

1 **Integration of space-borne DInSAR data in a multi-method monitoring concept for alpine mass**
2 **movements**

3 Robert Kenner¹, Giulia Chinellato², Christian Iasio², David Mosna³, Giovanni Cuzzo², Elisa Benedetti⁴,
4 Maria Grazia Visconti⁵, Marcia Phillips¹, Volkmar Mair³, Andreas Zischg⁷, Benni Thiebes², Claudia
5 Strada³

6 ¹ WSL Institute for Snow and Avalanche Research SLF;

7 ² European Academy of Bolzano/Bozen (EURAC);

8 ³ Office for Geology and building materials testing, Autonomous Province of Bolzano, Italy;

9 ⁴ Sapienza University of Rome

10 ⁵ Politecnico di Milano

11 ⁶ National Research Council of Italy, Institute for Electromagnetic Sensing of the Environment (IREA)

12 ⁷ University of Bern, Institute of Geography, Oeschger Centre for Climate Change Research, Mobiliar Lab for Natural Risks

13

14 **Abstract**

15

16 This study presents the results of an experimental application of a multi-method measurement
17 concept for the monitoring of alpine mass movements. Satellite-borne differential interferometric
18 synthetic aperture radar (DInSAR) was applied as the key technology. To improve the information
19 contents of the DInSAR displacement data for an individual mass movement, a complementary
20 measurement was carried out with a three dimensional measurement system. The information on
21 the 3D movement characteristics obtained by this complementary measurement was used to
22 extrapolate subsequent DInSAR measurements to 3D. Terrestrial laser scanning (TLS) and Global
23 Navigation Satellite System (GNSS) data were tested as complementary 3D measurement systems.
24 The deviations between the single measurement systems were mainly controlled by the error
25 budgets of the different methods. An exception were short term GNSS single point time series which
26 included small scale surface movements that were not captured by the other methods. TLS proved to
27 be the most suitable complementary method. A single TLS repeat measurement was sufficient to
28 create a mask, which enables the projection of DInSAR displacement data to 3D. The application of
29 satellite-borne DInSAR in alpine terrain is challenging; signal decorrelation is a problem due to fast
30 terrain movements and snow coverage and can cause failure of the measurement system.

31

32 **Keywords:** Monitoring; Remote sensing; Alpine mass movements; Terrestrial laser scanning; DInSAR;

33 GNSS

34 **1. Introduction**

35 The densification of infrastructure and settlement areas in the European Alps requires careful
36 consideration of natural hazards and the activity of geomorphological processes, in particular in the
37 light of climate change. Alpine mass movements like landslides, permafrost creep or rock slope
38 failures are the source of potentially hazardous processes. They can show a low activity for decades or even centuries without being seriously
39 hazardous – before unexpectedly accelerating. Some of these accelerations can be forecasted or at
40 least detected in real time if the mass wasting zone is monitored on a long term basis. These include
41 for example rock glacier creep surges (Delaloye et al., 2013; Kenner et al., 2014), accelerations of
42 landslides and deep seated gravitational slope deformations (Angeli et al., 2000) or rock fall events
43 (Abellán et al., 2010; Kenner et al., 2011).

45 However, decision makers in natural hazard prevention are confronted with a considerable number
46 of critical mass movement processes in alpine environments, which are often insufficiently
47 documented. Long term monitoring tasks using classical monitoring methods are in many cases
48 associated with high costs. This may force the natural hazard managers into a mode of reactionary
49 operation. In many cases potentially dangerous mass movements are only focused on after the
50 occurrence of larger events, which is suboptimal in terms of hazard prevention (Tobler et al., 2012). A
51 cost-efficient long term monitoring system, mostly working in a self-operating mode over regions of
52 hundreds of square kilometers, with cm accuracy and capturing changes in existing mass movement
53 processes or even detecting new ones, would clearly help decision makers. This is the point on which
54 the current study was based on. Our aim was to develop a measurement concept that enable a
55 supra-regional, long term monitoring of as many individual mass movements as possible.

56 Numerous measurement technologies are currently used for monitoring tasks in alpine terrain (table
57 1). They can be classified into in-situ and remote sensing methods but also into single point tracking
58 systems or those enabling data acquisition over wide areas. The information content of the data
59 obtained varies substantially according to the measurement system. Mass movement processes

