGXYO=NCNS	VV2	18
	SXXOT=NCNS	VV2	19
	SYOT=NCNS	VV2	20
	SXYOT=NCNS	VV2	21
	ALL=LWIDTH*LAMA	VV2	22
	OVK=5*NA5	VV2	23
	SIBXXA=NULAYB	VV2	24
	SIBXXC=NULAYB	VV2	25
	ASXLR=NULAYB	VV2	26
	AILR=NULAYB	VV2	27
	ZCRD=NULAYB	VV2	28
	TSHEAR=NULAYB	VV2	29
	ITYPE=NULAYB	VV2	30
	ESX=NULAYB*NBEAMX	VV2	31
C		VV2	32
	RETURN	VV2	33
	END	VV2	34
	SUBROUTINE VV3 (LAMA,NA5,NIETE,NUMBP,NA1B,NA5B,LWDTHB)	VV3	2
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	VV3	3
	COMMON /CB3/ LWIDTH,NITA,NPSEGM	VV3	4
	COMMON /OV3/ C,X,Y,ALL,AL,H,V,FORCE,LILA,DISP	VV3	5
	INTEGER C,X,Y,ALL,AL,H,V,FORCE,LILA,DISP	VV3	6
C		VV3	7
	C=NA1	VV3	8
	FORCE=NA1	VV3	9
	DISP=NA1	VV3	10
	LILA=NA1	VV3	11
	X=NA5	VV3	12
	Y=NA5	VV3	13
	H=LWIDTH	VV3	14
	V=LWIDTH	VV3	15
	AL=LWIDTH*NIETE	VV3	16
	ALL=LWIDTH*LAMA	VV3	17
C		VV3	18
	RETURN	VV3	19
	END	VV3	20
	SUBROUTINE VV4 (NA1B,NA5B,LWDTHB)	VV4	2
	COMMON /CB2/ NBEAMX,NIX,NULAYB	VV4	3
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOSM,NNN	VV4	4
	COMMON /OV4/ SIG,DSIG,ESX,ESXC,ZCRDC,ASXLR,EPSPSC,AILRC,ITYPC,SIPV	VV4	5

	1XXA,ZCRD,ASXLR,EPSPS,AILR,ITYPE,TSHEAR,XLENC,LILB,FORCE4,TEMP,B,AKVV4	6
	2,XB,YB,CB,ALLB	7
	INTEGER SIG,DSIG,ESX,ESXC,ZCRDC,ASXLRC,EPSPSC,AILRC,ITYPC,SIBXXA,ZVV4	8
C	1CRD,ASXLR,EPSPS,AILR,ITYPE,TSHEAR,XLENC,LILB,FORCE4,TEMP,B,AK,XB,YVV4	9
	2B,CB,ALLB	10
	SIG=NULAYB*PPERBM	11
	DSIG=SIG	12
	ESX=NULAYB*NBEAMX	13
	ESXC=SIG	14
	ZCRDC=SIG	15
	ASXLRC=SIG	16
	EPSPSC=SIG	17
	AILRC=SIG	18
	ITYPC=SIG	19
	SIBXXA=NULAYB	20
	ASXLR=NULAYB	21
	AILR=NULAYB	22
	ZCRD=NULAYB	23
	TSHEAR=NULAYB	24
	ITYPE=NULAYB	25
	EPSPS=NULAYB	26
	XLENC=PPERBM	27
	LILB=NA1B	28
	FORCE4=NA1B	29
	TEMP=NA1B	30
	B=NA1B	31
	AK=NA1B*NA1B	32
	CB=NA1B	33
	XB=NA5B	34
	YB=NA5B	35
	ALLB=LWOTHB*NA1B	36
C		37
	RETURN	38
	END	39
	SUBROUTINE VV10	40
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	VV10 2
	COMMON /CB2/ NBEAMX,NIX,NULAYB	VV10 3
	COMMON /CB310/ NELX,NSMAT,NELY	VV10 4
	COMMON /OV10/ ESX,SIBXXA,ITYPE,DUM,EXTRA,EXTRA1,EXTRA2,EXTRA3,AS1,VV10	VV10 5
	1AFT,AST,XSEG,YSEG	VV10 6
	INTEGER ESX,SIBXXA,ITYPE,DUM,EXTRA,EXTRA1,EXTRA2,EXTRA3,AS1,AFT,ASVV10	VV10 7
	1T,XSEG,YSEG	VV10 8
C		VV10 9
	ESX=NULAYB*NBEAMX	VV10 10
	SIBXXA=NULAYB	VV10 11
	ITYPE=NULAYB	VV10 12
	DUM=NULAYB	VV10 13
	EXTRA=NELX	VV10 14
	EXTRA1=NULAYB*NELX	VV10 15
	EXTRA2=EXTRA1	VV10 16
	EXTRA3=EXTRA1	VV10 17
	AS1=NUMEL*NCNS	VV10 18
	AST=NCNS	VV10 19
	AFT=NCNS	VV10 20
	XSEG=NELX	VV10 21
	YSEG=NELY	VV10 22
	RETURN	VV10 23
		VV10 24

