

# The onset of sediment transport in vegetated channels predicted by turbulent kinetic energy

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## Key Points:

- The critical velocity for sediment motion decreases with increasing vegetation density
- Near-bed turbulent kinetic energy describes the critical condition for sediment motion for both bare and vegetated channels
- A model for critical velocity based on near-bed turbulence unifies bare-bed and vegetated channel conditions

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## Abstract

This laboratory study advances our understanding of sediment transport in vegetated regions, by describing the impact of stem density on the critical velocity,  $U_{crit}$ , at which sediment motion is initiated. Sparse emergent vegetation was modeled with rigid cylinders arranged in staggered arrays of different stem densities. The sediment transport rate,  $Q_s$ , was measured over a range of current speeds using digital imaging, and the critical velocity was selected as the conditions at which the magnitude of  $Q_s$  crossed the noise threshold. For both grain sizes considered here (0.6-0.85mm and 1.7-2mm),  $U_{crit}$  decreased with increasing stem density. This dependence can be explained by a threshold condition based on turbulent kinetic energy,  $k_t$ , suggesting that near-bed turbulence intensity may be a more important control than bed shear stress on the initiation of sediment motion. The turbulent kinetic energy model unified the bare-bed and vegetated channel measurements.

## 1 Introduction

Aquatic vegetation provides important ecosystem services [e.g. *Micheli and Kirchner, 2002; Kremen, 2005; Danielsen et al., 2005*], whose global value has been estimated to be in the tens of trillion dollars per year [*Costanza et al., 1997*]. However, a large amount of aquatic vegetation has been lost in recent decades, including over half of the wetlands and thousands of acres of seagrass in the U.S [*Marani et al., 2011*]. An understanding of sediment transport in vegetated regions is essential for vegetation restoration because the evolution of vegetated landscape occurs through the interplay of flow, vegetation and sediment accretion. Yet, there are currently no predictive models for sediment transport in vegetation. This study takes a first step toward developing a sediment transport model by quantifying the incipient conditions for sediment transport in vegetated regions.

In a bare channel the critical velocity defining incipient sediment motion,  $U_{crit}$ , has historically been related to the time-mean bed shear stress ( $\tau$ ) [*Shields, 1936*]. However, more recent studies support the role of turbulence in initiating sediment motion [*Heather-shaw and Thorne, 1985; Nelson et al., 1995; Diplas et al., 2008*]. In a bare channel, the role of turbulence may be inherently represented in the Shields diagram because the turbulent kinetic energy and  $\tau$  are linearly related [*Stapleton and Huntley, 1995*]. In a vegetated channel, however, the turbulence is predominantly generated by the vegetation [*Tanino and Nepf, 2008a; Stoesser et al., 2010*], such that  $\tau$  is no longer a surrogate for near-bed turbulence. This may explain why bed shear stress models based on open channel studies do

not work in vegetated channels [Yager and Schmeeckle, 2013; Hongwu et al., 2013; Tinoco and Coco, 2014].

In this paper, we assume that the near-bed turbulence plays the central role in initiating sediment motion, as also proposed by Nelson et al. [1995] and Diplas et al. [2008]. A prediction for  $U_{crit}$  is devised based on the near-bed turbulence generated by both bed shear stress and vegetation wakes. The model is shown to be consistent with measurements made in bare and sparsely vegetated channels.

