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Highlights for “Comparing and combining physically-based and empirically-based approaches for estimating the hydrology of ungauged catchments” by Booker and Woods

- Methods for estimating various hydrological indices at ungauged sites were compared.
- Methods included a TopNet rainfall-runoff model and a Random Forest empirical model.
- TopNet estimates were improved through correction using Random Forest estimates.
- Random Forests provided the best estimates of all indices except mean flow.
- Mean flow was best estimated using an already published empirical method.

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5 estimating the hydrology of ungauged catchments.

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27 Short title: hydrological estimates for ungauged catchments

28

29           **Abstract**

30           Predictions of hydrological regimes at ungauged sites are required for various  
31 purposes such as setting environmental flows, assessing availability of water resources or  
32 predicting the probability of floods or droughts. Four contrasting methods for estimating  
33 mean flow, proportion of flow in February, 7-day mean annual low flow, mean annual high  
34 flow, the all-time flow duration curve and the February flow duration curve at ungauged sites  
35 across New Zealand were compared. The four methods comprised: 1) an uncalibrated  
36 national-coverage physically-based rainfall-runoff model (TopNet); 2) data-driven empirical  
37 approaches informed by hydrological theory (Hydrology of Ungauged Catchments); 3) a  
38 purely empirically-based machine learning regression model (Random Forests); and 4)  
39 correction of the TopNet estimates using flow duration curves estimated using Random  
40 Forests. Model performance was assessed through comparison with observed data from 485  
41 gauging stations located across New Zealand. Three model performance metrics were  
42 calculated: Nash-Sutcliffe Efficiency, a normalised error index statistic (the ratio of the root  
43 mean square error to the standard deviation of observed data) and the percentage bias. Results  
44 showed that considerable gains in TopNet model performance could be made when TopNet  
45 time-series were corrected using flow duration curves estimated from Random Forests. This  
46 improvement in TopNet performance occurred regardless of two different parameterisations  
47 of the TopNet model. The Random Forests method provided the best estimates of the flow  
48 duration curves and all hydrological indices except mean flow. Mean flow was best estimated  
49 using the already published Hydrology of Ungauged Catchments method.

50

51   Key words: hydrological indices; flow duration curves; ungauged sites; rainfall-runoff model;  
52   random forests.

53

## 54 **1. Introduction**

55 River water provides a valuable resource for out-of-stream water use as well as for  
56 supporting in-stream environmental values. Alteration of natural river flow regimes is  
57 increasing globally as water is taken for human, agricultural and industrial use and power  
58 production, threatening both river biodiversity and security of human water use (Vörösmarty  
59 et al., 2010). Globally, this has led to a variety of legislative processes aimed at promoting  
60 prudent and rational use of natural water resources which seek to judge the trade-off between  
61 economic development and impact to the natural environment (e.g. EC, 2000; New Zealand  
62 Government, 2011). For example, default limits to water resource use for all rivers in New  
63 Zealand must comprise at least a minimum flow (the flow below which no water can be  
64 abstracted) and an allocation limit (a limit on the amount of abstraction taken from the  
65 resource) (New Zealand Government, 2011; Snelder et al., 2013).

66 Information summarising natural flow regimes is therefore required to assess both the  
67 in-stream environmental and out-of-stream economic effects of potential alterations to flow  
68 regimes. This information may take the form of various hydrological indices describing  
69 different aspects of the flow regime such as low flows, high flows or flow variability (Olden  
70 and Poff, 2003; Poff et al., 2010). Flow duration curves (FDCs) may also be utilised for  
71 various purposes including low flow analysis (Smakhtin, 2001), quantifying reliability of  
72 water supply (Snelder et al., 2011) and quantifying alterations to hydrological regimes (Vogel  
73 et al., 2007). This type of hydrological information is ideally derived from observed flow  
74 time-series at the site, or sites, of interest. However, flow time-series are only available at a  
75 small number of locations where flow gauges have been maintained and operated.  
76 Hydrological estimates are therefore often required at ungauged sites across a catchment or  
77 landscape (Sivapalan et al., 2003).

78           A variety of approaches can be used to provide estimates of hydrological indices at  
79 ungauged sites. In theory, these approaches range from purely physically-based to purely  
80 empirically-based. Physically-based approaches have also been referred to as deterministic  
81 (Chow et al., 1988), distributed (Beven and Binley, 1992), physics-based (Pechlivanidis et  
82 al., 2011), process-based or Newtonian (Yaeger et al., 2012). Empirically-based approaches  
83 have also been referred to as stochastic (Chow et al., 1988), metric (Pechlivanidis et al.,  
84 2011) data-based or Darwinian (Yaeger et al., 2012). Physically-based approaches are those  
85 that aim to estimate streamflow by utilising a conceptual understanding of the physics  
86 describing various parts of the hydrological cycle by approximating physical processes such  
87 as interception, evaporation, and storage (e.g. Beven and Kirkby, 1979; Clark et al., 2008).  
88 However, assumptions about physical processes are necessarily required to apply this  
89 understanding (Beven, 1997). For example, assumptions about continuity of volumes,  
90 discretisation of governing equations and some form of spatial averaging may be required for  
91 a physically-based approach to be spatially-distributed (Beven, 1989; Bloschl and Sivapalan,  
92 1995; Singh and Frevert, 2006). Similarly, time dependence must be represented by updating  
93 state variables through a sequence of time steps (Singh, 1995). Physically-based approaches  
94 may also require spatially distributed input data such as information on soil characteristics  
95 such as water holding capacity, rainfall time-series or temperature time-series (e.g. Clark et  
96 al., 2008). This has led to much analysis and debate relating to data needs, parameter  
97 calibration and uncertainty in physically-based hydrological models (Beven, 1997; Beven,  
98 1989; Singh and Woolhiser, 2002; Gupta et al., 2006).

99           Empirically-based approaches are those that seek to estimate hydrological indices by  
100 quantifying patterns between observed hydrological indices and catchment characteristics.  
101 These patterns can be quantified using a variety of techniques including linear regression  
102 (e.g. Engeland and Hisdal, 2009), or machine learning techniques (e.g. Booker and Snelder,



103 2012). One advantage of empirically-based approaches is that their relative simplicity has  
104 allowed them to be transferred to ungauged catchments by way of regionalisation (e.g.  
105 Castellarin et al., 2004), generalisation or dissimilarity modelling (e.g. Booker and Snelder,  
106 2012).

107 In practice, many physically-based models have empirical components and many  
108 empirical models incorporate some level of knowledge about physical processes. A balance  
109 between model complexity and data availability must be found for both physically-based  
110 (Fenicia et al., 2008) and empirically-based (Jakeman and Hornberger, 1993) approaches. All  
111 physically-based approaches require some parameterisation, and are known to perform best  
112 when calibrated against observed data (e.g. Clark et al., 2008; McMillan et al., 2013).  
113 Similarly, the independent variables used in empirically-based approaches are often chosen  
114 after consideration of physical principles and the form of fitted empirical relationships can  
115 also be interrogated to ensure consistency with physical principles (e.g. Booker and Snelder,  
116 2012). Hybrid metric-conceptual models are those that seek to combine the strengths of  
117 empirically-based and physically-based conceptual models (Pechlivanidis et al., 2011).

118 Despite the variety of approaches available for estimating hydrological conditions at  
119 ungauged sites, few studies have compared estimates calculated using contrasting  
120 approaches. The aim of this work was to compare a variety of available methods for  
121 estimating several hydrological indices and flow duration curves at ungauged catchments  
122 across New Zealand. These methods employed a range of approaches from a physically-  
123 based rainfall-runoff model to empirically-based regressions. The primary aim was to  
124 objectively judge which method was best able to estimate several hydrological indices across  
125 New Zealand given current climatic and landcover conditions. The secondary aim was to  
126 assess the advantages of combining two approaches by correcting physically-based estimated  
127 time-series using empirically-based estimated FDCs.

128 **2. Data Description**

129 *a. Flow time-series*

130 A flow time-series database was collated that comprised mean daily flows observed at  
131 485 gauging stations with available records of 5 full years or longer. Available mean daily  
132 flow time-series from the National Institute of Water and Atmospheric Research's (NIWA)  
133 national database were collated alongside data supplied by particular regional councils  
134 (Northland Regional Council, Auckland Council, Waikato Regional Council, Greater  
135 Wellington Regional Council, and Environment Canterbury). The time-series database  
136 contained only sites that were not affected by large engineering projects such as dams,  
137 diversions or substantial abstractions, according to information given by each data provider.  
138 See Snelder et al. (2005) and Booker (in press) for further details on gauging station  
139 selection. These gauging stations were located throughout New Zealand (Figure 1) and  
140 represented a wide range of hydrological conditions (Table 1). The observed time-series did  
141 not all cover the same time periods.

142 It is known that hydrological regimes may not be stationary (constant mean and  
143 constant variance through time; Hamilton, 1994) due to the presence of trends and temporal  
144 autocorrelations (Milly et al., 2008). This is because hydrological regimes may be influenced  
145 by a variety of factors including land cover change (e.g. Fahey & Jackson, 1997), inter-  
146 decadal climatic patterns (e.g. Kiem et al., 2003) and longer-term climate shifts (Parry et al.,  
147 2007). However, the purpose of this study was to compare the ability of various approaches  
148 to characterise differences in flow regimes between sites across New Zealand given current  
149 climatic and land cover conditions rather than to characterise differences through time. For  
150 empirically-based methods it was therefore assumed that differences in hydrological regimes  
151 between sites far exceeded any differences in hydrological regimes that may have occurred  
152 due to differences in observation periods (which were different for each observed time-series)

153 despite some evidence for inter-decadal patterns in some, but not all, indices for particular  
154 regions of New Zealand but not others (e.g. McKerchar and Henderson, 2003; Booker, in  
155 press).

