

AN ABSTRACT OF THE THESIS OF

Jordan Michael Pelphrey for the degree of Master of Science in Civil Engineering presented on June 16, 2006.

Title: An Investigation of Oregon Weigh-In-Motion Data for Bridge Rating Implementation and Evaluation.

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The LRFR Manual, within commentary Article C6.4.4.2.3, contains provisions for development of site-specific live load factors. In Oregon, truck Weigh-in-Motion (WIM) data were used to develop live load factors for use on state-owned bridges. The factors were calibrated using the same statistical methods that were used in the original development of LRFR. This procedure maintains the nationally accepted structural reliability index for evaluation, even though the resulting state-specific live load factors were smaller than the national standard. The first part of this report describes the jurisdictional and enforcement characteristics in the state, the modifications used to describe the alongside truck population based on the unique truck permitting conditions in the state, the WIM data filtering, sorting, and quality

control, as well as the calibration process, and the computed live load factors. Large WIM data sets from four sites were used in the calibration and included different truck volumes, seasonal and directional variations, and WIM data collection windows. Finally, policy implementation for actual use of the factors and future provisions for maintenance of the factors are described.

For bridge rating and evaluation, notional truck models are commonly used to simulate the load effects produced by the truck population. The recently developed Load Resistance and Factor Rating (LRFR) Bridge Evaluation Manual was calibrated based on the 3S2 truck configuration as the notional model. Using GVW as the parameter for establishing live load factors to reflect load effects may not necessarily provide consistent outcomes across all bridge span lengths, indeterminacies, or specific load effects. This is because the load effects are dependent on the distributions of the axle weights, the axle spacing, and the number of axles, in addition to the span geometry and support conditions.

The Oregon Department of Transportation currently uses a suite of 13 rating vehicles for evaluation of their bridge inventory. The load effects for Oregon's bridge rating vehicles have also been calculated for various span lengths and support conditions in the second part of this report. These load effects, both unfactored and factored, were compared with load effects calculated using vehicles from large sets of WIM data. Further, because no established standard of time or

quantity of WIM data has previously been recognized, a separate study was conducted in order to determine an acceptable window of WIM data. The objective of this analysis was to determine if the load effects and the live load factors developed for bridge rating produced by the suite of vehicles envelope load effects produced by an acceptable window of collected vehicle data for a variety of bridge span lengths and types. Observations and suggestions are made based on the results of these analyses.

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An Investigation of Oregon Weigh-In-Motion Data for Bridge Rating
Implementation and Evaluation

by
Jordan Michael Pelphrey

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Dr. Christopher Higgins assisted with the data interpretation and writing of Chapters 2 and 3. Bala Sivakumar assisted with the data implementation and writing of Chapter 2. The Oregon Department of Transportation assisted with the writing of Chapter 2.

TABLE OF CONTENTS

| | <u>Page</u> |
|---|-------------|
| CHAPTER 1: General Introduction | 1 |
| CHAPTER 2: Site-Specific LRFR Live Load Factors Using Weigh-In-Motion | |
| Data | 2 |
| Abstract | 3 |
| CE Database Subject Headings | 4 |
| Introduction and Background | 4 |
| Live Load Factor Methodology and Analysis | 5 |
| WIM Data Collection Windows | 8 |
| Traffic Volume | 8 |
| Seasonal Variation | 8 |
| WIM Data Cleaning, Filtering, and Weight Table Sorting | |
| Methodology | 9 |
| Live Load Factor Results | 16 |
| Significant Findings from Calibration Process | 17 |
| Discussion of Results | 20 |
| Policy Implementation | 21 |
| Conclusions | 24 |
| References | 39 |
| Appendix A | 40 |

TABLE OF CONTENTS (Continued)

| | <u>Page</u> |
|---|-------------|
| CHAPTER 3: Evaluation of Bridge Rating Vehicles Using Weigh-In-Motion | |
| Data | 44 |
| Abstract | 45 |
| CE Database Subject Headings | 46 |
| Introduction | 46 |
| Background | 48 |
| WIM Data..... | 49 |
| Cleaning and Filtering the WIM Data..... | 50 |
| WIM Site Selection | 51 |
| Weight Table Sorting Methodology..... | 53 |
| Conventional Sort | 53 |
| Modified Sort | 54 |
| Selecting an Appropriate Window of Data..... | 56 |
| Calculation of Load Effects | 59 |
| Oregon-Specific Live Load Factors | 62 |
| Tabular and Graphical Results | 64 |
| Significant Findings | 64 |
| Unfactored Load Effects | 65 |
| Factored Load Effects | 65 |

TABLE OF CONTENTS (Continued)

| | <u>Page</u> |
|---------------------------------------|-------------|
| Rating Vehicle Summary | 66 |
| HL-93 Loading..... | 68 |
| Notional Rating Load Vehicles..... | 68 |
| Conclusions and Recommendations | 69 |
| References..... | 96 |
| Appendix A..... | 98 |
| Appendix B | 112 |
| Appendix C | 128 |
| Appendix D..... | 136 |
| CHAPTER 4: General Conclusions..... | 157 |
| BIBLIOGRAPHY | 161 |

LIST OF FIGURES

| <u>Figure</u> | <u>Page</u> |
|---|-------------|
| 2.1 Maximum loading event for calibration of live load factors..... | 26 |
| 2.2 Map of Oregon WIM sites used in the study | 27 |
| 2.3 Live load factors for WIM sites at different seasons and different WIM data windows..... | 28 |
| 3.1 Example of raw WIM output | 73 |
| 3.2 Location of I-5 Booth Ranch NB..... | 73 |
| 3.3 Frequency histogram of the number of axles per vehicle | 74 |
| 3.4 Projection plots for various time windows for I-5 Booth Ranch NB..... | 75 |
| 3.5 Projection plots for various time windows for US 97 Klamath Falls NB..... | 76 |
| 3.6 Projected GVW values for all time windows for I-5 Booth Ranch NB..... | 77 |
| 3.7 Projected GVW values for all time windows for US 97 Klamath Falls NB | 78 |
| 3.8 Shear & moment locations for simple span analysis | 79 |
| 3.9 Shear & moment locations for 2-span continuous bridge analysis | 80 |
| 3.10 CDF plots for unfactored moment for 100-ft simple span bridge model..... | 81 |
| 3.11 CDF plots for factored moment for 100-ft simple span bridge model..... | 82 |

LIST OF TABLES

| <u>Table</u> | <u>Page</u> |
|---|-------------|
| 2.1 ODOT rating vehicle classifications | 29 |
| 2.2 Selected WIM sites, locations, and ADTT..... | 30 |
| 2.3 Results of sorting methods for Weight Table classification | 31 |
| 2.4 Statistics from controlling WIM data sets used in live load factor calibration..... | 32 |
| 2.5 Computed Oregon-specific live load factors for legal loads and LRFR Table 6-5 values | 33 |
| 2.6 Computed Oregon-specific live load factors for permit loads and upper portion of LRFR Table 6-6 values | 34 |
| 2.7 Directional influence for live load factors at the I-5 Woodburn NB and SB sites for January, 2005..... | 35 |
| 2.8 Sensitivity analysis for alongside vehicle variability for select rating vehicles during summer season (2 Weeks - 1st - 14th)..... | 36 |
| 2.9 ODOT Adaptation of LRFR Table 6-5 Generalized Live-Load Factors for Legal Loads: γ_L | 37 |
| 2.10 ODOT adaptation of upper portion of LRFR Table 6-6 for ODOT Routine Permits..... | 38 |
| 3.1 Current ODOT rating vehicles | 83 |
| 3.2 Information for I-5 Booth Ranch NB..... | 84 |
| 3.3 Number of axles per vehicle per month | 85 |

LIST OF TABLES (Continued)

| <u>Table</u> | <u>Page</u> |
|--|-------------|
| 3.4 Comparing sorting methods for table classification at I-5 Booth Ranch NB for 2005 | 86 |
| 3.5 Maximum projected GVW for varying time windows for I-5 Booth Ranch NB | 87 |
| 3.6 Statistical parameters for varying time windows for I-5 Booth Ranch NB | 88 |
| 3.7 Maximum projected GVW for varying time windows for US 97 Klamath Falls NB..... | 89 |
| 3.8 Statistical parameters for varying time windows for US 97 Klamath Falls NB | 90 |
| 3.9 Five-year extrapolated load effects for various span types and lengths..... | 91 |
| 3.10 Comparison of shear effects between rating and WIM vehicles..... | 92 |
| 3.11 Comparison of moment effects between rating and WIM vehicles..... | 93 |
| 3.12 Comparison of 2-span continuous load effects between rating and WIM vehicles..... | 94 |
| 3.13 Factored rating vehicle sufficiency | 95 |

LIST OF APPENDIX FIGURES

| <u>Figure</u> | <u>Page</u> |
|---|-------------|
| A3.1 ODOT legal rating vehicles | 99 |
| A3.2 ODOT continuous trip permit rating vehicles..... | 100 |
| A3.3 ODOT single trip permit rating vehicles..... | 101 |
| A3.4 ODOT load rating vehicle descriptions..... | 102 |
| A3.5 AASHTO notional rating loads (specialized hauling vehicles) | 103 |
| A3.6 Permit Weight Table 1 [Oregon Motor Carrier] | 104 |
| A3.7 Permit Weight Table 2 [Oregon Motor Carrier] | 105 |
| A3.8 Permit Weight Table 3 [Oregon Motor Carrier] | 106 |
| A3.9 Permit Weight Table 4 [Oregon Motor Carrier] | 108 |
| A3.10 Permit Weight Table 5 [Oregon Motor Carrier] | 110 |
| B3.1 Sample error file showing the end of the record produced by the Liger program | 113 |
| C3.1 Oregon WIM site data and locations..... | 129 |
| D3.1 CDF plots for unfactored shear for 50-ft simple span bridge model | 137 |
| D3.2 CDF plots for factored shear for 50-ft simple span bridge model | 138 |
| D3.3 CDF plots for unfactored shear for 100-ft simple span bridge model | 139 |
| D3.4 CDF plots for factored shear for 100-ft simple span bridge model | 140 |
| D3.5 CDF plots for unfactored shear for 150-ft simple span bridge model | 141 |
| D3.6 CDF plots for factored shear for 150-ft simple span bridge model | 142 |
| D3.7 CDF plots for unfactored shear for 200-ft simple span bridge model | 143 |

LIST OF APPENDIX FIGURES (Continued)

| <u>Figure</u> | <u>Page</u> |
|---|-------------|
| D3.8 CDF plots for factored shear for 200-ft simple span bridge model | 144 |
| D3.9 CDF plots for unfactored moment for 50-ft simple span bridge model..... | 145 |
| D3.10 CDF plots for factored moment for 50-ft simple span bridge model..... | 146 |
| D3.11 CDF plots for unfactored moment for 100-ft simple span bridge model... | 147 |
| D3.12 CDF plots for factored moment for 100-ft simple span bridge model..... | 148 |
| D3.13 CDF plots for unfactored moment for 150-ft simple span bridge model... | 149 |
| D3.14 CDF plots for factored moment for 150-ft simple span bridge model..... | 150 |
| D3.15 CDF plots for unfactored moment for 200-ft simple span bridge model... | 151 |
| D3.16 CDF plots for factored moment for 200-ft simple span bridge model..... | 152 |
| D3.17 CDF plots for unfactored shear for 50-ft 2-span continuous bridge model..... | 153 |
| D3.18 CDF plots for factored shear for 50-ft 2-span continuous bridge model ... | 154 |
| D3.19 CDF plots for unfactored negative moment for 50-ft 2-span continuous bridge model..... | 155 |
| D3.20 CDF plots for factored negative moment for 50-ft 2-span continuous bridge model..... | 156 |

LIST OF APPENDIX TABLES

| <u>Table</u> | <u>Page</u> |
|--|-------------|
| A2.1 Statistics for I-5 Woodburn NB, June 2005 (2 weeks, 1st - 14th) | 41 |

General Introduction

Oregon allows vehicle loads and configurations on the state highways that are different from many other states. Many of the vehicles are above the federal legal weight limit but are allowed on the highways under permits. Oregon bridge design and rating have relied on national models that are based on data collected in other countries and states. This data may not accurately reflect the loads found in Oregon.

The Oregon Department of Transportation (ODOT) collects data on vehicle weight and axle spacing lengths at WIM scale locations throughout the state. Using this data, analyses were performed to:

- Calculate Oregon-specific live load factors for rating following the methodology in the Manual for Condition Evaluation and Load and Resistance Factor Rating (LRFR) of Highway Bridges;
- Establish an appropriate window of WIM data by extrapolation;
- Evaluate the current ODOT bridge rating vehicles;
- Evaluate the Motor Carrier Transportation Division (MCTD) weight tables;
- Evaluate Oregon's permit classifications.

State-Specific LRFR Live Load Factors Using Weigh-in-Motion Data

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State-Specific LRFR Live Load Factors Using Weigh-in-Motion Data

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Abstract

The LRFR Manual, within commentary Article C6.4.4.2.3, contains provisions for development of site-specific live load factors. In Oregon, truck Weigh-in-Motion (WIM) data were used to develop live load factors for use on state-owned bridges. The factors were calibrated using the same statistical methods that were used in the original development of LRFR. This procedure maintains the nationally accepted structural reliability index for evaluation, even though the resulting state-specific live load factors were smaller than the national standard. This paper describes the jurisdictional and enforcement characteristics in the state, the modifications used to described the alongside truck population based on the unique truck permitting conditions in the state, the WIM data filtering, sorting, and quality control, as well as the calibration process, and the computed live load factors. Large WIM data sets from four sites were used in the calibration and included different truck volumes,

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seasonal and directional variations, and WIM data collection windows. Finally policy implementation for actual use of the factors and future provisions for maintenance of the factors are described.

CE Database subject headings: load factors, bridges, bridge loads, ratings

Introduction and Background

Transportation agencies are beginning to transition from the American Association of State Highway Officials (AASHTO) Manual for Condition Evaluation of Bridges (1994) to the AASHTO Load and Resistance Factored Rating (LRFR) Specifications (2003) for bridge rating and evaluation. The LRFR Specifications extend the limit states design philosophy from AASHTO Load and Resistance Factor Design (LRFD) (2004) to evaluation of existing bridges. Employing structural reliability principles, the Specifications provide the flexibility to provide uniform target safety levels by reducing uncertainty (Minervino *et al.* 2004) and further provide a means of incorporating advancements in analysis methods, load models, and material and member characterization in the evaluation process. For evaluation of existing bridges, site-specific information can be collected to characterize the local uncertainty, rather than relying on generalized information. One area where it is possible to reduce uncertainty is in the live loads through collection and characterization of site-specific traffic data. The generalized load factors given in the LRFR Specifications are representative of bridges nationwide

with similar traffic volumes. The LRFR Specifications provide procedures for calculating site-specific load factors using truck weight data collected from weigh-in-motion (WIM) sites that follows the same format used in the derivation of LRFD live load factors. Site-specific load factors are more refined because they are characteristic of a particular bridge site, route, or jurisdiction and reflect the actual truck traffic and likely maximum loadings over the exposure period.

Following the methodology developed in NCHRP Project No. 12-46 (Moses 2001) and incorporated in the LRFR Specifications, live load factors for strength evaluation were developed for state-owned bridges in Oregon using WIM data from sites across the state. Adaptation of the methods was necessary to account for unique characteristics of truck loads and permitting regulations in the state. Live load factors were developed using WIM data from four sites, including state and interstate routes, considering possible seasonal variations, and different WIM data collection windows. This paper describes the analysis methods used to determine the site-specific live load factors based on WIM data, the resulting live load factors, policy implementation, and plans for updating factors in the future.

Live Load Factor Methodology and Analysis

The LRFR Manual provides a procedure for calculating site-specific load factors using truck weight data from WIM sites that follow the format used in the derivation of live load factors contained in the LRFD Specifications. The LRFR

approach is to determine the statistics associated with the 3S2 truck population to characterize the uncertainty associated with the alongside truck. The Ontario truck weight data used in calibration of the LRFR specifications were reasonably matched by a 3S2 truck with a normal distribution and a mean of 68 kips and standard deviation of 18 kips. The weight parameters fit the heaviest one-fifth of the truck weight population and it was assumed that the remaining trucks have no influence on the maximum loading events. The maximum loading event for calibration assumes a legal truck or a permit truck in one lane and a random truck (referred to as the alongside vehicle) in the adjoining lane as illustrated in Fig. 1. Therefore, the load factor applied to the permit vehicle depends on the random alongside truck. Live load factors are higher for spans with higher average daily truck traffic (ADTT) and smaller for heavier permitted vehicles. Live load factors for permit loads are smaller compared with legal load rating values to account for the reduced probability of simultaneous crossing events and also reduced likelihood that a permit truck will be significantly overloaded.

In the LRFD calibration, Nowak (1999) showed that the maximum expected lifetime loading in each lane for two-lane loading is 0.85 times the single lane expected maximum lifetime loading. Therefore, in a two-lane loading situation, the extreme occasional overloads that may be present within the various truck categories are not influential in the calibration of live load factors. This also suggests that data for long periods of time to identify such loads would not be very

beneficial for calibration purposes. The key to reliable calibration statistics is the quality and not necessarily the quantity of data.

Significant differences in permitting requirements exist in the State of Oregon, compared to other jurisdictions as illustrated in Table 1. These include a higher legal gross vehicle weight (GVW) of 80,000 lbs compared to the national level of 72,000 lbs, large numbers of CTP vehicles, and extended legal weight CTP vehicles to 105,500 lbs on state highways. As a result, the 3S2 truck population statistics alone may not necessarily characterize the alongside truck variability. Therefore, the alongside truck population in Oregon was taken as consisting of legal trucks (Weight Table 1), Extended Weight Table 2 (105,500 lbs maximum) and 98,000-lb CTP vehicles from Weight Table 3. Inclusion of permitted trucks (the CTPs) in the along-side truck population is a conservative departure from past load factor calibration work, but characteristic of the jurisdiction.

WIM data were used to develop the state-specific live load factors based on the characteristic vehicle population in the state. Three major variables were considered in the selection of WIM data. These included length of the WIM data collection window, truck volume, and seasonal variability. Each is described in additional detail below.

WIM Data Collection Windows

Typically, in practice, two-weeks of WIM data are used to compute site specific live load factors; however no established standard of time or quantity of WIM data has previously been established. To assess the effect of different WIM data collection windows on the corresponding live load factors, three different windows of time were considered in each month: 1) data from the entire month, 2) 2 weeks of data from 1st – 14th, and 3) 2 weeks of data from 15th – 28th. Comparisons were made between each of the two-week data windows and further compared with the all-month data windows.

Traffic Volume

There are four highways/interstates of interest in Oregon that collect WIM data. These are Interstate-5, Interstate-84, Oregon State Highway-58, and US Highway-97. From these highways, individual WIM data collection sites were selected based on ADTT volume. The WIM sites chosen are shown in Table 2 and Fig. 2. These sites enabled calculation of live load factors considering different truck volume conditions.

Seasonal Variation

To assess possible variations in the data occurring at different periods of the year, four “seasons” were selected for each WIM site. WIM sites are intended to collect a continuous record of data for vehicles crossing the WIM scales. However, due to

local conditions such as roadway construction or hardware or electronics problems, data were not always continuous over an entire month. Therefore, the months selected for analysis were chosen based on availability of complete months of data within each “season.” These included: November through January for winter, April for spring, May and June for summer, and October for fall. Some months strayed outside of traditional “seasonal” boundaries, but only when necessary due to noncontinuous data sets. Table 2 lists the specific months from which WIM data were available for each of the sites. Site specific live load factors were computed for each of these timeframes. Data collection for Bend NB did not begin until June, 2005. Therefore, live load factors could not be calculated for spring, but these will be computed as data become available.

WIM data cleaning, filtering, and Weight Table Sorting Methodology

The raw WIM records from each collection site were provided in text format for subsequent data processing. The data were cleaned and filtered to remove records with formatting mistakes, spurious data, and other errors. Error types that were removed in the cleaning process were:

1. Record where the GVW value is equal to 0.0.
2. Record does not follow the general record pattern; this could be any inconsistency in the time stamp, words out of place from the status quo, incomplete records, etc.
3. Records with misplaced characters, such as a letter where a number should be or a number where a letter should be.
4. Record where an individual axle is greater than 50 kips.
5. Record where the speed is less than 10 mph.
6. Record where the speed is greater than 99 mph.

7. Record where the length is greater than 200 ft.
8. Record where the sum of the axle spacing lengths are greater than the length of the truck.
9. Record where the sum of the axle spacing lengths are less than 7 ft.
10. Record where the first axle spacing is less than 5 ft.
11. Record where the # of axles is greater than 13.
12. Record where the GVW is greater than 280 kips.
13. Record where any axle spacing is less than 3.4 ft.
14. Record which has a GVW +/- the sum of the axle weights by more than 7%.
15. Record which has a GVW less than 2.0 kips

Classifying and sorting the WIM data into the appropriate permit weight table classification is a key step in developing site live load factors. Data processing should remove permitted trucks from the WIM data representing the alongside truck population. Two separate sorting methods for the WIM data were investigated and compared. These are defined as “Conventional Sort” and “Modified Sort.”

The Conventional Sort method sorts vehicles based on their GVW, axle group weights, and length (GVW + Axle Group Sort). It is the method currently used by the Motor Carrier Transportation Division (MCTD) of ODOT to classify vehicles into Weight Table 1, Weight Table 2, Weight Table 3, Weight Table 4, Weight Table 5, or Table X (the overflow table classification). Permits are issued based on a vehicle’s Weight Table classification. This method accounts for the axle weights and spacing in assigning each vehicle to an appropriate Weight Table and assigns more vehicles to higher Weight Tables than the Modified Sort (described subsequently). Proportionately more heavy vehicles that could have been

interpreted as “rogue” legal vehicles are assigned to Weight Table 3 and above and are thus considered as legitimate permit vehicles. The sort yields lower coefficients of variation and as seen subsequently yields lower live load factors compared to the Modified Sort. While it is less conservative than the Modified Sort, it is thought to better represent the permitted truck population in Oregon as will be discussed later.

The Modified Sort method sorts vehicles based only on their GVW and rear-to-steer axle length, and it does not account for axle groupings (GVW + Truck Length Sort). The method assigns more vehicles to lower Weight Tables than the Conventional Sort. Proportionately more heavy vehicles that could have been interpreted as legitimate permit vehicles are conservatively assigned to Weight Tables 1 and 2 and are thus considered “rogue” legal vehicles. The sort produces higher coefficients of variation and higher live load factors compared to the Conventional Sort. While it is more conservative, it may unfairly penalize Oregon’s regulatory and enforcement policies, than the Conventional Sort.

Oregon has a well established permitting process that contributes to reduced overloads on state highways. These include minimal cost of overweight permits, large numbers of such permits authorized, the ease of access in obtaining them (such as through the Internet), a weight-mile tax that results in lower taxes for loads placed on more axles, development and fostering of the “Trusted Carrier” program which enhances cooperation and load compliance by trucking companies, and the

significant enforcement and cost of penalties imposed on vehicles and drivers that are non-compliant. The compliance to weight limits for trucks in Oregon was verified in a study by Strathman and Theisen (2002) that demonstrated there was no statistically significant evidence of overweight truck scale avoidance. Further, there are few detour routes available to skirt scales on the major state highways.

The two different sorting methods were used on the WIM data sets and results are shown in Table 3 for the Weight Table breakdown. The live load factors herein were calculated based on the Conventional Sort method because it better represents the regulatory and enforcement procedures in Oregon. In contrast to some other states where truckers generally know the vehicle GVW but may not know their axle grouping weights, MCTD of ODOT report that Oregon truckers are generally aware of their axle and tandem weights, usually to within 2,000 lbs, which proves beneficial in obtaining a continuous trip permit.

After careful quality control measures and independent checks were performed on the WIM data cleaning, filtering, and sorting routines, statistics were generated based on GVW for the rating truck and the alongside truck using only the top 20% of the truck weight data from each category. This was consistent with the projection of the upper tail of the weight histogram (Nowak 1999; LRFR 2003). Statistical parameters were calculated for the alongside truck population from Weight Tables 1, 2 and CTPs from Weight Table 3. Additionally, statistical

parameters were calculated for just the 3S2 truck population. The statistical parameters are reported in Table 4 for the controlling data sets. Using these statistical values, live load factors were determined for each of the ODOT rating vehicles for the different WIM sites, data windows, and seasons.

The LRFR live load factor for rating is given in Equation 39 of NCHRP Report 454, as:

$$\gamma_L = 1.8 \frac{W_T}{240} \times \frac{72}{W} \quad [1]$$

where W is the gross weight of vehicle (legal truck or permit truck with units of kips) and W_T is the expected maximum total weight of rating and alongside vehicles, computed as:

$$W_T = R_T + A_T \quad [2]$$

where, R_T is the rating truck and is computed for legal loads as:

$$R_T = W^* + t_{ADTT} \sigma_{3S2}^* \quad [3a]$$

or for permit loads as:

$$R_T = P + t_{ADTT} \sigma_{along}^* \quad [3b]$$

where W^* is the mean value of the top 20% of legal trucks taken from the 3S2 population, σ_{3S2}^* is the standard deviation of the top 20% of legal trucks, P is the weight of permit truck, σ_{along}^* is the standard deviation of the top 20% of the alongside trucks. The alongside truck, A_T , is computed as :

$$A_T = W_{along}^* + t_{ADTT} \sigma_{along}^* \quad [4]$$

where W_{along}^* is the mean of the top 20% of alongside trucks (taken from Weight Tables 1 and 2, as well as CTPs from Weight Table 3 for the Oregon data). In the above expressions, t_{ADTT} is the fractile value corresponding to the number of side-by-side events, N . The number of side-by-side crossings is computed as:

$$N (\text{legals}) = (\text{ADTT}) \times (365 \text{ days/yr}) \times (\text{Evaluation period}) \times (P_{s/s}) \times (\% \text{ of record}) \quad [5a]$$

$$N (\text{permits}) = (N_p) \times (365 \text{ days/yr}) \times (\text{Evaluation period}) \times (P_{s/s}) \quad [5b]$$

for legal trucks and permit trucks, respectively, where N_p is the number of observed STP in the WIM data extrapolated over the evaluation period and $P_{s/s}$ is the probability of side-by-side concurrence. LRFD and LRFR calibrations assumed a 1/15 (6.7%) probability of side-by-side events for truck passages. This assumption was based on visual observations and is conservative for most sites. Recent WIM studies completed under NCHRP 12-63 indicate much lower multiple-presence probabilities even for very high ADTT sites. In the NCHRP study, very accurate time stamps were collected and analyzed for WIM sites on I-84 in Idaho and I-75 in Ohio to estimate the number side-by-side events over several days in 2004 and 2005. Results showed maximum side-by-side probability of 3.35% for a three-lane site with >5000 ADTT (Ohio) and 1.37% for a two-lane site with >2500 ADTT (Idaho). These calculated probabilities considered all trucks within a headway separation of 60 feet to constitute a side-by-side event. This larger and more conservative definition of headway separation may produce a higher multiple presence but may have a lower total moment on most spans. The I-5 site in the current study is comparable to the three-lane >5000 ADTT site reported above. For

the calibration purposes, a 1/30 (3.4%) probability of side-by-side events was adopted as being more representative of likely concurrence for the sites in Oregon.

The ADTT values specific to each site were used in calculating the t_{ADTT} statistic and were listed previously in Table 2. The number of permits per day used in calculating the t_{ADTT} statistic was derived from the Conventional Sort method as shown in Table 3. Once the data were sorted according to the ODOT table classification, the number of Weight Table 3 CTP vehicles with 5 axles and GVW less than 99 kips were removed and placed into Weight Table 2, thereby including them as part of the routine traffic stream. The number of permits was then calculated by summing the remaining trucks in Weight Table 3 as well as those in Weight Tables 4, 5, and X, and then dividing by the number of days in the WIM record. This represents the average number of STP vehicles passing the WIM site each day.

Considering a 5 year evaluation period for which the bridge rating would be considered valid the LRFR live load factors were computed for the various sites and an example calculation procedure is shown in the Appendix. The state-specific load factors represent a target beta level corresponding to the Operating level of 2.5.

Live Load Factor Results

The computed live load factors for all sites, for all seasons, and for all ODOT rating vehicles are shown in Fig. 3. The data used for this calibration process included over 930,000 individual WIM records spanning over 4 months of the year and represents significantly more data than was used in the original calibration of the national specifications.

The computed live load factors are intended to replace Table 6-5 and Table 6-6 (upper portion) in the LRFR manual with the Oregon-specific values based on the actual population of trucks on the state highways. Live load factors for ADTT greater than 5000 correspond to the Woodburn NB (I-5) site. Live load factors for ADTT equal to 1500 correspond to the Emigrant Hill WB (I-84) site. Live load factors for ADTT less than 500 correspond to the Lowell WB (OR58) and Bend NB (US97) sites. For each rating vehicle and represented truck traffic volume level, the live-load factors were conservatively chosen as the upper bound of all the factors from each of the four seasons and each of three data sampling periods. These selected live load factors are lower than the values found in the LRFR manual as shown in Tables 5 and 6. ODOT's MCTD issues STPs in large numbers on a routine basis without specific structural review and as a result, they are treated the same as "Routine or Annual" in Table 6 (upper portion of LRFR Table 6-6). Several of the controlling live load factors were shared by more than one season and/or time-frame and illustrates the degree of consistency between data sets over

the period considered. Full data sets, statistics, and details are reported by Pelphrey and Higgins (2006).

Significant Findings from Calibration Process

Significant findings based on results of this calibration process are presented below. These include information on seasonal, directional, and traffic-volume variations between sites, interstate versus non-interstate traffic, and WIM data collection windows.

The variation of live load factors for the different seasons at all four sites can be seen in Fig. 3. I-5 Woodburn NB and US97 Bend NB show very little change from season to season, while OR58 Lowell WB and I-84 Emigrant Hill WB show a slight variation between select seasons. The greatest variation for OR58 is for the Oregon Legal Load (2 Weeks, 1st – 14th) from a Summer live load factor of 1.12 to a Fall live load factor of 1.25 (12% change). The greatest variation for I-84 is for the STP-4A (2 Weeks, 15th – 28th) from a Fall live load factor of 1.18 to a Summer live load factor of 1.32 (13% change). Some of these seasonal variations are attributed to movement of construction equipment and agricultural products in the summer and fall.

To investigate if there were directional influences in the calibrated factors, another site – Woodburn SB for January 2005 – was investigated and compared to its

counterpart, Woodburn NB. The live load factors for Woodburn NB and SB in each WIM data window during January, 2005 are shown in Table 7. The results show that the computed live load factors were not sensitive to the direction of travel.

Interstate traffic produced higher ADTT values, which in turn produced higher live load factors. This follows the national trend of higher live load factors for higher ADTT values. Calibration of the live load factors for different ADTT volume sites across the state permits them to be used statewide for both interstate and non-interstate routes on state-owned bridges.

Live load factors were calculated for three different windows of time in each month: 1) All month, 2) 2 weeks – 1st – 14th, and 3) 2 weeks – 15th – 28th. This was done to determine if results would change significantly if more WIM data were used to develop the factors. As shown in Fig. 3, there was little difference between the WIM data collection windows. This would suggest that reasonable characterization of the WIM sites (even the lower ADTT volume sites) could be made from any two continuous weeks of data within the month of interest. Here, again it is important to note that high quality data is required and not only a large quantity of data.

A sensitivity analysis was performed to determine how changes in the mean and standard deviation values of the alongside vehicles (Weight Tables 1 and 2, and CTP's < 99 kips from Weight Table 3) affect the live load factors. All four sites

were investigated for the summer season using the first two weeks of data (1st – 14th). The analysis determined the magnitude of change required in the alongside vehicle mean and standard deviation to result in the live load factor increasing by 0.05. The two statistical parameters were assessed independent of each other (first, changing only the mean for a live load factor change of 0.05, and then changing only the standard deviation for a live load factor change of 0.05). The results of this analysis are shown in Table 8. As seen in this table, the mean would have to change by about 10% for all sites, and the standard deviation by about 15% on the interstates, and approximately 25% on the state highways.

A sensitivity analysis was also performed for the statistics on the 3S2 population. The live load factor for legal vehicles is the only factor affected by these statistics. Results from this analysis were similar to that observed for the alongside vehicle population, except that the standard deviation would have to be more than twice as large as that for the alongside population. Increasing mean GVW indicates a shift in truck weights while an increase in standard deviation indicates higher dispersion in the data. Changes in these parameters may be caused by changes in policy, compliance, or enforcement, and would indicate a need to recalibrate the load factors.

Discussion of Results

As described earlier, calibration of the LRFR specifications was performed using Ontario vehicle weight data of 1975 which were reasonably described by a 3S2 truck with a normal distribution and a mean of 68 kips and standard deviation of 18 kips for the top 20% of the truck weight population. The corresponding parameters for the Oregon weight data, calibrated using large WIM data sets, had higher mean but reduced standard deviations for the alongside truck population at each of the sites. The parameters indicate that there were significantly more overloads in the Ontario random truck data than are present in the Oregon legal loads or in the truck population grouped as the alongside truck. The maximum loading event for the LRFR calibration of load factors was controlled by the overloaded random trucks. It was shown that even when a permit truck of known weight up to 125 kips crosses the bridge, the expected maximum loading is lower compared with the maximum random legal loading event due to the many overloads in the random traffic (Moses 2001). That is, most routine permits do not affect the critical loading, which was governed by the non-permit overloads. The reduced overloads in the Oregon data explain the reduced site-specific load factors. For example, the LRFR live load factor for legal loads is 1.80 for $ADTT \geq 5000$, while the Oregon-specific value is 1.40. Similar reductions in live load factors were seen for lower ADTT ranges, as well as for permit vehicles (Oregon's CTP and STP vehicles). These results are the outcome of the regulatory and enforcement environment in Oregon. The permit issuance and regulatory environment encourages the routine operation at above-

legal load levels by means of low-cost continuous trip permits, and inhibit the operation of heavily overloaded “legal” vehicles within the traffic stream. The major factors affecting this condition include low cost and ease of obtaining permits, a weight-mile tax system that encourages loads spread onto more axles, development of the “Trusted Carrier” program that enhances cooperation and load compliance by trucking companies, and significant enforcement and hefty penalties for non-compliance. Previous research showed no statistically significant evidence of overweight truck scale avoidance (Strathman and Theisen 2002). The ability to minimize uncertainties in the truck population through the effective means described above have the effect of reducing the live load factors.

Policy Implementation

The ODOT Bridge Engineering Section plans to implement the AASHTO LRFR Specifications for rating and evaluation of state-owned bridges. The agency expects this implementation will preserve the safety of the traveling public in Oregon and to the greatest extent possible, facilitate the unrestricted movement of freight on Oregon’s highways. These stated purposes are best served by assessing the load carrying capacity of Oregon’s bridges as accurately as possible, to avoid the unnecessary restriction of freight movements while maintaining the nationally accepted reliability index. The large and diverse WIM data sets used in the live load factor calibration process produced consistent results and allowed establishment of Oregon specific versions of Tables 6-5 and 6-6 in the LRFR

Manual. The results are applicable only to bridges on Oregon's state-owned highway system and provide an operational rating condition corresponding to a reliability index of 2.5. Live load factors from the I-5 Woodburn Northbound site (ADTT of 5500) were taken to represent $ADTT \geq 5000$, and factors from the I-84 Emigrant Hill site (ADTT of 1786) were taken to represent $ADTT = 1500$. The worst case of the factors from the sites on OR 58 at Lowell (ADTT of 581) and US 97 at Bend (ADTT of 607) was taken as representative of $ADTT \leq 500$.

The calibrated live load factors described previously were adjusted for use in the ODOT policy implementation. It is recognized that calibrated live load factors in LRFR are merely statistical adjustments to the loads effects to maintain a uniform level of structural reliability and are not traditional amplification load factors, as were used to provide a margin of safety in the AASHTO Standard Design Specifications (2004). However, to assure additional conservatism where the calibration process resulted in very low live load factors, a minimum value of 1.0 was used. Additionally, the statistical calibration process used to compute the live load factors does not provide precision to the 100th decimal place. Therefore, rounding was applied to the live load factors, generally to the next higher 0.05 increment. The final tables for use in Oregon are shown in Tables 9 and 10.

To investigate possible changes in the truck population in the state, at three year intervals starting in 2008 until 2011 and every five years thereafter, ODOT will

review the calibration process using two-week windows of WIM data for each of the same four sites for each season, or will follow nationally accepted protocols that may emerge. If the mean or standard deviation values change enough to cause any live load factor to change by 0.05 or greater, based on the sensitivity analysis study, the Federal Highway Administration will be notified and a complete recalibration of the live load factors will be performed. This is a much more stringent standard of calibration data currency than has been applied to the calibration in the LRFR Manual. In addition to these scheduled reviews, the Oregon-specific live load factors will be reviewed any time a significant statutory or administrative rule change occurs in the vehicle permit regulatory structure (how permits are issued and the fine structure for ticketed overloads) or if a significant change occurs in overweight vehicle enforcement procedures.

In the event that a future review or regulatory change triggers a decision to recalibrate the Oregon-specific live load factors, the calibration procedure will be repeated as described in the above methodology, or in accordance with any nationally accepted protocols that may have been established. The revised Oregon-specific live load factors will be applied to all subsequent load ratings. If the new live load factors are higher (more conservative) than before, ODOT will assess the accumulated body of LRFR load ratings and determine a minimum rating factor threshold to warrant re-rating of bridges. Conservatively, this threshold would be set to match the upper bound percentage increase in the calibrated live load factors

for any rating vehicle. Any bridges that have rating factors below this threshold will have the load ratings updated and load restrictions applied, as required. Additional detail regarding the implementation plans are reported by Groff (2006).