## 2 Theory

Previous studies have shown that the initiation of grain motion is connected to the passage of turbulent eddies and the associated fluctuations in near-bed pressure, which generate sufficient instantaneous lift and drag forces to destabilize the grains [Smart and Habersack, 2007; Zanke, 2003]. Because the magnitude of the fluctuating drag and lift forces are correlated with the near-bed turbulent kinetic energy ( $k_t$ ), we propose using  $k_t$  as a predictor of the incipient condition of sediment transport. This proposal is supported by observations of sediment erosion over a bare bed and a bed with *Spartina anglica* at different stem densities [Widdows et al., 2008]. In the Widdows study, the relation for open channel flow,  $\tau = 0.19k_t$ , was used to estimate the mean bed stress from the measured turbulent kinetic energy  $k_t = (\overline{u'^2} + \overline{v'^2} + \overline{w'^2})/2$ , with  $u'$ ,  $v'$ ,  $w'$  denoting the velocity fluctuation [Stapleton and Huntley, 1995]. However, as discussed in [Nepf, 2012a; Ricardo et al., 2014; Yang et al., 2015], this relation assumes turbulence production is linked to bed stress, which is not true in vegetated systems, for which turbulence production is primarily associated with the vegetation. Therefore, Widdows' conclusion that the critical  $\tau$  was unchanged between bare and vegetated beds (Figure 6 in [Widdows et al., 2008]) was incorrect, and in fact their data actually shows that the threshold for erosion was defined by a critical value of  $k_t$ . Finally, the duration of the turbulence-driven lift and drag is also important, and may be characterized by an impulse parameter [e.g. Celik et al., 2010, 2013]. The duration is connected to eddy scale, which in turn is connected to the grain size and stem diameter, so that for a fixed grain size and stem diameter,  $k_t$  alone should set the critical threshold.

Over a bare channel, the near bed  $k_t$  is proportional to  $\tau$  [Stapleton and Huntley, 1995], and  $\tau$  is proportional to the time-mean, depth-average velocity squared  $U^2$  [Wilcock,

1996; Julien, 2010]. Specifically,  $k_{tb} = C_b U^2$ , with  $C_b$  a coefficient dependent on the bed roughness. In a vegetated channel, both the bed-generated turbulence and the vegetation-generated turbulence contribute to the near bed  $k_t$ . For simplicity, we assume that the total near-bed  $k_t$  is the sum of the two, neglecting any mutual influence. For a sparse emergent canopy, specifically  $d/s_n < 0.56$  with  $d$  and  $s_n$  denoting the stem diameter and the average surface-to-surface distance between the nearest stem neighbor, respectively, which we consider in this study, stem-scale eddies can exist throughout the canopy, so that the vegetation-generated turbulence can be described by equation (4.1) in [Tanino and Nepf, 2008a]:

$$k_{tv} = 1.2 \left[ C_D \frac{\phi}{(1-\phi)\pi/2} \right]^{2/3} U^2. \quad (1)$$

Here  $C_D$  is the stem drag coefficient and  $\phi$  is the solid volume fraction within the canopy. In order for equation (1) to apply, the stem Reynolds number  $Re_d (= Ud/\nu)$  must be larger than 120 for stem wake turbulence to be generated [Liu and Nepf, 2016]. In this study  $600 < Re_d < 2500$  and  $d/s_n \leq 0.25$  ( $\phi \leq 5\%$ ), so that  $k_{tv} \approx 1.2 \left( \frac{C_D \phi}{\pi/2} \right)^{2/3} U^2 \approx 0.9 C_D^{2/3} \phi^{2/3} U^2$ . Assuming that the total near-bed  $k_t$  is the sum of  $k_{tb}$  and  $k_{tv}$ , the total near bed  $k_t$  in a sparse emergent canopy can be estimated as:

$$k_t = C_b U^2 + 0.9 C_D^{2/3} \phi^{2/3} U^2. \quad (2)$$

If  $k_t$  sets the threshold for incipient sediment motion, then equation (2) can be used to predict the critical velocity. The critical  $k_t$  should be a function of the sediment size ( $d_s$ ). For vegetated and bare channels with the same  $d_s$ ,  $C_b U_{crit}^2 + 0.9 C_D^{2/3} \phi^{2/3} U_{crit}^2 = C_b U_o^2$ , with  $U_o$  denoting the critical velocity for a bare bed, i.e.  $U_{crit} = U_o$  when  $\phi = 0$ . Rearranging,

