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Penetration of Water into Uncracked and Cracked Steel Reinforced Concrete Elements; Visualization by Means of Neutron Radiography

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Abstract

Service life of reinforced concrete structures depends on the quality of concrete, the cover thickness, and the aggressiveness of the environment. Water and harmful compounds dissolved in water are taken up into the porous structure of concrete by capillary action and diffusion or a combination thereof. The combined process is very complex and cannot be described by simple models. Therefore prediction of service life of reinforced concrete structures in aggressive environment remains unreliable. In this contribution it will be shown that neutron radiography is a very powerful method to study moisture uptake and movement in porous materials such as concrete. Results will provide us with a solid basis for reliable predictive models.

First fundamentals of the test method will be briefly described. It will be shown how this method can be applied to follow the time dependent process of capillary suction quantitatively. In addition uptake of water in the vicinity of cracks has been studied in some detail. It is of particular interest that movement of water into the interface between steel reinforcement and concrete and into the fracture process zone ahead of a crack tip can be visualized and determined quantitatively by means of neutron radiography. It is the first time that these decisive processes with respect to durability can be studied in detail. The obtained results will be discussed and the potential of the test method for future investigations will be outlined. Results presented in this contribution will help us to understand better deteriorating processes in reinforced concrete structures and to find ways to improve durability.

Keywords: Neutron radiography, concrete, capillary suction, durability, service life.

Eindringen von Wasser in Stahlbetonbauteile mit und ohne Risse; Visualisieren mit Hilfe der Neutronenradiographie

Zusammenfassung

Die Nutzungsdauer eines Stahlbetontragwerkes hängt in erster Linie von der Betonqualität, der Dicke der Überdeckung und der Aggressivität der Umgebung ab. Sowohl Wasser als auch die in Wasser gelösten Schadstoffe werden durch Kapillarkraft und Diffusion beziehungsweise eine Kombination dieser beiden Mechanismen in den Porenraum des Betons aufgenommen. Der kombinierte Transportprozess ist komplex und kann deswegen nicht mit Hilfe von einfachen Modellen wirklichkeitsnah beschrieben werden. Aus diesem Grund bleibt die Vorhersage der Nutzungsdauer von Stahlbetontragwerken in aggressiver Umgebung bis auf den heutigen Tag unzuverlässig. In diesem Beitrag wird gezeigt, dass die Neutronenradiographie eine leistungsfähige Methode ist, mit der die Aufnahme und Bewegung in porösen Werkstoffen wie etwa Beton mit hoher Genauigkeit untersucht werden kann. Die Ergebnisse liefern uns eine solide Basis für die Entwicklung zuverlässiger Modelle für die Vorhersage der Nutzungsdauer.

Zunächst werden die Grundlagen der hier vorgestellten Untersuchungsmethode kurz beschrieben. Dann wird gezeigt, wie diese Methode dazu verwendet werden kann, die kapillare Wasseraufnahme quantitativ zu verfolgen. Zusätzlich wurden noch die Wasseraufnahme durch Risse und der Transport aus Rissen in das angrenzende poröse Material ins Einzelne gehend untersucht. Es ist von ganz besonderem Interesse, dass die Wasseraufnahme der Grenzfläche zwischen Stahlbewehrung und Beton sowie das Eindringen von Wasser in die Rissprozesszone (FPZ) vor der Spitze eines wirklichen Risses mit Hilfe der Neutronenradiographie sichtbar gemacht und quantitativ bestimmt werden können. Auf diese Weise ist es erstmals gelungen, diese für die Beständigkeit von Stahlbetontragwerken entscheidenden Prozesse visuell nachzuweisen und detailiert zu untersuchen. Die erhaltenen Ergebnisse werden am Ende dieser Veröffentlichung diskutiert und das Potential für weitergehende Untersuchungen wird skizziert. Die hier vorgestellten Ergebnisse werden dazu beitragen, schädigende Prozesse in Stahlbetontragwerken besser zu verstehen und neue Wege zu finden, die Nutzungsdauer zu verlängern.

