PAVEMENT REHABILITATION OPTIONS IN INDIANA FOR INDOT

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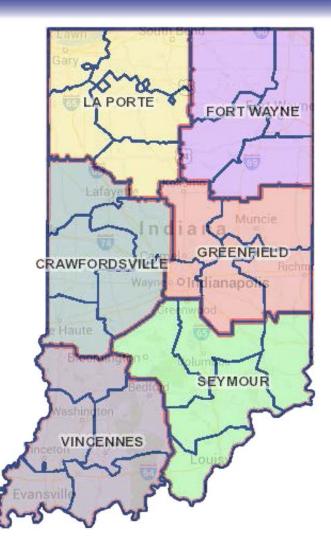
2015 Purdue Road School





INDOT Profile

- Six district offices
- 3,404 employees
- \$1 billion/annual capital expenditures
- 28,400 total roadway lane miles
- 5,300 INDOT-owned bridges
- Assists 42 railroads in planning & development of more than 3,880 miles of active rail lines
- Supports 69 Indiana State Aviation System Plan airports





The Stage

In 1818 the Institution of Civil Engineers was founded in London, and in 1820 the eminent engineer <u>Thomas Telford</u> became its first president. The institution received a Royal Charter in 1828, formally recognizing civil engineering as a profession. Its charter defined civil engineering as:

The art of directing the great sources of power in nature for the use and convenience of man, as the means of production and of traffic in states, both for external and internal trade, as applied in the construction of roads, bridges, aqueducts, canals, river navigation and docks for internal intercourse and exchange, and in the construction of ports, harbors, moles, breakwaters and lighthouses, and in the art of navigation by artificial power for the purposes of commerce, and in the construction and application of machinery, and in the drainage of cities and towns.



Institution of Civil Engineers, 1 Great George Str, Westminster, London

Indiana

The Stage

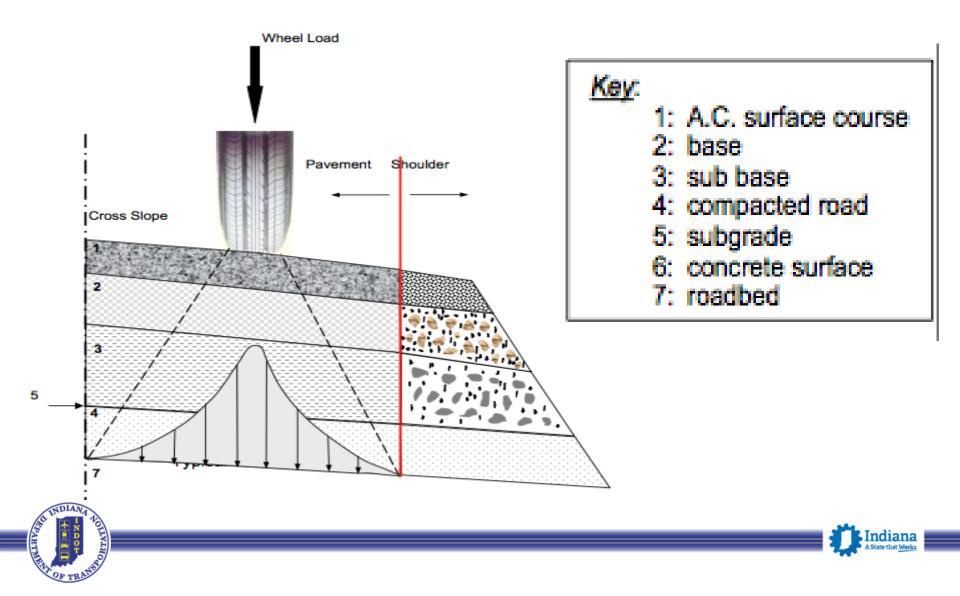
The art of directing the great sources of power in nature for the use and convenience of man, as the means of production and of traffic in states, ...



Institution of Civil Engineers, 1 Great George Str, Westminster, London



Definitions – Flexible Pavement



HMA pavement cross section



1.5" Surface
2.5" Intermediate
3"+ Dense graded base
3" Open graded base
3" Dense graded base