$$\frac{U_{crit}}{U_o} = \frac{1}{\sqrt{1 + C \phi^{2/3}}}. \quad (3)$$

in which the coefficient  $C = 0.9 C_D^{2/3} / C_b$ . Estimations of  $C_b$  and  $C_D$  can be found in [Julien, 2010] and [Tanino and Nepf, 2008b; Cheng and Nguyen, 2011], respectively. Hereafter equation (3) is referred to as the turbulence model for sparse vegetation. For dense vegetation ( $d/s_n \geq 0.56$ ), the turbulence generated in the stem wakes has a weaker dependence on solid volume fraction [Figure 14 and equation (4.1) in Tanino and Nepf, 2008a], so that  $U_{crit}$  is expected to have a weaker dependence on  $\phi$  compared with the sparse vegetation (equation (3)).

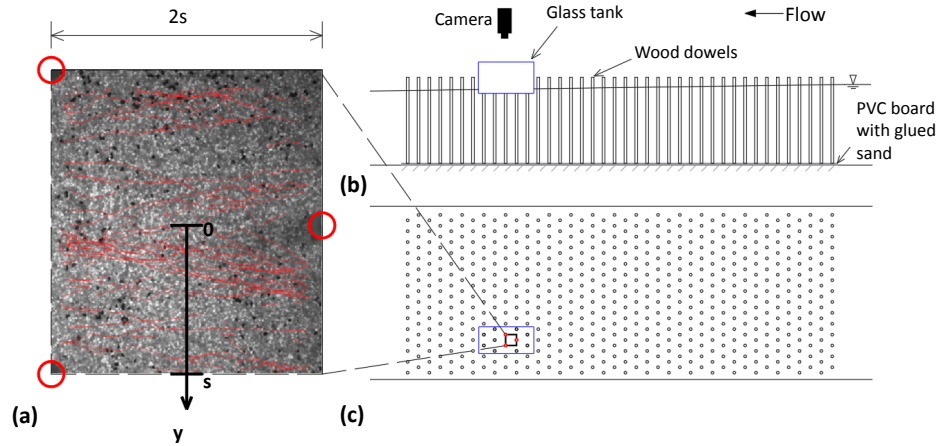
### 3 Methods

The model emergent vegetation was created using rigid circular cylinders with diameter  $d = 6.3\text{mm}$  fixed in a staggered pattern in PVC boards (Figure 1). The solid volume fraction ( $\phi$ ) ranged from 0.006 to 0.05, similar to conditions found in marshes [Nepf, 2012b]. One layer of sieved light-brown sand was glued to the PVC boards. Two sand sizes ( $d_s$ ) were used: 0.6 to 0.85mm and 1.7 to 2mm. The boards were placed in a horizontal recirculating flume with a 1-m wide and 10-m long test section. Flow was generated by a centrifugal pump and measured with an in-line flow meter with  $0.001\text{m}^3/\text{hr}$  precision. The measured flow rate,  $Q$ , was used to estimate the average channel velocity  $U = Q/(wh(1 - \phi))$ , with  $w$  and  $h$  denoting channel width and water depth, respectively. A digital camera was placed approximately 1m downstream from the leading edge of the boards to observe the sand motion (Figure 1b).  $h$  was measured close to the camera and was controlled to be 20 to 22cm.

A layer of black sand with the same size distribution as the light-brown sand was spread on top of the light-brown sand. The motion of the black sand was recorded at 60 frames per second with a 1280×960-pixels camera and a 35mm fixed focal length lens (Figure 1b). The original imaging area was around 10cm by 7.6cm with one grain diameter corresponding to 10 to 20 pixels. To capture the spatial heterogeneity of sediment motion, the imaging window contained an integral number of the repeated pattern of dowels (Figure 1a). To eliminate distortions in the image due to water surface movement, a small glass tank was positioned above the channel and extended less than 1 cm below the water surface (Figure 1c).