60 were traditionally monitored using total stations (Veulliet et al., 2009). Air-borne photogrammetry
61 has been used for several decades for long term monitoring tasks (Fabris et al., 2005; Käab, 1999;
62 Kaufmann and Ladstädter, 2003). In recent years, this method has been applied more frequently and
63 facilitated by the development of air-borne digital sensors (ADS), the availability of drone-borne
64 photogrammetry (Bühler et al., 2012; Eisenbeiß, 2009) and software improvements. Meanwhile
65 GNSS measurements have become a widely used additional method to the classical surveying
66 techniques (Lambiel and Delaloye, 2004; Wirz et al., 2015). Since the beginning of the 2000s, laser
67 scanning technology has developed rapidly and allowed the terrestrial and air-borne acquisition of
68 widespread 3D terrain data (Bauer et al., 2003; Kenner et al., 2014; Sailer et al., 2012). This was a
69 notable development, especially for monitoring inaccessible and steep terrain areas. Simultaneously,
70 radar technology proved to be a very powerful method for high accuracy applications in mass
71 movement monitoring (Colesanti and Wasowski, 2006; Gischig et al., 2009; Kos et al., 2013; Strozzi et
72 al., 2005), allowing to monitor millimeter scale deformations of the ground surface over areas of up
73 to hundreds of square kilometers. Table 1 gives an overview of the characteristics of these
74 technologies. Several of the methods have been applied by Strozzi et al. (2010) for the monitoring of
75 a rock slide in high alpine terrain. This study provides additional information about the characteristics
76 of the methods and differences between them.

77 Based on this evaluation of measurement systems, our study focuses on satellite-borne differential
78 interferometric synthetic aperture radar (DInSAR) as a key technology to realize the aim of a self-
79 operating large scale monitoring concept providing centimeter accuracy. Space-borne DInSAR is an
80 established method to detect and monitor large scale surface displacements. Similarly it can be used
81 to monitor numerous small scale displacements distributed over areas of several hundred square
82 kilometres (Pritchard and Simons, 2002). In the last decade, a large number of case studies
83 successfully applied DInSAR methods for detecting and monitoring ground deformations in alpine
84 environments, and in particular in permafrost-affected areas (Barboux et al., 2012; Barboux et al.,
85 2015; Echelard et al., 2013; Strozzi et al., 2004). A comprehensive overview of differential InSAR and

86 the application of the method to alpine environments and the detection of ground deformations is
87 provided by Barboux et al. (2015).

88 However all these studies also underline several limitations of DInSAR, making it difficult to rely
89 exclusively on DInSAR data when analyzing and monitoring a mass movement; the one-
90 dimensionality of movement measurements has to be mentioned here in particular. We tried to
91 overcome this weakness by embedding DInSAR measurements into a monitoring concept that can
92 implement the results of a complementary measurement system if necessary. A locally and
93 temporally limited application of such complementary data provides 3D movement information on
94 individual mass movements to understand their general movement characteristics. The
95 complementary data should enable the operator to draw clear conclusions from further DInSAR
96 measurements and also monitor complex movement processes using DInSAR only.

97

98 **2. Study site**

99 The study was carried out at a test site located above Pontresina in the Upper Engadin valley, Grisons
100 (Switzerland). The monitoring area includes three individual active rock glaciers in a West oriented
101 mountain cirque called Foura da l'amd Ursina. The rock glacier complex henceforth referred to as
102 Ursina (Figure 1) ranges between 2700 and 2900 m asl. The steep surrounding ridges and rock walls
103 reach elevations over 3000 m and are subject to intermittent rock fall. The lowest rock glacier has a
104 steep front in the top of the Val Giandains gully, which is the source area of potential debris flows. A
105 protection dam was built at the base of the gully above Pontresina in 2003.

106

107 **3. Methods**

108 **3.1. Monitoring concept**

109 Satellite-borne differential interferometric synthetic aperture radar (DInSAR) (Bamler and Hartl,
110 1998; Skolnik, 1980) provides data on surface changes over areas of several hundred square
111 kilometers with up to mm accuracy and a minimum of effort for the recipient of the data.

112 However, there are also considerable problems using DInSAR. The InSAR-specific effects regarding
113 overlay, foreshortening and shadowing (Speck et al., 2007) prevent the complete surface coverage.
114 Fast surface movements can lead to a decorrelation of multi-temporal InSAR images (Bingyuan et al.,
115 2008). Closed snow- and dense vegetation cover entirely prevent the capture of surface movements
116 using DInSAR. However, the major disadvantage of DInSAR is that surface changes are only displayed
117 one dimensionally along the line of sight (LOS) of the satellite, making it difficult to infer the type of
118 movement and its exact kinematic characteristics. Additional information is therefore necessary to
119 correctly interpret DInSAR data. This information can be obtained via a spatially and temporally
120 limited reconnaissance campaign using a different measurement system that provides information
121 on the 3D surface kinematics. The three dimensional interpretability of the DInSAR signal can thus be
122 established for individual sites by referencing it to the reconnaissance campaign. Ideally DInSAR
123 measurements, supported by process information obtained from such local reconnaissance
124 campaigns can then be used to perform an autarkical long term monitoring and provide estimations
125 of the 3D kinematics. The complementary measurement system applied for the reconnaissance
126 campaign must be embedded in the measurement geometry of DInSAR and be suitable to acquire
127 information on the unknown detailed process kinematics. We therefore defined requirements for the
128 complementary measurement system:

129 - The reconnaissance campaign is temporally limited, so a high accuracy is required to capture even
130 slow movements

131 - Measurement results should be provided in direct 3D

132 - If not already available, a digital elevation model (DEM) should be derivable from the measurement
133 data to determine slope and to project the DInSAR data onto

134 - In some cases the moving terrain is not accessible and must be monitored remotely

135 Terrestrial laser scanning (TLS) offers adequate solutions for all these issues but depends on an
136 unchanging surface structure for creep detection (see 3.4.) and its accuracy is limited to a few cm.