156 *b. Observed hydrological indices*

157 Several hydrological indices were calculated for each observed flow time-series  
158 (Table 2). These indices were chosen because they represent a range of hydrological  
159 conditions including floods and droughts, can be used to estimate water resource availability,  
160 and are used in environmental flow setting procedures. Mean flow,  $Q_{\text{bar}}$ , represents total  
161 potential water availability, is used for scaling of dimensionless metrics such as standardised  
162 flow duration curves (e.g. Booker and Snelder, 2012) and may be used when comparing sites  
163 for ecological studies (e.g. Leathwick et al., 2005). The proportion of flow in each month  
164 may be of interest when investigating seasonality of flow. The proportion of flow in  
165 February,  $Q_{\text{Feb}}$ , was chosen as an example because the mid-summer month of February  
166 represents a generally dry month in which both irrigation demand (the largest consumptive  
167 water use in New Zealand) and ecological stress are likely to be high. The 7-day mean annual  
168 low flow,  $Q_{\text{MALF}}$ , is often used as an indicator of low flow in ecological studies (e.g. Caruso,  
169 2002; Suren and Jowett, 2006) and to represent one component of the flow regime in  
170 environmental flow assessments (e.g. Richter et al., 1997; Poff et al., 1997). Since limits to  
171 water resource use may be expressed as proportions of  $Q_{\text{MALF}}$ , this index is of particular  
172 interest in New Zealand (MFE, 2008). Mean annual flood,  $Q_{\text{F}}$ , may be used for flood risk  
173 assessment and flood design, but may also be used as a surrogate for physical disturbance  
174 (e.g. Poff and Ward, 1989; Poff, 1996) especially when compared to geomorphological  
175 characteristics such as sediment grain size and channel slope (Clausen and Plew, 2004). All  
176 four of these hydrological indices may also be used for data driven environmental  
177 classifications (e.g. Snelder and Booker, 2012). Many further hydrological indices could have

178 been compared, but it was desirable to provide an expedient analysis and there is known to be  
179 a high degree of covariance within sets of these indices (Clausen and Biggs, 1997; Olden and  
180 Poff, 2003).

181 In order to minimise the likelihood of low flow periods crossing years, each day in  
182 each observed time-series was assigned to a water year starting on the 1st of October. Water  
183 years with more than 30 days of missing data were excluded from the analysis. Calculations  
184 of ( $Q_{MALF}$ ), and mean annual flood ( $Q_F$ ) were based on water years.  $Q_{MALF}$  was calculated as  
185 being the mean of the 7-day running average annual low flow in each water year.

186 Many hydrological indices are scale-dependent; bigger catchments have larger values  
187 of  $Q_5$ ,  $Q_{MALF}$ ,  $Q_F$  and  $Q_{bar}$  than smaller catchments. The values for these indices were  
188 therefore standardised by dividing by catchment area. Further transformations were then  
189 applied in order to more closely approximate normal distributions (Table 2).

#### 190 *c. Flow duration curves*

191 FDCs represent the relationship between magnitude and frequency of flow by  
192 defining the proportion of time for which any discharge is equalled or exceeded (Vogel and  
193 Fennessey, 1994; Vogel and Fennessey, 1995). Flow duration curves are a useful tool for  
194 quantifying flow regimes for both resource availability (Snelder et al., 2011) and for  
195 departure from a reference state (Vogel et al., 2007). For each flow time-series two observed  
196 FDCs were calculated from mean daily flows. FDCs were calculated from: a) mean daily  
197 flows in all months of the year; and b) mean daily flows in February. These two FDCs  
198 represent the probability distribution of flow over all-time and the probability distribution of  
199 flow for the month of February over all years. As above, February was chosen to represent a  
200 dry month in which both irrigation demand and ecological stress are likely to be high.

201 For calculation of each FDC, mean daily flows for each gauging station were sorted  
202 lowest to highest and then interpolated onto percentile values from 0 to 100 in intervals of 1

203 to determine the proportion of the time that each flow was not exceeded. Each FDC was  
204 therefore characterised using the same number of data points (101), providing for a balanced  
205 study design in further statistical analysis. All daily flows were divided by catchment area to  
206 allow modelling of differences in mean flow whilst standardising for differences in catchment  
207 size. This was in contrast to the method of Booker and Snelder (2012) which investigated  
208 only the shapes of FDCs after having standardised by  $Q_{\text{bar}}$ .

209 *d. Catchment characteristics*

210 A GIS representation of the New Zealand river network comprising 550,000 segments, their  
211 unique upstream catchments and an associated database of catchment characteristics were  
212 used to provide information for each gauging station. The catchment characteristics include a  
213 range of categorical and continuous variables (Snelder and Biggs, 2002; Snelder et al., 2004;  
214 Leathwick et al., 2011). The GIS river network and associated databases have previously  
215 been used to define a hierarchical classification of New Zealand's rivers called the River  
216 Environment Classification (REC; Snelder and Biggs 2002). These databases provide  
217 inventories for river resource analysis and management purposes (Snelder and Hughey, 2005;  
218 Leathwick et al., 2011; Clapcott et al., 2010; Clapcott et al., 2011). They have also been used  
219 to create nationwide models for estimating flow statistics such as flood flows (Pearson and  
220 McKerchar, 1989), low flows (Pearson, 1995), mean flow (Woods et al., 2006) and shapes of  
221 FDCs (Booker and Snelder, 2012) at ungauged sites using relationships between these  
222 hydrological metrics and catchment characteristics. Snelder et al. (2005) showed that  
223 grouping river segments by nested categorical subdivisions of climate and topography,  
224 known as the Source-of-Flow grouping factor (Table 3), provided an a priori hydrological  
225 regionalisation.

### 226 3. Estimation methods

227 For this study four methods for calculating hydrological indices and FDCs at  
228 ungauged locations were compared (Figure 2). Method 1 used a physically-based approach.  
229 Method 2 used a data-driven empirical approach that was informed by hydrological theory to  
230 estimate each hydrological index separately. Method 2 can be classified as being a hybrid  
231 metric-conceptual approach under the classification proposed by (Pechlivanidis et al., 2011).  
232 Method 2 was named after a sequence of projects collectively known as the Hydrology of  
233 Ungauged Catchments (HUC) projects. Method 3 used an empirically-based regression  
234 approach. Method 4 combined a physically-based and empirically-based approach. All  
235 methods were able to produce estimates for all reaches that comprise the NZ river network  
236 and were therefore applicable to ungauged sites across New Zealand.

#### 237 a. Method 1 TopNet

238 Topnet is a spatially distributed time-stepping hydrological model which combines  
239 TOPMODEL concepts of sub-surface storage controlling the dynamics of the saturated  
240 contributing area and baseflow recession (Beven and Kirkby, 1979; Beven et al., 1995) with  
241 submodels for snow and plant canopies, and a kinematic wave channel routing algorithm  
242 (Goring, 1994). See McMillan et al. (2013) for further detailed description and Clark et al.  
243 (2008) for complete model equations.

244 TopNet has two fundamental components: (i) simulating the water balance over sub-  
245 catchments throughout a river basin, and (ii) routing streamflow from each sub-catchment to  
246 the basin outlet. The water balance model includes simulating the storages and fluxes of  
247 water in the canopy, snowpack, unsaturated and saturated soil zone. TopNet also accounts for  
248 time delay due to flow routing within each sub-basin. Runoff from each sub-basin flows into  
249 a digital stream network and is routed through the river network. For this application TopNet  
250 models used daily precipitation and temperature data from the New Zealand Virtual Climate

251 Station Network (Tait, 2008, Tait et al., 2006), which was then disaggregated to hourly  
252 resolution using stochastic disaggregation for precipitation (Rupp et al., 2009). Additional  
253 model boundary conditions were estimated directly from GIS data on topography, soil and  
254 vegetation (Clark et al., 2008; McMillan et al., 2013).

255 For catchment specific applications TopNet parameters can be calibrated to optimise  
256 model performance (e.g. Bandaragoda et al., 2004; McMillan et al. 2013). However, in this  
257 case uncalibrated national TopNet models of New Zealand (Henderson et al., 2011) were run  
258 using an hourly timestep over the period 1973-2010. Two different versions of TopNet were  
259 available. National TopNet Version 0 was discretised using Strahler-1 sub-catchments from  
260 the REC. The typical catchment area of a Strahler-1 catchment is  $0.7 \text{ km}^2$ . This version had a  
261 spatially uniform value for the parameter,  $f$ , which represents the decline in saturated  
262 hydraulic conductivity of the soil with depth (Clark et al., 2008). This parameter effectively  
263 controls responsiveness of river flow to rainfall. National TopNet Version 1 was discretised  
264 using Strahler-3 sub-catchments from the REC. This version had a spatially distributed set of  
265 values for  $f$ . The  $f$  parameter took different values according to the hydrological  
266 regionalisation described by Toebe and Palmer (1969), ranging from values more than  $8 \text{ m}^{-1}$   
267 for steep catchments in the Southern Alps to less than  $1 \text{ m}^{-1}$  in flat catchments on the volcanic  
268 plateau in the central North Island (see Figure 1 for place names). Where flow time-series  
269 were required for Strahler-1 and Strahler-2 catchments flow data were downscaled by  
270 multiplying flows from the nearest available Strahler-3 node in the REC network by the ratio  
271 of the catchment area of the required location with that of the substitute location. For both  
272 Version 0 and Version 1 hourly data for the river reach in which each gauging station was  
273 located were averaged over each calendar day to obtained mean daily flow time-series.  
274 Hydrological indices were then calculated using the same algorithms as were applied to the  
275 observed flow time-series.