The following steps were used to estimate the sand transport rate,  $Q_s$ . First, in each frame the percentage of the pixels occupied by black sand grains was defined as the black sand occupancy,  $P_{blk}$ . Second, the trajectory of each black grain (Figure 1a) was identified using IDL particle tracking MatLab code written by Crocker and Grier [1996]. Third, the average streamwise velocity of the black sand grains ( $U_p$ ) was calculated from the identified trajectories, producing an average velocity for all particles over 30 seconds. The volume of particles in motion per unit bed area ( $\gamma$ ) was estimated from the number of moving particles averaged over the 1800 frames. The black sand transport rate was then calculated as  $Q_{blk} = U_p \gamma$  [Wong et al., 2007; Furbish et al., 2012]. Assuming sand motion only occurred in the top layer [Houssais et al., 2015] and all sand motion followed the

138 same probability distribution, the total sediment transport rate  $Q_s$  for a full bed of loose  
 139 grains can be estimated as  $Q_s = Q_{blk}/P_{blk}$ . We calculated  $Q_s$  for different imaging dura-  
 140 tions and found that  $Q_s$  converged to a constant value at less than 30 seconds.



141 **Figure 1.** (a) The gray-scale image of the sand bed with superimposed trajectories of moving black sand  
 142 (the red lines). The flow was right to left in the image. The red circles indicate the positions of dowels. LDV  
 143 measurements were taken along the  $y$  axis defined, as shown in (a), at the mid-point of the unit cell of the  
 144 repeating dowel pattern.  $y = 0$  (the center of the image) was in line with the upstream dowel and mid-way  
 145 between the upstream and downstream dowel rows.  $y = s$  (lower edge of the image) was in line with the  
 146 downstream dowel. (b) Side view of the test section. Vertical circular cylinders represent emergent stems of  
 147 vegetation. The digital camera, with polarizing lens, was positioned above a glass tank with 15cm width and  
 148 30cm length in the horizontal plane. Several dowels under the tank were cut to allow the bottom of the tank to  
 149 touch the water surface. (c) Top view of the test section shown in (b). The dowels were placed in a staggered  
 150 pattern. The black box with red circles inside the blue box (the glass tank) represents the region captured by  
 151 the camera.

152 For each vegetation solid volume fraction ( $\phi$ ) the sediment transport rate ( $Q_s$ ) was  
 153 measured at several channel velocities ( $U$ ). A reference video with no black sand was also  
 154 recorded for each flow condition and used to define the noise level. For each sediment  
 155 size, the maximum noise value was used as the threshold criterion for sediment motion,  
 156  $Q_{s-crit}$ . For each vegetation density, an upper and lower bound for  $U_{crit}$  was chosen such  
 157 that above the upper bound all the measured  $Q_s$  were larger than  $Q_{s-crit}$  and below the

158 lower bound all the measured  $Q_s$  were smaller than  $Q_{s-crit}$ . The mean between the upper  
 159 and lower bound was chosen as  $U_{crit}$ .

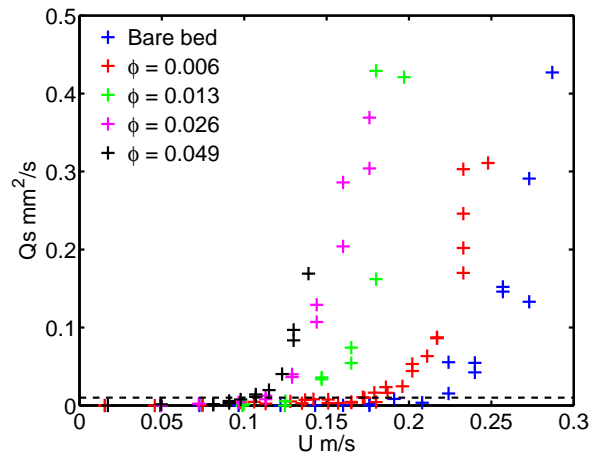
160 The instantaneous velocity ( $u, w$ ) was recorded at the critical condition ( $U = U_{crit}$ )  
 161 using backscatter Laser Doppler velocimetry (LDV) at 1mm above the bed. Here ( $u, v, w$ )  
 162 refer to the velocity along the  $x, y, z$  axes, corresponding to the streamwise, spanwise and  
 163 vertical directions, respectively. The distance to the bed (1mm) was chosen to character-  
 164 ize the near bed flow condition because the fluid structures that trigger particle motion  
 165 scale with sediment size [Vowinckel *et al.*, 2016]. The LDV probe (Dantec Dynamics)  
 166 was mounted on a manually-driven positioning system, and the velocity was measured  
 167 above the bed area where the sediment transport videos were recorded. For each velocity  
 168 record, 10000 samples were collected at frequencies from 5 to 100 Hz. For the major-  
 169 ity of the measurements, the sampling frequency was greater than twice the integral time  
 170 scale from the autocorrelation function [Nezu *et al.*, 1994; Kundu *et al.*, 2008], indicating  
 171 that the sampling frequency was sufficient to capture the characteristics of the turbulence.  
 172 The running average of turbulent statistics (including Reynolds stress and  $k_t$ ) converged  
 173 to stable values within 10000 velocity samples, consistent with a previous study [Buffin-  
 174 B elanger and Roy, 2005].