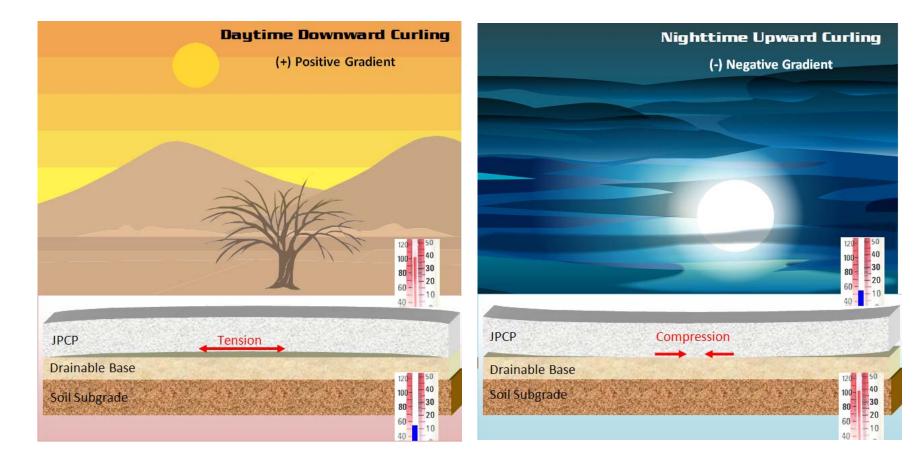
a 14" Soil treatment

Foundation Soil





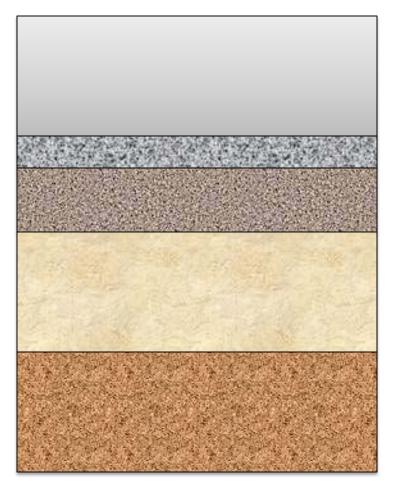
Stress and strain in rigid pavement

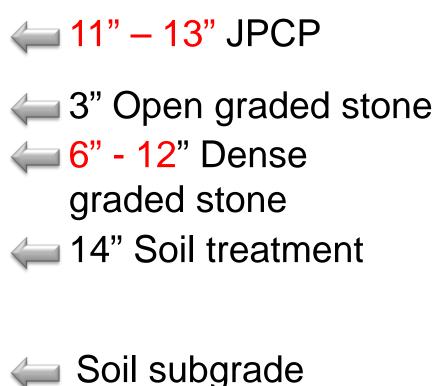






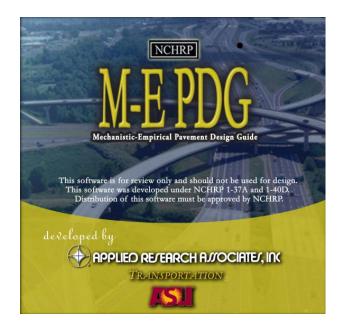
JPCP cross section





Foundation Soil

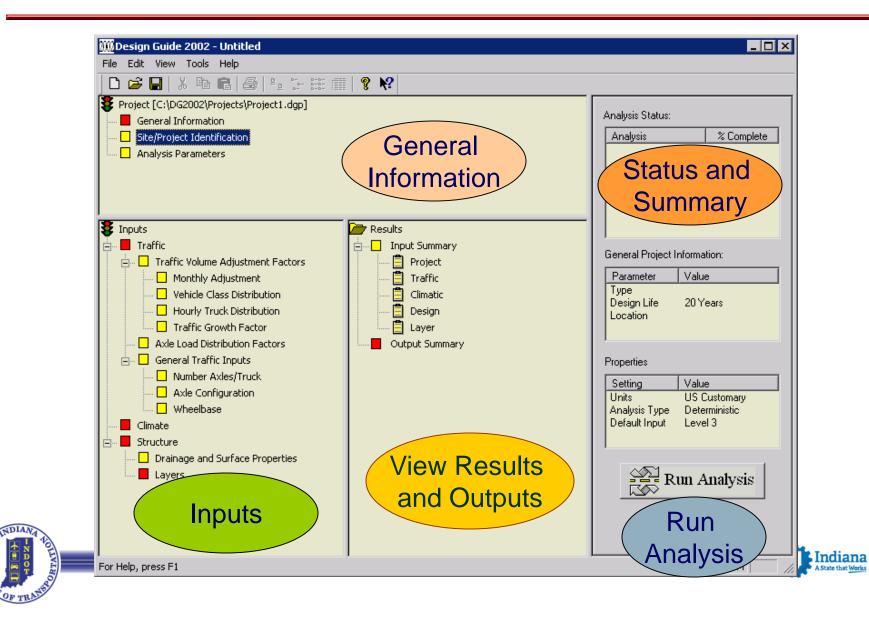




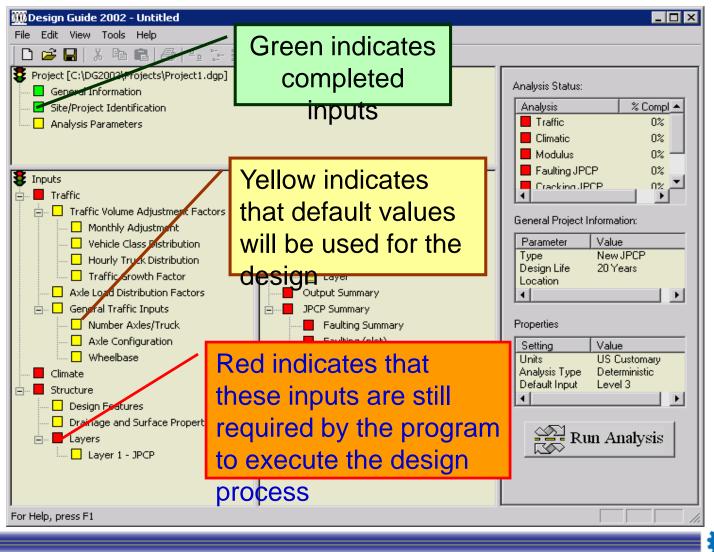




Main project screen



Color-coded status icons

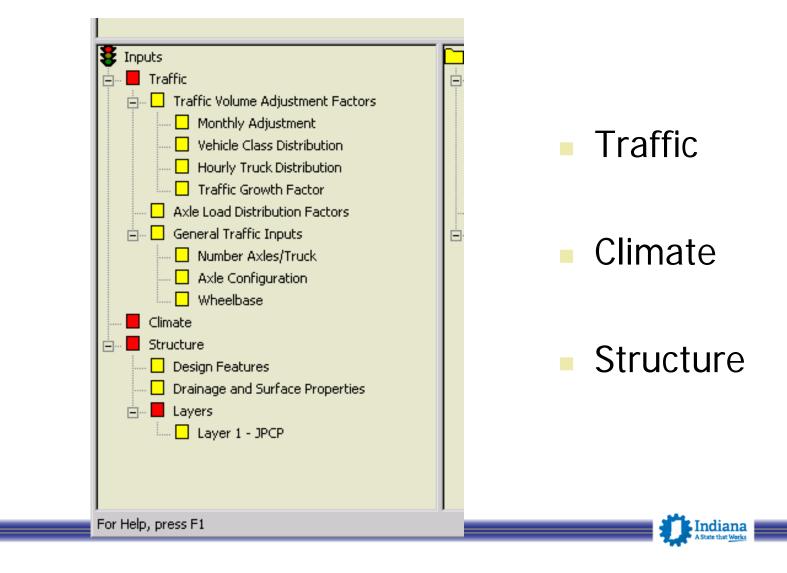






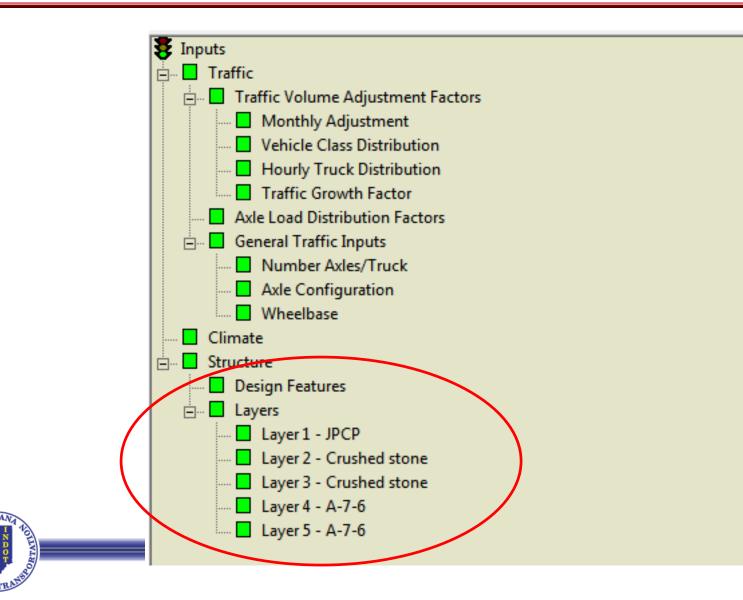


Design inputs



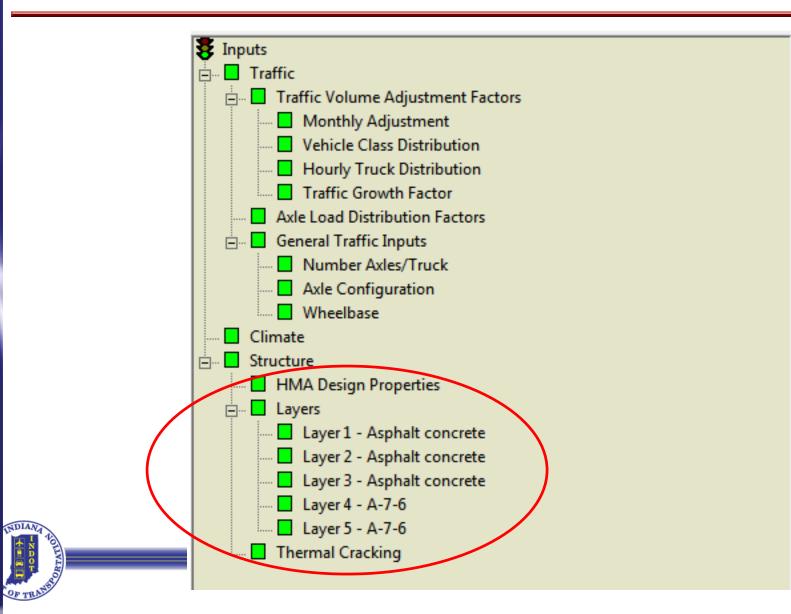


JPCP design feature, layers, and material properties



Indiana

HMA design properties, layers, and thermal cracking

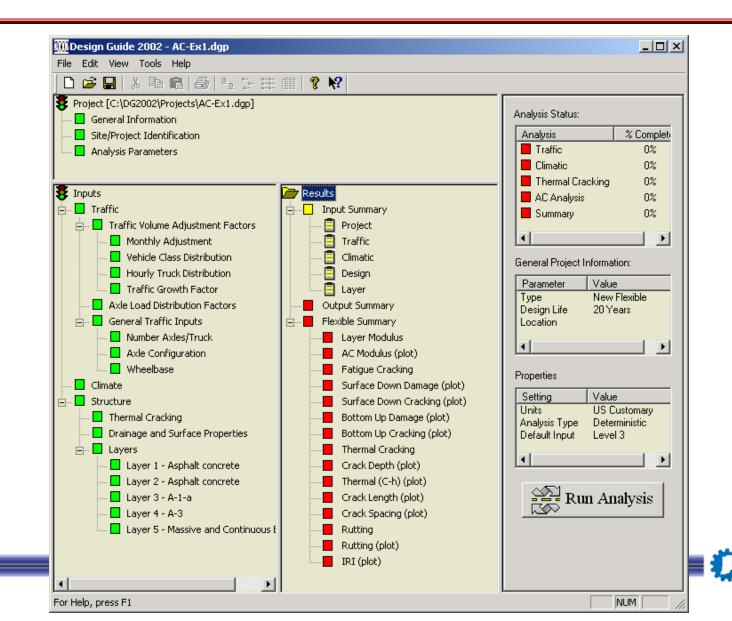


A State that Works



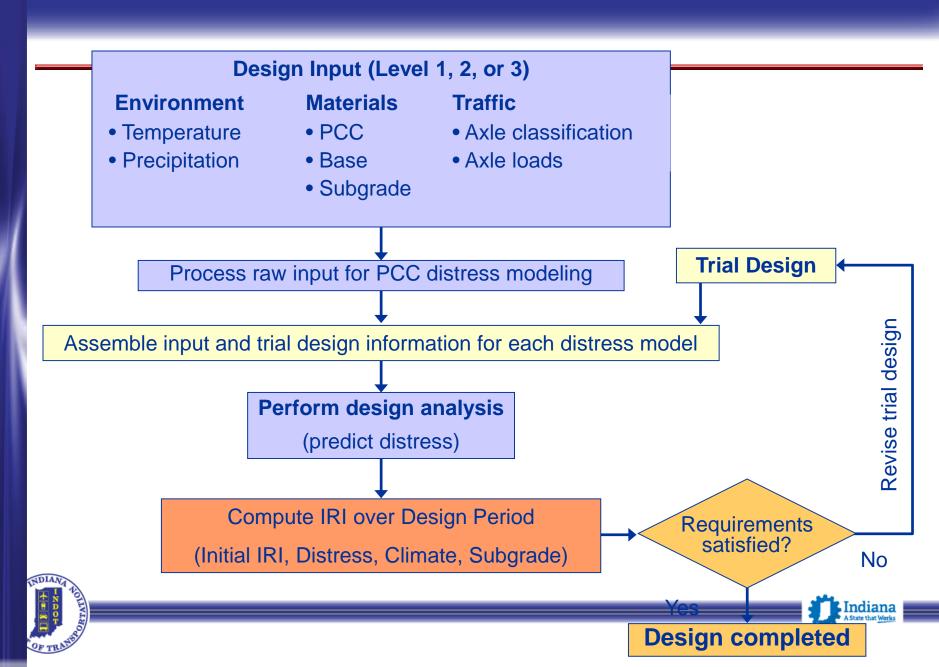
NDIANA

OF TR



Indiana





Historical pavement AM – Pre 1970s

- We've been managing pavements since there have been roads!
- AASHTO Road Test (1950s-60s)
 - Limited loading weights and cycles compared to today
 - Now 50-yrs old data
 - Truck weights and age vastly different
- BEST WE HAD AT THE TIME!





Historical pavement AM - 1970s/80s

- Subject matter expert based project selection
- Case-by-case
- Informal network analysis
- Professional memory based
- Developing objective theory
- Establishing some objective measures
 - IRI, roughness, etc.





Historical pavement AM - 1990s

- Initially interstate only ('91-'92)
 - INDOT interstate program centrally managed