276 Ideally both observed and estimated time-series would be available for a very long  
277 period (e.g. 100 years). However, the available observed flow time-series did not all cover the  
278 same period, and TopNet data were available for a uniform time period (1973-2010). This  
279 provided the opportunity to test the sensitivity of correspondence between observed and  
280 estimated hydrological indices to synchronisation of the observed and TopNet estimated  
281 time-series. Observed and TopNet Version 1 estimated indices were compared using two  
282 different procedures. For the first procedure, indices calculated from all available observed  
283 flows (5 years or more) were compared with those calculated from all available TopNet  
284 Version 1 estimated flows (1973-2010). Essentially this procedure assumed that, when  
285 averaged over time, both the observed and TopNet estimated time-series represented the long  
286 term hydrological conditions (i.e. that both observed and TopNet estimated time-series were  
287 stationary and that records were sufficiently long to characterise long term conditions). For  
288 the second procedure only the time period for which both observed flows and TopNet  
289 estimated flows were available was identified for each gauging station. Observed indices for  
290 this period were then compared with TopNet Version 1 estimated indices for the same period  
291 at each gauged location. Better fit between synchronised observed and estimated values (the  
292 second procedure) in comparison to non-synchronised (the first procedure) would indicate  
293 non-stationarities in the observed hydrological regimes that were detectable in the TopNet  
294 time-series. Some observed time-series fell completely outside of the TopNet time-series.  
295 This reduced the number of time-series available for the second procedure compared to the  
296 first.

297 *b. Method 2 HUC*

298 The approach used to estimate  $Q_{\text{bar}}$  for Method 2 (HUC) is described in Woods et al.  
299 (2006). Woods et al. (2006) evaluated four simple models of mean annual runoff throughout  
300 New Zealand, predominantly based on precipitation information and estimated



301 evapotranspiration. Model results were compared to observed data and synthesised estimates  
302 of catchment runoff. The preferred model of Woods et al. (2006) subtracts an estimate of  
303 annual actual evapotranspiration from a precipitation surface. Annual actual  
304 evapotranspiration is estimated according to the ratios of potential evapotranspiration with  
305 annual precipitation, and a single water balance parameter which is estimated by independent  
306 calibration. This method applies a regional bias correction to the results of a previously  
307 uncorrected model.

308         The approach used to estimate  $Q_{Feb}$  for Method 2 was to employ a regionalisation of  
309  $Q_{Feb}$  based on Source-of-Flow groupings in the REC and New Zealand island (i.e. North  
310 Island or South Island, Figure 1), where Source-of-Flow is a combination of the climate and  
311 topography classes of a catchment (Table 3). For each region  $Q_{Feb}$  was the mean of the  $Q_{Feb}$   
312 for all observed flow records that belong to that class in that island. For cases where no  
313 measured flow was available, expert judgement was applied to make use of data from other  
314 classes.

315         The approach used to estimate  $Q_{MALF}$  for Method 2 is described in Henderson et al.  
316 (2004). Figure 3 shows a schematic description of the model and its parameters. These fall  
317 into three categories: a) climate parameters ( $T$  the average length of a dry season,  $N$  the  
318 number of rain events in that season,  $P$  the amount of rain in the dry season); b) flow  
319 parameters ( $Q_{mean}$  the mean flow,  $Q_0$  the average flow at the start of the dry season,  $\alpha$  the  
320 fraction of that rain that affects the streamflow); and c) catchment parameters that describe  
321 the way in which water is released from catchments during the dry season ( $b$  and  $T^*$ ).  
322 Estimates of all these input parameters have previously been developed for all of New  
323 Zealand (Henderson et al, 2004). The parameter  $Q_0$  corresponds to the average flow at the  
324 start of the dry season. The predictions are most sensitive to the value of the  $b$  parameter,  
325 which describes the type of river flow recession. For example, catchments in dry catchments

326 typically have b values near 1, hill country catchments typically have b values near 2, and  
327 catchments with volcanic geology typically have b values of 3 or larger.

328 The approach used to estimate  $Q_F$  for Method 2 is described in Pearson and  
329 McKerchar (1989) and McKerchar and Pearson (1989). Essentially, these estimates are  
330 gained from interpolation onto ungauged sites from a contour map of  $Q_F$  which was itself  
331 derived from a spatial interpolation of observed data. Since this approach used instantaneous  
332 flow data to calculate  $Q_F$ , rather than mean daily values, it was anticipated that the approach  
333 would overestimate  $Q_F$  in comparison to observed values derived from mean daily values.  
334 However, the estimates were still included in the analysis.

335 The approach used to estimate FDCs for Method 2 was to assume a log-normal  
336 probability distribution as a model of the flow duration curves. This is a log transformation of

$$337 \quad g(x, \vartheta) = \left(1/\sqrt{2\pi}\vartheta_2\right)\exp\left[-1/2\left((x-\vartheta_1)/\vartheta_2\right)^2\right], \quad \text{Equation 1}$$

338 which has two parameters,  $\theta_1$  and  $\theta_2$ . It was further assumed that  $\theta_1$  could be estimated as the  
339 mean flow ( $Q_{\text{bar}}$  from Method 2) and that  $\theta_2$  would be estimated as a linear function of the b  
340 parameter, which was also used to calculate  $Q_{\text{MALF}}$  for Method 2. The approach used to  
341 estimate  $\text{FDC}_{\text{Feb}}$  was to scale the estimated FDC for Method 2 by the estimated  $Q_{\text{Feb}}$  for  
342 Method 2.

### 343 *c. Method 3 Random Forests*

344 A regression technique called Random Forests was used to apply a regression of each  
345 observed hydrological index (Table 2) and each of the three parameters describing a GEV  
346 distribution of the all-time FDC and the FDC for February as a function of available  
347 catchment characteristics (Table 4). This method uses machine-learning by combining many  
348 regression trees into an ensemble to produce more accurate regressions by drawing several  
349 bootstrap samples from the original training data and fitting a tree to each sample (Breiman,  
350 2001; Cutler et al., 2007). Random forest models fitted using catchment characteristics have

351 previously been shown to be able to explain variation in hydrological patterns such as  
352 parameters describing FDCs (Booker and Snelder, 2012), the frequency of events that exceed  
353 three time the median flow (Booker, in press) and various other hydrological indices (Snelder  
354 and Booker, 2012). Each random forest was developed by growing 500 trees. As the number  
355 of trees (k) increases the generalisation error always converges and it was assumed that use of  
356 500 trees was sufficiently high to ensure convergence.

357 The predictions from random forest models were tested using a leave-one-out cross  
358 validation procedure referred to here as jack-knifing (Efron, 1982; Booker and Snelder,  
359 2012). This cross-validation procedure was applied by leaving out all data associated with  
360 each of the 485 sites and then estimating each hydrological index for the left-out site from all  
361 remaining sites. The results from this procedure produced estimates as if each site were  
362 ungauged (Ganora *et al.*, 2009). Comparison between observed and jack-knifed values  
363 allowed an assessment of both the robustness and reliability for estimation at ungauged sites  
364 (Castellarin *et al.*, 2004).

365 For each time-series, the parameters describing a GEV distribution,

$$366 \quad G(x, \vartheta) = \exp\left[-\left(1 - (\vartheta_3(x - \vartheta_1))/\vartheta_2\right)^{\vartheta_3}\right], \quad \text{Equation 2}$$

367 were fitted to all observed mean daily flows and all observed mean daily flows in February.  
368 In both cases observed mean daily flows were divided by catchment area for each gauging  
369 station prior to fitting the GEV parameters. The GEV distribution is described by three  
370 parameters and has shown to represent the range of shapes of standardised FDCs found  
371 across New Zealand. See Booker and Snelder (2012) for further discussion of estimating  
372 standardised FDCs at ungauged sites across New Zealand using various statistical techniques  
373 to generalise parameters describing various probability distributions.

374 *d. Method 4 TopNet Corrected*

375 FDCs calculated using the jack-knifed Random Forests method represent a unique  
376 FDC at any location in the New Zealand river network as if each location were ungauged.  
377 This provided the opportunity to correct for bias in the TopNet estimated FDCs using the  
378 Random Forests estimated FDC at each site as if it were an observed FDC. Therefore the  
379 jack-knifed Random Forests FDCs were used to calculate a correction factor for each  
380 percentile,  $i$ , of the TopNet FDC for each site,  $j$ .

381 
$$\text{TopNet Corrected}_{ij} = \text{TopNet}_{ij} * (\text{Random Forest}_{ij} / \text{TopNet}_{ij}) \quad \text{Equation 3}$$

382 Since the exceedance percentile of each datum in each TopNet time-series was known, these  
383 corrections could also be applied to each TopNet time-series. This allowed re-calculation of  
384 each hydrological index from each corrected time-series. This procedure was repeated  
385 separately for TopNet Version 0 FDCs and TopNet Version 1 FDCs.

