Investigation on the Mechanisms Governing the Robustness of Self-Compacting Concrete at Paste Level

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Abstract In spite of the many advantages, the use of self-compacting concrete (SCC) is currently widely limited to application in precast factories and situations in which external vibration would cause large difficulties. One of the main limitations is the higher sensitivity to small variations in mix proportions, material characteristics and procedures, also referred to as the lower robustness of SCC compared to vibrated concrete. This paper investigates the mechanisms governing the robustness at paste level. Phenomenological aspects are examined for a series of paste mixtures varying in water film thickness and superplasticizer-to-powder ratio. The impact of small variations in the water content on the early-age structural buildup and the robustness of the paste rheology is investigated using rotational and oscillating rheometry.

Key words: *Self-compacting concrete, SCC, Robustness, Sensitivity, Rheology, Storage Modulus.*

Introduction

Self-compacting concrete (SCC) is a high performance concrete, characterized by the absence of the need of external compaction. As a result, less construction errors are made and significantly less man effort is required. However, despite the many

benefits of SCC, the use in actual structures is mainly limited to precast concrete products and situations requiring a high flowability or in which external compaction would result in large difficulties. One of the major limitations for the use of SCC is its lower robustness compared to vibrated concrete, which is its sensitivity to small changes in the material properties, material proportions, or production methods.

This lower robustness imposes a more rigorous quality control demand on material properties and mix proportioning, skilled and experienced staff, and a better understanding of the mix design. Regarding the mix design, the following trends have been observed:

- A surplus of fines in the aggregate grading curve results in a higher robustness of SCC [1, 2]. The surplus of fines prevents the coarse aggregate particles from dominating the rheology.
- In SCC having a high plastic viscosity, the robustness against small variations in the water content increases as the amount of powder in the mixture is high [2, 3]. For SCC with a low plastic viscosity, an opposite trend is observed [3].
- An increase of the water-to-powder ratio increases the robustness of the V-funnel flow-time against variations in the water content [3, 4]. However, Kwan and Ng [1] have shown that a lower water-to-powder ratio increases the robustness of the slump flow against variations in the superplasticizer content. More fundamental research on this topic is necessary.
- The choice of superplasticizer [5, 6] and VMA [4, 6, 7] also affects the robustness. The addition of a VMA in the mix design can increase or decrease the robustness of the mixture [2-4, 7].
- A possible link between the thixotropy and robustness has been suggested [8, 9]. Low alkali cement is also reported to be less robust than high alkali cement [10]. Low alkali cement contains less SO₄²⁻ and C₃A, which results in a lower heat of hydration and less structural buildup [11].

Although many other parameters and influences can cause the rejection of a SCC batch [12-14], this experimental program focusses on variations in the water content. Variations in the water content have the largest impact on the rheology in concrete plants since the dosage of admixture and powders is measured very precisely and variations in the properties and grading curve of sand and gravel have a relatively smaller impact [15]. According to the European guidelines [16], a good SCC mix design should allow variations of 5 to 10 l/m³ in the water content, which corresponds with about 3 to 6% of the water content. The ACI 117-90 and EN 117-90 codes allow variations up to 3% of the water content during the industrial production of concrete.

In this study, the Water Film Thickness (WFT) of all mixtures was evaluated. According to Li and Kwan [17], the water in fresh concrete can be divided into two

parts: the filling water which fills the voids in between the solid particles, and the excess water which forms a water film on the surface of the solid particles and contributes to the fluidity of the fresh concrete. The WFT can be calculated using Equations 1-3 (Table I: Definition of parameters). The maximum packing density ϕ_{max} is calculated based on the maximum possible density of the paste obtained by variations in the water content.

$$WFT = \frac{u_w - u_{min}}{A_s} \qquad (Eq. 1) \qquad u_w = \frac{1 - \phi - \varepsilon_a(\phi)}{\phi} \qquad (Eq. 3)$$
$$u_{min} = \frac{1 - \phi_{max} - \varepsilon_a(\phi_{max})}{\phi_{max}} \qquad (Eq. 2)$$

Symbol	Unit	Name	Meaning
INET	[m]	Water Film	Thickness of the excess water layer covering the
VV F I	[111]	Thickness	solid particles.
φ	[%]	Packing density	The volume of solids divided by the bulk volume
4	F0/1	Maximum	The maximum possible packing density possible
ϕ_{max}	[70]	packing density	for this mixture under varying water content.
$\varepsilon_a(\phi)$	[%]	Air content	The volume of air divided by the bulk volume.
	[%]	Water ratio	The volume of water divided by the volume of
u_w		water ratio	solids.
	F0/1	Minimum voids	The water ratio corresponding with the maximum
u_{min}	[70]	ratio	packing density.
4	[m2/m3]	Specific surface	The total surface of al solids in one volumetric
A _S	$[m^2/m^3]$	area of the solids	unit.

