On the calculation of energy release rate and mode mixity in delaminated laminated beams

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Abstract

A method is presented for the analysis of laminated beams with general stacking sequences and arbitrarily located, through-the-width delaminations. First, the relative displacements and concentrated forces at the crack tip are determined based on classical lamination theory and Timoshenko beam kinematics. Next, new quantities, called crack-tip displacement rates, are defined as the relative displacements per unit increase in crack length. The previously computed quantities are then used to calculate the energy release rate and its mode I and II contributions via an adaptation of the virtual crack closure technique. Results for homogeneous and bimaterial delaminated beams are presented and compared to the predictions of other methods in the literature. Lastly, applications to some non-standard delamination test specimens are illustrated.

Keywords: Composites; Laminated beam; Delamination; Energy release rate; Mixed-mode fracture.

Nomenclature

a	(half) crack length
a, a_1, a_2	extensional compliances of laminate and sublaminates
A, A_1, A_2	extensional stiffnesses of laminate and sublaminates
A, B	cross sections defining the crack-tip segment
b, b_1, b_2	bending-extension coupling compliances of laminate and sublaminates
b_0	unbalance parameter
В	laminate width
B, B ₁ , B ₂	bending-extension coupling stiffnesses of laminate and sublaminates
c, c_1, c_2	shear compliances of laminate and sublaminates
C, C_1, C_2	initial crack-tip point positions
C , C_1 , C_2	shear stiffnesses of laminate and sublaminates
С	initial crack-tip cross section
d , d_1 , d_2	bending compliances of laminate and sublaminates
D	crack-tip point after elementary crack growth
D , D_1 , D_2	bending stiffnesses of laminate and sublaminates
D	crack-tip cross section after elementary crack growth
E_x, E_z	Young's moduli in the x- and z-directions
$E_x^{(1)}, E_x^{(2)},, E_x^{(n)}$	Young's moduli of the layers of a laminated beam
$f_{uM}, f_{uN}, f_{wQ}, f_{\phi M}, f_{\phi N}$	flexibility coefficients
$F_1, F_2,, F_9$	integration constants for the elastic interface model
G	energy release rate
G_c	fracture toughness

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C	and the second second second second second
$G_{ m I},G_{ m II},G_{ m III}$	modal contributions to the energy release rate
G_{zx}	shear modulus in the zx-plane
$G_{zx}^{(1)}, G_{zx}^{(2)},, G_{zx}^{(n)}$	shear moduli of the layers of a laminated beam
h, h_1, h_2	half thicknesses of laminate and sublaminates
H, H_1, H_2	thicknesses of laminate and sublaminates
i	layer index
k_1, k_2, k_3, k_4	coefficients by Schapery and Davidson [17]
k_x, k_z	elastic-interface constants
L	laminated beam length
M, M_1, M_2, M_3	bending moments in laminate and sublaminates
M_1^C, M_2^C	sublaminates bending moments in segment S_C
M_a, M_s	antisymmetric and symmetric bending moments
M_C	crack-tip bending moment
$M_C^{\rm I}, M_C^{\rm II}$	mode I and mode II crack-tip bending moments
\overline{M}	applied bending moment on cantilever beam
$egin{array}{c} M \ \overline{M}_1 \ , \ \overline{M}_2 \end{array}$	
M_1, M_2	applied bending moments in the DCB-UBM test
n	number of layers
N, N_1, N_2, N_3	axial forces in laminate and sublaminates
N_1^{C}, N_2^{C}	sublaminates axial forces in segment S_C
N_a, N_s	antisymmetric and symmetric axial forces
N_C	crack-tip axial force
$N_C^{\rm I}, N_C^{\rm II}$	mode I and mode II crack-tip axial forces
\overline{N}	applied axial force on cantilever beam
0	origin of reference system
P	applied load in the ADCB test
P_d , P_u	downward and upward loads in the MMB test
Q, Q_1, Q_2, Q_3	shear forces in laminate and sublaminates
Q_1^C, Q_2^C	sublaminates shear forces in segment S_C
Q_a, Q_s	antisymmetric and symmetric shear forces
Q_C	crack-tip transverse force
Q_C^{I}, Q_C^{II}	mode I and mode II crack-tip transverse forces
$ar{\mathcal{Q}}$	applied shear force on cantilever beam
S, S ₀	crack-tip segment before and after elementary crack growth
S_C	crack-tip segment with crack-tip forces applied
t_1, t_2	thicknesses of layers in the MMB test with doublers
u, u_1, u_2	axial displacements of laminate and sublaminates
u_{C1}, u_{C2}	axial displacements of points C_1 and C_2
\overline{u}	free-end axial displacement of cantilever beam
w, w_1, w_2	transverse displacements of laminate and sublaminates
w_{C1}, w_{C2}	transverse displacements of points C_1 and C_2
\overline{w}^{C1} , w^{C2}	free-end transverse displacement of cantilever beam
	coordinates
x, y, z	
z_0, z_1, \ldots, z_n	ordinates of the top and bottom surfaces of layers
α	laminate number
$\alpha_1, \alpha_2, \alpha_3$	coefficients of characteristic equation
Δa	crack length increment
$\Delta u, \Delta w, \Delta \phi$	crack-tip relative displacements
\mathcal{E} , \mathcal{E}_1 , \mathcal{E}_2	mid-plane axial strains of laminate and sublaminates

cross sections' rotations of laminate and sublaminates
cross sections' rotations of points C_1 and C_2
free-end cross-section rotation of cantilever beam
shear strains of laminate and sublaminates
sublaminates thickness ratio
curvatures of laminate and sublaminates
parameters used to express the crack-tip forces
dimensionless elastic-interface constants
parameters used to express the crack-tip forces
Poisson's ratio in the zx-plane
crack-tip displacement rates
crack-tip displacement rates in segment S_C
mode I crack-tip displacement rates
mode II crack-tip displacement rates
peak normal interfacial stress
peak tangential interfacial stress
mode-mixity angle
roots of characteristic equation
asymmetric double cantilever beam
asymmetric mixed-mode bending
double cantilever beam
double cantilever beam loaded with uneven bending moments
linear elastic fracture mechanics
mixed-mode bending
virtual crack closure technique

1. Introduction

Delamination is one of the most common and insidious failure modes for composite laminates. Interlaminar cracks propagating at the interface between two adjacent laminae may originate from different causes and lead to global failure of a laminate. A huge number of experimental, analytical, and numerical studies have been devoted to this problem during the last decades [1–5].

The analysis of delamination is commonly conducted in the context of fracture mechanics. In particular, based on linear elastic fracture mechanics (LEFM), the *energy release rate*, G, is the most used parameter to predict the initiation and growth of delamination cracks. On one hand, analytical and numerical models are developed to compute the expected values of G in laboratory specimens or real structural components [6, 7]. On the other hand, experimental techniques are used to assess the critical energy release rate, or *interlaminar fracture toughness*, G_c , of specific materials [8]. The analysis is however complicated by the fact that delamination cracks usually propagate under a mix of the three basic fracture modes (I or opening, II or sliding, and III or tearing) and the fracture toughness depends on the *mode*

mixity (a measure of the relative amounts of fracture modes). The energy release rate turns out to be the sum of three modal contributions, $G_{\rm I}$, $G_{\rm II}$, and $G_{\rm III}$, that the theoretical models should be able to predict. Besides, experimental testing should be carried out to determine the delamination toughness not only in pure fracture modes, but also under mixed-mode conditions. To this aim, several alternative theoretical models and experimental techniques have been developed. Their results, however, appear sometimes conflicting. Ducept et al. [9] conducted tests on symmetric and asymmetric delamination specimens. They analysed their experimental results based on the global method [10], local method [11], and virtual crack closure technique (VCCT) [12, 13], obtaining significant differences. Also, Harvey et al. [14, 15] observed relevant discrepancies between different fracture mode partitioning theories in the interpretation of experimental results.

In this paper, attention is focused on laminated beams containing delaminations under I/II mixed-mode conditions. This problem was analysed in a pioneering work by Pook [16], who used plate theory to calculate the energy release rate and the mode I and II stress intensity factors for several configurations of homogeneous and isotropic beams and plates with midplane delaminations. Williams [10] used beam theory to compute the energy release rate and the mode I and II contributions for homogeneous beams with delaminations located at arbitrary depth. Williams' *global method* assumes that fracture mode I is produced when opposite bending moments act on the two sublaminates into which the laminate is split; besides, mode II is assumed when the delaminating sublaminates have equal curvatures. Such assumptions, however, lead to incorrect results if the sublaminates have different thicknesses. To overcome this drawback, Schapery and Davidson [17] proposed a method based on classical laminated plate theory, where the mode mixity depends on a numerical parameter to be determined via finite element analysis. Independently, Nilsson and Storåkers [18] analysed layered plates and discussed several alternatives for the partitioning of fracture modes.

An elasticity theory approach was instead proposed by Suo and Hutchinson [11], who considered the problem of a semi-infinite crack between two homogeneous and isotropic elastic layers subjected to axial forces and bending moments. According to their *local method*, the energy release rate is computed based on beam theory, while the mode mixity is obtained by solving a plane elasticity problem. Suo [19] extended the local method to orthotropic materials by using the concept of *orthotropy rescaling* [20, 21]. Li *et al.* [22] and Andrews and Massabò [23] added the effects of shear forces for isotropic and orthotropic materials, respectively. In a recent paper, Li *et al.* [24] use classical beam theory and fitting of finite

element results to reproduce Suo and Hutchinson's predictions for the mode mixity. They also present an interesting discussion on the relationship between the concentrated crack-tip forces of the beam-theory model and the singular stress distributions of the elasticity solution.

A different approach to the problem was proposed by Allix and Ladevèze [25], who modelled delamination by considering an elastic interface between the separating layers of a laminate. Subsequently, *elastic-interface models* were used many other authors [26–35]. Elastic-interface models can be regarded as particular cases of the more general class of *cohesive-zone models* [36–39]. Based on such models, the work of cohesive tractions (corresponding to the energy release rate of LEFM) is calculated based on the values of the interfacial stresses at the delamination front.

Most methods in the literature are limited to homogeneous or bimaterial beams. Only a few authors consider laminated beams. Amongst these, Carlsson *et al.* [40] and Lundsgaard–Larsen *et al.* [41] used classical lamination theory to analyse sandwich beams. Rask and Sørensen [42] proposed evaluating the J integral for multidirectional laminated specimens from the measures of beam curvatures and bending moments. Wang and Harvey and coworkers proposed an orthogonal decomposition method to partition fracture modes in laminated beams [43–45].

The aim of this paper is to retrieve the spirit of Williams [10] and set up a method to calculate the energy release rate and mode mixity based solely on beam theory. Williams' assumptions on fracture mode partitioning are replaced by new assumptions, which hold also for laminates with delaminations not placed on the mid-plane. Furthermore, the proposed method applies not only to homogeneous and isotropic beams, but also to laminated beams with general stacking sequences, including those with bending-extension coupling.

The mechanical model underlying the present method is on purpose kept as simple as possible in order to obtain simple analytical expressions for all the quantities of interest. This is done at the risk of affecting the accuracy of results, but in the hope that the proposed approach could help to shed light on a subject, which still raises some controversy in the literature. For instance, it is assumed that the delaminating sublaminates are perfectly clamped at the crack-tip cross section. Indeed, it is well known that taking into account the so-called *root rotations* [46] could be crucial to get accurate predictions of experimental test results (in particular, in terms of the displacements and deformations of laboratory specimens). Moreover, the present method is based on Timoshenko beam kinematics [47], which assumes that plane cross sections remain plane and undeformed, but may rotate freely with respect to the beam centreline. Thus, the present model unavoidably disregards some strain

contributions (for instance, those related to the Poisson effect), which would require an elasticity approach to be taken into account. As a consequence, it is expected that the results of the present method – in particular, for the mode mixity – may differ from those of more complex analytical and numerical methods in the literature. Nonetheless, the present method can serve as a benchmark for the validation of more complex methods in the limit case where classical beam theory assumptions are fulfilled (see, in what follows, the comparison with the elastic-interface model). Advantages of the proposed method compared to other ones include also the following:

- (i) laminated beams with arbitrary stacking sequences can be analysed, including those with bending-extension coupling; instead, most methods in the literature are limited to homogeneous and isotropic beams;
- (ii) analytical expressions are obtained for the energy release rate and its mode I and II contributions; there is no need to resort to finite element analyses, numerical calculations, or numerical parameters given in tabulated form;
- (iii) shear forces and the effects of shear deformability are taken into account, which may be particularly relevant for composite laminates;
- (iv) the method can be used for the design and data reduction of non-standard delamination toughness tests, *i.e.* tests on multidirectional laminated specimens with possibly asymmetric delaminations.