386 **4. Observed versus predicted values**

387 Scatterplots of observed versus predicted values after having standardised and  
388 transformed each index (Table 2) were plotted for each index for each method. These  
389 scatterplots were overlaid with a linear regression with observed values on the y-axis as  
390 recommended by Piñeiro et al. (2008). Following the suggestion of Moriasi et al. (2007),  
391 three model performance metrics were calculated for each set of observed versus predicted  
392 values: Nash-Sutcliffe efficiency (NSE); percent bias (pbias); and ratio of the root mean  
393 square error to the standard deviation of observed data (RSR). NSE is a dimensionless metric  
394 that determines the relative magnitude of the residual variance (“noise”) compared to the  
395 observed data variance (“information”) (Nash and Sutcliffe, 1970). NSE values of 1 indicate  
396 a perfect match between estimates and observations, whereas values of 0 indicate  
397 performance equal to estimating the mean observed value across all observations. pbias  
398 measures the average tendency of the simulated data to be larger or smaller than their

399 observed counterparts (Gupta et al., 1999). Negative pbias values represent overestimation  
400 and positive values indicate underestimation. RSR standardises RMSE using the observations  
401 standard deviation, and it combines both an error index and the additional information  
402 recommended by Legates and McCabe (1999). Lower RSR values indicate better model  
403 performance, with 0 indicating perfect correspondence between estimates and observations.  
404 See Moriasi et al. (2007) and references therein for full details of these performance  
405 evaluation metrics. The same metrics were applied to 101 points representing log specific  
406 (flow per unit catchment area) FDCs for each site for each method for the February and all-  
407 time FDCs separately.

## 408 **5. Results**

### 409 *a. Hydrological indices*

410 Synchronisation of TopNet Version 1 with the observed time-series made little impact  
411 on the performance metrics (NSE, RSR and pbias) when compared to using the full TopNet  
412 time-series (Table 5). This was especially the case for  $Q_{\text{bar}}$ ,  $Q_{\text{MALF}}$  and  $Q_{\text{F}}$ . For  $Q_{\text{bar}}$ ,  
413 synchronisation marginally reduced an overestimation bias, but also resulted in a small  
414 reduction in performance in terms of NSE and RSR (reduced NSE, increased RSR). For  
415  $Q_{\text{MALF}}$ , synchronisation resulted in increased overprediction bias, but marginally improved  
416 performance in terms of NSE and RSR. The process of synchronisation did alter performance  
417 for  $Q_{\text{F}}$  as synchronisation improved performance in terms of NES and RSR, but substituted  
418 an overprediction bias with an underprediction bias of the same magnitude. These results  
419 indicate that it was not the case that there were non-stationarities in observed hydrological  
420 regimes that were generally detectable in the TopNet time-series for  $Q_{\text{bar}}$ ,  $Q_{\text{MALF}}$  or  $Q_{\text{F}}$ . This  
421 may not have been the case for  $Q_{\text{Feb}}$ . This is an understandable result as  $Q_{\text{bar}}$ ,  $Q_{\text{MALF}}$  and  $Q_{\text{F}}$   
422 will be less sensitive to inter-annual variability than  $Q_{\text{Feb}}$ . This is because  $Q_{\text{bar}}$  is an average  
423 calculated over all the record, and both  $Q_{\text{MALF}}$  and  $Q_{\text{F}}$  are both averages of indices calculated

424 for each year of record, whereas  $Q_{Feb}$  is calculated over a smaller time-window in each year  
425 of record.

426 Overall there was more difference in performance between TopNet Version 0 and  
427 TopNet Version 1 than there were differences between synchronisation and non-  
428 synchronisation of TopNet Version 1. This indicates that TopNet results are more sensitive to  
429 changes to the TopNet  $f$  parameter than to either the assumption that the 1973-2010 time-  
430 series represent the long-term flow regime, or any non-stationarities combined with relatively  
431 short records in the observed time-series.

432 When compared to TopNet Version 0, TopNet Version 1 reduced an overestimation  
433 of  $Q_{bar}$ , but reduced performance in terms of NSE and RSR. For  $Q_{Feb}$ , TopNet Version 1  
434 marginally improved NSE, reduced an overestimation pbias, but increased RSR. For  $Q_{MALF}$ ,  
435 TopNet Version 1 dramatically improved NSE, improved RSR and replaced a large  
436 overestimation with an underestimation of lesser magnitude. For  $Q_F$ , TopNet Version 1  
437 reduced performance of all metrics when compared to TopNet Version 0. This indicates that  
438 high flows were not better predicted following the regionalisation of the TopNet  $f$  parameter.  
439 However, over all four indices there were greater differences between methods (TopNet,  
440 HUC and Random Forests) than there was between the two TopNet versions (Table 5, Figure  
441 4).

442 The TopNet time-series was corrected using the jack-knifed Random Forests FDC  
443 estimates and then used to estimate the hydrological indices. For all indices and both TopNet  
444 versions, corrected estimates improved performance in terms of NSE and RSR when  
445 compared to the uncorrected TopNet estimates. Corrected estimates produced less bias as  
446 indicated by smaller magnitude pbias when compared to uncorrected estimates from both  
447 TopNet versions for all indices except  $Q_{Feb}$  for Version 1 and  $Q_F$  for version 0. Correction of  
448 TopNet Version 1 caused an increase in overprediction of  $Q_{Feb}$ . Correction of TopNet

449 Version 0 caused an overprediction to change to an underprediction of greater magnitude.  
450 Overall, correction greatly reduced differences in performance between the two TopNet  
451 versions (Table 5, Figure 4).

452 For  $Q_{\text{bar}}$  and  $Q_{\text{Feb}}$  there was more difference between TopNet Version 0 and TopNet  
453 Version 1 than there was between TopNet Version 1 and TopNet 1 Corrected. After  
454 correction, the performance of  $Q_{\text{bar}}$  estimated from both TopNet versions matched the  
455 performance of those estimated using Random Forests. This was because the correction  
456 procedure forced the TopNet corrected estimated FDCs to match jack-knifed Random Forests  
457 estimated FDCs and therefore TopNet corrected  $Q_{\text{bar}}$  matched jack-knifed Random Forests  
458 estimated  $Q_{\text{bar}}$ .

459 NSE was positive (negative values indicate that the mean observed value is a better  
460 predictor than the simulated value) for all indices for all methods except  $Q_{\text{F}}$  for Method 2  
461 HUC (Table 5). This indicates that, except for  $Q_{\text{F}}$  from the HUC method, all methods  
462 provided some degree of useful information about patterns in the estimated values. In this  
463 comparison HUC estimates of instantaneous  $Q_{\text{F}}$  were compared with observed  $Q_{\text{F}}$  calculated  
464 from mean daily flow data. Poor performance and, in particular, overestimation of  $Q_{\text{F}}$  for  
465 Method 2 HUC was therefore not surprising. In fact, McKerchar and Pearson (1989)  
466 previously showed that the method was able to explain a substantial fraction of the observed  
467 variation in  $Q_{\text{F}}$  when compared to observed values calculated from instantaneous flow data.

468 For  $Q_{\text{bar}}$  the HUC method performed best in terms of both NSE and RSR. This is the  
469 method already recommended by Woods, et al. (2006). For  $Q_{\text{MALF}}$ ,  $Q_{\text{F}}$  and  $Q_{\text{Feb}}$  the Random  
470 Forests method performed best in terms of both NSE and RSR. The Random Forests method  
471 also gave the lowest magnitude pbias for  $Q_{\text{F}}$  and  $Q_{\text{Feb}}$  but not for  $Q_{\text{MALF}}$  (Table 5). These  
472 findings correspond well with visual inspection of observed against predicted values, which

473 indicated that the Random Forests method reduced scatter and produced unbiased estimates  
474 for all four indices but was out-performed by Method 2 HUC for  $Q_{\text{bar}}$  (Figure 4).

475 *b. Flow duration curves*

476 More sites had better performance as indicated by higher NSE values, lower RSR  
477 values and lower magnitude pbias for all-time FDCs compared to February FDCs regardless  
478 of estimation method (Figure 5). This indicates greater uncertainties associated with  
479 estimation of February FDCs compared to all-time FDCs. More sites had better performance  
480 in terms of NSE, RSR and pbias for TopNet Version 1 in comparison to TopNet Version 0  
481 for the all-time FDC and the February FDC in particular. Negative pbias values for many  
482 TopNet Version 0 estimated February FDCs indicated consistent underestimation. This  
483 consistent underestimation was not present for TopNet Version 1, which showed an equal  
484 likelihood for either underestimation or overestimation of the February FDC. This indicated  
485 that regionalisation of the TopNet  $f$  parameter improved flow estimation, particularly in  
486 February.

487 Both the HUC and the Random Forests methods performed better than either of the  
488 uncorrected TopNet methods for both the all-time and February FDCs. Both all-time and  
489 February FDCs had more sites with higher NSE, lower RSR and lower magnitude pbias when  
490 estimated using the Random Forests method compared to the other methods. Since the  
491 TopNet 1 Corrected estimated all-time FDC was corrected using the jack-knifed Random  
492 Forests estimated FDC, performance of the TopNet 1 Corrected estimated all-time FDC was  
493 the same as the jack-knifed Random Forests estimated FDC.