 - Van trips post-data analysis, SME input
 - dTIMS AM software obtained
 - Limited models !
 - Data limitations !
 - IRI / Rut / PCR (10% sampling)





Historical pavement AM - 1990s

- Non-interstate model developed '96-'97
 - Limited models
 - Data limitations
 - IRI / Rut / PCR (10% sampling)

Computer processing improvements





Decision-Support Information needed:

- Traffic: AADTT, truck volumes
- Condition: IRI, rut, cracking type & severity, friction, structural adequacy, drainage,
- Inventory: location, geometrics
- Materials: soils, HMA mix, PCC mix
- History: maintenance, construction, jurisdictional





How is the road?

Condition adequacy

What do you need to do?

- Engineering perspective
- Business perspective





Initial engineering perspective

- No problems
- Minor flaws
- Major flaws
- REAL MAJOR PROBLEMS





Engineering problem - AM perspective

- No problems
- Lack of maintenance
- Rough ride
- Beginning of structural deterioration
- Advanced structural deterioration
- Structurally failed
- Roadside / drainage problems





Business owner perspective

- It is about money
- Is the pavement unacceptable or not?
- How much is it going to cost to address?
- How long will it not be a problem?
- Different managerial approaches depending on the previous question's answer



Pavement is unacceptable now

- Do something now!
- WORST FIRST maybe
- Priority of effort
- Not necessarily a strategic fix
- GET IT OUT OF UNACCEPTABLE category
- Maybe least bad solution?