137 These limitations can be narrowed, if necessary, using single point measurements such as GNSS or
138 terrestrial surveying with a total station. Both provide a higher accuracy - up to mm (table 1) and
139 capture movements based on the tracking of individual points. Their disadvantages include the need
140 for accessible terrain and the spatially sporadic data records in contrast to the area-covering TLS
141 method. In our pilot study we tested both TLS and GNSS as complementary methods to DInSAR.

142

143 **3.2. Reference framework**

144 Each of the measurement systems used has its own reference system. GNSS and DInSAR operate
145 within the World Geodetic System 1984 (WGS84) but their registration can differ due to satellite
146 orbit uncertainties. The TLS data are referenced in a local frame of control points. To create
147 coherence between the measurements these reference frames must be linked. We therefore
148 installed two local terrestrial control points. Theoretically a third linking point would be necessary to
149 join the reference frames exactly; however, due to the limited spatial extent of the study site and the
150 high temporal stability of the reference frames we could neglect the remaining deviations between
151 the reference frames. The linking reference points were set on stable bedrock and were equipped
152 with an artificial InSAR reflector, a GNSS antenna base and a TLS reflector (Figure 2). The linking
153 points acted as stable reference points for the TLS and DInSAR measurements and the GNSS base
154 stations were located on them. The InSAR corner reflectors consist of an orthogonal open aluminium
155 trihedral with a short edge length of 57 cm (Mair et al., 2016 in Press). This size results in a
156 theoretical radar cross section of approximately 20 dB for the COSMO-SkyMed© X-band data used.
157 To reduce the weight of the reflector and to facilitate snow melt, a regular grid of circular holes was
158 drilled in the 3 mm aluminium plates. The hole diameter of 6 mm does not interfere with the
159 wavelength used by the satellite system. The TLS reflectors consist of metal plates laminated with
160 retro-reflecting foil. In addition to the linking points four other reflectors were used to define the TLS
161 reference frame. They were mounted on stable bedrock at various distances from the scan position,
162 i.e. shorter distances for the positioning and longer ones for the orientation of the scanner.

163

164 **3.3. GNSS**

165 Relative GNSS measurements were carried out using two base stations in stable terrain and one
166 rover for the data acquisition in the mass wasting zone. The double frequency sensors Leica Viva
167 GS10 and Leica GS530 were used. A network of GNSS measurement platforms was installed, taking
168 into account the expected surface kinematics and the geometrical GNSS requirements (i.e. satellite
169 visibility, baseline length). The platforms consisted of measurement poles mounted on large
170 boulders; 14 measurement points were defined in addition to the base stations (Figure 1). One GNSS
171 campaign was carried out in summer 2012 and three during the summers 2013 and 2014 (table 2).
172 To obtain highly accurate results, each point was measured in static mode for at least 30 minutes
173 with a 5 second sampling rate.

174 The post-processing of the collected GNSS data was performed using the 'Leica Geo Office' software,
175 by Leica Geosystems. Baselines of all rover points to both base stations were calculated and the
176 relative coordinates of these points were defined. The comparison of the baselines from two
177 different base stations allowed the identification of large errors. Random errors of single points were
178 minimized by calculating a least squares adjustment of the GPS network baselines. In order to refer
179 the local net to the global reference system the base stations were connected to permanent
180 reference stations belonging to the Swiss geodetic network (SWIPOS). The results of two
181 measurement campaigns A and B were referred against each other by correcting the point
182 coordinates of the repeat measurement B by the deviation A-B of the barycenter between the local
183 reference stations. The degree of precision between two measurement campaigns is equivalent to
184 the accuracy of the displacement values. This precision was equivalent to the remaining deviation of
185 the reference station coordinates between two measurement campaigns, after they have been
186 corrected by Barycenter bias (accuracy specifications are given in the Results section).

187 For calculating displacement velocities from the GNSS data we carried out a pre-selection of rover
188 points. A few of these rover points showed movements below the level of significance (Figure 7), a

189 few others could not be measured during each campaign due to logistic reasons and thus show data
190 gaps. Both types of rover points were therefore excluded from the calculation. For the remaining
191 points a mean velocity per measurement interval was defined.

192

193 **3.4. TLS**

194 Annual TLS measurements have been carried out for the entire site since 2009. Shorter measurement
195 intervals showed no significant movement results. The instruments used were a Riegl LPM321, and
196 from 2013 onwards a Riegl VZ6000. Both scanner systems are specified for long range applications
197 (for technical details see: www.riegl.com). Measurements were carried out from a single fixed scan
198 platform on the mountain ridge Mout da Barba Peider (Figure 1). The scans were performed with a
199 resolution of < 10 cm (range dependent). Referenced images of the scan areas were taken using the
200 camera integrated in the scanners. To reference the data, six retro-reflecting reference points
201 including the reference frame linking points were scanned at a very high resolution. The absolute
202 coordinates (WGS84) of the reference points were defined in advance, using a Leica TPS1200 total
203 station.