175 Because the 2-D LDV only measured the vertical ( $w$ ) and streamwise ( $u$ ) velocity  
 176 the turbulent kinetic energy was approximated as  $k_t = (2\overline{u'^2} + \overline{w'^2})/2$ . This approximation  
 177 is justified because previous measurements have shown that within an emergent array the  
 178 kinetic energy contributed by span-wise velocity fluctuation ( $\overline{v'^2}$ ) is approximately equal  
 179 to ( $\overline{u'^2}$ ) [Tanino and Nepf, 2007]. For the vegetated cases, velocity was recorded along a  
 180 lateral transect midway between rows at  $y/s = 0, 0.25, 0.5, 0.75$  and 1, where  $s$  is the dis-  
 181 tance between dowels defined in Figure 1a. For the bare bed, the velocity was measured at  
 182 two positions located laterally 5cm apart.

183 To show that the selected lateral positions captured the flow heterogeneity and pro-  
 184 vided representative spatial averages, we also measured the velocity for 3 minutes each at  
 185 18 vertical locations at the selected lateral positions. The average channel velocity esti-  
 186 mated from these vertical profiles agreed with the flow meter reading within 10%, indicat-  
 187 ing that our selected lateral positions were adequate to characterize the flow heterogeneity.  
 188 Furthermore, the LES results shown in figure (10) of [Stoesser *et al.*, 2010] confirm that  
 189 for sparse vegetation our lateral transect captured most of the flow heterogeneity.

## 4 Results

Above a critical channel velocity, the sediment transport rate  $Q_s$  increased steadily with increasing  $U$  (Figure 2). The black dashed line denotes the noise threshold,  $Q_{s-crit} = 0.01 \text{ mm}^2/\text{s}$ . Once sediment transport was initiated, the sediment transport rate was consistently higher for cases with model vegetation, compared to bare-bed, and at the same channel velocity the sediment transport rate generally increased with increasing vegetation solid volume fraction ( $\phi$ ).

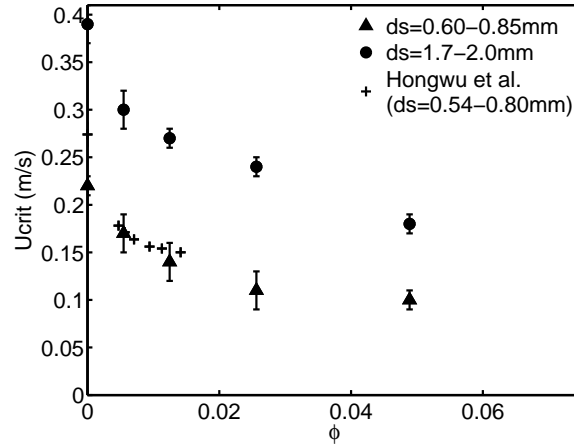


**Figure 2.** Sediment transport rate,  $Q_s$ , versus channel velocity,  $U$ , for bare bed and four vegetation solid volume fractions ( $\phi$ ). The sediment size  $d_s = 0.6$  to  $0.85 \text{ mm}$ . The horizontal dashed line denotes the critical sediment transport rate,  $Q_{s-crit} = 0.01 \text{ mm}^2/\text{s}$  defined based on the noise threshold.