Table I. Definition of the parameters used in Equations 1 to 3

When performing tests on paste, the shear forces during mixing and testing have a different order of magnitude compared to the concrete level [18, 19]. This causes differences in the flocculation of fines [20], the hydration speed [21], the thixotropic behavior [22], and relationships are difficult to establish between workability tests on paste and concrete. The investigated parameters might also affect the robustness of the stability against segregation. As a result, extrapolations from paste level to concrete level should be treated with great prudence.

Experimental setup

In order to investigate the influence of the water film thickness and paste fluidity on the robustness of the rheology against small variations in the water content, nine selfcompacting pastes, varying in water-to-powder volumetric ratio (0.85, 0.90, and 0.95) and superplasticizer dosage (0.118%, 0.159%, and 0.200% of the cement weight) were tested. In order to cover a wider range of WFT, four additional mixtures were tested with water-to*Table II.* Chemical composition of the cement and limestone filler

	Cement	Limesto
	I 52.5 N	ne filler
	[%]	[%]
CaO	63.01	0.00
CaCO ₃	0.00	98.8
SiO ₂	18.55	0.11
Al ₂ O ₃	5.83	0.04

powder / superplasticizer dosage combinations of respectively 0.75/0.200%, 0.80/0.200%, 1.00/0.159%, and 1.00/0.118%. The cement-topowder ratio (by weight) was always kept at 0.6. The material properties and grading curves of the cement and limestone are summarized in Figure 2 and Table II. Tap water and a polycarboxylate (PCE) superplasticizer with a concentration of 35% were also used in the experiments. The specific surface of the cement and limestone were calculated based on the particle size distribution which is given in Figure 1.

Table III summarizes all mix compositions. For each mix composition, two additional mixtures were fabricated in order to evaluate the robustness against small variations in the water content. One with 3% more water, and one with 3% less water. Each mixture was made according to the mixing and testing procedure given in Table IV in a Hobart mixer.

Fe ₂ O ₃	4.09	0.04
MgO	1.22	0.32
K ₂ O	0.60	0.00
Na ₂ O	0.53	0.01
SO ₃	2.97	0.02
Cl ²⁻	0.086	< 0.008
L.O.I.	1.24	-
Insolubl e rest	0.94	-
Density	3116	2674
	kg/m³	kg/m³
Specific	339	434
surface	m²/kg	m²/kg



and limestone powder

Table III. Mix proportions of the 14 reference self-compacting paste mixtures

Mix nr	Water-to-	Super-	Cement	Limeston	Water	Super-
	powder	plasticizer		e powder		plasticizer
	ratio	dosage				dosage
	[-]	[%]	[kg/m ³]	[kg/m ³]	[kg/m ³]	[kg/m ³]
1	0.85	0.118	948	632	459	1.12
2	0.90	0.118	923	615	474	1.09
3	0.95	0.118	899	600	487	1.06
4	1.00	0.118	877	585	500	1.03
5	0.85	0.159	948	632	459	1.51
6	0.90	0.159	923	615	474	1.47
7	0.95	0.159	899	600	487	1.43
8	1.00	0.159	877	585	500	1.39
9	0.75	0.200	1002	668	429	2.00
10	0.80	0.200	974	650	444	1.95
11	0.85	0.200	948	632	459	1.90
12	0.90	0.200	923	615	474	1.85
13	0.95	0.200	899	600	487	1.80

