

MSE Walls Retrofit Design and Construction 110th Annual Road School 2024





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- 1. What is an MSE
- 2. External Stability
- 3. Internal Stability 9. SR
- MSE Walls Failures Survey
 for 30 States
- 5. MSE Walls Retrofit
- 6. Contract B-43196 (40 MSE
 Wall Repair Statewide
 INDOT)

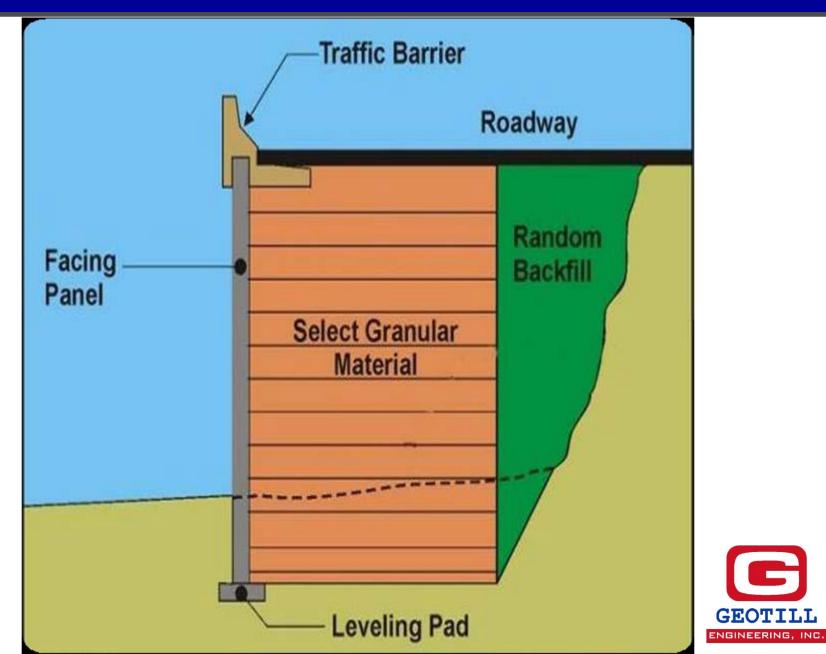
- 7. I-65 and I-80-I-94-Wall
- 8. I-69 over SR 57
- SR-641 Over McDaniel

Road

- 10. 82nd St Over I-465
- 11. INDOT Various Projects



E Typical section & components of MSE structures



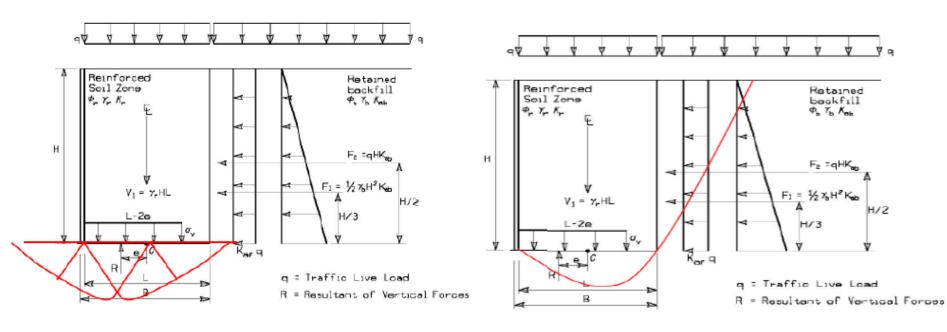




- Bearing resistance
- Settlement
- Sliding
- Overturning
- Overall Stability







(a) Assumed Bearing Failure Zone of Current Practice

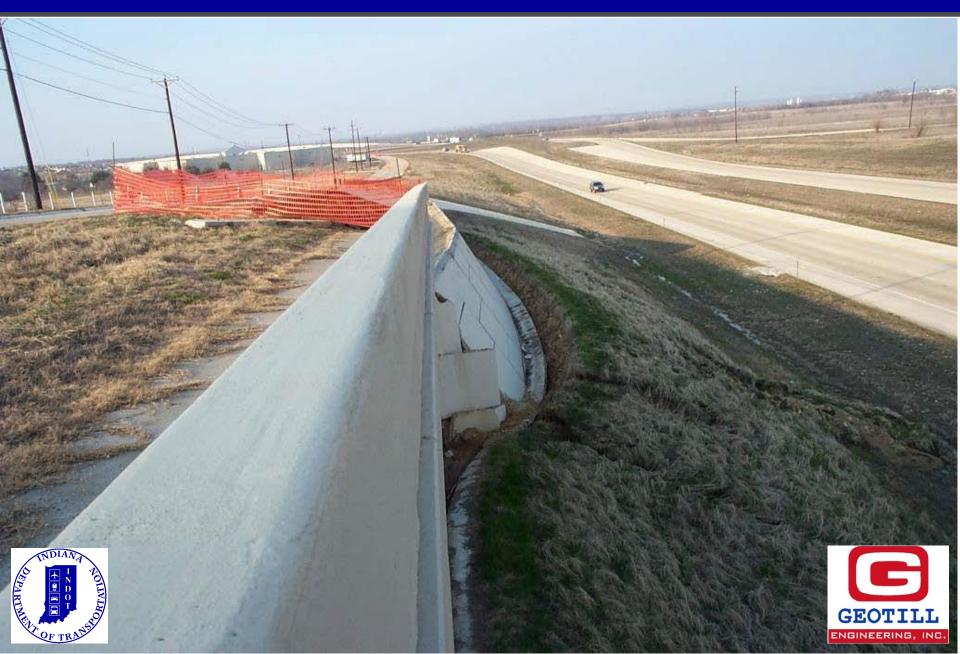
(b) Possible Failure Zone

Bearing Capacity of Retaining Walls

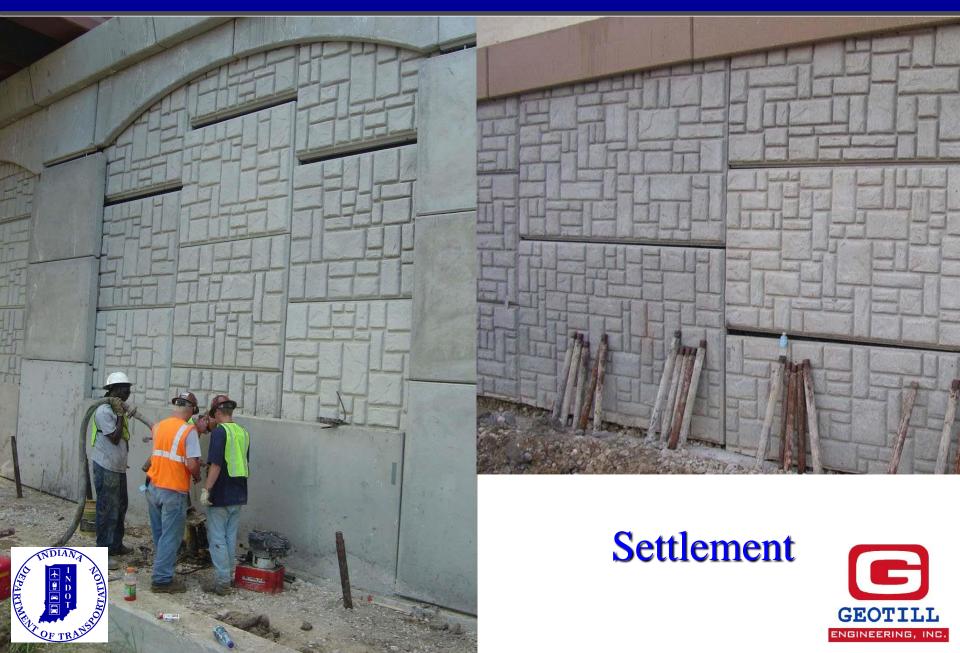




Bearing Failure









TYPE OF FACING	LIMIT OF DIFFERENTIAL SETTLEMENT		
CIP Gravity Wall	1/500	2 In. in 100 Ft.	
Welded Wire Facing	1/50	2 Ft. in 100 Ft.	
5' x 5' Panels with 3/4" Joints	1/100	1 Ft. in 100 Ft.	
5' x 10' Panels with 3/4" Joints	1/200	6 In. in 100 Ft.	
Modular Block Walls	1/200	6 In. in 100 Ft.	
Full Height Facing Panels	1/500	2 In. in 100 Ft.	
Differential Sett	lement	GEOTILI ENGINEERING, IN	

ENGINEERING, INC.