Pavement is acceptable

- Least cost of ownership approach
 - \$/lane-mile year of service purchased
- Optimized cost-effective right-treatment at right time for right cost approach
- Or bridging strategy or approach





Possible fixes

- Do nothing
- Routine maintenance
- Reactive maintenance
- Preventative maintenance or PPI (pavement preservation initiative) treatment
- Structural treatments
- Each approach has several optional treatments
 - Options have cost, time & benefit ranges



Comprehensive list of <u>NEEDS!</u>

Process this list through business guidance

- Priority of resourcing / effort
- Effectiveness of relative improvements
- Priority of relative improvements
- Funding





Problem assessment and statement

Possible solutions

Treatment options

COA screening and evaluation

- Worst first worst, but necessary
- Engineering economics intervention point optimization
- Temporary bridging strategy or approach





COA screening and evaluation

- Delineated factors & considerations
 - Your successor might need to know
 - I call it the "dumb bunny' innoculation
- FAS-DC
- Recorded
 - Where did you use _____ logic
 - worst first worst, but necessary
 - engineering economics intervention point optimization
 - temporary bridging strategy or approach





COA screening and evaluation

Engineering economics intervention point optimization

- Echelons of treatments
 - Routine maintenance
 - Reactive maintenance
 - Preventative maintenance
 - Functional/smoothness treatments
 - Structural minor rehab treatments
 - Structural major rehab treatments
 - Structural pavement replacement

<\$1K/ln-mi/svc yr? ? / TBD \$5K/ln-mi/svc yr? \$7-20K/ln-mi/svc yr? \$10-25K/lm-mi/svc yr(?) \$25-35K/ln-mi/svc yr(?) \$1Mil/ln-mi/svc yr(+)(?)





speaker note - talk about:

- \$33 vs. \$9 Million
- Last Friday
- Repeated internal/external examples
- That which you inspect gets done well





Requirements for Treatment Selection

- What are my Options?
- Which One is Best Value?
- Prove It, and I'll Spend Taxpayer Dollars!







Decision to select treatment options

Rehabilitation Treatment Overview





Objectives

- Identify maintenance/rehabilitation treatments.
- Benefits of good timing.
- Preventive maintenance and its principles.





Introduction

- How do PCC pavements typically deteriorate?
- When is functional performance impaired?
- What about structural performance?
- What treatments are commonly used?





PCC Rehabilitation Treatments

- PCC Overlays
- HMA Overlays
- PCC Pavement Recycling
- Accelerated Rigid Paving Techniques
- Feasible Treatment Identification





Treatment Information

- Definitions
- Purpose and Applications
- Limitations and Effectiveness
- Design Considerations
- Pavement Surveys
- Cost Considerations
- Construction Considerations
- Equipment





Identification of Candidate Treatments

- Specific Distresses Present
- Condition
 - Functional
 - Structural
- Loadings and Environment
- Available Tools
 - Decision trees
 - Decision matrices





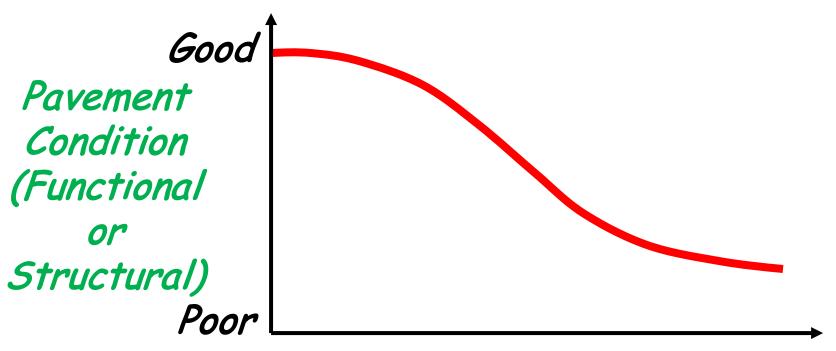
Treatment Timing Issues

- What factors affect treatment timing?
- When is too soon?
- Too late?





Typical Pavement Performance Curve

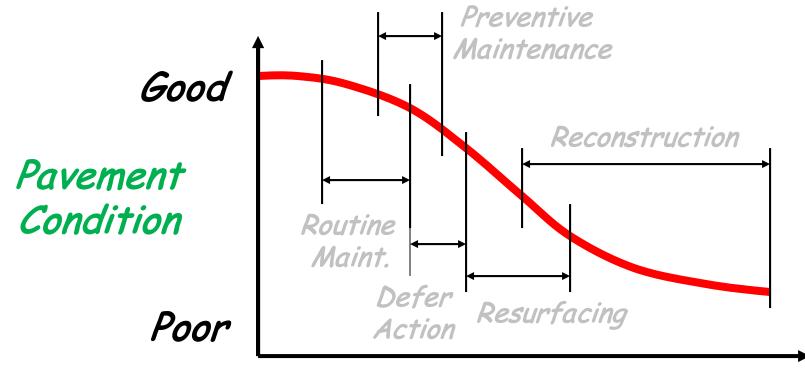


Time (Years)





Typical Pavement Performance Curve

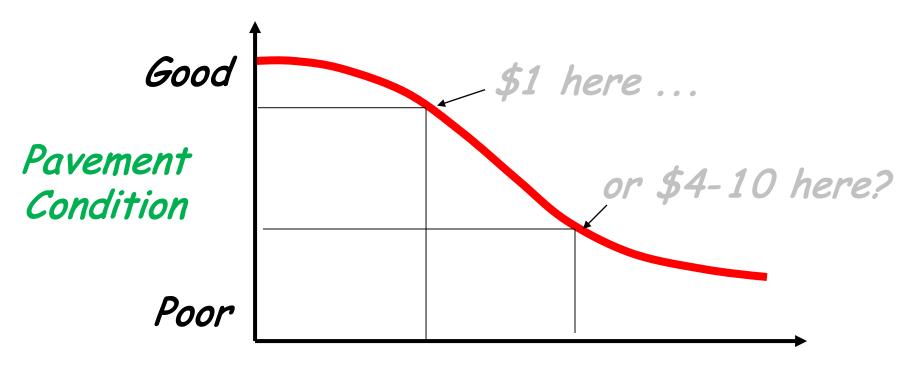


Time (Years)





Cost Effects



Time (Years)

Indiana



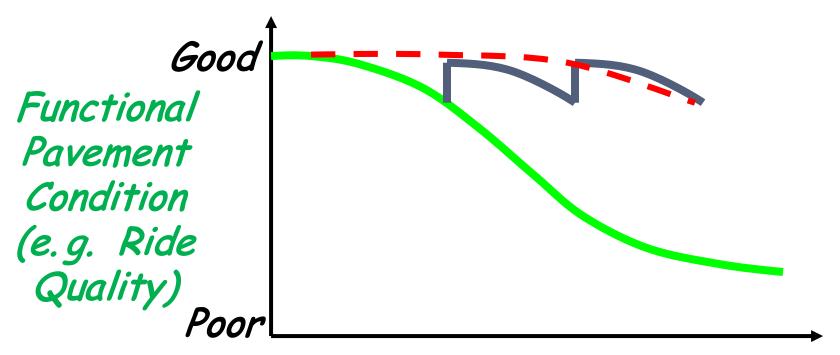
Preventive Maintenance

- Planned strategy
- Preserves the system
- Retards future deterioration
- Maintains or improves functional condition





Anticipated PM Benefits



Time (Years)





Anticipated PM Benefits

- Functional Performance?
- Structural Performance?
- Costs:
 - To the agency?
 - To the user?