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Time	Duration	Step	Mixing
			speed
0 min	1 min	Mixing of cement, limestone powder and water	140 rpm
1 min	1 min	Adding the superplasticizer	140 rpm
2 min	1 min	Mixing	285 rpm
3 min	2 min	A thin layer of paste is scraped from the mixing arm	0 rpm
		and the walls and bottom of the mixing bowl	
5 min	1 min	Mixing	285 rpm
6 min	11 min	Rest	0 rpm
17 min	1 min	Remixing	285 rpm
18 min	2 min	Rest	0 rpm
20 min	3 min	Rotational rheometry: determination of the Modified	-
		Bingham parameters	
25 min	45 min	Start oscillatory rheometry: measurement of G'	-
		buildup	
29 min	1 min	Remixing	285 rpm
30 min	5 min	Measuring the density and air content	-

Table IV: Mixing and testing procedure

The Modified Bingham parameters [23] (Equation 4 and Table V) of each paste were determined using rotational rheometry in an Anton Paar MCR 201 rheometer with a wide gap concentric cylinder configuration. The inner cylinder has a radius of 20 mm, a height of 60 mm and it is covered with a sand-blasted surface; the outer cylinder has a radius of 35 mm and is provided with ribs to prevent wall slip. The rotational velocity profile, illustrated in Figure 2, consists of a preshear step, a stepwise decreasing rotational velocity profile, and the determination of a segregation point. When the torque measured during a rotational velocity step was not in equilibrium, this data point was not used for the analysis. A plug flow correction was performed when plug flow occurred [24].

$$\tau = \tau_0 + \mu \cdot \dot{\gamma} + c \cdot \dot{\gamma}^2 \tag{Eq. 4}$$

Modified Bingham equation (Eq. 4)							
Symbol	Unit	Meaning					
τ	[Pa]	Shear stress					
Ϋ́	$[s^{-1}]$	Shear rate					
$ au_0$	[Pa]	Yield stress					
		Modified					
μ	[Pa.s]	Bingham					
		linear term					
		Modified					
6	$[\mathbf{D}_{2}, c^{2}]$	Bingham					
Ĺ		second order					
		term					

Table V: Symbols used in the



applied in rotational rheometry

Oscillatory rheometry was used to monitor the structural buildup of the paste sample at rest. The storage modulus G' evolution was measured using an Anton Paar MCR 201 rheometer with a vane in cylinder setup. In these experiments, a vane with a diameter of 15 mm and a height of 40 mm vibrates within a small angle and a frequency of 1 Hz in an outer cylinder with a radius of 35 mm. After destroying the structure in a 2 minutes time sweep with a strain of 50% (above the critical strain), the structural buildup inside the paste was monitored in a time sweep with a small strain of 0.1% (below the critical strain) for 20 minutes using the storage modulus G' [25-27]. A typical example of a measurement is shown in Figure 3.



Figure 3. Monitoring of the Storage Modulus G' using oscillatory rheology

Results and Discussion

The measured properties of the 13 reference mixtures are listed in Table VI. The range of covered WFT values is illustrated in Figure 4 and are based on the maximum packing densities of the powder measured in wet condition. A smaller dosage of superplasticizer leads to a higher maximum packing densities and smaller WFT values at a similar water-to-powder ratio. A clear link between the WFT and the rheology of the mixture can be observed in Figure 5.

Mix	Wate	Supe	Max.	Air	WFT	Yiel	MB	MB	Incre	Incre
nr	r-to-	r-	pack	cont-		d	linea	2^{nd}	ase	ase
	pow	plasti	-ing	ent		stres	r	order	in G'	in G'
	der	-	densi			S	term	term	(1)	(2)
	ratio	cizer	ty							
		dosa								
		ge								
	[-]	[%]	[%]	[%]	[um]	[Pa]	[Pa.s	[Pa.s	[Pa]	[Pa]
	LJ	[,]	[,]	[,]	[piii]	[- "]]	2	[* "]	[1 "]
1	0.85	0.11	0.57	1.0	0.18	16.3	1 59	0.00	1073	1873
1	0.05	8	7	1.0	8	8	1.57	00	1075	1075
2	0.90	0.11	0.57	0.8	0.19	7.05	0.85	0.00	711	1218
2	0.70	8	7	0.0	8	7.05	0.05	05	/11	1210
3	0.95	0.11	0.57	0.5	0.27	2 55	0.30	0.00	717	1665
5	0.75	8	7	0.5	6	2.55	0.37	24	/1/	1005
4	1.00	0.11	0.57	0.5	0.30	2.17	0.20	0.00	514	1220
4	1.00	8	7	0.5	4	2.17	0.30	12	514	1220
5	0.95	0.15	0.58	0.4	0.20	260	0.41	0.00	222	265
5	0.85	9	9	0.4	9	2.08	0.41	48	322	303