Wall Embedment

Slope in Front of Wall	Minimum to Top of Leveling Pad	
Horizontal (walls)	H / 20	H = 100 ft
Horizontal (abutments)	H / 10	Embed.
3 Horiz : 1 Vert	H / 10	10 ft
2 Horiz : 1 Vert	H/7	3
3 Horiz : 2 Vert	H / 5	

- For sloping ground provide a bench 4 ft bench in front of wall
- Embed at least 2 ft below anticipated scour depth
- Embed below frost depth, Indiana 3 ft







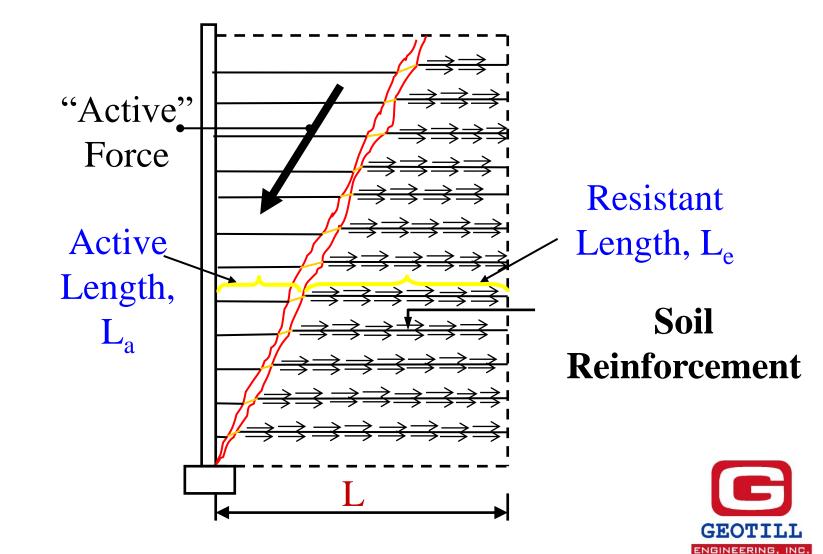








How do Reinforcements Hold Up the Wall?







Internal Stability Reinforcement Failure Modes

- Pull out
- Reinforcement tensile strength
- Connection









MSE Walls Failures Survey for 30 States

Survey done by Texas A&M in 2013 for TxDOT

- Massachusetts DOT. Oregon DOT. Nevada DOT. Idaho DOT. Indiana DOT. • Missouri DOT. • New Mexico DOT. • Alabama DOT. • Kansas DOT. • Connecticut DOT. • Illinois DOT. • Iowa DOT. • Maryland DOT. • New Hampshire DOT. • New York DOT. • South Carolina DOT. • Washington DOT.
 Wisconsin DOT. • California DOT. • Minnesota DOT. • Missouri DOT. • Montana DOT. • North Carolina DOT. • Vermont DOT. • Wyoming DOT. • Nebraska DOT. • Louisianan DOT. Texas DOT.
- All the DOTs have indicated that the FHWA design methodology has been followed to design the MSE walls.
- The minimum reinforcement length used for design is 0.7H or 8 ft, whichever is longer.
- In all, DOTs have different backfill specifications from FHWA guidelines. However, many of them have higher quality of material than the FHWA guidelines.





MSE Walls Failures Survey for 30 States

Survey Results

- According to the frequency of the failure modes being seen, the ranking of the failure modes with frequency descending is:
- Bearing Capacity.
- Sliding.
- Global Stability.
- Compound Failure.
- Overturning.







Retrofit that did not Work I-10





I-10 in Beaumont, Texas - Distressed Portion of the Roadway





Retrofit that did not Work I-10





The separation ran about 100 ft long and up to 3 inches wide. Cracking was also observed at the connection between the copping/barrier and the MSE wall panels. To prevent the propagation of the settlement and further separation, a retrofit measure was taken by dowelling the shoulder to the adjacent travel lanes. After the retrofit, cracks appeared in the same travel lanes again, but were shifted toward the centerline of the roadway





General Identified Failure Distress Modes in MSE Walls

- 1. Pavement cracking.
- 2. Pavement depression and separation.
- 3. MSE wall lateral movement.
- 4. MSE wall and bridge separation.
- **5. MSE wall longitudinal movement.**
- 6. MSE wall panel dislocation.
- **7. MSE wall sliding.**







Possible Causes of Failures in MSE Walls

- 1. Unsuitable backfill material.
- 2. Sizeable void within backfill.
- 3. Drainage and Erosion.
- 4. Foundation soil.
- 5. Tension failure of the reinforcements.
- 6. Failure of connection of the reinforcement
 - The sizeable void may be the result of poor construction, inappropriate gradation of backfill naterial or poor drainage.





When Visiting MSE Wall that has Issues (What to look for per minimum)

- 1. Check all drainage and erosions issues.
- 2. Check for any excessive settlements.
- 3. Check precast concrete panel facing do they need rejection or repair.
- 4. Check field modification limitations (e.g., cutting panels, resting panels, anchoring panels, use nails, etc.).
- 5. Check any backfill migration from facings.
- 6. Check backfill void mitigation and void-filling techniques.
- 7. Wall condition monitoring methods if needed.
- 8. Check distressed joint openings.
- 9. Check local facing repair or replacement strategies.
- 10. Check global repair or replacement strategies.
- 11. Check global wall stabilization.







Check List - Wall and Slope Distress Visual Indicators

Obse	erved			CHECKS (characteristics)		
Yes	No	NA				
			1.	Cracks in slope/pavement (circle size: marble,		
	golf b	all, tenr	nis ball	, football,)		
			2.	For cracks, circle: new/previous,		
	sealed/unsealed, previously treated Approximate distance from wall					
	face:			ft.)		
			3.	Formation of shallow surface slides,		
	depre	ssions	near to	p, bulging near bottom?		
			4.	New or increased dip in paving, curb, gutter		
	line o	r slope?	?			
			5.	Landscape areas (Yes/No) – Working irrigation		
	(Yes/	No)				
			6.	Ponding water?		
			7.	Joint separation in pavement and shoulder join		
	paver	nent?				
			8.	Erosion of soil on slope or around flatwork,		
	flume	s, guar	drails, I	light fixture base, electrical communication bores		
			9.	Soft and / or wet soil (greener grass, ruts)?		
			10.	Washout areas, silt buildup?		
			11.	Signs, structures or rails next to wall (plumb,		
	leanin	ng to/aw	ay froi	m wall faceinches overft.)		
Wall	Face					
Obse	erved			CHECKS (characteristics)		
Yes	No	NA				
			12.	Bulging of panels?		
				Daiging of partolo.		







Retrofit Options

- 1. All drainage and erosions issues shall be resolved, such as sealing all opening, install curbs & riprap channels to divert water from the wall.
- 2. Weep holes to provide an outlet for water to drain and escape.
- 3. Near the top of the wall if the reinforcements overstressed, it may be feasible to add additional reinforcements.
- 4. If access to the backfill is a problem, soil nails drilled through the existing wall face may be used. (Needs deformation to activate soil nails).







Retrofit Options

- 5. Bearing plates may be used to distribute nail loads at the wall face.
- 6. If the increase in lateral thrust and the potential for a sliding failure is a problem, the front of the wall may be buttressed.
- 7. Backfill may be modified to reduce the driving forces, such as grouting, partial replacement with lightweight fill, flowable fill, or decreasing the slope angle of the backfill surface.
- 8. Foundation soil treatment to stop settlement or strengthen the soil.







Shotcrete Retrofit of MSE Wall





British Columbia, Canada, the MSE wall was 26 ft high and 230 ft long, the backfill is fine dredged river sand



MSE Walls Retrofit

Free Draining Seal (Foam pieces wrapped in a fabric cloth)













MSE Walls Retrofit



BEJS

Bridge Expansion Joint System





B-43196 (40 MSE Wall Repair Statewide INDOT) Project Development Process for IDIQ MSE Wall Rehabilitation Projects

- Phase 1: Project Initiation and Programming.
- Phase 2: Project Letting and Post Award.
- Phase 3: Initiate Work Order requests and Site Visit.
- Phase 4: Process Work Order and Schedule Construction Activities.
- Finalize work at site and provide district Asset
 Management Staff as-built of completed work.
- Contractor submits as-built plans to ERMS for final record keeping.





