

# **BOLT-BEARING STRENGTH OF WOOD AND MODIFIED WOOD**

**BOLT-BEARING STRENGTH OF LABORATORY-MADE  
CROSS-BANDED YELLOW BIRCH COMPREG  
UNDER AIRCRAFT BOLTS**

**March 1945**



**This Report is One of a Series  
Issued In Cooperation with the  
ARMY-NAVY-CIVIL COMMITTEE  
on  
AIRCRAFT DESIGN CRITERIA  
Under the Supervision of the  
AERONAUTICAL BOARD**

**No. 1523-A**

**UNITED STATES DEPARTMENT OF AGRICULTURE  
FOREST SERVICE  
FOREST PRODUCTS LABORATORY  
Madison, Wisconsin**

**In Cooperation with the University of Wisconsin**



BOLT-BEARING STRENGTH OF WOOD AND MODIFIED WOOD

Bolt-bearing Strength of Laboratory-made Cross-banded

Yellow Birch Compreg Under Aircraft Bolts<sup>1, 2, 3</sup>

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Summary

This report presents the results of 354 bearing tests of single-bolt specimens of Laboratory-made, cross-banded yellow birch compreg under steel aircraft bolts. The tests were made to obtain information concerning (1) the bolt-bearing strength of the material for different combinations of thickness, grain direction, and bolt diameter, and (2) the minimum edge clearances and end margins required to develop the full bolt-bearing strength of the material. Edge clearance is defined here as the dimension perpendicular to the direction of loading from the center of the bolt hole to the edge of the member. End margin is defined as the dimension from the center of the bolt hole to the free end of the member. Three thicknesses (1/4, 1/2, and 1 inch) of compreg and three bolt diameters (1/4, 1/2, and 1 inch) were investigated under compressive, modified compressive, and tensile loading. Loads were applied at angles of 0°, 45°, and 90° to the face grain of the compreg.

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<sup>1</sup>This report is one of a series of progress reports prepared by the Forest Products Laboratory to further the Nation's war effort. Results here reported are preliminary and may be revised as additional data become available.

<sup>2</sup>This is one of a series of reports dealing with bolt-bearing strength of wood and modified wood. The first report was Report No. 1523. Other reports will be issued as data become available.

<sup>3</sup>For information on other properties of Laboratory-made cross-banded yellow birch compreg, see table 2-14 of ANC Bulletin No. 18.

The average ultimate bearing stresses, under 1/4-, 1/2-, and 1-inch bolts, as indicated by these tests, were approximately 34,000, 30,000, and 28,000 pounds per square inch, respectively. With one exception, the minimum values were at least 83 percent of these averages. The average bearing stresses at proportional limit were about 50 percent of the average ultimate stresses under tensile loading and about 60 percent under compressive loading.

With adequate edge clearance and end margin, the grain direction of the face plies had no apparent effect on either the proportional limit or the ultimate bearing stress of the compreg. The bearing stress decreased as the diameter of the bolt increased. The thickness of the compreg showed no consistent effect on the bearing stress.

Under compressive loading, an edge clearance equal to twice the diameter of the bolt was adequate to develop the full bearing capacity of the compreg regardless of the angle between the direction of loading and the grain of the face plies. Under tensile loading, however, the necessary edge clearance was larger than for compressive loading, and was influenced by the grain direction. The necessary edge clearances were 3, 2-1/2, and 4 diameters for 0°, 45°, and 90°, respectively. The necessary end margins for 1/4-inch bolts under tensile loading were 5, 3, and 4 diameters for 0°, 45°, and 90°, respectively, and slightly less for bolts of larger diameter. The necessary end margins as determined by modified compressive loading were essentially in agreement with those determined by tensile loading.

The bearing strength of compreg was sufficient to develop the double shear strength of a steel aircraft bolt at a bearing length equal to at least four times the diameter of the bolt.

No definite relationship between bolt-bearing strength and compressive strength is indicated by the results of these tests.