	END	VV10 25
	OVERLAY (ECCP,12,0)	OVRL10 2
	PROGRAM OVRL10	OVRL10 3
C		OVRL10 4
C	THIS OVERLAY DOES THE PRINTER PLOTS FOR THE BEAMS AND SLAB	OVRL10 5
C		OVRL10 6
	COMMON /CB01/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	OVRL10 7
	COMMON /CB2/ NBEAMX,NIX,NULAYB	OVRL10 8
	COMMON /CB4/ IN,IO	OVRL10 9
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	OVRL1010
	COMMON /CB310/ NELX,NSMAT,NELY	OVRL1011
	COMMON /CB311/ PRINTB,PRINTS,DGX,DGY	OVRL1012
	COMMON /ICORES/ ICR(9),ICORE	OVRL1013
	COMMON /BDIM/ RON(48)	OVRL1014
	COMMON /PBEAM/ NPBEAM(20)	OVRL1015
	COMMON /OVER10/ N1,N2,N3,N4,N5,N6,N7,N8,N9,N10,N11,N12,N13,N14	OVRL1016
	COMMON A(1)	OVRL1017
C		OVRL1018
	IF (ICORE.NE.0) GO TO 10	OVRL1019
	ICORE=LOC(A(1))+1	OVRL1020
	GO TO 30	OVRL1021
	10 CONTINUE	OVRL1022
C		OVRL1023
C	CHECK FOR BEAM PLOT	OVRL1024
C		OVRL1025
	IF (PRINTB.NE.3HYES) GO TO 20	OVRL1026
	IF (NBEAMX.NE.0) CALL PLOTB (NULAYB,NBEAMX,NPERBM,A(N1),A(N2),A(N3),	OVRL1027
	1),A(N4),A(N5),A(N6),A(N7),A(N8))	OVRL1028
	20 CONTINUE	OVRL1029
C		OVRL1030
C	CHECK FOR SLAB PLOT	OVRL1031
C		OVRL1032
	IF (PRINTS.NE.3HYES) GO TO 30	OVRL1033
	CALL PLOTS (NUMEL,NCNS,NELX,NELY,A(N9),A(N10),A(N11),A(N12),A(N13),	OVRL1034
	1)	OVRL1035
	30 CONTINUE	OVRL1036
	END	OVRL1037
	SUBROUTINE PLOTB (G,H,F,ESX,SIBXXA,ITYPE,DUM,EXTRA,EXTRA1,EXTRA2,EXTRA3)	PLOTB 2
	1XTRA3)	PLOTB 3
C		PLOTB 4
C	THIS SUBROUTINE CALLS THE BEAM PLOT SUBROUTINE	PLOTB 5
C		PLOTB 6
	INTEGER G,H,F	PLOTB 7
	COMMON /CB2/ NBEAMX,NIX,NULAYB	PLOTB 8
	COMMON /CB4/ IN,IO	PLOTB 9
	COMMON /CB300/ NPERBM,NREADB,KSIG,KIN,KC,KMAT,KPS,NOBM,NNN	PLOTB 10
	COMMON /BDIM/ RON(6),ROM(6),EDOWN(6),STRAN(6),SIGO(6),FT(6),RONT(3),	PLOTB 11
	1),ROMT(3),EDOWNT(3),EDOWNR(3)	PLOTB 12
	DIMENSION ESX(G,H), SIBXXA(G), ITYPE(G), DUM(G), EXTRA(F), EXTRA1(PLOTB	PLOTB 13
	1G,F), EXTRA2(G,F), EXTRA3(G,F)	PLOTB 14
C		PLOTB 15
	REWIND 33	PLOTB 16
	READ (33) ESX	PLOTB 17
	CALL PLTT (NULAYB,NPERBM,ESX,IO,SIBXXA,SIGO,FT,ITYPE,NOBM,DUM,EXTR	PLOTB 18
	1A,EXTRA1,EXTRA2,EXTRA3,NBEAMX,KMAT)	PLOTB 19
C		PLOTB 20
	RETURN	PLOTB 21
	END	PLOTB 22