The incipient velocity ( $U_{crit}$ ) was identified from the  $Q_s$  and  $U$  data as described in the Methods section. For both grain sizes the value of  $U_{crit}$  decreased with increasing vegetation solid volume fraction ( $\phi$ ), with the greatest drop between the bare bed and the sparsest stem density (Figure 3). As expected, the values of  $U_{crit}$  were higher for the larger grain size. *Hongwu et al.* [2013] estimated similar values of  $U_{crit}$  as a function of stem density for a grain size comparable to our smaller grain (data included in Figure 3).

The spatially-averaged near-bed turbulence,  $k_t$ , was measured at the critical condition ( $U = U_{crit}$ ) for each stem density (red symbols in Figure 4). To clarify overlapping points, the  $x$ -axis of measured  $k_t$  has been shifted right by 0.001. The lower and upper bound of measured  $k_t$  correspond to the value of  $k_t$  measured at  $U$  equal to the lower and upper bound of  $U_{crit}$ , respectively. Note that the spatial heterogeneity of  $k_t$  is much

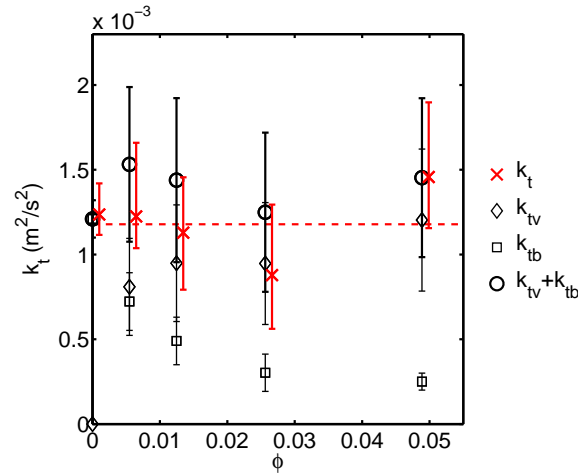




206 **Figure 3.** The critical velocity for sediment motion,  $U_{crit}$ , versus vegetation solid volume fraction,  $\phi$ . The  
 207 filled triangles and circles with error bars are from the present study. The crosses are from [Hongwu et al.,  
 208 2013].

214 smaller than the error of  $k_t$  due to the uncertainty in  $U_{crit}$ . Within uncertainty the critical  
 215  $k_t$  was constant across all stem densities and bare bed.

216 To further support the turbulence model, the near-bed turbulence was also estimated  
 217 using equation (2). The bed turbulence coefficient,  $C_b$ , was estimated from the measured  
 218 bare-bed values of  $k_t = 0.0012m^2/s^2$  and  $U_o = 0.22m/s$ , specifically  $C_b = k_t/U_o^2 =$   
 219  $0.025$ . Previous bare-bed studies have shown that the near-bed  $k_t \approx 5.3\tau/\rho$  [Stapleton  
 220 and Huntley, 1995] and  $\tau/\rho = C_f U^2$  [Wilcock, 1996]. For the conditions in this study  
 221 ( $h \approx 0.2m, d_s = 0.6 - 0.85mm$ )  $C_f \approx 0.0034$  (the Darcy-Weisbach friction relation  
 222 in [Julien, 2010]), from which a predicted  $C_b$  is 0.02, agreeing with the measure value  
 223 (0.025) within 25%. The value for  $C_D$  was approximated as  $1.0 \pm 0.5$  for our experimen-  
 224 tal conditions ( $600 < Re_d < 2500$ ) based on experimental data compiled in [Figure 4,  
 225 Cheng and Nguyen, 2011]. As  $\phi$  increased, the contribution from the bed-generated turbu-  
 226 lence  $k_{tb}$  (black squares in Figure 4) decreased significantly due to the decrease of  $U_{crit}$ .  
 227 In contrast, the estimated contribution from vegetation-generated turbulence  $k_{tv}$  (black di-  
 228 amonds in Figure 4) increased with increasing  $\phi$  because increasing the stem density pro-  
 229 vided more site for stem-wake turbulence generation (equation (1)). The predicted  $k_{tb} + k_{tv}$   
 230 (equation (2), or the black circles) agreed with the measured  $k_t$  (the red symbols) within  
 231 uncertainty, which both validated equation (2) and supported the hypothesis that the criti-  
 232 cal velocity was set by a threshold in near-bed  $k_t$ .