Stichwörter: Neutronenradiographie, Stahlbeton, Kapillares Saugen, Beständigkeit, Nutzungsdauer

1 Introduction

Moisture movement in porous building materials is a crucial process for their durability. Cement-based materials are often water saturated when the formwork is being removed. Then a long lasting drying process begins. Drying is at the origin of shrinkage and shrinkage causes frequently cracks in drying structural elements. In permanent contact with water cement-based materials may also undergo damage due to hydrolysis of hydration products. Whenever pre-dried concrete gets in contact with driving rain, capillary action of the porous structure will lead to quick absorption of considerable amounts of water. Ions such as chlorides or sulphates dissolved in the absorbed water are transported deep into the material. In this way noxious compounds can seriously damage the surface near zone and corrosion of steel reinforcement may be initiated. A porous material containing more water than a critical value may also be sensitive with respect to frost action. High corrosion rate of steel in concrete requires a critical amount of water content. Thermal insulation of brick masonry is strongly reduced at high water content. There are many more examples, which all prove that we need to understand processes of water movement and the development of moisture distributions in porous building materials in order to improve durability and serviceability of building materials and to ascertain required service life of buildings and structures.

There are different non-destructive methods to determine moisture content in porous materials. They can be subdivided into two groups: (1) NMR methods based on electromagnetic interaction with the water in the porous material [1, 2] and (2) radiography methods, based on weakening of radiation passing through the material [3, 4]. X-rays, γ -rays and neutrons have been used already in the past to visualize regions containing different amounts of water in building materials. Neutron radiography has also been applied in the past to study moisture movement in porous building materials [5-9]. In this contribution we will present new results obtained recently by neutron radiography.

The principle of radiography is based on recording the radiation passing through an object by means of a position sensitive detector [10]. The experimental technique is shown schematically in Fig. 1. An isotopic neutron source, an accelerator, a research reactor or a spallation source may serve as neutron source for neutron radiography. The beam coming from the source is formed by a collimator, which defines the geometric properties of the beam. The



Figure 1: Schematic representation of experimental set-up for neutron radiography

collimator generally also contains filters to influence the spectrum of the beam and to reduce beam pollution by γ -rays. Behind the object the beam is recorded by means of electronic detectors, which allow directly obtaining digital images. The registered neutrons can have passed the object without interaction or they can have been scattered from another region. In order to obtain realistic moisture distributions the obtained image has to be treated by transmission analysis [7].

The neutron cross section of water is far bigger than the corresponding value for common porous building materials [5]. Therefore the method is very sensitive with respect to determination of moisture content. On concrete plates with a thickness of 20 to 30 mm a few mg/cm³ of water can be observed with a spatial resolution of 1 mm or less [5].

This method shall be applied to observe the rate of penetration of water into pre-dried concrete. Water uptake of a region close to a crack will also be studied. The main aim of this contribution is, however, to point out the unique potential of this method for following moisture movement in porous materials such as concrete or mortar. In a follow up paper the influence of surface impregnation of concrete with a water repellent agent will be presented and discussed.

In this contribution capillary action has been considered to be the only driving force for moisture movement. This obviously is a simplification. The method shall be later applied to study moisture migration under the influence of frost suction and osmotic pressure.

2 Capillary Suction

If the surface of porous materials such as concrete or mortar is in contact with water or any other wetting liquid, the liquid will be absorbed by capillary action. In the simplest case, as for instance one single capillary with radius r, the absorbed amount of water as function of time can be described by means of the following equation [11]:

$$\Delta W = A \sqrt{t} \tag{1}$$

With *A* being the coefficient of capillary suction $[kg/m^2 s^{1/2}]$; It can be shown that *A* has the following physical meaning:

$$A = \Psi \rho \sqrt{\frac{r_{eff} \sigma \cos \Theta}{2\eta}}$$
(2)

In equation (2) Ψ stands for the water capacity of the absorbing material [m³/m³], that means the volume within the porous space, which can be filled by capillary suction, ρ stands for the density of the absorbed liquid [kg/m³], r_{eff} designates an effective radius [m], representing the pore size distribution of a given material, σ is the surface tension [Nm/m²] and Θ the wetting angle of the liquid, and finally η is the temperature dependent viscosity of the liquid [(N s)/m²].

The penetration depth of the absorbed liquid as function of time x(t) can be predicted by means of the following equation:

$$x(t) = B\sqrt{t} \tag{3}$$

In equation (3) *B* is the coefficient of capillary penetration $[m/s^{1/2}]$, and has the following meaning:

$$B = \frac{A}{\Psi \rho} \tag{4}$$

With equation (4) equations (2) and (3) can be linked. If the coefficient of capillary absorption Aand the water capacity Ψ have been determined experimentally, the coefficient of capillary penetration B can be calculated by means of equation (4). Obviously equations (1) to (4) have been derived and are valid strictly speaking for a single capillary or a parallel bundle of capillaries only. But within a certain range these equations can also be used as a first approximation to describe capillary absorption and penetration as function of time for real materials with complex porous systems such as cement-based materials.