	SUBROUTINE PLTT (NLAY,NBMS,ESX,IO,SIBXXA,SIGO,FT,ITYPE,NOBM,DUM,XL	PLTT	2
	1EN,ITYP,ESX0,SIG,NBEAMX,KMAT)	PLTT	3
C		PLTT	4
C	THIS SUBROUTINE DOES THE BEAM PRINTER PLOTS	PLTT	5
C		PLTT	6
	DIMENSION IDIST(25), PLAT(150), ESX(NLAY,NBEAMX), SIGO(KMAT), FT(KPLTT	PLTT	7
	1MAT), SIBXXA(NLAY), ITYPE(NLAY), DUM(NLAY), CHARN(10), CHARN(10),	PLTT	8
	2XLEN(NBMS), ESX0(NLAY,NBMS), SIG(NLAY,NBMS), ITP(NLAY,NBMS), DUMPLTT	PLTT	9
	3Y(1)	PLTT	10
	DATA (CHARN(I),I=1,10)/1H1,1H2,1H3,1H4,1H5,1H6,1H7,1H8,1H9,1H0/	PLTT	11
	DATA (CHARP(I),I=1,10)/1HA,1HB,1HC,1HD,1HE,1HF,1HG,1HH,1HI,1HJ/	PLTT	12
	DATA BLANK,X,STAR,SLASH/1H ,1HX,1H*,1H//	PLTT	13
	WRITE (IO,10)	PLTT	14
	10 FORMAT (///,1H ,*PLOT SHOWING CRACK-CRUSH AREAS, STRESSES, NODE A	PLTT	15
	1ND LAYER MARKERS FOR REFERENCE*)	PLTT	16
	WRITE (IO,20)	PLTT	17
	20 FORMAT (1H ,*(SEE USERS MANUAL FOR DETAILS)*)	PLTT	18
C		PLTT	19
	REWIND 3	PLTT	20
	REWIND 27	PLTT	21
C		PLTT	22
	DO 220 JOE=1,NOBM	PLTT	23
	NPERBM=NBMS	PLTT	24
	NSTART=(JOE-1)*NBMS	PLTT	25
	DO 30 J=1,NPERBM	PLTT	26
C		PLTT	27
	READ (3) SIBXXA	PLTT	28
	READ (27) XLNGTH,GKEI,NPIX,NPKX,ITYPE,DUM,DUM,DUM,DUM	PLTT	29
C		PLTT	30
	N=NSTART+J	PLTT	31
	XLEN(J)=XLNGTH	PLTT	32
C		PLTT	33
C	INITIALIZE FOR PLOTTING	PLTT	34
C		PLTT	35
	DO 30 I=1,NLAY	PLTT	36
	SIG(I,J)=SIBXXA(I)	PLTT	37
	ITYP(I,J)=ITYPE(I)	PLTT	38
	30 ESX0(I,J)=ESX(I,N)	PLTT	39
	WRITE (IO,40) JOE	PLTT	40
	40 FORMAT (///,1H ,*BEAM NUMBER *,I2,/) )	PLTT	41
C		PLTT	42
C	GET BEAM LENGTH	PLTT	43
C		PLTT	44
	DIST=0.	PLTT	45
	DO 50 I=1,NBMS	PLTT	46
	50 DIST=DIST+XLEN(I)	PLTT	47
C		PLTT	48
	IDIST(1)=1	PLTT	49
	SCALE=100./DIST	PLTT	50
	DO 60 I=1,NBMS	PLTT	51
	II=I+1	PLTT	52
	60 IDIST(II)=XLEN(I)*SCALE+.4+IDIST(I)	PLTT	53
	LAST=IDIST(NBMS+1)+4	PLTT	54
C		PLTT	55
C	INITIALIZE TO BLANK	PLTT	56
C		PLTT	57
	DO 70 I=1,LAST	PLTT	58
	70 PLAT(I)=BLANK	PLTT	59

C		PLTT	60
C	GET ELEMENT STAR PRINTOUT	PLTT	61
C		PLTT	62
	PLAT(3)=STAR	PLTT	63
	DO 80 I=1,NBMS	PLTT	64
	K1=IDIST(I+1)+2	PLTT	65
	PLAT(K1-1)=STAR	PLTT	66
80	PLAT(K1)=STAR	PLTT	67
	PLAT(LAST-3)=BLANK	PLTT	68
	WRITE (IO,190) (PLAT(K),K=1, LAST)	PLTT	69
	WRITE (IO,90)	PLTT	70
90	FORMAT (/)	PLTT	71
C		PLTT	72
C	DO BEAM PLOT	PLTT	73
C		PLTT	74
	DO 180 I=1,NLAY	PLTT	75
	DO 170 J=1,NBMS	PLTT	76
	K1=IDIST(J)+2	PLTT	77
	K2=IDIST(J+1)+2	PLTT	78
	PLAT(1)=STAR	PLTT	79
	PLAT(2)=BLANK	PLTT	80
	IF (ESX0(I,J).NE.0.) GO TO 130	PLTT	81
	IF (SIG(I,J).LT.0.) GO TO 110	PLTT	82
C		PLTT	83
	DO 100 K=K1,K2	PLTT	84
100	PLAT(K)=BLANK	PLTT	85
C		PLTT	86
	GO TO 170	PLTT	87
C		PLTT	88
110	DO 120 K=K1,K2	PLTT	89
120	PLAT(K)=SLASH	PLTT	90
	GO TO 170	PLTT	91
C		PLTT	92
130	CONTINUE	PLTT	93
	IF (SIG(I,J).LE.0) GO TO 150	PLTT	94
	K=ITYP(I,J)	PLTT	95
	IRATIO=10.*ABS(SIG(I,J))/ABS(FT(K))+1	PLTT	96
	IF (IRATIO.GT.10) IRATIO=10	PLTT	97
	DO 140 K=K1,K2	PLTT	98
140	PLAT(K)=CHARP(IRATIO)	PLTT	99
	GO TO 170	PLTT	100
150	K=ITYP(I,J)	PLTT	101
	IRATIO=10.*ABS(SIG(I,J))/ABS(SIG0(K))+1	PLTT	102
	IF (IRATIO.GT.10) IRATIO=10	PLTT	103
	DO 160 K=K1,K2	PLTT	104
160	PLAT(K)=CHARN(IRATIO)	PLTT	105
C		PLTT	106
170	CONTINUE	PLTT	107
C		PLTT	108
	PLAT(K2+1)=BLANK	PLTT	109
	PLAT(K2+2)=STAR	PLTT	110
180	WRITE (IO,190) (PLAT(K),K=1, LAST)	PLTT	111
190	FORMAT (1H ,135A1)	PLTT	112
C		PLTT	113
C	GET ELEMENT STAR PRINTOUT	PLTT	114
C		PLTT	115
	WRITE (IO,90)	PLTT	116
	DO 200 I=1, LAST	PLTT	117