204 The point clouds acquired were filtered to remove outliers and to homogenize the spatial resolution.
205 To achieve an optimal relative referencing of the multi-temporal scans, the iterative closest point
206 (ICP) algorithm was applied to match unchanged terrain parts in the scan (Chen and Medioni, 1991).
207 Subsequently the set of relative registered point clouds was transformed into global coordinates
208 using the least squares adjustment of all observations on reference points carried out for the single
209 scans. The point clouds were then transformed into grid based digital elevation models (DEM) with
210 20 cm resolution. Small data gaps were filled using a 3x3 cell mean interpolation.

211 Horizontal 2D displacements (i.e. rock glacier creep) were derived from the multi-temporal DEM
212 using the surface structure of the blocky terrain. The DEM grids were filtered using a high pass filter
213 which removed information about the raw topography from the grid and only conserved the high

214 frequency surface structure (blocks and boulders). These surface models were grey value scaled to
215 visualize the surface pattern and saved as images. Subsequently image patches of 10x10 m were
216 matched using the particle imaging velocimetry correlating method introduced by Roesgen and
217 Totaro (1995). This algorithm produced a 2D displacement vector for each patch. The resulting vector
218 field was filtered to eliminate faulty correlations (Kenner et al., 2014).

219 Regarding the vertical component of the displacement, a simple difference DTM would show the
220 surface change on a specified location in the reference system. The GNSS and DInSAR methods track
221 the movement of a single surface point or surface patch instead. This methodical difference would
222 not allow a direct comparison of the vertical displacement components between the different
223 measurement systems. An additional processing step was therefore applied to the TLS data. Initially,
224 the DEMs of the first measurement t_1 and a repeat measurement t_2 and the horizontal 2D
225 displacement vector field between both measurements were used. For the initial position of each
226 horizontal 2D displacement vector, the elevation value was defined by calculating a weighted
227 average between the four closest cell centres of DEM t_1 surrounding the point. Subsequently the
228 displacement value was added to the initial positions of the displacement vectors and the elevation
229 calculation was applied to this second set of points using the DEM t_2 . Both elevations were
230 subtracted to define the change in elevation for each point pair. This elevation difference represents
231 the dZ component of the final 3D displacement vector field. Figure 3 summarizes this procedure.

232 An accuracy analysis of the TLS measurements at this site was previously carried out by Kenner et al.
233 (2014), detailing a methodical approach to define accuracy specifications for TLS displacement
234 measurements in position and elevation (accuracy specifications are given in the Results section
235 below).

236

237 **3.5. DInSAR**

238 The spatial and temporal resolution of space-borne SAR imagery were important characteristics on
239 which the selection of a satellite system was based on. Additional analyses were carried out including
240 a topographical analysis, applying distortion masks to simulate the layover, shadowing and
241 foreshortening effects. Based on this, the satellite platform COSMO-SkyMed, operated by the Italian
242 Space Agency and the Italian Ministry of Defense was selected. It consists of four satellites with a
243 revisit time of 8-16 days. To capture the West oriented mountain cirque, descending orbits were
244 used with a resulting off-nadir angle of 30.620 degrees. The SAR acquisition was carried out in
245 stripmap mode with a nominal spatial resolution of 3 m.

246 Multi-temporal DInSAR uses stacks of SAR images acquired with the same geometry and exploits the
247 redundant information of phase difference to measure ground displacements. The available
248 techniques can be grouped in two main classes: Persistent Scatterers (PS; Ferretti et al., 2001) and
249 Small BAseline Subsets (SBAS; Berardino et al., 2002). Both methods can deliver displacements along
250 the line of sight (LOS) direction with accuracy in the order of a few millimetres per year (Pasquali et
251 al., 2014). In a recent paper, Barboux et al. (2015) applied PS and SBAS interferometry based on
252 TerraSAR-X for the monitoring of Swiss rock glaciers. One central finding of the aforementioned
253 study was that SBAS interferometry was able to detect maximum displacements ten times larger
254 than PS interferometry (3.5 cm/a versus 35 cm/a for SBAS) but inaccurate measurements due to
255 phase unwrapping errors were already observed for velocity rates larger than 20 cm/a. Referring to
256 the creep velocities of up to 25 cm/a we chose the SBAS technique developed by CNR IREA of Naples
257 (Lanari et al., 2004) for our data analysis. This technique uses the surface structure as natural scatter
258 and allows an analysis of the study area with a high spatial resolution. The study site, showing a
259 complete absence of vegetation and widespread rocks and boulders, was particularly suited for the
260 application of this algorithm.

261 The influence of atmospheric distortions such as refraction in SAR images declines with an increasing
262 number of images. To achieve a high accuracy, multi-interferometry of SAR data requires a minimum
263 of 20 images to effectively remove the distortions by applying spatial and temporal filtering

264 operations (Colesanti et al., 2003). The implementation of our concept showed that due to the short
265 snow free period at this elevation, only 5 images could be captured in 2012, 8 in 2013 and 9 in 2014.
266 This not only affected the accuracy of the results, but also the resolution of spatially differential
267 movements. The spatial resolution of the generated surface velocity maps was thus adapted to the
268 significance of the data and lowered to 15 m x 21 m.

