

Time Dependent Flexural Analysis of Reinforced Concrete Members

Prepared by

Noor Md. Sadiqul Hasan

B.Sc (Civil Engineering) and M.Sc (Structural Engineering)

A dissertation submitted for the degree of **Doctor of Philosophy (Structural Engineering)**

School of Civil, Environment and Mining Engineering
The University of Adelaide
Australia

ABSTRACT

Concrete is one of the most widely used building materials in the construction industry in the world. Time dependent behaviour of concrete is the major concern for the structural engineers due to its significant effect in the long term serviceability and durability. Reinforced concrete (RC) members are prone to the effect of time dependent deformations that are known as shrinkage and creep, can produce substantial deformations and deflections to the structure.

The mechanics of quantifying the serviceability deflection of RC beams is complex due to flexural cracking and the associated partial interaction (PI) behaviour of slip between the reinforcement and adjacent concrete. Add the additional complexity of time dependent concrete shrinkage to this partial-interaction (PI) behaviour and the problem becomes very complex.

Current design and analysis techniques to quantify serviceability deflection of reinforced concrete (RC) members are generally built on two major principles which are full interaction (FI) through the use of moment curvature approaches; and a uniform longitudinal shrinkage strain ε_{sh} within the member to simplify the analysis technique. Both of the premises are gross approximations and with regard to the first premise, RC beams are subject to flexural cracking and the associated partial interaction (PI) behaviour of slip between the reinforcement and adjacent concrete. Furthermore with regard to the second premise, numerous tests have shown that ε_{sh} varies along both the depth and width of the beam and which is far from uniform. Hence there are two major sources of error in the quantification of serviceability deflections of RC beams for design and which are due to the PI mechanisms that occur in practice; and that due to the time dependent material properties of creep and shrinkage.

This thesis deals with the development of PI numerical mechanics models with non-linear shrinkage strain variations achieved from a moisture diffusion model developed in this study and that is required to simulate the PI behaviour of RC beams in order to considerably reduce the source of error occurred due to the application of numerical mechanics model. Hence this new mechanics model will allow: the development of better design mechanics rules for serviceability deflection; and also assist in the better quantification of non-linear shrinkage and creep by removing or considerably reducing

the existing mechanics source of error. Importantly, this research provides mechanics solutions for all the facets that control the serviceability time dependent behaviour of RC beams and it is envisaged that these numerical mechanics solutions can provide researchers with the tools to develop simple design procedures as they simulate the major mechanisms influencing cracking and tension stiffening in reinforced concrete beams. Current shrinkage test methodology is having some limitations that are all surfaces are exposed to the environment and they are small scaled which leads to a uniformity of shrinkage strain and which are not present in real size RC beams. Therefore in this thesis, a new form of experimental setup for shrinkage have been proposed to better quantify the shrinkage variations along both the width and depth of RC members with varying the sizes and surface boundary conditions.

STATEMENT OF ORIGINALITY

I certify that this work contains no material which has been accepted for the award of any other degree or diploma in my name, in any university or other tertiary institution and, to the best of my knowledge and belief, contains no material previously published or written by another person, except where due reference has been made in the text. In addition, I certify that no part of this work will, in the future, be used in a submission in my name, for any other degree or diploma in any university or other tertiary institution without the prior approval of the University of Adelaide and where applicable, any partner institution responsible for the joint-award of this degree.

I give consequent to this copy of my thesis when deposited in the University Library, being made available for loan and photocopying, subject to the provisions of the Copyright Act 1968.

The author acknowledges that copyright of published works contained within this thesis resides with the copyright holder(s) of those works.

I also give permission for the digital version of my thesis to be made available on the web, via the University's digital research repository, the Library Search and also through web search engines, unless permission has been granted by the University to restrict access for a period of time.

Noor Md. Sadiqul Hasan	Date

ACKNOWLEDGEMENT

At first I would like to express my deepest gratitude and sincere appreciation to my principal supervisor Emeritus Professor Deric John Oehlers for his excellent supervision during my PhD study. I also would like to express my sincere appreciation to my cosupervisor Dr. Phillip Visintin for his wonderful supervision throughout the PhD research were an enormous help to me in the completion of this work. Their invaluable advice, continuing support, encouragement, patience and weekly regular meetings were generous help throughout the PhD work reached me to the final level.