B-43196 (40 MSE Wall Repair Statewide INDOT) Projects List

- 01- 82nd St Over I465
- 02- Kenilworth Road over US31
- 03- SR168 Over I-69
- 04- SR 56-61 over I-69
- 05- SR 57 over I-69
- 06 US31 Over CR125
- 07 US31 & SR 32 Hamilton Co
- 08 US-24 at 154+89
 Over Burick Rd
- 09 US-24 at 156+96-N Webster
- 10 US-24 at RP153+49 over Doyle Rd
 - 1-69 over CR 710 S



- 13- US41 over Norfolk Southern
- 14- New Road US31 & US20
- 15- US-31 Over Main Street
- 16- US-31 Over Kern Road
- 17- US-6 Bridge over CSX R & Tracy Rd
- 18- US31 over 1st Road
- 19- US31 over 3A Road (LaPorte)
- 20- Ameriplex and I-70
- 21-10th St over I-465

- 22- US31 over 3A Rd
- 23- US31 over 1st Rd
- 24- Bridge over CSX RR & Tracy Rd US-6
- 25- Burr St over I-8094
- Lloyd Expressway & 9th Ave.
- 27- I-69 & CR1250S (Nobel Chapel Rd)
- 28- I-69 at N CR450 E
- 29-I-69 at MM77.5 over RR
- 30- 136th and 31 damage
- 31- US-31 over SR-931



B-43196 (40 MSE Wall Repair Statewide INDOT) Projects List

- 32-80-94 EB near Cline Ave
- 33- I74-over-white-river
- 34- SR641 Over McDaniel Rd
- 35- SR641 Over Indiana R.R
- **36- I-65 and I-80-I-94-Wall #41**
- 37- SBL of I-465 under NBL of Shadeland Avenue
- 38- US 31 over Creek close to 156th
- 39- SR 66 (Lloyd Expressway)
- 40-SR 46 & SR 11, SW of Colombus





Most Issues & Problems that have been Noticed

Sand leaking from joints.

- Settlement and tilting of panels.
- Uncontrolled drainage.
- Deteriorating panels.
- Tilting.
- Differential movement.

Voids, Erosion & Approach Slabs.