Table VI. Properties of the reference self-compacting paste mixtures

6	0.90	0.15 9	0.58 9	0.5	0.23 6	2.23	0.32	0.00 34	306	364
7	0.95	0.15 9	0.58 9	0.2	0.30 8	0.62	0.28	0.00 18	302	411
8	1.00	0.15 9	0.58 9	0.2	0.34 0	0.49	0.14	0.00 31	225	521
9	0.75	0.20 0	0.60 3	0.6	0.15 1	8.71	0.68	0.00 86	701	1007
10	0.80	0.20 0	0.60 3	0.5	0.20 9	3.22	0.28	0.00 66	265	462
11	0.85	0.20 0	0.60 3	0.3	0.26 7	0.77	0.32	0.00 47	206	202
12	0.90	0.20 0	0.60 3	0.1	0.28 9	0.00	0.22	0.00 31	256	504
13	0.95	0.20 0	0.60 3	0.3	0.35 5	0.00	0.18	0.00 22	382	2836





Figure 4. The range of WFT covered in this experimental program

Figure 5. Influence of the WFT on the rheology

The robustness against small variations in the water content is tested by changing the water content with $\pm 3\%$. Table VII summarizes the impact on the rheological characteristics. Because the variations in the test results should be compared by the value of the reference mixture, all changes are expressed as percentages.

Figure 6 illustrates an increasing waterto-powder ratio or superplasticizer dosage increases the robustness of the shear stress at a shear rate of 20 s⁻¹ (τ (20 s⁻¹). A similar trend can be observed using the shear stress inclination at 20 s⁻¹ ($\frac{d\tau}{d\dot{\gamma}}$ (20 s⁻¹). The effect on the yield stress seems to be independent of water-to-powder ratio (similar slopes are obtained).



Figure 6. The influence of the WFT and

superplasticizer dosage on the robustness of the rheology

			content			
Mix nr	Water-	Superpla	WFT	Changes	Changes	Changes
	to-	sticizer		in the	in the	in the
	powder	dosage		shear	yield	shear
	ratio			stress at	stress	stress
				20s-1		inclinati
						on at
						20s-1
	[-]	[%]	[µm]	[%]	[%]	[%]
1	0.85	0.118	0.188	167	176	160
2	0.90	0.118	0.198	111	126	105
3	0.95	0.118	0.276	140	201	115
4	1.00	0.118	0.304	87	118	73
5	0.85	0.159	0.209	134	257	77
6	0.90	0.159	0.236	112	196	91
7	0.95	0.159	0.308	94	313	75
8	1.00	0.159	0.340	57	121	51
9	0.75	0.200	0.151	120	118	95
10	0.80	0.200	0.209	128	199	89
11	0.85	0.200	0.267	53	245	48
12	0.90	0.200	0.289	81	248	57
13	0.95	0.200	0.355	86	90	73

Table VII. Robustness of the reference mixtures against small changes in the water content

Figure 7 shows the changes of the rheology in a rheogram. A logarithmic scale is used to illustrate the graphs because the impact of a change in the rheological parameters depends on the value of the parameter itself. A change of 0.1 Pa on the yield stress has a more pronounced impact on a mixture with a yield stress of 0.2 Pa than on a mixture with a yield stress of 50 Pa. Based on the concept of robustness area described by Billberg and Westerholm [7] on concrete rheograms, a definition of robustness is proposed. Assuming a rectangle surrounding the changes in rheology on the logarithmic graphs in Figure 8 illustrates the sensitivity of pastes to small changes in the water content, the definition of the robustness value is defined as one divided by the area of the rectangle in a logarithmic scale (Equation 5). The higher R is, the more robust is a paste system.

$$R = \frac{1}{\log\left(\frac{\tau_{0,max}}{\tau_{0,min}}\right) * \log\left(\frac{\mu_{max}}{\mu_{min}}\right)} \quad (Eq. 5)$$

This definition of the robustness allows to compare the combined changes relative to the original values of the two parameters describing the rheological behavior of the paste. Table VIII summarizes the robustness value R of all mixtures. The table also summarizes the increases in storage modulus G' during the first and second 20 minutes of structural buildup during the oscillatory rheometry (G'₁ and G'₂).