200	PLAT(I)=BLANK	PLTT 118
	PLAT(3)=STAR	PLTT 119
	DO 210 I=1,NBMS	PLTT 120
	K1=IDIST(I+1)+2	PLTT 121
	PLAT(K1-1)=STAR	PLTT 122
210	PLAT(K1)=STAR	PLTT 123
	PLAT(LAST-3)=BLANK	PLTT 124
	WRITE (IO,190) (PLAT(K),K=1,LAST)	PLTT 125
220	CONTINUE	PLTT 126
	RETURN	PLTT 127
	END	PLTT 128
	SUBROUTINE PLOTS (C,D,F,P,AS1,AFT,AST,X,Y)	PLOTS 2
C		PLOTS 3
C	THIS SUBROUTINE CALLS FOR THE SLAB PLOT	PLOTS 4
C		PLOTS 5
	INTEGER C,D,F,P	PLOTS 6
	COMMON /CB301/ NUMEL,NANA,NUMNP,NETA,NA1,NULAY,NSLAYR,NCNS	PLOTS 7
	COMMON /CB4/ IN,IO	PLOTS 8
	COMMON /CB310/ NELX,NSMAT,NELY	PLOTS 9
	COMMON /CB311/ PRINTB,PRINTS,DGX,DGY	PLOTS 10
	DIMENSION AS1(C,D), AFT(D), AST(D), X(F), Y(P)	PLOTS 11
	CALL PLTS (NELX,NELY,NULAY,NCNS,NSLAYR,NUMEL,AS1,AFT,AST,X,Y,DGX,D	PLOTS 12
	1GY,IO)	PLOTS 13
	RETURN	PLOTS 14
	END	PLOTS 15
	SUBROUTINE PLTS (NELX,NELY,NULAY,NCNS,NSLAYR,NUMEL,AS1,AFT,AST,X,Y	PLTS 2
	1,DGX,DGY,IO)	PLTS 3
C		PLTS 4
C	THIS SUBROUTINE DOES THE SLAB PLOT	PLTS 5
C		PLTS 6
	INTEGER VARF	PLTS 7
	DIMENSION X(NELX), Y(NELY), AS1(NUMEL,NCNS), AFT(NCNS), AST(NCNS),	PLTS 8
	1 NUMB(20), IFM(13E), MM(4), VARF(5)	PLTS 9
	COMMON /PBEAM/ NPBEAM(20)	PLTS 10
	DATA (NUMB(I),I=1,20)/1H1,1H2,1H3,1H4,1H5,1H6,1H7,1H8,1H9,1H0,1H*,	PLTS 11
	11H ,1HH,1HC,1H=,1HY,1HT,1HC,5H(1H ,5H(A1))/	PLTS 12
C		PLTS 13
C	READ TAPES	PLTS 14
C		PLTS 15
	REWIND 28	PLTS 16
	REWIND 31	PLTS 17
	READ (31) AS1	PLTS 18
	DO 10 I=1,NUMEL	PLTS 19
	READ (28) XL,XL,XL,XL,XL,XL,XL	PLTS 20
10	CONTINUE	PLTS 21
	READ (28) X,Y	PLTS 22
C		PLTS 23
C		PLTS 24
C	SCALE FACTOR	PLTS 25
C		PLTS 26
	XL=0.0	PLTS 27
	DO 20 I=1,NELX	PLTS 28
	XL=XL+X(I)	PLTS 29
20	CONTINUE	PLTS 30
	YL=0.0	PLTS 31
	DO 30 I=1,NELY	PLTS 32
	YL=YL+Y(I)	PLTS 33
30	CONTINUE	PLTS 34