The most consistent critical edge clearances and end margins and the most consistent bearing stress values were obtained for compreg having face grain at 45° to the direction of loading.

### Introduction

The bolt-bearing strength of compreg is greater than that of either wood or plywood. Although a comparatively new material, compreg has found application as a reinforcing material in bolted connections in wood aircraft. Designers have not been able to take full advantage of its greater bolt-bearing strength, however, because of lack of appropriate information. This investigation of the bearing strength of Laboratory-made compreg under steel aircraft bolts was undertaken at the request of the ANC Subcommittee on Wood Design Criteria to provide preliminary design data. Specific information was desired concerning (1) the bolt-bearing strength of cross-banded yellow birch compreg for different combinations of thickness, grain

direction, and bolt diameter, and (2) the minimum edge clearances necessary under both tensile and compressive loading and the minimum end margins necessary under tensile loading to develop the full bolt-bearing strength of the material.

Edge clearance is defined as the dimension perpendicular to the direction of loading from the center of the bolt hole to the edge of the member. End margin is defined as the dimension from the center of the bolt hole to the free end of the member. Referring to either edge clearance or end margin, the critical dimension is that which is barely adequate to prevent failure under load in a manner other than in bearing.

The investigation covered by this report was preparatory to a more comprehensive study of the bolt-bearing properties of commercial cross-banded compregs. It afforded an opportunity to develop testing techniques and served to define the scope of the subsequent work. The conclusions presented herein should be regarded as tentative pending confirmation by the tests of commercial compreg.

### Description of Material

The compreg was made at the Forest Products Laboratory from 1/16-inch yellow birch veneers impregnated with water-soluble phenol-formaldehyde resin and bonded together without the use of additional adhesives. The number of plies varied with the finished thickness of compreg, the 1/4-, 1/2-, and 1-inch material containing 7, 13, and 25 plies, respectively. The veneers were cross banded, placed between metal cauls, and pressed at 1,300 to 1,800 pounds per square inch for periods of 25 to 60 minutes, the length of time depending on the thickness of the panel. The temperature of the panels during pressing was 310° F. All panels were removed from the press while still hot. Additional detailed information including specific gravity for each panel is presented in table 1.

Steel aircraft bolts conforming to Army-Navy Specification AN-B-3a were used. This specification contemplates the use of bolt stock with a tensile yield strength of approximately 96,000 pounds per square inch and an ultimate strength of approximately 125,000 pounds per square inch. It provides the following tolerances in bolt diameters: 1/4 inch, 0.249 (+0.0000:-0.0030) inch; 1/2 inch, 0.499 (+0.0000:-0.0035) inch; and 1 inch, 0.999 (+0.0000:-0.0055) inch.

### Preparation of Specimens

The variables studied in this investigation included diameter of bolt, thickness of compreg, edge clearance, end margin, grain direction of face ply, and type of loading. For the bolt-bearing tests each initial series consisted of from 2 to 5 specimens, the majority containing 3, in

which either edge clearance or end margin was varied over a range intended to bracket the critical dimension. Supplemental specimens were prepared, where required, to extend the range of dimensions or to reduce the increments and thus facilitate more precise determination of the critical dimension. Specimens of each initial series were cut adjacent to each other; supplemental specimens were cut from locations in the same panel as near as possible to the initial series. Specimens for minor compression and specific gravity tests were taken at random from each panel and were presumed to represent the entire panel.

Each bolt-bearing specimen was marked to indicate the panel number, manner of loading, and the numerical order of specimens taken from that panel for that manner of loading. For example, the marking 1-BT-3 designated the third bolt-bearing specimen selected from panel No. 1 for tensile loading. A layout sheet was made for each panel to show the locations of all specimens.

Specimens were cut by means of hollow-ground high-carbon steel saws, such as are used for cutting soft metals. Saws varied from 8 to 14 inches in diameter and were rotated at speeds varying from 870 to 2,670 revolutions per minute, with the larger saws and slower speeds used for the thicker material.