Figure 7b. Robustness of the rheology illustrated in a rheogram (SP 0.159%)



Figure 7a. Robustness of the rheology illustrated in a rheogram (SP 0.118%)



Figure 7c. Robustness of the rheology illustrated in a rheogram (SP 0.200%)

Mi	Water-	Superp	WFT	Yield	Inclinatio	Incre	Incre	Robustn		
Х	to-	lasticiz		stress	n of the	ase in	ase in	ess		
nr	powde	er			shear	G'1	G'2	value R		
	r ratio	dosage			stress at					
					20s-1					
	[-]	[%]	[µm]	[Pa]	[Pa]	[Pa]	[Pa]	[-]		
1	0.85	0.118	0.188	16.38	1.59	1073	1873	2.44		
2	0.90	0.118	0.198	7.05	0.87	711	1218	4.28		
3	0.95	0.118	0.276	2.55	0.49	717	1665	5.22		
4	1.00	0.118	0.304	2.17	0.35	514	1220	5.59		
5	0.85	0.159	0.209	2.68	0.60	322	365	3.81		

Table VIII. The robustness evaluation of all reference mixtures

6	0.90	0.159	0.236	2.23	0.46	306	364	2.29
7	0.95	0.159	0.308	0.62	0.35	302	411	2.81
8	1.00	0.159	0.340	0.49	0.26	225	521	8.69
9	0.75	0.200	0.151	8.71	1.03	701	1007	4.40
10	0.80	0.200	0.209	3.22	0.54	265	462	3.38
11	0.85	0.200	0.267	0.77	0.50	206	202	3.66
12	0.90	0.200	0.289	0.00	0.35	256	504	6.45
13	0.95	0.200	0.355	0.00	0.27	382	2836	8.99

Based on the robustness definition, the following trends and influence factors are observed:

- Figures 8 and 9 illustrate the correlation between the robustness and the water-to-powder volumetric ratio (SP 0.118%: $R^2 = 0.91$; SP 0.159%: $R^2 = 0.45$; SP 0.200%: $R^2 = 0.68$) or a higher WFT ($R^2 = 0.47$). The relation between the water-to-powder ratio and the robustness depends on the superplasticizer dosage. Similar trends can be found based on the ratio of the packing density to the maximum packing density ϕ/ϕ_{max} of the mixtures ($R^2 = 0.42$).
- No clear influence of the yield stress τ_0 , inclination of the shear stress at 20 s⁻¹ $\frac{d\tau}{d\dot{\gamma}}$ (20 s⁻¹), or the shear stress at 20 s⁻¹ τ (20 s⁻¹) on the robustness is observed.





Figure 8. The influence of the water-to-powder ratio on the robustness value

Figure 9. The influence of the WFT on the robustness value

• In Figure 10, the relation between the sensitivity of the rheology and the structural buildup as measured by the storage modulus G' buildup at rest is illustrated. Mixtures with a higher G' buildup rate were more sensitive to changes in the shear rate and the inclination of the shear rate of mixtures (Table VIII). However, no relation between the structural buildup and the robustness value can be established.



Figure 10. Influence of the structural buildup on the robustness of the shear stress at 20 s^{-1}

The observed relation between the water-to-powder ratio or WFT and the rate of structural buildup is valid on paste level. Because the pastes were mixed in the absence of the ball-bearing effect of aggregates and in a Hobart mixer, the hydration reaction rate differs from a paste mixed inside a concrete mixture.

Conclusions

Based on an extensive experimental program, some possible mechanisms governing the robustness of the paste rheology against small variations in the water content have been investigated. Thirteen mixtures varying in water-to-powder volumetric ratio and superplasticizer dosage were subjected to variations of $\pm 3\%$ of their water dosage. Higher water-to-powder ratios resulted in a higher water film thicknesses (WFT), and in more robust mixtures. A higher superplasticizer dosage resulted in a higher WFT and also more robust mixtures. A higher early age structural buildup as measured by the increase in storage modulus G' at rest resulted in less robust mixtures. No clear influence of the viscosity of the mixtures was observed.

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