I would also like to thank Dr. Terry Bennett for his good collaboration in solving my issues, providing guidance and his valuable advices throughout my PhD research.

The financial support from the Adelaide Scholarship International (ASI) through the University of Adelaide (UoA) are highly appreciated.

Finally, I am very much grateful to my parents, family members and relatives for their love, continuing support, motivation, understanding and encouragement during my PhD start to the final day. A special thanks goes to my beloved Wife for her continuous support when it was needed, distraction when it was required and motivation when I lost mine. Last but not the least, I would like to dedicate my PhD thesis to my beloved parents.

List of Publication

Based on the research work one journal paper has been submitted for publication in Proceedings of the Institution of Civil Engineers – Structures and Buildings.

Hasan, N. M. S., Bennett, T., Visintin, P., Oehlers, D. J. (2016). "Mechanics of simulating the serviceability deflection of RC beams allowing for partial interaction and non-linear shrinkage" (Submitted to Proceedings of the Institution of Civil Engineers – Structures and Buildings).

Table of Contents

ABST	RAC'	T	l
STATI	EME:	NT OFORIGINALITY	III
ACKN	OWI	LEDGEMENT	IV
LIST (OF PU	UBLICATIONS	V
LIST (OF FI	GURES	IX
LIST (OF TA	ABLES	XIV
Chap	ter 1	I Introduction	1
1.1	Int	roduction	1
1.2	Sc	ope of the Research	6
1.3	Ai	ms and Objectives of the Research	6
1.4	Stı	ructure of the Thesis	7
Chap	ter 2	2 Literature Review	9
Intro	ducti	ion	g
2.1	Sh	rinkage and its types	9
2.2	Ef	fect of Shrinkage and Creep on Structures	12
2.3	Pı	rediction of Shrinkage Strains	14
2.4	Us	sing Moisture Diffusion to quantify Shrinkage	17
2.4	4.1	Moisture and Humidity Diffusion in Concrete	18
2.4	4.2	Shrinkage strain in Concrete	22
2.4	4.3	Correlations in between with Moisture loss, Humidity and Sh of Concrete	•
2.5	Me	ember Behaviour	37
2.:	5.1	Models to predict long term Deflections	37
2.6	Su	mmarv	41

Chapt	er 3 Journal Paper on Non-linear Shrinkage	43
Chapt	er 4 Simulating Shrinkage Strain using Moisture Diffusi	on84
4.1	Introduction	84
4.2	Moisture diffusion equation	84
4.3	Moisture diffusion coefficient	86
4.4	Finite difference method on moisture diffusion analysis	87
	Relationship between pore relative humidity and free shrinkage strain	
(concrete	88
4.6	Quantification of moisture diffusion coefficient	90
4.7	Four way flow in rectangular beam	92
4.8	Three way flow in a beam	94
4.9	Two way flow in a beam	95
4.10	One way flow in a beam	96
4.11	First simulation with Asad, Baluch et al. (1997)	98
4.12	Second simulation with Jafarifar (2012)	100
4.13	Third simulation with Kim and Lee (1999)	105
Chapt	er 5 Proposed Experimental Work	114
5.1	Introduction	114
5.2	Purpose of tests	114
5.3	Sizes of specimen	115
5.4	Testing for material properties	116
5.5	Standard shrinkage test	116
5.6	Instrumentation in details	116
5.7	Concluding remarks	130

(Chapt	er 6 Long term Beam Deflection using Segmental Approach	131
	6.1	Introduction	131
	6.2	Partial interaction segmental analysis	132
	6.3	Prior to cracking segmental analysis	133
	6.4	Accommodation of cracking in the segmental approach	135
	6.5	Partial interaction tension stiffening model	137
	6.6	Partial-interaction segmental model	143
	6.7	Constant longitudinal shrinkage along depth and width	144
	6.8	Variation in longitudinal shrinkage strain along depth	147
	6.9	Variation in longitudinal shrinkage strain along depth and width	149
	6.10	Parametric study	151
	6.11	Application to test specimens	154
(Chapt	er 7 Conclusions and Recommendations	158
	Refer	rences	161