MSE Walls Retrofit - I-65 and I-80-I-94-Wall #41

Going I-65 South to I-80 West

MSE Wall

ana University Northwest

MSE Walls Retrofit - I-65 and I-80-I-94-Wall #41

MSE Wall

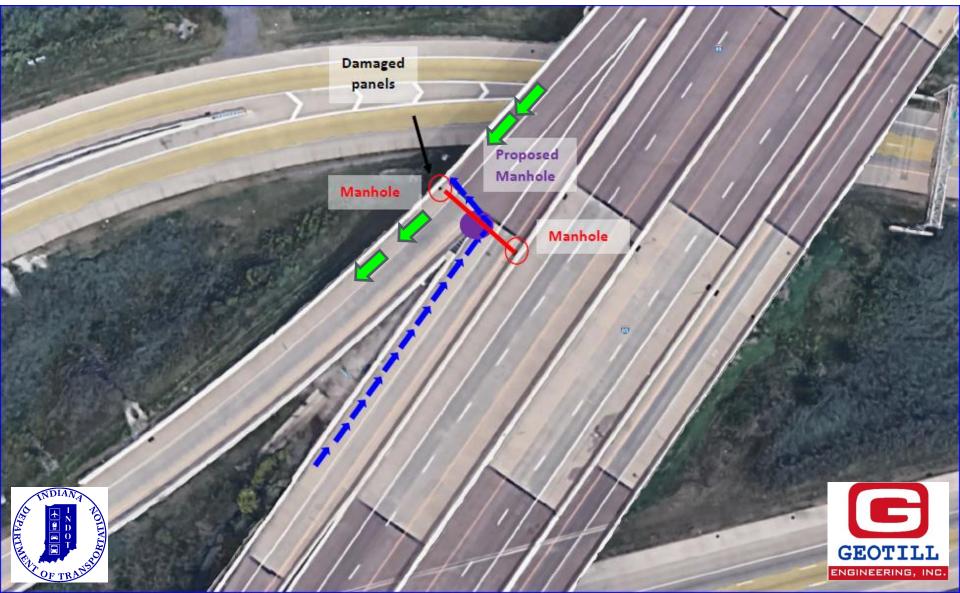
Going I-65 South to I-80 West

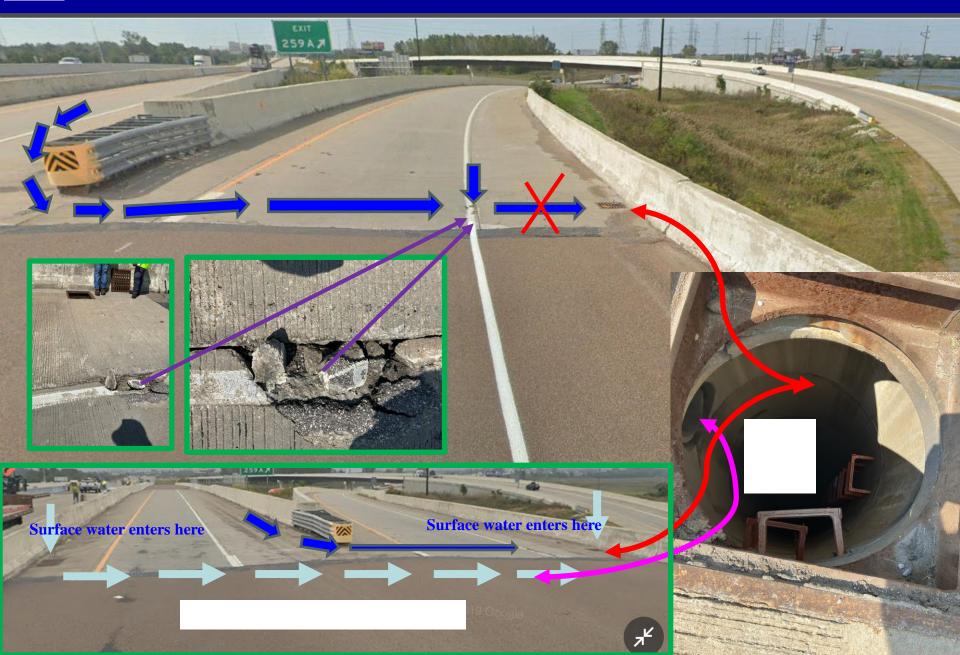
Bellaboots Play and Discovery Center

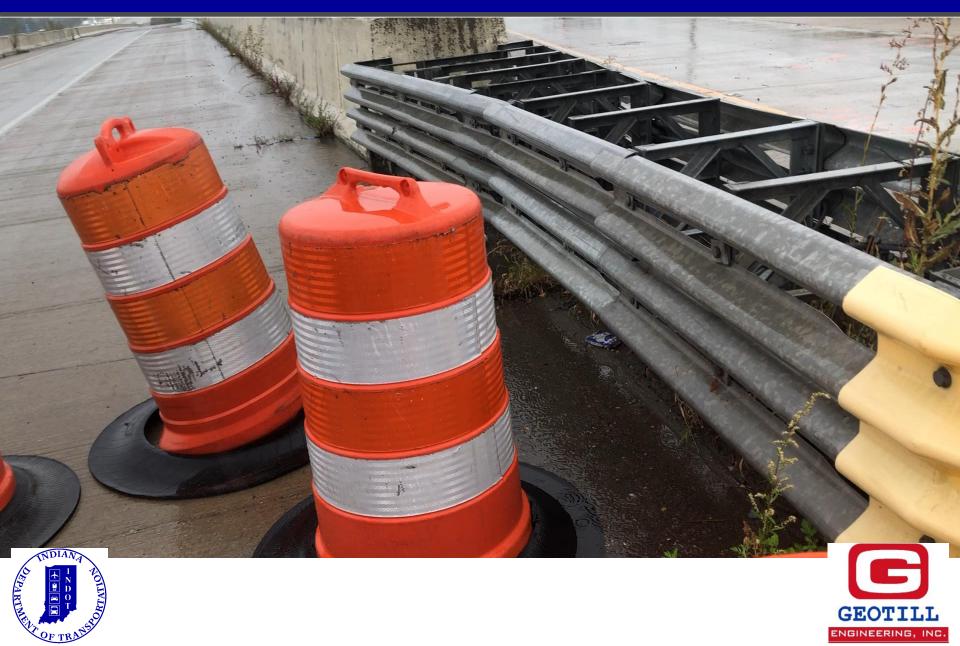
Parking Lot

MSE Walls Retrofit - I-65 and I-80-I-94-Wall #41

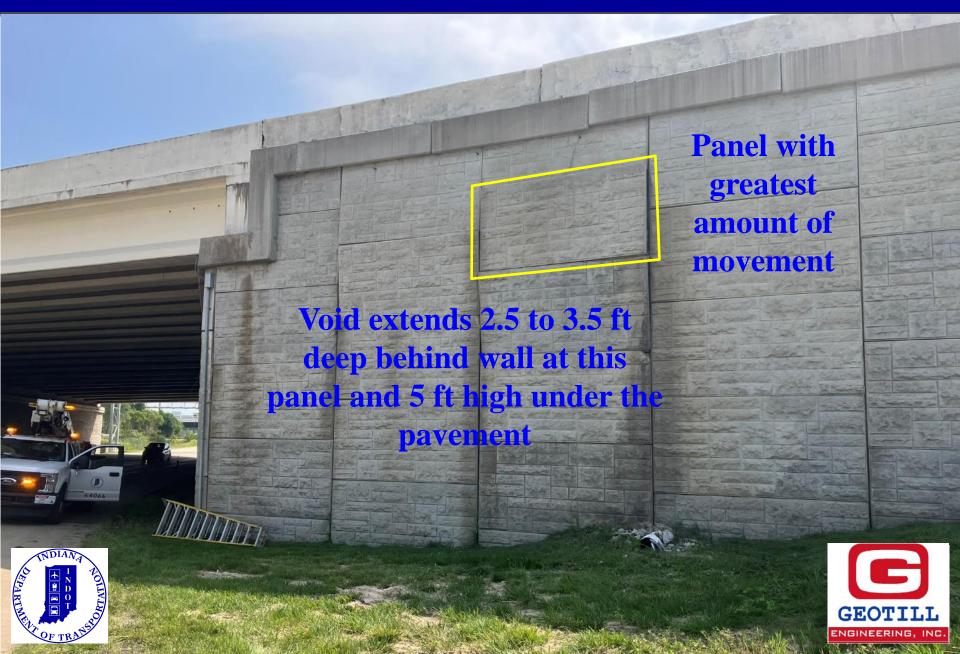
Going I-65 South to I-80 West



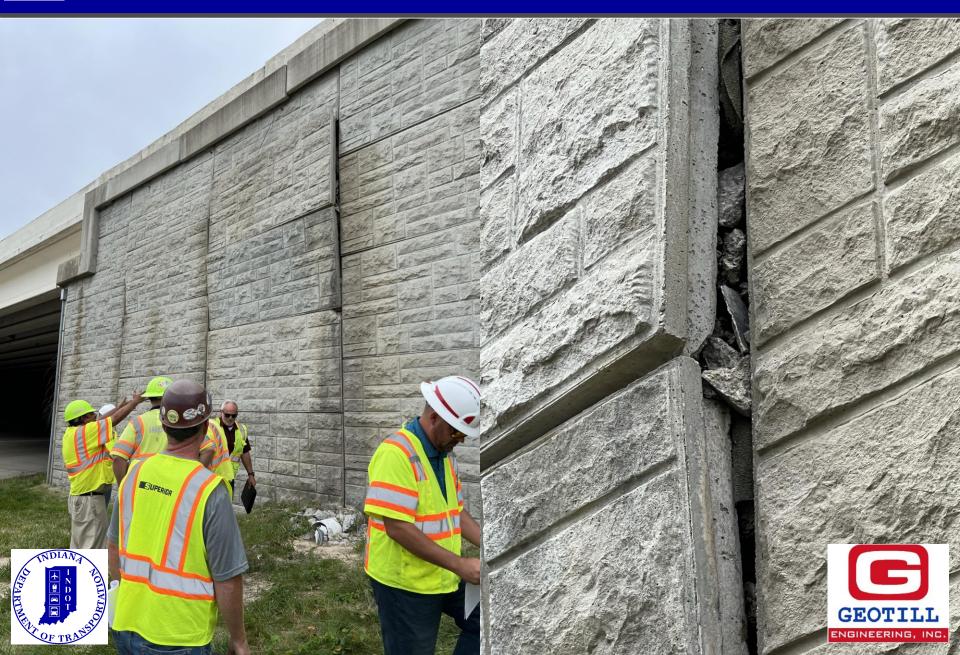








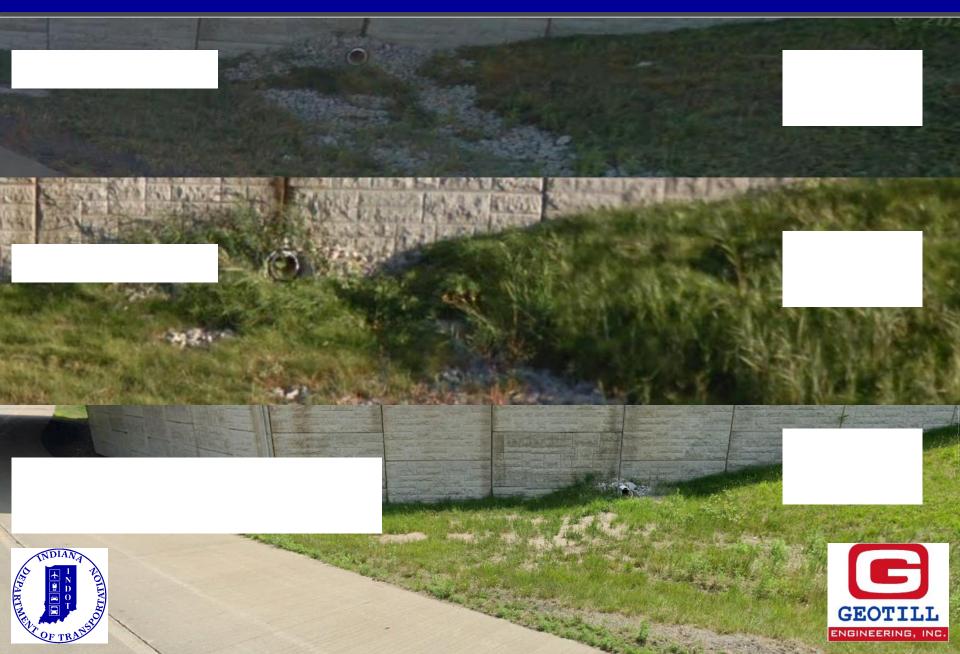