Conventional Rehabilitation Treatment

HMA Pavement Overlay





Introduction

- Most popular method
- Relatively fast and cost-effective means for:
 - Correcting deficiencies
 - Restoring user satisfaction
 - Adding structural capacity
- Poor performance is NOT uncommon





Definitions

- Functional performance Ability to provide a safe, smooth riding surface
- Structural performance Ability to carry traffic without distress
- Empirical Design based on past experience or observation
- Mechanistic Design based upon engineering mechanics





Purpose and Applications

- Improve functional and/or structural characteristics
- Wide range of applications
 - Road surface categories
 - Climate and support conditions





Characteristics of Typical HMA Overlay

- Dense graded HMA
- Flexible or rigid surface
- 25 to 200 mm (1 to 8 in) thickness
- Mill and Fill





Limitations and Effectiveness

Why do we have premature failures?

- Improper selection
- Wrong type
- Inadequate design
- Insufficient preoverlay repair
- Lack of consideration of reflection cracking





Limitations and Effectiveness

- What limits the effectiveness of HMA overlays?
- Distress exhibited in HMA
- Intended design life of the overlay
- Availability of quality materials





Limitations and Effectiveness

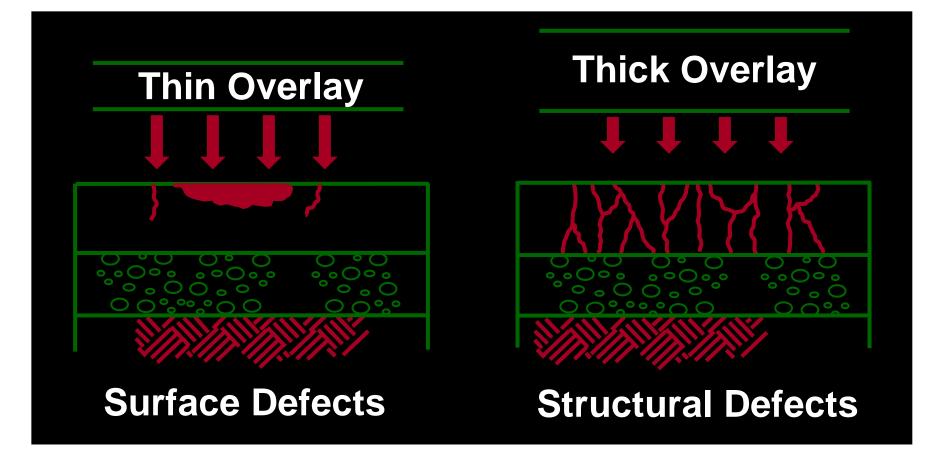
How can we improve our overlays?

- Preoverlay treatments
- Better materials and practices
- Sound engineering judgment





Overlay Selection to Correct Deficiencies







What Are Considerations in Overlay Selection?

- Construction feasibility
 - Traffic control
 - Constructibility
 - Vertical clearances
 - Utilities
- Performance periodFunding





Preoverlay Treatment and Repair

Dependent upon:

- Type of overlay
- Structural adequacy of existing pavement
- Existing types of distress
- Future traffic
- Physical constraints
- Cost





To Repair or Not to Repair?







Types of Preoverlay Treatments

- Localized repair (patching)
- Surface leveling
- Controlling reflection cracking
- Drainage improvements





Conventional Rehabilitation Treatment

Concrete Pavement Overlay





Types of Whitetopping Overlays

Conventional Whitetopping

- Slabs greater than 100 mm (4 in.) thick
- Placed directly on HMA pavement (little preoverlay repair)

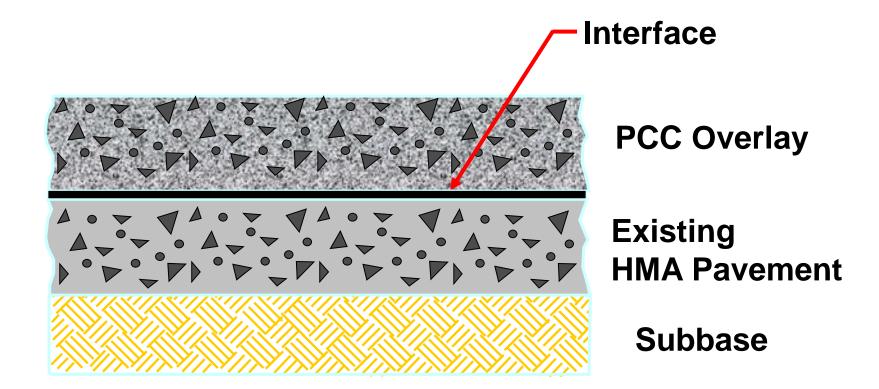
Ultra-Thin Whitetopping

- Thin slabs (50 to 100 mm thick) (2 to 4 in.)
- Short joint spacing (0.6 to 1.8 m) (2 to 6 ft)
- Bonded to existing HMA to increase loadcarrying capacity





Conventional Whitetopping







Applicability

Conventional Whitetopping

- Badly deteriorated HMA pavements
- Most any traffic volume

Ultra-Thin Whitetopping

- Low volume roads exhibiting rutting, shoving, potholing
- Urban intersections where recurrent rutting/washboarding has been a problem





Overlay Selection

- Detailed pavement evaluation (distress, FWD, coring)
- Construction feasibility
- Performance period
- Cost effectiveness





Whitetopping Feasibility—Constructibility

Conventional

Vertical Clearance

Can be a problem

Traffic Control May be difficult to construct under traffic

Construction

No special equipment





Whitetopping Feasibility—Performance Period

Conventional

Existing Condition

Extent of Repair

Future Traffic Very deteriorated HMA pavements

Limited to very severe areas

Any traffic level

Historical Reliability

Very good





Design Considerations

- Slab thickness
- Joint design
- Drainage design
- Reinforcement design
- PCC mix design
- Preoverlay repair and surface preparation





Preoverlay Repairs Whitetopping Overlays

- Localized repair of failed areas
- Filling of potholes
- Milling if rutting greater than 50 mm (2 in.)
- Repair of severe alligator cracking if poor support would otherwise result







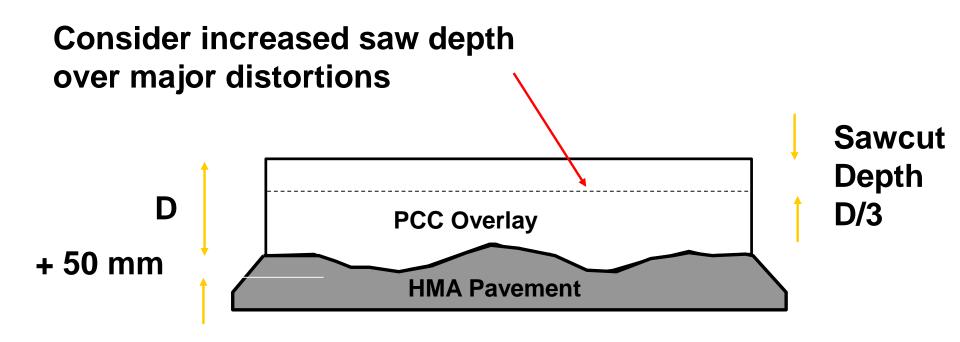
Construction — Whitetopping Overlays—

- Conventional PCC paving equipment and construction practices are used
- PCC may be placed directly on HMA or on milled or leveled HMA surface
- Whitewashing of HMA surface may be required on hot days





Whitetopping —Joint Sawing—



Indiana



SR-161 Whitetopping







SR-161 Whitetopping





Rehabilitation Option

Hot In-Place Recycling





Hot In-Place Recycling Description

Three methods

- Surface recycling
- Remixing
- Repaving
- Typical depth: 15 mm
 50 mm (0.6 2.0 in)
- RAP mixed with additives and relaid
- Immediate opening to traffic
- Applicable for all traffic levels
- Resurfacing usually required.