The structure of the paper is the following. In Section 2, a laminated beam is considered with a general stacking sequence and an arbitrarily located, through-the-width delamination. Attention is focused on a crack-tip segment, namely an infinitesimally short portion of the beam including one of the delamination crack tips. Based on classical lamination theory [48] and Timoshenko beam kinematics [47], the relative displacements occurring at the crack tip (after an elementary crack growth) and the concentrated forces acting at the crack tip (prior to the elementary crack growth) are determined. Furthermore, new quantities, called *crack-tip displacement rates*, are defined as the relative displacements per unit increase in crack length [49]. In Section 3, the crack-tip displacement rates are used to calculate the energy release rate and its modal contributions via an adaptation of the virtual crack closure technique [13, 50]. Section 4 presents results for homogeneous and bimaterial delaminated beams. The present method is compared to the elastic-interface model [35] and the local method [11, 22]. Also, applications to some non-standard delamination test specimens with general stacking sequences are illustrated. Discussion of the obtained results follows in Section 5.

2. Static and kinematic analysis of a crack-tip segment

2.1. Delaminated laminated beam and crack-tip segment

Let us consider a laminated beam of length L, thickness H=2h, and width B, with an arbitrarily located, through-the-width delamination of length 2a (Fig. 1). A Cartesian reference system Oxyz is fixed with the origin at the geometric centre of one of the end sections of the beam, the x-axis in the longitudinal direction, the y- and z-axes aligned with the width and height directions, respectively. It is assumed that geometry, material properties, restraints, and loads are such that the beam can be modelled as a planar beam in the Ozx-plane. Within such assumptions, the beam can be made of layers of different materials and thicknesses and/or fibre-reinforced laminae with different orientations. Quite general stacking sequences are allowed by the present model, including those with bending-extension coupling, provided that the laminate and the sublaminates resulting from delamination are free from out-of-plane effects (e.g. torsion, out-of-plane shear, etc.). As an example, many symmetric and non-symmetric angle-ply and cross-ply laminates can be analysed.

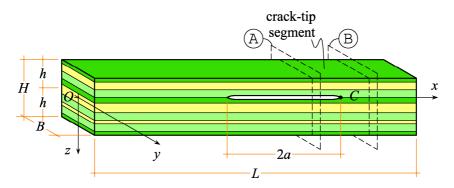


Fig. 1. Delaminated laminated beam and crack-tip segment.

Attention is now restricted to a *crack-tip segment*, defined as an infinitesimally short portion of the beam included between two cross sections, A and B, located immediately behind and ahead of the delamination crack tip, C (Fig. 2). The crack-tip segment can be thought of as an assemblage of three short laminates, connected to each other at the crack-tip cross section, C (Fig. 2a): the upper and lower parts of the delaminated beam, denoted as sublaminates No. 1 and 2, respectively, and the undelaminated part, denoted as laminate No. 3. The sublaminate thicknesses are indicated with H_1 and H_2 . Besides, in order to simplify some of the forthcoming expressions, the half thicknesses, $h_1 = H_1/2$ and $h_2 = H_2/2$, are introduced.

According to classical lamination theory [48], a laminated beam can be modelled as a homogeneous beam, provided that equivalent stiffnesses are defined (see also Appendix A).

Hence, let A_{α} , B_{α} , C_{α} , and D_{α} respectively denote the *extensional stiffness*, *bending-extension coupling stiffness*, *shear stiffness*, and *bending stiffness* per unit width of the three laminates (Fig. 2b). Here, the subscript $\alpha \in \{1, 2, 3\}$ refers to the laminate number.

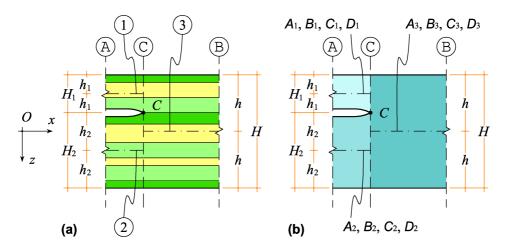


Fig. 2. Crack-tip segment: (a) laminated beam; (b) assemblage of equivalent homogeneous beams.

The internal forces are defined as the resultant forces acting on the laminate centrelines. Accordingly, the *axial force*, *shear force*, and *bending moment* are denoted with N_{α} , Q_{α} , and M_{α} , respectively (Fig. 3). It is assumed that no external loads are applied on the crack-tip segment, which therefore will be in equilibrium under the action of the internal forces only. In the limit case of a crack-tip segment of vanishing length, the internal forces in laminate No. 3 turn out to be

$$N_3 = N_1 + N_2$$
, $Q_3 = Q_1 + Q_2$, and $M_3 = M_1 + M_2 - N_1 h_2 + N_2 h_1$. (1)

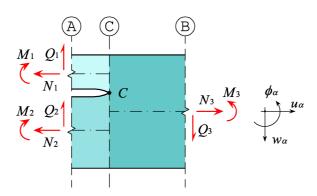


Fig. 3. Internal forces and generalised displacement convention.

Let u_{α} and w_{α} respectively denote the *axial* and *transverse displacements* of the laminate centreline, and ϕ_{α} their *cross-section rotation*, positive if counter-clockwise (Fig. 3). In line with the Timoshenko beam theory [47], the *axial strain*, *shear strain*, and *curvature* are

respectively defined as

$$\mathcal{E}_{\alpha} = \frac{du_{\alpha}}{dx}, \quad \gamma_{\alpha} = \frac{dw_{\alpha}}{dx} + \phi_{\alpha}, \quad \text{and} \quad \kappa_{\alpha} = \frac{d\phi_{\alpha}}{dx}.$$
 (2)

According to classical lamination theory [48], the internal forces and strain measures can be related as follows:

$$N_{\alpha} = B (A_{\alpha} \varepsilon_{\alpha} + B_{\alpha} \kappa_{\alpha}), \quad Q_{\alpha} = B C_{\alpha} \gamma_{\alpha}, \quad \text{and} \quad M_{\alpha} = B (B_{\alpha} \varepsilon_{\alpha} + D_{\alpha} \kappa_{\alpha}),$$
 (3)

or, by inversion of Eqs. (3),

$$\varepsilon_{\alpha} = \frac{1}{B} (\mathbf{a}_{\alpha} N_{\alpha} + \mathbf{b}_{\alpha} M_{\alpha}), \quad \gamma_{\alpha} = \frac{1}{B} \mathbf{c}_{\alpha} Q_{\alpha}, \quad \text{and} \quad \kappa_{\alpha} = \frac{1}{B} (\mathbf{b}_{\alpha} N_{\alpha} + \mathbf{d}_{\alpha} M_{\alpha}), \tag{4}$$

where

$$a_{\alpha} = \frac{D_{\alpha}}{A_{\alpha}D_{\alpha} - B_{\alpha}^2}, \quad b_{\alpha} = -\frac{B_{\alpha}}{A_{\alpha}D_{\alpha} - B_{\alpha}^2}, \quad c_{\alpha} = \frac{1}{C_{\alpha}}, \quad \text{and} \quad d_{\alpha} = \frac{A_{\alpha}}{A_{\alpha}D_{\alpha} - B_{\alpha}^2}$$
 (5)

are the *extensional compliance*, bending-extension coupling compliance, shear compliance, and bending compliance, respectively. Note that if the sublaminates are uncoupled (which is the case for homogeneous and unidirectional laminates), $B_{\alpha} = 0$ and $b_{\alpha} = 0$, so that Eqs. (3)–(5) can be conveniently simplified.

2.2. Crack-tip relative displacements

Let us now consider an *elementary crack growth*, whereby the delamination crack increases its length by a small amount, Δa , under constant external loads. The initial cracktip segment, S, where the crack tip is located at point C (Fig. 4a), transforms into a new segment, S_0 , where the crack tip moves to a new position, D, while point C splits into two points, C_1 and C_2 , belonging to sublaminates No. 1 and 2, respectively (Fig. 4b). The cross section passing through point D is denoted with D. It is assumed that the internal forces acting on cross sections A and B do not change appreciably with the elementary crack growth, but points C_1 and C_2 generally undergo non-zero relative displacements (Fig. 5). The *crack-tip relative displacements* are defined as follows:

$$\Delta u = u_{C2} - u_{C1} = (u_2 - \phi_2 h_2) - (u_1 + \phi_1 h_1),$$

$$\Delta w = w_{C2} - w_{C1} = w_2 - w_1, \text{ and}$$

$$\Delta \phi = \phi_{C2} - \phi_{C1} = \phi_2 - \phi_1,$$
(6)

where u_{C1} and u_{C2} are the axial displacements of points C_1 and C_2 , respectively, w_{C1} and

 w_{C2} are the corresponding transverse displacements, and ϕ_{C1} and ϕ_{C2} are the cross-section rotations. In particular, Δu is the *crack-tip sliding displacement*, Δw is the *crack-tip opening displacement*, and $\Delta \phi$ is the *crack-tip relative rotation*.

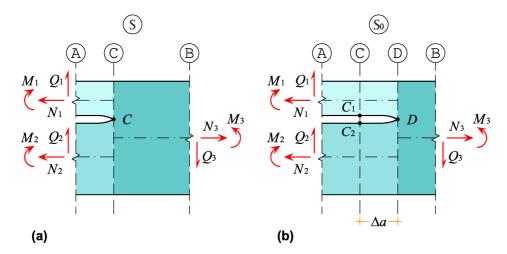


Fig. 4. Elementary crack growth: (a) initial crack-tip segment; (b) new crack-tip segment.

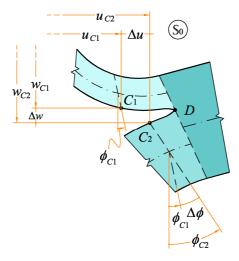


Fig. 5. Deformed crack-tip segment after the elementary crack growth.

The crack-tip relative displacements are here evaluated by assuming that the two delaminating sublaminates deform as cantilever beams of length Δa , clamped at cross section D and loaded by the internal forces, N_1 , Q_1 , M_1 and N_2 , Q_2 , M_2 , at the cross sections passing through points C_1 and C_2 . From the solution of the auxiliary problem of a laminated cantilever beam loaded at its end (see Appendix B), it is easily shown that

$$\Delta u \cong \frac{1}{B} [(a_1 + b_1 h_1) N_1 - (a_2 - b_2 h_2) N_2 + (b_1 + d_1 h_1) M_1 - (b_2 - d_2 h_2) M_2] \Delta a,$$

$$\Delta w \cong \frac{1}{B} (c_1 Q_1 - c_2 Q_2) \Delta a, \text{ and}$$

$$\Delta \phi \cong \frac{1}{B} (b_1 N_1 - b_2 N_2 + d_1 M_1 - d_2 M_2) \Delta a,$$
(7)

where powers of Δa of order higher than one have been neglected.

The clamped-end assumption for the sublaminates is a quite strong assumption, that will affect the results. Nevertheless, this assumption is made here for the sake of simplicity. A possible improvement to the model could be obtained by considering the root rotations, for instance, by introducing rotational springs at the sublaminate end sections [46].