SX=10.0\*DGX/XL  
SY=6.0\*DGY/YL

PLTS 35  
PLTS 36  
PLTS 37

C

WRITE (10,40)  
40 FORMAT (//,1H0,5X,\*SLAB PLOT\*,//,11X,\*NODE J\*,63X,\*NODE L\*,//,18X,PLTS  
1\*ELEMENT NUMBER\*,19X,\*Y = NUMBER OF YIELDED LAYERS\*,//,18X,\*C = NUPLTS  
2MBER OF CRUSHED LAYERS T = NUMBER OF CRACKED LAYERS\*,//,11X,\*NPLTS  
30DE I\*,63X,\*NODE K\*,//)

PLTS 38  
PLTS 39  
PLTS 40  
PLTS 41  
PLTS 42

C

ILAST=NELX  
IFIRST=1  
50 CONTINUE  
WRITE (10,60)  
60 FORMAT (///1X)

PLTS 43  
PLTS 44  
PLTS 45  
PLTS 46  
PLTS 47  
PLTS 48

C

PLOT ANOTHER PORTION OF THE BRIDGE SLAB

PLTS 50

C

PLTS 51

C

PLTS 52

C

FIND REQUIRED SPACES FOR PLOTTING

PLTS 53

C

NX=1

PLTS 54

PLTS 55

DO 70 I=IFIRST,NELX

PLTS 56

N1=X(I)\*SX

PLTS 57

IF (N1.LT.13) N1=13

PLTS 58

IF (N1.GT.130) N1=130

PLTS 59

IF ((N1+NX-1).GT.130) GO TO 80

PLTS 60

ILAST=I

PLTS 61

NX=N1+NX-1

PLTS 62

70 CONTINUE

PLTS 63

80 CONTINUE

PLTS 64

C

PLTS 65

C

SET TAPE 4

PLTS 66

C

PLTS 67

REWIND 4

PLTS 68

DO 110 J=1,NELY

PLTS 69

REWIND 32

PLTS 70

JEL=NELX\*(NELY-J)

PLTS 71

C

PLTS 72

IF (JEL.EQ.0) GO TO 100

PLTS 73

DO 90 I=1,JEL

PLTS 74

READ (32) AFT,AST

PLTS 75

90 CONTINUE

PLTS 76

100 CONTINUE

PLTS 77

C

PLTS 78

DO 110 I=1,NELX

PLTS 79

READ (32) AFT,AST

PLTS 80

IF (I.LT.IFIRST.OR.I.GT.ILAST) GO TO 110

PLTS 81

WRITE (4) AFT,AST

PLTS 82

110 CONTINUE

PLTS 83

REWIND 4

PLTS 84

C

PLTS 85

SET HOLLERITH CODE FOR PLOTTING POSITIONS

PLTS 86

C

PLTS 87

CALL PLTC (3,NX,MM)

PLTS 88

NN=0

PLTS 89

NN=NN+1

PLTS 90

VARF(NN)=NUMB(19)

PLTS 91

NN=NN+1

PLTS 92

	VARF(NN)=NUMB(MM(1))	PLTS 93
	NN=NN+1	PLTS 94
	VARF(NN)=NUMB(MM(2))	PLTS 95
	NN=NN+1	PLTS 96
	VARF(NN)=NUMB(MM(3))	PLTS 97
	NN=NN+1	PLTS 98
	VARF(NN)=NUMB(20)	PLTS 99
C		PLTS 100
C	FIRST STAR LINE FOR THIS ROW	PLTS 101
C		PLTS 102
C	DO WE HAVE A BEAM AT THIS NODE POINT	PLTS 103
C		PLTS 104
	J=0	PLTS 105
	IB=3HNO	PLTS 106
	NPI=1+(NELY-J)*(NELX+1)	PLTS 107
	DO 120 II=1,20	PLTS 108
	IF (NPBEAM(II).EQ.0) GO TO 130	PLTS 109
	IF (NPI.EQ.NPBEAM(II)) IB=3HYES	PLTS 110
	120 CONTINUE	PLTS 111
	130 CONTINUE	PLTS 112
C		PLTS 113
	NN=0	PLTS 114
	DO 140 II=1,NX	PLTS 115
	NN=NN+1	PLTS 116
	IFM(NN)=NUMB(11)	PLTS 117
	IF (IB.EQ.3HYES) IFM(NN)=1HB	PLTS 118
	140 CONTINUE	PLTS 119
C		PLTS 120
C	NUMBER NODE POINTS	PLTS 121
C		PLTS 122
	J=0	PLTS 123
	II=ILAST+1	PLTS 124
	NN=0	PLTS 125
	DO 160 I=IFIRST,II	PLTS 126
	NPI=I+(NELY-J)*(NELX+1)	PLTS 127
	M=1	PLTS 128
	IF (NPI.GT.9) M=2	PLTS 129
	IF (NPI.GT.99) M=3	PLTS 130
	IF (NPI.GT.999) M=4	PLTS 131
	CALL PLTC (M,NPI,MM)	PLTS 132
	IF ((NN+M).GT.NX) NN=NX-M	PLTS 133
	DO 150 JJ=1,M	PLTS 134
	NN=NN+1	PLTS 135
	IFM(NN)=NUMB(MM(JJ))	PLTS 136
	150 CONTINUE	PLTS 137
	IF (I.GT.ILAST) GO TO 160	PLTS 138
	NP3=X(I)*SX	PLTS 139
	IF (NP3.GT.130) NP3=130	PLTS 140
	IF (NP3.LT.13) NP3=13	PLTS 141
	NN=NN+NP3-M-1	PLTS 142
	160 CONTINUE	PLTS 143
C		PLTS 144
	WRITE (IO,VARF) (IFM(II),II=1,NX)	PLTS 145
	DO 410 J=1,NELY	PLTS 146
	NY=SY*Y(NELY+1-J)	PLTS 147
	IF (NY.LT.5) NY=5	PLTS 148
C		PLTS 149
C	FOR THIS ROW FIND FIRST LINE ,SECOND LINE, LAST LINE	PLTS 150