Conclusions

The first ever state-wide calibration of live load factors for LRFR bridge evaluation and rating has been performed. This calibration employed the methodology described in the LRFR Manual commentary Article C6.4.4.2.3 for development of site-specific live load factors. WIM data were used to develop the live load factors for evaluation and rating of state-owned bridges. The factors were calibrated using the same statistical methods used in the original development of the LRFR Specifications. Due to the unique jurisdictional and enforcement characteristics in the state, modifications were used to describe the alongside truck population and conservatively included continuous trip permit vehicles in this population. WIM data were filtered, sorted, and checked for quality as part of the calibration process. Using the statistical data from the four WIM sites with different ADTT volume, at different times of the year, and over different WIM data collection windows, live load factors were computed. The Oregon-specific live load factors were smaller than those in the LRFR Specification. The factors were smaller for the lower volume sites and smaller for the heavier permit trucks. The high volume site showed little seasonal variation, was insensitive to direction of travel, and two-weeks of data were sufficient to produce consistent factors. For the lower volume

sites, some seasonal variation was observed with higher load factors during summer and fall due to agricultural and construction transport. In all cases, the largest computed live load factor from each data set was used to describe the WIM site. By employing the procedures used to develop the LRFR Specification, the resulting live load factors maintain the nationally accepted structural reliability index for evaluation, even though the resulting state-specific live load factors were smaller than the national standard. The large WIM data sets used in the state-specific calibration process were significantly larger than that used in the original LRFD or LRFR calibration process. Finally, policy implementation for the Oregon-specific factors included rounding the computed values to the nearest 0.05, set a lower limit of 1.0 for the live load factors, and established provisions for maintenance of the factors into the future.

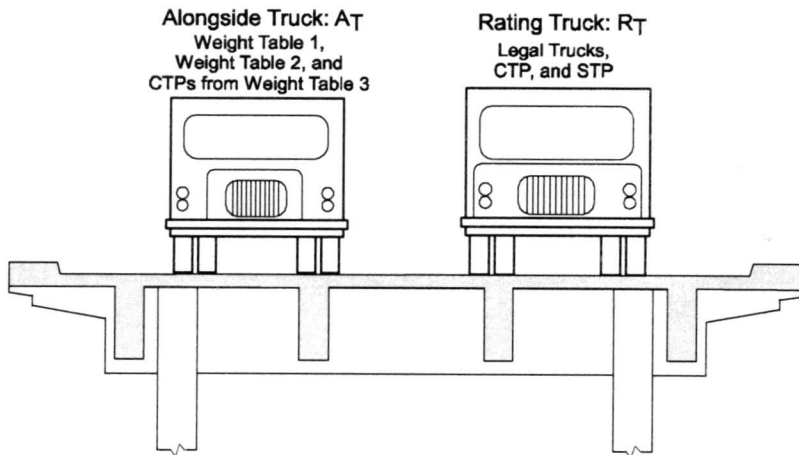


Figure 2.1: Maximum loading event for calibration of live load factors.



Figure 2.2: Map of Oregon WIM sites used in the study.

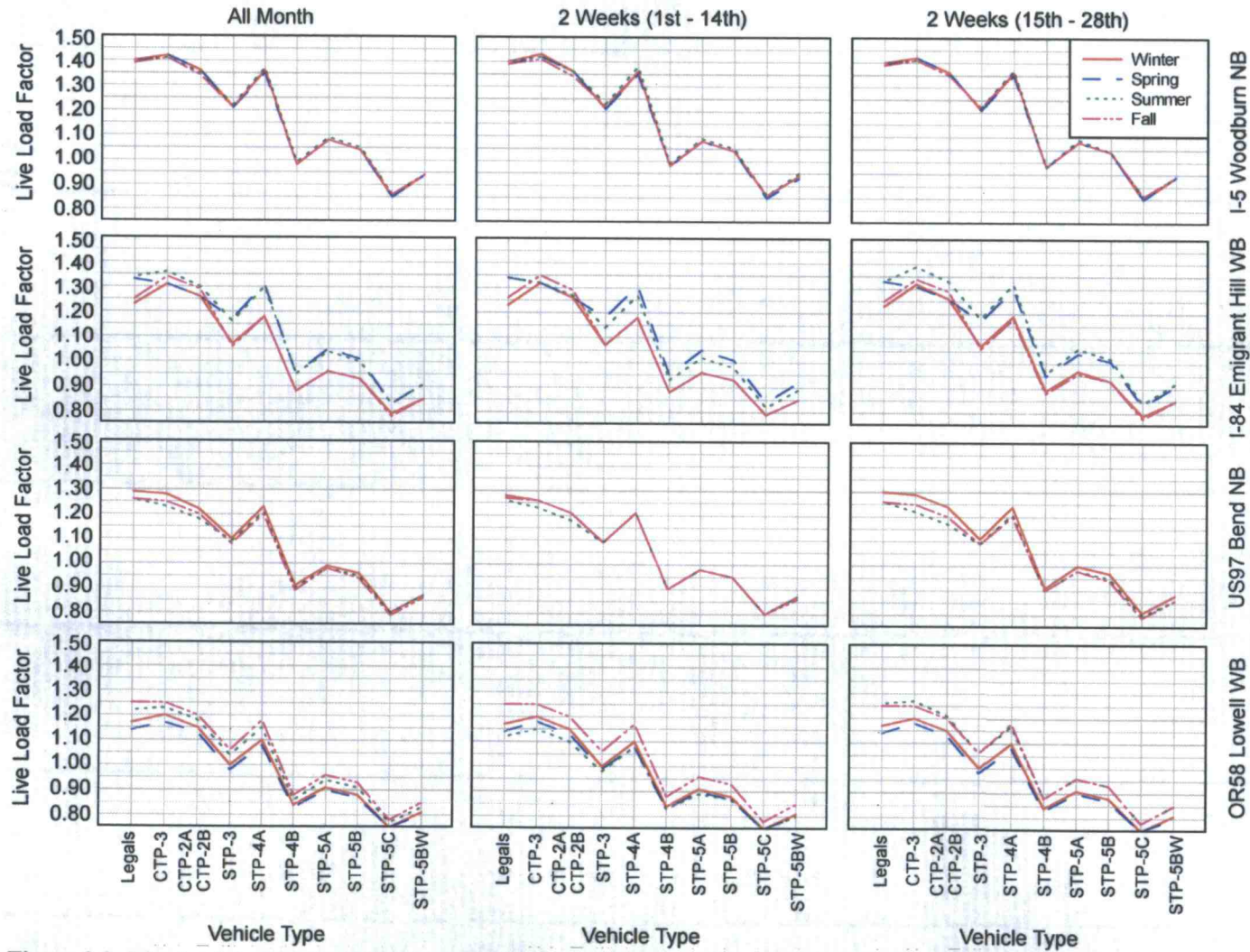


Figure 2.3: Live load factors for WIM sites at different seasons and different WIM data windows.

Table 2.1: ODOT rating vehicle classifications.

| Rating Vehicle | Live Load Factor Designation | GVW (kips) |
|----------------|------------------------------|------------|
| Legal Type 3 | Oregon Legal Loads | 50 |
| Legal Type 3S2 | | 80 |
| Legal Type 3-3 | | 80 |
| OR-CTP-2A | CTP-2A,2B | 105.5 |
| OR-CTP-2B | | 105.5 |
| OR-CTP-3 | CTP-3 | 98 |
| OR-STP-3 | STP-3 | 120.5 |
| OR-STP-4A | STP-4A | 99 |
| OR-STP-4B | STP-4B | 185 |
| OR-STP-5A | STP-5A | 150.5 |
| OR-STP-5B | STP-5B | 162.5 |
| OR-STP-5C | STP-5C | 258 |
| OR-STP-5BW | STP-5BW | 204 |

Table 2.2: Selected WIM sites, locations, and ADTT.

| Corridor | Site Location | Site Designation | ADTT | ADTT % of ADT | Winter 2005 | Spring 2005 | Summer 2005 | Fall 2005 |
|----------|------------------|------------------|------|---------------|-------------|-------------|-------------|-----------|
| I-5 | Woodburn NB | WBNB | 5550 | 13% | Jan | Apr | June | Oct |
| US97 | Bend NB | BNB | 607 | 8% | Dec | - | June | Oct |
| OR58 | Lowell WB | LWB | 581 | 7% | Jan | Apr | June | Oct |
| I-84 | Emigrant Hill WB | EHWB | 1786 | 36% | Nov | Apr | May | Oct |

Table 2.3: Results of sorting methods for Weight Table classification.

| | Site | Sort Method* | Table 1 | Table 2 | Table 3 | Table 4 | Table 5 | Table X | Total Records | CTP from WT3 to WT2 [§] | STP Per Day |
|--------|-----------|--------------|---------|---------|---------|---------|---------|---------|---------------|----------------------------------|-------------|
| Winter | I-5 WBNB | C | 124062 | 13175 | 1788 | 44 | 1 | 32 | 139102 | 477 | 45 |
| | | M | 125014 | 13690 | 366 | 29 | 2 | 1 | 139102 | | |
| | US97 BNB | C | 9776 | 411 | 398 | 9 | 0 | 1 | 10595 | 185 | 7 |
| | | M | 9954 | 535 | 105 | 1 | 0 | 0 | 10595 | | |
| | OR58 LWB | C | 15157 | 469 | 30 | 3 | 0 | 0 | 15659 | 4 | 1 |
| | | M | 15164 | 477 | 17 | 1 | 0 | 0 | 15659 | | |
| | I-84 EHWP | C | 43416 | 2224 | 72 | 2 | 0 | 0 | 45714 | 14 | 2 |
| | | M | 43447 | 2253 | 14 | 0 | 0 | 0 | 45714 | | |
| Spring | I-5 WBNB | C | 136364 | 13065 | 1835 | 57 | 1 | 25 | 151347 | 609 | 44 |
| | | M | 137374 | 13554 | 392 | 21 | 2 | 4 | 151347 | | |
| | US97 BNB | C | - | - | - | - | - | - | 0 | - | - |
| | | M | - | - | - | - | - | - | 0 | | |
| | OR58 LWB | C | 17455 | 433 | 17 | 3 | 0 | 0 | 17908 | 3 | 4 |
| | | M | 17460 | 442 | 6 | 0 | 0 | 0 | 17908 | | |
| | I-84 EHWP | C | 37249 | 3433 | 7177 | 73 | 2 | 77 | 48011 | 3688 | 121 |
| | | M | 39846 | 5964 | 2191 | 9 | 1 | 0 | 48011 | | |
| Summer | I-5 WBNB | C | 143018 | 13684 | 4713 | 89 | 4 | 47 | 161555 | 1938 | 97 |
| | | M | 145524 | 15001 | 1004 | 19 | 6 | 1 | 161555 | | |
| | US97 BNB | C | 15676 | 763 | 2304 | 9 | 1 | 20 | 18773 | 1616 | 24 |
| | | M | 16640 | 1811 | 314 | 7 | 1 | 0 | 18773 | | |
| | OR58 LWB | C | 24765 | 954 | 95 | 12 | 1 | 3 | 25830 | 45 | 2 |
| | | M | 24813 | 982 | 32 | 3 | 0 | 0 | 25830 | | |
| | I-84 EHWP | C | 45109 | 4206 | 1057 | 13 | 0 | 8 | 50393 | 596 | 16 |
| | | M | 45450 | 4563 | 378 | 0 | 0 | 0 | 50393 | | |
| Fall | I-5 WBNB | C | 135964 | 12136 | 3912 | 93 | 14 | 46 | 152165 | 1436 | 85 |
| | | M | 137776 | 13298 | 1025 | 47 | 19 | 0 | 152165 | | |
| | US97 BNB | C | 18028 | 708 | 304 | 12 | 4 | 11 | 19067 | 117 | 7 |
| | | M | 18167 | 831 | 60 | 7 | 2 | 0 | 19067 | | |
| | OR58 LWB | C | 25235 | 1278 | 202 | 9 | 1 | 13 | 26738 | 141 | 3 |
| | | M | 25388 | 1309 | 36 | 5 | 0 | 0 | 26738 | | |
| | I-84 EHWP | C | 48426 | 3084 | 49 | 0 | 0 | 1 | 51560 | 10 | 1 |
| | | M | 48447 | 3101 | 12 | 0 | 0 | 0 | 51560 | | |

+: C= Conventional sort, M=Modified Sort

§: CTP from WT3 to WT2 are records of CTP trucks in Weight Table 3 that were moved into Weight Table 2 to be included in the alongside truck population.

*: STP per day computed as total number of vehicles in Weight Tables 3 (minus the CTPs moved into Weight Table 2), 4, 5, and X divided by the number of days in the month.

Table 2.4: Statistics from controlling WIM data sets used in live load factor calibration.

| Vehicle | Statistic | Site | | | |
|---------------------------|------------------|----------|-----------|----------|----------|
| | | I-5 WBNB | I-84 EHWP | US97 BNB | OR58 LWB |
| Legals (Type 3, 3S2, 3-3) | W | 75.06 | 71.32 | 76.66 | 69.17 |
| | σ_{3S2} | 1.98 | 3.40 | 1.25 | 2.93 |
| | W_{along} | 83.90 | 80.84 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 8.53 | 8.38 | 8.46 |
| CTP-3 | W_{along} | 84.01 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.85 | 10.23 | 8.38 | 8.46 |
| CTP-2A CTP-2B | W_{along} | 84.01 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.85 | 10.23 | 8.38 | 8.46 |
| STP-3 | W_{along} | 83.90 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.46 |
| STP-4A | W_{along} | 83.90 | 80.82 | 80.78 | 76.11 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.04 |
| STP-4B | W_{along} | 83.90 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.46 |
| STP-5A | W_{along} | 83.90 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.46 |
| STP-5B | W_{along} | 83.90 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.46 |
| STP-5C | W_{along} | 83.90 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.46 |
| STP-5BW | W_{along} | 83.90 | 80.82 | 80.78 | 75.79 |
| | σ_{along} | 9.73 | 10.23 | 8.38 | 8.46 |

Table 2.5: Computed Oregon-specific live load factors for legal loads and LRFR Table 6-5 values.

| Traffic Volume (one direction) | Load Factor | |
|-----------------------------------|-------------|-----------------|
| | LRFR | Oregon-Specific |
| Unknown | 1.80 | 1.40 |
| ADTT \geq 5000 | 1.80 | 1.40 |
| ADTT = 1500 | 1.67 | 1.34 |
| ADTT \leq 500 | 1.51 | 1.30 |

Table 2.6: Computed Oregon-specific live load factors for permit loads and upper portion of LRFR Table 6-6 values.

| Permit Type | Frequency Condition | Loading Condition | DF | Permit Vehicle | Live load Factor γ_L by ADTT (one direction) | | | | | |
|--------------------------|----------------------------------|---|-----------------|----------------|---|-----------------|--------|-----------------|-------|-----------------|
| | | | | | > 5000 | | = 1500 | | < 500 | |
| | | | | | LRFR | Oregon-Specific | LRFR | Oregon-Specific | LRFR | Oregon-Specific |
| Continuous Trip (Annual) | Unlimited Crossings | Mix w/traffic (other vehicles may be on the bridge) | 2 or more lanes | CTP-2A | 1.75 | 1.36 | 1.58 | 1.33 | 1.45 | 1.24 |
| | | | | CTP-2B | 1.75 | 1.36 | 1.58 | 1.33 | 1.45 | 1.24 |
| | | | | CTP-3 | 1.80 | 1.43 | 1.63 | 1.39 | 1.49 | 1.29 |
| Single Trip | Route-Specific Limited Crossings | Mix w/traffic (other vehicles may be on the bridge) | 2 or more lanes | STP-3 | 1.60 | 1.23 | 1.46 | 1.18 | 1.35 | 1.11 |
| | | | | STP-4A | 1.80 | 1.38 | 1.63 | 1.32 | 1.49 | 1.24 |
| | | | | STP-4B | 1.30 | 0.99 | 1.21 | 0.96 | 1.14 | 0.91 |
| | | | | STP-5A | 1.30 | 1.09 | 1.21 | 1.06 | 1.14 | 1.00 |
| | | | | STP-5B | 1.30 | 1.05 | 1.21 | 1.02 | 1.14 | 0.97 |
| | | | | STP-5C | 1.30 | 0.86 | 1.21 | 0.84 | 1.14 | 0.81 |
| | | | | STP-5BW | 1.30 | 0.95 | 1.21 | 0.92 | 1.14 | 0.88 |

Table 2.7: Directional influence for live load factors at the I-5 Woodburn NB and SB sites for January, 2005.

| Location | Time-Frame | Legals | CTP-3 | CTP-2A/2B | STP-3 | STP-4A | STP-4B | STP-5A | STP-5B | STP-5C | STP-5BW |
|----------|-------------|--------|-------|-----------|-------|--------|--------|--------|--------|--------|---------|
| I-5 WBNB | All Month | 1.40 | 1.42 | 1.36 | 1.21 | 1.36 | 0.98 | 1.08 | 1.04 | 0.85 | 0.94 |
| I-5 WBSB | All Month | 1.39 | 1.42 | 1.36 | 1.22 | 1.37 | 0.98 | 1.09 | 1.05 | 0.86 | 0.94 |
| I-5 WBNB | 1st - 14th | 1.40 | 1.43 | 1.36 | 1.21 | 1.36 | 0.98 | 1.08 | 1.04 | 0.86 | 0.94 |
| I-5 WBSB | 1st - 14th | 1.38 | 1.42 | 1.36 | 1.22 | 1.37 | 0.98 | 1.08 | 1.04 | 0.86 | 0.94 |
| I-5 WBNB | 15th - 28th | 1.40 | 1.42 | 1.36 | 1.21 | 1.36 | 0.98 | 1.08 | 1.04 | 0.85 | 0.94 |
| I-5 WBSB | 15th - 28th | 1.39 | 1.43 | 1.36 | 1.23 | 1.38 | 0.99 | 1.09 | 1.05 | 0.86 | 0.95 |

Table 2.8: Sensitivity analysis for alongside vehicle variability for select rating vehicles during summer season (2 Weeks - 1st - 14th).

| Site Info | | Original Statistics* | | Increase W to Increase γ_L by 0.05 | | Increase σ to Increase γ_L by 0.05 | |
|-----------|---------------------------------------|----------------------|-----------------|---|----------|--|----------|
| | | W (kips) | σ (kips) | W (kips) | % Change | σ (kips) | % Change |
| I-5 WBNB | Legals $\gamma_L = 1.40$ to 1.45 | 83.9 | 9.7 | 91.3 | 9% | 11.5 | 18% |
| | CTP-3 $\gamma_L = 1.42$ to 1.47 | 83.9 | 9.7 | 93.0 | 11% | 10.9 | 12% |
| | STP-4A $\gamma_L = 1.38$ to 1.43 | 83.9 | 9.7 | 93.1 | 11% | 11.0 | 13% |
| US97 BNB | Legals $\gamma_L = 1.26$ to 1.31 | 81.7 | 6.5 | 89.1 | 9% | 8.5 | 31% |
| | CTP-3 $\gamma_L = 1.23$ to 1.28 | 81.7 | 6.5 | 90.8 | 11% | 7.9 | 21% |
| | STP-4A $\gamma_L = 1.21$ to 1.26 | 81.7 | 6.5 | 90.7 | 11% | 7.9 | 22% |
| OR58 LWB | Legals $\gamma_L = 1.12$ to 1.17 | 68.2 | 6.3 | 75.6 | 11% | 8.3 | 32% |
| | CTP-3 $\gamma_L = 1.15$ to 1.20 | 68.2 | 6.3 | 77.3 | 13% | 7.7 | 22% |
| | STP-4A $\gamma_L = 1.08$ to 1.13 | 68.2 | 6.3 | 77.4 | 13% | 8.2 | 30% |
| I-84 EHWB | Legals $\gamma_L = 1.34$ to 1.39 | 80.8 | 8.5 | 88.2 | 9% | 10.4 | 22% |
| | CTP-3 $\gamma_L = 1.32$ to 1.37 | 80.8 | 8.5 | 89.9 | 11% | 9.8 | 15% |
| | STP-4A $\gamma_L = 1.27$ to 1.32 | 80.8 | 8.5 | 90.0 | 11% | 10.0 | 17% |

*Statistics derived from WT1, WT2, & CTP's < 99.0k from WT3 (alongside vehicle)

Table 2.9: ODOT Adaptation of LRFR Table 6-5 Generalized Live-Load Factors for Legal Loads: γ_L

| | Liveload Factor γ_L by ADTT ^a (one direction) ^b | | | |
|--------------------------------|--|-------------|--------|------------|
| Traffic Volume (one direction) | Unknown | ≥ 5000 | = 1500 | ≤ 500 |
| Liveload Factor γ_L | 1.40 | 1.40 | 1.35 | 1.30 |

Notes:

^a Interpolate the Liveload Factor by ADTT values. Liveload Factors from this table should not be used when advanced methods of analysis are employed.

^b If there are two directions of traffic, use only half of the structure ADTT to determine the Liveload Factors.

Table 2.10: ODOT adaptation of upper portion of LRFR Table 6-6 for ODOT Routine Permits.

| Permit Type | Frequency | Loading Condition | DF ^a | Permit Vehicle | Liveload Factor γ_L by ADTT ^b (one direction) ^c | | | |
|-----------------------------|----------------------------------|---|-----------------|----------------|---|-------------|----------|------------|
| | | | | | Unknown | ≥ 5000 | $= 1500$ | ≤ 500 |
| Continuous Trip (Annual) | Unlimited Crossings | Mix w/traffic (other vehicles may be on the bridge) | 2 or more lanes | CTP-2A | 1.35 | 1.35 | 1.35 | 1.25 |
| | | | | CTP-2B | 1.35 | 1.35 | 1.35 | 1.25 |
| | | | | CTP-3 | 1.45 | 1.45 | 1.40 | 1.30 |
| Single Trip | Route-Specific Limited Crossings | Mix w/traffic (other vehicles may be on the bridge) | 2 or more lanes | STP-3 | 1.25 | 1.25 | 1.20 | 1.10 |
| | | | | STP-4A | 1.40 | 1.40 | 1.35 | 1.25 |
| | | | | STP-4B | 1.00 | 1.00 | 1.00 | 1.00 |
| | | | | STP-5A | 1.10 | 1.10 | 1.05 | 1.00 |
| | | | | STP-5B | 1.05 | 1.05 | 1.05 | 1.00 |
| | | | | STP-5C | 1.00 | 1.00 | 1.00 | 1.00 |
| | | | | STP-5BW | 1.00 | 1.00 | 1.00 | 1.00 |

Notes:

- ^a DF = LRFD Liveload Distribution Factor. When one-lane distribution factor controls for an exterior girder, the built-in Multiple Presence Factor for one lane (1.2) should be divided out of the Distribution Factor.
- ^b Interpolate the Liveload Factor by ADTT values. Liveload Factors from this table should not be used when advanced methods of analysis are employed.
- ^c If there are two directions of traffic, use only half of the structure ADTT to determine the Liveload Factors.
- ^d DF = LRFD Liveload Distribution Factor. When a one-lane Distribution Factor is used, the built-in Multiple Presence Factor for one lane (1.2) should be divided out of the Distribution Factor.

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APPENDIX A

Appendix A

Example Calculation of Live Load Factors

The following section provides a detailed example for calculating live load factors. Data from the I-5 Woodburn NB site for June 2005 (2 weeks, 1st – 14th) is used to illustrate the procedure. Live load factors are calculated for Oregon Legal Loads, CTP-2A, CTP-2B, CTP-3, and STP-3. The statistics used in demonstration of the calculation for the live load factors are shown in Table A1.

Table A2.1: Statistics for I-5 Woodburn NB, June 2005 (2 weeks, 1st - 14th)

| Vehicle | Using the Top 20% of the WIM Record | | |
|-----------------|-------------------------------------|-------------------|------------------|
| | Max GVW | Mean W* | σ^* |
| 3S2 - Legal | 80 ^K | 75.1 ^K | 2.0 ^K |
| Alongside Truck | 105.5 ^K | 83.9 ^K | 9.7 ^K |

1) Load Factor for Oregon Legal Loads.

Using a 1/30 probability of side-by-side events for two legal trucks, a 5 year evaluation period, an ADTT=5550, and taking the top 20% of the record; the number of side-by-side events N:

$$N = (5550)(365)(5)(1/30)(1/5) = 67,525$$

$$1/N = 1.4809 \times 10^{-5}$$

From NCHRP 454, Appendix A: $t_{ADTT} = 4.18$

$$R_T = 75.1 + 4.18 \times 2.0$$

$$= 83.3^K$$

$$A_T = 83.9 + 4.18 \times 9.7$$

$$= 124.5^K$$

$$W_T = 83.3^K + 124.5^K$$

$$= 207.8^K$$

$$\gamma_L = 1.8 \times \frac{207.8}{240} \times \frac{72}{80}$$

= **1.40** → This is the controlling value for ADTT ≥ 5000

2) Load Factors for Continuous Trip Permits (CTP).

ODOT has estimated that CTPs are about 30% of legal truck traffic on I-5 for determining the number of side-by-side events, N (CTP adjacent to a legal truck).

$$N = 67525 \times 0.30 = 20258$$

$$1/N = 4.9364 \times 10^{-5}$$

From NCHRP 454, Appendix A: $t_{ADTT} = 3.89$

$$A_T = 83.9 + 3.89 \times 9.7$$

$$= 121.8^k$$

a) For 105.5^k CTP (CTP-2A/2B)

$$R_T = 105.5 + 3.89 \times 9.7$$

$$= 143.4^k$$

$$W_T = 143.4^k + 121.8^k$$

$$= 265.2^k$$

$$\gamma_L = 1.8 \times \frac{265.2}{240} \times \frac{72}{105.5}$$

= **1.36** → This is the controlling value for ADTT ≥ 5000

b) For 98^k CTP (CTP-3A)

$$R_T = 98 + 3.89 \times 9.7$$

$$= 135.9^k$$

$$W_T = 135.9^k + 121.8^k$$

$$= 257.7^k$$

$$\gamma_L = 1.8 \times \frac{257.7}{240} \times \frac{72}{98}$$

$$= 1.42$$

3) Load Factor for 120.5^K STP-3 (same method for all STP vehicles)

From Table 3, $N_p = 97$:

$$N = (97)(365)(5)(1/30) = 5901$$

$$1/N = 1.6947 \times 10^{-4}$$

From NCHRP 454, Appendix A: $t_{ADTT} = 3.58$

$$\begin{aligned} A_T &= 83.9^K + 3.58 \times 9.7^K \\ &= 118.8^K \end{aligned}$$

$$\begin{aligned} R_T &= 120.5 + 34.7 \\ &= 155.4^K \end{aligned}$$

$$\begin{aligned} W_T &= 155.4^K + 118.8^K \\ &= 274.1^K \end{aligned}$$

$$\gamma_L = 1.8 \times \frac{274.1}{240} \times \frac{72}{120.5}$$

= 1.23 → This is the controlling value for ADTT ≥ 5000

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Evaluation of Bridge Rating Vehicles Using Weigh-In-Motion Data

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Abstract

For bridge rating and evaluation, notional truck models are commonly used to simulate the load effects produced by the truck population. The recently developed Load Resistance and Factor Rating (LRFR) Bridge Evaluation Manual was calibrated based on the 3S2 truck configuration as the notional model. LRFR also permits development of site-specific live-load factors and provides a methodology for their calculation based on GVW of the local truck population. Using GVW as the parameter for establishing live load factors to reflect load effects may not necessarily provide consistent outcomes across all bridge span lengths, indeterminacies, or specific load effects. This is because the load effects are dependent on the distributions of the axle weights, the axle spacing, and the number of axles, in addition to the span geometry and support conditions.

The Oregon Department of Transportation currently uses a suite of 13 rating vehicles for evaluation of their bridge inventory. Live load factors were developed for this suite of trucks, based on weigh-in-motion (WIM) measured GVW data from sites located across the state. The load effects for Oregon's bridge rating

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vehicles have been calculated for various span lengths and support conditions. These load effects, both unfactored and factored, were compared with load effects calculated using vehicles from large sets of WIM data. Further, because no established standard of time or quantity of WIM data has previously been recognized, a separate study was conducted in order to determine an acceptable window of WIM data. The objective of this analysis was to determine if the load effects and the live load factors developed for bridge rating produced by the suite of vehicles envelope load effects produced by an acceptable window of collected vehicle data for a variety of bridge span lengths and types. Observations and suggestions are made based on the results of these analyses.

CE Database Subject Headings: bridges, analysis, bridge loads, ratings, load factors

Introduction

For bridge rating and evaluation, notional truck models are commonly used to simulate the load effects produced by the truck population. The recently developed Load and Resistance Factor Rating (LRFR) Bridge Evaluation Manual was calibrated based on the 3S2 truck configuration, as the notional model. The LRFR manual also permits development of site-specific live-load factors and provides a methodology for their calculation based on the gross vehicle weight (GVW) of the local truck population. Using GVW as the parameter for establishing live load

factors to reflect load effects may not necessarily provide consistent outcomes across all bridge span lengths, indeterminacies, or specific load effects. This is because the load effects are dependent on the distributions of the axle weights, the axle spacing, and the number of axles (Kim *et al.* 1997), in addition to the span geometry and support conditions.

The Oregon Department of Transportation currently uses a suite of 13 rating vehicles for evaluation of their bridge inventory. Recently, live load factors were developed for this suite of trucks, based on weigh-in-motion (WIM) measured GVW data from sites located across the state, as reported in Pelphrey and Higgins (2006). To supplement that study, the load effects for Oregon's bridge rating vehicles have been calculated for various span lengths and support conditions. These load effects, both factored and unfactored, were compared with load effects calculated using vehicles from large sets of WIM data. Further, because no established standard of time or quantity of WIM data has previously been recognized, a separate study was conducted in order to develop recommended WIM data collection windows. The objective of this analysis was to determine if the load effects and the live load factors developed for bridge rating produced by the suite of vehicles envelope load effects produced by an acceptable window of collected vehicle data for a variety of bridge span lengths and types. Observations and suggestions are made based on the results of this analysis.

Background

Oregon allows vehicle loads and configurations on the state highways that are different from many other states. Many of the vehicles are above the federal legal weight limit but are allowed on the highways under permits. Oregon bridge design and rating have relied on national models that are based on data collected in other countries and states. This data may not accurately reflect the loads found in Oregon.

The Oregon Department of Transportation (ODOT) collects data on vehicle weight and axle spacing lengths at WIM scale locations throughout the state. Using this data, analyses were performed to establish an appropriate window of WIM data necessary to reasonably extrapolate future loading events, evaluate the current ODOT bridge rating vehicles, the Motor Carrier Transportation Division (MCTD) weight tables, and permit classifications. The current ODOT rating vehicles are shown and illustrated in Table 1. Also shown in Appendix A are the five MCTD weight tables.

Each of the ODOT rating vehicles was previously selected by MCTD to be representative of one of the five MCTD weight tables, as shown in the right column of Table 1. There are at least two rating vehicles for each weight table, which attempt to capture the range of load effects produced within the weight tables, although no previous analysis has been performed to validate them.

WIM Data

WIM data is collected at various sites along Oregon's interstate and highway systems. It is the process of collecting vehicle weight and axle configuration while the vehicle is moving (Daniels, 2004). The WIM system is designed to monitor and record individual characteristics for each passing vehicle. These include the date and time, type/class of vehicle as classified by ODOT, lane position, speed, gross vehicle weight, overall length, equivalent single axle load (ESAL) value, total number of axles, overall axle weights, left axle weight, right axle weight, axle spacing lengths relative to each other, and the allowable axle weights according to MCTD's Weight Table 1. Some of the records include additional markings like "TAG_H: 000545968675", which designate that the vehicle is equipped with a transponder for use in Oregon's Green Light (Preclearance) Program (Fifer, 2005). Fig. 1 shows an example of a WIM recorded vehicle event.

There is a $\pm 2-3\%$ error rate as a result of the fluctuation of weight distribution due to the vehicle being in motion (Fifer, 2002). This error is most evident for vehicles hauling liquids, livestock, and for log trucks without middle supports (Daniels, 2004).

WIM data are divided up into two types, REALTIME and raw. The analysis herein focuses on the raw data format. Data are recorded continuously to a text file, which

is stored on a hard drive located at the site. This data is retrieved monthly and posted on an ftp server for download.

Cleaning and Filtering the WIM Data

In order to use the raw WIM data, a considerable amount of pre-processing must take place. Once downloaded from the ftp site, the data must be reformatted for use in subsequent analyses and cleaned to remove erroneous records. Two FORTRAN programs were written to accomplish this task: Wingnut and Liger. The Wingnut program formats the data according to a specified fixed-width and stores it to a new file. This program also filters out some of the obvious errors that are encountered. The Liger program cleans the data from Wingnut. It reads the new text file created by the Wingnut program and filters out spurious data. It checks all vehicle records to make sure they contain realistic numerical values (vehicle-specific criteria) and are free from invalid characters (such as letters where numbers should be, etc.). A detailed justification summary of the Liger program, as well as documented quality control checks for processing the WIM data, can be found in Appendix B.

Cleaning and filtering of the raw WIM data were performed to remove the following:

1. Record where the gross vehicle weight (GVW) value is equal to 0.0.
2. Record does not follow the general record pattern; this could be any inconsistency in the time stamp, words out of place from the status quo, incomplete records, etc.

3. Records with misplaced characters, such as a letter where a number should be or a number where a letter should be.
4. Record where an individual axle is greater than 50 kips.
5. Record where the speed is less than 10 mph.
6. Record where the speed is greater than 99 mph.
7. Record where the length is greater than 200 ft.
8. Record where the sum of the axle spacing lengths are greater than the length of the truck.
9. Record where the sum of the axle spacing lengths are less than 7 ft.
10. Record where the first axle spacing is less than 5 ft.
11. Record where the # of axles is greater than 13.
12. Record where the GVW is greater than 280 kips.
13. Record where any axle spacing is less than 3.4 ft.
14. Record which has a GVW +/- the sum of the axle weights by more than 7%.
15. Record which has a GVW less than 2.0 kips.

After both of these programs have been executed on the WIM data file, the results are then used for sorting and analysis.

WIM Site Selection

There are currently five highways/interstates in Oregon which are collecting WIM data: I-5, I-84, OR58, US26, and US97. From these highways, one individual site was selected for load effect analysis – I-5 Booth Ranch NB. The two criteria in choosing this site were the volume of average daily truck traffic (ADTT), and the amount of available continuous raw WIM data. The WIM site chosen is shown circled in Fig. 2. A complete breakdown of the WIM sites located throughout the state can be found in Appendix C.

Previous research has shown that high ADTT sites produce higher live load factors due to the increased likelihood of side-by-side concurrence (Moses 2001, Pelphey & Higgins 2006). Thus, a site was selected for this analysis that had a relatively high ADTT in order to capture the upper tail of the vehicle population. The I-5 Booth Ranch NB site was selected in part because it matched this criteria. Only I-5 Woodburn NB, I-5 Woodburn SB, and I-84 Cascade Locks EB have higher ADTT values. I-5 Booth Ranch NB was selected over these three sites because of the amount of available continuous raw WIM data, as explained in the next paragraph. The ADTT for I-5 Booth Ranch NB is shown in Table 2, along with other pertinent information (Fifer, 2005).

The site selected for this analysis had a complete and continuous year of raw WIM data. Only one other site, I-5 Ashland NB, contained data for the entire year of 2005. I-5 Booth Ranch NB was selected because the ADTT was greater than that of I-5 Ashland NB (ADTT of 2979). A summary of the measured vehicle traffic mix is presented in Table 3. Figure 3 presents a frequency histogram of the number of axles per vehicle. Data are included for all vehicles captured by the WIM scale at I-5 Booth Ranch NB for each month in 2005. There were a total of 981,226 valid WIM vehicles passing the site over the entire year.

Weight Table Sorting Methodology

Classifying and sorting the WIM data proved to be an important issue. Two separate WIM data sorting methods were investigated and compared to one another. These are the Conventional Sort method and the Modified Sort method.

1. Conventional Sort (“GVW + Axle Group Sort”)

- This method sorts vehicles based on their GVW, axle group weights, and length. It is the method currently used by the Oregon Department of Transportation to classify vehicles as Weight Table 1, Weight Table 2, Weight Table 3, Weight Table 4, Weight Table 5, or Table X (the overflow table classification). Permits are issued based on a vehicle’s Weight Table classification.
- It accounts for the axle spacing in assigning each vehicle to the appropriate Motor Carrier Transportation Division (MCTD) Weight Table.
- It assigns more vehicles to higher Weight Tables than the Modified Sort (described subsequently) based on the axle weights.
- Proportionately more heavy vehicles that could have been interpreted as “rogue” legal vehicles are assigned to Weight Table 3 and above and are now considered as legitimate permit vehicles.
- It yields lower coefficients of variation compared to the Modified Sort.

- It yields lower live load factors compared to the Modified Sort (Pelphrey & Higgins, 2006).
- It is less conservative, but is thought to better represent the permitted truck population in Oregon, than the Modified Sort.

2. Modified Sort (“GVW + Truck Length Sort”)

- This method sorts vehicles based only on their GVW and rear-to-steer axle length, and it does not account for axle groupings.
- Assigns more vehicles to lower Weight Tables than the Conventional Sort.
- Proportionately more heavy vehicles that could have been interpreted as legitimate permit vehicles are conservatively assigned to Weight Tables 1 & 2 and are thus considered “rogue” legal vehicles.
- It yields higher coefficients of variation compared to the Conventional Sort.
- It yields higher live load factors compared to the Conventional Sort (Pelphrey & Higgins, 2006).
- It is more conservative, but may unfairly penalize Oregon’s well established, easily and simply available, and inexpensive permitting process, than the Conventional Sort.