2.3. Crack-tip displacement rates

The *crack-tip displacement rates* are defined as the crack-tip relative displacements per unit increase in crack length [49]:

$$\rho_{u} = \lim_{\Delta a \to 0} \frac{\Delta u}{\Delta a}, \quad \rho_{w} = \lim_{\Delta a \to 0} \frac{\Delta w}{\Delta a}, \quad \text{and} \quad \rho_{\phi} = \lim_{\Delta a \to 0} \frac{\Delta \phi}{\Delta a}. \tag{8}$$

In particular, ρ_u is the *crack-tip sliding displacement rate*, ρ_w is the *crack-tip opening displacement rate*, and ρ_{ϕ} is the *crack-tip relative rotation rate*. By substituting Eqs. (7) into (8), the crack-tip displacement rates turn out to be

$$\rho_{u} = \frac{1}{B} [(\mathbf{a}_{1} + \mathbf{b}_{1}h_{1})N_{1} - (\mathbf{a}_{2} - \mathbf{b}_{2}h_{2})N_{2} + (\mathbf{b}_{1} + \mathbf{d}_{1}h_{1})M_{1} - (\mathbf{b}_{2} - \mathbf{d}_{2}h_{2})M_{2}],$$

$$\rho_{w} = \frac{1}{B} (\mathbf{c}_{1}Q_{1} - \mathbf{c}_{2}Q_{2}), \text{ and}$$

$$\rho_{\phi} = \frac{1}{B} (\mathbf{b}_{1}N_{1} - \mathbf{b}_{2}N_{2} + \mathbf{d}_{1}M_{1} - \mathbf{d}_{2}M_{2}).$$
(9)

By substituting Eqs. (3) into (9), and expressing the laminate stiffnesses as functions of the compliances through inversion of Eqs. (5), the following expressions are also obtained:

$$\rho_{u} = \varepsilon_{1} - \varepsilon_{2} + \kappa_{1}h_{1} + \kappa_{2}h_{2}, \quad \rho_{w} = \gamma_{1} - \gamma_{2}, \quad \text{and} \quad \rho_{\phi} = \kappa_{1} - \kappa_{2}, \tag{10}$$

which show that the crack-tip displacement rates are a measure of the difference in strains at the crack tip between the two delaminating sublaminates. The crack-tip displacement rates describe the kinematics of an elementary crack growth. They will be used in the following to calculate the energy release rate and partition fracture modes. It is interesting to note that the first of Eqs. (10) – by recalling also Eqs. (8) – turns out to be very similar to the expression

used by Carlsson *et al.* [40] to evaluate the tangential relative displacement of the crack faces of a double cantilever beam (DCB) sandwich specimen. Also, it is worth mentioning that similar quantities have been tacitly used by Schapery and Davidson [17].

2.4. Crack-tip forces

The initial crack-tip segment S (prior to the elementary crack growth) can be obtained by superimposing the segments S_0 (after the elementary crack growth) and S_C , where suitable self-equilibrated pairs of axial forces, N_C , shear forces, Q_C , and bending moments, M_C , are applied at points C_1 and C_2 (Fig. 6). Such *crack-tip forces* can be regarded as the forces acting at the crack tip prior to crack propagation. These are different from the internal forces acting on the crack-tip segment prior to crack growth, which are applied on the sublaminate centrelines. The axial and bending components, N_C and M_C , were introduced by Schapery and Davidson [17]. Harvey and Wang [44] considered also the shear component, Q_C . Li et al. [24] interpreted the crack-tip forces as the resultants of the singular stress fields at the crack tip of the plane elasticity solution.

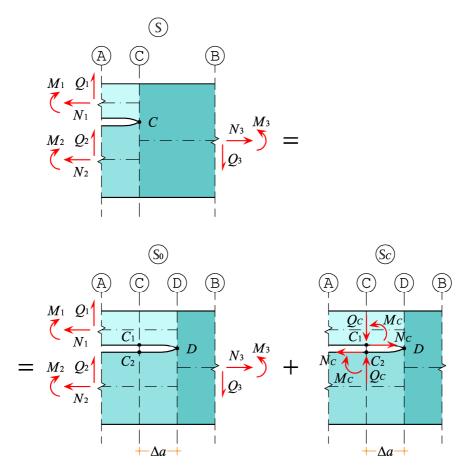


Fig. 6. The initial crack-tip segment S as the superposition of segments S_0 and S_C .

The intensities of the crack-tip forces can be determined by observing that the crack-tip

relative displacements resulting from the superposition of segments S_0 and S_C should all be equal to zero. This is equivalent to impose the annihilation of the superimposed crack-tip displacement rates. Now, the internal forces in segment S_C turn out to be

$$N_1^C = -N_C, \quad Q_1^C = -Q_C, \quad \text{and} \quad M_1^C = -M_C - N_C h_1;$$

 $N_2^C = N_C, \quad Q_2^C = Q_C, \quad \text{and} \quad M_2^C = M_C - N_C h_2.$ (11)

Hence, by substituting Eqs. (11) into (9), the crack-tip displacement rates in \mathbb{S}_{c} ,

$$\rho_u^C = -f_{uN}N_C - f_{uM}M_C, \quad \rho_w^C = -f_{wQ}Q_C, \quad \text{and} \quad \rho_\phi^C = -f_{\phi N}N_C - f_{\phi M}M_C, \tag{12}$$

can be computed. In Eqs. (12) the *flexibility coefficients*,

$$f_{uN} = \frac{1}{B} (\mathbf{a}_{1} + \mathbf{a}_{2} + 2\mathbf{b}_{1}h_{1} - 2\mathbf{b}_{2}h_{2} + \mathbf{d}_{1}h_{1}^{2} + \mathbf{d}_{2}h_{2}^{2}),$$

$$f_{uM} = f_{\phi N} = \frac{1}{B} (\mathbf{b}_{1} + \mathbf{b}_{2} + \mathbf{d}_{1}h_{1} - \mathbf{d}_{2}h_{2}),$$

$$f_{wQ} = \frac{1}{B} (\mathbf{c}_{1} + \mathbf{c}_{2}), \text{ and}$$

$$f_{\phi M} = \frac{1}{B} (\mathbf{d}_{1} + \mathbf{d}_{2}),$$

$$(13)$$

have been introduced. The flexibility coefficients describe the deformability of the crack-tip segment. In passing, it is noted that similar quantities have been defined within a finite element framework in a revised formulation of the VCCT [51].

To annihilate the crack-tip relative displacements in S, it is necessary that

$$\rho_u + \rho_u^C = 0, \quad \rho_w + \rho_w^C = 0, \quad \text{and} \quad \rho_\phi + \rho_\phi^C = 0.$$
 (14)

Hence, by substituting Eqs. (12) into (14), the crack-tip displacement rates,

$$\rho_u = f_{uN} N_C + f_{uM} M_C, \quad \rho_w = f_{wQ} Q_C, \quad \text{and} \quad \rho_\phi = f_{\phi N} N_C + f_{\phi M} M_C,$$
 (15)

are determined as functions of the crack-tip forces. It is noted that the coefficient f_{uM} (equal to $f_{\phi N}$ by virtue of Betti's reciprocity theorem) introduces a coupling between M_C and ρ_u and, vice versa, between N_C and ρ_{ϕ} . This coupling vanishes for symmetrically delaminated beams with uncoupled sublaminates (for which $b_1 = b_2 = 0$ and $d_1h_1 = d_2h_2$) and, more generally, for delaminated laminated beams such that $b_1 + d_1h_1 = -b_2 + d_2h_2$.

Lastly, by inversion of Eqs. (15), the crack-tip forces,

$$N_{C} = \frac{f_{\phi M} \rho_{u} - f_{uM} \rho_{\phi}}{f_{uN} f_{\phi M} - f_{uM} f_{\phi N}}, \quad Q_{C} = \frac{\rho_{w}}{f_{wO}}, \quad \text{and} \quad M_{C} = \frac{f_{uN} \rho_{\phi} - f_{\phi N} \rho_{u}}{f_{uN} f_{\phi M} - f_{uM} f_{\phi N}},$$
(16)

are obtained as functions of crack-tip displacement rates. The crack-tip forces can also be expressed as functions of the internal forces in the sublaminates, as explained in Appendix C.

3. Energy release rate and fracture mode partitioning

3.1. Energy release rate

The *energy release rate*, G, is the decrease of potential energy of the system spent in the crack growth process, per unit area of new surface created. According to Irwin [52], the energy spent to produce an extension of the crack is equal to the work done to close the crack by the forces acting on the crack faces prior to crack extension. In analogy with the VCCT used in finite element analyses [12, 13], the work of closure can be defined as the work done by the crack-tip forces on the corresponding relative displacements. Hence,

$$G = \lim_{\Delta a \to 0} \frac{1}{2B\Delta a} (N_C \Delta u + Q_C \Delta w + M_C \Delta \phi). \tag{17}$$

Schapery and Davidson [17] used an equation similar to Eq. (17), but without the shear term, $Q_c \Delta w$. This term is needed here because, according to the Timoshenko beam theory [47], there are three independent, generalised displacement components.

By recalling the definition of the crack-tip displacement rates, Eqs. (8), the energy release rate becomes

$$G = \frac{1}{2B} (N_C \rho_u + Q_C \rho_w + M_C \rho_\phi). \tag{18}$$

Furthermore, by substituting Eqs. (15) into (18), the energy release rate can be expressed in terms of the crack-tip forces,

$$G = \frac{1}{2B} \left[f_{uN} N_C^2 + (f_{\phi N} + f_{uM}) N_C M_C + f_{\phi M} M_C^2 + f_{wQ} Q_C^2 \right], \tag{19}$$

or, by substituting Eqs. (16) into (18), in terms of the crack-tip displacement rates,

$$G = \frac{1}{2B} \left[\frac{f_{\phi M} \rho_u^2 - (f_{\phi N} + f_{uM})\rho_u \rho_\phi + f_{uN} \rho_\phi^2}{f_{uN} f_{\phi M} - f_{\phi N} f_{uM}} + \frac{\rho_w^2}{f_{wQ}} \right].$$
 (20)

Eqs. (18)–(20) offer three equivalent ways to compute the energy release rate for a given problem. In practice, once the internal forces at the cross sections behind and above of the

crack tip are known, Eqs. (9) furnish the crack-tip displacement rates. Then, the crack-tip forces can be determined from Eqs. (16) and *G* obtained via Eqs. (18) or (19). Alternatively, the energy release rate can be directly obtained from Eq. (20).

3.2. Fracture mode partitioning

Delamination cracks generally propagate under a mix of fracture modes. In I/II mixed-mode problems, the energy release rate is the sum of two modal contributions,

$$G = G_{\mathbf{I}} + G_{\mathbf{II}}, \tag{21}$$

respectively corresponding to fracture modes I (opening) and II (sliding). By inspection of Eqs. (18)–(20), it is not obvious how to identify the contributions of fracture modes I and II. On one hand, it is intuitive that the terms depending on Q_C and ρ_w should contribute only to G_I (incidentally, such terms are relevant only if shear deformability is taken into account); on the other hand, the terms depending on N_C , M_C and ρ_u , ρ_ϕ are strongly tied one another and, hence, contribute to both fracture modes. It is also noted that this coupling depends on the flexibility coefficient $f_{uM} = f_{\phi N}$, which is generally non-zero in the case of asymmetric delamination cracks.

Here, the partitioning of fracture modes is based on the following simple kinematic assumption: pure mode I fracture occurs when the crack-tip sliding displacement, Δu , is equal to zero (or, equivalently, when the corresponding crack-tip displacement rate $\rho_u = 0$). The modal contributions to the energy release rate can then be computed in a way similar to that proposed elsewhere for the VCCT [50].

Let us imagine that the relative displacements between the crack faces are closed in an ideal two-step process. In the first ideal step, corresponding to the mode II contribution, suitable axial forces, $N_C^{\rm II}$, are applied at points C_1 and C_2 , while neither shear forces, nor bending moments are applied at the same points (Fig. 7a). The intensity of $N_C^{\rm II}$ should be such as to exactly close the gap in the *x*-direction, Δu , produced by the elementary crack growth. In practice, the first of Eqs. (15) can be used to determine $N_C^{\rm II}$ as the axial force producing a crack-tip sliding displacement rate equal to ρ_u . To sum up, in the first ideal step the following crack-tip forces are applied:

$$N_C^{\rm II} = \frac{\rho_u}{f_{uN}}, \quad Q_C^{\rm II} = 0, \quad \text{and} \quad M_C^{\rm II} = 0.$$
 (22)

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Substitution of Eqs. (22) into (15) shows that the aforementioned crack-tip forces produce the following crack-tip displacement rates:

$$\rho_u^{\text{II}} = \rho_u, \quad \rho_w^{\text{II}} = 0, \quad \text{and} \quad \rho_\phi^{\text{II}} = \frac{f_{\phi N}}{f_{uN}} \rho_u.$$
 (23)

Eqs. (23) indicate that in the first ideal step, the gap between the crack faces in the x-direction is completely closed, the gap in the z-direction is left unchanged, while the relative rotation may be partly decreased or increased depending on the sign of the coupling flexibility coefficient, $f_{\phi N}$.