C	NY1=NY/3.0	PLTS 151
	NY2=2*NY/3+1	PLTS 152
	NY3=NY-1	PLTS 153
	DO 350 INY=1,2	PLTS 154
	NYP=NY1-1	PLTS 155
	IF (INY.EQ.2) NYP=NY2-NY1-1	PLTS 156
C		PLTS 157
C	FIRST SET OF NYP BLANK LINES	PLTS 158
C		PLTS 159
	IF (NYP.GT.0) CALL PLTD (NYP,NELX,SX,IFIRST,ILAST,X,IFM,NUM3,IO,VA	PLTS 160
	1RF,NX)	PLTS 161
	NN=0	PLTS 162
	NN=NN+1	PLTS 163
	IFM(NN)=NUMB(11)	PLTS 164
	DO 340 I=IFIRST,ILAST	PLTS 165
C		PLTS 166
C	FIND X POSITIONS WITHIN EACH ELEMENT	PLTS 167
C		PLTS 168
	NP1=X(I)/3.0*SX-1.0	PLTS 169
	NP2=2.0*X(I)*SX/3.0	PLTS 170
	NP3=SX*X(I)	PLTS 171
	IF (NP3.GE.13) GO TO 170	PLTS 172
	NP1=3	PLTS 173
	NP2=8	PLTS 174
	NP3=13	PLTS 175
	170 CONTINUE	PLTS 176
	IF (NP3.LE.133) GO TO 180	PLTS 177
	NP1=42	PLTS 178
	NP2=86	PLTS 179
	NP3=130	PLTS 180
	180 CONTINUE	PLTS 181
C		PLTS 182
C	FIND NUMBER OF BLANKS	PLTS 183
C		PLTS 184
	NB1=NP1-3	PLTS 185
	NB2=NP2-NP1-4	PLTS 186
	NB3=NP3-NP2-3	PLTS 187
	IF (NB1.LE.0) GO TO 200	PLTS 188
	DO 190 II=1,NB1	PLTS 189
	NN=NN+1	PLTS 190
	IFM(NN)=NUMB(12)	PLTS 191
	190 CONTINUE	PLTS 192
C		PLTS 193
C	GET ELEMENT NUMBER	PLTS 194
C		PLTS 195
	200 CONTINUE	PLTS 196
	M=NELX*(NELY-J)+I	PLTS 197
	IF (INY.NE.1) GO TO 210	PLTS 198
	CALL PLTC (4,M,MM)	PLTS 199
	NF=4	PLTS 200
	GO TO 230	PLTS 201
C		PLTS 202
C	GET CRACKED/CRUSHED LAYERS	PLTS 203
C		PLTS 204
	210 CONTINUE	PLTS 205
	IC=0	PLTS 206
	IT=0	PLTS 207
		PLTS 208