Pavement Condition

Before 08/2012

After 08/2012



















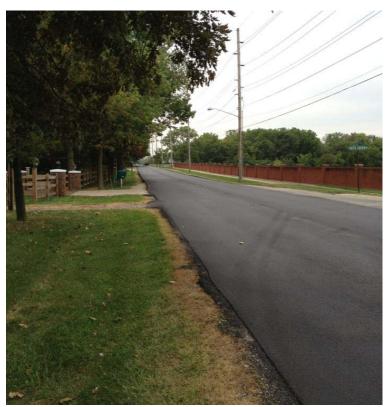


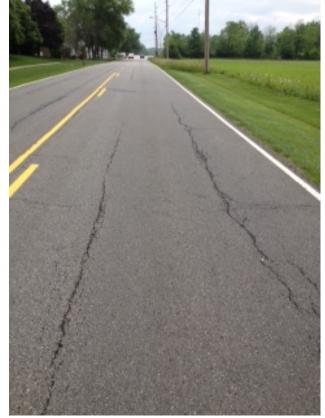


Pavement Condition

08/2012

06/2014









Rehabilitation Option

Cold In-Place Recycling





Cold In-Place Recycling Description



Asphalt Stabilized using emulsified asphalt or expanded (foamed) asphalt.

- Cold process
- Milling depth: 50 mm
 - 100 mm (2 to 4 in)
- RAP mixed with additives and relaid
- Resurfacing is typically required
- Most commonly used on secondary and lowvolume roads



Benefits

- Conserves energy and materials
- Preserves geometrics
- Many surface distresses eliminated
- Improves profile
- Modifies material characteristics
- Relatively inexpensive





Pavement Condition



US 40 condition

- Aged surface
- Minor rutting
- Heavy patching due to stripped HMA layer





Pavement Milling



- Milling operation will cut up to 4" depth and windrow material
- Can incorporate virgin aggregate during milling operation



Stabilization



- Water, additives and stabilizing materials are incorporated into the windrow material
- The windrow is remilled to mix the materials





Spreading



- The stabilized material is picked up by a windrow elevator
- The paver spreads the material
- Compaction is achieved using steel drum and pneumatic tire rollers





Overlay Preparation

- The CIR is tacked prior to the HMA overlay
- Paving commences
 US-40 had a 165 lb/sys
 9.5 mm surface atop the CIR base

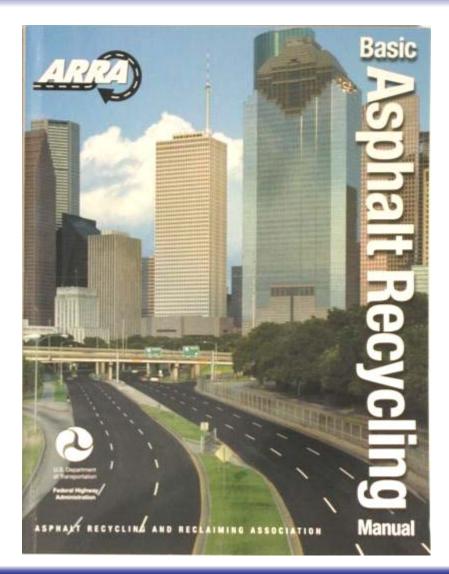








Basic Asphalt Recycling Manual







In-Place Recycling

Measure of Effectiveness

Corrects	Slows/Reduces Seve
Poor friction	Cracking
Roughness	Moisture dama
Bleeding	
Raveling	
Rutting	
Poor cross slope	
Provents/Delays	Negatively Affects

erity

ige

Negatively Affects
None



Rehabilitation Option

Full Depth Reclamation (FDR)





Definition of Full-Depth Reclamation

 Method of flexible pavement reconstruction that utilizes the existing asphalt, base, and subgrade material to produce a new stabilized base course for a chip seal, asphalt, or concrete wearing surface.





Types of Reclamation Methods

- Mechanical Stabilization
- Bituminous Stabilization
 - emulsified asphalt
 - expanded (foamed) asphalt

Chemical Stabilization

- Portland cement, slag cement, lime, fly ash, other
- Maximum pavement depth of ~14"





Challenges Facing Our Roadways

- Continuing growth
- Rising expectations from users
- A heavily used, aging system
- Environmental compatibility
- Changes in the workforce
- Funding limitations

Combined with large increases in traffic volumes and/or allowable loads often leads to serious roadway base failures!





How do you know if you have a base problem and not just a surface deficiency?







Examples of Pavement Distress

- Alligator cracking
 - Rutting
- Excessive patching
- Base failures
- Potholes
- Soil stains on surface







Advantages of the FDR Process

- Use of in-place materials
- Little or no material hauled off and dumped
- Maintains or improves existing grade
- Conserves virgin material
- Saves cost by using in-place "investment"
- Saves energy by reducing mining and hauls
- Very sustainable process







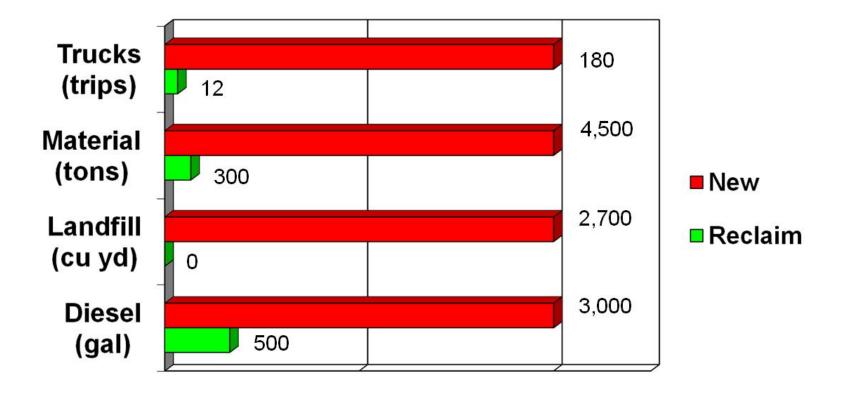
Rehabilitation Strategies

	Rehabilitation Strategy				
Attribute	FDR	Structural Overlay	Removal and Replacement		
New pavement structure	\checkmark	\checkmark	\checkmark		
Fast construction	\checkmark	\checkmark	Х		
Minimal traffic disruption	\checkmark	Х	Х		
Minimal material in/out	\checkmark	Х	Х		
Conserves resources	\checkmark	Х	Х		
Maintains existing elevation	\checkmark	Х	\checkmark		
Low cost	\checkmark	Х	Х		