In the second ideal step, corresponding to mode I, the remainders of the crack-tip forces,

$$N_C^{\rm I} = N_C - N_C^{\rm II}, \quad Q_C^{\rm I} = Q_C, \quad \text{and} \quad M_C^{\rm I} = M_C,$$
 (24)

are applied at points C_1 and C_2 (Fig. 7b). By substituting Eqs. (24) into (15) and recalling Eqs. (16) and (22), the crack-tip displacement rates produced by such crack-tip forces are determined as follows:

$$\rho_{u}^{I} = 0, \quad \rho_{w}^{I} = \rho_{w}, \quad \text{and} \quad \rho_{\phi}^{I} = \rho_{\phi} - \frac{f_{\phi N}}{f_{e N}} \rho_{u}.$$
 (25)

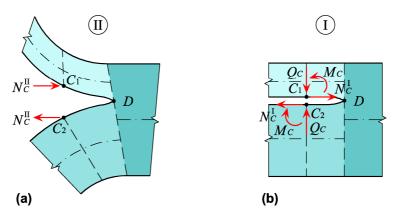


Fig. 7. Crack closure forces: (a) mode II; (b) mode I.

According to Eqs. (25), in the second ideal step, the gap in the *x*-direction (already closed in the first step) is left unchanged, while the gap in the *z*-direction is closed. Furthermore, from Eqs. (23) and (25) it follows that $\rho_{\phi}^{I} + \rho_{\phi}^{II} = \rho_{\phi}$, which means that the relative rotation between the crack faces vanishes as a result of the application of the crack-tip forces in the two ideal steps. Thus, the two ideal steps correspond to the complete closure of the crack.

The modal contributions to the energy release rate can now be determined by considering the work of closure done by the systems of crack-tip forces, Eqs. (22) and (24), on the

corresponding crack-tip displacement rates, Eqs. (23) and (25). To this aim, it is worth noting that the two systems of crack-tip forces are energetically orthogonal, which means that their mutual work is zero [50, 51]. Note also that N_C^{II} is in general distinct from N_C , but if $f_{uM} = f_{\phi N} = 0$ then $\rho_u = f_{uN} N_C$ and $N_C^{II} = N_C$. Eqs. (13) show that this happens when the unbalance parameter $b_0 = b_1 + b_2 + d_1 h_1 - d_2 h_2 = 0$. This condition includes, as special cases, homogeneous and unidirectional laminates with symmetrically located delaminations.

By substituting Eqs. (22)–(25) into (18), and recalling Eqs. (16), the energy release rate contributions corresponding to fracture modes I and II,

$$G_{\rm I} = \frac{1}{2B} \left[\frac{1}{f_{uN}} \frac{(f_{uN}\rho_{\phi} - f_{\phi N}\rho_{u})^{2}}{f_{uN}f_{\phi M} - f_{uM}f_{\phi N}} + \frac{\rho_{w}^{2}}{f_{wQ}} \right] \quad \text{and} \quad G_{\rm II} = \frac{1}{2B} \frac{\rho_{u}^{2}}{f_{uN}}, \tag{26}$$

are determined. By substituting Eqs. (15) into (26), the modal contributions can also be expressed as functions of the crack-tip forces:

$$G_{\rm I} = \frac{1}{2B} \left(\frac{f_{uN} f_{\phi M} - f_{uM} f_{\phi N}}{f_{uN}} M_C^2 + f_{wQ} Q_C^2 \right) \quad \text{and} \quad G_{\rm II} = \frac{1}{2B} \frac{(f_{uN} N_C + f_{uM} M_C)^2}{f_{uN}}. \tag{27}$$

Eqs. (26) and (27) show that both $G_{\rm I}$ and $G_{\rm II}$ are non-negative quantities, in line with their physical meaning [50, 51]. Furthermore, the above equations reveal the conditions for having pure fracture modes: pure mode I ($G_{\rm II}=0$) is obtained when $\rho_u=0$ (as per the initial assumption on fracture mode partitioning), while pure mode II ($G_{\rm I}=0$) occurs when both $M_C=0$ and $Q_C=0$ (the latter condition is relevant only if shear deformability is taken into account). It is worth noting that the above conditions coincide with those obtained by Harvey and Wang [44] by using the Timoshenko beam theory [47]. Also, the above result is in line with the observation by Li *et al.* [24] that pure mode I cannot correspond to $N_C=0$, as this resultant force could vanish even if the distributed shear stresses at the crack tip are non-zero.

By recalling Eqs. (10), it is also possible to determine expressions for $G_{\rm I}$ and $G_{\rm II}$ as functions of the strain measures in the delaminating sublaminates,

$$G_{\rm I} = \frac{1}{2B} \left\{ \frac{1}{f_{uN}} \frac{\left[f_{uN} (\kappa_1 - \kappa_2) - f_{\phi N} (\varepsilon_1 - \varepsilon_2 + \kappa_1 h_1 + \kappa_2 h_2) \right]^2}{f_{uN} f_{\phi M} - f_{uM} f_{\phi N}} + \frac{(\gamma_1 - \gamma_2)^2}{f_{wQ}} \right\} \quad \text{and}$$

$$G_{\rm II} = \frac{1}{2B} \frac{(\varepsilon_1 - \varepsilon_2 + \kappa_1 h_1 + \kappa_2 h_2)^2}{f_{uN}}.$$
(28)

Eqs. (28) shed some light on the discussion about the relationship between modal

contributions and curvatures of the delaminating sublaminates [16–18, 42]. In particular, it is apparent that equal curvatures ($\kappa_1 = \kappa_2$) do not necessarily imply mode II fracture. On the other hand, opposite curvatures ($\kappa_1 = -\kappa_2$) correspond to mode I fracture, provided that the sublaminates have also equal axial strains ($\varepsilon_1 = \varepsilon_2$) and thicknesses ($h_1 = h_2$).

In I/II mixed-mode fracture problems, the mode mixity can be characterised by the phase angle of the complex stress-intensity factor [6]. Here, the *mode-mixity angle* is defined as

$$\psi = \pm \arctan \sqrt{\frac{G_{II}}{G_{I}}},\tag{29}$$

where the \pm sign has to be taken equal to the sign of $N_{\scriptscriptstyle C}^{\scriptscriptstyle \rm II}$ or $\rho_{\scriptscriptstyle u}$.

4. Results

4.1. Homogeneous beams

For homogeneous beams, the sublaminate stiffnesses are

$$A_{\alpha} = E_x H_{\alpha}, \quad B_{\alpha} = 0, \quad C_{\alpha} = \frac{5}{6} G_{zx} H_{\alpha}, \quad \text{and} \quad D_{\alpha} = \frac{1}{12} E_x H_{\alpha}^3,$$
 (30)

where E_x and G_{zx} respectively are the longitudinal Young's modulus and shear modulus of the material. The subscript $\alpha=1$ denotes the upper sublaminate, $\alpha=2$ denotes the lower one. In what follows, also the elastic modulus in the transverse direction of the beam, E_z , will be used (for isotropic materials, $E_x=E_z$). By substituting Eqs. (30) into (5), (9), (13), and (20), the energy release rate,

$$G = \frac{1}{2B^{2}E_{x}} \frac{H_{1}^{3} + H_{2}^{3}}{H^{3}} \frac{H_{1}H_{2}}{H} \left(\frac{N_{1}}{H_{1}} - \frac{N_{2}}{H_{2}} \right)^{2} + \frac{3}{5B^{2}G_{zx}} \frac{H_{1}H_{2}}{H} \left(\frac{Q_{1}}{H_{1}} - \frac{Q_{2}}{H_{2}} \right)^{2} + \frac{6}{B^{2}E_{x}} \left[\frac{M_{1}^{2}}{H_{1}^{3}} + \frac{M_{2}^{2}}{H_{2}^{3}} - \frac{(M_{1} + M_{2})^{2}}{H^{3}} \right],$$

$$(31)$$

is obtained as a function of the internal forces in the delaminating sublaminates. Likewise, from Eqs. (26), the mode I and II contributions,

$$G_{\rm I} = \frac{3}{8B^2 E_x} (H_1 - H_2)^2 \frac{H_1 H_2}{H^3} \left(\frac{N_1}{H_1} - \frac{N_2}{H_2} \right)^2 + \frac{3}{5B^2 G_{zx}} \frac{H_1 H_2}{H} \left(\frac{Q_1}{H_1} - \frac{Q_2}{H_2} \right)^2 + \frac{3}{2B^2 E_x} \frac{H_1 H_2}{H^3} \left(\frac{3H_1 + H_2}{H_1^2} M_1 - \frac{H_1 + 3H_2}{H_2^2} M_2 \right)^2 \quad \text{and}$$

$$G_{\rm II} = \frac{1}{8B^2 E_x} \frac{H_1 H_2}{H} \left(\frac{N_1}{H_1} - \frac{N_2}{H_2} \right)^2 + \frac{9}{2B^2 E_x} \frac{H_1 H_2}{H} \left(\frac{M_1}{H_1^2} + \frac{M_2}{H_2^2} \right)^2,$$

$$(32)$$

are determined as functions of the internal forces.

It can be shown that Eq. (31) is equivalent to the expressions by Williams [10], except for a term stemming from the moment balance of axial forces. However, the modal contributions given by Eqs. (32) are different from Williams' expressions because of the different underlying assumptions.

4.1.1. Load cases

In the following Sections, results for homogeneous beams will be presented and compared to the predictions of the elastic-interface model, where the two delaminating sublaminates are modelled as Timoshenko beams connected by an elastic interface [35] (see also Appendix D). Furthermore, the predictions of the local method [11, 22] will be shown for reference. In order to consider all possible load conditions, the general load system acting on the crack-tip segment is decomposed into the sum of three elementary load conditions, respectively corresponding to axial forces, shear forces, and bending moments applied to the delaminating sublaminates (Fig. 8). In turn, the elementary load conditions are decomposed into the sum of three symmetric load cases, where

$$N_s = \frac{N_1 + N_2}{2}, \quad Q_s = \frac{Q_1 - Q_2}{2}, \quad \text{and} \quad M_s = \frac{M_1 - M_2}{2},$$
 (33)

and three antisymmetric load cases, where

$$N_a = \frac{N_1 - N_2}{2}, \quad Q_a = \frac{Q_1 + Q_2}{2}, \quad \text{and} \quad M_a = \frac{M_1 + M_2}{2}.$$
 (34)

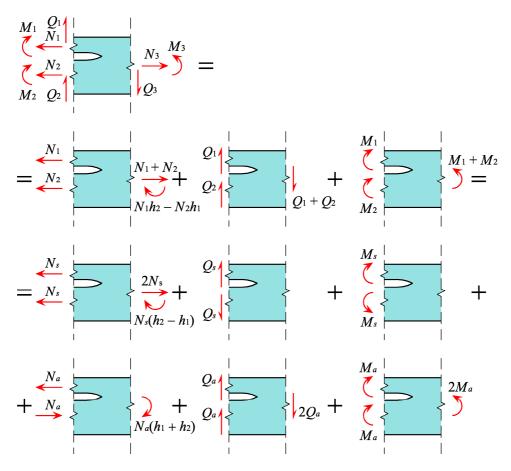


Fig. 8. Load decomposition for homogeneous beams.