JF TRANSPORT



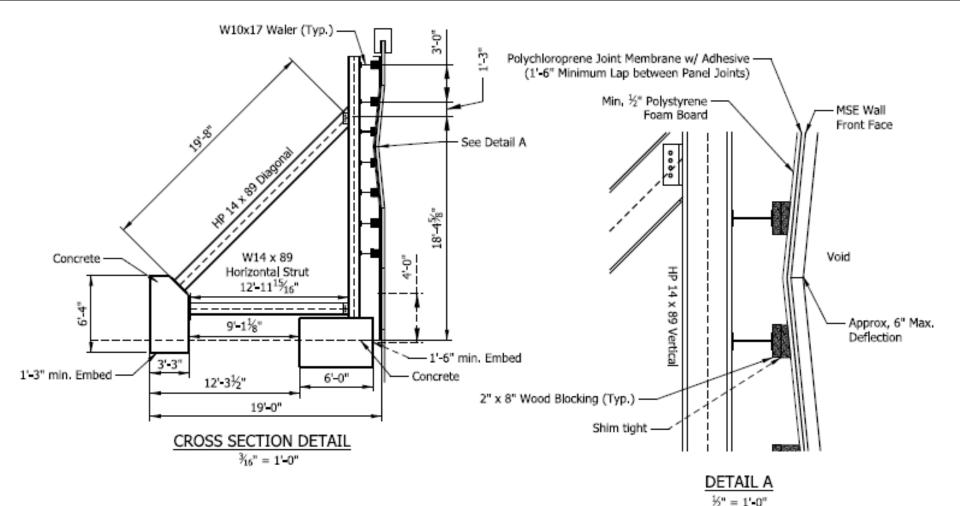
5 ft High

3.5 Wide











Temporary Fix





Temporary Bracing System

ORTATIO





Going I-69 Northbound Near Southwest Wall

Petersburg

MSE Wall

Dubois

Princeton

Oakland City

Fort Branch



PF TR

57





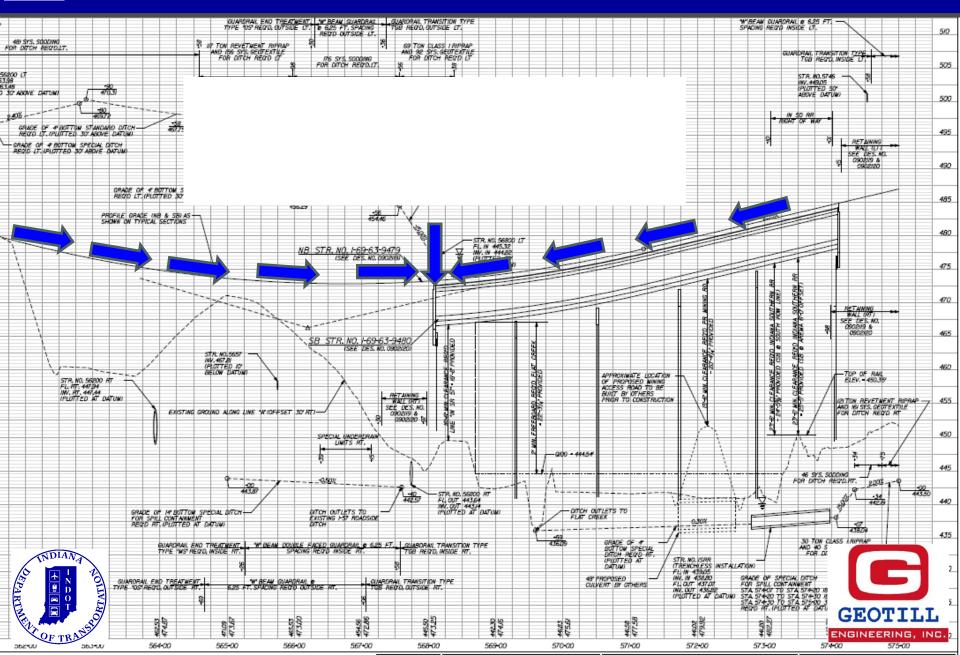
Southbound

Northbound

Showing Northeast Wall Looking Northeast



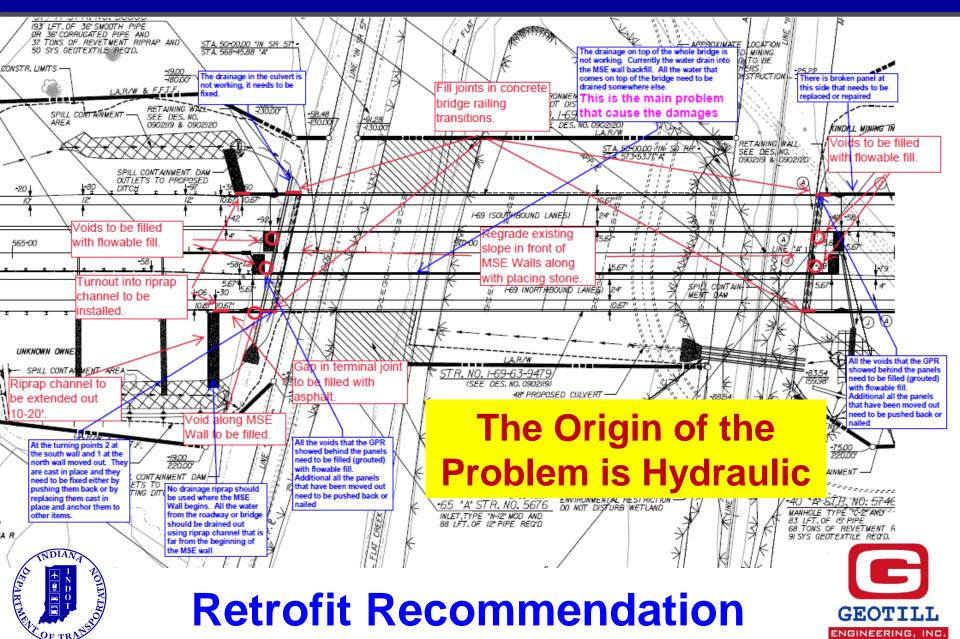
















VDIA/

2F TRA

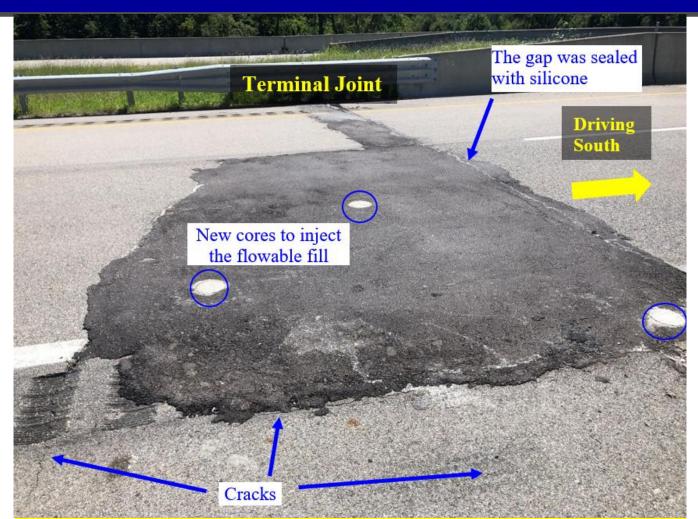
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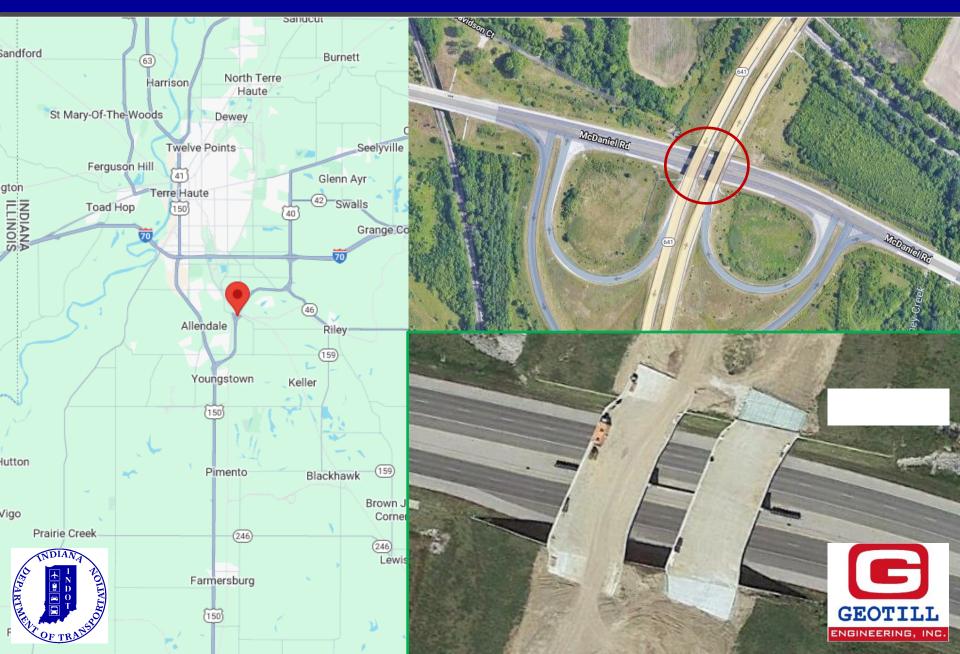