Sustainable Element of FDR Process



1 mile of 24-foot wide, 2-lane road, with a 6-inch base





FDR in Indiana



Other Options for FDR







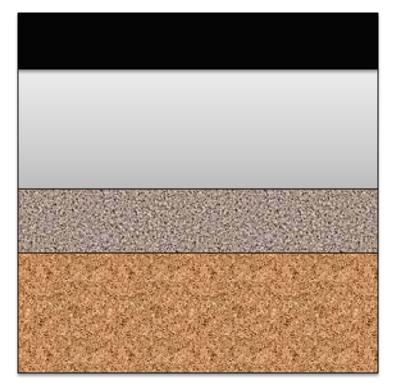
Design Issue

Pavement Rehabilitation Design





Existing pavement section



🚄 4" HMA overlay

⇐ 8.5" JPCP

a" Dense sand

🦛 Soil subgrade





Proposed rehabilitation



12 year LCCA

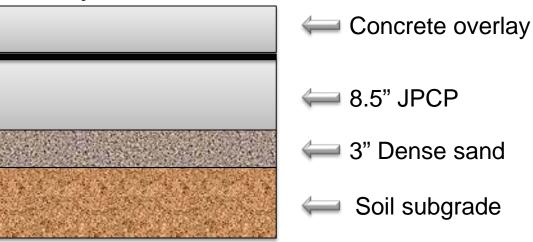
🚧 HMA overlay

듲 8.5" JPCP

듲 3" Dense sand

📁 Soil subgrade

25 year LCCA



Indiana



Design alternatives

Explorer 🛛 🕹 🗙	0400228 - US 31 (Conc:Project 0400.	228 - US 31 (HMA. O:Project			• X	Running Integrated Climatic	100
Projects	General Information	Performance Criteria		Limit	Reliability	Extending climate solution	100
	Design type: Overlay	Initial IRI (in./mile)		70	riolicionicy	Calculating modulus subgra	100
Foundation Support	Pavement type: JPCP over JPCP (unbc -	Terminal IRI (in./mile)			35	Calculating Effective Thick	100
JPCP Design Properties	Design life (years): 25 -	JPCP transverse cracking (percent slabs)			35	Preparing PCC Inputs	100
Climate	Existing construction: July - 1975 -	Mean joint faulting (in.)			35	Preparing thermal gradient file	100
Layer 1 PCC : JPCP	Pavement construction; July V 2012 V	Mean joint faulting (in.)		U.Z 8	50	Calculating Faulting	100
Layer 2 Flexible : Asphalt concrete	Traffic opening: Septen 2012					Calculating Cracking	100
Layer 3 Stabilized Base : JPCP (existing)	Tranic opening.					Calculating JPCP IRI	100
Layer 4 Subgrade : A-4 Layer 5 Subgrade : A-4							
	🜗 Add Layer 🗯 Remove Layer	Darwin ME. Rigid Pavement Unbonded Calibration	GridObiect			0400228 - US 31 (HMA.	%
Project Specific Calibration Factors	- Add Ebyer	-			· · ·	Running Integrated Climatic	100
						Extending climate solution	100
		PCC IRI J4 PCC IRI JPCP Std.Dev.	 ✓ 25.24 ✓ 5.4 		-	Calculating modulus subgra	100
	Click here to edit Layer 1 PCC : JPCP	PCC Punchout	V 3.4			Preparing PCC Inputs	100
		PCC CRCP C1	✓ 2			Preparing thermal gradient file	
Unbonded Rigid	Click here to edit Layer 3 Stabilized Base : JP(PCC CRCP C2	✓ 1.22		_	Calculating Effective Thick	100
Optimization	A ADDITION OF THE OWNER OWNER OF THE OWNER OWNE	PCC CRCP C3 PCC CRCP C4	 ✓ 216.842 ✓ 33.1579 			-	
PDF Output Report	Click here to edit Layer 4 Subgrade : A-4	PCC CRCP C5	✓ -0.58947		-	Calculating Cracking	100
0400228 - US 31 (HMA. Overlay)		PCC CRCP Crack	✓ 1			Preparing Thermal Cracking	100
	- Contraction of the second	PCC Reliability PO Standard Deviation	2+2.2593*POW(PO,0.48	82)	_	Running Thermal Cracking	100
JPCP Design Properties		 Identifiers Display name/identifier 				Asphalt Damage Calculations	100
AC Layer Properties	and the second				-	Asphalt Rutting and Fatigue	100
FoundationSupport	Click here to edit Layer 5 Subgrade : A-4	Display name/identifier Display name of object/material/project for out	toute and graphical interface			JPCP Cracking Reliability	100
	Contraction of the second s	Display hame of object/hatena/project for our	puts and graphical interface			Asphalt IRI	100
Layer 1 Flexible : Asphalt concrete							
Layer 2 Flexible : Asphalt concrete							
Layer 3 PCC : JPCP (existing)	Error List				ųχ		
Layer 4 Subgrade : A-4	Project Object Property Des	cription					
Layer 5 Subgrade : A-4							
Project Specific Calibration Factors							
New Flexible							
Rehabilitation Flexible							
New Rigid	📃 Output [🎉 Error List 🔤 Compare						





Backcalculation inputs

	Select Station	Station	Modulus Subgrade Reaction				
•	V	NB	260	 FWD Backcalculation data by layer 	2 back calculation layers		
		SB	276	Identifiers	2 Dack calculation layers		
			270	Display name/identifier	NB		
				Description of object	FWD testing		
				Author	YJ		
				Date created	8/8/2011		
				Approver	TEN		
				Date approved	8/8/2011		
				State	IN		
				District	LaPorte		
				County	St. Joseph		
				Highway	US-31		
				Direction of travel	NB and SB		
				From station (miles)	253+74		
				To station (miles)	255+43		
				User defined field 1			
				User defined field 2			
				User defined field 3			
				Revision Number	0		
				Item Locked?	False		





JPCP optimization

0400228 - US 31	(HMA. O:Project 040	00228 - L	JS 31:Optimizati	on 040022	28 - US 31	(Conc. O:Project	• ×
Last Optimized Thick	kness 9	Design	Layers				
Layer Thickness	Results	Use	Layer	Default Thi	ickness	Minimum Thickness	Maximum Thickness
	Failed		ayer 1 PCC : JPCP	9		6	12
12	Passed						
9	Passed						
7.5	Failed						
B	Failed						
8.5	Failed						
		Optimi	ization Rules				
			Use Property		Rules	Cri	iteria
		+	Dowel Diamete	er (in.) 🔻]		
		*					
	Optimize Thickness						
	opunizo monicas	,					





JPCP optimization result

Design Str	ructure					Traffic	
Layer 1 PCC ; JPC	Layer type	Material Type	Thickness (in.):	Joint Design:		Age (year)	Heavy Trucks
Layer 3 Stabilized	PCC	JPCP	9.0 (Optimized)	Joint spacing (ft)	15.0	Age (year)	(cumulative)
Layer 4 Subgrade	Flexible	Asphalt concrete	2.0	Dowel diameter (in.)	1.25	2012 (initial)	6,000
The second	Stabilized	JPCP (existing)	8.5	Slab width (ft)	12.0	2024 (12 years)	14,273,700
Løyer 5 Subgrade	Subgrade	A-4	24.0			2037 (25 years)	31,794,300
	Subgrade	A-4	Semi-infinite				