269 Geometrical or electrical changes in the properties of the Earth's surface between data acquired at
270 different times are potential sources of temporal decorrelation. If the temporal decorrelation
271 between two consecutive images is high (usually a threshold of 0.7 is used), no reliable interferogram
272 or deformation map can be created. Moreover, if the deformation along the LOS is greater than half
273 the wavelength (i.e. 1.55 cm in the COSMO-SkyMed data) aliasing problems can occur. Similar
274 problems arise for parts of a scene covered by snow. In our study area, the snow-free period only
275 lasted for 3-4 months each year and thus hindered the determination of interannual displacements.

276 **3.6. Integration of multi methodical measurement results**

277 To compare the results acquired with GNSS and TLS, each GNSS displacement vector was linked to
278 the mean of all TLS displacement records in its 15 m x 15 m surroundings. The deviation between
279 both solutions was calculated for each data pair in all three spatial components. Subsequently the
280 mean absolute error deviation was specified for each directional component.

281 To allow a comparison of the DInSAR monitoring results and the results obtained from TLS and GNSS,
282 the 3D movement information of the latter had to be projected on the line of sight (LOS) of the SAR
283 satellite. In a Cartesian coordinate system both the displacement and the LOS of the radar sensor are
284 represented by a 3D vector. The scalar product of the displacement vector $\vec{a} = (a_x, a_y, a_z)$ and the
285 unit vector $\hat{b} = (b_x, b_y, b_z)$ of the radar LOS results in the projection of \vec{a} on \hat{b} . The TLS and GNSS
286 displacement vectors were projected on the SAR LOS to achieve comparability between the different
287 datasets.

288 The GNSS measurements were carried out with a similar temporal resolution as the DInSAR
289 measurements and were used as comparative data. All DInSAR displacement values in a 15 m radius
290 around each GNSS point were averaged and linked to the GNSS displacement result. These values
291 were normalized by time and then directly compared to define the absolute accuracy of DInSAR
292 monitoring results.

293 So far, data from different measurement systems were only compared with each other. In the next
294 step the three dimensional information of a TLS reference measurement is linked to DInSAR results
295 of another measurement period, to extrapolate these DInSAR results into 3D. The information
296 content of the DInSAR is thus clearly improved. To do this, TLS displacement data was transformed
297 into an extrapolation mask for DInSAR in the following way: TLS displacement vectors \vec{a} for the
298 monitoring period 2013-14 were projected onto the LOS of the SAR sensor. This projection is called \vec{c} .
299 Subsequently all three spatial displacement components of \vec{a} were expressed as a fraction of the
300 length of the vector \vec{c} (equations 1-3).

301 (1) $a_x = u \cdot |\vec{c}|$

302 (2) $a_y = v \cdot |\vec{c}|$

303 (3) $a_z = w \cdot |\vec{c}|$

304 Using the vector specific fraction parameters u , v and w a 1D DInSAR displacement value can be
305 extrapolated to 3D under the assumption that the creep directions remain constant. We
306 extrapolated the DInSAR displacements in summer 2013 to 3D by splitting each DInSAR displacement
307 value into the components dx , dy and dz by multiplication with the mean fraction parameters of all
308 TLS vectors in their 15x15 m vicinity.

309 In a following step, the reliability of the DInSAR derived 3D dataset was tested. Although TLS and
310 DInSAR datasets were now both available in 3D, they could not be compared directly due to their
311 different temporal resolution. TLS measurements were taken with an annual resolution, whereas no
312 interannual DInSAR processing was possible. However, the relative spatial distribution of the creep

313 velocity was assumed to be similar between the different but temporally close observation periods of
314 both datasets. This implies that the ratio in velocity between a fast moving zone and a slow moving
315 zone should be approximately the same for both methods. Although the relative velocity pattern
316 remained constant, the absolute displacement values differed between different observation periods
317 due to: a) temporal creep velocity differences and b) different durations of the monitoring intervals.

318 To eliminate this scale difference between both datasets, the TLS displacement vectors were scaled
319 on the DInSAR vectors. The scaling factor was defined using the ratio of the mean norm of all DInSAR
320 displacement vectors and the mean norm of all TLS displacement vectors. Subsequently root mean
321 square error and mean absolute deviation between the datasets were calculated.

322

323 **4. Results**

324 **4.1. Results and comparability of GNSS and TLS**

325 The individual results of GNSS and TLS measurements were plausible and coherent. TLS captured
326 highly differential surface movements with a high area covering resolution (Figure 4). GNSS single
327 point measurements confirmed the vector field obtained by TLS and provided a higher temporal
328 resolution during the summer months (Figure 5).