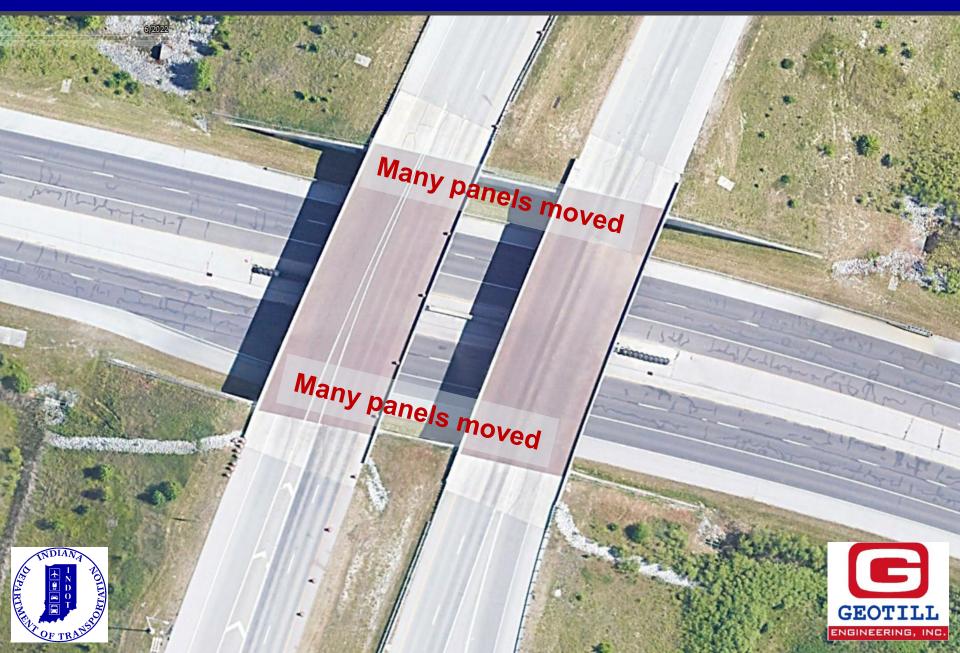




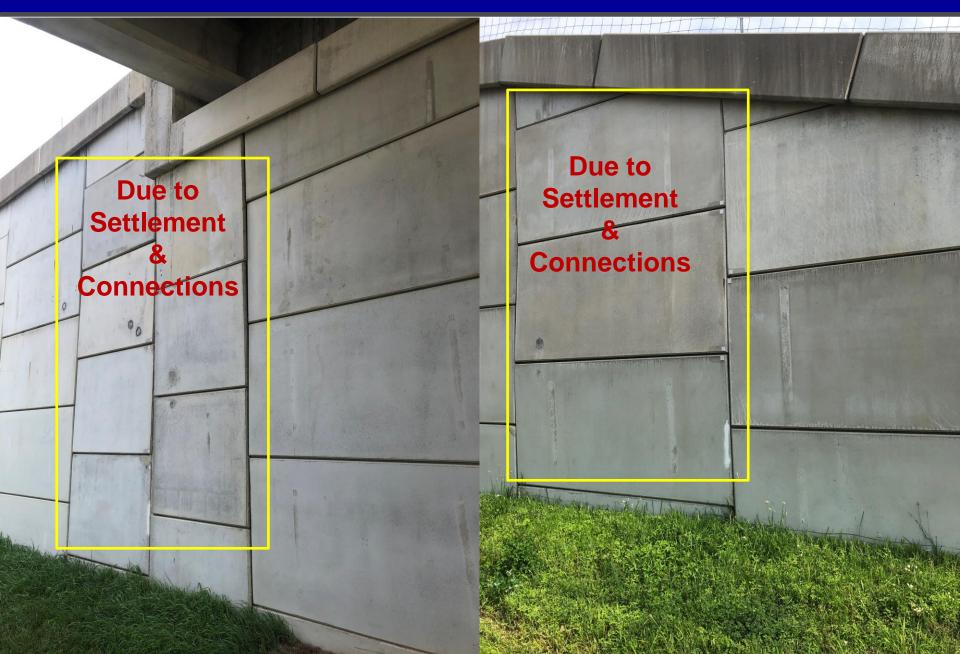
Since the origin of the problem is Hydraulic, permanent repair is scheduled to be done by the summer of 2024. The permanent repair will involve replacing the bridge approaches and installing a new drain system that can handle the amount of water that come on the top of the bridge.

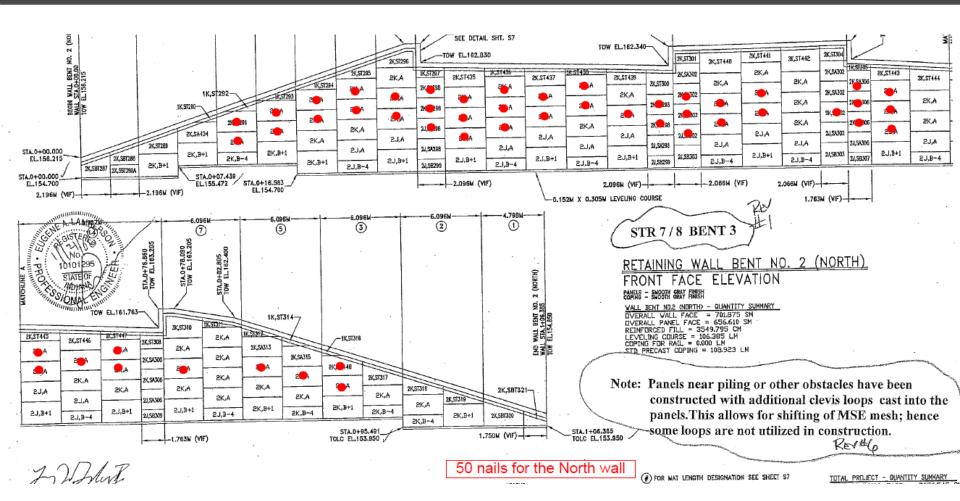








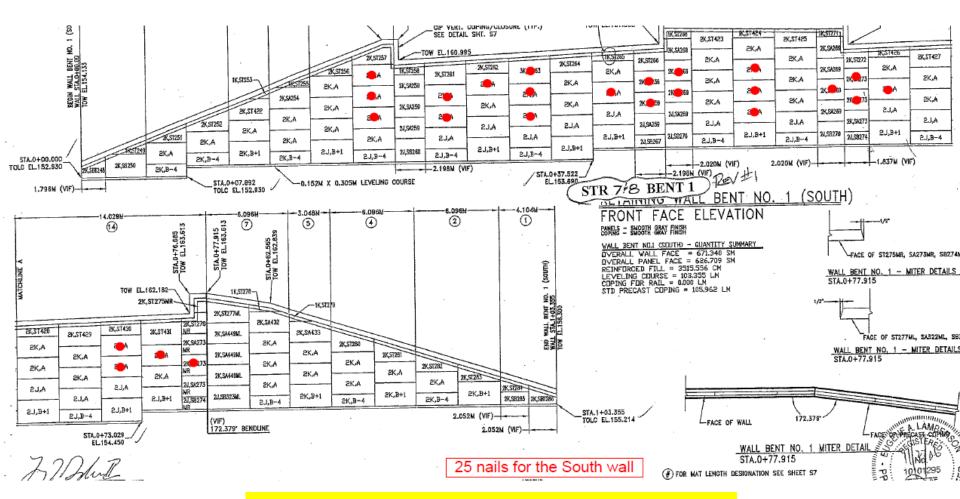






Soil Nail Locations (50 Nails) for the North Wall by INDOT & Geotill







Soil Nail Locations (25 Nails) for the South Wall by INDOT & Geotill





Issues Observed

- Backfill leak from Northwest side.
- Broken CIP copings.
- Some bulging panels.
- Pavement issues probably voids.
- Widen and tighten panel gaps, possibly because of settlement.
- Some blocked underdrain









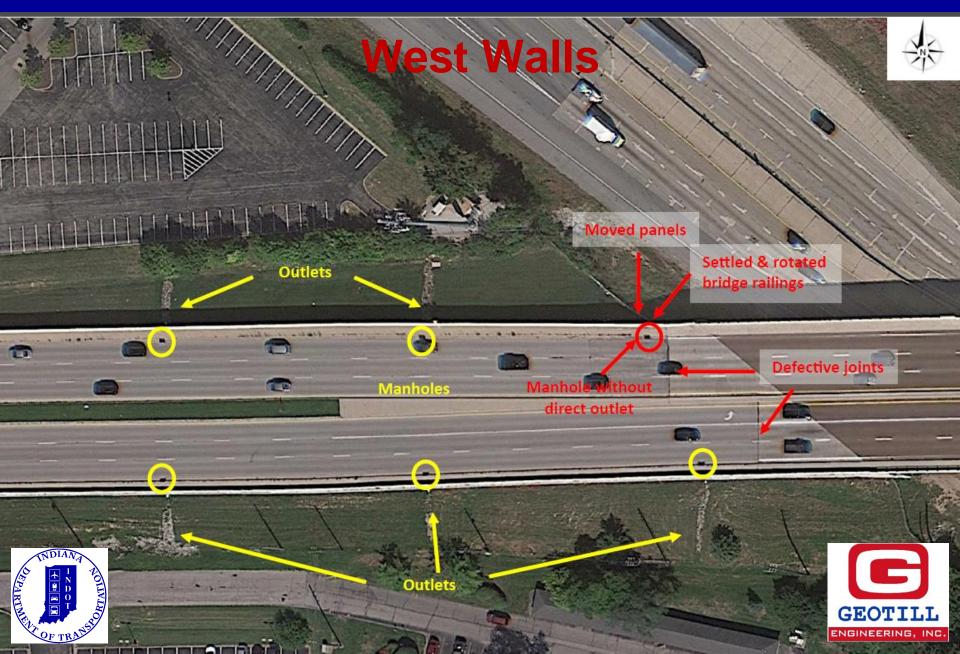




The main problem for this location is that the drainage system is not able to drain all the water away from the MSE walls. The water seeps through the damaged expansion joints and some manholes and then seep on top of the **MSE** walls especially at the northwest wall. In addition, to the known connections problems that Tricon MSE Wall system has.