A crack can be considered to be a slit like capillary as a first approximation. Obviously in this case tortuosity and surface roughness of the crack surface is neglected when the above mentioned equations are applied. The rate of penetration into a crack, however, can then be estimated by means of the following equation:

$$\frac{dx}{dt} = \frac{B}{2\sqrt{t}} \tag{5}$$

The width of visible cracks in concrete is of the order of magnitude of 0.1 to 0.5 mm, while the effective radius of cement-based materials is of the order of magnitude of some μ m. We can expect that a crack will be filled nearly instantaneously as compared with the water penetration into the uncracked porous material.

In this simplifying theoretical approach the influence of gravity on capillary rise is neglected. In wide cracks this is certainly not justified and a more rigorous approach shall be adopted in the future. It must be mentioned at this point that a macroscopic theory is not applicable to describe liquid and vapour flow in gel pores of hardened cement paste. In this case the intense interaction of water with the solid surfaces has to be taken into consideration [12, 13]. This aspect of basic research shall be considered in more detail the future.

3 Preparation of Test Specimens

All specimens have been prepared with the same type of mortar. The composition is shown in Table 1. Ordinary Portland cement, type 42.5 (C) and local river sand with a maximum grain size of 2.5 from Qingdao area (S) have been mixed with tap water. The maximum grain size has been limited to a value of 2.5 mm in order to have a representative volume in slices with a thickness of approximately 25 mm. Prismatic specimens with the following dimensions have been cast in steel forms: 100 x 100 x 300 mm³.

From the prisms slices with a thickness of approximately 25 mm have been cut after 14 days of moist curing. These slices were dried in a ventilated oven at 50 °C for four days. After this drying period constant weight had been achieved. The square surfaces and two opposite small surfaces (25 x 100 mm²) were then covered with self adhesive aluminium foils.

Part of the mortar prisms have been reinforced with six steel bars having a diameter of 8 mm each, as shown in Figure 2. These steel reinforced prisms were cut with a diamond saw into three slices after 14 days moist curing along the long axis of the prisms (see Fig. 2). The slices obtained in this way have been loaded by three point bending under well controlled conditions in order to induce one single crack with a given crack width in the middle (see Fig. 3). The crack width could be chosen to between 0.1 and 0.5 mm. From the inner part of the

W/C	C (kg/m ³)	S (kg/m ³)	W (kg/m ³)	Compressive strength (MPa)		
				3 days	7 days	28 days
0.6	500	1650	300	10.3	20.0	29.2

 Table 1:
 Composition of the mortar specimens and compressive strength



Figure 2: Position of the steel reinforcement in the mortar prisms and cutting lines



Figure 3: Formation of a centre crack under three point bending and detached specimen in contact with water for observation of capillary suction

cracked specimens slices with a width of 100 mm and the induced crack in the centre have been cut as shown in Figure 3. These slices $(25 \times 100 \times 100 \text{ mm}^3)$ have been oven dried until constant weight was achieved and then covered with self adhesive aluminium foils as described above for the unreinforced samples.

Uncracked and cracked specimens were then ready for measuring the penetration depth of water as function of time by neutron radiography. All specimens were placed in contact with water in the neutron beam.

4 Results and Discussion

4.1 Water Penetration into the Uncracked Material

Typical results obtained on a specimen without a crack are shown in Figure 4. The suction time is indicated in hours. At time 0 no moisture can be observed in the pre-dried sample. After 30 minutes of contact with water a penetration front becomes visible. This irregular front gradually moves into the material. It becomes obvious that the mortar samples

are not completely homogeneous, penetration rate is higher in the left part of the sample. This can most probably be explained by the manufacturing process of the samples and the direction of casting.

The obtained data have been further evaluated by means of earlier developed software IDL. In this way penetration profiles as shown in Figure 5 have been obtained. For the determination of the profiles the rectangular area marked in Figure 4 has been selected. The resulting profiles are shown in Figure 5.

When we plot the penetration depth as function of square root of time the relation shown in Figure 6 is obtained. In this case we have chosen the inflexion point of the profiles shown in Fig. 5 as penetration depth. This corresponds to a water content of approximately 0.01 g/cm^3 . The relation shown in Figure 6 clearly indicates that equation (3) is valid for a limited suction time only. For a period of up to four hours the penetration depth of water increases linearly with the square root of time. At higher contact times the rate of penetration decreases more than predicted by equation (3). This has been observed earlier but the physical background of this phenomenon is still to be clarified.