All holes were drilled  $1/64$  inch undersize with a twist drill, sharpened as for metal, and then reamed to the finished dimension. Specimens with a  $1/4$ - or  $1/2$ -inch hole were drilled and reamed on a hand-feed drill press using a speed of about 500 revolutions per minute for drilling and about 300 revolutions per minute for reaming. A rate of feed less than 1 inch per minute was used for drilling and about 10 inches per minute for reaming. Specimens with a 1-inch hole were drilled and reamed on a vertical boring mill using a speed of about 100 revolutions per minute for drilling and about 50 revolutions per minute for reaming. The hand-feed attachment of the boring mill was used to secure a rate of feed similar to that employed for the  $1/4$ - and  $1/2$ -inch holes. The lower reaming speed was used for all holes to lessen the possibility of overheating and damaging the reamer. As a safety precaution, all specimens were clamped tightly to the bed of the machine before drilling and reaming. The edges of the reamed holes were sharp and clean-cut, and the walls had a pronounced shine. The fit of the bolt in the hole was such that insertion required only hand pressure.

All specimens were conditioned at  $70^{\circ}$  F. and 64 percent relative humidity for at least 48 hours before testing. This conditioning was intended to relieve internal stresses that might have been caused by sawing, drilling, or reaming, but was not of sufficient duration to bring the specimens to moisture equilibrium. Attainment of moisture equilibrium would have necessitated a much longer conditioning period and would have delayed the testing schedule excessively.

## Test Procedure

### General Discussion of Bolt-bearing Tests

The methods and apparatus employed in all bolt-bearing tests were designed to determine the true bearing properties of compreg, uninfluenced by friction or other mechanical restraint between the compreg and the fittings. It was recognized that these factors are present in varying degrees in all bolted fastenings and that they tend to augment the actual bearing strength of the material. Because the degree of restraint thus imposed, however, cannot be readily evaluated, accurate appraisal of the results of bolt-bearing tests cannot be made unless their influence is eliminated.

In order to minimize errors due to external restraint, all fittings used in this investigation were equipped with interchangeable, hardened steel, recessed bushings of the type shown in figure 1. This type of bushing was developed through exploratory bolt-bearing tests. In experiments employing unrecessed bushings, accurate determination of the ultimate bearing load was virtually impossible because of the added restraint imposed by compacting the extruded fibers of compreg between the specimen and the bushing.

In the improved bushing, the projecting lug supports the bolt at the face of the specimen and, together with the slotted recess above the bolt, provides clearance of 1/4 inch along the loaded surface of the bolt. This clearance permits free lateral movement of the crushed fibers of compreg and prevents their being compacted against the bushing. The width of the recess is equal to the diameter of the bolt hole; that of the lug is made slightly less to avoid sharp, easily broken edges and corners. Proper orientation is insured by either a set screw or a key inserted in the fitting so as to engage the flattened side of the bushing.

As an added precaution, a clearance of at least one-sixteenth inch was left between the specimen and the vertical fittings and between the head of the test bolt and the bushing. The nut was omitted from the bolt.

The rate of loading was such as to produce a deformation of about 0.01 inch per minute. In general, this rate made possible at least 10 readings of load and deformation below the proportional limit. Bolts were inspected before use and after each test, and bent bolts were discarded.

Bolt-bearing strengths and critical dimensions were determined by tests employing three different methods of loading: compressive, modified compressive, and tensile. Supplementary tests were made to determine compressive strength.

With few exceptions, all combinations of diameter of bolt and thickness of compreg were tested with the load applied at angles of 0°, 45°, and 90° to the grain of the face plies.

## Bolt-bearing Tests Under Compressive Loading

The specimen was supported on an aircraft bolt resting in two vertical mild steel fittings as shown in figure 2. The fittings were separated at the base by steel spacer blocks of various lengths to accommodate specimens of different thicknesses. Figure 3 shows the inner face of half of the fitting with the test bolt, bushing, and spacer blocks in place. Load was applied to the entire top surface of the specimen by means of a flat plate attached rigidly to the upper platen of the testing machine. No spherical bearing block was used because of difficulties encountered in obtaining correct seating of the head in tests of wide specimens of 1/4-inch compreg. To achieve uniformity in all tests with compressive and modified compressive loading, the rigid plate was used throughout.