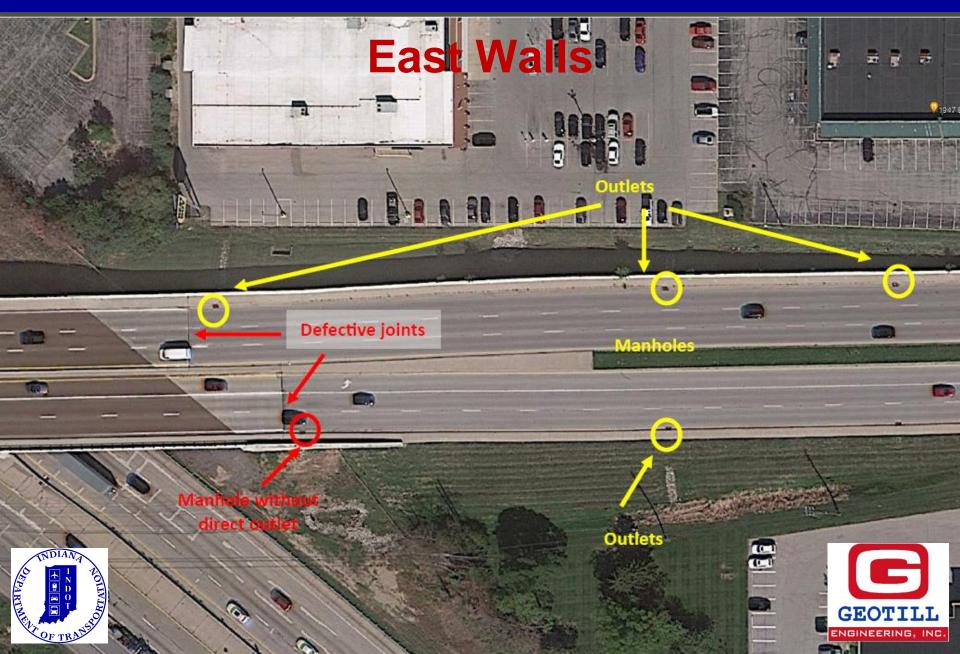














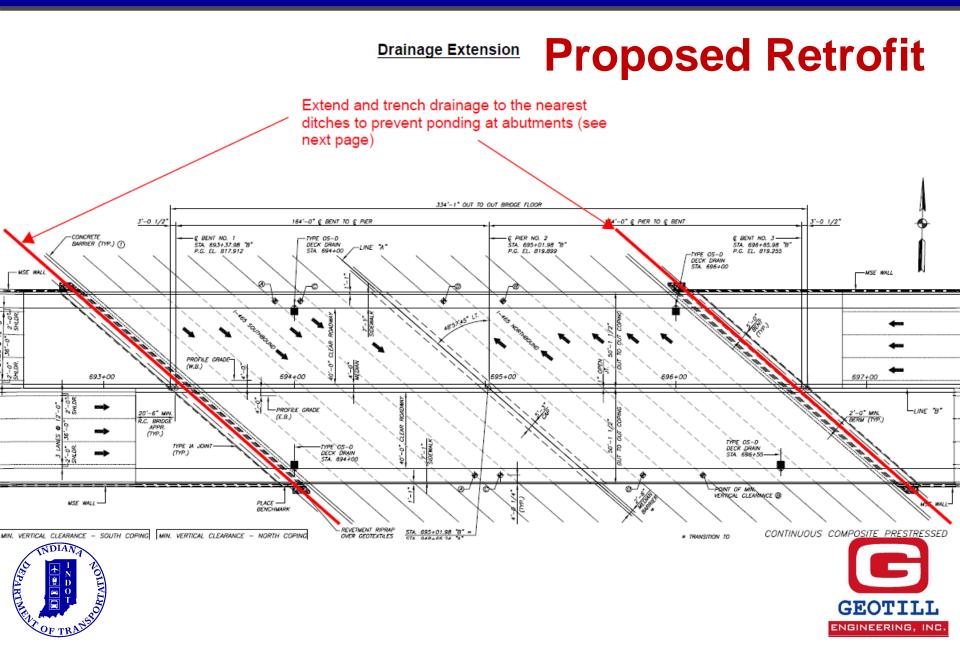




Image showing the wing wall and panels with air gap GPR signatures (red).







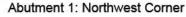
Extend Drains to nearest ditch



Abutment 3: Northeast Corner



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Drainage Extension Proposed Retrofit

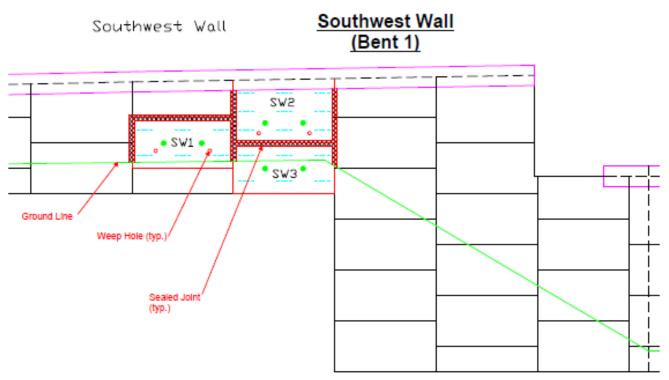


Abutment 1: Standing water due to inadequate drainage



Abutment 1: Southwest Corner

Proposed Retrofit

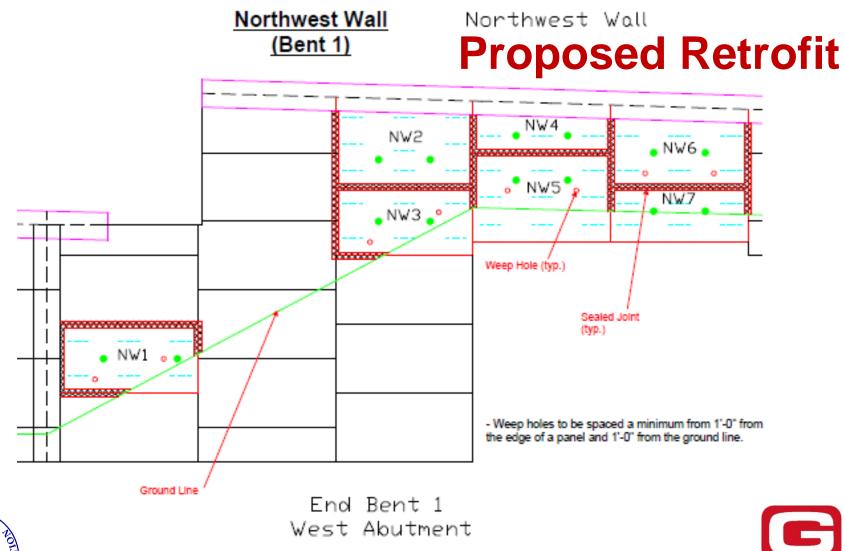


End Bent 1 West Abutment

- Weep holes to be spaced a minimum from 1'-0" from the edge of a panel and 1'-0" from the ground line.







GEOTILL ENGINEERING, INC.





Hollow Bar Anchor System

Geo-Drill Injection Anchor System

The Williams Geo-Drill Injection Anchor System is today's solution for a fast and efficient anchoring system into virtually any type of soil. The system has historically been known as a "self-drilling anchoring" because the hollow fully-threaded bar serves as both the drill string and the grouted anchor, thus installation is performed in a single operation. The sacrificial drill bit is threaded onto the end of the Hollow Injection Bar and left in place following drilling. The drilling fluid (air, water, or grout) is introduced through the hollow bar and allows the spoils to flush from the borehole.

The Geo-Drill System is particularly suitable for soils that do not allow for open-hole drilling (i.e. granular soils that are collapsible in nature). In such cases, drilling with a grout fluid serves the purpose of flushing spoils from the borehole and prevents looser, surrounding material from collapsing due to the higher relative density of the grout. Williams Geo-Drill Injection Anchor System should be considered on any project requiring fast production that would otherwise need to involve a casing system in order to maintain borehole stability.

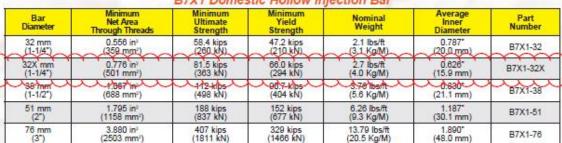
Proposed Retrofit



- · Fully domestic system available.
- · Fast, single-step anchoring system with simple equipment.

(MANADARANA ANTITI A ANTI

- Eliminates the need for a cased borehole in collapsing soils.
- Efficient installation since drilling and grouting can be performed in a single operation, saving both time and money.
- Continuously drilling and grouting under high pressure causes the grout to permeate into looser soils and creates a bulb-effect for increased bond capacity.
- · Suitable for working in limited space and areas of difficult access.
- · Multiple ranges of drill bits suitable for most soil conditions.
- Installed with standard track drill (top hammer) or hand-held drilling equipment, eliminating the need for larger casing rigs.
- · Continuously threaded bar pattern can be cut and coupled anywhere along its length.
- · Domestic available in 10' or 20' lengths, non-domestic available in 3 meter lengths only.
- Corrosion protection systems available upon request.
- · FHWA approved for use as a micro pile or soil nail (Domestic Hollow Injection Bar only)



B7X1 Domestic Hollow Injection Bar





Proposed Retrofit



HC Hardened Bit

Hardened cross cut drill bit, suitable for the majority of applications including narrow bands of soft rock. Soil Types: Fills and Medium Dense Gravels

B7XB Drill Bits



CC Carbide Bit

Tungsten carbide cross-cut drill bit. Excellent choice for majority of granular soils with mixed hard formations. Soil Types: Fills, Gravels, Shale & Seamy Rock Formations.



SB Sand/Clay Bit

Two stage cross cut drill bit, suitable for loose to medium dense ground and fills. Soil Types: Sand, Clay and Light Gravels



BB Button Bit

Tungsten carbide hemispherical button drill bit for moderately strong to strong rock, boulders and rubble. Rock Types: Mudstone, Limestone, and Granite

CB Cobble Bit

Offset face cross cut drill bit suitable for drilling in cobbles with silt and gravel as well as sedimentary bedrock material.