Table 4 compares the Weight Table breakdown for each sorting method. The load effect analysis herein is based on the Conventional Sort method because it better represents the permitted truck population in Oregon. In contrast to some other states where truck drivers generally know the vehicle GVW but may not know their axle grouping weights, MCTD and ODOT report that Oregon truckers are generally aware of their axle and tandem weights, usually to within 2,000 lbs, which proves beneficial in obtaining a continuous trip permit (CTP) (Groff, 2006).

By comparing the number of Table X vehicles in the Conventional Sort to those of the Modified Sort, it is apparent that heavy axle groups control this vehicle classification. Because ODOT reviews all Table X vehicles internally by structural analysis, it is unlikely that there are over 800 real Table X vehicles crossing the site. This realization might infer that there are more rogue vehicles in the system than previously anticipated. However, previous data collection at a larger volume site, I-5 Woodburn NB, revealed that there were roughly 450 Table X vehicles by the Conventional Sort method for one year (Pelphrey & Higgins, 2006). The large number of Table X vehicles also might be related to the percent of error associated with the WIM equipment in capturing accurate individual axle weights, and the sensitivity of the weight table classifications to the individual axle groups. Nowak and Ferrand report that the accuracy is ± 20 percent for axle loads (Nowak and Ferrand, 2004). Another explanation for the large number of Table X vehicles may be a result of not imposing the two Weight Table 1 exceptions in the original sort

routines. The first exception allows two consecutive tandem axles to weigh up to 34,000 pounds each if the minimum axle spacing between tandems is 30 feet or more with a permit, or 36 feet or more without a permit. The second exception allows a group of four axles consisting of a set of tandem axles and two axles spaced nine feet or more apart to have a loaded weight more than 65,000 pounds and up to 70,000 pounds if the minimum axle spacing is 35 feet or more with a permit. The minimum axle spacing refers to the distance between the first and last axle of the group. All vehicles were sorted without regard to these exceptions. Because the Table X vehicles are not represented by a specific rating vehicle, they were compared with the operating level HL-93 configuration of the LRFR manual.

Selecting an Appropriate Window of Data

Typically, in practice, two-weeks of continuous WIM data are used for various types of analysis; however no established standard of time or quantity of WIM data has previously been recognized. For example, as shown by Nowak and Hong 1991, Nowak 1993, LRFD 1994, and Nowak 1999, the live-load model used in the Load Resistance and Factor Design (LRFD) bridge design code was calibrated using roughly 2 weeks of WIM data. Also, as shown in Moses 2001, LRFR 2003, and Minervino and others 2004, the live load factors used in the LRFR bridge rating manual were calibrated using the same 2 weeks of WIM data. Therefore, in order to determine an acceptable window of WIM data, a separate study was conducted. This study investigated the top 20 percent of selected WIM data according to

vehicle GVW with increasing windows of time. A complete year of WIM data for both a high-volume (I-5 Booth Ranch NB, ADTT = 3442) and a low-volume site (US 97 Klamath Falls NB, ADTT = 769) were analyzed. The projection windows for both sites began with June 1st, 2005. The projection periods included 2-day, 7-day, 14-day, 30-day, 60-day, 120-day, 1-year, and 5-year extrapolation lengths. For each projection window, the data was plotted as a cumulative distribution function (CDF) (A detailed description of the CDF function is described in the following section, "Load Effect Procedure"). A best fit line was applied to the upper tail of each CDF. Each line was then extrapolated out to the selected projection periods to determine an estimated maximum GVW. Fig. 4 and 5 plot each of the seven projection periods with corresponding extrapolation lines for I-5 Booth Ranch NB and US 97 Klamath Falls NB, respectively. The equations represented by the extrapolation lines are shown on each plot. Projected GVW values were solved for by using these equations. Tables 5 and 6 show the maximum projected GVW values for each projection time window and corresponding statistical parameters, respectively, for I-5 Booth Ranch NB. Tables 7 and 8 show the maximum projected GVW values for each projection time window and corresponding statistical parameters, respectively, for US 97 Klamath Falls NB. Fig. 6 and 7 show the projected GVW values graphically for I-5 Booth Ranch NB and US 97 Klamath Falls NB, respectively.

The results for the high ADTT site, I-5 Booth Ranch NB, show a steady decrease in extrapolated GVW values for increasing time windows, as shown in Table 5. For example, using 2 days of data, the 5-year extrapolation GVW is 349.5 kips, while using 7 days of data, the same extrapolation GVW is 318.2 kips. This trend is the same for all windows of time at this site. The percent change values between each of the adjoining windows of time are also shown in Table 5. The percent change between 2 and 7 days, and 7 and 14 days is greater than or equal to 9%. Then, between 14 and 30 days and following, the percent change decreases to a constant $\sim 4\text{-}6\%$. This would suggest that an appropriate window of time for this site for collection of WIM data would be between 14 and 30 days. This criterion was met and exceeded for the load effect portion of this study by using one full year of data. The coefficient of variation for each window of time was a constant 12%, as shown in Table 6.

The results for the low ADTT site, US 97 Klamath Falls NB, generally show a steady decrease in extrapolated GVW values for increasing time windows, as shown in Table 7. Only one time window, the 7 days of data window, does not follow the expected trend. This can be seen more clearly in Fig. 7. The percent change between 2 and 7 days is $\sim -25\%$, and between 7 and 14 days is $\sim +25\%$. The extrapolation values for the 7-day window might show a significant decrease because of the abnormally low standard deviation, which produces a higher slope, as shown in Table 8. The percent change between adjoining windows of time levels

out between 14 and 30 days. This would suggest that an appropriate window of time for this site for analyzing data would be 30 days. This criterion is again met and exceeded for the load effect portion of this report by using one full year of data.

From the results presented, it is recommended that at least 14-30 days of data at a high-volume site be used for WIM data analysis. Low-volume sites typically require longer windows of time to capture load effects represented in the upper tail. Also, as shown above, a low-volume site does not produce the same level of consistency as a high-volume site and when making decisions over an entire network, it is recommended to use the highest volume site.

Calculation of Load Effects

Once the data for each month was cleaned, filtered, and sorted according to the MCTD Weight Table classifications, it was used to compute load effects in a suite of bridges. The maximum shears and moments were computed for each of the 981,226 WIM records at selected locations for specified spans and span lengths. Four simply supported span lengths were analyzed: 50-ft, 100-ft, 150-ft, and 200-ft. For this study, shear values on the simply supported spans were calculated at a distance 4 ft from the support, and moment values were calculated at midspan. These locations were selected to capture the maximum load effects for each span length. Fig. 8 shows the locations of the selected points for the simple span configuration. A two-span continuous bridge model with 50-ft span lengths, typical

of 1950's vintage reinforced concrete deck girder bridges (Higgins *et al.* 2004), was also analyzed. Negative moment was evaluated at the center support, while shear was evaluated at a distance 4-ft to the left of the center support, as shown in Fig. 9.

In order to obtain an accurate projection of the upper tail of the WIM load effect histogram, only the largest 20 percent of all vehicle load effects were considered as the basis for fixing the vehicle load effect spectrum (Moses, 2001). Statistical data are presented in the form of cumulative distribution functions (CDF). This scale is used to present and compare the critical upper tails. The distributions were plotted on normal probability paper (Laman and Nowak, 1993). Two different CDF's were plotted (as shown in the next section, "Graphical Results") on each graph. The first CDF represents the top 20 percent of all vehicles for the entire year. The second CDF represents the top 20 percent of 3S2 vehicles for the entire year. In this study, a 3S2 vehicle was defined in the following way: five-axle vehicle with no specified maximum GVW, with the first axle spacing greater than 5.5 ft, second axle spacing less than 5.5 ft, third axle spacing greater than 5.5 ft, and the fourth axle spacing less than 5.5 ft.

The CDF's were plotted on normal probability paper for moments and shears for each of the span lengths and types. The vertical scale, z , is,

$$z = \Phi^{-1} [F(x)] \quad [1]$$

where $F(x)$ = cumulative distribution function of x , where x is the moment M or shear V ; Φ^{-1} = inverse of the standard normal distribution function. More information about the inverse of the standard normal distribution function can be found in Nowak 1999, Nowak and Collins 2000, and Haldar and Mahadevan 2000.

Since bridge ratings are typically evaluated at 5-year intervals, a 5-year projection was used. A line of best fit was applied to the tail end of each CDF. Each line was then extrapolated out to the 5-year line to determine the estimated maximum load effect. Let N be the total number of vehicles in time period T . The number of vehicles in the top 20 percent of the record at I-5 Booth Ranch NB for 2005 was 196,247. Because the WIM data represents one year of traffic, the number of vehicles, N , in $T = 5$ years will be 981,235. The probability level corresponding to N is $1/N$, and for $N = 981235$, it is $1/981235 = 1.02 \times 10^{-8}$, which corresponds to $z = 4.75$ on the vertical scale, as shown on each plot as the upper solid, horizontal line. The same approach was applied for the 3S2 vehicles, with a corresponding $z = 4.41$, as shown on each plot as the lower solid, horizontal line.

The rating vehicles, along with the HL-93 loading configuration, and AASHTO's four Notional Rating Load (NRL) vehicles, were also analyzed for each of the span configurations. The NRL vehicles were derived from the Federal Highway

Administration's "Specialized Hauling Vehicles", and represent short and heavy Legal vehicles. They were adopted at the 2005 AASHTO Bridge Meeting and will appear as an optional rating load in the 2006 LRFR Interim. Pictograms for the NRL vehicles can be found in Appendix A. Maximum moments and shears were calculated and are represented on each plot as vertical lines. Two plots were created for each span length and load effect: the first with unfactored rating vehicles and the second with factored rating vehicles.

Oregon-Specific Live Load Factors

Following the methodology developed in NCHRP Project No. 12-46 (Moses 2001) and incorporated in the LRFR Specifications, live load factors for strength evaluation were developed for state-owned bridges in Oregon using WIM data. Adaptation of the methods was necessary to account for unique characteristics of truck loads and permitting regulations in the state. Live load factors were developed using WIM data from four sites, including state and interstate routes, considering possible seasonal variations, and different WIM data collection windows (Pelphrey & Higgins, 2006). The computed live load factors represent the two lanes loaded case only. They account for the vehicle under consideration, for example, a Type 3S2 Legal vehicle or any of the vehicles depicted in Table 1, plus a likely alongside vehicle.

The load effects produced by the WIM data were compared to the load effects produced by Oregon's rating vehicles, both factored and unfactored. The factored values were obtained by multiplying the nominal force effect by a corresponding state-specific live load factor, as mentioned above. The live load factors used for the I-5 Booth Ranch NB site conservatively followed the ADTT ≥ 5000 category, as reported by Pelphrey & Higgins (2006).

Live load factors were also applied to the HL-93 configuration and to the NRL vehicles. Following the procedures of the LRFR, an operating level factor of 1.35 was applied to the HL-93 configuration (section 6.4.3.2.2 of LRFR, 2003). The same live load factor applied to the legal rating vehicles (Weight Table 1) of 1.40 was used for the NRL vehicles (Groff, 2006).

Maximum load effects corresponding to longer periods of time were calculated by extrapolation of the vehicle WIM data. The CDF representing all of the vehicles was used to calculate extrapolated load effects. These results are compared to the factored HL-93 (operating level) loading configuration, as shown in Table 9. Most of the ratio values are close to 1.0, with the majority exceeding 1.0. The operating level for the HL-93 loading configuration represents the 5-year extrapolation load effects produced by the WIM vehicles reasonably well.

Tabular and Graphical Results

Fig. 10 shows the CDF plot for unfactored moment for the 100-ft simple span bridge model and the corresponding Weight Table breakdown plots. Fig. 11 shows the CDF plot for factored moment for the 100-ft simple span bridge model and the corresponding Weight Table breakdown plots. Plots for both unfactored and factored moments and shears for all span types and lengths are shown in Appendix D.

Table's 10 and 11 show all results for simple span shear and moment, respectively. Table 12 shows the load effects for the two-span continuous model. The columns entitled "Ratio" describe whether or not the load effects for each rating vehicle exceed that of the highest observed WIM vehicle per table classification. A ratio greater than unity denotes that the rating vehicle adequately envelopes the load effect in question. Shaded values denote a ratio value less than 1.0.

Significant Findings

Significant findings based on results of this analysis are presented below. These include comparisons between the load effects produced by each of Oregon's rating vehicles and the load effects produced by the top 20 percent of traffic for a complete year of WIM data at I-5 Booth Ranch NB.

Unfactored Load Effects

Comparison between the unfactored load effects and the WIM data in Table's 10, 11, and 12 reveal further justification for the use of live load factors. The results show that the unfactored rating vehicles did not produce sufficient demand to represent the service level loads of the WIM data. Further, there was a need to include the likelihood of an alongside truck also being present on the bridge. Live load factors account for this condition using a two-lane loaded calibration. The magnitude of the alongside truck in 3S2 equivalents is shown in Table's 10, 11, and 12 as a percent of 3S2 value. For example, a value of 24 percent means the lane with the rating vehicle receives 100 percent of the load effect from the rating vehicle and also gets 24 percent of the maximum load effect from an alongside vehicle in 3S2 equivalents.

Factored Load Effects

The results show that the factored rating vehicles did a relatively good job providing sufficient demands to envelope the load effects of the WIM data. There were only a few factored rating vehicles at select span lengths that did not exceed the corresponding WIM value. Table 13 identifies the factored rating vehicles which were sufficient and insufficient with respect to the WIM data. It was not necessary for all of the rating vehicles within a table classification to eclipse the load effects of the WIM data. Rather, only one of the representative rating vehicles from each of the Weight Tables was needed to exceed the WIM results to be

deemed satisfactory. Ratios in Table 13 that are shaded represent rating vehicles that did not envelope the WIM data.

Rating Vehicle Summary

The factored rating vehicles representing Weight Table 1 effectively cover the load effect spectrum produced by the Table 1 WIM vehicles. All load effects for all span lengths are enveloped by at least one of the three representative vehicles. The Type 3S2 and Type 3-3 Legal vehicles provide sufficient capacity for all load effects analyzed, which suggests the Type 3 Legal vehicle could be eliminated. These vehicles were also fairly consistent with regards to the percent of adjacent 3S2 equivalents for the varying span lengths, which suggests a level of uniform reliability.

The factored rating vehicles representing Weight Table 2 effectively cover the load effect spectrum produced by the Table 2 WIM vehicles. The Type CTP-2A and the Type CTP-2B vehicles produce sufficient factored load effects for all span lengths considered. These vehicles were also fairly consistent with regards to the percent of adjacent 3S2 equivalents for the varying span lengths, which suggests a level of uniform reliability.

The factored rating vehicles representing Weight Table 3 effectively cover the load effect spectrum produced by the Table 3 WIM vehicles. The Type CTP-3 and the

Type STP-3 vehicles produce sufficient factored load effects for all spans considered. The STP-3 vehicle is fairly consistent with regards to the percent 3S2 values for varying span lengths, which suggests a level of uniform reliability. However, the CTP-3 is not as consistent. The percent of 3S2 values are higher for the 50-ft and 100-ft simple spans, then decrease for the 150-ft and 200-ft simple spans, as shown in Table's 10 and 11.

The factored rating vehicles representing Weight Table 4 do not effectively cover the load effect spectrum produced by the Table 4 WIM vehicles. The Type STP-4A vehicle is effective for load effects for span lengths of 50 and 100 ft, but not for span lengths of 150 and 200 ft. The Type STP-4B vehicle is not adequate for moments and shears at any span length. As a result, the load effects for the Weight Table 4 WIM vehicles exceed both rating vehicles for span lengths of 150 and 200 ft. The STP-4B vehicle is fairly consistent with regards to the percent 3S2 values for varying span lengths, which suggests a uniform level of reliability. However, the STP-4A is not as consistent. The percent of 3S2 values are higher for the shorter simple spans, and decrease with span length.

The factored rating vehicles representing Weight Table 5 effectively cover the shear spectrum produced by the Table 5 WIM vehicles, but do not effectively cover the moment spectrum for the 50-ft span. At least one of the Table 5 rating vehicles for all other span types and ranges envelop the WIM load effects. These vehicles

are fairly consistent with regards to the percent of 3S2 values for varying span lengths, which suggests a level of uniform reliability. The STP-5BW shows the most consistency, while the STP-5B is the least consistent.

Table X is ODOT's overflow table classification. These are vehicles that fall outside of Weight Table 5, and require axle weight and configuration approval by the ODOT bridge group. Therefore, rating vehicle adequacy does not apply to this classification of vehicles. However, the HL-93 factored at the LRFR operating level exceeded most Table X load effects as described further below.

HL-93 Loading

The operating level HL-93 loading configuration was also compared to the WIM data. It is represented on all CDF plots by a solid vertical line. The factored HL-93 configuration envelopes all exclusion traffic for the surveyed WIM data except for negative moment on the two-span continuous bridge model.

Notional Rating Load Vehicles

The NRL vehicles representing Weight Table 1 effectively cover the load effect spectrum produced by the Table 1 WIM vehicles. All load effects for all span lengths are enveloped by all four representative vehicles, except for the SU4 vehicle for negative moment on the two-span continuous bridge model. However, the implementation of the NRL vehicles as "Legal" vehicles is redundant, as the

existing three legal rating vehicles already adequately envelope the WIM load effects. Further, the NRL vehicles produced inconsistent results over the varying span lengths. The current representative AASHTO 3S2 legal vehicle produced more consistent load effects than the NRL vehicles. The percent of adjacent 3S2 load effect values in Table's 10, 11, and 12 vary significantly for the NRL vehicles over the different span lengths and appear to provide nonuniform levels of reliability.

Conclusions and Recommendations

A study was conducted to determine an amount of WIM data needed to extrapolate future loading events for both high and low ADTT volume sites. In a separate study, load effects for ODOT's suite of 13 bridge rating vehicles were calculated for various span lengths and types. These load effects, both factored and unfactored, were compared to the load effects calculated from vehicles in the WIM data. One full year of WIM data was collected, cleaned, filtered, sorted, and analyzed for I-5 Booth Ranch NB, a relatively high-volume ADTT site. The analyses included shear, positive moment, and negative moment values for various span types and lengths. Load effects were plotted as cumulative distribution functions on normal probability paper. Oregon-specific live load factors, developed from previous research, were applied to the lane-load effects for the suite of evaluation vehicles. The analysis presented herein analyzed lane-load effects, and

did not consider component-specific effects. Based on the findings, the following conclusions and recommendations are made:

- For a high ADTT volume site (approximately 3500 ADTT), approximately two weeks of WIM data is needed to adequately extrapolate future upper tail events. For a low ADTT volume site (approximately 500 ADTT), one month of WIM data is needed.
- Additional WIM data should be collected and analyzed. One year of data from two sites was used in this study to project loading events to a five year extrapolation window. As additional data become available, two and five years of collected data should be analyzed and results compared to the rating vehicles, and also to the one-year extrapolation values.
- The factored rating vehicles provided reasonably sufficient demands to envelope the load effects of the WIM data, including that attributed to an adjacent equivalent 3S2 alongside vehicle.
- The contribution of the alongside vehicle in 3S2 equivalents for each of the rating vehicles was presented as a percent of the nominal value to examine the consistency of the reliability between varying span lengths and load effects. Most of the factored rating vehicles produced a fairly uniform level of reliability.
- The Oregon-specific live load factors applied to the rating vehicles adequately enveloped the load effects produced by the WIM data. Some of the rating vehicles that are in current use do not quite produce the same

level of demand compared to some WIM vehicles observed on Oregon's state-owned highways. However, the ratios of the rating vehicle load effect to the WIM vehicle load effect that were below 1.0 were reasonably close to 1.0. Considering the level of uncertainty in WIM axle weight measurements, as well as the calibration process, this difference was minor.

- The Type 3 Legal vehicle could be eliminated from the suite of rating vehicles. Additional research should be conducted to further support this recommendation, as stated in subsequent bullets.
- No immediate changes, such as increases in axle weights or reduction of axle spacing lengths, are necessary for the suite of ODOT rating vehicles.
- The use of the NRL vehicles to represent Table 1 vehicle classification in Oregon is redundant, and need not be incorporated into to the suite of rating vehicles. Further, the NRLs provided nonuniform levels of reliability compared with the current Table 1 representative vehicles.
- Only one WIM site was considered in this study for comparison of load effects. Additional analyses should be conducted for other routes in Oregon, with varying ADTT, directionality, and freight corridors taken into account.
- Additional span types and lengths should be analyzed. This may include three-span, four-span, and five-span continuous models with varying span lengths.
- Load effects at the girder level should be calculated and compared for both the WIM data and the rating vehicles using girder distribution factors.

- The factored HL-93 loading (at the operating level) was found to adequately envelope most Table X loading scenarios.

| (834) LANE A CLASS 11 GVW 75.4 kips LENGTH 67 ft | | | | | | |
|---|--------------------|-------------------|--------------------|--------------------|---------------------|--|
| ESAL 3.221 SPEED 61 mph MAX GVW 80.0 kips wed Jun 16 00:04:52.40 2004 | | | | | | |
| AXLE | SEPARATION (ft) | LEFT WT (kips) | RIGHT WT (kips) | TOTAL WT (kips) | ALLOWABLE (kips) | |
| 1 | | 5.5 | 5.5 | 11.1 | 13.2 | |
| 2 | 16.7 | 8.4 | 7.6 | 16.0 | 17.0 | |
| 3 | 4.6 | 7.9 | 7.7 | 15.6 | 17.0 | |
| 4 | 32.4 | 8.5 | 7.5 | 16.0 | 17.0 | |
| 5 | 4.1 | 9.1 | 7.6 | 16.7 | 17.0 | |
| [OFF] | | | | | | |

Figure 3.1: Example of raw WIM output.

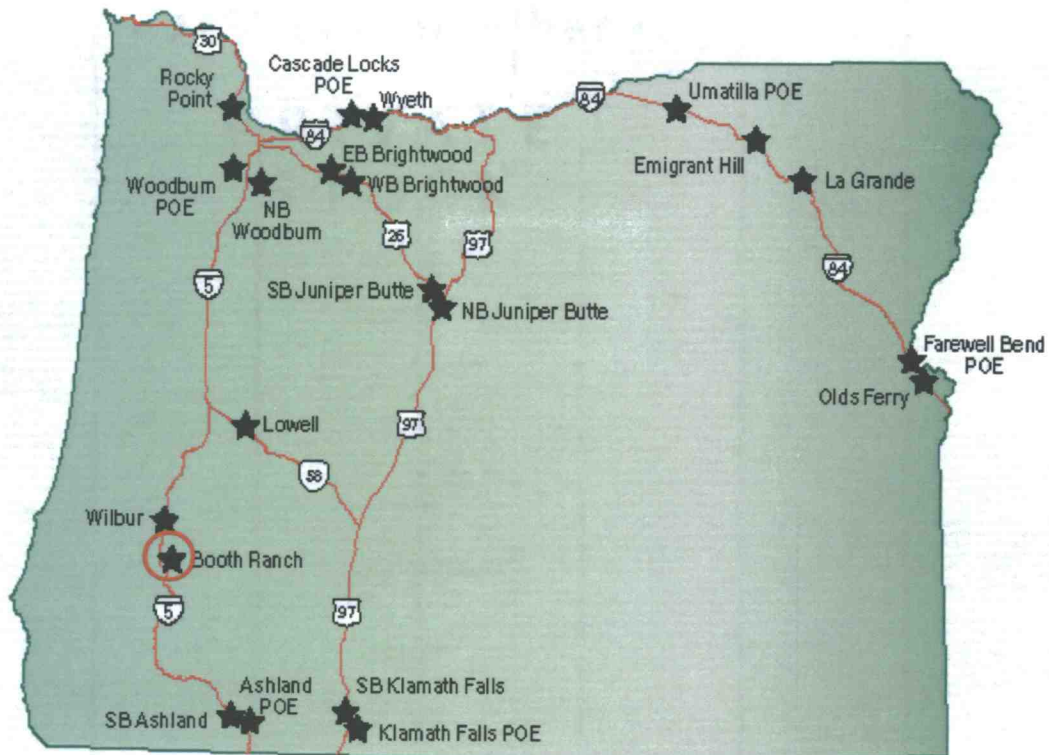


Figure 3.2: Location of I-5 Booth Ranch NB.

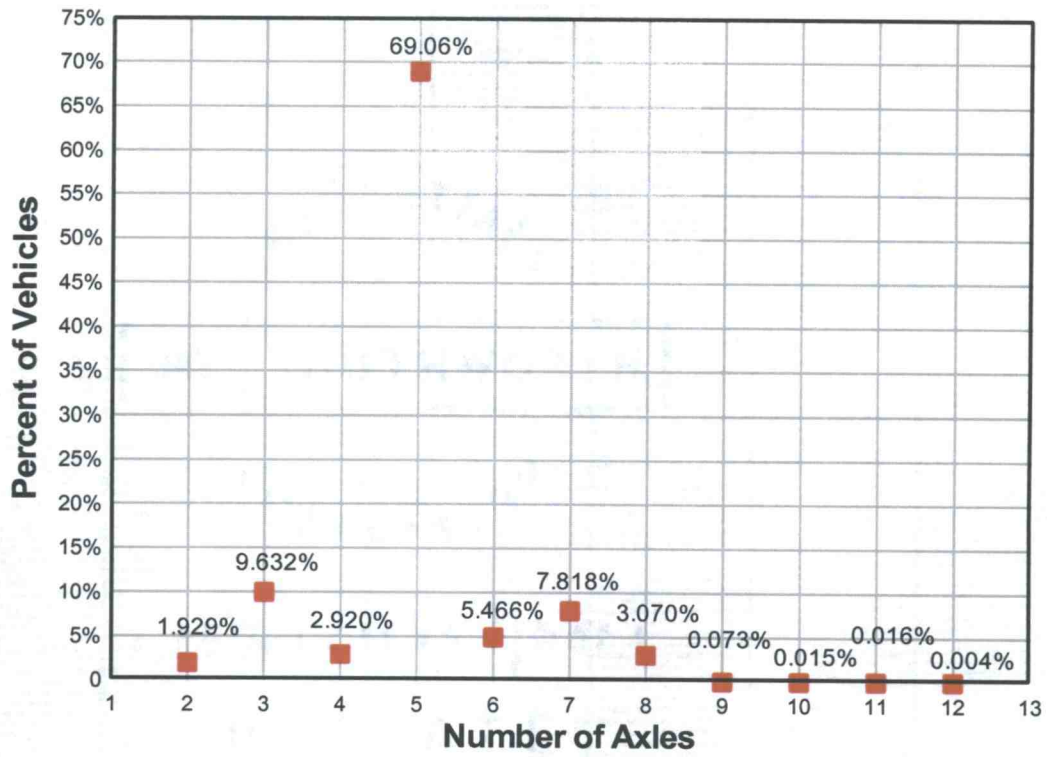


Figure 3.3: Frequency histogram of the number of axles per vehicle.

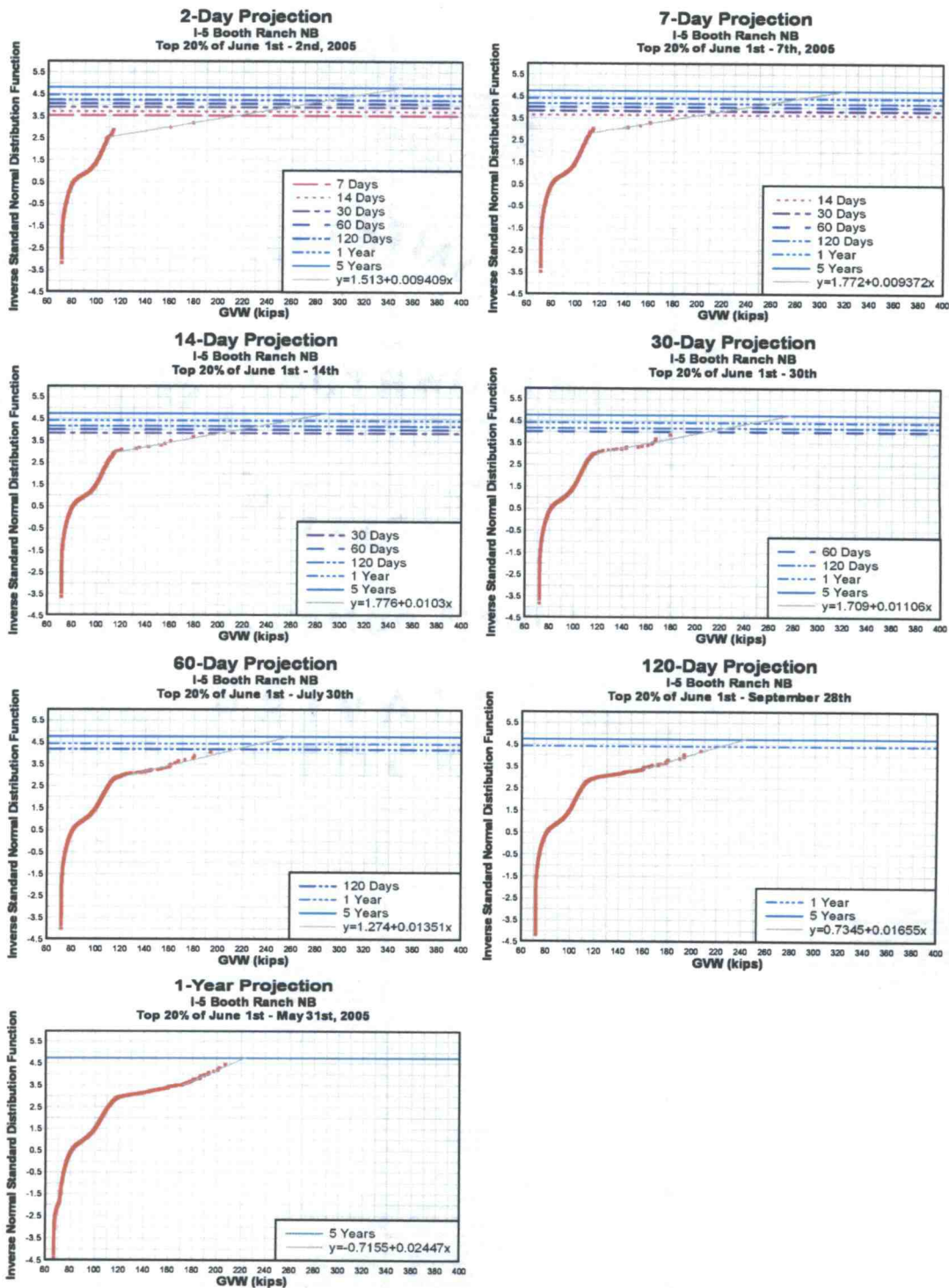


Figure 3.4: Projection plots for various time windows for I-5 Booth Ranch NB.

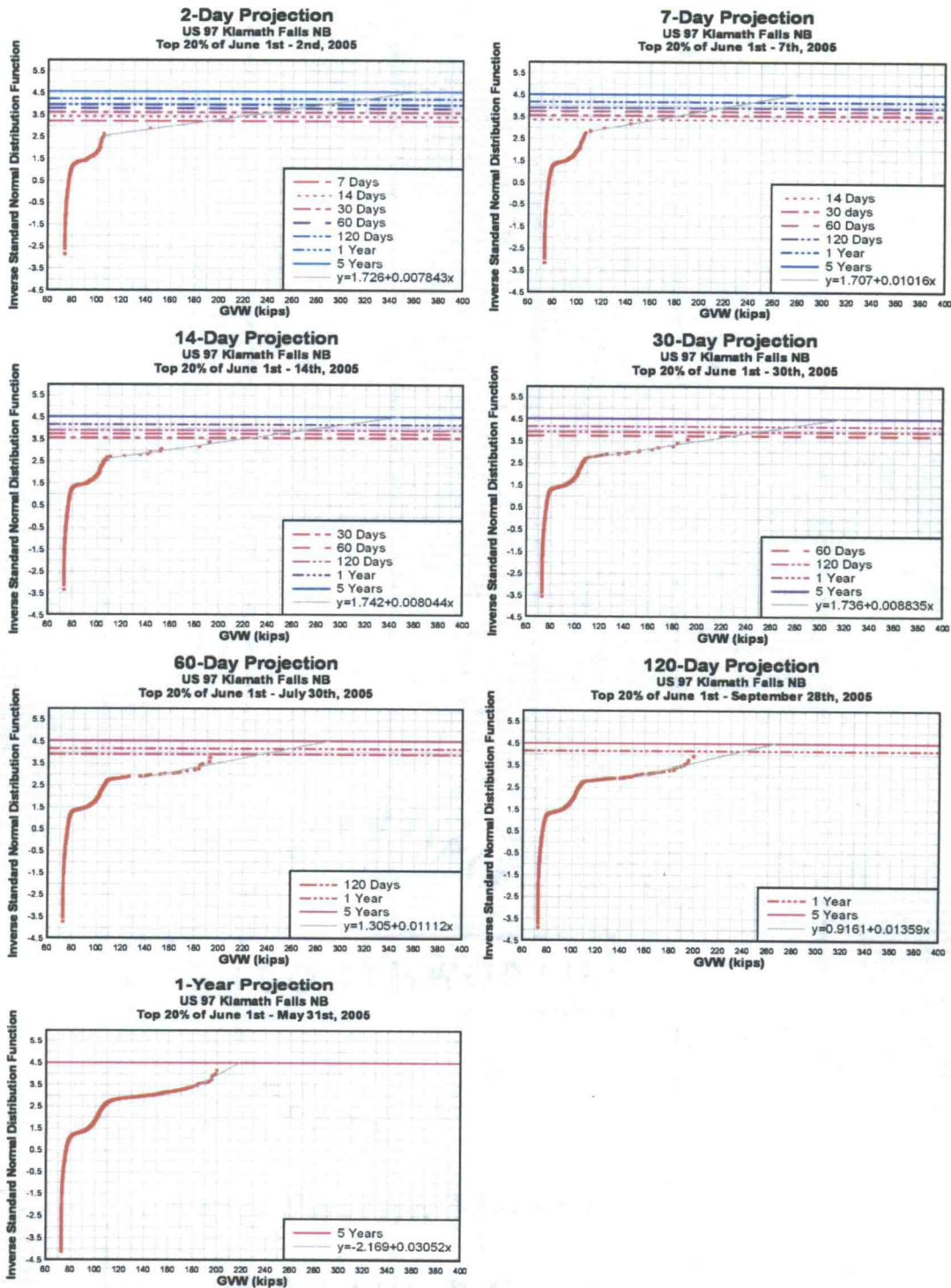


Figure 3.5: Projection plots for various time windows for US 97 Klamath Falls NB.

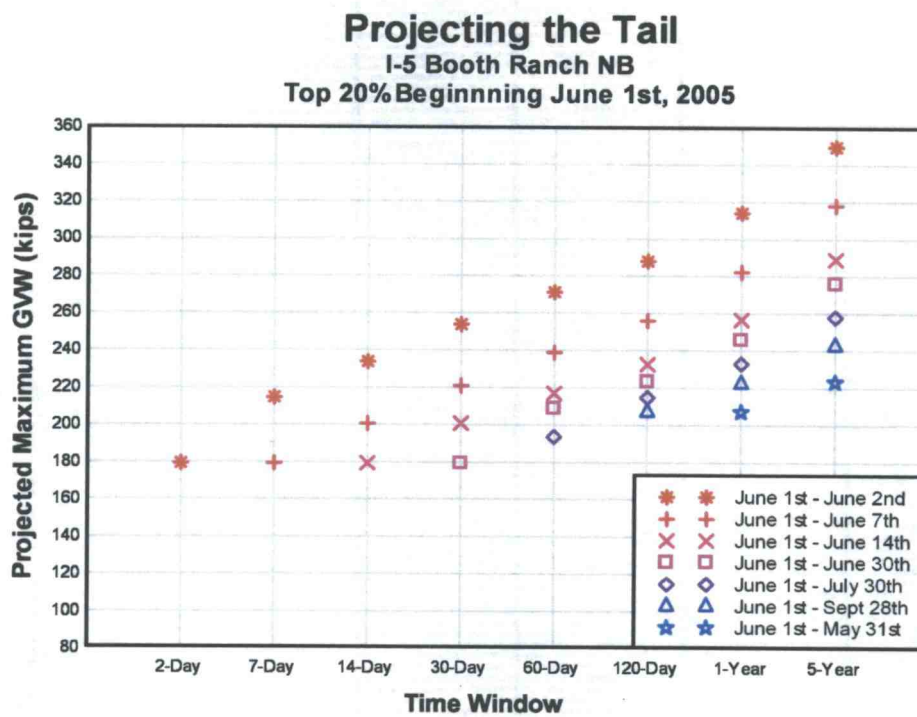


Figure 3.6: Projected GW values for all time windows for I-5 Booth Ranch NB.