It is worth noting that the symmetric and antisymmetric load cases correspond to fracture modes I and II, respectively, for laminates with symmetrically placed delaminations, but result in mixed-mode fracture in general. In what follows, the effects of axial forces, shear forces, and bending moments will be examined separately. For the sake of illustration, the Poisson's ratio is assumed as $V_{zx} = 0.3$ and the shear modulus computed as $G_{zx} = E_x / [2(1+V_{zx})]$.

4.1.2. Effect of axial forces

Figure 9 shows the energy release rate, G, produced by axial forces as a function of the sublaminate thickness ratio, $\eta = H_1/H_2$. As customary in the literature [22, 23], G is non-dimensionalised with respect to the applied load and the geometric and material parameters of the problem in order to make results independent of units of measurement. Without loss of generality, here and in the following, attention is restricted to the case $\eta \in [0,1]$. For both symmetric (Fig. 9a) and antisymmetric (Fig. 9b) axial forces, the energy release rate turns out to be a decreasing function of η in the considered interval. The elastic-interface model [35] and local method [11] furnish for G the same results of the present method. Note that for the

symmetric load case, G = 0 for $\eta = 1$, which means that uniform traction does not produce fracture propagation in a symmetrically delaminated laminate.

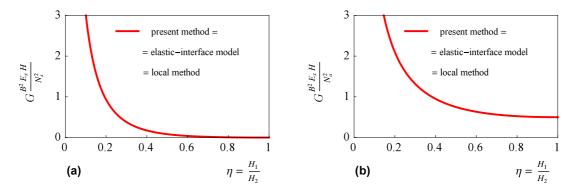


Fig. 9. Dimensionless energy release rate due to (a) symmetric and (b) antisymmetric axial forces.

Figure 10 shows the mode-mixity angle, ψ , produced by axial forces as a function of η . The plot is valid for both the symmetric and antisymmetric load cases. Actually, for axial forces, the mode mixity turns out to have the same dependency on the sublaminate thickness ratio in both cases. Results are presented for the present method (red, continuous curve), the elastic-interface model (orange, dashed curves), and the local method (blue, dotted curve). The results of the elastic-interface model depend on the values of the constants of the tangential and normal distributed springs, k_x and k_z . The figure shows four curves corresponding to four values (1, 10, 100, and 1000) of the dimensionless elastic-interface constants, $\mu_x = k_x H / G_{zx}$ and $\mu_z = k_z H / E_z$. Note that as μ_x and μ_z increase, the predictions of the elastic-interface model tend towards those of the present method, to which they converge asymptotically for $\mu_x = \mu_z \to +\infty$ (rigid interface limit case). The predictions of the local method are approximately equal to those of the elastic-interface model for $\mu_x = \mu_z = 10$ and $\eta \in [0.1, 1]$. Major differences among the compared methods are observed for the smaller values of η . For $\eta \to 0$ (thin film limit case), the present method predicts mixed-mode fracture with $\psi = 30^{\circ}$. This result is qualitatively consistent with the local method, which however predicts a different value of $\psi = 52.1^{\circ}$. Instead, the elastic-interface model predicts pure mode II fracture ($\psi = 90^{\circ}$), regardless of the values of the interface constants. Interestingly, this latter result is consistent with the analysis of the peel test by De Lorenzis and Zavarise [53]. Lastly, it is noted that for $\eta = 1$ (symmetric delamination), all the compared methods yield $\psi = 90^{\circ}$, which means that if the fracture propagates (G > 0) then it does in mode II.

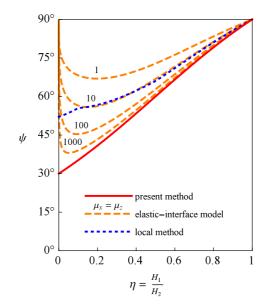


Fig. 10. Mode-mixity angle due to (symmetric and antisymmetric) axial forces.

4.1.3. Effect of shear forces

Figure 11 shows the energy release rate, G, produced by shear forces as a function of η . As for the axial force load case, G is non-dimensionalised with respect to the parameters of the problem. All the compared methods predict the energy release rate to be a decreasing function of η in the considered interval. For the elastic-interface model, four curves are shown, corresponding to four values (1, 10, 100, and 1000) of the dimensionless interface constants, μ_x and μ_z . As the interface constants increase, the predictions of the elastic-interface model tend towards those of the present method, to which they converge asymptotically in the limit case of a rigid interface. Note that for the antisymmetric load case (Fig. 11b), the present method predicts G=0 for $\eta=1$ (uniform shear does not produce fracture propagation in a symmetrically delaminated laminate), but the compared methods predict G>0. This is likely because the other methods make different assumptions on the deformation of the crack-tip cross section.

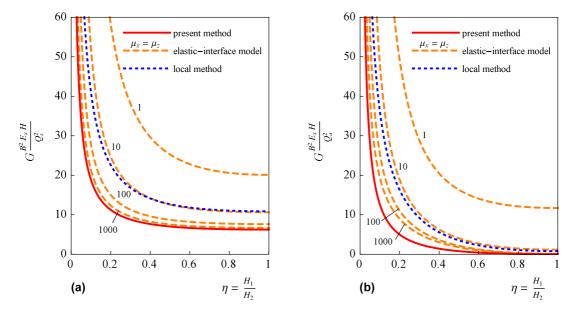


Fig. 11. Dimensionless energy release rate due to (a) symmetric and (b) antisymmetric shear forces.

Figure 12 shows the mode-mixity angle, ψ , produced by shear forces as a function η . The present method predicts $\psi=0^\circ$ (mode I fracture) for both the symmetric (Fig. 12a) and antisymmetric (Fig. 12b) load cases, for all values of η . This means that, according to the present method – based on the Timoshenko beam theory – shear forces do not contribute to G_{II} [54]. Instead, both the elastic-interface model [35] and local method (in the version by Li et al. [22]) predict generally non-zero values for the mode-mixity angle. However, at least for the symmetric load case, the values of ψ are very small (note the different axis scales in figures 12a and 12b). The predictions of the elastic-interface model (curves are plotted for μ_x and μ_z equal to 10, 100, 1000, and 10^4) tend towards those of the present method as $\mu_x = \mu_z \to +\infty$ (rigid interface limit case). The trends predicted by the local method appear qualitatively similar to those of the elastic-interface model, but quantitatively different. For $\eta=1$ (symmetric delamination), both the elastic-interface model and local method predict $\psi=0^\circ$ (mode I fracture) under symmetric shear loads.

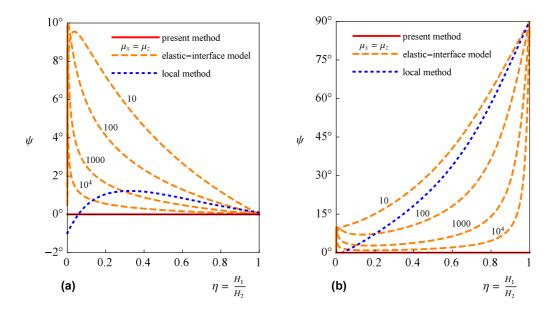


Fig. 12. Mode-mixity angle due to (a) symmetric and (b) antisymmetric shear forces.

4.1.4. Effect of bending moments

Figure 13 shows the energy release rate, G, produced by bending moments as a function of η . As for the previous load cases, G is non-dimensionalised with respect to the parameters of the problem. For both symmetric (Fig. 13a) and antisymmetric (Fig. 13b) loads, the energy release rate is a decreasing function of η in the considered interval. The elastic-interface model [35] and local method [11] furnish for G the same results of the present method.

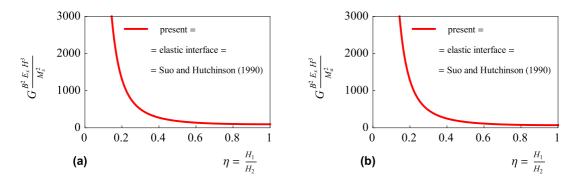


Fig. 13. Dimensionless energy release rate due to (a) symmetric and (b) antisymmetric bending moments.

Figure 14 shows the mode-mixity angle, ψ , produced by symmetric and antisymmetric bending moments as a function of η . For the elastic-interface model, four values (1, 10, 100, and 1000) of the dimensionless interface constants, μ_x and μ_z , are considered. Comments to figure 14 are similar to those applying to figure 10 (axial force load case). For $\eta \to 0$ (thin film limit case), the present method agrees qualitatively with the local method in predicting mixed-mode fracture, although they yield different values for ψ (60° and 37.9°, respectively).

The elastic-interface model predicts $\psi = 0^{\circ}$ (mode I fracture), regardless of the values of the interface constants. This result is again consistent with the analysis of the peel test by De Lorenzis and Zavarise [53]. Lastly, it is noted that for $\eta = 1$ (symmetric delamination), all the compared methods yield $\psi = 0^{\circ}$ (mode I fracture) for symmetric loads and $\psi = 90^{\circ}$ (mode II fracture) for antisymmetric loads.

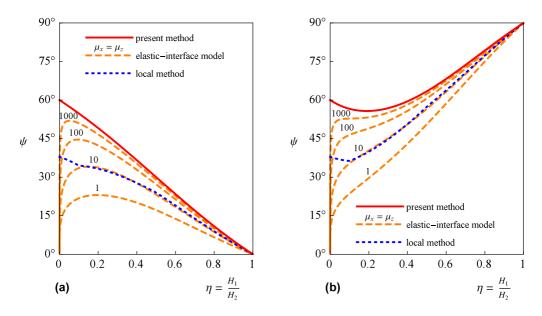


Fig. 14. Mode-mixity angle due to (a) symmetric and (b) antisymmetric bending moments.

4.2. Bimaterial beams

Bimaterial beams, namely laminated beams made of two different materials (Fig. 15), are now considered. For simplicity, each material is supposed to be homogeneous and isotropic. Furthermore, it is assumed that the two sublaminates have equal thicknesses ($H_1 = H_2 = h$) in order to focus on the effects of the mismatch in the elastic moduli.

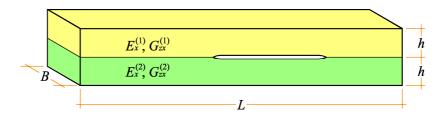


Fig. 15. Delaminated bimaterial beam.

The sublaminate stiffnesses are the following:

$$A_{1} = E_{x}^{(1)}h, \quad B_{1} = 0, \quad C_{1} = \frac{5}{6}G_{zx}^{(1)}h, \quad \text{and} \quad D_{1} = \frac{1}{12}E_{x}^{(1)}h^{3};$$

$$A_{2} = E_{x}^{(2)}h, \quad B_{2} = 0, \quad C_{2} = \frac{5}{6}G_{zx}^{(2)}h, \quad \text{and} \quad D_{2} = \frac{1}{12}E_{x}^{(2)}h^{3},$$

$$(35)$$

where $E_x^{(1)}$ and $G_{zx}^{(1)}$ respectively are the Young's modulus and shear modulus of the upper sublaminate, $E_x^{(2)}$ and $G_{zx}^{(2)}$ are the corresponding quantities of the lower sublaminate. By substituting Eqs. (35) into (5), (9), (13), and (26), the mode I and II contributions to the energy release rate,

$$G_{I} = \frac{3}{8B^{2}h^{3}} \frac{1}{E_{x}^{(1)} + E_{x}^{(2)}} \frac{1}{E_{x}^{(1)}} + 14 + \frac{E_{x}^{(2)}}{E_{x}^{(1)}} + 14 + \frac{E_{x}^{(2)}}{E_{x}^{(1)}} + 7$$

$$\cdot \left[\left(\frac{E_{x}^{(2)}}{E_{x}^{(1)}} - 1 \right) N_{1}h + \left(\frac{E_{x}^{(1)}}{E_{x}^{(2)}} - 1 \right) N_{2}h - 2 \left(\frac{E_{x}^{(2)}}{E_{x}^{(1)}} + 7 \right) M_{1} + 2 \left(\frac{E_{x}^{(1)}}{E_{x}^{(2)}} + 7 \right) M_{2} \right]^{2} +$$

$$+ \frac{3}{5B^{2}h} \frac{G_{zx}^{(1)} / G_{zx}^{(2)}}{G_{zx}^{(1)} + G_{zx}^{(2)}} \left(\frac{G_{zx}^{(2)}}{G_{zx}^{(1)}} Q_{1} - Q_{2} \right)^{2} \quad \text{and}$$

$$G_{II} = \frac{1}{8B^{2}h^{3}} \frac{E_{x}^{(1)} / E_{x}^{(2)}}{E_{x}^{(1)} + E_{x}^{(2)}} \left(\frac{E_{x}^{(2)}}{E_{x}^{(1)}} N_{1}h - N_{2}h + 6 \frac{E_{x}^{(2)}}{E_{x}^{(1)}} M_{1} + 6 M_{2} \right)^{2},$$

$$(36)$$

are obtained as functions of the internal forces. The total energy release rate can be obtained by summing the two modal contributions given by Eqs. (36). It can be shown that the resulting expression for G would be identical to the one by Suo and Hutchinson [11], for axial forces and bending moments, and to that by Andrews and Massabò [23], if also shear forces are considered (apart from a term stemming from root rotations, not considered here). Obviously, the modal contributions turn out to be different because of the different assumptions on fracture mode partitioning.