Nominal	Available Drill Bit Diameters						
Bar Diameter	HC	CC	SB	BB	CB		
32 mm (1-1/4*)	2* (51 mm)	2" (51 mm))i	2-1/2" (65 mm)			
	2-1/2" (65 mm)	2-1/2* (65 mm)		3* (76 mm)			
	3" (76 mm)	3* (76 mm)	5* (127 mm)	3-1/2" (89 mm)	4" (102 mm		
	3-1/2* (89 mm)	3-1/2" (89 mm)		4*			
	4" (102 mm)	4" (102 mm)		(102 mm)			
38 mm (1-1/2*)	2-1/2* (65 mm)	2-1/2" (65 mm)	5	2-1/2* (65 mm)			
	3" (75 mm)	3* (76 mm)	(127 mm)	3* (76 mm)	4* (102 mm		
	3-1/2"	. 3-1/2" . (89 mm)		3-1/2" (89 mm)			
	(mm 68)	4" (102 mm)	6" (152 mm)	4* (102 mm)			
	(102 mm)	4-1/2" (114 mm)		(127 mm)			
51 mm (2*)	10	(76 mm)	6* (152 mm)	3" (76 mm)			
		3-1/2" (90 mm)		3-1/2*	4-3/4" (121 mm)		
		4* (102 mm)	5 <u>1</u>	(mm 02)			
		4-1/2" (114 mm)	8" (203 mm)	4* (102 mm)			
		5* (127 mm)		5"	(152 mm)		
		6" (152 mm)		(127 mm)			
76 mm (3*) T40	12	5* (125 mm)	(175 mm)	(125 mm)			
		6" (152 mm)	10"	5"	-		
		7* (175 mm)	(254 mm)	(152 mm)			
	12	3* (76 mm)	6"	2-1/2* (65 mm)			
		4* (102 mm)	(152 mm)	3" (76 mm)	- 28		
T52		4" (102 mm)	7* (178 mm)	-	17		





<mark>Northwest</mark> MSE Wall

- Seven panels were repaired by installing two nails in each panel. Two weepholes were installed at each bottom panel, a total of 8 weepholes.
- The damaged and spalling concrete in the MSE wall and abutment area, caused by expansion was repaired by removing the damaged and unsound concrete, followed by cast-in-place concrete panel to restore the structural integrity of the wall and prevent the backfill loss.

Southwest MSE Wall

- Three panels were repaired by installing two nails in each panel. Two weepholes were installed at each bottom panel, a total of 4 weepholes.
- The damaged and spalling concrete in the MSE wall and abutment area, caused by expansion was repaired by removing the damaged and unsound concrete, followed by cast-in-place concrete panel to restore the structural integrity of the wall and prevent the backfill loss.
- The chipped coping was patched with concrete. It has been difficult to verify if pins or rebar



anchors were used to anchor the patched concrete. `

Items that have been Repaired



<mark>Southeast</mark> MSE Wall

• The damaged and spalling concrete in the MSE wall and abutment area, caused by expansion was repaired by removing the damaged and unsound concrete, followed by cast-in-place concrete panel to restore the structural integrity of the wall and prevent the backfill loss.

Northeast MSE Wall

Vegetation and trees grown between the coping and the bridge railing were trimmed.

East end bent:

• The joint between the phased walls has been sealed using a conventional sealant.

West end bent:

• The joint between the phased walls has been sealed using a conventional sealant.



Items that have been Repaired





Backfill behind Backfill behind bottom core top core **Coring for Soil Nails and Weep Holes**

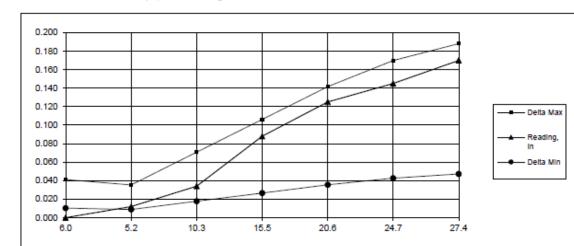




Soil Nail Proof Test Report

Location: In Client: IN	43196 82nd St over I-465 dianapolis DOT 5-111					
Tie Back #:	SW1-R	Equipment				
Date Installed:	3/4/2023	Pump:	PE554	Jack No.	S16993	
Date Tested:	3/8/2023	Gauge:	5405OVPJ	1 Kip=	83.77	PSI
Design Load (Kips	3) 20.6	Tested By:	ESB			
Bonded Length (ft) 17.5					
Unbonded length	(ft) 1	Boring Information				
Free Length (ft)	3	Diameter		Inches		
32mm Hollow Bar	0.776	Yards of cement		External		
Modulus of Elastic	ity (ksi) 28700	Cubic feet of grout		Internal		

Value of	Loading	Gauge	Movement	Delta, Min		Delta, Max	Ten Minute Hold	
Design Load	Kips	Pressure, PSI	Reading, In	Inches		Inches	Minutes	Reading, In
Alignment	6.0	503	0.000	0.010		0.041	1	0.170
0.25	5.2	431	0.012	0.009		0.035	2	0.170
0.50	10.3	863	0.034	0.018		0.071	3	0.170
0.75	15.5	1294	0.088	0.027		0.106	4	0.170
1.00	20.6	1726	0.125	0.036		0.142	5	0.170
1.20	24.7	2071	0.145	0.043		0.170	6	0.170
1.33	27.4	2295	0.170	0.047		0.188	10	0.170
							Creep =	0.000
Lock Off at 21 Kips, Dial Reading = 0.132							Result =	PASS

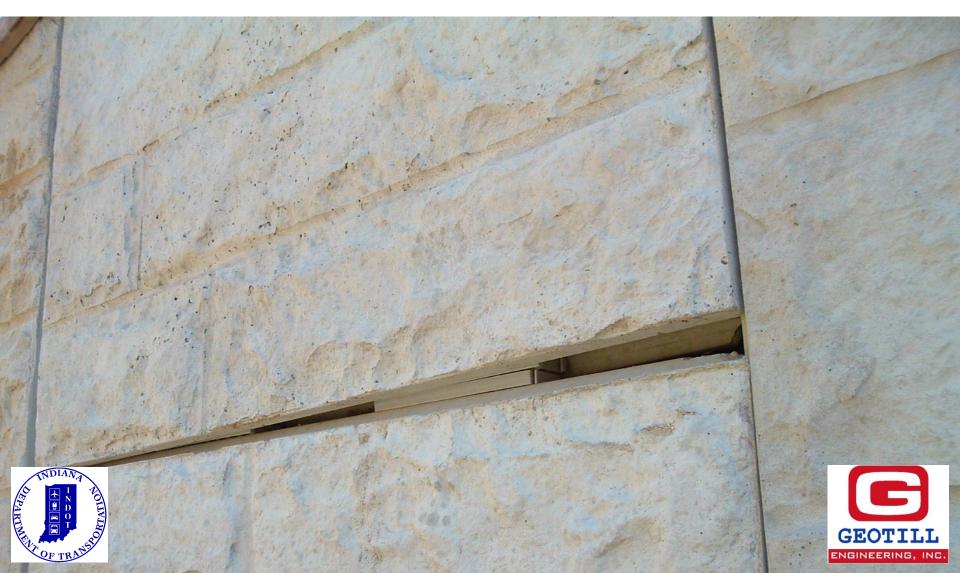








Settlement of Panels



Panels Damaged & Coping Tilting Due to Abutment Movement



Panels & Coping Tilting Due to Uncontrolled Drainag







Uncontrolled Drainage Weephole







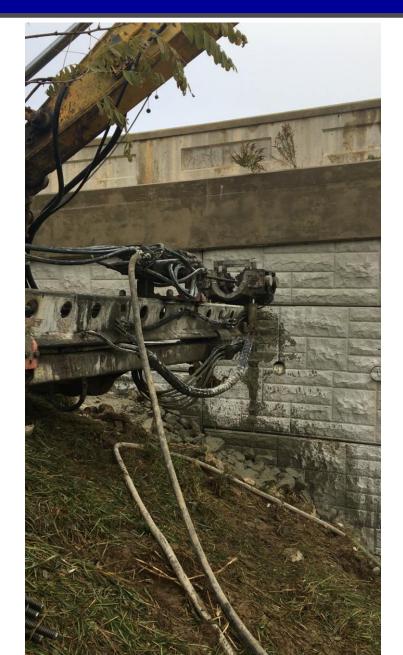
MSE Wall Repair with Soil Nail











MSE Wall Repair with Soil Nail





MSE Wall Repair with Soil Nail Soil Nail Pull Test





MSE Wall Repair Extending Curb to Control Drainage



And a second sec



MSE Wall Repair Sealing Separation at the Concrete Jersey Barrier to Control Drainage







When Should We Evaluate Retaining Wall Assets?

- During construction (QA/QC).
- Periodic (routine) performance inspections.
- Extreme events (seismic, flood, impact).
- Before widening, load changes, rehabilitation.
- As part of plan to extend useful life as part of asset management.







Questions?

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