Projecting the Tail
US 97 Klamath Falls NB
Top 20% Beginning June 1st, 2005

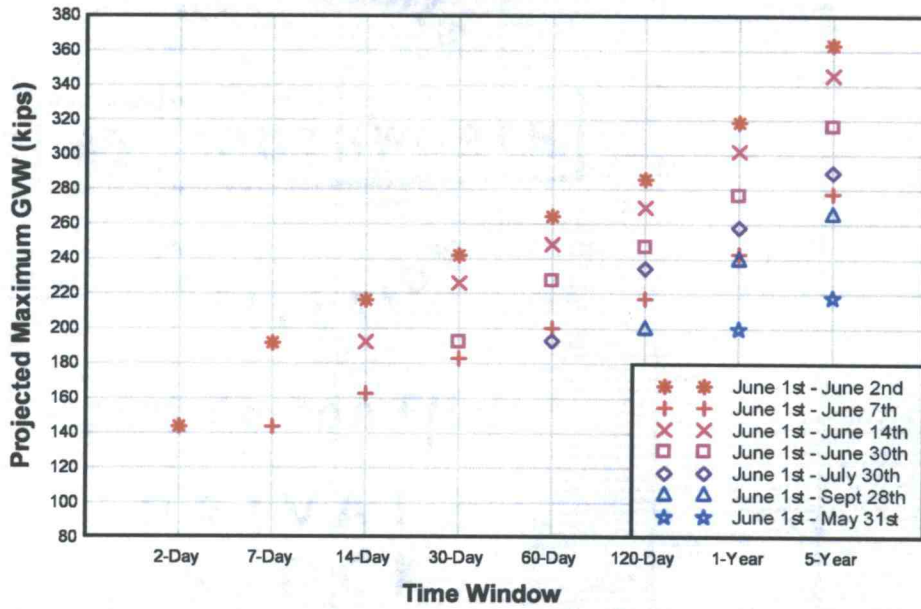


Figure 3.7: Projected GWW values for all time windows for US 97 Klamath Falls NB.

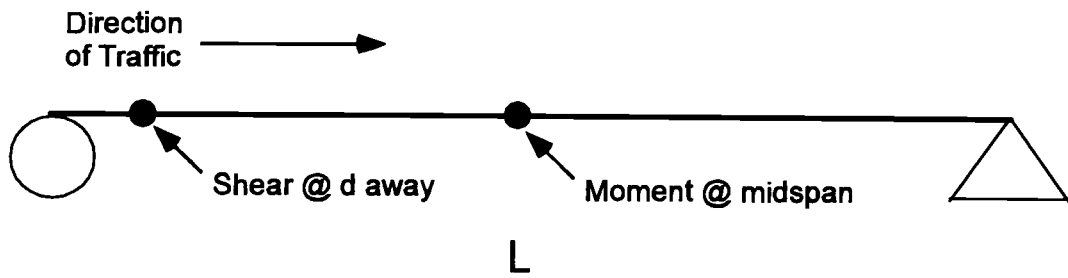


Figure 3.8: Shear & moment locations for simple span analysis.

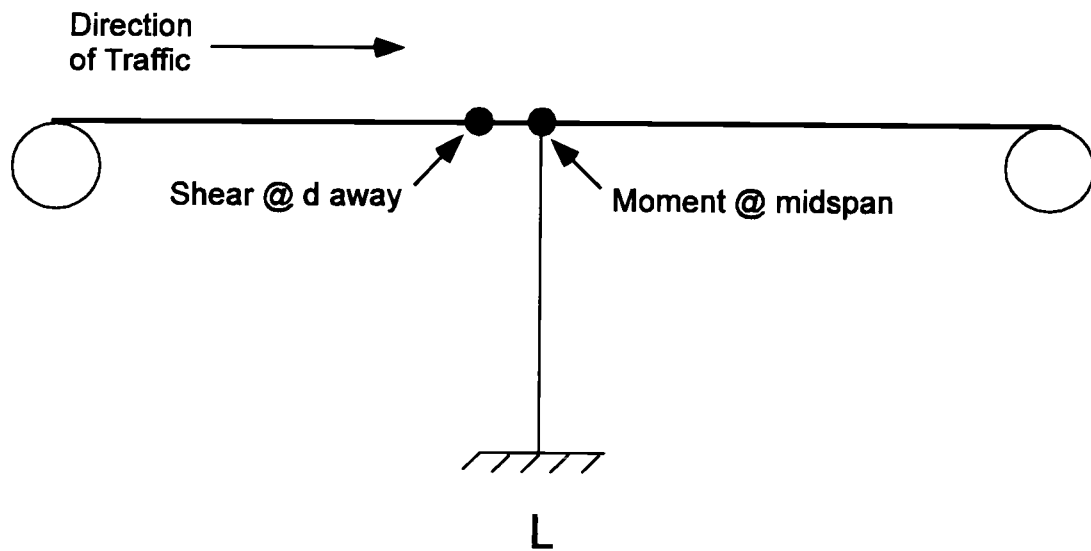


Figure 3.9: Shear & moment locations for 2-span continuous bridge analysis.

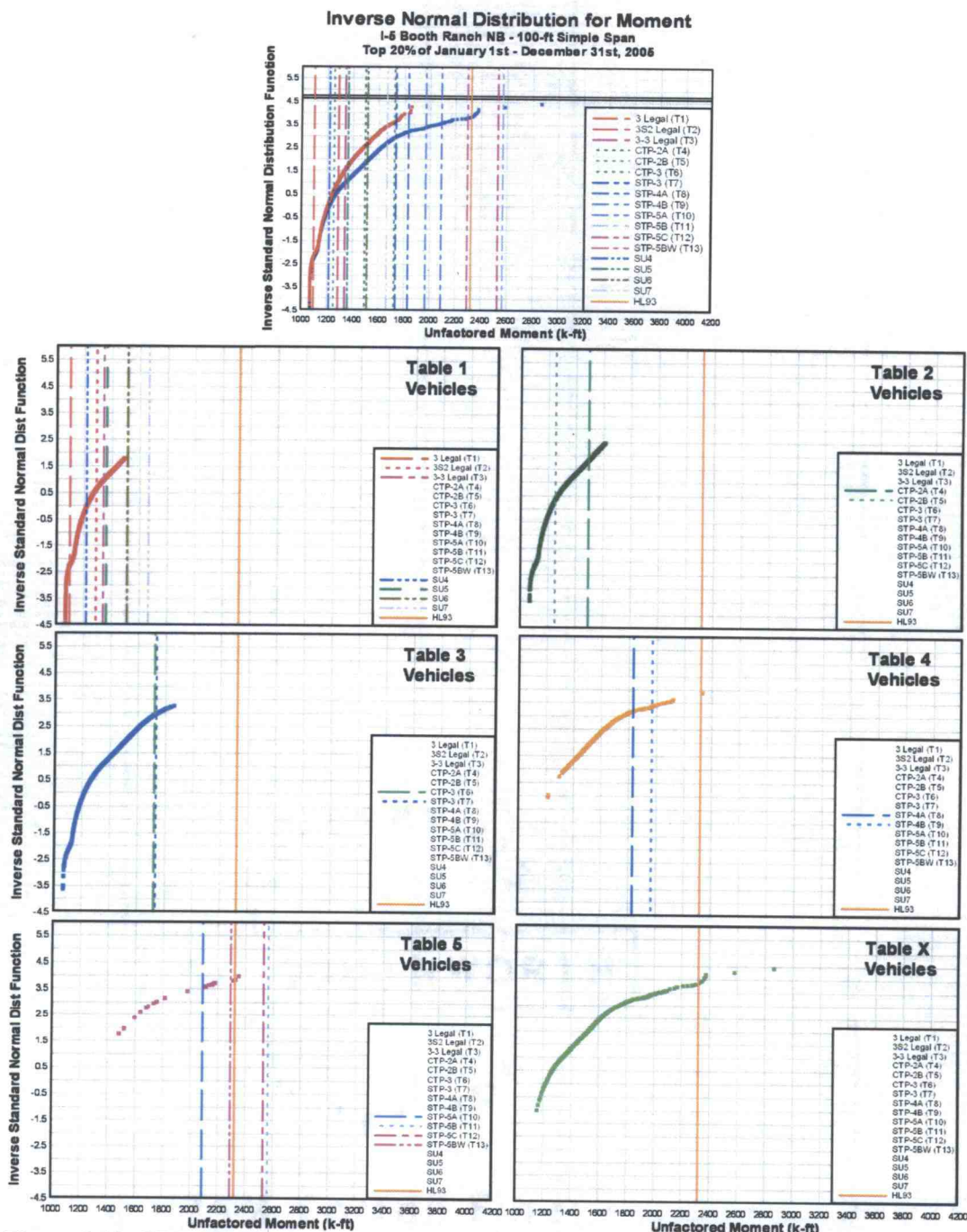


Figure 3.10: CDF plots for unfactored moment for 100-ft simple span bridge model.

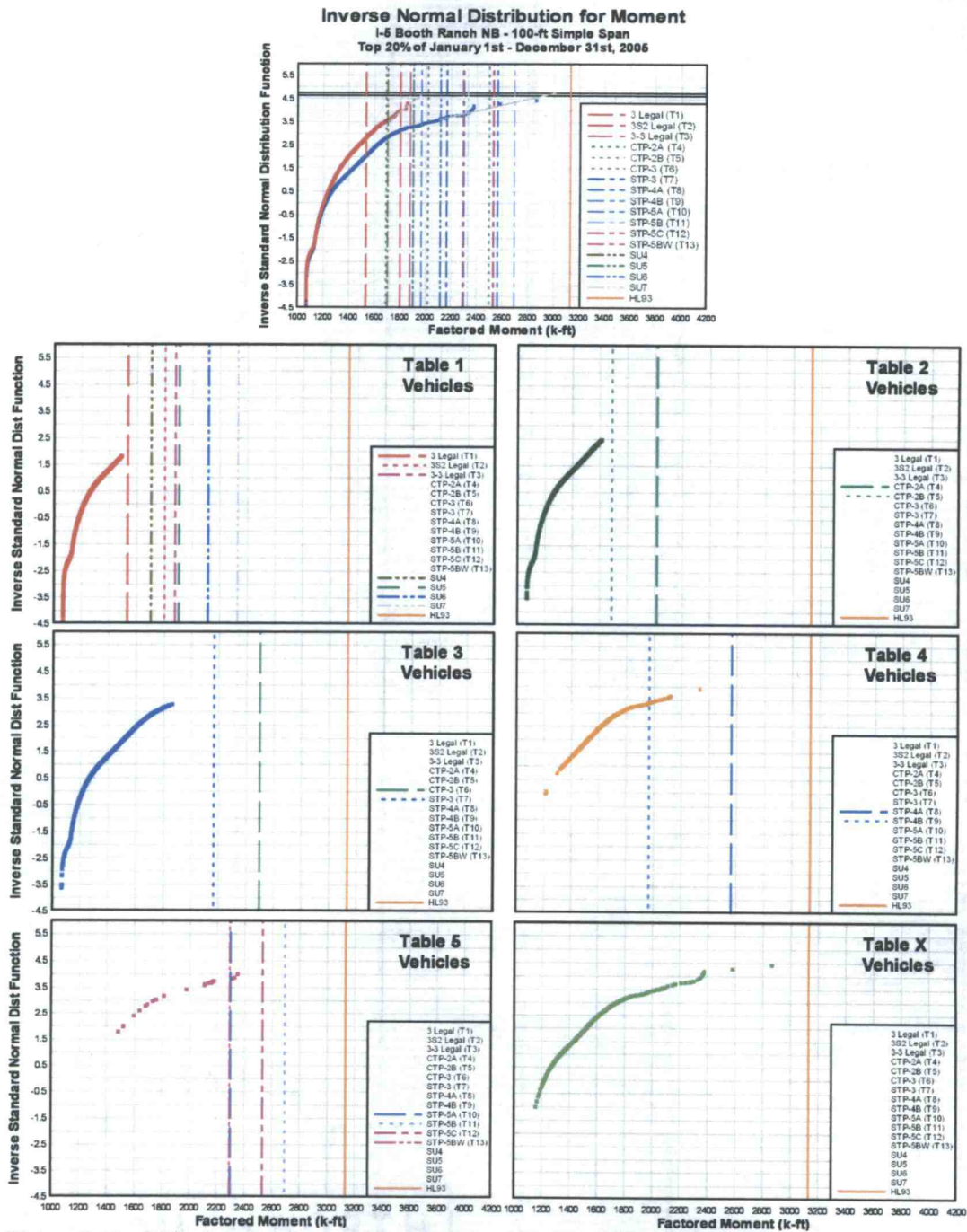


Figure 3.11: CDF plots for factored moment for 100-ft simple span bridge model.

Table 3.1: Current ODOT rating vehicles.

| Load Group | Rating Vehicle | OSU Designation | # Axles | Length (ft) | GVW (kips) | Representative of MCTD Weight Table: |
|-------------------------|----------------|-----------------|---------|-------------|------------|--------------------------------------|
| Legal Loads | Legal Type 3 | T1 | 3 | 19 | 50 | 1 |
| | Legal Type 3S2 | T2 | 5 | 51 | 80 | 1 |
| | Legal Type 3-3 | T3 | 6 | 54 | 80 | 1 |
| Continuous Trip Permits | OR-CTP-2A | T4 | 8 | 82 | 105.5 | 2 |
| | OR-CTP-2B | T5 | 8 | 75.5 | 105.5 | 2 |
| | OR-CTP-3 | T6 | 5 | 43 | 98 | 3 |
| Single Trip Permits | OR-STP-3 | T7 | 6 | 70 | 120.5 | 3 |
| | OR-STP-4A | T8 | 5 | 39 | 99 | 4 |
| | OR-STP-4B | T9 | 9 | 100 | 185 | 4 |
| | OR-STP-5A | T10 | 8 | 73.5 | 150.5 | 5 |
| | OR-STP-5B | T11 | 8 | 65 | 162.5 | 5 |
| | OR-STP-5C | T12 | 13 | 126 | 258 | 5 |
| | OR-STP-5BW | T13 | 9 | 99 | 204 | 5 |

Table 3.2: Information for I-5 Booth Ranch NB.

| | |
|--------------------------|------------------------|
| Location (MP) | 111.07 |
| ADT | 12,619 |
| ADTT | 3,442 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Aug 05 |
| Calibration Interval | 6 mths. (or as needed) |

Table 3.3: Number of axles per vehicle per month.

| Month | Number of axles | | | | | | | | | | | Total |
|--------------|-----------------|--------------|--------------|---------------|--------------|--------------|--------------|------------|------------|------------|-----------|---------------|
| | 2 | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 | 12 | |
| January | 1135 | 7332 | 1663 | 54018 | 4286 | 5444 | 2466 | 38 | 15 | 10 | 1 | 76408 |
| February | 1216 | 7474 | 2003 | 52280 | 4267 | 5498 | 2490 | 44 | 11 | 9 | 1 | 75293 |
| March | 1652 | 7999 | 3074 | 60582 | 4878 | 6595 | 2950 | 53 | 6 | 6 | 1 | 87796 |
| April | 1631 | 7209 | 3084 | 57635 | 4673 | 6162 | 2875 | 53 | 15 | 8 | 10 | 83355 |
| May | 1714 | 7450 | 2499 | 56393 | 4519 | 6262 | 2545 | 56 | 10 | 14 | 2 | 81464 |
| June | 1926 | 8509 | 2967 | 58283 | 4693 | 6835 | 2703 | 63 | 12 | 14 | 1 | 86006 |
| July | 1949 | 9330 | 3090 | 55195 | 4392 | 6765 | 2327 | 64 | 10 | 19 | 3 | 83144 |
| August | 1730 | 9562 | 2698 | 57125 | 4572 | 7155 | 2726 | 74 | 14 | 18 | 5 | 85679 |
| September | 1687 | 8737 | 2223 | 54232 | 4228 | 6788 | 2442 | 78 | 20 | 16 | 2 | 80453 |
| October | 1625 | 7412 | 1991 | 57006 | 4472 | 6761 | 2306 | 69 | 16 | 19 | 5 | 81682 |
| November | 1342 | 7094 | 1739 | 56237 | 4241 | 6281 | 2230 | 57 | 7 | 12 | 3 | 79243 |
| December | 1325 | 6404 | 1621 | 58617 | 4415 | 6167 | 2065 | 66 | 11 | 10 | 2 | 80703 |
| Total | 18932 | 94512 | 28652 | 677603 | 53636 | 76713 | 30125 | 715 | 147 | 155 | 36 | 981226 |

Table 3.4: Comparing sorting methods for table classification at I-5 Booth Ranch NB for 2005.

| Month | Sort Method | Table 1 | Table 2 | Table 3 | Table 4 | Table 5 | Table X | Total # |
|-----------|-------------------|---------|---------|---------|---------|---------|---------|---------|
| January | Conventional Sort | 69731 | 2553 | 3993 | 70 | 2 | 59 | 76408 |
| | Modified Sort | 71370 | 3701 | 1331 | 3 | 2 | 1 | |
| February | Conventional Sort | 67714 | 3081 | 4403 | 49 | 2 | 44 | 75293 |
| | Modified Sort | 70201 | 4094 | 992 | 6 | 0 | 0 | |
| March | Conventional Sort | 76981 | 3799 | 6849 | 83 | 0 | 84 | 87796 |
| | Modified Sort | 80825 | 5524 | 1438 | 7 | 1 | 1 | |
| April | Conventional Sort | 70033 | 3463 | 9564 | 135 | 5 | 155 | 83355 |
| | Modified Sort | 73958 | 6351 | 3028 | 14 | 2 | 2 | |
| May | Conventional Sort | 71914 | 3497 | 5894 | 87 | 2 | 70 | 81464 |
| | Modified Sort | 75156 | 4991 | 1310 | 6 | 1 | 0 | |
| June | Conventional Sort | 76148 | 3676 | 6030 | 72 | 1 | 79 | 86006 |
| | Modified Sort | 79577 | 5211 | 1212 | 6 | 0 | 0 | |
| July | Conventional Sort | 74208 | 3338 | 5462 | 68 | 1 | 67 | 83144 |
| | Modified Sort | 77422 | 4726 | 987 | 6 | 3 | 0 | |
| August | Conventional Sort | 76208 | 3909 | 5414 | 81 | 3 | 64 | 85679 |
| | Modified Sort | 79343 | 5330 | 997 | 7 | 2 | 0 | |
| September | Conventional Sort | 71884 | 3801 | 4681 | 39 | 2 | 46 | 80453 |
| | Modified Sort | 74676 | 5025 | 744 | 6 | 2 | 0 | |
| October | Conventional Sort | 73326 | 3590 | 4648 | 44 | 4 | 70 | 81682 |
| | Modified Sort | 76133 | 4738 | 799 | 10 | 2 | 0 | |
| November | Conventional Sort | 70988 | 3373 | 4760 | 58 | 2 | 62 | 79243 |
| | Modified Sort | 73735 | 4601 | 891 | 14 | 1 | 1 | |
| December | Conventional Sort | 72429 | 3204 | 4942 | 64 | 0 | 64 | 80703 |
| | Modified Sort | 75411 | 4412 | 872 | 6 | 2 | 0 | |

Table 3.5: Maximum projected GWW for varying time windows for I-5 Booth Ranch NB.

| Projection Time Window | Maximum Projected GWW (k) | | | | | | | | | | | | | |
|------------------------|---------------------------|----------|---------------------|----------|----------------------|----------|----------------------|----------|----------------------|----------|----------------------|----------|---------------------|--|
| | 2 Days of Data | % Change | 7 Days of Data | % Change | 14 Days of Data | % Change | 30 Days of Data | % Change | 60 Days of Data | % Change | 120 Days of Data | % Change | 1 Year of Data | |
| | June 1st - June 2nd | | June 1st - June 7th | | June 1st - June 14th | | June 1st - June 30th | | June 1st - July 30th | | June 1st - Sept 28th | | June 1st - May 31st | |
| 2-Day | 179.3 | | | | | | | | | | | | | |
| 7-Day | 214.7 | -16% | 179.3 | | | | | | | | | | | |
| 14-Day | 233.8 | -14% | 200.7 | -11% | 179.3 | | | | | | | | | |
| 30-Day | 253.8 | -13% | 221.1 | -9% | 200.9 | -11% | 179.3 | | | | | | | |
| 60-Day | 271.3 | -12% | 238.9 | -9% | 217.1 | -4% | 209.0 | -7% | 193.7 | | | | | |
| 120-Day | 288.2 | -11% | 256.0 | -9% | 232.7 | -4% | 223.5 | -4% | 214.9 | -4% | 207.3 | | | |
| 1-Year | 314.1 | -10% | 282.4 | -9% | 256.6 | -4% | 245.7 | -5% | 233.1 | -5% | 222.5 | -7% | 207.3 | |
| 5-Year | 349.5 | -9% | 318.2 | -9% | 289.3 | -5% | 276.1 | -7% | 258.0 | -6% | 242.8 | -8% | 223.3 | |

*GWW values are calculated using the equation generated from D-plot's line fit.

Equations From D-Plot (all equations are first order equations).

| | | |
|---------|----------------------|--------------------------|
| 2-Day | $y=1.513+0.009409x$ | $x=(y-1.513)/(0.009409)$ |
| 7-Day | $y=1.772+0.009372x$ | $x=(y-1.772)/(0.009372)$ |
| 14-Day | $y=1.776+0.0103x$ | $x=(y-1.776)/(0.0103)$ |
| 30-Day | $y=1.709+0.01106x$ | $x=(y-1.709)/(0.01106)$ |
| 60-Day | $y=1.274+0.01351x$ | $x=(y-1.274)/(0.01351)$ |
| 120-Day | $y=0.7345+0.01655x$ | $x=(y-0.7345)/(0.01655)$ |
| 1-Year | $y=-0.7155+0.02447x$ | $x=(y+0.7155)/(0.02447)$ |

Table 3.6: Statistical parameters for varying time windows for I-5 Booth Ranch NB.

| | Projection Time Window | | | | | | |
|--------------------|------------------------|---------------------|----------------------|----------------------|----------------------|----------------------|---------------------|
| | 2 Days of Data | 7 Days of Data | 14 Days of Data | 30 Days of Data | 60 Days of Data | 120 Days of Data | 1 Year of Data |
| | June 1st - June 2nd | June 1st - June 7th | June 1st - June 14th | June 1st - June 30th | June 1st - July 30th | June 1st - Sept 28th | June 1st - May 31st |
| Number | 1392 | 3857 | 7753 | 17202 | 33831 | 65531 | 196246 |
| Mean | 81.4 | 80.8 | 81.0 | 81.5 | 81.1 | 81.3 | 81.3 |
| St Dev | 10.16 | 9.63 | 9.63 | 9.63 | 9.54 | 9.63 | 9.91 |
| COV | 12% | 12% | 12% | 12% | 12% | 12% | 12% |
| y-intercept | 1.51 | 1.77 | 1.78 | 1.71 | 1.27 | 0.73 | -0.72 |
| Slope | 0.0094 | 0.0094 | 0.0103 | 0.0111 | 0.0135 | 0.0166 | 0.0245 |

Table 3.7: Maximum projected GVW for varying time windows for US 97 Klamath Falls NB.

| Projection Time Window | Maximum Projected GVW (k) | | | | | | | | | | | | | |
|------------------------|---------------------------|----------|---------------------|----------|----------------------|----------|----------------------|----------|----------------------|----------|----------------------|----------|---------------------|--|
| | 2 Days of Data | % Change | 7 Days of Data | % Change | 14 Days of Data | % Change | 30 Days of Data | % Change | 60 Days of Data | % Change | 120 Days of Data | % Change | 1 Year of Data | |
| | June 1st - June 2nd | | June 1st - June 7th | | June 1st - June 14th | | June 1st - June 30th | | June 1st - July 30th | | June 1st - Sept 28th | | June 1st - May 31st | |
| 2-Day | 143.7 | | | | | | | | | | | | | |
| 7-Day | 191.7 | -25% | 143.7 | | | | | | | | | | | |
| 14-Day | 216.3 | -25% | 162.8 | 18% | 192.4 | | | | | | | | | |
| 30-Day | 242.0 | -24% | 182.9 | 24% | 226.2 | -15% | 192.4 | | | | | | | |
| 60-Day | 264.4 | -24% | 200.4 | 24% | 248.4 | -8% | 227.7 | -15% | 193 | | | | | |
| 120-Day | 285.9 | -24% | 217.2 | 24% | 269.6 | -8% | 247.0 | -5% | 234.7 | -15% | 199.9 | | | |
| 1-Year | 318.7 | -24% | 242.9 | 24% | 302.0 | -8% | 276.5 | -7% | 258.1 | -7% | 239.3 | -16% | 199.9 | |
| 5-Year | 363.2 | -24% | 277.6 | 25% | 345.9 | -9% | 316.4 | -8% | 289.8 | -8% | 265.3 | -18% | 218.0 | |

*GVW values are calculated using the equation generated from D-plot's line fit.

Equations From D-Plot (all equations are first order equations).

| | | |
|---------|---------------------|--------------------------|
| 2-Day | $y=1.726+0.007843x$ | $x=(y-1.726)/(0.007843)$ |
| 7-Day | $y=1.707+0.01016x$ | $x=(y-1.707)/(0.01016)$ |
| 14-Day | $y=1.742+0.008044x$ | $x=(y-1.742)/(0.008044)$ |
| 30-Day | $y=1.736+0.008835x$ | $x=(y-1.736)/(0.008835)$ |
| 60-Day | $y=1.305+0.01112x$ | $x=(y-1.305)/(0.01112)$ |
| 120-Day | $y=0.9161+0.01359x$ | $x=(y-0.9161)/(0.01359)$ |
| 1-Year | $y=-2.169+0.03052x$ | $x=(y+2.169)/(0.03052)$ |

Table 3.8: Statistical parameters for varying time windows for US 97 Klamath Falls NB.

| | Projection Time Window | | | | | | |
|--------------------|------------------------|---------------------|----------------------|----------------------|----------------------|----------------------|---------------------|
| | 2 Days of Data | 7 Days of Data | 14 Days of Data | 30 Days of Data | 60 Days of Data | 120 Days of Data | 1 Year of Data |
| | June 1st - June 2nd | June 1st - June 7th | June 1st - June 14th | June 1st - June 30th | June 1st - July 30th | June 1st - Sept 28th | June 1st - May 31st |
| Number | 460 | 1286 | 2535 | 5608 | 11032 | 21428 | 54986 |
| Mean | 78.3 | 77.8 | 77.8 | 77.7 | 77.5 | 77.7 | 78.2 |
| St Dev | 7.0 | 6.5 | 7.7 | 7.5 | 7.7 | 7.7 | 8.1 |
| COV | 9% | 8% | 10% | 10% | 10% | 10% | 10% |
| y-intercept | 1.73 | 1.71 | 1.74 | 1.74 | 1.31 | 0.92 | -2.17 |
| Slope | 0.0078 | 0.0102 | 0.0080 | 0.0088 | 0.0111 | 0.0136 | 0.0305 |

Table 3.9: Five-year extrapolated load effects for various span types and lengths.

| | Span | Actual Load Effect | 5-YR Projected Load Effect | Factored HL-93 | Ratio |
|-----------------------------|--------------------|--------------------|----------------------------|----------------|-------|
| Shear (k) - Simple Span | 50-ft | 79 | 83 | 89 | 1.08 |
| | 100-ft | 109 | 121 | 124 | 1.03 |
| | 150-ft | 134 | 148 | 150 | 1.01 |
| | 200-ft | 151 | 164 | 174 | 1.06 |
| Moment (k-ft) - Simple Span | 50-ft | 1019 | 1180 | 1107 | 0.94 |
| | 100-ft | 2869 | 3007 | 3132 | 1.04 |
| | 150-ft | 4816 | 5391 | 5697 | 1.06 |
| | 200-ft | 7332 | 8112 | 8801 | 1.08 |
| Cont Span | Shear - 50-ft | 91 | 102 | 101 | 0.99 |
| | Neg Moment - 50-ft | 744 | 813 | 702 | 0.86 |

Inverse Standard Normal Distribution Function

| | Days | # Vehicles | Probability | Inv Nor |
|-----|------|------------|-------------|--------------|
| All | 365 | 196247 | 5.096E-06 | 4.413 |
| | 1825 | 981235 | 1.019E-06 | 4.750 |
| 3S2 | 365 | 111314 | 8.984E-06 | 4.289 |
| | 1825 | 556570 | 1.797E-06 | 4.634 |

Table 3.10: Comparison of shear effects between rating and WIM vehicles.

| | Type | γ_L | 50-ft Span | | | | | 100-ft Span | | | | | 150-ft Span | | | | | 200-ft Span | | | | |
|--------------|-----------|------------|------------|---------------------|-------|-------|------|-------------|---------------------|-------|-------|------|-------------|---------------------|-------|-------|------|-------------|---------------------|-------|-------|------|
| | | | V | $\gamma_L \times V$ | WIM V | Ratio | %3S2 | V | $\gamma_L \times V$ | WIM V | Ratio | %3S2 | V | $\gamma_L \times V$ | WIM V | Ratio | %3S2 | V | $\gamma_L \times V$ | WIM V | Ratio | %3S2 |
| Tab 1 | 3 Legal | 1.40 | 38.6 | 54.0 | 44.1 | 1.22 | 26% | 44.2 | 61.9 | 59.6 | 1.04 | 4% | 46.2 | 64.7 | 66.3 | 0.98 | -2% | 47.1 | 66.0 | 69.6 | 0.95 | -5% |
| | 3S2 Legal | 1.40 | 37.5 | 52.5 | 44.1 | 1.19 | 22% | 56.7 | 79.4 | 59.6 | 1.33 | 35% | 64.4 | 90.2 | 66.3 | 1.36 | 37% | 68.4 | 95.7 | 69.6 | 1.37 | 38% |
| | 3-3 Legal | 1.40 | 37.2 | 52.0 | 44.1 | 1.18 | 21% | 57.6 | 80.6 | 59.6 | 1.35 | 37% | 65.0 | 91.0 | 66.3 | 1.37 | 38% | 68.8 | 96.3 | 69.6 | 1.38 | 39% |
| Tab 2 | CTP-2A | 1.35 | 39.7 | 53.5 | 44.0 | 1.22 | 25% | 60.9 | 82.1 | 67.8 | 1.21 | 25% | 75.3 | 101.7 | 80.2 | 1.27 | 33% | 82.5 | 111.3 | 86.5 | 1.29 | 36% |
| | CTP-2B | 1.35 | 39.0 | 52.7 | 44.0 | 1.20 | 23% | 63.2 | 85.4 | 67.8 | 1.26 | 31% | 75.8 | 102.4 | 80.2 | 1.28 | 34% | 82.0 | 110.8 | 86.5 | 1.28 | 36% |
| Tab 3 | CTP-3 | 1.45 | 51.1 | 74.1 | 54.7 | 1.36 | 52% | 73.5 | 106.5 | 83.5 | 1.28 | 41% | 81.0 | 117.4 | 107.1 | 1.10 | 16% | 84.7 | 122.8 | 120.7 | 1.02 | 3% |
| | STP-3 | 1.25 | 46.2 | 57.7 | 54.7 | 1.06 | 8% | 76.1 | 95.1 | 83.5 | 1.14 | 20% | 90.1 | 112.6 | 107.1 | 1.05 | 9% | 96.9 | 121.2 | 120.7 | 1.00 | 1% |
| Tab 4 | STP-4A | 1.40 | 55.8 | 78.1 | 64.0 | 1.22 | 38% | 76.3 | 106.8 | 94.3 | 1.13 | 22% | 83.2 | 116.5 | 124.5 | 0.94 | -12% | 86.6 | 121.3 | 140.8 | 0.86 | -29% |
| | STP-4B | 1.00 | 60.4 | 60.4 | 64.0 | 0.94 | -10% | 93.1 | 93.1 | 94.3 | 0.99 | -2% | 122.3 | 122.3 | 124.5 | 0.98 | -3% | 137.0 | 137.0 | 140.8 | 0.97 | -6% |
| Tab 5 | STP-5A | 1.10 | 54.8 | 60.3 | 66.3 | 0.91 | -16% | 91.7 | 100.8 | 99.6 | 1.01 | 2% | 109.5 | 120.4 | 133.8 | 0.90 | -21% | 118.9 | 130.8 | 152.1 | 0.86 | -31% |
| | STP-5B | 1.05 | 62.9 | 66.0 | 66.3 | 0.99 | -1% | 105.7 | 111.0 | 99.6 | 1.11 | 20% | 123.4 | 129.6 | 133.8 | 0.97 | -7% | 132.2 | 138.8 | 152.1 | 0.91 | -19% |
| | STP-5C | 1.00 | 69.0 | 69.0 | 66.3 | 1.04 | 7% | 103.6 | 103.6 | 99.6 | 1.04 | 7% | 144.6 | 144.6 | 133.8 | 1.08 | 17% | 172.9 | 172.9 | 152.1 | 1.14 | 30% |
| | STP-5BW | 1.00 | 66.9 | 66.9 | 66.3 | 1.01 | 1% | 106.4 | 106.4 | 99.6 | 1.07 | 12% | 138.8 | 138.8 | 133.8 | 1.04 | 8% | 155.1 | 155.1 | 152.1 | 1.02 | 4% |
| Tab x | HL93 | 1.35 | 66.2 | 89.4 | 78.9 | 1.13 | 28% | 91.8 | 124.0 | 109.4 | 1.13 | 26% | 111.0 | 149.9 | 134.3 | 1.12 | 24% | 128.6 | 173.6 | 151.1 | 1.15 | 33% |
| NRL Vehicles | SU4 | 1.40 | 42.7 | 59.7 | 44.1 | 1.35 | 42% | 48.3 | 67.6 | 59.6 | 1.14 | 14% | 50.2 | 70.3 | 66.3 | 1.06 | 6% | 51.2 | 71.6 | 69.6 | 1.03 | 3% |
| | SU5 | 1.40 | 47.2 | 66.0 | 44.1 | 1.50 | 59% | 54.6 | 76.4 | 59.6 | 1.28 | 30% | 57.0 | 79.8 | 66.3 | 1.20 | 21% | 58.3 | 81.6 | 69.6 | 1.17 | 18% |
| | SU6 | 1.40 | 49.1 | 68.8 | 44.1 | 1.56 | 66% | 59.1 | 82.7 | 59.6 | 1.39 | 41% | 62.3 | 87.3 | 66.3 | 1.32 | 33% | 64.0 | 89.6 | 69.6 | 1.29 | 29% |
| | SU7 | 1.40 | 51.0 | 71.4 | 44.1 | 1.62 | 73% | 63.9 | 89.5 | 59.6 | 1.50 | 53% | 68.3 | 95.6 | 66.3 | 1.44 | 46% | 70.5 | 98.7 | 69.6 | 1.42 | 42% |

Table 3.11: Comparison of moment effects between rating and WIM vehicles.

| | Type | γ_L | 50-ft Span | | | | | 100-ft Span | | | | | 150-ft Span | | | | | 200-ft Span | | | | |
|--------------|-----------|------------|------------|---------------------|-------|-------|------|-------------|---------------------|-------|-------|------|-------------|---------------------|-------|-------|------|-------------|---------------------|-------|-------|------|
| | | | M | $\gamma_L \times M$ | WIM M | Ratio | %3S2 | M | $\gamma_L \times M$ | WIM M | Ratio | %3S2 | M | $\gamma_L \times M$ | WIM M | Ratio | %3S2 | M | $\gamma_L \times M$ | WIM M | Ratio | %3S2 |
| Tab 1 | 3 Legal | 1.40 | 471 | 659 | 544 | 1.21 | 24% | 1096 | 1534 | 1493 | 1.03 | 3% | 1721 | 2409 | 2491 | 0.97 | -4% | 2345 | 3283 | 3489 | 0.94 | -6% |
| | 3S2 Legal | 1.40 | 481 | 673 | 544 | 1.24 | 27% | 1286 | 1801 | 1493 | 1.21 | 24% | 2286 | 3201 | 2491 | 1.29 | 31% | 3287 | 4601 | 3489 | 1.32 | 34% |
| | 3-3 Legal | 1.40 | 394 | 551 | 544 | 1.01 | 2% | 1340 | 1876 | 1493 | 1.26 | 30% | 2339 | 3275 | 2491 | 1.31 | 34% | 3340 | 4676 | 3489 | 1.34 | 36% |
| Tab 2 | CTP-2A | 1.35 | 491 | 663 | 567 | 1.17 | 20% | 1494 | 2017 | 1617 | 1.25 | 31% | 2790 | 3766 | 2910 | 1.29 | 37% | 4090 | 5521 | 4226 | 1.31 | 39% |
| | CTP-2B | 1.35 | 504 | 680 | 567 | 1.20 | 23% | 1250 | 1688 | 1617 | 1.04 | 6% | 2513 | 3393 | 2910 | 1.17 | 21% | 3775 | 5097 | 4226 | 1.21 | 26% |
| Tab 3 | CTP-3 | 1.45 | 567 | 822 | 659 | 1.25 | 34% | 1722 | 2497 | 1866 | 1.34 | 49% | 2922 | 4237 | 3718 | 1.14 | 23% | 4122 | 5976 | 5723 | 1.04 | 8% |
| | STP-3 | 1.25 | 588 | 735 | 659 | 1.12 | 16% | 1732 | 2166 | 1866 | 1.16 | 23% | 3207 | 4009 | 3718 | 1.08 | 13% | 4682 | 5853 | 5723 | 1.02 | 4% |
| Tab 4 | STP-4A | 1.40 | 617 | 864 | 749 | 1.15 | 24% | 1829 | 2561 | 2337 | 1.10 | 17% | 3042 | 4259 | 4279 | 1.00 | -1% | 4254 | 5956 | 6536 | 0.91 | -18% |
| | STP-4B | 1.00 | 698 | 698 | 749 | 0.93 | -11% | 1966 | 1966 | 2337 | 0.84 | -29% | 4055 | 4055 | 4279 | 0.95 | -10% | 6318 | 6318 | 6536 | 0.97 | -7% |
| Tab 5 | STP-5A | 1.10 | 742 | 817 | 828 | 0.99 | -2% | 2088 | 2296 | 2355 | 0.98 | -5% | 3938 | 4331 | 4776 | 0.91 | -19% | 5788 | 6366 | 7197 | 0.88 | -25% |
| | STP-5B | 1.05 | 766 | 805 | 828 | 0.97 | -5% | 2565 | 2693 | 2355 | 1.14 | 26% | 4552 | 4779 | 4776 | 1.00 | 0% | 6540 | 6867 | 7197 | 0.95 | -10% |
| | STP-5C | 1.00 | 806 | 806 | 828 | 0.97 | -5% | 2530 | 2530 | 2355 | 1.07 | 14% | 4645 | 4645 | 4776 | 0.97 | -6% | 7795 | 7795 | 7197 | 1.08 | 18% |
| | STP-5BW | 1.00 | 773 | 773 | 828 | 0.93 | -11% | 2292 | 2292 | 2355 | 0.97 | -5% | 4691 | 4691 | 4776 | 0.98 | -4% | 7241 | 7241 | 7197 | 1.01 | 1% |
| Tab x | HL93 | 1.35 | 820 | 1107 | 1019 | 1.09 | 18% | 2320 | 3132 | 2869 | 1.09 | 20% | 4220 | 5697 | 4816 | 1.18 | 39% | 6520 | 8801 | 7332 | 1.20 | 45% |
| NRL Vehicles | SU4 | 1.40 | 541 | 757 | 544 | 1.39 | 44% | 1216 | 1702 | 1493 | 1.14 | 16% | 1891 | 2647 | 2491 | 1.06 | 7% | 2565 | 3592 | 3489 | 1.03 | 3% |
| | SU5 | 1.40 | 585 | 819 | 544 | 1.51 | 57% | 1360 | 1904 | 1493 | 1.28 | 32% | 2135 | 2989 | 2491 | 1.20 | 22% | 2910 | 4074 | 3489 | 1.17 | 18% |
| | SU6 | 1.40 | 649 | 909 | 544 | 1.67 | 76% | 1512 | 2117 | 1493 | 1.42 | 49% | 2374 | 3324 | 2491 | 1.33 | 36% | 3236 | 4531 | 3489 | 1.30 | 32% |
| | SU7 | 1.40 | 701 | 982 | 544 | 1.81 | 91% | 1664 | 2329 | 1493 | 1.56 | 65% | 2626 | 3677 | 2491 | 1.48 | 52% | 3588 | 5024 | 3489 | 1.44 | 47% |