494 *c. National estimates for New Zealand*

495 All methods were able to provide predictions for ungauged sites across New Zealand  
496 which reproduced the major regional variations in observed  $Q_{\text{MALF}}$  (Figure 6). These  
497 geographical patterns included a strong east-west gradient in the South Island as well as the



498 influence of the Southern Alps (see Figure 1 for place names). As they cross the eastern  
499 plains of the South Island, large mountain-fed rivers with markedly higher  $Q_{MALF}$  stand out  
500 against a background of comparatively lower-yielding lowland streams. To the northeast of  
501 the central North Island, the rivers draining a volcanic plateau have relatively high  $Q_{MALF}$ ,  
502 with large storage capacity in the thick pumice and ash layers sustaining low flows (Mosley  
503 and Pearson, 1997). Both Random Forests (Figure 6c) and TopNet (Figure 6d) predicted  
504 lower values of  $Q_{MALF}$  than HUC (Figure 6b) for the south west coast of the South Island, but  
505 predicted slightly higher  $Q_{MALF}$  for most other locations in comparison with HUC. It should  
506 be noted that none of the methods were designed to take account of large engineering  
507 schemes such as those currently in place on several of New Zealand's large rivers (e.g. the  
508 Waikato, Rangitata, Waitaki, Clutha and Waiau rivers).

## 509 **6. Discussion**

510 A limited set of hydrological indices along with both the all-time and February FDCs  
511 were investigated (Table 2). This set of hydrological indices included those representing both  
512 high and low flow extremes as well as an aspect of seasonality. These indices are commonly  
513 used for water resource planning in New Zealand, however not all aspects of the flow regime,  
514 such as the frequency of mid-range flows, were represented. This aspect of the flow regime  
515 could have been included by calculating various additional indices such as the number of  
516 events exceeding three times the long-term median flow (FRE3; Biggs 2000), but no HUC  
517 method was available for estimating this index. National estimates of FRE3 using random  
518 forests, including comparison with observed values, were calculated and compared with  
519 observations by Booker (in press).

520 For the Random Forests method FDCs were described using the three parameter GEV  
521 distribution. Other distributions could have been used including log Pearson Type III (LP3;  
522 Ganora et al., 2009) or a mixed gamma distribution (Cheng et al., 2012). Booker and Snelder

523 (2012) showed that, although the LP3 distribution may provide better fits to observed FDCs  
524 when standardised by mean flow, uncertainties in generalising the LP3 parameters from  
525 catchment characteristics meant that a method using the GEV distribution to parameterise the  
526 shape of the FDC gave better performance for prediction at ungauged locations.

527         The same set of independent variables was used to model all four hydrological  
528 indices. Procedures designed to optimise the set of independent variables such as the Model  
529 Improvement Ratio (Murphy et al. 2010) were not employed to optimise the predictor data  
530 set. This approach may not have provided optimal Random Forest models in all cases as one  
531 would expect different sets and different numbers of independent variables to best predict  
532 each dependent variable. For example, summer temperature might be expected to be related  
533 to low flows, but not flood flows. Despite this the Random Forests method still outperformed  
534 the other methods even when a leave-one-out cross validation procedure was applied to allow  
535 for independent assessment of estimation performance against observed data.

536         Although many performance metrics are available to assess model performance, NSE,  
537 RSR and pbias were used as recommended by Moriasi et al., (2007). Although these three  
538 metrics are designed to quantify different aspects of model performance, they often gave  
539 consistent information regarding model performance.

540         The aim of this work was to assess the ability of various methods to estimate  
541 hydrological conditions for ungauged catchments in the absence of major hydrological  
542 alterations such as that caused by abstraction, storage or diversion. The ability to estimate the  
543 effects of either climate change or land cover change were not assessed. It may be necessary  
544 to assess the potential effects of climate change (Zemansky et al., 2012; Earman and  
545 Dettinger 2011), land use change (Scanlon et al., 2007) or their combined effects (Brekke et  
546 al., 2004) on flow regimes to develop rational management strategies. Both TopNet and the  
547 Random Forests models described above have inputs that could be changed to assess the

548 impacts of climate change. However, the validity of this approach was not tested here. It  
549 should be noted that there are several issues relating to model structure and parameterisation  
550 that would need to be resolved when using physically-based models to predict the  
551 hydrological impacts of environmental change (Wagener, 2007). Similarly, when using  
552 flexible empirically-based models such as Random Forests to predict outside of the fitted  
553 model domain it is important to understand how the algorithms perform when projected into  
554 the new environmental conditions (Elith and Graham, 2009).

555         These results indicate that Random Forests outperformed both TopNet versions for all  
556 four hydrological indices as well as for FDCs. This finding corresponds well with the  
557 findings of others. For example, Parkin et al. (1996) found that streamflow predictions from  
558 an a priori parameterised physically-based model contained considerable uncertainty. It  
559 should be noted that, although TopNet Version 1 arguably represents the best currently  
560 available physically-based approach for application to ungauged sites across New Zealand,  
561 this method was uncalibrated. It is known that calibration of TopNet parameters can  
562 significantly improve estimation performance by optimising model performance against  
563 observed flows (e.g. Bandaragoda et al., 2004; McMillan et al., 2013). Calibration procedures  
564 are only possible for catchment specific applications with available flow data. It is possible to  
565 transfer calibrated parameter sets to ungauged sites (e.g. Yu and Yang, 2000) given a suitable  
566 regionalisation procedure (McDonnell and Woods, 2004; Li et al., 2010; Olden et al., 2012;  
567 Coopersmith et al., 2012). Although calibration procedures have been applied to TopNet for  
568 several catchments (Bandaragoda et al., 2004; Clark et al., 2008; McMillan et al. 2013), a  
569 procedure to regionalise the calibrated parameter values is not currently available. Such  
570 procedures can be hampered by issues such as equifinality within the calibration parameter  
571 sets (Beven 2006; Bárdossy, 2007).

572           The Random Forests method can be used to estimate a unique FDC at any location in  
573 the New Zealand river network. These estimated FDCs could be used to provide a more  
574 reliable regionalisation than would be the case using data from observed locations alone  
575 because they represent variability across all of New Zealand rather than a sample of observed  
576 FDCs (Snelder and Booker, 2012). Furthermore, the Random Forests estimated FDC's at  
577 ungauged locations could provide the opportunity to calibrate TopNet parameters against an  
578 estimated FDC for ungauged locations in the New Zealand river network. This would require  
579 a method that allowed calibration against an observed (or estimated) FDC (e.g. Yu and Yang  
580 2000; Yadav et al., 2007; Westerberg et al., 2011). Such a method may be developed as part  
581 of future work. However, considerable improvements in performance were gained when both  
582 TopNet versions were corrected using the jack-knifed estimated FDCs from Random Forests.  
583 This indicates that TopNet performance can be increased considerably without automated  
584 parameter set calibration procedures (Yu and Yang, 2000) or increased understanding of  
585 hydrological processes controlling variability of FDCs across catchments (Yaeger et al.,  
586 2012). Furthermore, the correction procedure reduced differences in performance between  
587 TopNet Version 0 and TopNet Version 1.

588           The TopNet correction procedure tested here represents one relatively crude method  
589 of combining a process-based approach with a data-based approach. The procedure provides  
590 estimates calculated using a data-based approach to correct for bias within FDCs calculated  
591 using a process-based approach. This contrasts with alternative approaches which have  
592 augmented stochastic approaches with more process-based approaches by incorporating  
593 different components of catchment dynamic responses into stochastic models (e.g. Botter et  
594 al., 2007a, 2007b, 2009; Muneeppeerakul et al., 2010; Cheng et al., 2012) or by applying a  
595 water balance modelling framework to divide the FDC into three parts (Yokoo and Sivapalan,  
596 2011).

597           The TopNet correction procedure provided results that matched the performance of  
598 Random Forests for  $Q_{\text{bar}}$  and the all-time FDC, but not for  $Q_{\text{Feb}}$ ,  $Q_{\text{MALF}}$  or  $Q_{\text{F}}$ . It should be  
599 noted that this procedure allowed improved estimation of the entire time-series of flows using  
600 both TopNet versions. This method has a major advantage over the Random Forest method  
601 because any required hydrological indices can be calculated from the estimated time-series.  
602 In contrast, the Random Forests method requires fitting of new models to any newly  
603 calculated indices prior to estimation of these new indices at ungauged sites.

## 604 **7. Conclusion**

605           Results showed the Random Forests method provided the best estimates of both FDCs  
606 and all four hydrological indices except mean flow. Mean flow was best estimated using the  
607 already published HUC method (Woods et al., 2006). Results also showed that considerable  
608 gains in estimation performance can be made by correcting estimates calculated using  
609 physically-based models with estimated values calculated using empirically-based models.

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893 **Tables**

894

895 Table 1. Codes, descriptions and numbers of sites used in the analysis. See Snelder and Biggs  
896 (2002) and Snelder and Hughey (2005) for full descriptions of codes.

897

898 Table 2. Hydrological Indices derived from observed mean daily flows.

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900 Table 3. Summary of the defining characteristics, categories and category membership  
901 criteria that combine to define Source-of-Flow groupings within the REC.

902

903 Table 4. Codes and descriptions of independent variables used to fit regression models. See  
904 Leathwick et al., (2011) for full descriptions.

905

906 Table 5. Various metrics quantifying correspondence between observed and predicted values  
907 for four hydrological indices (Table 2) using various estimation methods.