329 The 3D mean absolute error (MAE) between displacement records of GNSS and TLS was found to be
330 3.7 cm and the root mean square error (RMSE) 3.4 cm. There is no significant difference in the
331 deviation for the single directional components. The deviations are slightly higher than the TLS
332 precision of 3.3 cm for the position, and 3 cm for the elevation and clearly greater than the GNSS
333 precision of 2 mm on the East-, 0.8 mm on the North- and 6 mm on the elevation component.

334

335 **4.2. Results, validation and integration of DInSAR measurements**

336 The rock glacier movement at Foura da l'amd Ursina could also be verified with the satellite-borne
337 DInSAR measurements. As mentioned in the method section the DInSAR displacement solutions only
338 exist for individual summer seasons and not on a full annual basis. A first comparison shows that the
339 zones of mass movements detected by DInSAR are almost the same as the zones detected by
340 terrestrial laser scanning. These zones correspond to the geomorphologically defined spatial extent
341 of the rock glaciers (Figure 6). DInSAR captured the mass movement area with a similar reliability as
342 TLS at this site, yet with a lower spatial resolution. Unfortunately, movement rates could not be
343 quantified in some of the fast moving zones of the rock glacier complex by DInSAR. No correlation
344 between the limited number of SAR images could be established here by the DInSAR processing. As a
345 couple of GNSS basements are located in these fast moving zones the comparison of GNSS and
346 DInSAR displacement results is based on a small statistical baseline. This comparison was intended to
347 give an estimation of accuracy and reliability on the DInSAR results. However, we obtained
348 differences between both methods that cannot be explained by the error budgets alone. Figure 7
349 shows the differences of GNSS and DInSAR displacement records projected on the LOS of the radar
350 sensor. The values are given in mm per day and are based on the monitoring period 2013 (61 days for
351 GNSS and 76 days for DInSAR) (table 2). Although the 3D GNSS displacements include larger error
352 influences compared to the 2D solutions, they are probably more relevant in this figure, as the
353 elevation component of the movement is disproportionately strong represented in the LOS
354 projection. Parts of the differences between GNSS and DInSAR are related to measurement errors.
355 The error bars for the GNSS measurements are included in the figure. Certainly also the DInSAR
356 results show sub-optimal accuracy. This is likely due to the relatively low number of available
357 acquisitions; as a rule of thumb, an accurate SBAS analysis requires 20-25 images – however, for our
358 analyses, a lower number of images was available. Another problematic issue is the uneven
359 distribution of the images over time. Moreover, the analysed deformation phenomena are
360 characterised by fast ground displacements (i.e. in relation to the X-band wavelength) and high
361 spatial (Figure 4) and temporal (Figure 5) heterogeneity. These conditions are not ideal for DInSAR

362 applications and lead to errors in the phase unwrapping, particularly in fast moving areas. Another
363 error source contributing to the deviations between GNSS and DInSAR in Figure 7, is the spatial
364 averaging of the DInSAR displacements over a 15m radius around each GNSS monitoring point. While
365 the GPS value represents the displacement of one point, DInSAR represents the displacement of a
366 large area surrounding this point. However, also GNSS points that are surrounded by homogeneous
367 DInSAR displacement values (Figure 6, e.g. point 18 and 27) show high deviations (Figure 7).
368 Another explanation for the deviations is therefore, that the GNSS single point measurements
369 include small scale surface movements that superimpose the large scale rock glacier creep and
370 disturb the creep signal, especially for short measurement periods (Wirz et al., 2014). These small
371 scale effects e.g. the toppling or subsiding of single rocks are not included in the DInSAR signal that is
372 achieved by the correlation of larger surface patches. An accuracy assessment using single point
373 measurements is therefore only reasonable for a longer monitoring period.

374 The 3D vector field extrapolated from DInSAR displacement values with the help of the TLS
375 extrapolation mask showed visually plausible results (Figure 8). Unfortunately, large parts of the
376 moving area are not included in the dataset: on the one hand the areas of DInSAR decorrelation and
377 on the other the areas in the TLS shadow. The comparison with the scaled TLS 3D vectors resulted in
378 an MAE of 1.1 cm and a RMSE of 1.5 cm. The highest displacement values within the reference
379 period, i.e. the DInSAR monitoring period of 2013, were around 6cm. We can therefore consider the
380 relative velocity pattern of TLS and DInSAR as significantly coherent.

381

382 **5. Discussion**

383 Space-borne DInSAR is an established method to detect and monitor large scale surface
384 displacements or numerous small scale displacements that are distributed over large areas. One aim
385 of this pilot study was to identify an appropriate complementary method for large scale DInSAR data
386 at local scales and for a limited time period to provide 3D movement information of individual mass