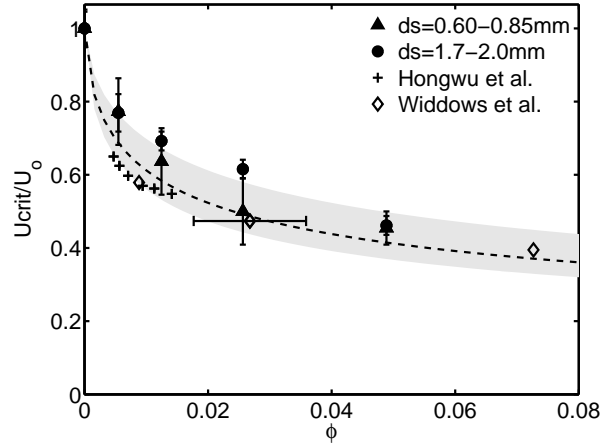


233 **Figure 4.** The measured turbulent kinetic energy ( $k_t$ , red symbols) and the predicted contribution from  
 234 bed-generated turbulence  $k_{tb}$  and vegetation-generated turbulence  $k_{tv}$  at the critical condition ( $U = U_{crit}$ ) for  
 235  $d_s = 0.6-0.85\text{mm}$ . The horizontal dashed line represents the average of measured  $k_t$  weighted by 1 over the  
 236 error of  $k_t$ . The errors in  $k_{tb}$  and  $k_{tv}$  were contributed by the uncertainty in  $U_{crit}$  and  $C_D$  through standard  
 237 propagation of error [e.g. Taylor, 1997]. For  $k_{tv}$ , the uncertainty in  $C_D$  contributed to the majority of its  
 238 error.

239 In Figure 5, the measured  $U_{crit}$ , normalized by the critical velocity for bare bed,  $U_o$ ,  
 240 is compared to the turbulence model described by equation (3). First, normalizing by  $U_o$   
 241 collapsed the two data sets (triangle and circles in Figure 5) within uncertainty, consistent  
 242 with equation (3). Second, the model prediction with  $C_b = 0.025$  and  $C_D = 1$  is shown  
 243 with a black dashed curve. The uncertainty in the predicted  $U_{crit}/U_o$  (gray shadow in  
 244 Figure 5)) arose mainly from the uncertainty in  $C_D (= 1 \pm 0.5)$ . Both of our experimental  
 245 data sets and the data from [Hongwu et al., 2013] (plus signs) agree with the turbulence  
 246 model within uncertainty.

247 The model also has good agreement with the measurements of Widdows et al. [2008]  
 248 which use natural vegetation (*Spartina anglica*) and natural mud with median grain size  
 249 around  $17.4 \pm 0.2\mu\text{m}$  (diamonds in Figure 5). In their study,  $U_{crit}$  was defined as the ve-  
 250 locity required to erode 10 grams of sediment per unit bed area, extrapolated from the  
 251 suspended mass versus velocity curve. We estimated the solid volume fraction of *Spartina*  
 252 ( $\phi$ ) as  $\pi d^2 n/4$  with  $n$  denoting the number of stems per bed area, which is reported in  
 253 the paper. *Spartina anglica* leaves are 1.5cm wide at the base and taper to a point, so that  
 254 0.75 cm was chosen as an estimation of  $d$ . The values of  $U_{crit}/U_o$  versus  $\phi$  (diamonds

255 in Figure 5) agree with the turbulence model within uncertainty, indicating that the turbu-  
 256 lence model can be extended to conditions with natural vegetation and fine sediment.