908

909 **Figures**

910

911 Figure 1. Map showing the locations of the gauging stations used in this study.

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913 Figure 2. Schematic showing different methods used to estimate hydrological indices and  
914 flow duration curves (FDCs).

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916 Figure 3. Hydrology of ungauged catchments (HUC) low flow model and parameters.

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918 Figure 4. Observed against calculated values for each index for each method (n = 485). Grey  
919 dashed line is linear regression. Black line is 1:1 such that x-limits are equal to y-limits for all  
920 plots. Qbar is mean flow. QFeb is proportion of flow in February. QMALF is 7-day mean  
921 annual low flow. QF is mean annual flood.

922

923 Figure 5. Box and whisker plots of Nash-Sutcliffe efficiency, RSR (ratio of the root mean  
924 square error to the standard deviation of observed data) and pbias (average tendency of the  
925 calculated data to be larger or smaller than their observed counterparts) at each site for all-  
926 time and February flow duration curves for each method (n = 101 points at each of 485sites).

927

928 Figure 6. All observations and for each method predictions of 7-day mean annual low flow  
929 (MALF) for all rivers of Strahler order greater than three. TopNet results are for uncorrected  
930 TopNet Version 1.

Figures

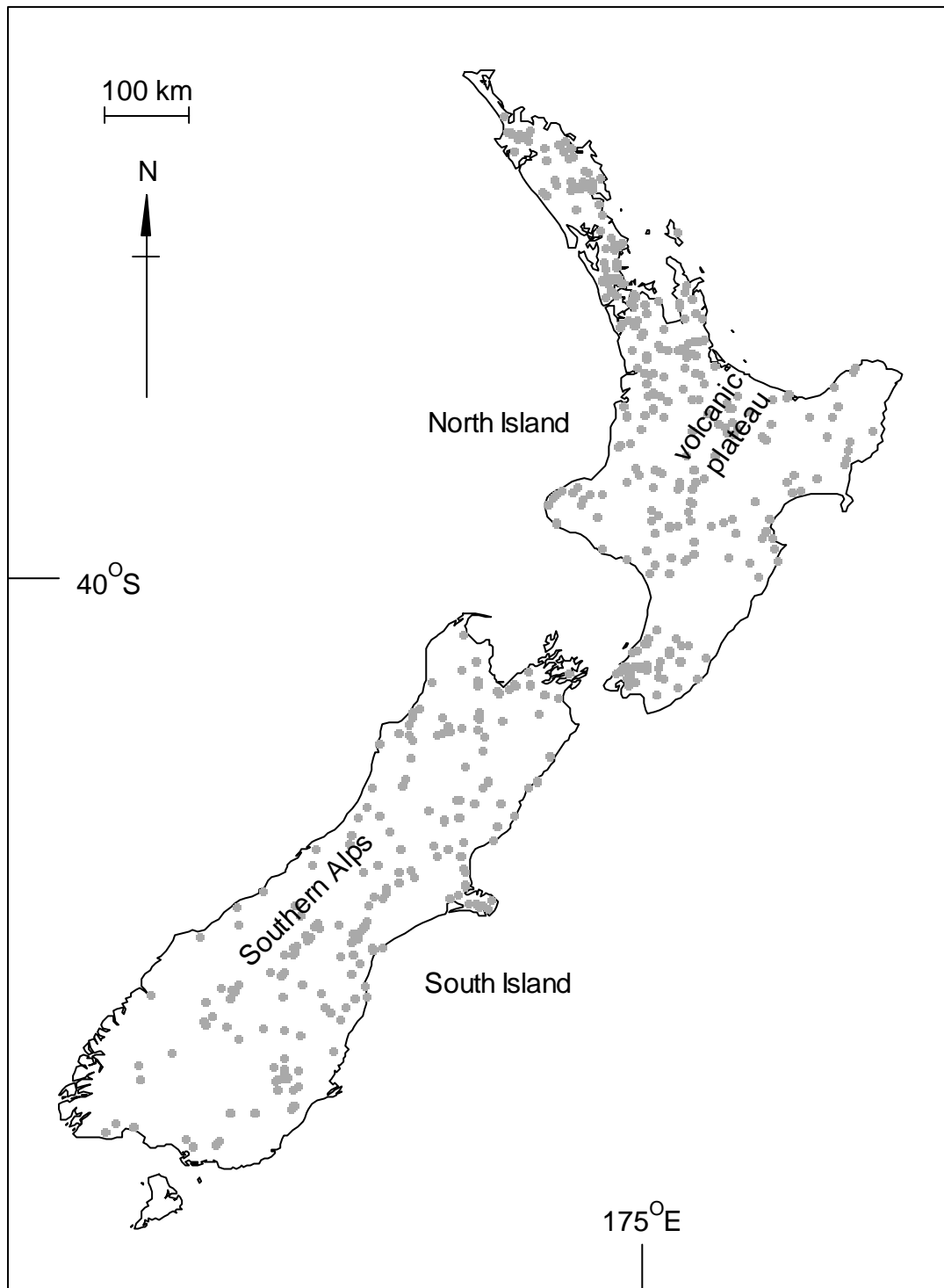


Figure 1. Map showing the locations of the gauging stations used in this study.

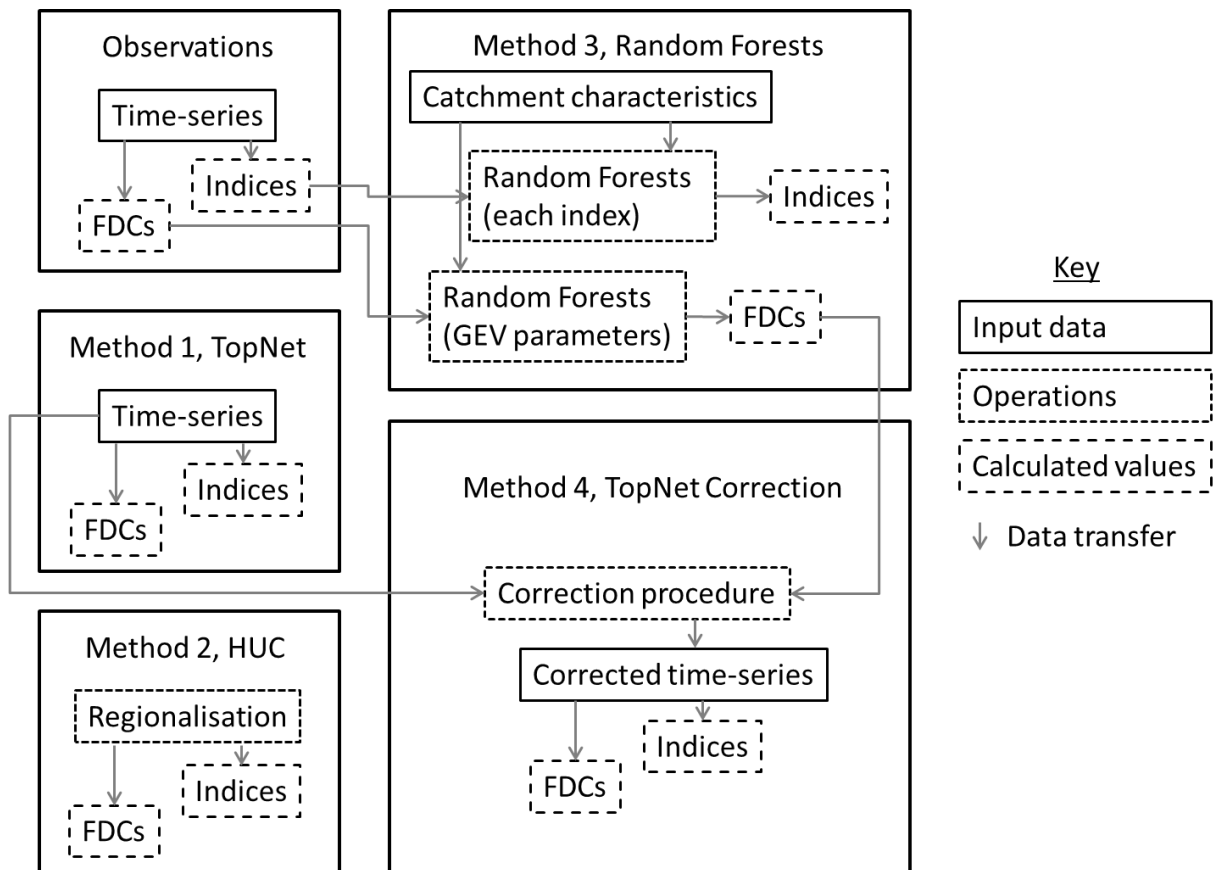


Figure 2. Schematic showing different methods used to estimate hydrological indices and flow duration curves (FDCs).