Figure 4: Direct observation of the penetration front of water into mortar by neutron radiography up to a contact time of 72 hours



Figure 5: Moisture profiles as measured after different durations of contact between the mortar sample and water



Figure 6: Penetration depth of water along a vertical axis as function of square root of contact time



Figure 7: Water penetration in the vicinity of a crack as function of time (hours)

4.2 Water Penetration into Cracked Reinforced Concrete

Results of neutron radiography of the plate with a crack are shown in Fig. 7. The contact time is indicated in hours. At time 0 that means in the dry state, the position of the steel reinforcement is indicated. When the surface of the mortar sample is put in contact with water, the artificial crack is filled immediately with water. It was not possible to follow the kinetics of the rising water front in the crack. It is of major interest that also the damaged interface between the reinforcement and the mortar is immediately water filled. The interface is damaged due to the imposed bending to create the crack.

Once the crack is water filled, water penetrates by capillary action horizontally into the mortar. It can be seen that the rate of horizontal capillary suction out of the crack and the vertical capillary suction from the adjacent water reservoir are approximately equal. The interface of both the lower and the upper reinforcement obviously has been damaged when the crack has been induced. Water penetrates far along the interfaces. Ahead of the real crack tip a fracture process zone has been created. Water can also penetrate easily into this pre-damaged zone.

In order to get the water profiles in a quantitative way, the water content has been calculated along the two axes shown in the radiograph taken after two hours of contact with water. The vertical axis has been placed just left of the crack and the horizontal axis slightly below the centre. The results obtained are shown in Figures 8 and 9 up to a maximum suction time of 24 hours.

We can see from Figure 8 that the penetration close to the bottom of the slab (depth = 0) initially resembles the penetration into an uncracked slab. But at the same time the interface (at a distance of approximately 20 mm) absorbs water from the crack. After about eight hours the mortar below the reinforcement has reached a state of saturation. The absorbed water content corresponds to the water capacity. After 24 hours of contact with water the water capacity of the material in between the two reinforcement bars has also been reached. Above the upper reinforcement bar the material has been severely damaged by localized loading under three point bending. Therefore the material can absorb more water by capillary action. The increased water capacity in this region is a measure for the induced damage. The position of the steel bars is clearly reproduced by the two minima of water content in the profile. The lower interface of both steel bars embedded in concrete absorbs more water than the upper interface. This is most probably due to influence of gravity.

Water profiles along the horizontal axis shown in Figure 7 are reproduced in Figure 9. It can be seen that the crack is water filled before the first measurement, which corresponds to about one minute after contact of the sample's surface with water. Then water penetrates out of the crack horizontally into the porous material. This penetration process is approximately symmetric with respect to the position of the crack.

The penetration depth along the horizontal axis is shown as function of square root of time (hours) in Figure 10. It can be seen that in this case the penetration depth is very well predicted by equation (3) for at least 12 hours. This may be an indication that the well known skin effect of concrete [14] and the influence of gravity on vertical water penetration contribute to the deviation of results shown in Fig. 6. Precise data obtained from neutron radiography will provide us for the first time with the necessary information for the development of realistic predictive models to describe uptake of water and the noxious chemical compounds dissolved in water by porous materials such as concrete. This will finally provide us with a solid basis for the prediction of service life of reinforced concrete structures.



Figure 8: Moisture distribution along a vertical axis near the crack as function of time. It can be seen that the interfaces and the pre-damaged zone where the force has been applied during three point bending absorb more water.



Figure 9: Moisture distribution along a horizontal axis just below the centre. The crack is immediately filled with water. Then the water migrates under the influence of capillary force symmetrically normal to the crack surface.



Figure 10: Penetration depth of water along a horizontal axis as function of square root of contact time

5 Conclusions

From results presented in this contribution we may conclude:

- Neutron radiography is a powerful method to observe uptake into and movement of water in porous materials such as concrete, mortar, wood, or brick masonry.
- Due to the high precision and high spatial resolution of this method, the obtained data provide us with a solid basis for the development of predictive models.
- Fine cracks are instantaneously filled with water, if the surface of concrete is placed in contact with water. In this way dissolved noxious chemical compounds can be transported deep into structural elements in a very short time.
- Water penetrates from a water filled crack into the adjacent material following a square root of time law.
- The water capacitance of mechanically damaged cement-based materials increases substantially. This is one of the synergetic effects of combined mechanical and environmental loads.
- The increased water capacitance measured by neutron radiography is a measure for the mechanically induced damage.

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