The deformation of the specimen at the bolt hole was measured by means of a 1/10,000-inch dial mounted on the extension arm shown in figure 2. This equal-lever extension arm was calibrated and found to be sufficiently rigid to operate satisfactorily the comparatively stiff spring in such a dial.

Only bearing strength and edge clearance were determined by this method. The specimens used for 1/4- and 1/2-inch bolts were 4 inches high with the center of the hole 2 inches from the top or loaded end; those used for 1-inch bolts were 6 inches high with center of hole 4 inches from the top. Five specimens, each of a different width, usually constituted a series.

This series was repeated for three grain directions and for eight combinations of thickness and bolt diameter, consisting of 1/4- and 1/2-inch bolts in 1/4-, 1/2-, and 1-inch compreg and 1-inch bolts in 1/2- and 1-inch compreg.

## Bolt-bearing Tests Under Modified Compressive Loading

The tests with modified compressive loading differed from those with compressive loading only in the area of contact between the specimen and the loading head. In modified compressive loading, the load was applied symmetrically to the top of the specimen for a distance of only 1-1/2 inches from each edge by means of steel blocks (fig. 4). The desirable gap between the loading blocks was determined by exploratory tests. This method was intended to produce stresses in the specimen and a type of failure resembling those produced by tensile loading. The unloaded middle portion of the specimen above the bolt was free to fail by shearing or bending as long as the end margin was insufficient to develop the full bearing strength. This method permitted approximate determination of critical end margins for tensile loading more rapidly and for wider ranges of diameter of bolt and thickness of compreg than was possible by the method employing tensile loading.

Tests made by this method included 1/4-inch bolts in 1/4- and 1/2-inch compreg, 1/2-inch bolts in 1/4-, 1/2-, and 1-inch compreg, and 1-inch bolts in 1/2- and 1-inch compreg. Most of these combinations were tested with the face grain at angles of 0°, 45°, and 90° with the direction of loading. Specimens for both 1/4- and 1/2-inch bolts were 5 inches wide; those for 1-inch bolts were 6 inches wide. Sufficient margin was maintained below the bolt in all tests so that the specimen as a whole could not fail as a beam. In general, five specimens, each with a different end margin, constituted a series for a given bolt and thickness of compreg.

#### Bolt-bearing Tests Under Tensile Loading

In the tests to determine the bolt-bearing strength under tensile loading the specimen was suspended on an aircraft bolt between two fittings of a specially designed tension jig, and a self-aligning tension grip was attached to the lower end (figs. 5 and 6). Both the jig and the tension grip were equipped with spherical bearing blocks to facilitate alignment of the entire assembly. For specimens not more than 2 inches wide, the jaws of the grip were at least as wide as the specimen; for all specimens wider than 2 inches, the 2-inch jaws were used. The deformation of the specimen at the bolt hole was measured by two symmetrically placed, 1/10,000-inch dial gages attached to the fittings and actuated by a cross-bar in knife-edge contact with the top of the specimen.

Both edge clearance and end margin were determined by tensile loading for 1/4- and 1/2-inch bolts in 1/4- and 1/2-inch compreg. Tensile loading was limited to this range because the loads obtained for 1/2-inch bolts in 1/2-inch compreg approached the capacity of the apparatus. Although the gripped surfaces were sanded, considerable difficulty was experienced in preventing slippage of the more heavily loaded specimens.

In general, five specimens of varying width constituted an edge-clearance series, and five specimens of varying end margins constituted an end-margin series. All specimens were 7 inches long. In the edge-clearance specimens, the centerline of the hole was 2-1/2 inches from the top of the specimen. After the critical edge clearance was determined for a given combination of diameter and thickness, this dimension was increased by at least the diameter of the bolt, and the increased dimension was used as the constant edge clearance for the end-margin specimens for that particular combination.