Table 3.12: Comparison of 2-span continuous load effects between rating and WIM vehicles.

| | Type | γ_L | 2-Span Continuous - 50-ft Spans | | | | | | | | | |
|--------------|-----------|------------|---------------------------------|---------------------|-------|-------|------|------|---------------------|-------|-------|------|
| | | | M | $\gamma_L \times M$ | WIM M | Ratio | %3S2 | V | $\gamma_L \times V$ | WIM V | Ratio | %3S2 |
| Tab 1 | 3 Legal | 1.40 | 214 | 300 | 367 | 0.82 | -18% | 38.2 | 53.5 | 48.0 | 1.12 | 15% |
| | 3S2 Legal | 1.40 | 371 | 519 | 367 | 1.41 | 41% | 38.0 | 53.2 | 48.0 | 1.11 | 14% |
| | 3-3 Legal | 1.40 | 315 | 442 | 367 | 1.20 | 20% | 39.5 | 55.3 | 48.0 | 1.15 | 19% |
| Tab 2 | CTP-2A | 1.35 | 415 | 560 | 460 | 1.22 | 27% | 49.5 | 66.8 | 50.4 | 1.32 | 43% |
| | CTP-2B | 1.35 | 432 | 583 | 460 | 1.27 | 33% | 43.9 | 59.3 | 50.4 | 1.18 | 23% |
| Tab 3 | CTP-3 | 1.45 | 415 | 602 | 575 | 1.05 | 7% | 56.1 | 81.3 | 60.1 | 1.35 | 56% |
| | STP-3 | 1.25 | 505 | 631 | 575 | 1.10 | 15% | 56.3 | 70.4 | 60.1 | 1.17 | 27% |
| Tab 4 | STP-4A | 1.40 | 395 | 553 | 659 | 0.84 | -29% | 59.9 | 83.8 | 70.7 | 1.19 | 35% |
| | STP-4B | 1.00 | 628 | 628 | 659 | 0.95 | -8% | 67.5 | 67.5 | 70.7 | 0.96 | -8% |
| Tab 5 | STP-5A | 1.10 | 648 | 713 | 689 | 1.04 | 7% | 69.1 | 76.0 | 74.6 | 1.02 | 4% |
| | STP-5B | 1.05 | 623 | 655 | 689 | 0.95 | -9% | 79.2 | 83.1 | 74.6 | 1.11 | 22% |
| | STP-5C | 1.00 | 585 | 585 | 689 | 0.85 | -28% | 79.0 | 79.0 | 74.6 | 1.06 | 12% |
| | STP-5BW | 1.00 | 720 | 720 | 689 | 1.05 | 8% | 74.8 | 74.8 | 74.6 | 1.00 | 1% |
| Tab x | HL93 | 1.35 | 520 | 702 | 744 | 0.94 | -11% | 75.0 | 101.3 | 90.5 | 1.12 | 28% |
| NRL Vehicles | SU4 | 1.40 | 240 | 337 | 367 | 0.92 | -8% | 41.5 | 58.1 | 48.0 | 1.21 | 27% |
| | SU5 | 1.40 | 268 | 375 | 367 | 1.02 | 2% | 44.6 | 62.4 | 48.0 | 1.30 | 38% |
| | SU6 | 1.40 | 296 | 414 | 367 | 1.13 | 13% | 48.2 | 67.5 | 48.0 | 1.41 | 51% |
| | SU7 | 1.40 | 322 | 451 | 367 | 1.23 | 23% | 53.2 | 74.5 | 48.0 | 1.55 | 70% |

Table 3.13: Factored rating vehicle sufficiency.

| | Type | 50-ft Span | | 100-ft Span | | 150-ft Span | | 200-ft Span | | 2-Span Cont - 50-ft | |
|--------------|-----------|------------|----------|-------------|----------|-------------|----------|-------------|----------|---------------------|----------|
| | | r_{LV} | r_{LM} | r_{LV} | r_{LM} | r_{LV} | r_{LM} | r_{LV} | r_{LM} | r_{LV} | r_{LM} |
| Tab 1 | 3 Legal | 1.22 | 1.21 | 1.04 | 1.03 | 0.98 | 0.97 | 0.95 | 0.94 | 1.12 | 0.82 |
| | 3S2 Legal | 1.19 | 1.24 | 1.33 | 1.21 | 1.36 | 1.29 | 1.37 | 1.32 | 1.11 | 1.41 |
| | 3-3 Legal | 1.18 | 1.01 | 1.35 | 1.26 | 1.37 | 1.31 | 1.38 | 1.34 | 1.15 | 1.20 |
| Tab 2 | CTP-2A | 1.22 | 1.17 | 1.21 | 1.25 | 1.27 | 1.29 | 1.29 | 1.31 | 1.32 | 1.22 |
| | CTP-2B | 1.20 | 1.20 | 1.26 | 1.04 | 1.28 | 1.17 | 1.28 | 1.21 | 1.18 | 1.27 |
| Tab 3 | CTP-3 | 1.36 | 1.25 | 1.28 | 1.34 | 1.10 | 1.14 | 1.02 | 1.04 | 1.35 | 1.05 |
| | STP-3 | 1.06 | 1.12 | 1.14 | 1.16 | 1.05 | 1.08 | 1.00 | 1.02 | 1.17 | 1.10 |
| Tab 4 | STP-4A | 1.22 | 1.15 | 1.13 | 1.10 | 0.94 | 1.00 | 0.86 | 0.91 | 1.19 | 0.84 |
| | STP-4B | 0.94 | 0.93 | 0.99 | 0.84 | 0.98 | 0.95 | 0.97 | 0.97 | 0.96 | 0.95 |
| Tab 5 | STP-5A | 0.91 | 0.99 | 1.01 | 0.98 | 0.90 | 0.91 | 0.86 | 0.88 | 1.02 | 1.04 |
| | STP-5B | 0.99 | 0.97 | 1.11 | 1.14 | 0.97 | 1.00 | 0.91 | 0.95 | 1.11 | 0.95 |
| | STP-5C | 1.04 | 0.97 | 1.04 | 1.07 | 1.08 | 0.97 | 1.14 | 1.08 | 1.06 | 0.85 |
| | STP-5BW | 1.01 | 0.93 | 1.07 | 0.97 | 1.04 | 0.98 | 1.02 | 1.01 | 1.00 | 1.05 |
| Tab x | HL93 | 1.13 | 1.09 | 1.13 | 1.09 | 1.12 | 1.18 | 1.15 | 1.20 | 1.12 | 0.94 |
| NRL Vehicles | SU4 | 1.35 | 1.39 | 1.14 | 1.14 | 1.06 | 1.06 | 1.03 | 1.03 | 1.21 | 0.92 |
| | SU5 | 1.50 | 1.51 | 1.28 | 1.28 | 1.20 | 1.20 | 1.17 | 1.17 | 1.30 | 1.02 |
| | SU6 | 1.56 | 1.67 | 1.39 | 1.42 | 1.32 | 1.33 | 1.29 | 1.30 | 1.41 | 1.13 |
| | SU7 | 1.62 | 1.81 | 1.50 | 1.56 | 1.44 | 1.48 | 1.42 | 1.44 | 1.55 | 1.23 |

 Shaded boxes depict ratios less than 1.

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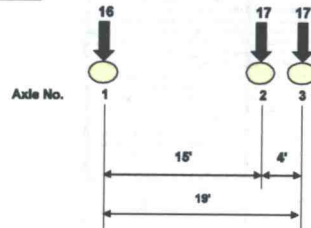
APPENDIX A

OREGON LEGAL LOADS - Load Rating Tier-2

Indicated concentrated loads are axle loads in kips

TYPE 3 Legal Truck

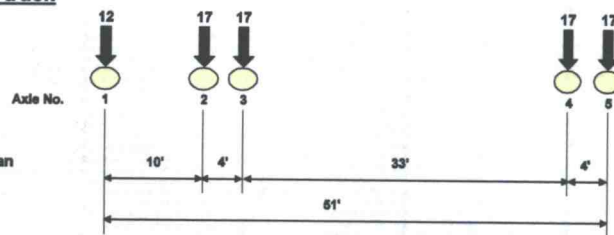
3 Axle Vehicle
Gross Weight = 60 k



TYPE 3S2 Legal truck

5 Axle Vehicle
Gross Weight = 80 k

Note:
This truck is greater than
the standard AASHTO
Type 3S2, which has
Gross Weight = 72 k



TYPE 3-3 Legal Truck

6 Axle Vehicle
Gross Weight = 80 k

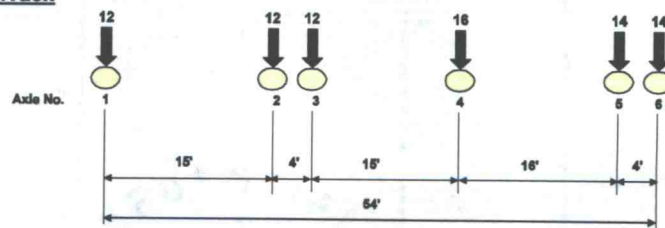


Figure A3.1: ODOT legal rating vehicles.

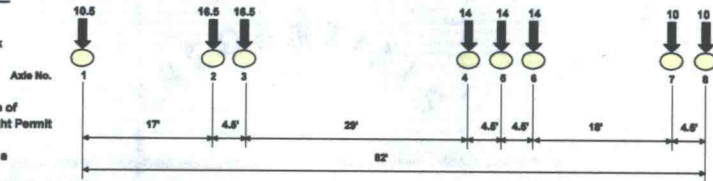
OREGON CONTINUOUS TRIP PERMIT (CTP) LOADS - Load Rating Tier-2

Indicated concentrated loads are axle loads in kips

Type OR-CTP-2A

8 Axle Vehicle
Gross Weight = 105.5 k

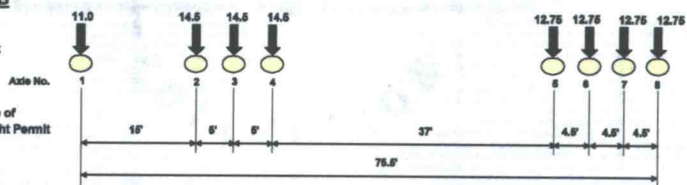
Representative Sample of
Annual Extended Weight Permit
Weight Table 2
MCTD refers to this as a
"Canadian Mule Train"
(This load was not used in Tier-1)



Type OR-CTP-2B

8 Axle Vehicle
Gross Weight = 105.5 k

Representative Sample of
Annual Extended Weight Permit
Weight Table 2
Maximum 4-axle group
(This load was not used in Tier-1)

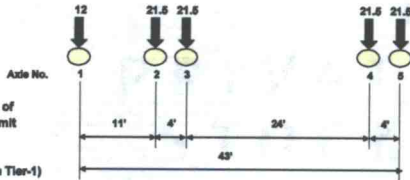


Type OR-CTP-3

5 Axle Vehicle
Gross Weight = 96 k

Representative Sample of
Annual Heavy Haul Permit
Weight Table 3

(Similar to "Permit-1" in Tier-1)



Note:

"Extended Weight" is a term that refers to trucks with axles or tandems the same as Legal Loads (20 k single-axle, 34 k tandem) but have a maximum GVW of 105.5 k. These are found in Weight Table 2. Examples of these include log trucks and milk tank trucks.

Figure A3.2: ODOT continuous trip permit rating vehicles.

OREGON SINGLE-TRIP PERMIT (STP) LOADS - Load Rating Tier-2

Indicated concentrated loads are axle loads in kips

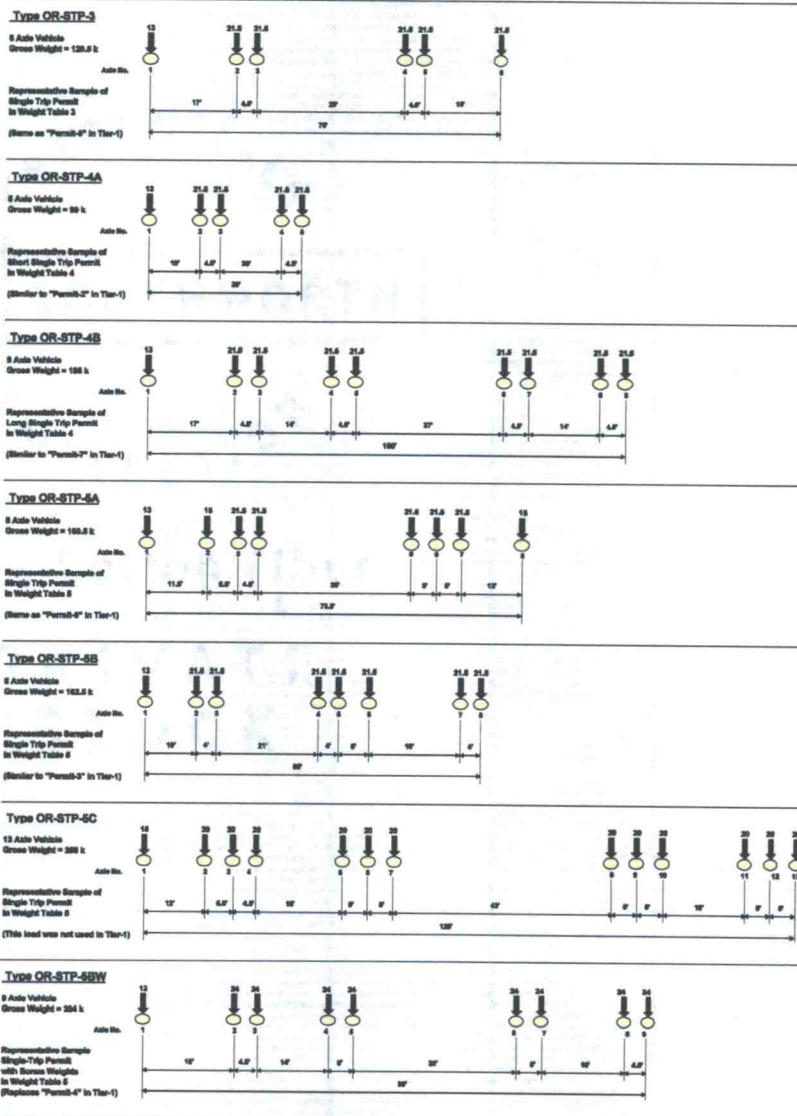


Figure A3.3: ODOT single trip permit rating vehicles.

OREGON LOAD RATING TRUCKS - Load Rating Tier-2

| LOAD GROUP | TIER-2 LOAD DESIGNATION | TYPE OF LOAD | G.V.W. | MCTD WEIGHT TABLE | NOTES | Corresponding Tier-1 Truck Designation | Corresponding OSU Study Designation |
|------------------------------|-----------------------------|--|---------|-----------------------------------|--|--|-------------------------------------|
| Design Loads | — | Design Load | 72 k | — | Required for NBI reporting in the past, not used in Tier-2. | HS-20 | Vehicle 1 |
| | HL-93 Truck | Design Load | 72 k | — | The most critical of the three HL-93 combination loads (below) will now be used for NBI reporting. Not used in Tier-1. | — | — |
| | HL-93 Tandem | Design Load | 60 k | — | — | — | — |
| | HL-93 Truck + Lane | Design Load Combination (required by LRFR) | — | — | HL-93 Truck applied with 0.640 k/ft lane | — | — |
| | HL-93 Tandem + Lane | Design Load Combination (required by LRFR) | — | — | HL-93 Tandem applied with 0.640 k/ft lane | — | — |
| | HL-93 Truck Train + Lane | Design Load Combination (required by LRFR) | — | — | Train of 2 HL-93 Trucks @ 90% applied with 0.640 k/ft lane @ 90% | — | — |
| Legal Loads | Type 3 | Legal Load | 60 k | Weight Table 1 | Same as AASHTO Legal Type 3 | Type 3 | Vehicle 2 |
| | Type 3S2 | Legal Load | 80 k | Weight Table 1 | Different than standard AASHTO 3S2, which is 72 k. | Type 3S2 | Vehicle 3 |
| | Type 3-3 | Legal Load | 80 k | Weight Table 1 | Same as AASHTO Legal Type 3-3 | Type 3-3 | Vehicle 4 |
| | Type 3-3 Train + Legal Lane | Legal Load Combination (required by LRFR) | — | — | Train of 2 Legal Type 3-3 @ 75% applied with 0.2 k/ft lane load. Not used in Tier-1. | — | — |
| | Type 3-3 + Legal Lane | Legal Load Combination (required by LRFR) | — | — | Legal Type 3-3 @ 75% applied with 0.2 k/ft lane load. Used only for Spans > 200 ft. Not used in Tier-1. | — | — |
| Continuous Trip Permit Loads | Type CTP-2A | Annual Extended Weight Permit | 105.5 k | Weight Table 2 | MCTD refers to this as a "Canadian Mule Train" | (Permit 8) | — |
| | Type CTP-2B | Annual Extended Weight Permit | 105.5 k | Weight Table 2 | Contains maximum allowable 4-axle cluster | (Permit 9) | — |
| | Type CTP-3 | Annual Heavy Haul Permit | 95 k | Weight Table 3 | Heavy Haul that maximizes Weight Table 3 | Permit 1 | Vehicle 5 |
| Single Trip Permit Loads | Type STP-3 | Single Trip Permit | 120.5 k | Weight Table 3 | In Tier-1, was used for Local Agency bridges only | Permit 5 | Vehicle 9 |
| | Type STP-4A | Single Trip Permit | 99 k | Weight Table 4 | In Tier-1, was considered representative of CTP's | Permit 2 | Vehicle 6 |
| | Type STP-4B | Single Trip Permit | 185 k | Weight Table 4 | In Tier-1, was used for Local Agency bridges only | Permit 7 | Vehicle 11 |
| | Type STP-5A | Single Trip Permit | 150.5 k | Weight Table 6 | In Tier-1, was used for Local Agency bridges only | Permit 6 | Vehicle 10 |
| | Type STP-5B | Single Trip Permit | 162.5 k | Weight Table 5 | In Tier-1, was considered representative of Weight Table 4 | Permit 3 | Vehicle 7 |
| | Type STP-5C | Single Trip Permit | 250 k | Weight Table 5 | Represents upper range of WT-5, heavy 8-axle group in 36 ft | — | — |
| | Type STP-5D | Single Trip Permit | 204 k | Weight Table 5 with Bonus Weights | In Tier-1, was considered representative of Weight Table 5 | Permit 4 | Vehicle 8 |
| | Type STP-5E | Single Trip Permit | 204 k | Weight Table 5 with Bonus Weights | In Tier-1, was considered representative of Weight Table 5 | Permit 4 | Vehicle 8 |

Shading indicates the loadings to be investigated and reported for ODOT Tier-2 load ratings. The "Type 3-3 + Legal Lane" loading applies only to spans > 200 ft.

LIVELOAD LEVELS OF SERVICE - Load Rating Tier-2

| TYPE OF LOAD | Max. G.V.W. | WEIGHT TABLE | REPRESENTATIVE RATING TRUCKS |
|---|-------------|----------------|--------------------------------------|
| Oregon Legal Loads | 80 k | Table 1 | Types 3, 3S2, 3-3 |
| Annual (Continuous Trip) Extended Weight Permit | 105.5 k | Table 2 | Types CTP-2A, CTP-2B |
| Annual (Continuous Trip) Heavy Haul Permit | 95 k | Table 3 | Type CTP-3 |
| Single Trip Permits | 220 k | Table 3 | Type STP-3 |
| | 266 k | Table 4 | Types STP-4A and STP-4B |
| | 304 k | Table 5 | Types STP-5A, STP-5B, STP-5C, STP-5D |
| | 304 k | Table 5 | Types STP-5A, STP-5B, STP-5C, STP-5D |
| Super-Loads (require specific evaluation) | — | Beyond Table 5 | — |

Figure A3.4: ODOT load rating vehicle descriptions.

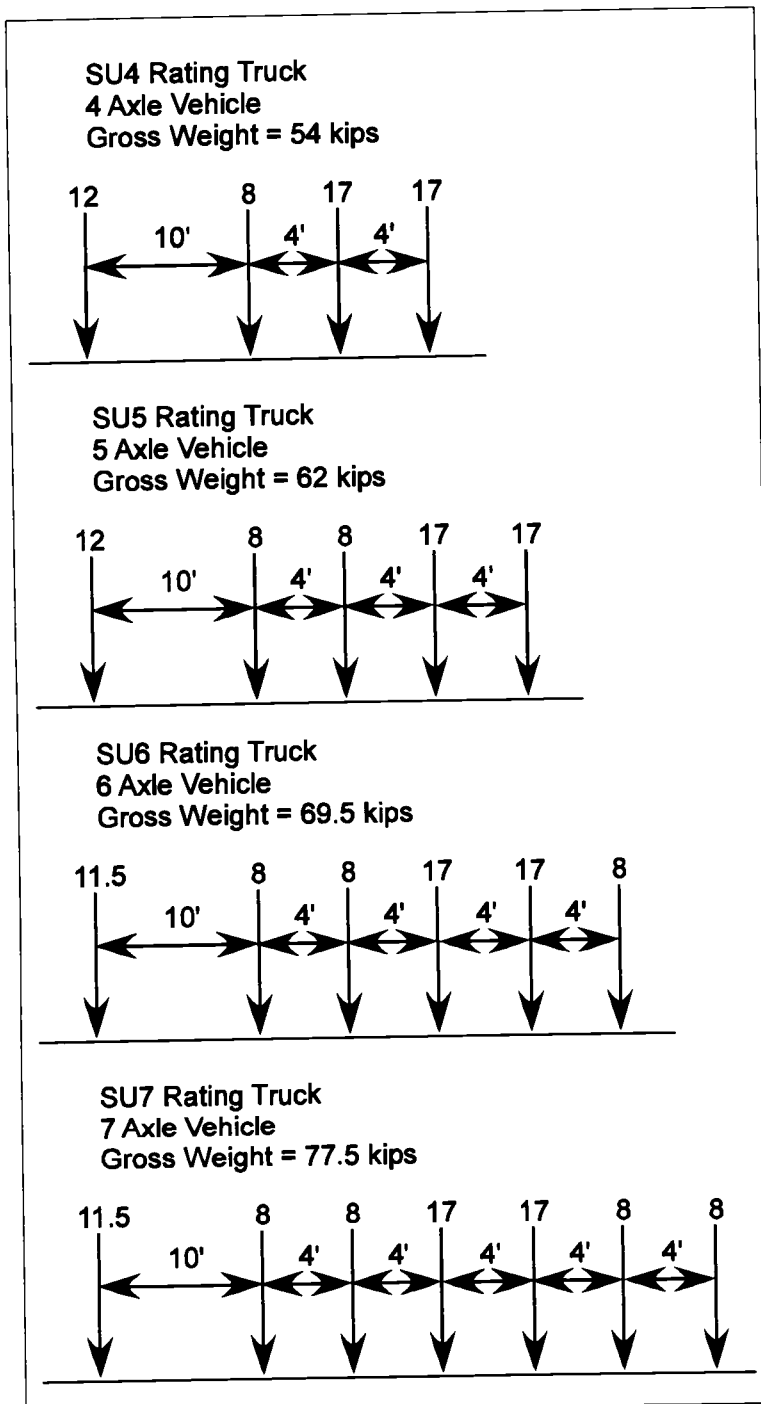


Figure A3.5: AASHTO notional rating loads (specialized hauling vehicles).

OREGON DEPARTMENT OF TRANSPORTATION
MOTOR CARRIER TRANSPORTATION DIVISION
TRANSPORTATION PERMIT UNIT
 500 CAPITOL ST. NE
 SALEM OREGON 97301-3536

Permit Weight Table 1

The following exceptions apply to the table of weights shown below:

Exception 1: Two consecutive tandem axes may weigh up to 34,000 pounds each if:

| Minimum Axle Spacing Required | Interstate Highways | Non-Interstate Highways |
|-------------------------------|---------------------|-------------------------|
| 30 feet or more | Permit Required | No Permit Required |
| 36 feet or more | No Permit Required | No Permit Required |

Exception 2: A group of four axes consisting of a set of tandem axes and two axes spaced nine feet or more apart may have a loaded weight of more than 65,500 pounds and up to 70,000 pounds if:

| Minimum Axle Spacing Required | Interstate Highways | Non-Interstate Highways |
|-------------------------------|---------------------|-------------------------|
| 35 feet or more | Permit Required | No Permit Required |

Minimum axle spacing is the distance between the first and last axle of any group shown above.

| Wheelbase In Feet □ | Number of Axles | | | | | | Wheelbase In Feet □ | Number of Axles | | | | | |
|---------------------------|-----------------|--------|--------|--------|--------|--------------|---------------------------|-----------------|--------|--------|--------|--------|--------------|
| | 2 | 3 | 4 | 5 | 6 | 7 Or More | | 2 | 3 | 4 | 5 | 6 | 7 Or More |
| 4 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 31 | 40,000 | 59,000 | 62,500 | 67,500 | 72,500 | 78,000 |
| 5 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 32 | 40,000 | 60,000 | 63,500 | 68,000 | 73,000 | 78,500 |
| 6 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 33 | 40,000 | 60,000 | 64,000 | 68,500 | 74,000 | 79,000 |
| 7 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 34 | 40,000 | 60,000 | 64,500 | 69,000 | 74,500 | 80,000 |
| 8 & less | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 34,000 | 35 | 40,000 | 60,000 | 65,000 | 70,000 | 75,000 | 80,000 |
| Over 8 | 38,000 | 42,000 | 42,000 | 42,000 | 42,000 | 42,000 | 36 | 40,000 | 60,000 | 66,000 | 70,500 | 75,500 | 80,000 |
| 9 | 38,000 | 42,500 | 42,500 | 42,500 | 42,500 | 42,500 | 37 | 40,000 | 60,000 | 66,500 | 71,000 | 76,000 | 80,000 |
| 10 | 40,000 | 43,500 | 43,500 | 43,500 | 43,500 | 43,500 | 38 | 40,000 | 60,000 | 67,500 | 71,500 | 77,000 | 80,000 |
| 11 | 40,000 | 44,000 | 44,000 | 44,000 | 44,000 | 44,000 | 39 | 40,000 | 60,000 | 68,000 | 72,500 | 77,500 | 80,000 |
| 12 | 40,000 | 45,000 | 50,000 | 50,000 | 50,000 | 50,000 | 40 | 40,000 | 60,000 | 68,500 | 73,000 | 78,000 | 80,000 |
| 13 | 40,000 | 45,500 | 50,500 | 50,500 | 50,500 | 50,500 | 41 | 40,000 | 60,000 | 69,500 | 73,500 | 78,500 | 80,000 |
| 14 | 40,000 | 46,500 | 51,500 | 51,500 | 51,500 | 51,500 | 42 | 40,000 | 60,000 | 70,000 | 74,000 | 79,000 | 80,000 |
| 15 | 40,000 | 47,000 | 52,000 | 52,000 | 52,000 | 52,000 | 43 | 40,000 | 60,000 | 70,500 | 75,000 | 80,000 | 80,000 |
| 16 | 40,000 | 48,000 | 52,500 | 58,000 | 58,000 | 58,000 | 44 | 40,000 | 60,000 | 71,500 | 75,500 | 80,000 | 80,000 |
| 17 | 40,000 | 48,500 | 53,500 | 58,500 | 58,500 | 58,500 | 45 | 40,000 | 60,000 | 72,000 | 76,000 | 80,000 | 80,000 |
| 18 | 40,000 | 49,500 | 54,000 | 59,000 | 59,000 | 59,000 | 46 | 40,000 | 60,000 | 72,500 | 76,500 | 80,000 | 80,000 |
| 19 | 40,000 | 50,000 | 54,500 | 60,000 | 60,000 | 60,000 | 47 | 40,000 | 60,000 | 73,500 | 77,500 | 80,000 | 80,000 |
| 20 | 40,000 | 51,000 | 55,500 | 60,500 | 66,000 | 66,000 | 48 | 40,000 | 60,000 | 74,000 | 78,000 | 80,000 | 80,000 |
| 21 | 40,000 | 51,500 | 56,000 | 61,000 | 66,500 | 66,500 | 49 | 40,000 | 60,000 | 74,500 | 78,500 | 80,000 | 80,000 |
| 22 | 40,000 | 52,500 | 56,500 | 61,500 | 67,000 | 67,000 | 50 | 40,000 | 60,000 | 75,500 | 79,000 | 80,000 | 80,000 |
| 23 | 40,000 | 53,000 | 57,500 | 62,500 | 68,000 | 68,000 | 51 | 40,000 | 60,000 | 76,000 | 80,000 | 80,000 | 80,000 |
| 24 | 40,000 | 54,000 | 58,000 | 63,000 | 68,500 | 74,000 | 52 | 40,000 | 60,000 | 76,500 | 80,000 | 80,000 | 80,000 |
| 25 | 40,000 | 54,500 | 58,500 | 63,500 | 69,000 | 74,500 | 53 | 40,000 | 60,000 | 77,500 | 80,000 | 80,000 | 80,000 |
| 26 | 40,000 | 55,500 | 59,500 | 64,000 | 69,500 | 75,000 | 54 | 40,000 | 60,000 | 78,000 | 80,000 | 80,000 | 80,000 |
| 27 | 40,000 | 56,000 | 60,000 | 65,000 | 70,000 | 75,500 | 55 | 40,000 | 60,000 | 78,500 | 80,000 | 80,000 | 80,000 |
| 28 | 40,000 | 57,000 | 60,500 | 65,500 | 71,000 | 76,500 | 56 | 40,000 | 60,000 | 79,500 | 80,000 | 80,000 | 80,000 |
| 29 | 40,000 | 57,500 | 61,500 | 66,000 | 71,500 | 77,000 | 57 or | 40,000 | 60,000 | 80,000 | 80,000 | 80,000 | 80,000 |
| 30 | 40,000 | 58,500 | 62,000 | 66,500 | 72,000 | 77,500 | more | | | | | | |

The loaded weight of any group of axes, vehicle, or combination of vehicles shall not exceed that specified in the table of weights shown above or any of the following:


- The manufacturer's side wall tire rating but not to exceed 600 pounds per inch of tire width.
- 600 pounds per inch of tire width.
- 20,000 pounds on any one axle, including any one axle of a group of axes.
- 34,000 pounds on any tandem axle.
- The sum of the permissible axle, tandem axle, or group of axle weights shown above, whichever is less.

Note exceptions 1 and 2 above.

735-8110 (8-02) STW 30057

Distance measured to the nearest foot; when exactly 1/2 foot or more, round up to the next larger number.

Figure A3.6: Permit Weight Table 1 [Oregon Motor Carrier].

|  OREGON DEPARTMENT OF TRANSPORTATION MOTOR CARRIER TRANSPORTATION DIVISION 550 CAPITOL ST NE SALEM OR 97301-2530 | | PERMIT WEIGHT TABLE 2 | | | |
|--|---------|--|---------|-----------------|--|
| WHEELBASE | 5 Axles | 6 Axles | 7 Axles | 8 or More Axles | |
| 47 | 77500 | 81000 | 81000 | 81000 | |
| 48 | 78000 | 82000 | 82000 | 82000 | |
| 49 | 78500 | 83000 | 83000 | 83000 | |
| 50 | 79000 | 84000 | 84000 | 84000 | |
| 51 | 80000 | 84500 | 85000 | 85000 | |
| 52 | 80500 | 85000 | 86000 | 86000 | |
| 53 | 81000 | 86000 | 87000 | 87000 | |
| 54 | 81500 | 86500 | 88000 | 91000 | |
| 55 | 82500 | 87000 | 89000 | 92000 | |
| 56 | 83000 | 87500 | 90000 | 93000 | |
| 57 | 83500 | 88000 | 91000 | 94000 | |
| 58 | 84000 | 89000 | 92000 | 95000 | |
| 59 | 85000 | 89500 | 93000 | 96000 | |
| 60 | 85500 | 90000 | 94000 | 97000 | |
| 61 | 86000 | 90500 | 95000 | 98000 | |
| 62 | 87000 | 91000 | 96000 | 99000 | |
| 63 | 87500 | 92000 | 97000 | 100000 | |
| 64 | 88000 | 92500 | 97500 | 101000 | |
| 65 | 88500 | 93000 | 98000 | 102000 | |
| 66 | 89000 | 93500 | 98500 | 103000 | |
| 67 | 90000 | 94000 | 99000 | 104000 | |
| 68 | 90000 | 95000 | 99500 | 105000 | |
| 69 | 90000 | 95500 | 100000 | 105500 | |
| 70 | 90000 | 96000 | 101000 | 105500 | |
| 71 | 90000 | 96500 | 101500 | 105500 | |
| 72 | 90000 | 96500 | 102000 | 105500 | |
| 73 | 90000 | 96500 | 102500 | 105500 | |
| 74 | 90000 | 96500 | 103000 | 105500 | |
| 75 | 90000 | 96500 | 104000 | 105500 | |
| 76 | 90000 | 96500 | 104500 | 105500 | |
| 77 | 90000 | 96500 | 105000 | 105500 | |
| 78 | 90000 | 96500 | 105500 | 105500 | |

See Weight Table 1, if using less than five axles or 47 feet wheelbase.

735-811112-001
● DISTANCE MEASURED TO THE NEAREST FOOT. WHEN EXACTLY 1/2 FOOT OR MORE, ROUND UP TO THE NEXT LARGER NUMBER ●
STK # 300550

Figure A3.7: Permit Weight Table 2 [Oregon Motor Carrier].



PERMIT WEIGHT TABLE

3

Table with columns for axle counts (2 to 20) and rows for weight limits (e.g., 43,000, 43,000, 43,000, etc.). Includes a 'WHEELBASE' section and a note: 'DISTANCE MEASURED TO THE NEAREST FOOT. WHEN EXACTLY 1/2 FOOT OR MORE, ROUND UP TO THE NEXT LARGER NUMBER.'

Figure A3.8: Permit Weight Table 3 [Oregon Motor Carrier].

Table with 21 columns (Axles 2-20) and rows of permit weight data. Values range from 43,000 to 364,400. Includes a note at the bottom: 'DISTANCE MEASURED TO THE NEAREST FOOT. WHEN EXACTLY 1/2 FOOT OR MORE, ROUND UP TO THE NEXT LARGER NUMBER'.

Figure A3.9 (Continued): Permit Weight Table 4 [Oregon Motor Carrier].