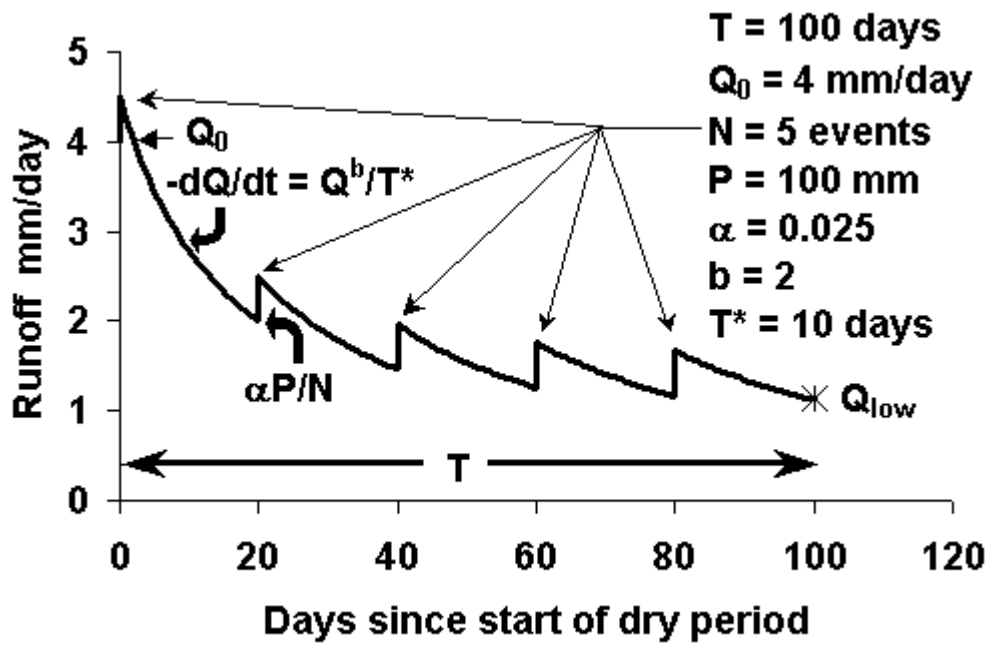


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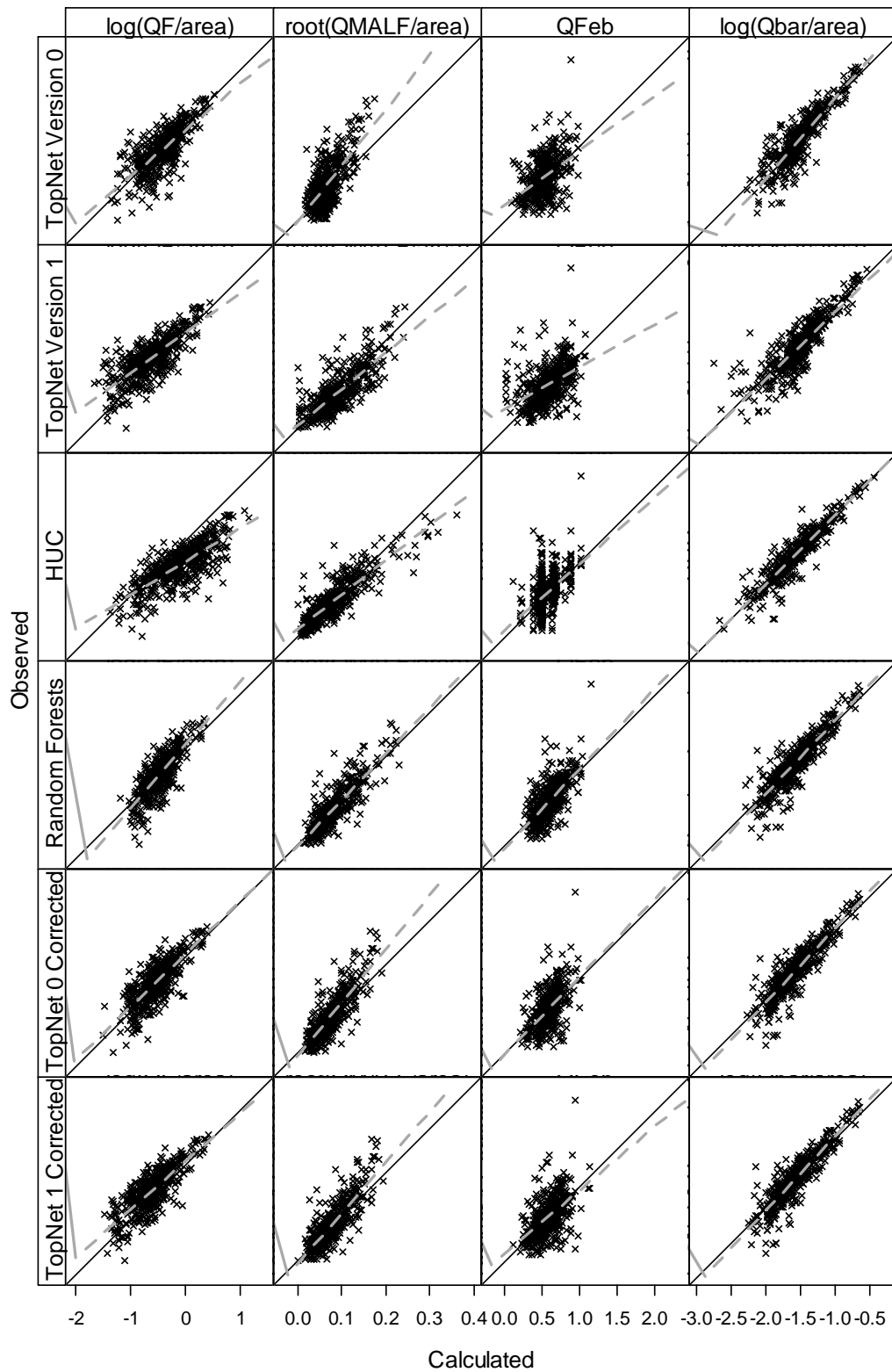


Figure 4. Observed against calculated values for each index for each method ( $n = 485$ ). Grey dashed line is linear regression. Black line is 1:1 such that x-limits are equal to y-limits for all plots. Qbar is mean flow. QFeb is proportion of flow in February. QMALF is 7-day mean annual low flow. QF is mean annual flood.

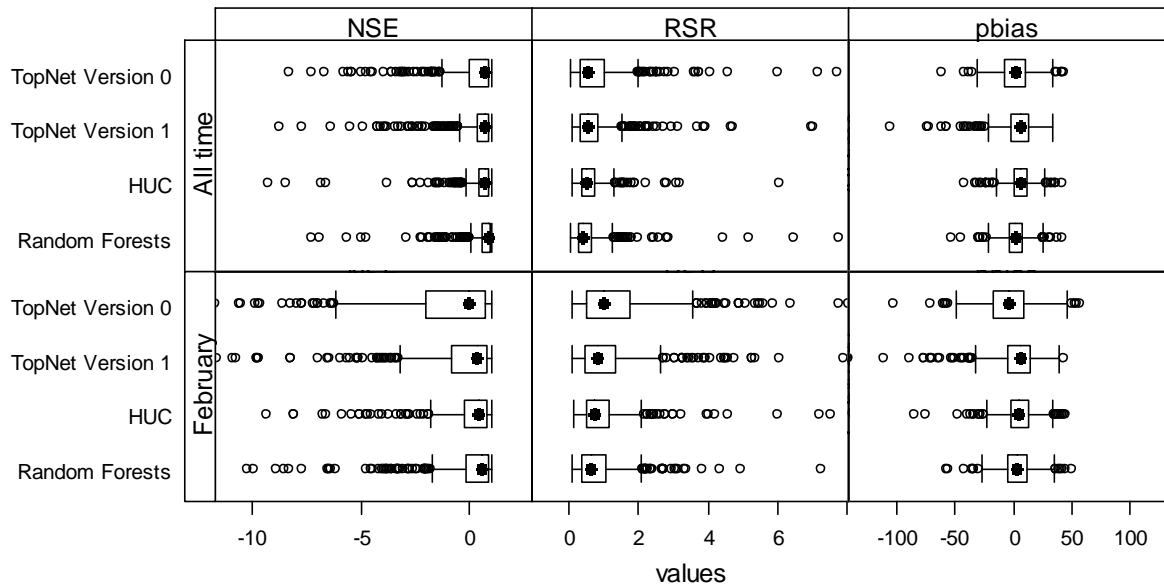


Figure 5. Box and whisker plots of Nash-Sutcliffe efficiency, RSR (ratio of the root mean square error to the standard deviation of observed data) and pbias (average tendency of the calculated data to be larger or smaller than their observed counterparts) at each site for all-time and February flow duration curves for each method (n = 101 points at each of 485sites).