List of Figures

Figure 1.1: Concrete strain component under sustained compressive stress (Gilbert and
Ranzi 2011)
Figure 1.2: Thesis layout of this research
Figure 2.1: Shrinkage stages and types of concrete
Figure 2.2: Shrinkage strain components in normal strength concrete (Sakata et al., 2004 cited in Gribniak et al., 2008)
Figure 2.3: Experimental and predicted shrinkage strain in a plain rectangular concrete prism ($80 \times 150 \times 500$ mm) using different codes and shrinkage prediction models 16
Figure 2.4: Experimental and predicted shrinkage strain in a standard concrete prism (50 \times 50 \times 300 mm) using different codes and shrinkage prediction models
Figure 2.5: Numerical moisture profiles compared with the experimental test results: (a) Plain CC mix; (b) Plain RCC mix; (c) SFR-CC mix (2.5%); (d) SFR-RCC mix (2.5%) (Jafarifar 2012)
Figure 2.6: Shrinkage strain variation along the thickness of concrete specimen for two different mixes and comparison with the analytical results (Kim and Lee 1998)
Figure 2.7: Calculated relative humidity compared to modified experimental results due to moisture diffusion only a) curing period = 3 days b) curing period = 28 days (Kim and Lee 1999)
Figure 2.8: Relationship between relative humidity and moisture diffusion on different moist curing period (Kim and Lee 1999)

Figure 2.9: Loss of moisture in concrete due to drying (Kim and Lee 1999)
Figure 2.10: Comparisons between experimental and numerical results (ambient
temperature 200 C): a) Exposed at 3 days ($w/c = 0.28$) b) Exposed at 28 days ($w/c = 0.28$)
c) Exposed at 3 days ($w/c = 0.40$) d) Exposed at 28 days ($w/c = 0.40$) e) Exposed at 3 days
(w/c = 0.68) (Kang et al 2012)
Figure 2.11: Relationship between ultimate shrinkage and relative humidity for cement
paste and mortar specimens (Bissonnette, Pierre et al. 1999)
Figure 2.12: Relation between shrinkage and weight loss for various types and sizes of
paste, mortar and concrete specimens (Bissonnette, Pierre et al. 1999)
Figure 2.13: Relationship between free shrinkage strain and interior relative humidity
(RH) of a) C30 concrete and b) C80 concrete (Zhang, J, Dongwei and Wei 2010) 32
Figure 2.14: Calculated free shrinkage strains and measured relative humidity at different
depths of the slab from exposed surface against drying period (Zhang, J, Dongwei and
Wei 2010)
Figure 2.15: Relationship between free shrinkage strain and moisture loss (ACI209R-92
1997; Asad, M 1995)
Figure 2.16: Experimental results of different concrete specimens drying shrinkage as a
function of weight loss (Granger, Torrenti and Acker 1997)
Figure 4.1: Flow chart diagram to perform moisture diffusion modelling process 89
Figure 4.2: a) Gilbert and Nejadi (2004) Beam B1a b) Sectional elevation of beam B1a at
A-A c) Four way flow in beam B1a d) Three way flow in beam B1a e) Two way flow in
beam B1a f) One way flow in beam B1a