APPENDIX B

Justification for Liger Filters

In order to obtain the highest quality of data possible, the WIM files need to be cleaned and filtered. The Liger program is the second of two cleaning programs, and its use depends on the output of the first program, Wingnut. Liger checks all vehicle records to make sure they contain realistic numerical values and are free from invalid characters. The Liger program employs 14 different filters, which are covered in more detail herein. An error counter is included for each error category to help point out areas of concern for the WIM files. Each record that contains an error is removed from the file and written to a separate error file, called Liger_Errors.txt. At the end of the error file, the total number of each type of error is displayed. See figure 1 for an error file example.

| | | | | | | | | | | | | | | | | | | | | | | |
|--|----------|----------|----------|----------|----------|----------|----------|----------|----------|----------|-----------|----------|----------|-----|----------------------|-----|-----|-----|-------|-----|-----|------|
| Mon Jun 28 23:11:25.12 2004 | 8 | A | 65 | 17.5 | 42.0 | 50 | 4 | 0.067 | 4.9 | 8.3 | 1.8 | 2.5 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 14.1 |
| Line 325 Error - Spacing < 3.4 ft | | | | | | | | | | | | | | | | | | | | | | |
| Mon Jun 28 23:28:22.94 2004 | 13 | A | 54 | 12.9 | 44.6 | 54 | 5 | 0.022 | 3.2 | 6.4 | 1.1 | 1.1 | 1.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 13.3 |
| Line 325 Error - Spacing < 3.4 ft | | | | | | | | | | | | | | | | | | | | | | |
| Mon Jun 28 23:28:30.56 2004 | 8 | A | 51 | 13.4 | 32.8 | 43 | 4 | 0.019 | 4.7 | 5.6 | 1.9 | 1.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 13.3 |
| Line 325 Error - Spacing < 3.4 ft | | | | | | | | | | | | | | | | | | | | | | |
| Mon Jun 28 23:42:24.98 2004 | 8 | A | 54 | 15.4 | 34.6 | 41 | 4 | 0.082 | 4.2 | 8.9 | 1.1 | 1.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 14.0 |
| Line 325 Error - Spacing < 3.4 ft | | | | | | | | | | | | | | | | | | | | | | |
| Mon Jun 28 23:51:13.66 2004 | 8 | A | 69 | 10.8 | 32.7 | 40 | 4 | 0.009 | 4.4 | 4.2 | 1.0 | 1.1 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 13.6 |
| Line 325 Error - Spacing < 3.4 ft | | | | | | | | | | | | | | | | | | | | | | |
| Truck Count from wingnut output: | | | | | | | | | | | | | | | | | | | | | | |
| NO. OF VALID TRUCKS = | | | | | | | | | | | 38867 | | | | NO. OF NULL TRUCKS = | | | | 15129 | | | |
| Total no. of valid trucks from Liger = 36265 | | | | | | | | | | | | | | | | | | | | | | |
| Total no. of null trucks from Liger = 2602 | | | | | | | | | | | | | | | | | | | | | | |
| Error142 | Error165 | Error177 | Error202 | Error216 | Error229 | Error242 | Error256 | Error269 | Error282 | Error296 | Errors309 | Error325 | Error344 | | | | | | | | | |
| 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 5 | 25 | 0 | 0 | 2560 | 0 | | | | | | | | | |

Figure B3.1: Sample error file showing the end of the record produced by the Liger program.

In addition to the Liger_Errors.txt file, another error file is created, called Liger_Special_Errors.txt. This file records errors that require a closer look, such as Error 202, which tosses records where an individual axle is greater than 50 kips.

Each error is named with a number, which corresponds to the actual line in the FORTRAN code where it can be found. However, after numerous revisions to the program, the number and the line of code no longer match.

Invalid Character Filters

The following three filters can be categorized as invalid character filters. These filters check the line for inconsistencies, and are easy to pick out.

Error 142 – Invalid Date

This error checks to see if the date stamp is of ordinary format. For instance, an ordinary date stamp looks like this: `Mon Jun 28 23:11:25.12 2004`

The 15th character in this statement is a colon (:), and the 25th-27th characters in this statement are the numbers (200). Both of these conditions should be true for each valid record. If not, then the record is tossed, and 1 is added to the Error 142 counter. Having either one of these conditions be false compromises the records' validity, and suggests that other characters might be incorrect also.

Error 165 – Non-Numerical Value

This error checks to see if characters 28 through 222 contain a non-numerical value. This includes all letters and symbols. The exception to this is at character 37, which allows the letters 'A', 'B', and 'C' to pass, which are sometimes used as lane signifiers, as an alternative to '1', '2', and '3'. All programs that are used for

statistics and analysis depend on the cleaned Liger file. That makes this particular error an important one, because future programs can only read numerical input values for characters 28-222.

Error 177 – Decimal Value in Wrong Place

This error checks to see if characters 25 through 49 and 61 through 76 contain a decimal point ('.'). This is a continuation from Error 165 in that it checks two sub-ranges inside of a larger range for misplaced decimal points. During the early phases of the cleaning process, certain data files would crash the program because of this problem.

Vehicle-Specific Filters

The rest of the error filters are specific to the vehicle itself. Some are justified based on physical constraints and others on engineering judgment. Four of the errors are worth a second look and are written to a second error file, named `Liger_Special_Errors.txt`. These errors are further investigated to see if the data is truly valid or not.

Error 202 – Individual Axle Weight > 50 kips

This error checks to see if a record contains an individual axle weight greater than 50 kips. This value was logically chosen based on how much weight, or pressure, a vehicle's tire can actually withstand. The 50-kip recommendation came from Dr.

Chris Higgins. This error is one of the four that is written to the Liger_Special_Errors.txt file for further investigation. Several of the data files had 40-50 records where individual axles were greater than 50 kips. Upon investigation, these records were deemed bogus and tossed out.

Error 216 – Speed < 10 mph

This error checks to see if the vehicle's speed is less than 10 mph. The speed of 10 mph was recommended by Bala Sivakumar of Lichtenstein Consulting Engineers, Inc. In his experience, vehicles traveling below 10 mph usually provide skewed data because the WIM equipment is not apt to handle such low speeds. Low speeds also imply traffic jams, which the equipment is not set up to handle.

Error 229 – Speed > 99 mph

This error checks to see if the vehicle's speed is greater than 99 mph. This speed was chosen as a cap value because it is highly unlikely that a vehicle, namely a truck, travels over 100 mph. A record with this error is probably bad in the first place and should be tossed. This error is written to the Liger_Special_Errors.txt for further investigation.

Error 242 – Length > 200 ft

This error checks to see if the vehicle's length, both bumper-to-bumper and rear-to-steer axle, is greater than 200 ft. Vehicles usually do not exceed 200 ft in length.

The records that do exceed 200 ft are written to the Liger_Special_Errors.txt for further investigation. The need for this filter was a result of several files having bad length data.

Error 256 – Bumper-to-Bumper Length + 10 ft < Rear-to-Steer Axle Length

This error checks to see if the bumper-to-bumper length of a vehicle, plus 10 ft, is less than the rear-to-steer axle length. This filter is used as an internal check to see if the records are valid and consistent. It is not physically possible for the rear-to-steer axle length to be greater than the bumper-to-bumper length, but calibration problems with the WIM equipment sometimes show this to be true.

According to David Fifer, who is the Oregon Department of Transportation's (ODOT) Intelligent Transportation Systems Specialist, inductance loops are used to measure the overall bumper-to-bumper length – one upstream and one downstream. Each loop is cut into the roadway in a 6ft square. Generally, the distance between the back of the upstream loop (Loop 1) to the front of the downstream loop (Loop 2) is 22.167 ft. Loop 1 starts the whole process as soon as it detects metal, which indicates when a vehicle "event" begins. The event ends when it no longer detects any metal. Loop 2 reacts in the same manner. A speed-distance formula is then used to calculate the "length" of the event. The bumper-to-bumper length accuracy of any event is dependent on when each loop begins and ceases to detect a vehicle.

According to Fifer, ODOT has the ability to dictate the process of when an event begins and ends. Each loop can be individually configured to expand or condense its "capture field." Once they've been properly calibrated, the information is very accurate/reliable. Occasionally though, because of age, traffic volume, condition of the roadway, etc., one or both loops can fall slightly out of calibration, giving longer or shorter length values. When this happens, ODOT reconfigures either or both to the proper level of accuracy. To account for this potential error in calibration, Error 256 adds 10 ft on to the bumper-to-bumper length. The September 2005 data file for La Grande EB has over 21,000 records where the bumper-to-bumper length is less than the rear-to-steer axle length, most being over by 2-5 ft. Once the 10 ft was added in, the number of errors was reduced to below 100.

The individual axle spacing lengths are derived by a combination of single load cell sensors and Dynax axle sensors. When each sensor is "hit" it counts an axle. The same type of speed-distance formula is used to calculate the distance between the axles. The sum of all axle spacing lengths provide the length of a vehicle's wheel base (center hub of the front axle to the center hub of the rear axle), or rear-to-steer axle length.

It seems highly unlikely that the equipment would be out of calibration so much that the difference between the two values would exceed 10 ft. Upon inspection it

was noted that the records which did exceed 10ft in difference had inconsistencies making them “bad”.

Error 269 – Bumper-to-Bumper Length & Rear-to-Steer Axle Length < 7 ft

This error checks to see if the bumper-to-bumper or rear-to-steer axle lengths are less than 7 ft. This filter was created to toss erroneous records from the file. It was noticed that sometimes the equipment would record incorrect length values. This could result from either of the inductance loops not picking up a correct starting or ending point of a vehicle.

Error 282 – Steer Axle < 5ft

This error checks to see if the steer axle is less than 5 ft from the second axle. Dr. Higgins suggested this value because vehicles cannot physically have a steer tandem. This error occurs on events that end prematurely, before the truck has completed crossing the loops, therefore beginning a new event starting with the tandem of the next trailer. This error is more common with the data that has first been adjusted by the WIM program rather than by the Wingnut program.

Error 296 – Axle # NE 1-13

This error checks to see if the axle number is not equal to 1 through 13. This would suggest an erroneous record, which would be tossed. Oregon does not allow vehicles to have greater than 13 total axles.

Error 309 – GVW > 280 kips

This error checks to see if the gross vehicle weight (GVW) is greater than 280 kips. Exceeding this value is highly unlikely. Records that are over 280 kips are written to the Liger_Special_Errors.txt for further investigation. This roughly equals 24 kips per axle for a 12 axle vehicle.

Error 325 – Any Axle Spacing < 3.4 ft

This error checks to see if any axle is less than 3.4 ft from the next axle on the vehicle. Tires have limitations on how close they can be spaced to each other before touching. This value is just below 4 ft. The reason 3.4 ft is used instead of 4 ft is to encompass all possible calibration errors that might be present. This is the most common error found in data files that are of the new format (those processed by Wingnut). Vehicles that are below this mark usually have an accompanying error in the original raw record, which looks like this: Warning: UnAX !

According to Fifer, this warning means there was an "unequal axles detected" error. This is the result of the dynax axle sensor picking up only one side of one of the axles (either the left or right) causing an unequal count - 3 on the left side, and 4 on the right. This occasionally happens when a vehicle doesn't hit the sensors square (may be in the process of changing lanes).

Error 344 – GVW > +/- 7% of the Sum of the Axle Weights

This error checks to see if the gross vehicle weight value differs from the sum of the axle weights by more than 7%. Like Error 256, this filter is an internal check for consistency within the record itself.

List of Errors*Invalid Character Filters*

- Error 142 – Invalid Date ((15:15) does not equal ‘:’ and (25:27) does not equal ‘200’)
- Error 165 – Non-Numerical Value (char. 28 through 222 contains a non-numerical value)
- Error 177 – Decimal Value in Wrong Place

Vehicle-Specific Filters

- Error 202 – Individual Axle Weight > 50 kips
- Error 216 – Speed < 10 mph
- Error 229 – Speed > 99 mph
- Error 242 – Length > 200 ft
- Error 256 – Bumper-to-Bumper Length + 10 ft < The Sum of the Axle Spacings
- Error 269 – Length < 7 ft AND Sum of Axles < 7 ft
- Error 282 – 1st Axle Spacing < 5 ft (steer axle)
- Error 296 – Axle # does not Equal 1 - 13
- Error 309 – GVW > 280 kips (check outcome)
- Error 325 – Any Axle Spacing < 3.4 ft
- Error 344 – GVW > +/- 7% of the Sum of the Axle Weights

Quality Control Checks for Processing WIM Data

The method used to clean, filter, and sort the raw WIM data includes the following tasks:

- Obtain raw WIM data from ODOT ftp site.
- Identify format errors in raw WIM data and reformat for subsequent processing (program Wingnut#.exe where # is the current version number).
- Identify WIM record errors (program Liger#.exe where # is the current version number).
- Review error files to ensure reported errors are captured and no records are lost.
- Sort data into weight-table classifications (program Tablesorter#.exe where # is the current version number).
- Filter records containing 3S2 configurations and compile the T2PCTP and T3MCTP records (program 3S2_Nubs2b).
- Spot check records to ensure proper sort.
- Plot GVW results to look for visual distinctions such as repeated records, spurious outliers, and other inconsistencies. It was observed that the cleaned and sorted records could contain replicate identical records, of which only one was true. This visual scanning of results is still necessary and it is not recommended to use a purely computerized process.
- Import weight-table records into Excel and sort top 20%.

As part of the data evaluation process, a series of quality control checks were performed to verify the accuracy of the data classification performed by OSU. The QC process included the following:

- Verification of WIM data record error identification.
- Verification of raw WIM record transcription to OSU usable format.
- Verification of sorting algorithm for weight-table classification.

All software programs written by OSU that were used for cleaning and sorting the raw WIM data were independently checked. The software programs were verified by creating sample input files for each step of the cleaning and sorting process. These sample input files contained each of the specific error identification types that were to be captured, as well as specific valid WIM records that were of known classification.

1. Raw WIM data are used for input into Wingnut#.exe for initial sorting. Eleven (11) errors are identified and removed by this program. Primarily errors at this stage are format issues. Data with formatting errors are removed and placed in error files. To check the program, a sample input file was made with over 50 entries. Some entries were valid WIM records and others included the specific errors to be found and omitted from the data set at this point. The order of the valid data and known errors were random. The output results from Wingnut# were checked against the errors that were intended. All errors were correctly

identified with the exception of Error 325 which only pertains to the old style WIM files with axle pictograms and as such not included in the sample file.

Error 144 – If line1(N:N+4).EQ.'W 0.0' (TYPE)

Error 184 – If line1(N:N+3).NE.'LANE' (TYPE)

Error 203 – If line1(N:N+4).EQ.'W 0.0' (CLASS)

Error 244 – If line1(N:N+3).NE.'LANE' (CLASS)

Error 253 – If line1(N:N+1).EQ.'TY'.OR.line1(N:N).EQ.'C' is not true

Error 275 – If line2(N:N).EQ.'U'

Error 300 – If line2(N:N).NE.'k' (18-K)

Error 327 – If line2(N:N).NE.'k' (ESAL)

Error 361 – If line3(N:N+3).EQ.'AXLE'.OR.'18-K'.OR.'ESAL'

Error 377 – If line4(N:N+3).EQ.'(ft)'

Error 325 – If line8(N:N+1).EQ.'Un'

2. The next step in the sorting process is program Liger#.exe. There are 14 errors identified and removed by this program. These are errors that identify outlier data that typically would be an erroneous record. Using the sample input file with specified errors and valid data, all the error types were properly captured and stored in the error files. The only issue that was detected was for speeds greater than 99 mph. The program read only 2 integers and so did not catch those trucks that might be traveling over 100 mph. This was corrected and

subsequently verified. There was no impact on the prior load factor results based on this format specification, particularly as the WIM system already identifies vehicles that are traveling too fast and does not record the data for such cases.

Error 142 – Invalid Date ((15:15) does not equal ‘:’ and (25:27) does not equal ‘200’)

Error 165 – Non-Numerical Value (char. 28 - 222 contains a non-numerical value)

Error 177 – Decimal Value in Wrong Place

Error 202 – Individual Axle Weight > 50 kips

Error 216 – Speed < 10 mph

Error 229 – Speed > 99 mph

Error 242 – Length > 200 ft

Error 256 – Bumper-to-Bumper Length + 10 ft < The Sum of the Axle Spacings

Error 269 – Length < 7 ft AND Sum of Axles < 7 ft

Error 282 – 1st Axle Spacing < 5 ft (steer axle)

Error 296 – Axle # does not Equal 1 - 13

Error 309 – GVW > 280 kips (check outcome)

Error 325 – Any Axle Spacing < 3.4 ft

Error 344 – GVW > +/- 7% of the Sum of the Axle Weights

3. The weight-table sort is performed with two FORTRAN programs that use the Liger cleaned WIM data. The data are sorted into the correct ODOT permit weight-table classifications. To verify this program, an input file was made that included 3 trucks from each of the weight tables (Tables 1 through 5). The three record examples for each table classification were taken from the lower, the middle, and the upper range of each table. The data were properly sorted by overall GVW into the correct weight tables.

4. The second program that sorts the cleaned truck data is 3S2_Nubs.exe. This program sorts the Liger data into 3S2's and T2PCTP (Table 2 with continuous trip permit trucks) and T3MCTP (Table 3 without the continuous permit trucks) folders for input into load factor statistics. A day in a month was run in this program to verify that all trucks sorted into 3S2 were 5 axles and met the axle spacing requirements for the 3S2. The spacing used was the default (>5.5'). The program correctly identified the 5 axle vehicles and these were further correctly sorted out into the 3S2 configurations.

Next, the T2PCTP and T3MCTP were verified against the output tables from table sorter. The Table 3 file was sorted by axles and then axle spacing to identify the 3S2 trucks and to verify the final number of these trucks matched those subtracted from the new T3MCTP file and the same number was added to T2PCTP (except for those vehicles with GVW > 80 kips).

The WIM data processing described above relies on specific data formatting. If the format is changed in the future, the programs will need to be updated. Additionally, the permit weight table sort used by OSU is based on the current ODOT permit weight tables: STK#300557 (Permit Weight Table 1), STK#300558 (Permit Weight Table 2), STK#300559 (Permit Weight Table 3), STK#300560 (Permit Weight Table 4), STK#300561 (Permit Weight Table 5). If these permit tables change, then the program Tablesorter will need to be revised accordingly.

To ensure changes can be properly implemented, ODOT should inform OSU of future changes when or if they occur.

APPENDIX C

Interstate 5

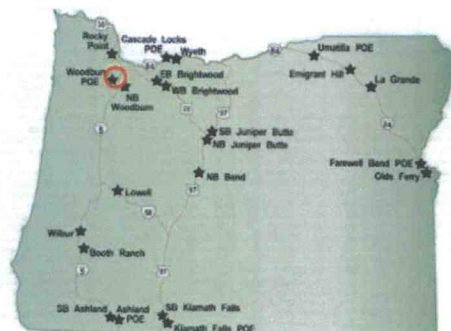
Woodburn NB

| | |
|--------------------------|------------------------|
| Location (MP) | I-5 (274.15) |
| ADT | 41,893 |
| ADTT | 5,550 |
| # Lanes | 3 |
| # Lanes Instrumented | 2 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | June 05 |
| Calibration Interval | 6 mths. (or as needed) |



Woodburn POE SB

| | |
|--------------------------|------------------------|
| Location (MP) | I-5 (274.18) |
| ADT | 44,748 |
| ADTT | 5,689 |
| # Lanes | 3 |
| # Lanes Instrumented | 2 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | June 05 |
| Calibration Interval | 6 mths. (or as needed) |



Wilbur SB

| | |
|--------------------------|------------------------|
| Location (MP) | 130.03 |
| ADT | 19,244 |
| ADTT | 2,602 |
| # Lanes | 2 |
| # Lanes Instrumented | 2 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept. 05 |
| Calibration Interval | 6 mths. (or as needed) |



Figure C3.1: Oregon WIM site data and locations.

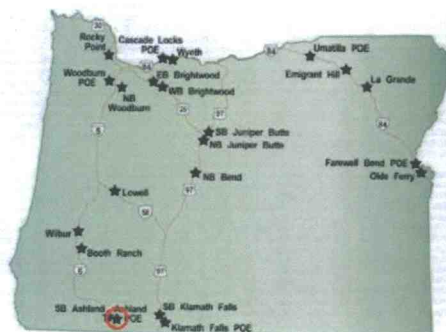
Booth Ranch NB

| | |
|--------------------------|------------------------|
| Location (MP) | 111.07 |
| ADT | 12,619 |
| ADTT | 3,442 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Aug 05 |
| Calibration Interval | 6 mths. (or as needed) |



Ashland POE NB

| | |
|--------------------------|------------------------|
| Location (MP) | 18.08 |
| ADT | 11,710 |
| ADTT | 2,979 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Dec 05 |
| Calibration Interval | 6 mths. (or as needed) |



Ashland SB

| | |
|--------------------------|------------------------|
| Location (MP) | 18.24 |
| ADT | 11,776 |
| ADTT | 2,838 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Dec 05 |
| Calibration Interval | 6 mths. (or as needed) |

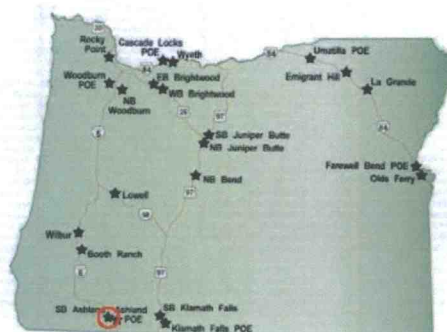


Figure C3.1 (Continued): Oregon WIM site data and locations.

Interstate 84

Cascade Locks POE EB

| | |
|--------------------------|------------------------|
| Location (MP) | 44.93 |
| ADT | 9,880 |
| ADTT | 4,602 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept 05 |
| Calibration Interval | 6 mths. (or as needed) |



Wyeth WB

| | |
|--------------------------|------------------------|
| Location (MP) | 54.30 |
| ADT | 7011 |
| ADTT | 2,158 |
| # Lanes | 2 |
| # Lanes Instrumented | 2 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Oct 05 |
| Calibration Interval | 6 mths. (or as needed) |



Emigrant Hill WB

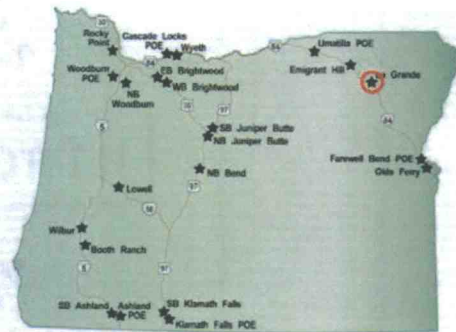
| | |
|--------------------------|------------------------|
| Location (MP) | 226.95 |
| ADT | 3,252 |
| ADTT | 1,786 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Oct 05 |
| Calibration Interval | 6 mths. (or as needed) |



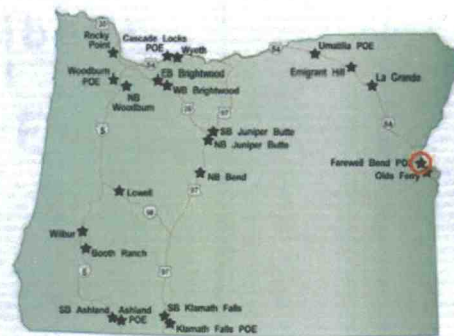
Figure C3.1 (Continued): Oregon WIM site data and locations.

La Grande EB

| | |
|--------------------------|------------------------|
| Location (MP) | 258.52 |
| ADT | 3,972 |
| ADTT | 2,327 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept 05 |
| Calibration Interval | 6 mths. (or as needed) |

**Farewell Bend POE WB**

| | |
|--------------------------|------------------------|
| Location (MP) | 353.31 |
| ADT | 2,866 |
| ADTT | 1,848 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept 05 |
| Calibration Interval | 6 mths. (or as needed) |

**Olds Ferry EB**

| | |
|--------------------------|------------------------|
| Location (MP) | 354.38 |
| ADT | 3,458 |
| ADTT | 2,045 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept 05 |
| Calibration Interval | 6 mths. (or as needed) |

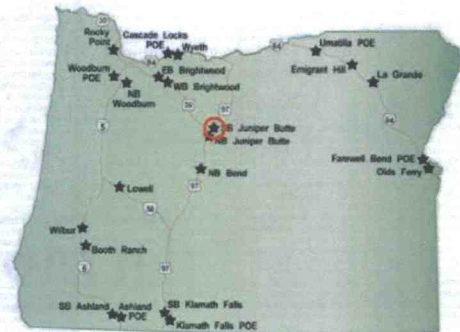


Figure C3.1 (Continued): Oregon WIM site data and locations.

US Highway 97

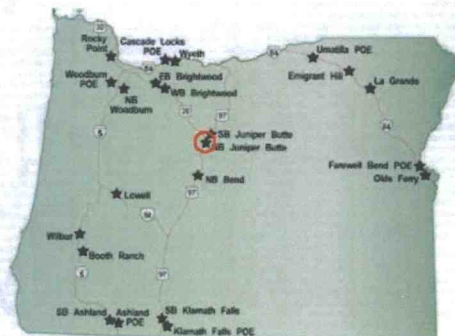
Juniper Butte SB

| | |
|--------------------------|------------------------|
| Location (MP) | 108.20 |
| ADT | 4,967 |
| ADTT | 935 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Nov 05 |
| Calibration Interval | 6 mths. (or as needed) |



Juniper Butte NB

| | |
|--------------------------|------------------------|
| Location (MP) | 106.90 |
| ADT | 4,792 |
| ADTT | 882 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Nov 05 |
| Calibration Interval | 6 mths. (or as needed) |



Bend NB

| | |
|--------------------------|------------------------|
| Location (MP) | 145.50 |
| ADT | 6,943 |
| ADTT | 607 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Oct 05 |
| Calibration Interval | 6 mths. (or as needed) |

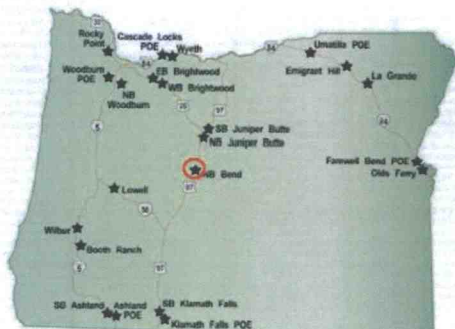


Figure C3.1 (Continued): Oregon WIM site data and locations.

Klamath Falls SB

| | |
|--------------------------|------------------------|
| Location (MP) | 271.41 |
| ADT | 3,129 |
| ADTT | 907 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Oct 05 |
| Calibration Interval | 6 mths. (or as needed) |

**Klamath Falls POE NB**

| | |
|--------------------------|------------------------|
| Location (MP) | 271.73 |
| ADT | 3,857 |
| ADTT | 769 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Oct 05 |
| Calibration Interval | 6 mths. (or as needed) |

**OR Highway 58****Lowell WB**

| | |
|--------------------------|------------------------|
| Location (MP) | 17.17 |
| ADT | 3,205 |
| ADTT | 581 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Nov 05 |
| Calibration Interval | 6 mths. (or as needed) |



Figure C3.1 (Continued): Oregon WIM site data and locations.

US Highway 26

Brightwood EB

| | |
|--------------------------|------------------------|
| Location (MP) | 36.51 |
| ADT | 4,761 |
| ADTT | 357 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept 05 |
| Calibration Interval | 6 mths. (or as needed) |



Brightwood WB

| | |
|--------------------------|------------------------|
| Location (MP) | 36.31 |
| ADT | 4,360 |
| ADTT | 787 |
| # Lanes | 2 |
| # Lanes Instrumented | 1 |
| WIM Equipment | Single Load Cell |
| Date of Last Calibration | Sept 05 |
| Calibration Interval | 6 mths. (or as needed) |



Figure C3.1 (Continued): Oregon WIM site data and locations.

APPENDIX D

Inverse Normal Distribution for Shear
I-5 Booth Ranch NB - 50-ft Simple Span
Top 20% of January 1st - December 31st, 2005

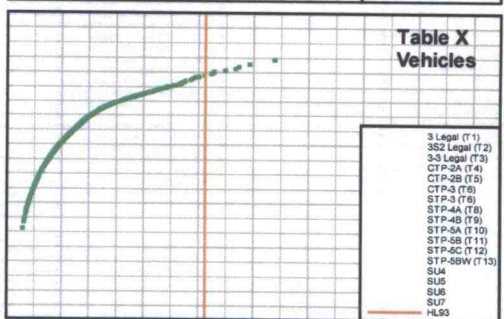
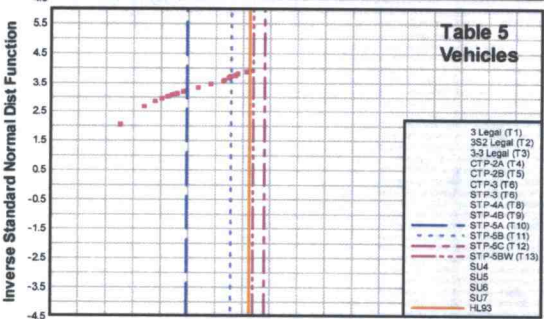
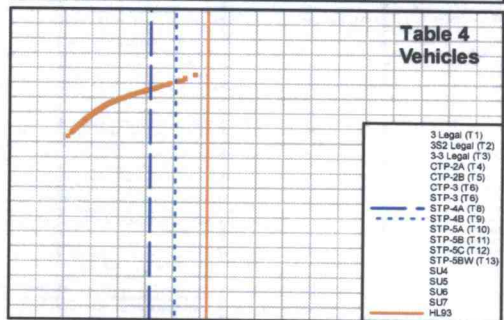
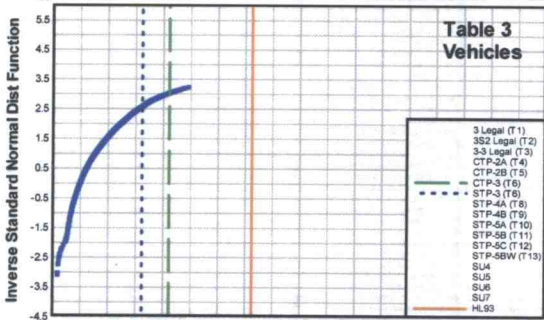
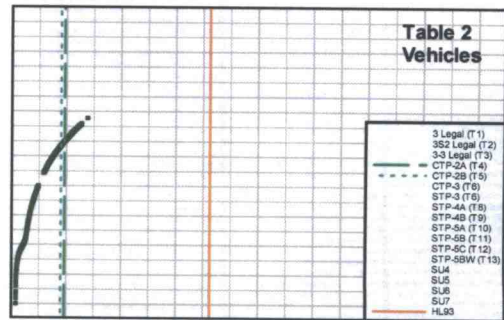
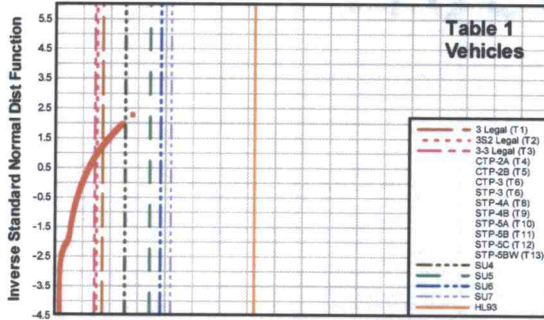
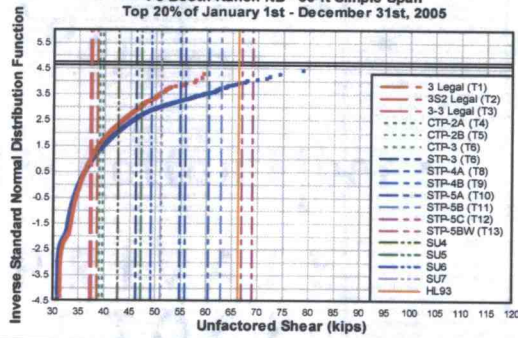


Figure D3.1: CDF plots for unfactored shear for 50-ft simple span bridge model.

Inverse Normal Distribution for Shear
I-5 Booth Ranch NB - 50-ft Simple Span
Top 20% of January 1st - December 31st, 2005

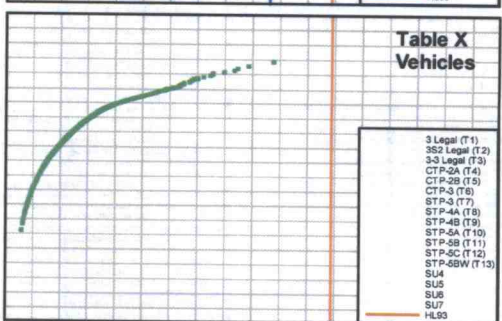
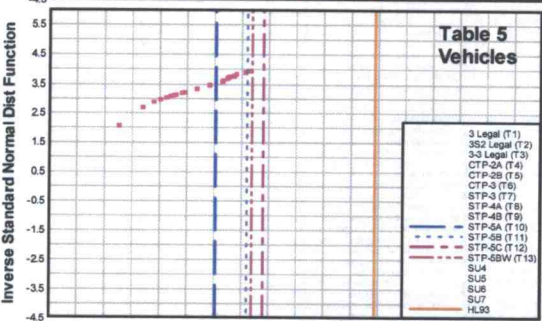
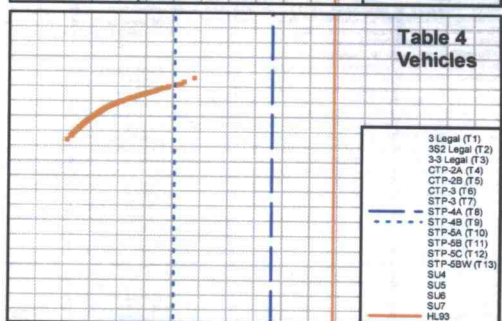
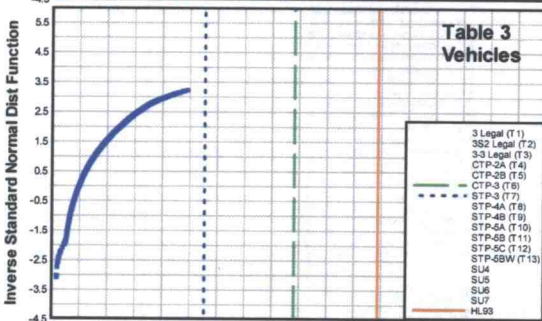
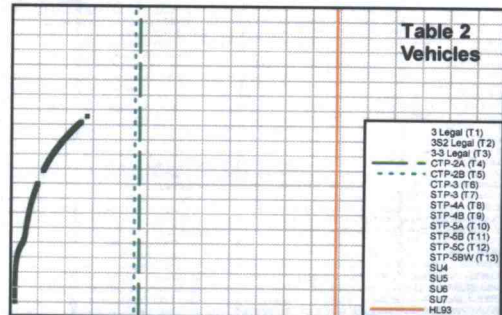
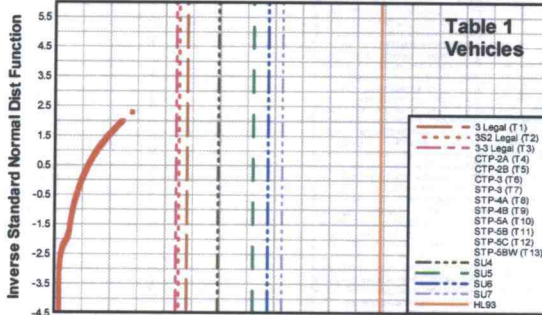
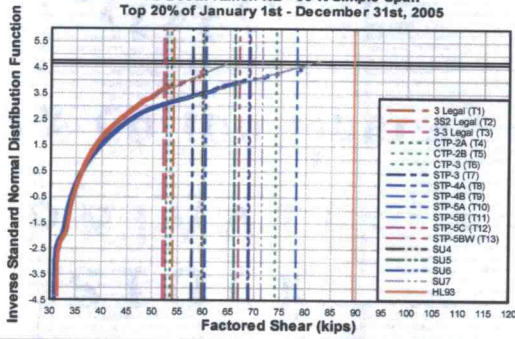


Figure D3.2: CDF plots for factored shear for 50-ft simple span bridge model.

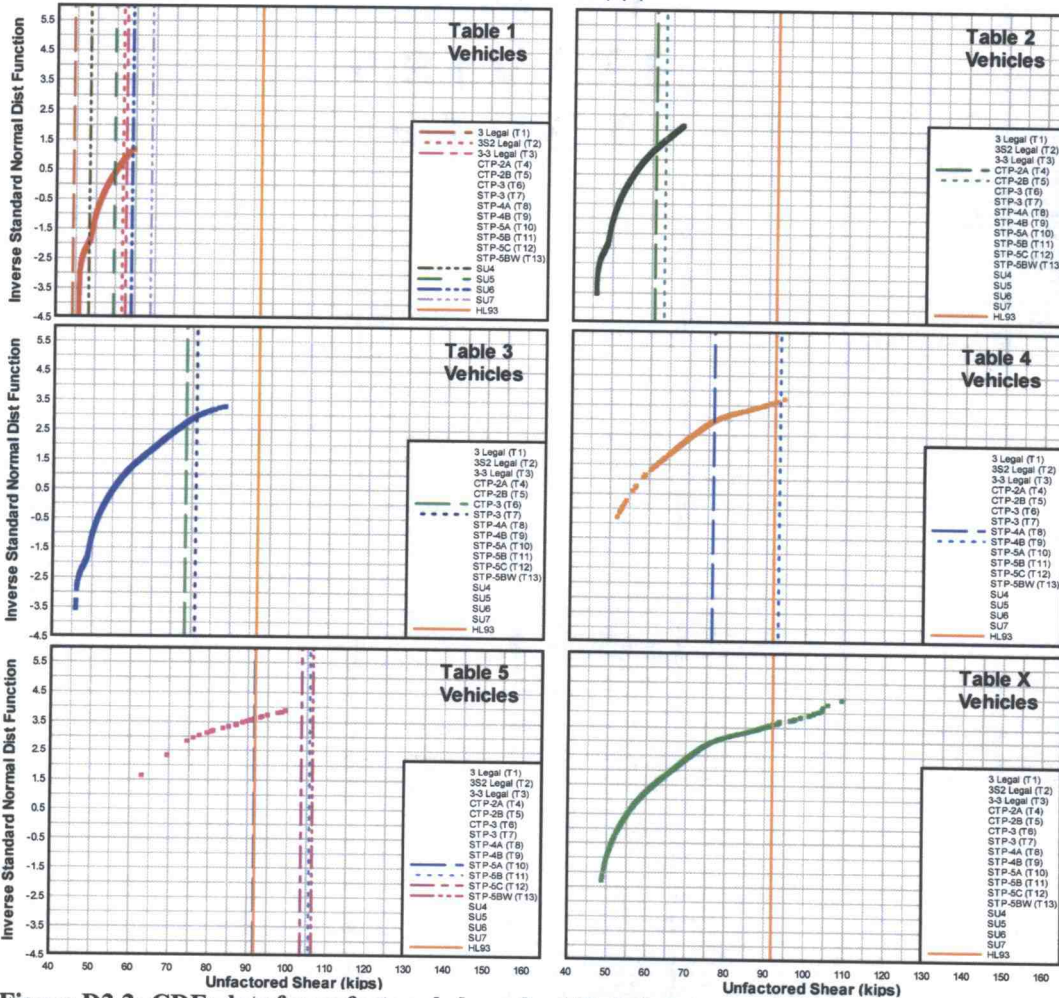
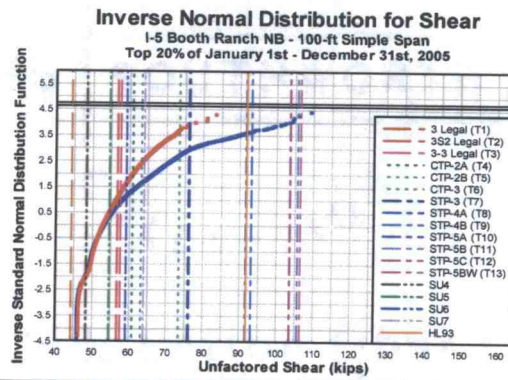


Figure D3.3: CDF plots for unfactored shear for 100-ft simple span bridge model.