For the sake of illustration, four load cases are considered: antisymmetric axial forces, symmetric shear forces, symmetric and antisymmetric bending moments (Fig. 16). The Poisson's ratio is assumed as $V_{zx} = 0.3$ and the shear modulus computed as $G_{zx}^{(\alpha)} = E_x^{(\alpha)} / [2(1+V_{zx})]$, where $\alpha \in \{1, 2\}$.

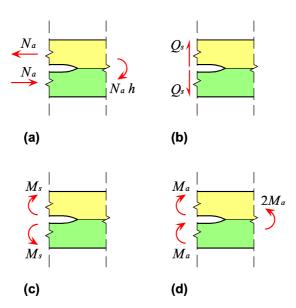
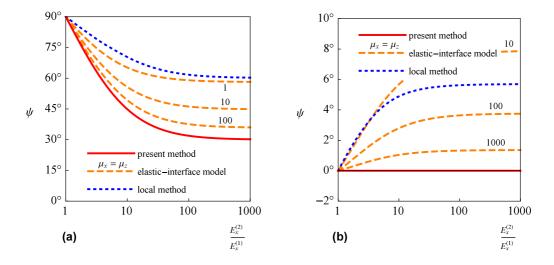


Fig. 16. Load cases for bimaterial beams: (a) antisymmetric axial forces; (b) symmetric shear forces; (c) symmetric bending moments; (d) antisymmetric bending moments.

Figure 17 shows the mode-mixity angle as a function of the ratio between the Young's moduli of the upper and lower sublaminates, $E_x^{(2)}/E_x^{(1)}$. The results of the present method are compared to those of the elastic-interface model [35] for three values of the dimensionless interface constants, $\mu_x = k_x H/G_{zx}^{(1)}$ and $\mu_z = k_z H/E_x^{(1)}$. It is noted that the predictions of the elastic-interface model tend towards those of the present method as $\mu_x = \mu_z \to +\infty$ (rigid interface limit case). For reference, also the predictions of the local method [11, 22] are plotted. The general trends are qualitatively similar, but differ quantitatively.



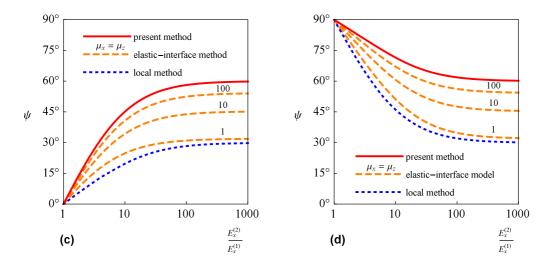


Fig. 17. Mode-mixity angle for bimaterial beams: (a) antisymmetric axial forces; (b) symmetric shear forces; (c) symmetric bending moments; (d) antisymmetric bending moments.

4.3. Laminated beams

For laminated beams with general stacking sequences, the energy release rate and mode mixity must be calculated by using the expressions of Section 3. The resulting general expressions are quite lengthy and will not be given here. However, as far as the total energy release rate is concerned, it can be shown that the present method yields exactly the same expression obtained by Schapery and Davidson [17] for the axial loads and bending moments contributions. Also, the expressions for the energy release rate obtained by Bao *et al.* [21] and Lundsgaard–Larsen *et al.* [41] for sandwich specimens can be retrieved.

By way of example, the method will be applied to some non-standard laboratory tests used to measure the delamination toughness of composite laminates:

- the asymmetric double cantilever beam (ADCB) test;
- the double cantilever beam loaded with uneven bending moments (DCB-UBM) test;
- the asymmetric mixed-mode bending (AMMB) test;
- the mixed-mode bending (MMB) test with doublers.
 Lastly, a comparison with the method by Schapery and Davidson [17] will be made.

4.3.1. Asymmetric double cantilever beam (ADCB) test

In the asymmetric double cantilever beam test, a laminated specimen with an initial delamination of length a is loaded by two opposite forces of equal intensity, P (Fig. 18). The mode mixity originates from the geometric and/or material asymmetry of the loaded arms of the specimen [31, 55].

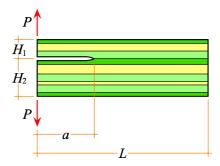


Fig. 18. Asymmetric double cantilever beam test.

The internal forces at the crack-tip cross section of an ADCB test specimen are

$$N_1 = 0, \quad Q_1 = P, \quad M_1 = Pa;$$

 $N_2 = 0, \quad Q_2 = -P, \quad M_2 = -Pa.$ (37)

By substituting Eqs. (37) into (9), the crack-tip displacement rates,

$$\rho_{u} = \frac{Pa}{B}(b_{1} + b_{2} + d_{1}h_{1} - d_{2}h_{2}), \quad \rho_{w} = \frac{P}{B}(c_{1} + c_{2}), \quad \text{and} \quad \rho_{\phi} = \frac{Pa}{B}(d_{1} + d_{2}), \tag{38}$$

are determined. Then, from Eqs. (13) and (16) the crack-tip forces,

$$N_C = 0$$
, $Q_C = P$, and $M_C = Pa$, (39)

are obtained. Lastly, from Eqs. (27) the mode I and II contributions to the energy release rate,

$$G_{I} = \frac{P^{2}a^{2}}{2B^{2}} \left[d_{1} + d_{2} - \frac{(b_{1} + b_{2} + d_{1}h_{1} - d_{2}h_{2})^{2}}{a_{1} + a_{2} + 2b_{1}h_{1} - 2b_{2}h_{2} + d_{1}h_{1}^{2} + d_{2}h_{2}^{2}} \right] + \frac{P^{2}}{2B^{2}} (c_{1} + c_{2}) \quad \text{and}$$

$$G_{II} = \frac{P^{2}a^{2}}{2B^{2}} \frac{(b_{1} + b_{2} + d_{1}h_{1} - d_{2}h_{2})^{2}}{a_{1} + a_{2} + 2b_{1}h_{1} - 2b_{2}h_{2} + d_{1}h_{1}^{2} + d_{2}h_{2}^{2}},$$

$$(40)$$

are calculated. The sum of the two modal contributions yields the total energy release rate,

$$G = \frac{P^2}{2R^2} \left[c_1 + c_2 + a^2 (d_1 + d_2) \right]. \tag{41}$$

It can be verified that, if the delaminating sublaminates are uncoupled ($b_1 = b_2 = 0$) and shear deformability is neglected ($c_1 = c_2 = 0$), then Eqs. (40) yield the same results computed by the elastic-interface model [31] in the limit case of a rigid interface ($\mu_x = \mu_z \to +\infty$).

4.3.2. Double cantilever beam loaded with uneven bending moments (DCB-UBM) test Sørensen *et al.* [56] have proposed a modified DCB test, where the specimen is loaded by bending moments, \overline{M}_1 and \overline{M}_2 (Fig. 19). Loading by moments instead of forces has several advantages: (i) the whole range of mode mixities (from pure mode I to mode II) can be tested

by varying the load ratio, $\overline{M}_1/\overline{M}_2$; (ii) delamination growth is stable during the test for all mode mixities; (iii) the test results are independent of the delamination length (which can be difficult to measure in some circumstances); (iv) shear deformability does not influence the test results. For symmetric specimens, the symmetric and antisymmetric parts of the applied loads respectively correspond to fracture modes I and II. If the specimen is not symmetric about the delamination plane, then an additional source of mode mixity is given by the geometric and/or material asymmetry of the loaded arms. For the sake of simplicity, a symmetrically delaminated specimen is considered here ($a_1 = a_2$, $b_1 = -b_2$, $c_1 = c_2$, $d_1 = d_2$, and $h_1 = h_2$).

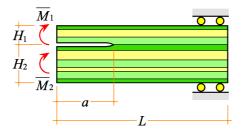


Fig. 19. Double cantilever beam loaded with uneven bending moments test.

The internal forces at the crack-tip cross section of a DCB-UBM test specimen are

$$N_1 = 0, \quad Q_1 = 0, \quad M_1 = \overline{M}_1;$$

 $N_2 = 0, \quad Q_2 = 0, \quad M_2 = \overline{M}_2.$ (42)

By substituting Eqs. (42) into (9), the crack-tip displacement rates,

$$\rho_u = \frac{1}{R}(b_1 + d_1h_1)(M_1 + M_2), \quad \rho_w = 0, \quad \text{and} \quad \rho_\phi = \frac{1}{R}d_1(M_1 - M_2),$$
 (43)

are determined. Then, from Eqs. (13) and (16) the crack-tip forces,

$$N_{C} = \frac{1}{2} \frac{b_{1} + d_{1}h_{1}}{a_{1} + 2b_{1}h_{1} + d_{1}h_{1}^{2}} (M_{1} + M_{2}), \quad Q_{C} = 0, \quad \text{and} \quad M_{C} = \frac{1}{2} (M_{1} - M_{2}), \tag{44}$$

are obtained. Lastly, from Eqs. (27) the modal contributions to the energy release rate,

$$G_{\rm I} = \frac{d_{\rm I}}{4B^2} (M_1 - M_2)^2$$
 and $G_{\rm II} = \frac{1}{4B^2} \frac{(b_{\rm I} + d_{\rm I}h_{\rm I})^2}{a_{\rm I} + 2b_{\rm I}h_{\rm I} + d_{\rm I}h_{\rm I}^2} (M_1 + M_2)^2$, (45)

are calculated.

4.3.3. Asymmetric mixed-mode bending (AMMB) test

In the mixed-mode bending (MMB) test, a laminated specimen with an initial delamination

of length a is loaded simultaneously by an upward load, P_u , and a downward load, P_d (Fig. 20). By suitably changing the relative intensities of the applied loads, a range of mode mixities can be tested [57]. The test is standardised for unidirectional laminated specimens with symmetric delaminations by ASTM [58], but has been used also for multidirectional and asymmetric specimens [9], bimaterial specimens [59, 60], multidirectional laminates [32, 33, 61], sandwich plates [62], and asymmetric adhesively bonded joints [63, 64]. In all the aforementioned cases, where there is geometric and/or material asymmetry about the delamination plane, the partitioning of fracture modes does not correspond to the decomposition of the applied loads into the symmetric and antisymmetric parts, as it does for symmetrically delaminated specimens. Unfortunately, this point appears not to have been fully appreciated in the literature with a few exceptions [32, 33, 61]. The present method furnishes a way to determine the mode I and II contributions to the energy release rate for asymmetric MMB test specimens within the context of beam theory.

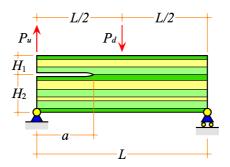


Fig. 20. Asymmetric mixed-mode bending test.