Figure 4.3: Moisture diffusivity vs moisture content or pore relative humidity, h for the
beams tested by Gilbert and Nejadi (2004)
Figure 4.4: Shrinkage strain profile in a four way flow Gilbert and Nejadi (2004) Beam
B1a after 100 days, 250 days and 400 days of drying
Figure 4.5: Shrinkage strain profile in a three way flow Gilbert and Nejadi (2004) Beam
B1a after 100 days, 250 days and 400 days of drying
Figure 4.6: Shrinkage strain profile in a two way flow Gilbert and Nejadi (2004) Beam
B1a after 100 days, 250 days and 400 days of drying
Figure 4.7: Shrinkage strain profile in a one way flow Gilbert and Nejadi (2004) Beam
B1a after 100 days, 250 days and 400 days of drying
Figure 4.8: Relationship between moisture content or pore relative humidity, h and
moisture diffusivity D
Figure 4.9: Numerical simulation of experimental and predicted values of moisture loss at
1 cm from the drying surface using finite difference method
Figure 4.10: Moisture diffusivity versus pore relative humidity or moisture content for
various types of concrete mixes (Jafarifar 2012)
Figure 4.11: Numerical moisture profiles compared with the experimental results
simulated using finite difference method: (a) Plain CC mix; (b) Plain RCC mix; (c) SFR-
CC mix; (d) SFR-RCC mix
Figure 4.12: Relationship between moisture diffusivity and moisture content for three
different types of concrete a) H ($w/c = 0.28$) b) M ($w/c = 0.40$) c) L ($w/c = 0.68$) with
moist cured for 3 days and d) H ($w/c = 0.28$) e) M ($w/c = 0.40$) f) L ($w/c = 0.68$) with
moist cured for 28 days $\frac{109}{100}$

Figure 4.13: Numerical simulation of experimental results for three different types of concrete after moist cured for 3 days
Figure 4.14: Numerical simulation of experimental results for three different types of concrete after moist cured for 28 days
Figure 5.1: Arrangements of demec gauge points along the depth at front (shown only) and rear surfaces and at both ends through the depth as well as width of the prism for measurements of shrinkage strains and total deformations of the prisms in one direction moisture diffusion process
Figure 5.2: Arrangements of demec gauge points along the depth at front (shown only) and rear surfaces of the prisms and at both ends through the depth as well as width of the prisms for measurements of shrinkage strains and total deformations of the prisms in two direction moisture diffusion processes
Figure 5.3: Arrangements of demec gauge points along the depth at front (shown only) and rear surfaces of the prisms and at both ends through the depth as well as width of the prisms for measurements of shrinkage strains along its depth and total deformations of the prisms in three direction moisture diffusion processes
Figure 5.4: Arrangements of demec gauge points along the width at top (shown only) and bottom surfaces of the prisms and at both ends through the depth as well as width of the prisms for measurements of shrinkage strains along its width and total deformations of the prisms in three direction moisture diffusion processes
Figure 5.5: Arrangements of demec gauge points along the depth at front (shown only) and rear surfaces of the prisms and at both ends through the depth as well as width of the prisms for measurements of shrinkage strains along its depth and total deformations of the prisms in four direction moisture diffusion processes

Figure 5.6: Arrangements of demec gauge points along the width at top (shown only) and
bottom surfaces of the prisms and at both ends through the depth as well as width of the
prisms for measurements of shrinkage strains along its width and total deformations of the
prisms in four direction moisture diffusion processes
Figure 6.1: A standard multi-crack segmental analysis
Figure 6.2: Separating elements of RC beam (Concrete element)
Figure 6.3: Separating elements of RC beam (Reinforcement element)
Figure 6.4: Flexural properties (M/ θ , M/ χ and M/EI)
Figure 6.5: Cracked segmental analysis
Figure 6.6: Tension stiffening prism
Figure 6.7: Local deformation of n^{th} segment in prism
Figure 6.8: Tension stiffening analysis
Figure 6.9: Linear shrinkage strain over width and depth (Concrete element) 146
Figure 6.10: Linear shrinkage strain over width and depth (Reinforcement element) 146
Figure 6.11: Non-linear shrinkage strain over depth (Concrete element)
Figure 6.12: Non-linear shrinkage strain over depth (Reinforcement element) 149
Figure 6.13: Non-linear shrinkage strain over width and depth (Concrete element 2b1)

Figure 6.14: Non-linear shrinkage strain over width and depth (Concrete element 2b2)
Figure 6.15: Non-linear shrinkage strain over width and depth (Reinforcement element)
Figure 6.16: Influence of slice number along the width of beam on member deflection
Figure 6.17: Predicted deflection of six beams tested by Gilbert and Nejadi (2004) 156
Figure 6.18: Influence of exposed surfaces on member deflection
<u>List of Tables</u>
Table 5.1: Specimen size details with V/S ratios for one up to four direction diffusion
processes
Table 5.2: Test details for material properties