Inverse Normal Distribution for Shear
 I-5 Booth Ranch NB - 100-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

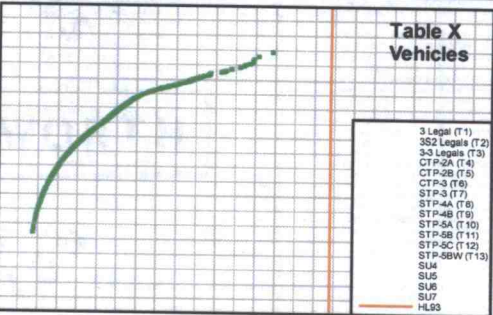
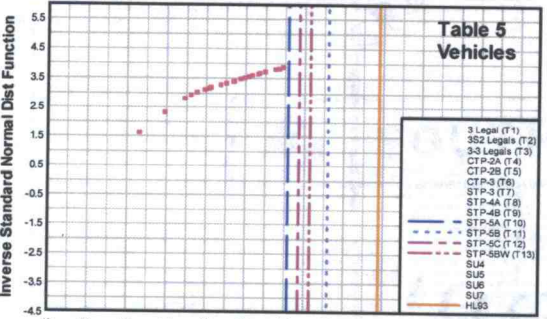
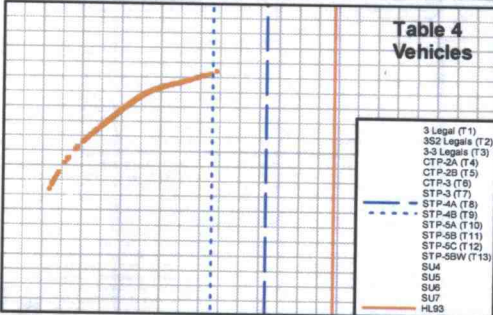
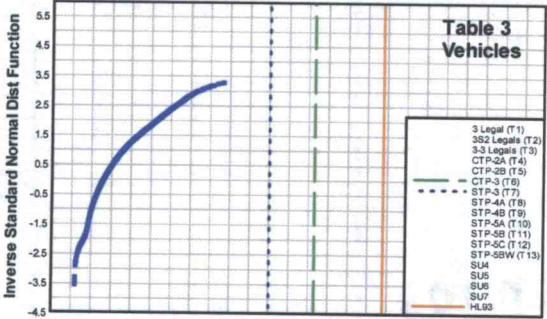
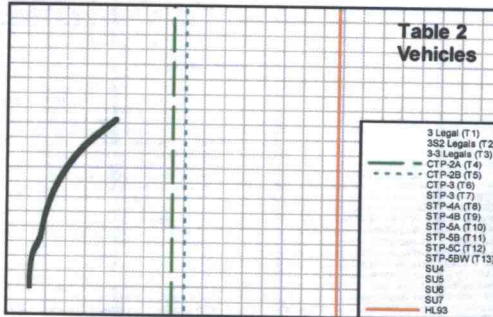
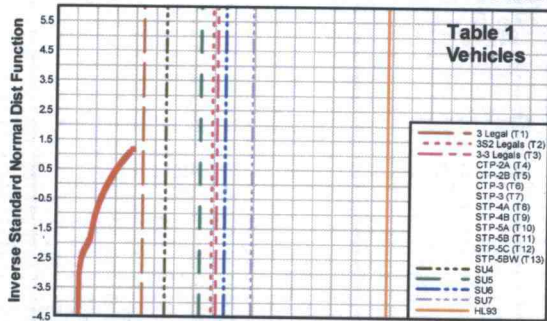
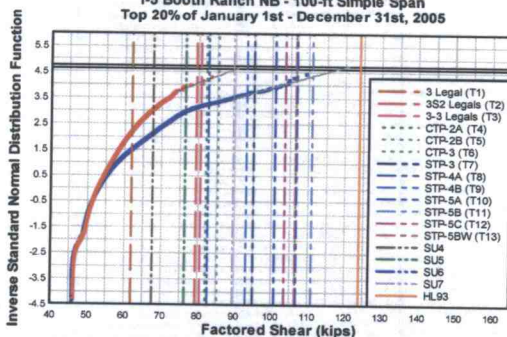


Figure D3.4: CDF plots for factored shear for 100-ft simple span bridge model.

Inverse Normal Distribution for Shear
 I-6 Booth Ranch NB - 150-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

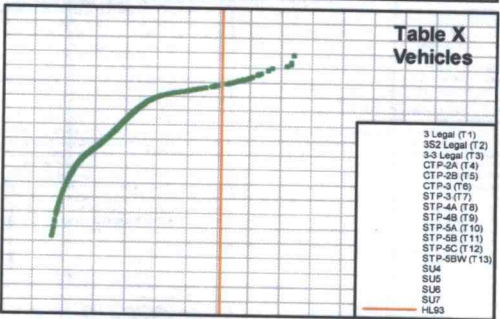
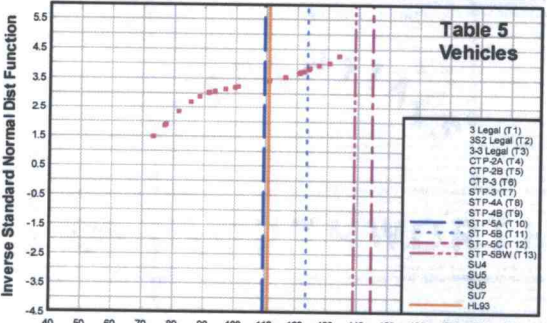
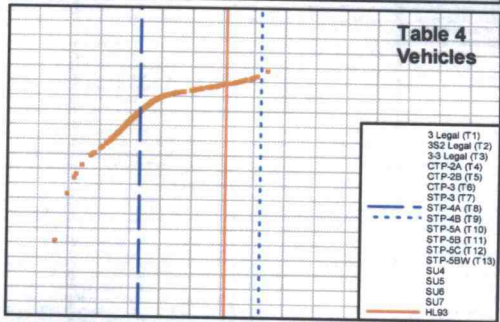
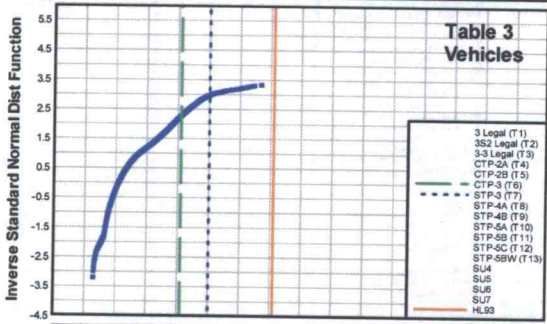
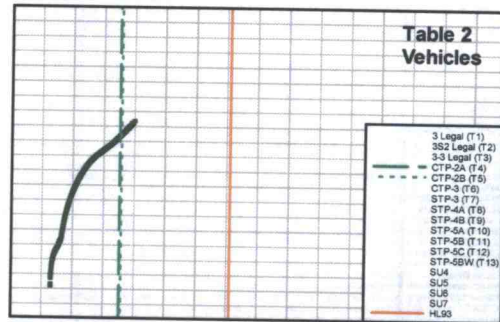
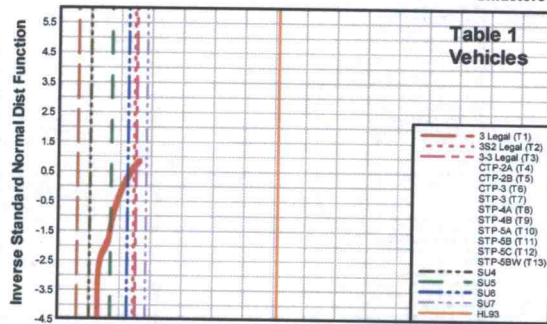
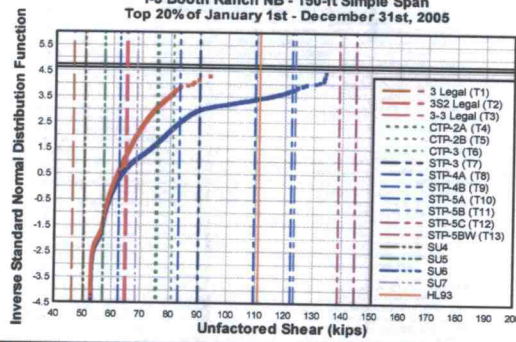


Figure D3.5: CDF plots for unfactored shear for 150-ft simple span bridge model.

Inverse Normal Distribution for Shear
I-5 Booth Ranch NB - 150-ft Simple Span
Top 20% of January 1st - December 31st, 2005

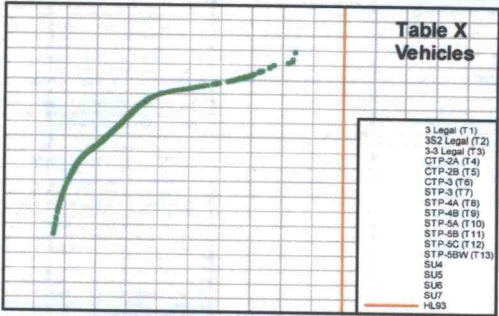
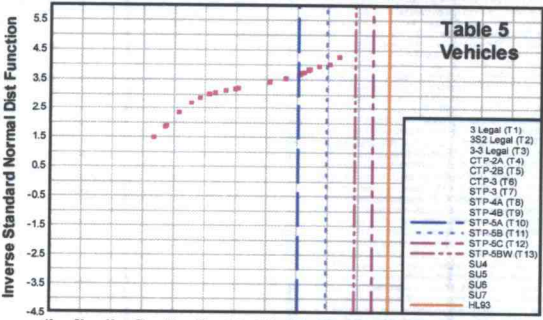
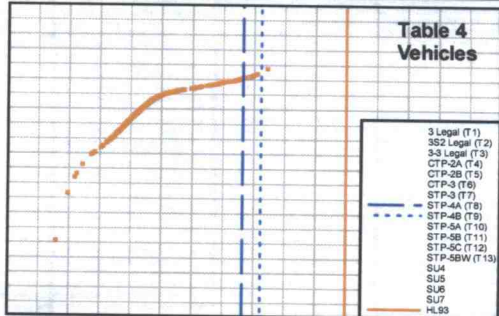
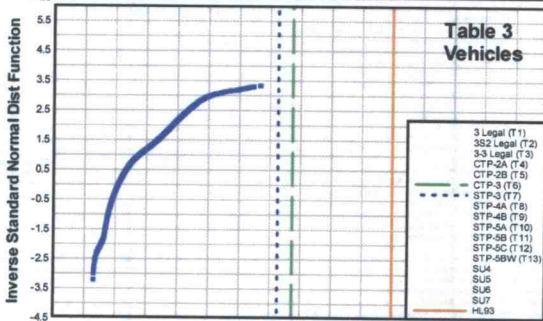
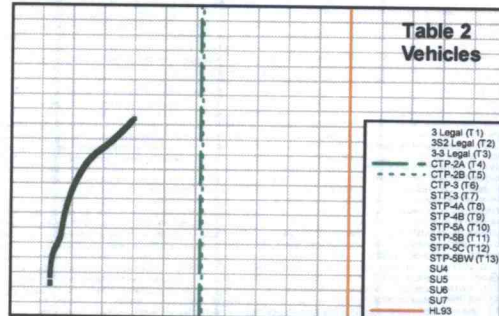
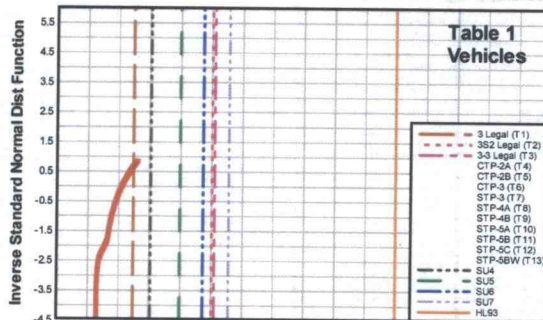
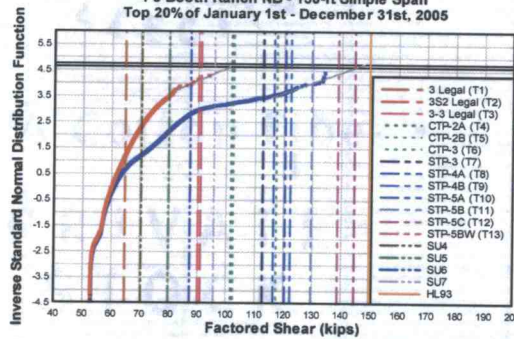


Figure D3.6: CDF plots for factored shear for 150-ft simple span bridge model.

Inverse Normal Distribution for Shear
 I-5 Booth Ranch NB - 200-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

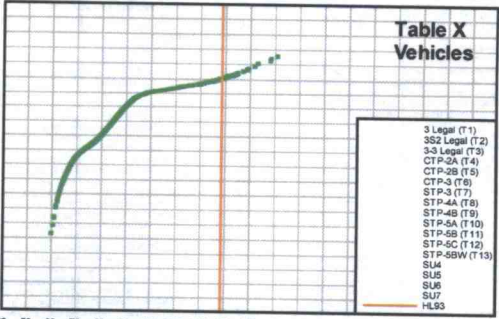
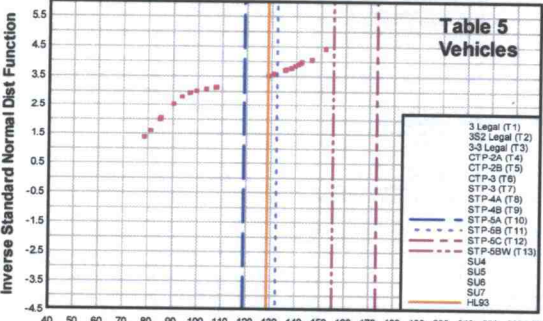
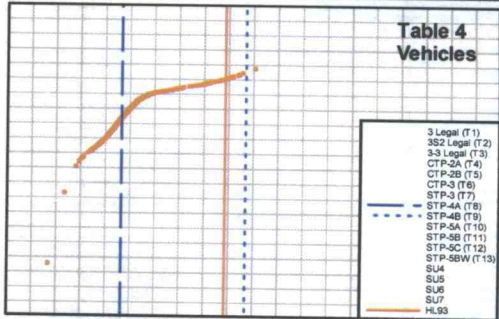
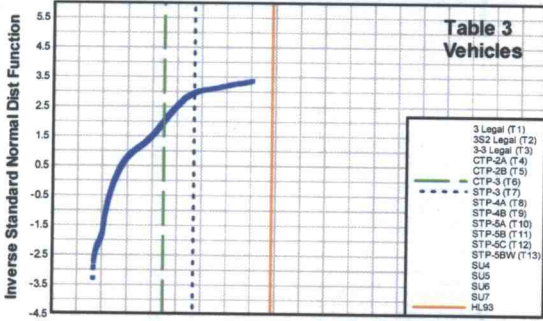
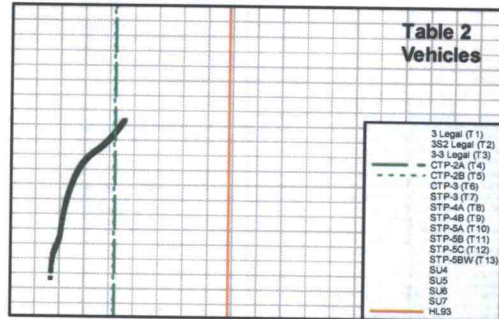
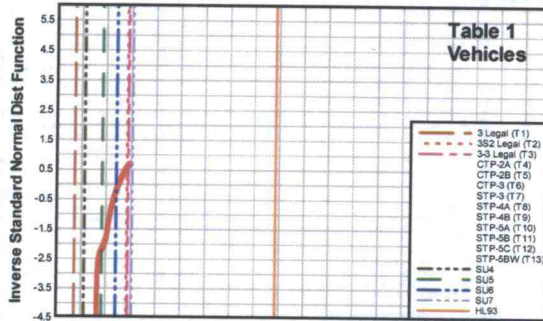
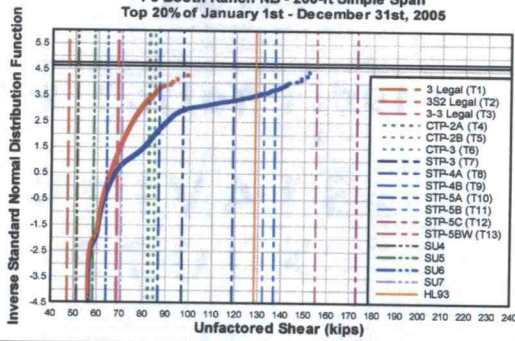


Figure D3.7: CDF plots for unfactored shear for 200-ft simple span bridge model.

Inverse Normal Distribution for Shear
 I-5 Booth Ranch NB - 200-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

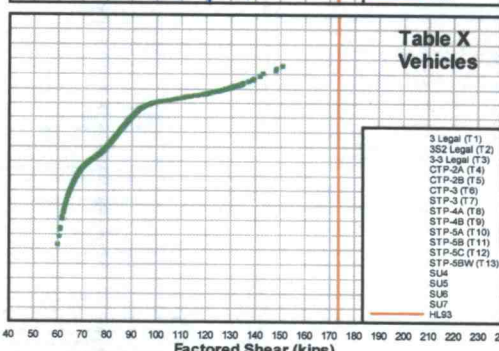
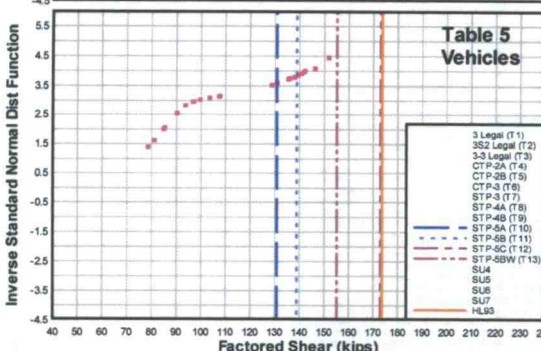
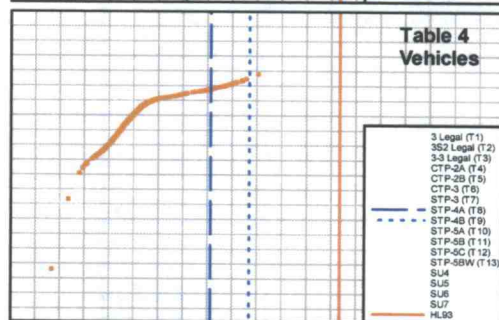
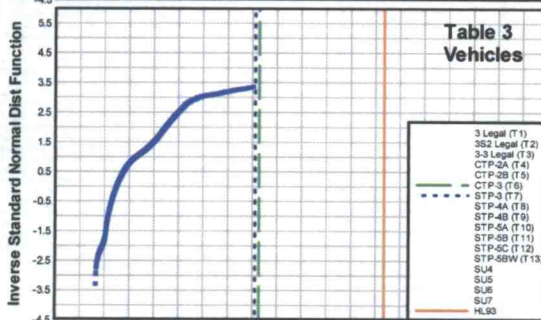
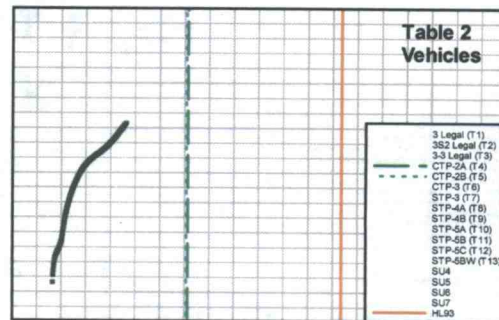
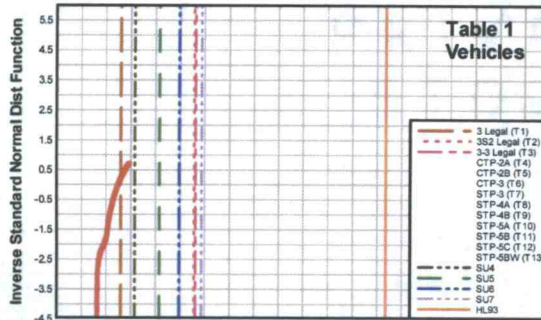
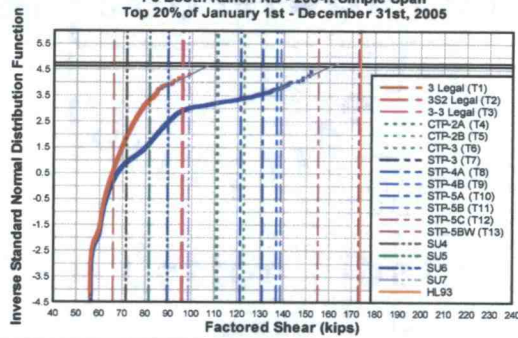


Figure D3.8: CDF plots for factored shear for 200-ft simple span bridge model.

Inverse Normal Distribution for Moment

I-5 Booth Ranch NB - 50-ft Simple Span
Top 20% of January 1st - December 31st, 2005

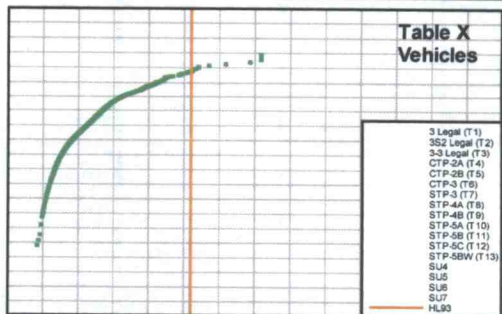
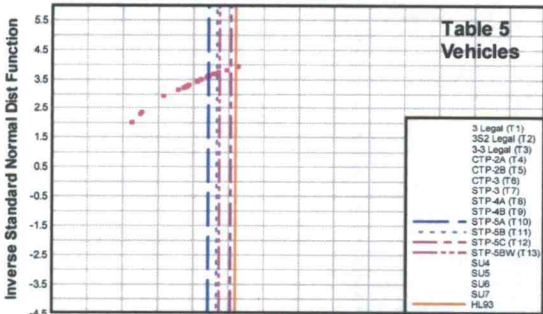
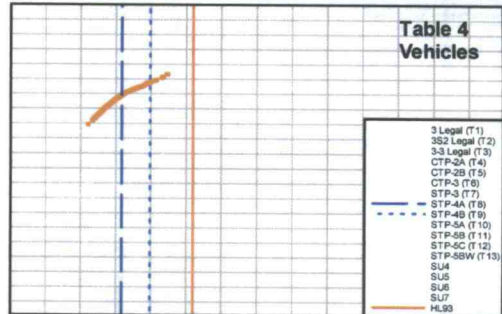
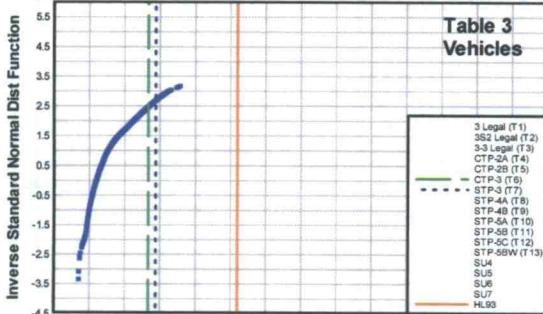
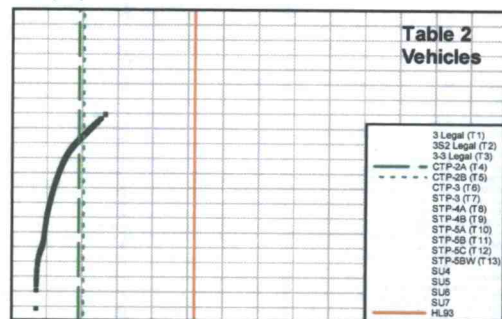
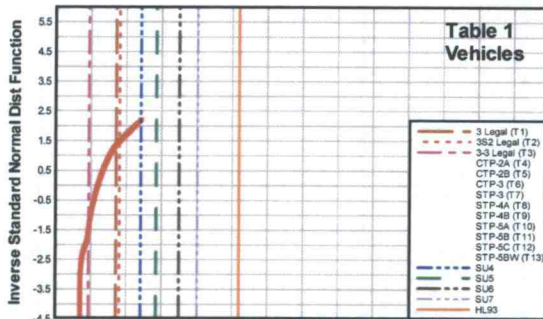
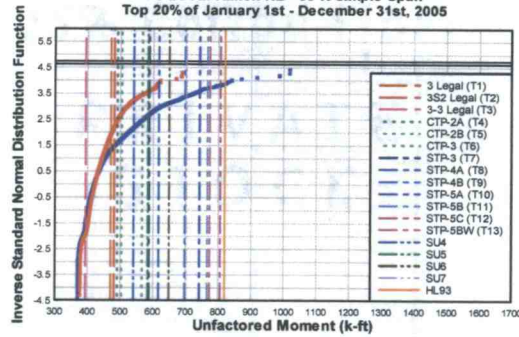


Figure D3.9: CDF plots for unfactored moment for 50-ft simple span bridge model.

Inverse Normal Distribution for Moment
 I-5 Booth Ranch NB - 50-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

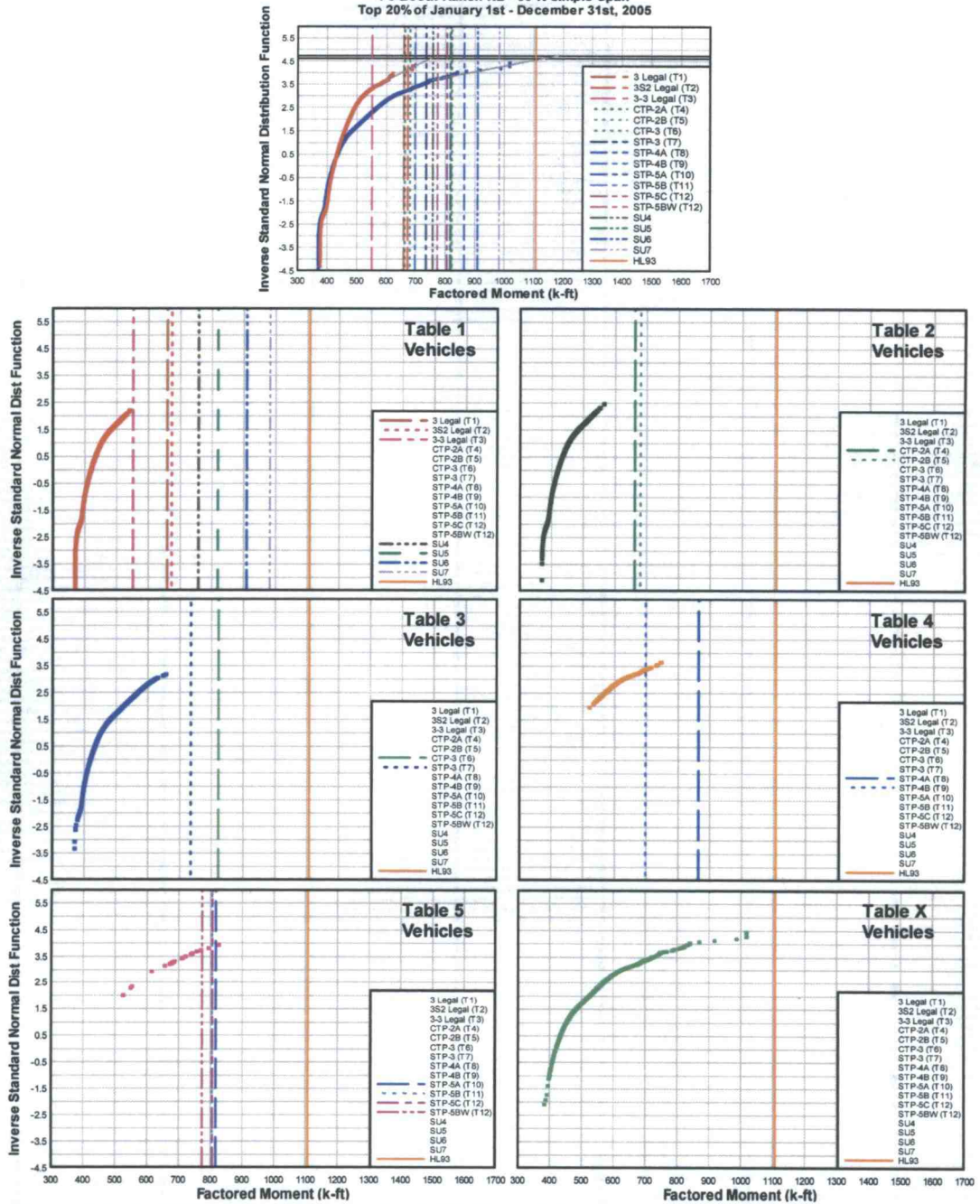


Figure D3.10: CDF plots for factored moment for 50-ft simple span bridge model.

Inverse Normal Distribution for Moment
 I-5 Booth Ranch NB - 100-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

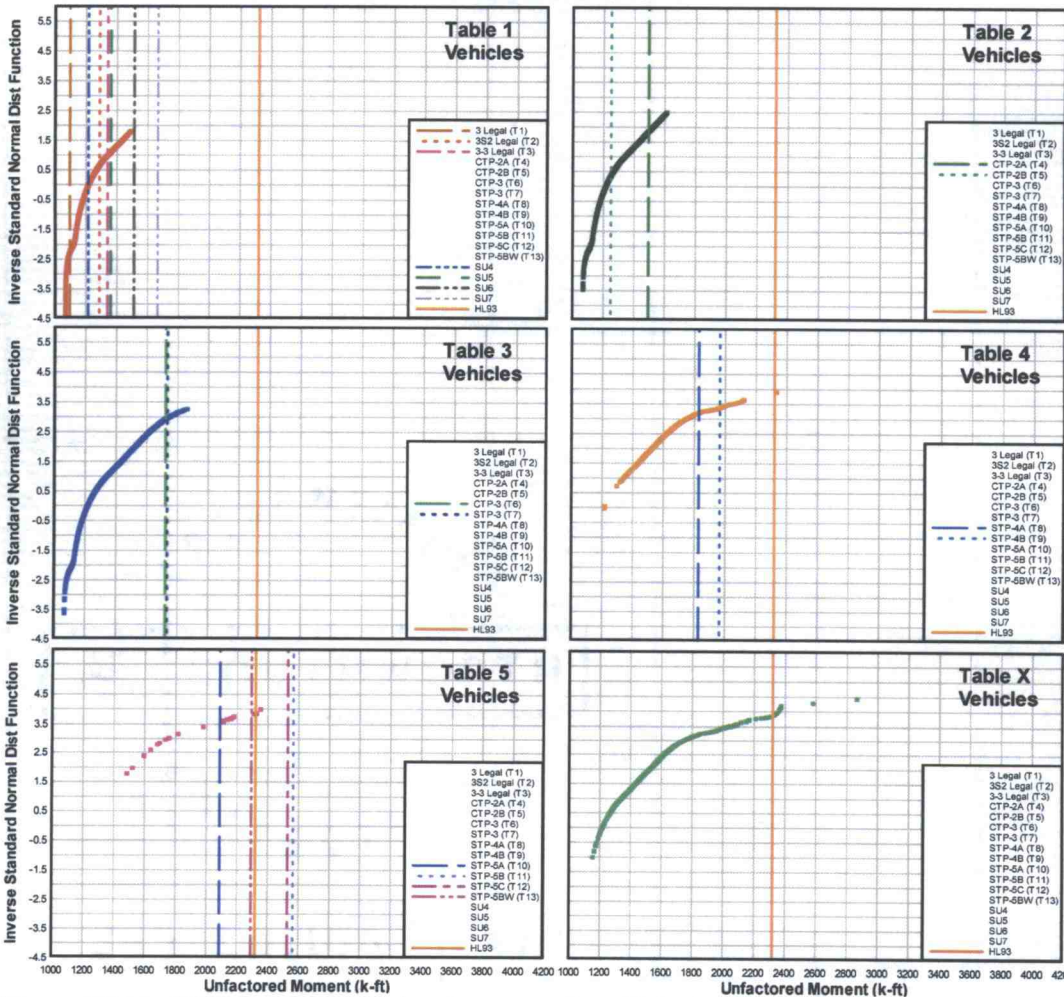
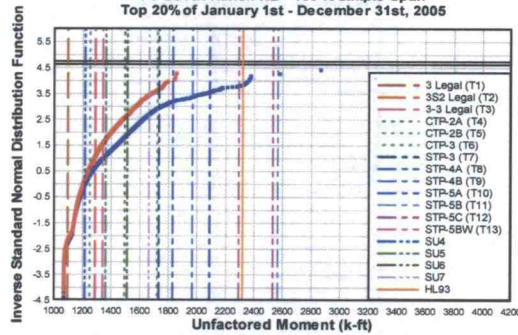


Figure D3.11: CDF plots for unfactored moment for 100-ft simple span bridge model.

Inverse Normal Distribution for Moment
I-5 Booth Ranch NB - 100-ft Simple Span
Top 20% of January 1st - December 31st, 2005

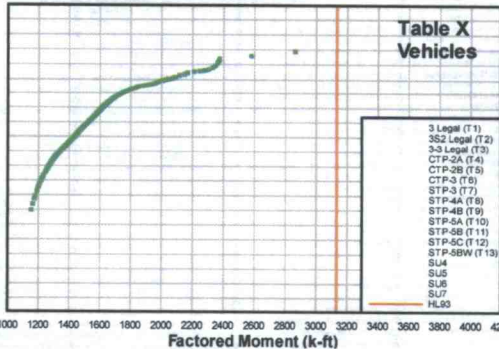
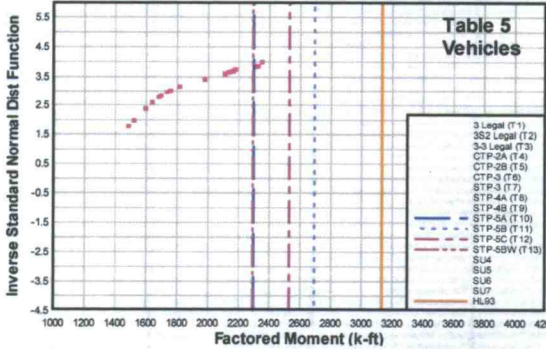
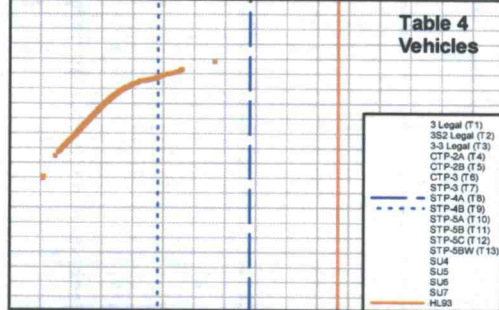
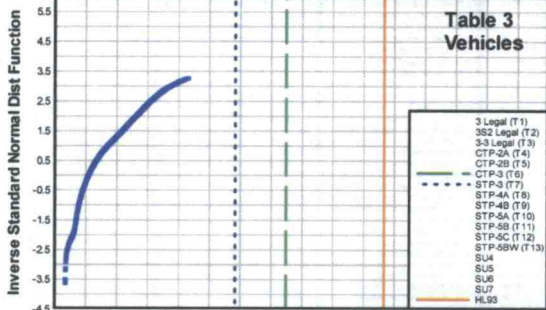
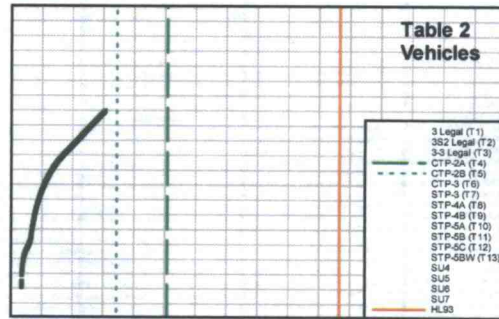
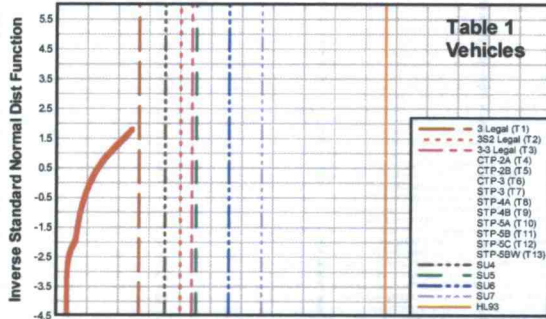
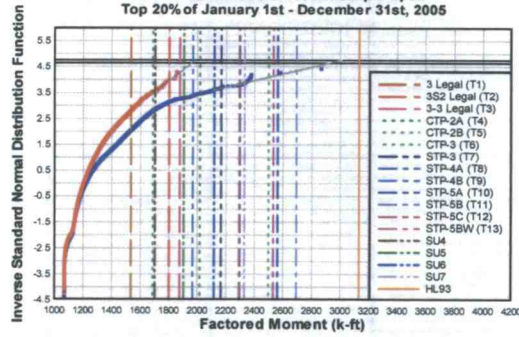


Figure D3.12: CDF plots for factored moment for 100-ft simple span bridge model.