The internal forces at the crack-tip cross section of an AMMB test specimen are

$$N_{1} = 0, \quad Q_{1} = P_{u}, \quad M_{1} = P_{u}a;$$

$$N_{2} = 0, \quad Q_{2} = \frac{1}{2}P_{d} - P_{u}, \quad M_{2} = \left(\frac{1}{2}P_{d} - P_{u}\right)a.$$
(46)

By substituting Eqs. (46) into (9), the crack-tip displacement rates,

$$\rho_{u} = \frac{P_{u}a}{B}(b_{1} + b_{2} + d_{1}h_{1} - d_{2}h_{2}) - \frac{P_{d}a}{2B}(b_{2} - d_{2}h_{2}), \quad \rho_{w} = \frac{P_{u}}{B}(c_{1} + c_{2}) - \frac{P_{d}}{2B}c_{2},$$
and
$$\rho_{\phi} = \frac{P_{u}a}{B}(d_{1} + d_{2}) - \frac{P_{d}a}{2B}d_{2},$$
(47)

are obtained. Such quantities can then be used to calculate the energy release rate and its modal contributions via Eqs. (20) and (26), respectively. However, the resulting general expressions are quite lengthy and will not be given here.

4.3.4. Mixed-mode bending (MMB) test with doublers

Reeder *et al.* [65] have proposed to reinforce mixed-mode bending test specimens (of thickness $2t_2$) experiencing premature bending failure by adding 'doublers', *i.e.* externally bonded sheets of stiffer material (of thickness t_1). The result is an MMB specimen, where the sublaminates are bimaterial beams with bending-extension coupling (Fig. 21).

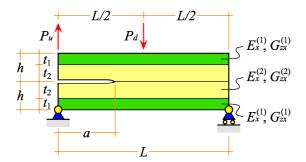


Fig. 21. Mixed-mode bending with doublers test.

The sublaminate equivalent stiffnesses turn out to be

$$A_{1} = A_{2} = E_{x}^{(1)} t_{1} + E_{x}^{(2)} t_{2}, \quad B_{1} = -B_{2} = -\frac{1}{2} (E_{x}^{(1)} - E_{x}^{(2)}) t_{1} t_{2},$$

$$C_{1} = C_{2} = \frac{5}{6} (G_{zx}^{(1)} t_{1} + G_{zx}^{(2)} t_{2}), \quad \text{and} \quad D_{1} = D_{2} = \frac{1}{12} [E_{x}^{(1)} t_{1} (t_{1}^{2} + 3t_{2}^{2}) - E_{x}^{(2)} t_{2} (t_{2}^{2} + 3t_{1}^{2})],$$

$$(48)$$

while the compliances can be calculated via Eqs. (5). By observing that the specimen is symmetric about the delamination plane ($a_1 = a_2$, $b_1 = -b_2$, $c_1 = c_2$, $d_1 = d_2$, and $h_1 = h_2$), the crack-tip displacement rates, Eqs. (47), can be simplified as follows:

$$\rho_{u} = \frac{P_{d}a}{2B}(b_{1} + d_{1}h_{1}), \quad \rho_{w} = \frac{4P_{u} - P_{d}}{2B}c_{1}, \quad \text{and} \quad \rho_{\phi} = \frac{(4P_{u} - P_{d})a}{2B}d_{1}.$$
(49)

By substituting Eqs. (49) into (26), and recalling Eqs. (13), the mode I and II contributions to the energy release rate,

$$G_{\rm I} = \frac{(4P_u - P_d)^2}{16B^2} (\mathbf{c}_1 + a^2 \mathbf{d}_1) \quad \text{and} \quad G_{\rm II} = \frac{P_d^2 a^2}{16B^2} \frac{(\mathbf{b}_1 + \mathbf{d}_1 \mathbf{h}_1)^2}{\mathbf{a}_1 + 2\mathbf{b}_1 \mathbf{h}_1 + \mathbf{d}_1 \mathbf{h}_1^2}, \tag{50}$$

are obtained.

4.3.5. Comparison with the method by Schapery and Davidson

For general laminated beams, Schapery and Davidson [17] expressed the mode I and II contributions to the energy release rate as follows:

$$G_{\rm I} = \frac{1}{2B} (k_1 N_C + k_2 M_C)^2$$
 and $G_{\rm II} = \frac{1}{2B} (k_3 N_C + k_4 M_C)^2$, (51)

where k_1 , k_2 , k_3 , and k_4 are real coefficients, which depend on the stacking sequence, but not on the load condition. The authors showed that k_2 , k_3 , and k_4 can be expressed as functions of k_1 . The latter coefficient can be determined numerically by using the finite element method to solve the particular problem of a delaminated beam loaded with the following internal forces (Fig. 22):

$$N_1 \neq 0, \quad Q_1 = 0, \quad M_1 = N_1 h_1;$$

 $N_2 = -N_1, \quad Q_2 = 0, \quad M_2 = N_1 h_2.$ (52)

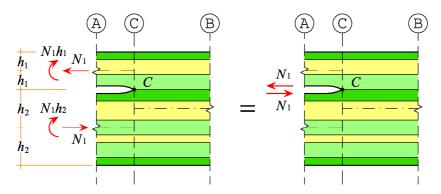


Fig. 22 Load condition analysed via the finite element method by Schapery and Davidson [17].

Schapery and Davidson [17] analysed a number of cases – including both homogeneous and bimaterial beams – and found that the values of k_1 are always close to zero. Hence, they concluded that further study would have been necessary to assess whether assuming $k_1 = 0$ could be a good approximation to evaluate the modal contributions without resorting to finite element calculations. In this respect, by comparing Eqs. (27) and (51) – regardless of the contribution of shear forces –, it can be seen that the present method is equivalent to Schapery and Davidson's method if the following expressions are assumed for their coefficients:

$$k_1 = 0, \quad k_2 = \sqrt{\frac{f_{uN} f_{\phi M} - f_{uM} f_{\phi N}}{f_{uN}}}, \quad k_3 = \sqrt{f_{uN}}, \quad \text{and} \quad k_4 = \frac{f_{uM}}{\sqrt{f_{uN}}}.$$
 (53)

Interestingly, by substituting Eqs. (52) into the expressions in Appendix C, it is found that

$$N_C = N_1, \quad Q_C = 0, \quad M_C = 0.$$
 (54)

Then, by substituting Eqs. (54) into (27), the modal contributions are found as

$$G_{\rm I} = 0$$
 and $G_{\rm II} = \frac{1}{2R} f_{uN} N_1^2$, (55)

which means that, according to the present method, the problem of figure 22 is a case of pure mode II fracture.

5. Discussion

The predictions of the present method for the energy release rate and mode mixity have been compared to those of the elastic-interface model [35] and local method [11, 22] for homogeneous and bimaterial delaminated beams.

For the axial force and bending moment load cases, the compared methods furnish the same results for the energy release rate. This is not surprising for the local method, which uses beam theory to compute G, except for the contribution of shear forces, which depends also on the root rotations [22, 23]. For the elastic-interface model, this result can be explained by observing that for an infinitely long beam subjected to axial forces and bending moments, the elastic interface furnishes an additional term to the compliance, which does not depend on the delamination length, a. Therefore, this contribution vanishes when differentiating the compliance with respect to a to obtain the energy release rate according to the well-known formula by Irwin and Kies [66]. Instead, the contribution to G stemming from shear forces depends on the values of the elastic-interface constants, which are in turn related with the roots rotations (disregarded by the present approach). Anyway, when the (dimensionless) interface constants, μ_x and μ_z , go to infinity, the predictions of the elastic-interface model converge to those of the present method.

As concerns the mode mixity, the predictions of the present method agree qualitatively with both the elastic-interface model and local method. However, differences in the values of the angle ψ have been obtained. Such differences are most likely due to the simplifying assumptions on which the present method is based. The predictions of the elastic-interface model tend towards those of the present method as the interface constants, μ_x and μ_z , increase and converge to the present method for $\mu_x = \mu_z \to +\infty$ (rigid interface limit case).

6. Conclusions

A method has been presented to calculate the energy release rate and mode mixity of laminated beams of rectangular cross section with through-the-width delaminations. According to classical lamination theory and Timoshenko beam kinematics, the axial, shear, and bending deformabilities, as well as the bending-extension coupling, have been taken into account. The kinematics of an elementary crack growth has been analysed by defining the

crack-tip displacement rates as the relative displacements occurring at the crack tip per unit increase in crack length. Such newly defined quantities have been used to compute the crack-tip forces. Furthermore, by adapting the virtual crack closure technique to the present context and assuming a suitable two-step process of closure of the crack, the energy release rate and its mode I and II contributions have been determined.

Results for homogeneous and bimaterial delaminated beams have been presented and compared to the predictions of the elastic-interface model [35] and local method [11, 22]. Perfect agreement between the compared methods has been obtained for the energy release rate due to axial forces and bending moments. Minor differences have been obtained for the shear force load case.

With respect to the mode mixity, qualitative agreement has been found between the compared methods, but some quantitative differences have emerged. Such differences are most likely due to the simplifying assumptions on which the present method is based. Taking into account some of the disregarded effects (root rotations, Poisson's effect, *etc.*) could probably help improve the accuracy of the present method. Furthermore, it has been observed that the results of the elastic-interface model converge to those of the present method for the limit case of a rigid interface, but is deduced in a completely independent way.

For general laminated beams, applications to some non-standard delamination toughness tests have been illustrated. Lastly, the present method has been compared to the method by Schapery and Davidson [17]. The two methods turn out to be equivalent if the coefficient k_1 defined in [17] is taken equal to zero. The present method nevertheless represents an improvement as it considers the effects of shear forces, not included in [17].

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Appendix A. Equivalent stiffnesses of a laminated beam

The present study considers laminated beams of constant, rectangular cross section of width B and height H = 2h (Fig. A1). A reference system Oyz is fixed in the cross-section plane with the origin at the geometric centre of the section and the y- and z-axes aligned with the width and height directions, respectively. The beam is made of n layers (or laminae) of

different materials and thicknesses. Let $E_x^{(1)}, E_x^{(2)}, ..., E_x^{(n)}$ and $G_{zx}^{(1)}, G_{zx}^{(2)}, ..., G_{zx}^{(n)}$ respectively be the longitudinal Young's moduli and shear moduli of the n layers. Furthermore, let $z_0, z_1, ..., z_n$ be the ordinates of the top and bottom surfaces of the layers.

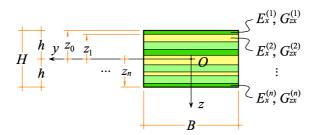


Fig. A1. Cross section of the laminated beam.

According to classical lamination theory [48], the laminated beam can be modelled as a homogeneous beam with the following *equivalent stiffnesses*:

$$A = \sum_{i=1}^{n} E_{x}^{(i)}(z_{i} - z_{i-1}), \quad B = \frac{1}{2} \sum_{i=1}^{n} E_{x}^{(i)}(z_{i}^{2} - z_{i-1}^{2}),$$

$$C = \frac{5}{6} \sum_{i=1}^{n} G_{zx}^{(i)}(z_{i} - z_{i-1}), \quad \text{and} \quad D = \frac{1}{3} \sum_{i=1}^{n} E_{x}^{(i)}(z_{i}^{3} - z_{i-1}^{3}).$$
(A1)

In particular, A, B, C, and D, respectively denote the *extensional stiffness*, *bending-extension coupling stiffness*, *shear stiffness*, and *bending stiffness* per unit width of the laminated beam. Inversion of the constitutive laws for the beam leads to define the corresponding *equivalent compliances*,

$$a = \frac{D}{AD - B^2}, \quad b = -\frac{B}{AD - B^2}, \quad c = \frac{1}{C}, \text{ and } d = \frac{A}{AD - B^2}.$$
 (A2)

Appendix B. End-loaded laminated cantilever beam

Let us consider a laminated cantilever beam of length L, loaded at its free end by an axial force, \overline{N} , a shear force, \overline{Q} , and a bending moment, \overline{M} (Fig. B1). A reference system Ozx is fixed in the principal plane of the beam with the origin at the clamped end and the x-axis aligned with the longitudinal direction. Let u and w denote the axial and transverse displacements of the beam centreline, respectively, and ϕ the cross-section rotation, positive if counter-clockwise. Let A, B, C, and D be the equivalent stiffnesses of the beam per unit width and let a, b, c, and d be the corresponding compliances.