257 **Figure 5.** The incipient velocity of sediment motion  $U_{crit}$  normalized by the incipient velocity for a bare  
 258 bed with the same sediment size ( $U_0$ ). The black dashed curve with gray shadow represents the turbulence  
 259 model (equation (3)) with  $C_b = 0.025$  and  $C_D = 1 \pm 0.5$ . The black dashed curve corresponds to  $C_D = 1$ , and  
 260 the upper and lower edge of the gray region represent  $C_D = 0.5$  and  $1.5$ , respectively. The horizontal bar for  
 261 the diamond symbol at  $\phi \approx 0.025$  represents the range of  $\phi$  reported for this condition [Widdows *et al.*, 2008].

262 Finally, the difference between  $U_{crit}$  values measured with different grain sizes  
 263 (Figure 3) is consistent with  $U_{crit}$  set by a critical value of  $k_t$ . As noted by previous re-  
 264 searchers [e.g. Smart and Habersack, 2007], let's assume that the eddy-induced pressure  
 265 fluctuations initiate sediment motion. From Bernoulli's equation, the eddy-induced pres-  
 266 sure fluctuation  $P_e$  should scale as  $\rho U_e^2$ , with  $U_e$  denoting the eddy velocity. Because  
 267  $U_e^2$  is proportional to  $k_t$ ,  $P_e$  is proportional to  $k_t$ . Consider a grain of size  $d_s$ , at the crit-  
 268 ical condition the lift force on the grain,  $P_e \pi d_s^2 / 4$ , approaches or exceeds the weight of  
 269 the grain,  $(\rho_s - \rho) \frac{1}{6} \pi d_s^3 g$ , with  $\rho_s$  denoting the sand density and  $g$  denoting the gravita-  
 270 tional acceleration. Therefore, the critical values of  $P_e$  and  $k_t$  increase linearly with  $d_s$ .  
 271 Because  $k_t$  is proportional to  $U^2$  for the same  $\phi$  (equation (1)),  $U_{crit}$  is expected to in-  
 272 crease linearly with  $\sqrt{d_s}$ . In the present study, the ratio of  $\sqrt{d_s}$  for the two grain sizes was  
 273 between 1.4 and 1.8, thus the ratio of  $U_{crit}$  was expected to be  $1.6 \pm 0.2$ . The correspond-  
 274 ing ratio between the measured  $U_{crit}$  values (Figure 3) was  $1.9 \pm 0.2(SD)$ . The agree-  
 275 ment within uncertainty between the measured  $U_{crit}$  ratio and the expected ratio further sup-  
 276 ports the turbulence model.

## 5 Conclusions

Turbulence has been recognized to play an important role in initiating sediment motion. In a vegetated channel, the generation of turbulence in the wakes of vegetation elements exceeds that associated with bed-shear, such that previous bare-bed sediment-transport models based on bed shear stress alone do not work. This study developed a model for the critical velocity for the onset of sediment transport ( $U_{crit}$ ) based on a threshold of near-bed turbulent kinetic energy ( $k_t$ ). The model was validated by laboratory experiments conducted in both bare channels and channels with sparse model vegetation. At the critical condition when sediment started to move, the measured  $k_t$  was roughly a constant for all vegetation densities and bare bed (Figure 4), indicating that  $k_t$  may be a universal metric that predicts sediment motion in both bare and obstructed channels. Further, the  $k_t$  model for  $U_{crit}$  unified both the bare-bed and vegetated channel measurements (Figure 5). Previous experimental data with natural vegetation and natural mud also agreed with the new model within uncertainty, suggesting that the model can be extended to predict  $U_{crit}$  in natural conditions.

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