Inverse Normal Distribution for Moment

I-5 Booth Ranch NB - 150-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

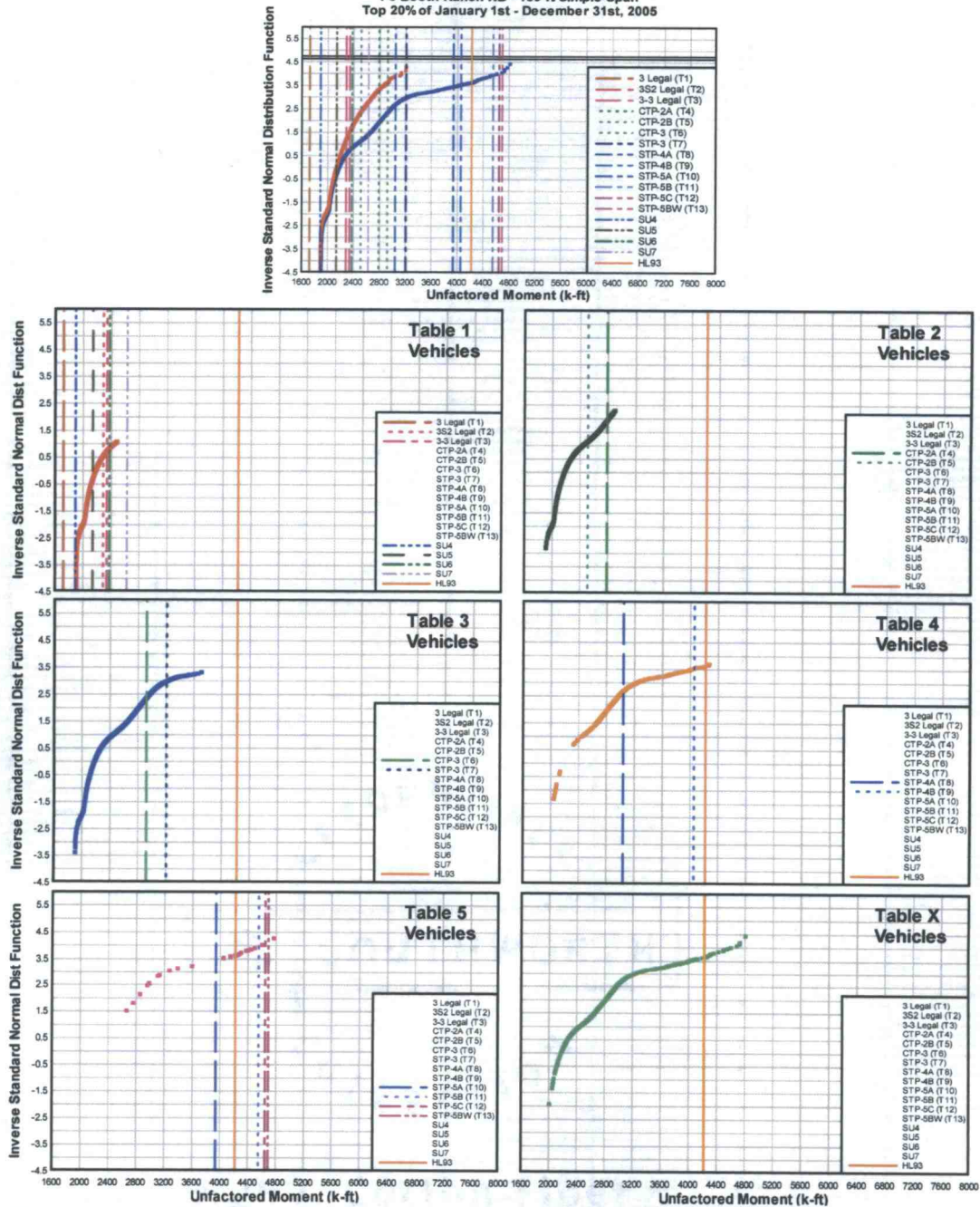


Figure D3.13: CDF plots for unfactored moment for 150-ft simple span bridge model.

Inverse Normal Distribution for Moment
I-5 Booth Ranch NB - 150-ft Simple Span
Top 20% of January 1st - December 31st, 2005

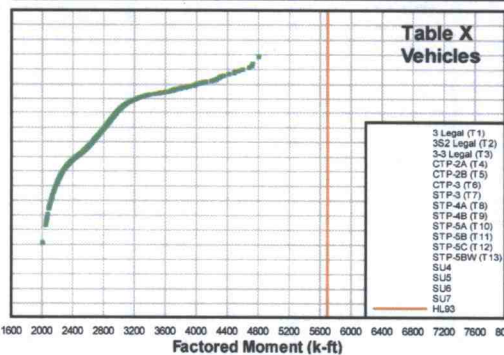
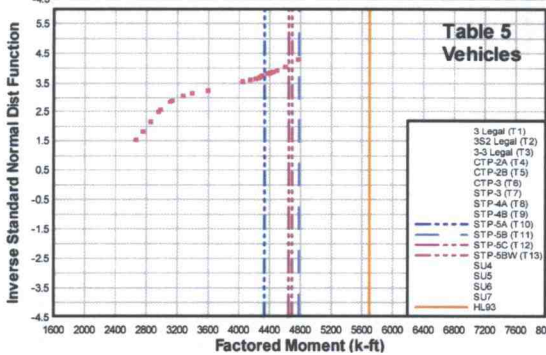
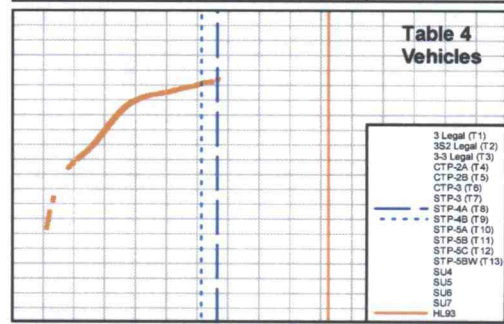
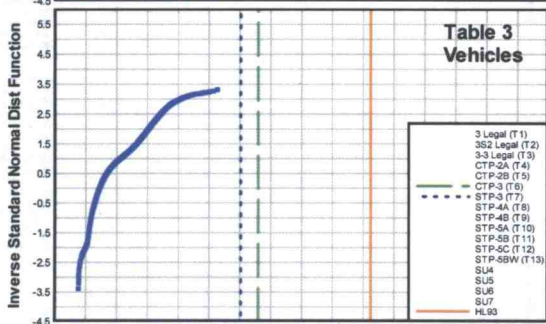
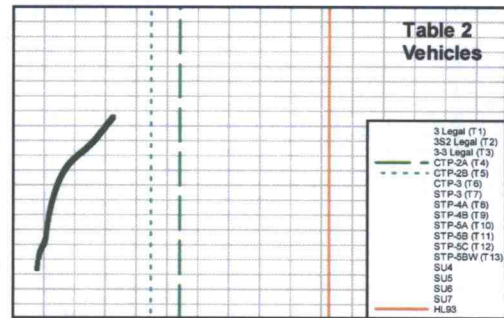
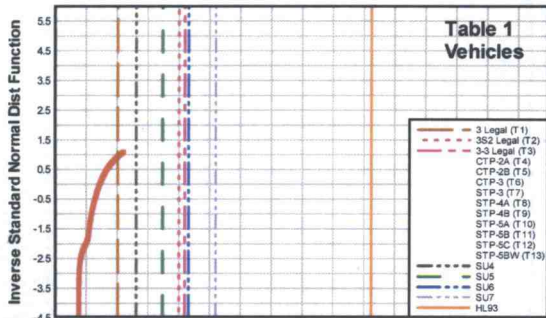
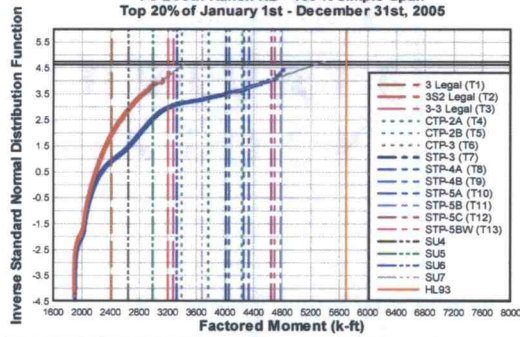


Figure D3.14: CDF plots for factored moment for 150-ft simple span bridge model.

Inverse Normal Distribution for Moment
 I-5 Booth Ranch NB - 200-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

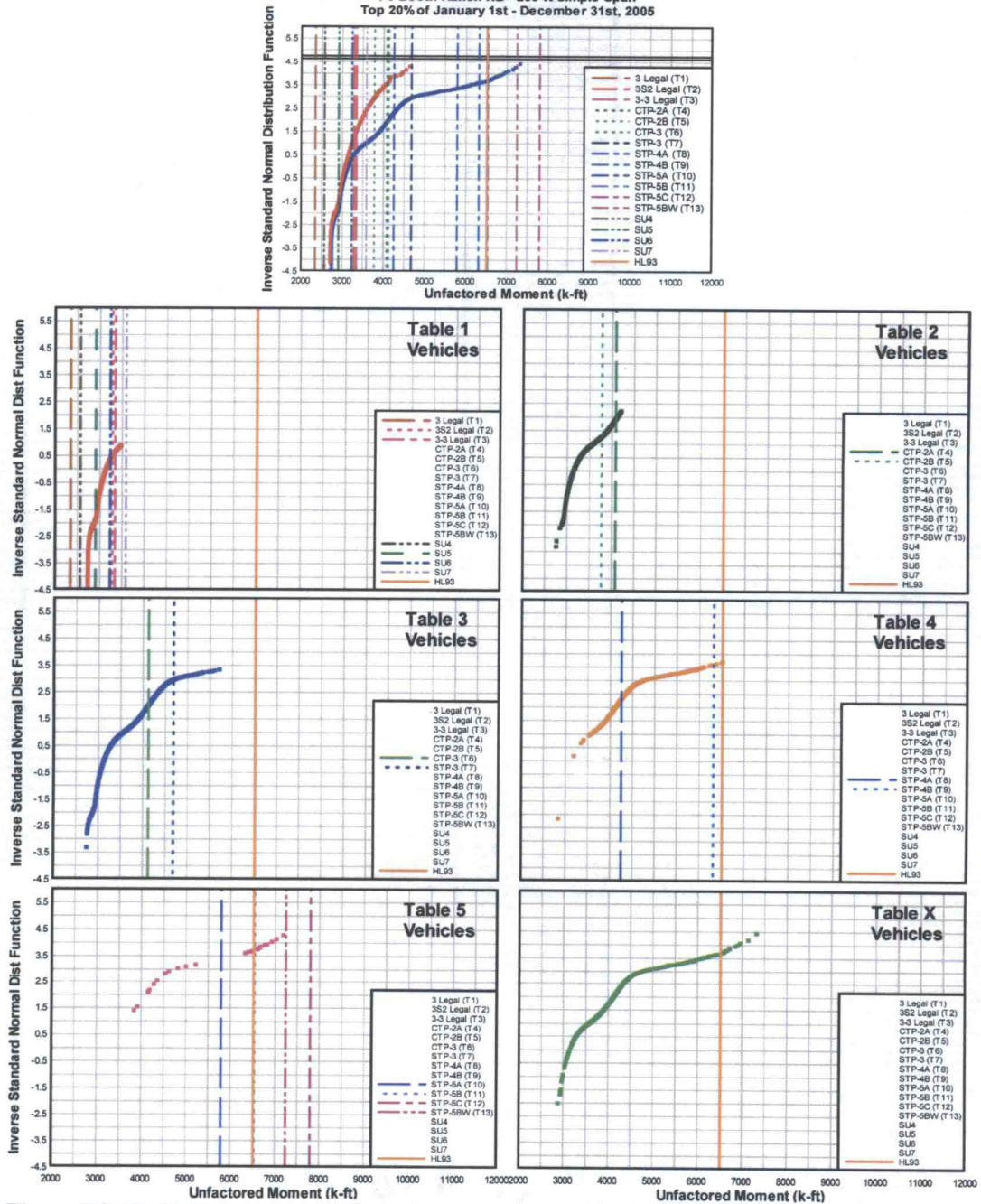


Figure D3.15: CDF plots for unfactored moment for 200-ft simple span bridge model.

Inverse Normal Distribution for Moment
 I-5 Booth Ranch NB - 200-ft Simple Span
 Top 20% of January 1st - December 31st, 2005

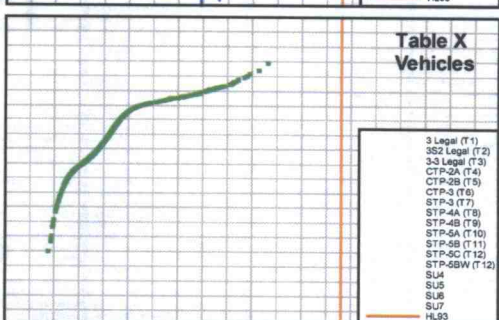
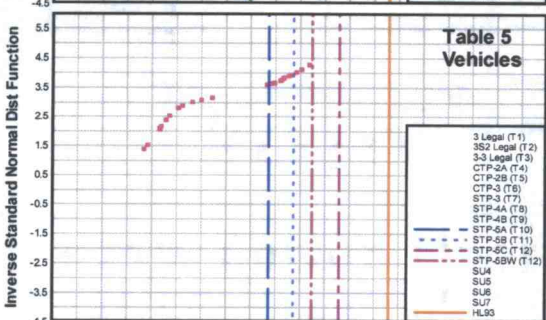
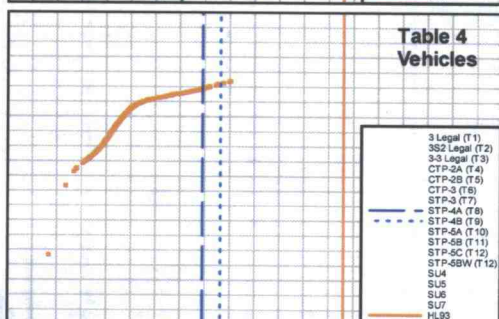
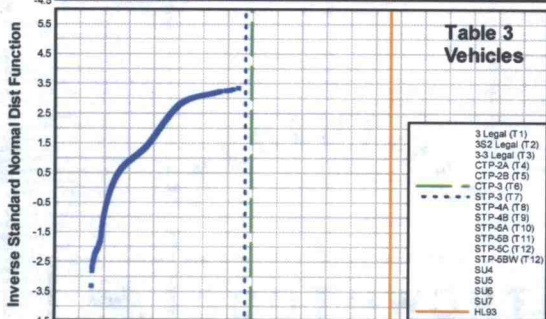
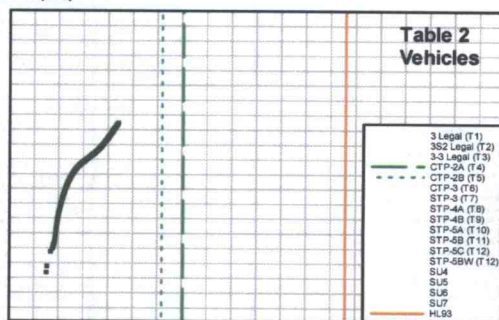
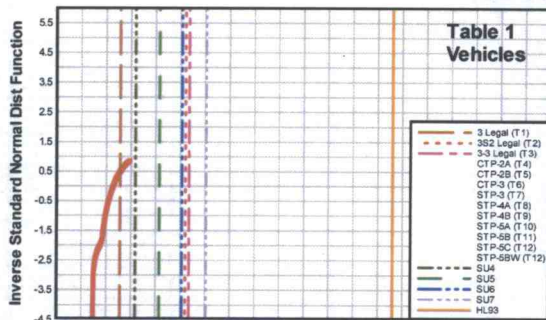
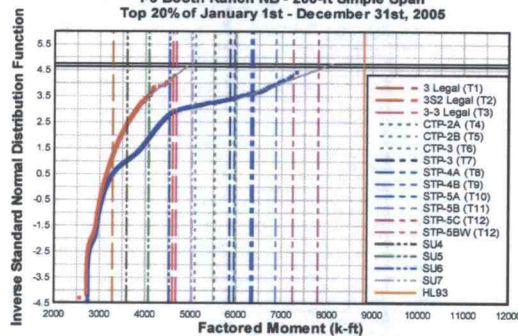


Figure D3.16: CDF plots for factored moment for 200-ft simple span bridge model.

Inverse Normal Distribution for Shear
 I-5 Booth Ranch NB - 2-Span Continuous - 50-ft Spans
 Top 20% of January 1st - December 31st, 2005

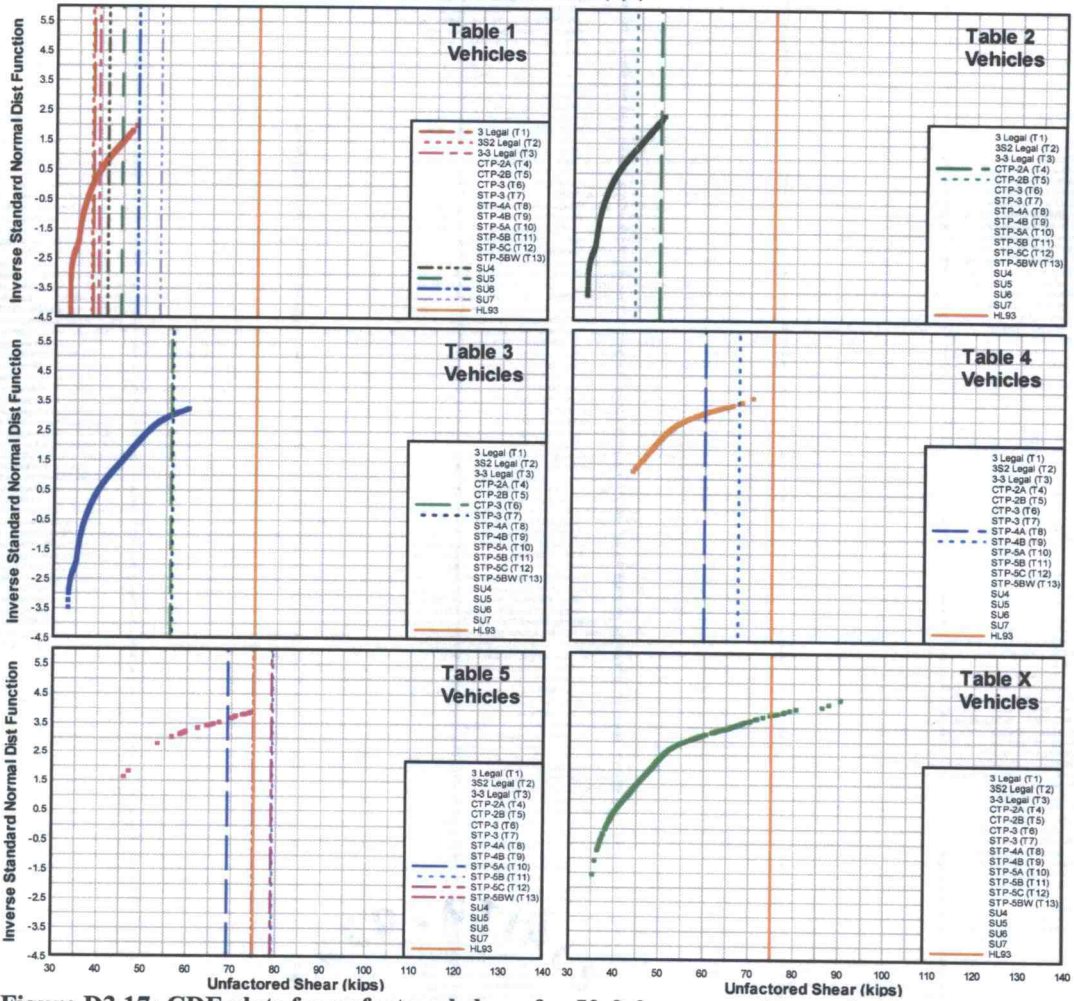
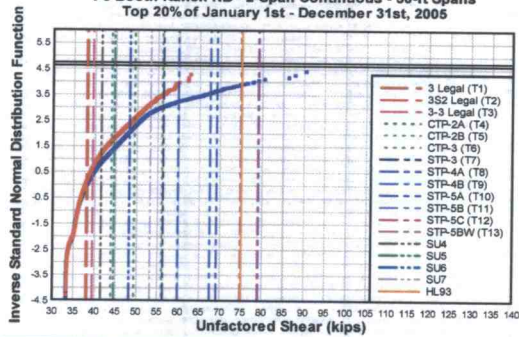


Figure D3.17: CDF plots for unfactored shear for 50-ft 2-span continuous bridge model.

Inverse Normal Distribution for Shear
I-5 Booth Ranch NB - 2-Span Continuous - 50-ft Spans
Top 20% of January 1st - December 31st, 2005

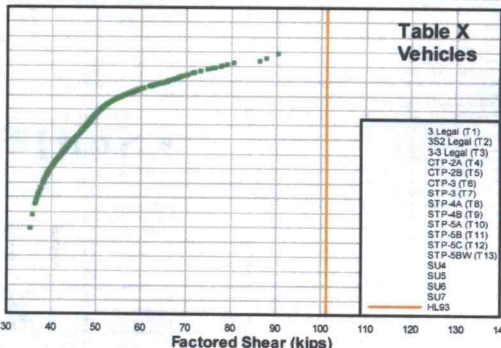
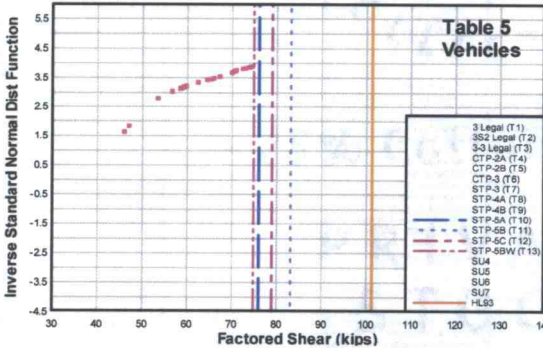
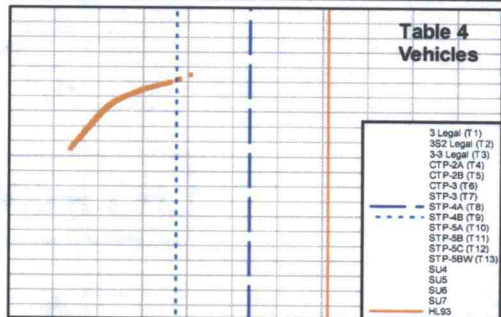
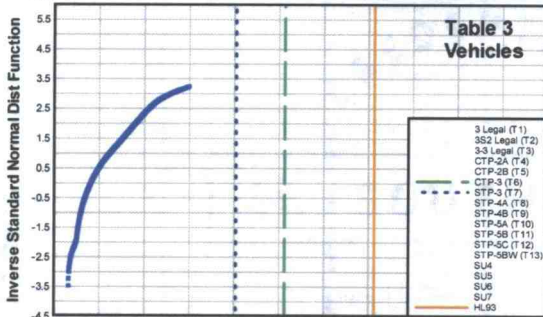
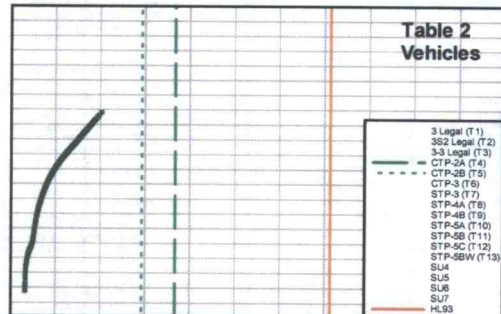
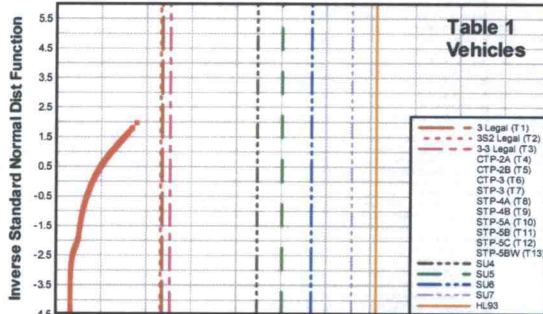
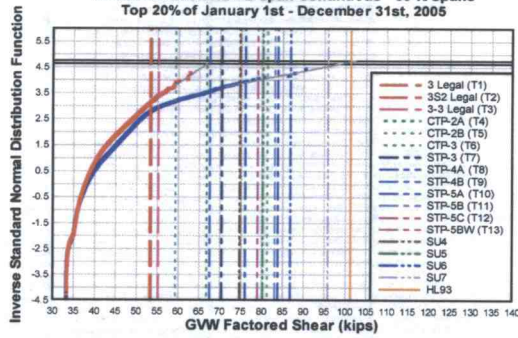


Figure D3.18: CDF plots for factored shear for 50-ft 2-span continuous bridge model.

Inverse Normal Distribution for Negative Moment
 I-5 Booth Ranch NB - 2-Span Continuous - 50-ft Spans
 Top 20% of January 1st - December 31st, 2005

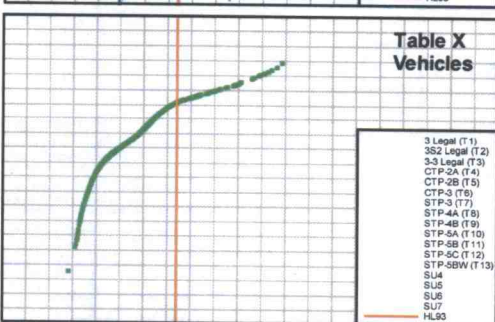
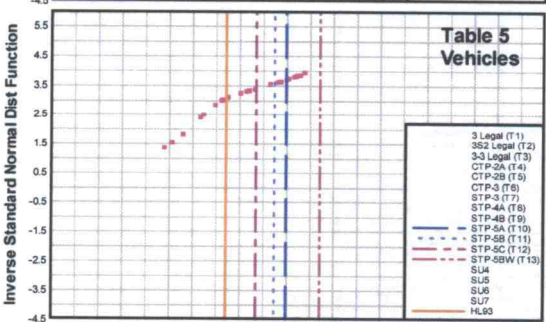
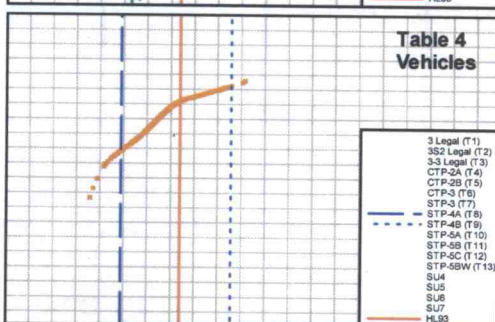
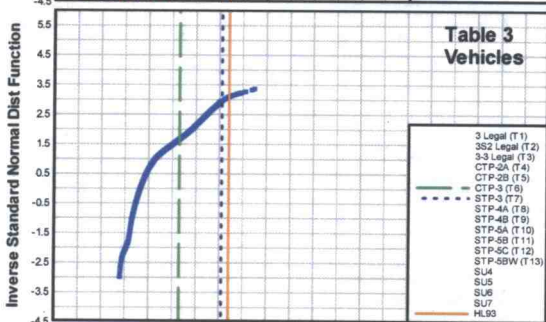
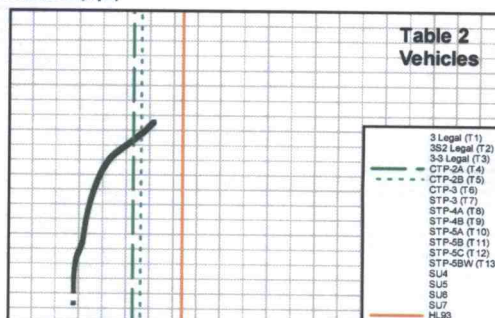
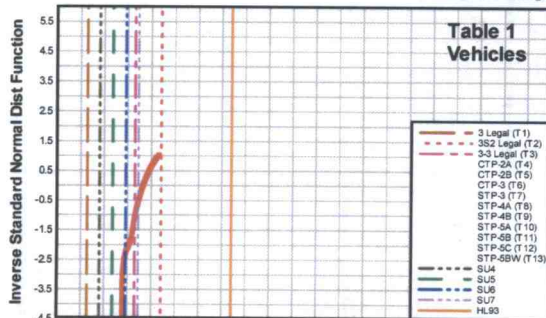
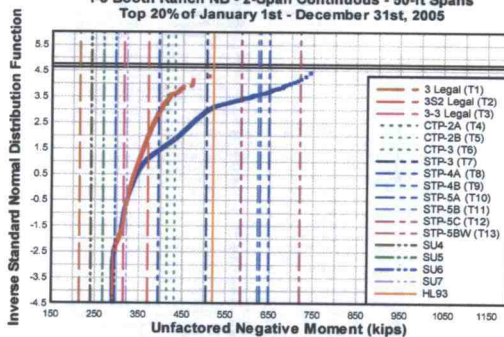


Figure D3.19: CDF plots for unfactored negative moment for 50-ft 2-span continuous bridge model.

Inverse Normal Distribution for Negative Moment
 I-5 Booth Ranch NB - 2-Span Continuous - 50-ft Spans
 Top 20% of January 1st - December 31st, 2005

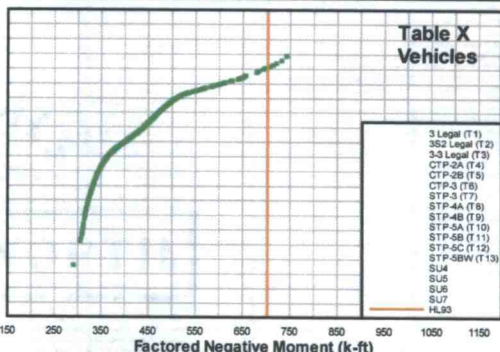
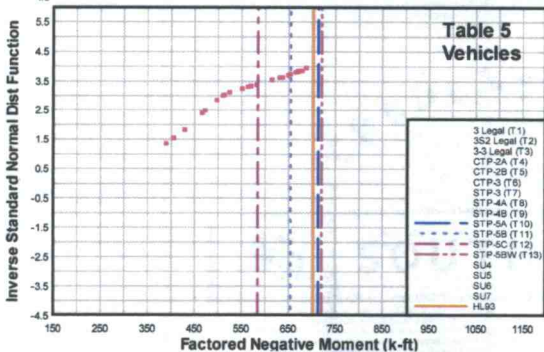
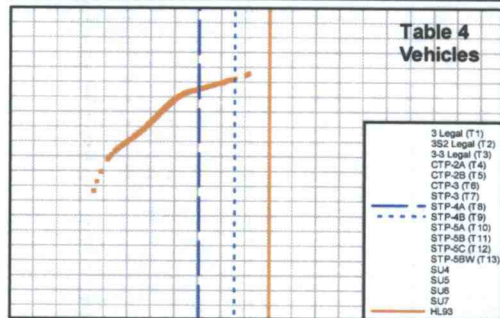
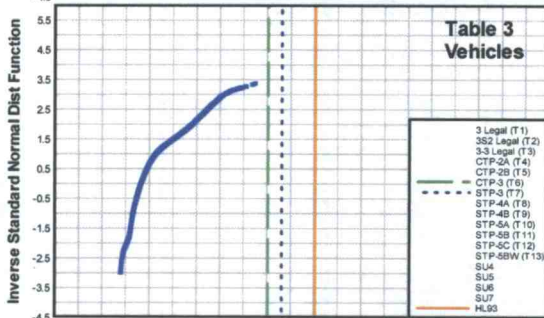
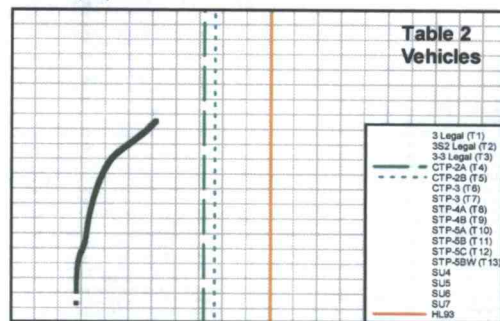
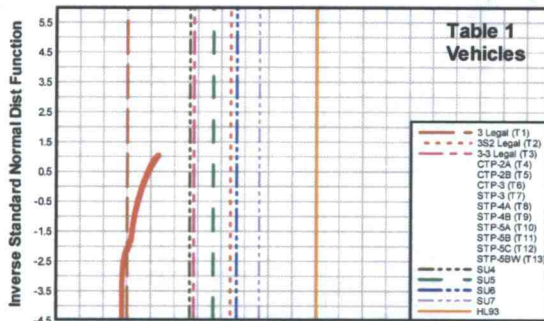
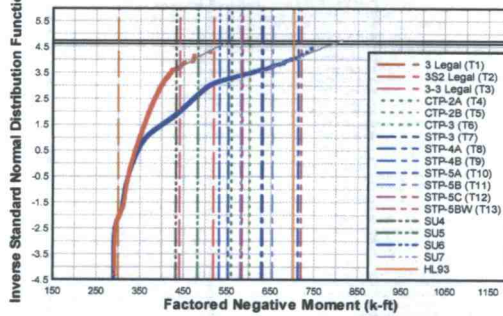


Figure D3.20: CDF plots for factored negative moment for 50-ft 2-span continuous bridge model.

General Conclusions

An investigation of Oregon's weigh-in-motion (WIM) data for bridge rating implementation and evaluation has been performed. The first ever state-wide calibration of live load factors for LRFR bridge evaluation and rating, following the LRFR Manual commentary Article C6.4.4.2.3 for development of site-specific live load factors, has been completed. In addition, a study was conducted to determine an amount of WIM data needed to extrapolate future loading events for both high and low ADTT volume sites. In a separate study, load effects for the Oregon Department of Transportation's (ODOT) suite of 13 bridge rating vehicles were calculated for various span lengths and types. These load effects, both factored and unfactored, were compared to the load effects calculated from vehicles in the WIM data. Based on observations of the data and prior research, the following observations and recommendations are presented:

- Using the statistical data from the four WIM sites with different ADTT volume, at different times of the year, and over different WIM data collection windows, live load factors were computed. The Oregon-specific live load factors were smaller than those in the LRFR Specification. The factors were smaller for the lower volume sites and smaller for the heavier permit trucks.
- The high volume site, I-5 Woodburn NB, showed little seasonal variation, was insensitive to direction of travel, and two-weeks of data were sufficient

to produce consistent factors. For the lower volume sites, some seasonal variation was observed with higher load factors during summer and fall due to agricultural and construction transport.

- By employing the procedures used to develop the LRFR Specification, the resulting live load factors maintain the nationally accepted structural reliability index for evaluation, even though the resulting state-specific live load factors were smaller than the national standard.
- Policy implementation for the Oregon-specific factors included rounding the computed values to the nearest 0.05, set a lower limit of 1.0 for the live load factors, and established provisions for maintenance of the factors into the future.
- For a high ADTT volume site (approximately 3500 ADTT), approximately two weeks of WIM data is needed to adequately extrapolate future upper tail events. For a low ADTT volume site (approximately 500 ADTT), one month of WIM data is needed.
- Additional WIM data should be collected and analyzed. One year of data from two sites was used to project loading events to a five year extrapolation window. As additional data become available, two and five years of collected data should be analyzed and results compared to the rating vehicles, and also to the one-year extrapolation values.

- The factored rating vehicles provided reasonably sufficient demands to envelope the load effects of the WIM data, including that attributed to an adjacent equivalent 3S2 alongside vehicle.
- The contribution of the alongside vehicle in 3S2 equivalents for each of the rating vehicles was presented as a percent of the nominal value to examine the consistency of the reliability between varying span lengths and load effects. Most of the factored rating vehicles produced a fairly uniform level of reliability.
- The Oregon-specific live load factors applied to the rating vehicles adequately enveloped the load effects produced by the WIM data. Some of the rating vehicles that are in current use do not quite produce the same level of demand compared to some WIM vehicles observed on Oregon's state-owned highways. However, the ratios of the rating vehicle load effect to the WIM vehicle load effect that were below 1.0 were reasonably close to 1.0. Considering the level of uncertainty in WIM axle weight measurements, as well as the calibration process, this difference was minor.
- The Type 3 Legal vehicle could be eliminated from the suite of rating vehicles. Additional research should be conducted to further support this recommendation, as stated in subsequent bullets.
- No immediate changes, such as increases in axle weights or reduction of axle spacing lengths are necessary for the suite of ODOT rating vehicles.

- The use of the NRL vehicles to represent Table 1 vehicle classification in Oregon is redundant, and need not be incorporated into to the suite of rating vehicles. Further, the NRLs provided nonuniform levels of reliability compared with the current Table 1 representative vehicles.
- Only one WIM site, I-5 Booth Ranch NB, was considered for comparison of load effects. Additional analyses should be conducted which compare load effects for other routes in Oregon, with varying ADTT, directionality, and freight corridors taken into account.
- Additional span types and lengths should be analyzed when comparing load effects. This may include three-span, four-span, and five-span continuous models with varying span lengths.
- Load effects at the girder level should be calculated and compared for both the WIM data and the rating vehicles using girder distribution factors.
- The factored HL-93 loading (at the operating level) was found to adequately envelope most Table X loading scenarios.

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