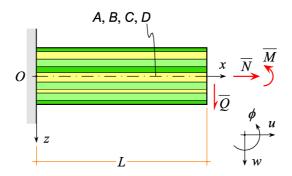


Fig. B1. Laminated cantilever beam loaded at its end.

The internal forces in the beam are

$$N(x) = \overline{N}, \quad Q(x) = \overline{Q}, \quad \text{and} \quad M(x) = \overline{M} - \overline{Q}(L - x).$$
 (B1)

Using the compatibility equations and inverse constitutive relationships for the laminated beam, the following relationships are obtained:

$$\varepsilon = \frac{du}{dx} = \frac{1}{B}(aN + bM), \quad \gamma = \frac{dw}{dx} + \phi = \frac{1}{B}cQ, \quad \text{and} \quad \kappa = \frac{d\phi}{dx} = \frac{1}{B}(bN + dM), \tag{B2}$$

where B is the width of the cross section. By substituting Eqs. (B1) into (B2) and integrating with respect to x (and also considering the boundary conditions at the clamped end, x = 0), the generalised displacements of the beam centreline,

$$u(x) = \frac{1}{B} \left[a \overline{N} x - b \overline{Q} \left(L - \frac{x}{2} \right) x + b \overline{M} x \right],$$

$$w(x) = \frac{1}{B} \left\{ -b \overline{N} \frac{x^2}{2} + \overline{Q} \left[c + d \left(L - \frac{x}{3} \right) \frac{x}{2} \right] x - d \overline{M} \frac{x^2}{2} \right\}, \text{ and}$$

$$\phi(x) = \frac{1}{B} \left[b \overline{N} x - d \overline{Q} \left(L - \frac{x}{2} \right) x + d \overline{M} x \right],$$
(B3)

are obtained as functions of x. In particular, the generalised displacements at the loaded end, x = L, are

$$\overline{u} = \frac{aL}{B}\overline{N} - \frac{bL^2}{2B}\overline{Q} + \frac{bL}{B}\overline{M},$$

$$\overline{w} = -\frac{bL^2}{2B}\overline{N} + \frac{1}{B}\left(cL + \frac{1}{3}dL^3\right)\overline{Q} - \frac{dL^2}{2B}\overline{M}, \text{ and}$$

$$\overline{\phi} = \frac{bL}{B}\overline{N} - \frac{dL^2}{2B}\overline{Q} + \frac{dL}{B}\overline{M}.$$
(B4)

Appendix C. Crack-tip forces as functions of the internal forces

By substituting Eqs. (9) into (16), after some calculations, the crack-tip forces,

$$N_{C} = \frac{v_{1}N_{1} + v_{2}N_{2} + v_{3}(M_{1} + M_{2})}{v_{4}},$$

$$Q_{C} = \frac{c_{1}Q_{1} - c_{2}Q_{2}}{c_{1} + c_{2}}, \text{ and}$$

$$M_{C} = \frac{\mu_{1}N_{1} + \mu_{2}N_{2} + \mu_{3}M_{1} + \mu_{4}M_{2}}{v_{4}},$$
(C1)

are obtained as functions of the internal forces in the sublaminates. In Eqs. (C1), the following constant parameters are used:

$$v_{1} = a_{1}(d_{1} + d_{2}) - b_{1}(b_{1} + b_{2}) + b_{1}d_{2}h,
v_{2} = -a_{2}(d_{1} + d_{2}) + b_{2}(b_{1} + b_{2}) + b_{2}d_{1}h,
v_{3} = b_{1}d_{2} - b_{2}d_{1} + d_{1}d_{2}h,
v_{4} = (a_{1} + a_{2})(d_{1} + d_{2}) - (b_{1} + b_{2})^{2} + 2(b_{1}d_{2} - b_{2}d_{1})h + d_{1}d_{2}h^{2};$$
(C2)

and

$$\mu_{1} = -a_{1}b_{2} + a_{2}b_{1} - a_{1}(d_{1}h_{1} - d_{2}h_{2}) + b_{1}^{2}h_{1} - b_{1}b_{2}(h + h_{2}) + b_{1}d_{2}hh_{2},$$

$$\mu_{2} = -a_{1}b_{2} + a_{2}b_{1} + a_{2}(d_{1}h_{1} - d_{2}h_{2}) + b_{2}^{2}h_{2} - b_{1}b_{2}(h + h_{1}) - b_{2}d_{1}hh_{1},$$

$$\mu_{3} = (a_{1} + a_{2})d_{1} - b_{1}(b_{1} + b_{2}) + b_{1}d_{2}h_{2} - b_{2}d_{1}(h + h_{2}) + d_{1}d_{2}hh_{2},$$

$$\mu_{4} = -(a_{1} + a_{2})d_{2} + b_{2}(b_{1} + b_{2}) + b_{2}d_{1}h_{1} - b_{1}d_{2}(h + h_{1}) - d_{1}d_{2}hh_{1}.$$
(C3)

The above expressions simplify significantly if the sublaminates are uncoupled in bendingextension ($b_1 = b_2 = 0$), in which case

$$N_{C} = \frac{(d_{1} + d_{2})(a_{1}N_{1} - a_{2}N_{2}) + d_{1}d_{2}h(M_{1} + M_{2})}{(a_{1} + a_{2})(d_{1} + d_{2}) + d_{1}d_{2}h^{2}},$$

$$Q_{C} = \frac{c_{1}Q_{1} - c_{2}Q_{2}}{c_{1} + c_{2}}, \text{ and }$$

$$M_{C} = \frac{-(d_{1}h_{1} - d_{2}h_{2})(a_{1}N_{1} - a_{2}N_{2}) + (a_{1} + a_{2})(d_{1}M_{1} - d_{2}M_{2}) + d_{1}d_{2}h(h_{2}M_{1} - h_{1}M_{2})}{(a_{1} + a_{2})(d_{1} + d_{2}) + d_{1}d_{2}h^{2}}.$$
(C4)

Appendix D. Elastic-interface model

Liu *et al.* [35] have developed a general solution for the stress analysis of adhesively bonded joints. Their mechanical model consists of two homogeneous and isotropic, shear-deformable beams [47], partly connected by an elastic interface, in turn made of distributed normal and tangential springs. Their model can be used also for delaminated laminates, if the

interface is interpreted as a conventional means to account for the laminate transverse deformability and not as representative of a physical layer of adhesive. The solution by Liu *et al.* [35] holds for beams of finite length. Here, the solution is presented for a semi-infinite bimaterial beam subjected to axial forces, shear forces, and bending moments (Fig. D1). This solution has been used for the comparisons shown in Section 4 of this paper.

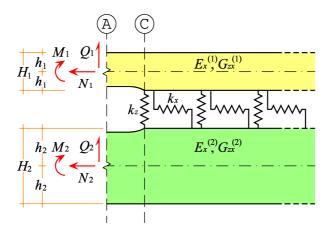


Fig. D1. Elastic-interface model of a semi-infinite bimaterial beam.

Let $E_x^{(1)}$, $G_{zx}^{(1)}$ and $E_x^{(2)}$, $G_{zx}^{(2)}$ respectively be the Young's modulus and shear modulus of the upper and lower beams. The equivalent stiffnesses can be computed from Eqs. (35) and the compliances from Eqs. (5). Let k_x and k_z respectively be the elastic constants of the distributed springs in the tangential and normal directions. The normal and tangential interfacial stresses attain peak values at the loaded cross section, whose values turn out to be

$$\sigma_0 = F_1 + F_2 + F_3 \quad \text{and}$$

$$\tau_0 = \frac{1}{b_0} \sum_{i=1}^{3} \left[\frac{1}{k_z} \frac{\Omega_i^{3/2}}{H^3} - (c_1 + c_2) \frac{\Omega_i^{1/2}}{H} + (d_1 + d_2) \frac{H}{\Omega_i^{1/2}} \right] F_i + F_7,$$
(D1)

where $b_0 = d_1 h_1 - d_2 h_2$ is the unbalance parameter (here, assumed $b_0 \neq 0$), $H = H_1 + H_2$ is the total thickness, F_1 , F_2 , F_3 , and F_7 are integration constants, and Ω_1 , Ω_2 , and Ω_3 are the roots of the dimensionless characteristic equation,

$$\Omega^3 - \alpha_1 \Omega^2 + \alpha_2 \Omega - \alpha_3 = 0, \tag{D2}$$

where

$$\alpha_{1} = \left[k_{x} (\mathbf{a}_{1} + \mathbf{a}_{2} + \mathbf{d}_{1} h_{1}^{2} + \mathbf{d}_{2} h_{2}^{2}) + k_{z} (\mathbf{c}_{1} + \mathbf{c}_{2}) \right] H^{2},$$

$$\alpha_{2} = \left[k_{x} k_{z} (\mathbf{c}_{1} + \mathbf{c}_{2}) (\mathbf{a}_{1} + \mathbf{a}_{2} + \mathbf{d}_{1} h_{1}^{2} + \mathbf{d}_{2} h_{2}^{2}) + k_{z} (\mathbf{d}_{1} + \mathbf{d}_{2}) \right] H^{4}, \text{ and}$$

$$\alpha_{3} = k_{x} k_{z} \left[(\mathbf{a}_{1} + \mathbf{a}_{2}) (\mathbf{d}_{1} + \mathbf{d}_{2}) + \mathbf{d}_{1} \mathbf{d}_{2} (h_{1} + h_{2})^{2} \right] H^{6}.$$
(D3)

The integration constants, F_1 , F_2 , F_3 , and F_7 , F_8 , F_9 , are calculated from the boundary conditions depending on the applied loads,

$$\begin{split} N_{1} &= \frac{B}{b_{0}} \sum_{i=1}^{3} \left[\frac{1}{k_{z}} \frac{\Omega_{i}}{H^{2}} - (c_{1} + c_{2}) + (d_{1} + d_{2}) \frac{H^{2}}{\Omega_{i}} \right] F_{i} - BF_{8}, \\ Q_{1} &= B \sum_{i=1}^{3} \frac{H}{\Omega_{i}^{1/2}} F_{i} + B \left[\frac{a_{1} + a_{2}}{d_{1}(h_{1} + h_{2})} + h_{1} \right] F_{7}, \\ M_{1} &= B \sum_{i=1}^{3} \left\{ \frac{h_{1}}{b_{0}} \left[\frac{1}{k_{z}} \frac{\Omega_{i}}{H^{2}} - (c_{1} + c_{2}) + (d_{1} + d_{2}) \frac{H^{2}}{\Omega_{i}} \right] - \frac{H^{2}}{\Omega_{i}} \right\} F_{i} + B \frac{a_{1}F_{8} + a_{2}F_{9}}{d_{1}(h_{1} + h_{2})}, \\ N_{2} &= -\frac{B}{b_{0}} \sum_{i=1}^{3} \left[\frac{1}{k_{z}} \frac{\Omega_{i}}{H^{2}} - (c_{1} + c_{2}) + (d_{1} + d_{2}) \frac{H^{2}}{\Omega_{i}} \right] F_{i} + BF_{9}, \\ Q_{2} &= -B \sum_{i=1}^{3} \frac{H}{\Omega_{i}^{1/2}} F_{i} + B \left[\frac{a_{1} + a_{2}}{d_{2}(h_{1} + h_{2})} + h_{2} \right] F_{7}, \text{ and} \\ M_{2} &= B \sum_{i=1}^{3} \left\{ \frac{h_{2}}{b_{0}} \left[\frac{1}{k_{z}} \frac{\Omega_{i}}{H^{2}} - (c_{1} + c_{2}) + (d_{1} + d_{2}) \frac{H^{2}}{\Omega_{i}} \right] + \frac{H^{2}}{\Omega_{i}} \right\} F_{i} + B \frac{a_{1}F_{8} + a_{2}F_{9}}{d_{2}(h_{1} + h_{2})}. \end{split}$$

From the peak values of the interfacial stresses, the modal contributions,

$$G_{\rm I} = \frac{1}{2} \frac{\sigma_0^2}{k_z}$$
 and $G_{\rm II} = \frac{1}{2} \frac{\tau_0^2}{k_z}$, (D5)

are determined. Lastly, the total energy release rate and mode-mixity angle are computed from Eqs. (21) and (29), respectively.

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