387 movements. TLS and GNSS were tested as complementary methods. When comparing TLS and GNSS
388 directly, we found consistent results; the deviations between both were mainly controlled by the
389 measurement precision of the TLS system. Static GNSS showed the higher precision, thus providing
390 the possibility of a higher temporal measurement resolution or rather shorter significant monitoring
391 intervals. This advantage turned out to be of limited value when comparing both complementary
392 methods to DInSAR. GNSS did indeed offer more accurate displacement values, however referring to
393 time scales of weeks to a couple of months, the GNSS measurements included deviations to DInSAR
394 that are probably not explainable by error budgets of both measurement systems alone. Possibly the
395 GNSS data include short term surface movements, which are not captured by DInSAR or TLS. Small
396 scale surface movements of the blocky surface (toppling, subsidence or acceleration of single
397 boulders) induced by the creep process may have distorted the signal of the large scale creep process
398 in the GNSS single point measurements (Wirz et al., 2014). In the short term, the deviations between
399 both measurement systems occasionally reached magnitudes higher than the creep process itself
400 and caused a total misalignment with the DInSAR results. An accuracy analysis of the DInSAR results
401 using GNSS data was difficult due to the unknown origin of these deviations. However these
402 deviations are random and become less significant with increasingly long measurement periods.
403 Therefore the phenomenon was not observed in the comparison of GNSS and TLS, as these results
404 were compared over a much longer time span. The GNSS advantage of high accuracy associated with
405 higher temporal resolution was nevertheless almost invalid. Instead, the weakness of GNSS in
406 relation to TLS became more relevant: this is mainly the very low spatial resolution of the data. It is
407 highly improbable that meaningful conclusions can be drawn from the GNSS displacements mapped
408 in the upper section of Figure 5 regarding the rest of the rock glacier complex. Nevertheless, it is still
409 possible to derive an overview of the seasonal creep velocity signal for the entire site out of the GNSS
410 data by using the spatial redundancy of multiple GNSS monitoring points to filter out the small-scale
411 distortions (lower section of Figure 5).

412 TLS was the more convincing complementary method to DInSAR in this study. It allowed the creation
413 of an area-wide 3D extrapolation mask for the DInSAR displacements and showed a high degree of
414 relative conformity with them. This is due to the similar procedure of displacement tracking of these
415 methods. Both correlate surface patterns of several square meters to detect movements. In contrast
416 to GNSS measurements, small-scale surface movements were therefore not captured; the movement
417 signal rather represents the movement of the entire rock glacier body instead. Moreover, TLS has the
418 great advantage of being a remote sensing method avoiding time consuming and potentially
419 dangerous work directly on the difficult terrain of the monitored object. However, in contrast to
420 GNSS, TLS fails due to visibility disruptions by cloudage, fog, snow coverage or terrain shadows.

421 For this study site, space-borne DInSAR measurements did not meet all the expectations of our
422 study. This was mainly due to the loss of SAR image correlation caused by a) the long period of snow
423 coverage and b) the too low frequency of SAR image acquisitions in relation to the rock glacier
424 movement. As both issues are valid for many high alpine mass movement zones, this must be
425 considered as being a characteristic limitation of the method. The long snow period not only
426 prevents the capture of inter-annual deformation time series, it also lowers the spatial resolution and
427 accuracy of intra-seasonal results. The decorrelation in fast moving terrain parts can theoretically be
428 limited with shorter measurement intervals. However, the planning of data acquisition is subject to
429 restrictions by the provider and this study used the highest temporal resolution available. Our study
430 area is small and not representative for all alpine mass movements. It is therefore likely, that other
431 problems and restrictions may occur when applying this concept to other alpine sites. In particular,
432 shadow effects inherent to the remote techniques TLS and DInSAR might play a much more
433 important role at other sites.

434 Apart from these limitations, DInSAR delivered reliable and plausible results and was in general
435 suitable for our measurement concept. The common reference frame proved to be stable for all
436 three methods and allowed data comparability. The DInSAR extrapolation mask created using the 3D
437 TLS information allowed a simple 3D projection of the radar data. In contrast to the original DInSAR

438 deformations, these 3D vector fields can be interpreted intuitively and allow a rapid recognition of
439 potentially hazardous accelerations: Depending on the SAR geometry, the raw DInSAR displacements
440 results show sometimes just a small fraction of the actual movement; the movement velocity and so
441 the absolute magnitude of a possible acceleration are strongly underestimated. This can lead to
442 misinterpretations regarding the risk assessment. The 3D extrapolation allows the estimation of the
443 real velocity and acceleration values. Moreover, the comparison with the TLS monitoring results
444 showed that the DInSAR method captured spatial velocity differences with a similar reliability as TLS.

445

446 **6. Conclusions**

447 Summarising, the applied measurement concept is a sufficient way of rationalising the monitoring
448 efforts for selected mass movements. This selection is mainly based on the applicability of
449 spaceborne DInSAR measurements regarding overlay, foreshortening and shadowing as well as the
450 duration of Snow coverage and the continuous availability of SAR images in a sufficient frequency.
451 Furthermore the movement directions of the mass movement have to stay constant and the
452 application of TLS or other terrestrial measurement systems must be feasible. Apart from this
453 limitations the initially defined requirements cost-efficiency, long term monitoring, self-operating
454 mode and large scale application with cm accuracy are fulfilled.