	READ (4) AFT,AST	PLTS 209
	DO 220 II=1,NULAY	PLTS 210
	IF (AFT(II).EQ.5HCRUSH.OR.AST(II).EQ.5HCRUSH) IC=IC+1	PLTS 211
	IF (AFT(II).EQ.5HCRACK.OR.AST(II).EQ.5HCRACK) IT=IT+1	PLTS 212
220	CONTINUE	PLTS 213
C		PLTS 214
C	SET CRUSH SYMBOL	PLTS 215
C		PLTS 216
	CALL PLTC (2,IC,MM)	PLTS 217
	NN=NN+1	PLTS 218
	IFM(NN)=NUMB(14)	PLTS 219
	IF (IC.EQ.0) IFM(NN)=NUMB(12)	PLTS 220
	NN=NN+1	PLTS 221
	IFM(NN)=NUMB(15)	PLTS 222
	IF (IC.EQ.0) IFM(NN)=NUMB(12)	PLTS 223
C		PLTS 224
C	SET NUMBER CODES	PLTS 225
C		PLTS 226
	NF=2	PLTS 227
230	CONTINUE	PLTS 228
C		PLTS 229
	DO 240 II=1,NF	PLTS 230
	NN=NN+1	PLTS 231
	IFM(NN)=NUMB(MM(II))	PLTS 232
240	CONTINUE	PLTS 233
C		PLTS 234
	IF (NB2.LE.0) GO TO 260	PLTS 235
	DO 250 II=1,NB2	PLTS 236
	NN=NN+1	PLTS 237
	IFM(NN)=NUMB(12)	PLTS 238
250	CONTINUE	PLTS 239
260	CONTINUE	PLTS 240
C		PLTS 241
	IF (INY.NE.1) GO TO 290	PLTS 242
C		PLTS 243
C	GET YIELDED LAYERS	PLTS 244
C		PLTS 245
	IY=0	PLTS 246
	IF (NSLAYR.EQ.0) GO TO 280	PLTS 247
	NF=NULAY+1	PLTS 248
	DO 270 II=NF,NCNS	PLTS 249
	IF (AS1(M,II).NE.999.0) IY=IY+1	PLTS 250
270	CONTINUE	PLTS 251
280	CONTINUE	PLTS 252
C		PLTS 253
C	SET UP SYMBOLS FOR YIELDED LAYERS	PLTS 254
C		PLTS 255
	NN=NN+1	PLTS 256
	IFM(NN)=NUMB(16)	PLTS 257
	IF (IY.EQ.0) IFM(NN)=NUMB(12)	PLTS 258
	CALL PLTC (2,IY,MM)	PLTS 259
	GO TO 300	PLTS 260
C		PLTS 261
C	SET UP SYMBOLS FOR CRACKED LAYERS	PLTS 262
C		PLTS 263
290	CONTINUE	PLTS 264
	NN=NN+1	PLTS 265
	IFM(NN)=NUMB(17)	PLTS 266

	IF (IT.EQ.0) IFM(NN)=NUMB(12)	PLTS 267
	CALL PLTC (2,IT,MM)	PLTS 268
C		PLTS 269
C	SET NUMBER CODES	PLTS 270
C		PLTS 271
	300 CONTINUE	PLTS 272
	NN=NN+1	PLTS 273
	IFM(NN)=NUMB(15)	PLTS 274
	IF (IFM(NN-1).EQ.NUMB(12)) IFM(NN)=NUMB(12)	PLTS 275
	DO 310 II=1,2	PLTS 276
	NN=NN+1	PLTS 277
	IFM(NN)=NUMB(MM(II))	PLTS 278
	310 CONTINUE	PLTS 279
C		PLTS 280
	IF (NB3.LE.0) GO TO 330	PLTS 281
	DO 320 II=1,NB3	PLTS 282
	NN=NN+1	PLTS 283
	IFM(NN)=NUMB(12)	PLTS 284
	320 CONTINUE	PLTS 285
	330 CONTINUE	PLTS 286
C		PLTS 287
	NN=NN+1	PLTS 288
	IFM(NN)=NUMB(11)	PLTS 289
C		PLTS 290
C	GO TO ANOTHER NELX	PLTS 291
C		PLTS 292
	340 CONTINUE	PLTS 293
C		PLTS 294
	WRITE (IO,VARF) (IFM(II),II=1,NX)	PLTS 295
C		PLTS 296
C	DO IT TWICE	PLTS 297
C		PLTS 298
	350 CONTINUE	PLTS 299
C		PLTS 300
C	LAST SET OF NYP BLANK LINES	PLTS 301
C		PLTS 302
	NYP=NY3-NY2-1	PLTS 303
	IF (NYP.GT.0) CALL PLTD (NYP,NELX,SX,IFIRST,ILAST,X,IFM,NUMB,IO,VARF,NX)	PLTS 304
		PLTS 305
C		PLTS 306
C	LAST STAR LINE	PLTS 307
C		PLTS 308
C	DO WE HAVE A BEAM AT THIS NODE POINT	PLTS 309
C		PLTS 310
	IB=3HNO	PLTS 311
	NPI=1+(NELY-J)*(NELX+1)	PLTS 312
	DO 360 II=1,20	PLTS 313
	IF (NPBEAM(II).EQ.0) GO TO 370	PLTS 314
	IF (NPI.EQ.NPBEAM(II)) IB=3HYES	PLTS 315
	360 CONTINUE	PLTS 316
	370 CONTINUE	PLTS 317
C		PLTS 318
	NN=0	PLTS 319
	DO 380 II=1,NX	PLTS 320
	NN=NN+1	PLTS 321
	IFM(NN)=NUMB(11)	PLTS 322
	IF (IB.EQ.3HYES) IFM(NN)=1HB	PLTS 323
	380 CONTINUE	PLTS 324