Design Outputs

Distress Prediction Summary

Distress Type		Specified ability	Reliab	Criterion Satisfied?	
	Target	Predicted	Target	Achieved	Sausheur
Terminal IRI (in./mile)	190.00	120.37	85.00	99.96	Pass
Mean joint faulting (in.)	0.20	0.07	85.00	100.00	Pass
JPCP transverse cracking (percent slabs)	12.00	9.49	85.00	92.70	Pass





HMA optimization

0400228 - US 31 (Conc. O:Project 0400	228 - US 31 (HMA. O:Proje	ct 0400228 - US	31:Optimization	• X			
Last Optimized Thickness	Design Layers						
Layer Thickness Results	Use Layer	Default Thickness	Minimum Thickness	Maximum Thickness			
	Layer 1 Flexible : As	1.5	1.5	3.0			
	Layer 2 Flexible : As	2.5	2.5	5			
Optimization Rules Optimization rules are currently available only for JPCP analyses.							
Adding a ba	se layer	is more	e appro	opriate			
Optimize Thickness							





HMA sensitivity

Use	Property	Layer	Default	Minimum	Maximum	# of Increments
	Two-way AADTT		6000			
	Thickness (in.):	Layer 1 Flexible : Asp	1.5			
	Binder Content (%)	Layer 1 Flexible : Asp	11.61			
	Air voids (%)	Layer 1 Flexible : Asp	8			
V	Thickness (in.):	Layer 2 Flexible : Asp	2.5	2.5	5	5
	Binder Content (%)	Layer 2 Flexible : Asp	10.66			
	Air voids (%)	Layer 2 Flexible : Asp	8			
	Thickness (in.):	Layer 3 PCC : JPCP (8.5			
	Thickness (in.):	Layer 4 Subgrade : A-4	24			
	Unbound Modulus	Layer 4 Subgrade : A-4	6000			
	Dowel diameter (in.)		1.25			
	PCC joint spacing (ft)		15			
	Slab width (ft)		12			
	PCC coefficient of th	Layer 3 PCC : JPCP (5.4			
	28-Day modulus of ru	Layer 3 PCC : JPCP (350			





HMA Sensitivity result

Design Structure



Layer type	Material Type	Thickness (in.):	Volumetric at Construction	n:
Flexible	Asphalt concrete	1.5	Effective binder	5
Flexible	Asphalt concrete	2.5	content (%) Air voids (%) 8.0	
PCC	JPCP (existing)	8.5		
Subgrade	A-4	24.0		
Subgrade	A-4	Semi-infinite		

Traffic

Age (year)	Heavy Trucks (cumulative)
2012 (initial)	6,000
2018 (6 years)	6,461,420
2024 (12 years)	13,661,300

Design Outputs

Distress Prediction Summary

Distress Type) Specified ability	Reliab	Criterion	
	Target	Predicted	Target	Achieved	Satisfied?
Terminal IRI (in./mile)	172.00	105.11	90.00	100.00	Pass
Permanent deformation - total pavement (in.)	0.75	0.20	90.00	100.00	Pass
Total Cracking (Reflective + Alligator) (percent)	100.00	7.33	-	-	-
AC thermal fracture (ft/mile)	250.00	217.40	90.00	95.93	Pass
JPCP transverse cracking (percent slabs)	15.00	19.72	90.00	74.75	Fail
AC bottom-up fatigue cracking (percent)	25.00	1.45	90.00	100.00	Pass
AC top-down fatigue cracking (ft/mile)	2000.00	257.71	90.00	100.00	Pass
Permanent deformation - AC only (in.)	0.25	0.20	90.00	98.85	Pass





FDR and New HMA design inputs

Explorer 4 ×	1200700_I70FullDepthHMA:Project					•)	x
⊡ 🔁 Projects ⊡, խ 0200700_170FullDepthHMA	General Information	_	Performance Criteria		Limit	Reliability	-
	Design type: New Pavement	•	Terminal IRI (in./mile)		160	90	_
Single Axle Distribution	Pavement type: Flexible Pavement	-	AC top-down fatigue cracking (ft/mile)		2000	90	
Tandem Axle Distribution	Design life (years): 25	-	AC bottom-up fatigue cracking (percent)		10	90	
	Base construction: May 💌 2012	-	AC thermal fracture (ft/mile)		50	90	Ξ
Climate	Pavement construction July - 2012	•	Permanent deformation - total pavement (in.)		0.75	90	
AC Layer Properties	Traffic opening: Septen - 2012	•	Permanent deformation - AC only (in.)		0.4	90	
Layer 1 Flexible : Asphalt concrete		_	Reflective cracking (percent)		10	50	
Layer 2 Flexible : Asphalt concrete							-
Layer 3 Flexible : Asphalt concrete Layer 4 Non-stabilized Base : Crushed stone	📲 Add Layer 🗱 Remove Layer		Layer 1 Asphalt Concrete:Asphalt concrete			-	-
Layer 5 Subgrade : A-6							
Layer 6 Subgrade : A-6			Asphalt Layer				
iainia Project Specific Calibration Factors	ETA FORDALE	R	Thickness (in.)	✓ 1.5			
	Click here to edit Layer 3 Flexible : Aspl	alt co	Mixture Volumetrics	173.5 Warning:	Value is a	transfor them a	Ξ
			Unit weight (pcf) Effective binder content (%)	 173.5 warning. ✓ 13.4 	value is g	reater than r	
🔜 Restore Rigid			Air voids (%)	✓ 7			
	Click here to edit Layer 4 Non-stabilized	Base		(calculated)			
Sensitivity	The second second		Mechanical Properties Dynamic modulus	✓ Input level:1			
Optimization		953		Use Viscosity based	d model (nationally (
Multiple Project Summary	Click here to edit Layer 5 Subgrade : A-6	m That	Reference temperature (deg F)	✓ 70			
Batch Run	and the second	1	Asphalt binder	Level 1 - Super	Pave:		-
	Click here to edit Layer 6 Subgrade : A-6		Thickness (in.)				
⊕ DARWin-ME Calibration Factors	and the second s	J.,	Thickness of the asphalt concrete layer. Minimum:1				
		10013	Maximum:20				





Decision making process

Treatment Selection





Treatment Selection Factors

- Available Funds
- Staged Construction
- Traffic Control
- Lane Closure
- Minimum Desired Life
- Future Maintenance
- Geometric Issues





Treatment Selection Factors (continued)

- Present and Future Utilities
- Right-of-Way Restrictions
- Regulatory Restrictions
- Available Materials and Equipment
- Contractor Expertise and Manpower
- Agency Policies





Selection Process

- Develop feasible alternatives for evaluation
- Identify key decision factors important to agency (e.g., cost, service life, traffic control, duration of construction, etc.)
- Assign weighting values for each decision factor
- Assign scoring values for each alternative
- Add scores and rank alternatives





Selection Worksheet

	Decision Factor 1	Decision Factor 2	Decision Factor 3	Decision Factor 4	TOTAL
Weight	Weight 1	Weight 2	Weight 3	Weight 4	SCORE
Alt 1					
Alt 2					
Alt 3					
Alt 4					