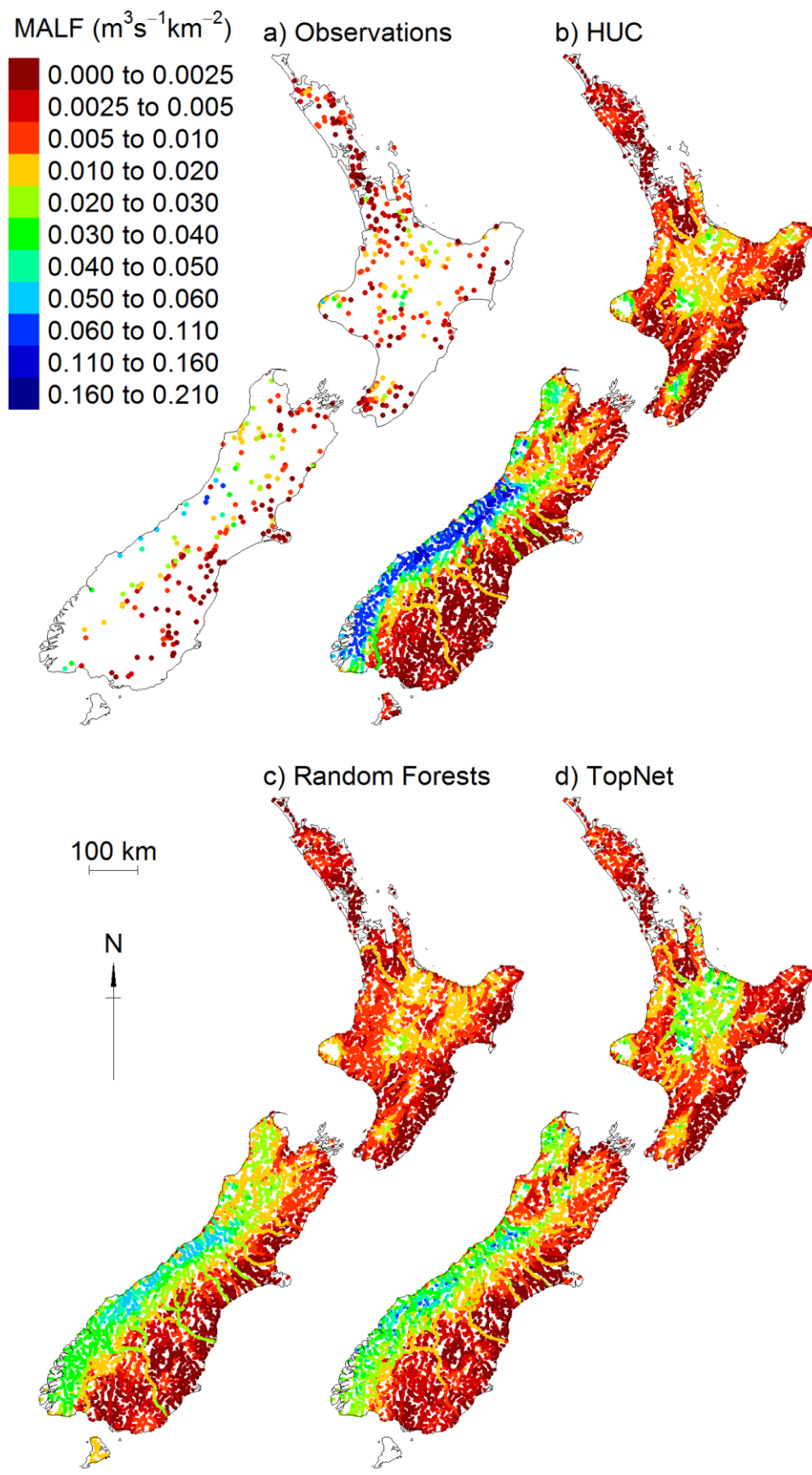


Figure 6. All observations and for each method predictions of 7-day mean annual low flow (MALF) for all rivers of Strahler order greater than three. TopNet results are for uncorrected TopNet Version 1.

## Tables

**Table 1. Codes, descriptions and numbers of sites used in the analysis. See Snelder and Biggs (2002) and Snelder and Hughey (2005) for full descriptions of codes.**

Code	Description	Number of sites, total
Island		
N	North Island	289
S	South Island	196
Climate		
WD	Warm-dry	18
WW	Warm-wet	152
WX	Warm-extremely wet	4
CD	Cool-dry	75
CW	Cool-wet	154
CX	Cool-extremely wet	82
Topographic source of flow		
GM	Glacial mountain	10
H	Hill	167
L	Low elevation	241
Lk	Lake	19
M	Mountain	48
Land cover		
B	Bare	16
EF	Exotic-Forest	22
IF	Indigenous-Forest	105
P	Pastoral	247
S	Scrub	17
T	Tussock	63
U	Urban	15

**Table 2. Hydrological Indices derived from observed mean daily flows.**

Index	Description	Calculation	Standardisation	Transformation
$Q_{\text{bar}}$	Mean flow over all time	Mean of all daily flows	Divide by catchment area to get specific mean flow ( $\text{m}^3 \text{s}^{-1} \text{km}^{-2}$ )	Log base 10
$Q_{\text{Feb}}$	Proportion of flow in February	Mean of all daily flows for each calendar month after having divided by the overall mean flow	Divide by mean flow over entire record to get proportion of flow in February (unitless)	None
$Q_{\text{MALF}}$	Mean of minimum 7-day flow in each year	Mean of minimum flow for each water year after having applied a running 7-day mean to the daily flows	Divide by catchment area to get specific QMALF ( $\text{m}^3 \text{s}^{-1} \text{km}^{-2}$ )	Square root
$Q_{\text{F}}$	Mean of maximum flow in each year	Mean of maximum flow for each water year	Divide by catchment area to get specific QF ( $\text{m}^3 \text{s}^{-1} \text{km}^{-2}$ )	Log base 10
FDC	Probability distribution of daily flow	Interpolation of the cumulative frequency distribution of daily flows on to 101 points (0 to 100 in steps of 1)	Divide by catchment area to get specific FDC ( $\text{m}^3 \text{s}^{-1} \text{km}^{-2}$ )	Log base 10
$\text{FDC}_{\text{Feb}}$	Probability distribution of daily flow for February	Interpolation of the cumulative frequency distribution of daily flows for each calendar month on to 101 points (0 to 100 in steps of 1)	Divide by catchment area to get specific FDC ( $\text{m}^3 \text{s}^{-1} \text{km}^{-2}$ )	Log base 10

**Table 3. Summary of the defining characteristics, categories and category membership criteria that combine to define Source-of-Flow groupings within the REC.**

Defining characteristic	Categories	Notation	Category membership criteria
Climate	Warm-extremely-wet	WX	Warm: mean annual temperature $\geq 12^{\circ}\text{C}$
	Warm-wet	WW	Cool: mean annual temperature $< 12^{\circ}\text{C}$
	Warm-dry	WD	Extremely Wet: mean annual effective precipitation <sup>a</sup> $\geq 1500$ mm
	Cool-extremely-wet	CX	Wet: mean annual effective precipitation $> 500$ and $< 1500$ mm
	Cool-wet	CW	Dry: mean annual effective precipitation $\leq 500$ mm
	Cool-dry	CD	
Topography	Glacial-mountain	GM	GM: M and % permanent ice $> 1.5\%$
	Mountain	M	M: $> 50\%$ annual rainfall volume above 1000 m ASL
	Hill	H	H: 50% rainfall volume between 400 and 1000 m ASL
	Low-elevation	L	L: 50% rainfall below 400 m ASL
	Lake	Lk	Lk: Lake influence index <sup>b</sup> $> 0.033$

a. Effective precipitation = annual rainfall – annual potential evapotranspiration

b. See Snelder and Biggs (2002) for a description.

**Table 4. Codes and descriptions of independent variables used to fit regression models. See Leathwick et al., (2011) for full descriptions.**

Variable name	Description
usPET_Q	Annual potential evapotranspiration of catchment (mm)
usRainDays10_Q	Catchment rain days, greater than 10 mm/month (days/year)
usAnRainVar_Q	Coefficient of variation of annual catchment rainfall (m)
usSteep_Q	% annual runoff volume from area of catchment with slope > 30° (%)
usCatElev	Average elevation in the upstream catchment (m)
usParticleSize_Q	Catchment average of particle size (ordinal scale)

**Table 5. Various metrics quantifying correspondence between observed and predicted values for four hydrological indices (Table 2) using various estimation methods.**

Index	Method	n	NSE	pbias	RSR
$\log(Q_{\text{bar}}/\text{area})$					
	TopNet_0	485	0.73	4.050	0.523
	TopNet_1 Sync	456	0.70	3.138	0.552
	TopNet_1	485	0.71	3.469	0.537
	HUC	485	0.87	0.298	0.363
	RFjacked	485	0.80	-0.241	0.446
	TopNet_0 Corrected	485	0.80	-0.410	0.447
	TopNet_1 Corrected	485	0.80	-0.433	0.447
$Q_{\text{Feb}}$					
	TopNet_0	485	0.09	11.733	0.955
	TopNet_1 Sync	456	0.29	-2.420	0.843
	TopNet_1	485	0.08	2.499	0.960
	HUC	485	0.22	5.354	0.884
	RFjacked	485	0.44	0.216	0.748
	TopNet_0 Corrected	485	0.31	2.872	0.828
	TopNet_1 Corrected	485	0.27	3.020	0.853
$\text{root}(Q_{\text{MALF}}/\text{area})$					
	TopNet_0	485	0.36	17.496	0.797
	TopNet_1 Sync	454	0.59	-11.031	0.643
	TopNet_1	485	0.58	-10.739	0.646
	HUC	485	0.71	-0.506	0.540
	RFjacked	485	0.75	0.157	0.499
	TopNet_0 Corrected	485	0.66	9.132	0.587
	TopNet_1 Corrected	485	0.67	5.923	0.571
$\log(Q_{\text{F}}/\text{area})$					
	TopNet_0	485	0.50	7.523	0.704
	TopNet_1 Sync	456	0.30	-36.797	0.837
	TopNet_1	485	0.31	-34.958	0.832
	HUC*	485	-0.45	73.012	1.206
	RFjacked	485	0.63	-0.674	0.609
	TopNet_0 Corrected	485	0.55	-16.521	0.668
	TopNet_1 Corrected	485	0.46	-31.733	0.734

\* Table footnote: In this comparison HUC estimates of instantaneous  $Q_{\text{F}}$  were compared with observed  $Q_{\text{F}}$  calculated from mean daily flow data.