C		PLTS 325
G	NUMBER NODE PCINTS	PLTS 326
C		PLTS 327
	II=ILAST+1	PLTS 328
	NN=0	PLTS 329
	DO 400 I=IFIRST, II	PLTS 330
	NPI=I+(NELY-J)*(NELX+1)	PLTS 331
	M=1	PLTS 332
	IF (NPI.GT.9) M=2	PLTS 333
	IF (NPI.GT.99) M=3	PLTS 334
	IF (NPI.GT.999) M=4	PLTS 335
	CALL PLTC (M,NPI,MM)	PLTS 336
	IF ((NN+M).GT.NX) NN=NX-M	PLTS 337
	DO 390 JJ=1,M	PLTS 338
	NN=NN+1	PLTS 339
	IFM(NN)=NUMB(MM(JJ))	PLTS 340
390	CONTINUE	PLTS 341
	IF (I.GT.ILAST) GO TO 400	PLTS 342
	NP3=X(I)*SX	PLTS 343
	IF (NP3.GT.130) NP3=130	PLTS 344
	IF (NP3.LT.13) NP3=13	PLTS 345
	NN=NN+NP3-M-1	PLTS 346
400	CONTINUE	PLTS 347
C		PLTS 348
	WRITE (IO,VARF) (IFM(II),II=1,NX)	PLTS 349
C		PLTS 350
C	GO TO ANOTHER NELY LINE	PLTS 351
C		PLTS 352
410	CONTINUE	PLTS 353
	IFIRST=ILAST+1	PLTS 354
	ILAST=NELX	PLTS 355
	IF (IFIRST.LE.NELX) GO TO 50	PLTS 356
	RETURN	PLTS 357
	END	PLTS 358
	SUBROUTINE PLTC (NF,M,MM)	PLTC 2
C		PLTC 3
C	FIND HOLLERITH CODE FOR NUMBER #M#	PLTC 4
C		PLTC 5
	DIMENSION MM(4)	PLTC 6
	MS=10**(NF-1)	PLTC 7
	MM(1)=M/MS	PLTC 8
	MSAVE=MM(1)*MS	PLTC 9
	IF (NF.EQ.1) GO TO 20	PLTC 10
	DO 10 I=2,NF	PLTC 11
	MS=MS/10	PLTC 12
	MM(I)=(M-MSAVE)/MS	PLTC 13
	MSAVE=MSAVE+MM(I)*MS	PLTC 14
10	CONTINUE	PLTC 15
20	CONTINUE	PLTC 16
C		PLTC 17
C	SET ZEROS	PLTC 18
C		PLTC 19
	DO 30 I=1,NF	PLTC 20
	IF (MM(I).EQ.0) MM(I)=10	PLTC 21
30	CONTINUE	PLTC 22
C		PLTC 23
C	SET LEADING ZEROS BLANK	PLTC 24
C		PLTC 25

	DO 40 I=1,NF	PLTC	26
	IF (MM(I).NE.10) GO TO 50	PLTC	27
	MM(I)=12	PLTC	28
	40 CONTINUE	PLTC	29
	50 CONTINUE	PLTC	30
	RETURN	PLTC	31
	END	PLTC	32
	SUBROUTINE PLTD (NYP,NELX,SX,IFIRST,ILAST,X,IFM,NUMB,IO,VARF,NA)	PLTD	2
C		PLTD	3
C	PRINT SPACER LINES	PLTD	4
C		PLTD	5
	INTEGER VARF	PLTD	6
	DIMENSION NUMB(20), IFM(136), X(NELX), VARF(5)	PLTD	7
	NN=0	PLTD	8
	NN=NN+1	PLTD	9
	IFM(NN)=NUMB(11)	PLTD	10
	DO 20 II=IFIRST,ILAST	PLTD	11
	NX=X(II)*SX	PLTD	12
	IF (NX.GT.130) NX=130	PLTD	13
	IF (NX.LT.13) NX=13	PLTD	14
	NXP=NX-2	PLTD	15
	DO 10 I=1,NXP	PLTD	16
	NN=NN+1	PLTD	17
	IFM(NN)=NUMB(12)	PLTD	18
	10 CONTINUE	PLTD	19
	NN=NN+1	PLTD	20
	IFM(NN)=NUMB(11)	PLTD	21
	20 CONTINUE	PLTD	22
	DO 30 II=1,NYP	PLTD	23
	WRITE (IO,VARF) (IFM(I),I=1,NA)	PLTD	24
	30 CONTINUE	PLTD	25
	RETURN	PLTD	26
	END	PLTD	27

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