455 Additionally we can draw the following conclusions:

- 456 - Referring to the initially defined requirements, TLS was shown to be the more suitable
457 complementary measurement method for DInSAR. The reasons are the similar tracking
458 method for surface displacements, the high spatial resolution and the efficient remote
459 sensing. It should be noted that there may be different results for other sites.
- 460 - On short time scales up to months, GNSS and DInSAR displacement values show large
461 deviations. They are probably the result of small scale surface movements included in the
462 GNSS signal or originate from error sources which could not be identified within this study.

- 463 - Space-borne DInSAR showed considerable limitations in alpine terrain due to a short
464 measurement season (long snow coverage), spatially differential creep velocities and
465 complex terrain. This led to spatial and temporal data gaps and a reduced accuracy and
466 spatial resolution.
- 467 - Nevertheless, reliable and valid 3D results could be extrapolated from the DInSAR records
468 with the help of a single TLS repeat measurement. These results are easy to interpret and
469 include considerably more information.
- 470 - Considering its limitations, a suitable practical application for space-borne DInSAR in
471 combination with a complementary reconnaissance method is the monitoring of extensive,
472 slightly active, long term mass movements that require observation. Furthermore, the area
473 wide acquisition and analysis of DInSAR data allows to detect hitherto unknown mass
474 movements which can then be investigated using the presented monitoring concept to
475 facilitate the early recognition of hazardous areas.

476

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580

Table 1: Most frequently applied monitoring technologies for mass movements in alpine terrain

	GNSS	Total Station	Terrestrial laser scanning	Air-borne laser scanning	Air-borne photogrammetry	Terrestrial Radar	Space-borne SAR
Platform	Terrestrial: On site	Terrestrial: On site	Terrestrial: Ground base close to site	Remote: Aeroplane/ Helicopter	Remote: Aeroplane/Drone	Terrestrial: Ground base close to site	Remote: Satellite
Highest possible Spatial resolution	Single Point measurement	Single Point measurement	few cm	> 50cm	> 25cm	> 1 m @ 1 km	> 1m
Spatial coverage	Local	Local	Local	Regional	Supra-regional	Local	Supra-regional
Dimensionality	Direct 3D point coordinates	Direct 3D point coordinates	Direct 3D surface coordinates	Direct 3D surface coordinates	Indirect 3D surface coordinates	Direct 1D coordinate differences	Direct 1D coordinate differences
Highest accuracy	>mm	mm	>cm	dm	sub-dm	mm	mm
Image information	Not available	Not available	Referenced images	Not available	Multiband image information	Not available	Not available
Natural radiation	Independent	Independent	Independent	Independent	Dependent	Independent	Independent
Topographical effects	Satellite shadowing	Surface shadowing	Surface shadowing	No influence	In steep terrain	Surface shadowing and layover	Surface shadowing and layover
Vegetation	No Influence	No influence	Influence	Influence	Influence	Influence	Influence
Effort (time for data acquisition, logistics, costs)	High, Manual measurement on every single point	High, Manual measurement on every single point	Medium, remote sensing over long ranges, Close to site access required	High, Long flight time, expensive	Medium, Efficient area coverage, expensive	Medium, Automatic operation, heavy equipment,	Low, Automatic operation, efficient, reasonable prices

Table 2: Dates of the measurement campaigns.

	TLS	GNSS	DInSAR
2009	Sep, 11	-	-
2010	Aug, 04	-	-
2011	Aug, 18	-	-
2012	Sep, 18	Sep, 18	Jul, 21 Aug, 06 Aug, 22 Sep, 07 Sep, 23
2013	Aug, 29	Jul, 24-25 Aug, 29 Sep, 23.	Jul, 24 Aug, 09 Aug, 22 Aug, 25 Sep, 07 Sep, 10 Sep, 14 Sep, 09
2014	Aug, 15	Jul, 09 Aug, 18-19 Sep, 30	Jul, 15 Jul, 23 Aug, 16 Aug, 25 Aug, 28 Sep, 13 Sep, 29 Oct, 03 Oct, 12

Figure Captions

Figure 1: Study site Ursina with measurement configurations. The area covered by DInSAR includes only these regions in which displacement rates or a proof of stability could be deduced from DInSAR data.

Figure 2: Example of a multi-functional control point. The TLS reflector target (bottom right), the InSAR Corner and on it the basement for the GNSS reference station with attached GNSS antenna.

Figure 3: Sketch showing the mode of extracting the z component of a movement from two TLS DTMs.

Figure 4: Results of the TLS measurements between 2009 and 2014. Horizontal 2D creep rates are shown for visualization. The vertical differences represent the differences between the DTMs.

Figure 5: Velocities of the GPS point displacements during the monitoring period 2012-2014, in spatial resolution (top) and temporal resolution (bottom).

Figure 6: sdkdjaksjh

Figure 7: Differences between GNSS displacements projected in the line of sight of the radar and the surrounding DInSAR displacement values; the values are based on the summer monitoring period 2013 (61 days for GNSS and 76 days for DInSAR). Systematic deviations are evident.

Figure 8: 3D Vector field extrapolated from DInSAR measurements using a TLS extrapolation mask.