#### Tests for Compressive Strength

In the tests to determine the compressive strength of 1/4-inch compreg, 1/4- by 1- by 4-inch specimens were supported laterally in the apparatus shown in figure 7. Loads were applied parallel to the grain of the face plies, and deformations measured over a 2-inch gage length by means of a Martens' mirror compressometer. The rate of loading corresponded to a no-load speed of 0.012 inch per minute. The proportional limit



and modulus of elasticity were determined by this method. After the proportional limit was attained, the test was suspended to avoid injury to the apparatus, and each specimen was cut to produce three 1/4- by 1- by 1-inch specimens, which were reconditioned for at least 48 hours and loaded to failure in compression without lateral support. Exploratory tests had demonstrated that ultimate strengths determined by this method were comparable with those obtained in tests of laterally supported 1/4- by 1- by 4-inch specimens.

For 1/2-inch compreg, 1/2- by 1/2- by 2-inch specimens, with grain of face plies parallel to the 2-inch dimension, were loaded axially and deformations measured over a 1-inch gage length by means of a Martens' mirror compressometer as shown in figure 8. The rate of loading corresponded to a no-load speed of 0.006 inch per minute. After the proportional limit was attained, the compressometer was removed and the test continued to failure.

For 1-inch compreg, 1- by 1- by 4-inch specimens were tested in a similar manner except that deformations were measured over a 2-inch gage length by means of a roller-type compressometer with averaging optical levers (fig. 9). The rate of loading corresponded to a no-load speed of 0.012 inch per minute.

#### Explanation of Tables

General information including dimensions, number of plies, manufacturing data, and specific gravity of each of the compreg panels from which specimens were prepared is presented in table 1.

A summary of the information obtained from both the bolt-bearing and compression tests is given in tables 2 through 5. In tables 2, 3, and 4, the data for bolt-bearing tests under compressive and modified compressive loadings are combined for angles of 0°, 45°, and 90° to the grain of the face plies, respectively. Table 5 consists of data for bolt-bearing tests under tensile loading for all three grain directions. In column 3 of each table are listed the numbers of the panels from which the specimens were cut. This information is included (1) to relate bolt-bearing specimens cut from the same panel of compreg, (2) to explain the duplication of compressive strengths associated with various bolt-bearing specimens cut from the same panel, and (3) to correlate information given in these tables with that given in table 1. Column 4 shows the total number of bolt-bearing specimens tested for each combination of bolt diameter and panel thickness.

Columns 5 and 6 show the number of specimens averaged to provide the data presented in columns 7 through 12. The number of specimens as listed in column 5 is usually smaller than in column 4 because the dimensions of some of the specimens were inadequate to develop the full proportional limit. The number in column 6 is always smaller than in column 5 because the minimum dimensions adequate for proportional limit were inadequate to develop the full bearing load. In table 2, for example, six 1/4-inch

specimens were tested for edge clearance with 1/2-inch bolts under compressive loading, and four for end margin under modified compressive loading. Two of the total of 10 failed to develop the full proportional limit. Of the remaining eight, two failed before reaching the full bearing load.

Columns 8 through 11, tables 2, 3, and 4, contain values of bearing strength and deformation obtained under both compressive and modified compressive loading, since there was no marked difference between the values obtained by these two methods. Maximum, average, and minimum values are included. The relationship between proportional limit and ultimate strength in bearing is shown by the ratios listed in column 12.

Similar data with respect to compressive strength are presented in columns 13 through 18. No values were obtained for deformation at the ultimate compressive stress because of precautions taken to protect the measuring apparatus. Values of the modulus of elasticity in compression are shown in column 19.

The relationship between the bearing and compressive stresses at proportional limit and ultimate is shown by the ratios in columns 20 and 21. Critical edge clearances and end margins are shown in columns 22 and 23. In tables 2, 3, and 4, the edge clearances shown were determined only by compressive loading; the end margins for tension, only by modified compressive loading. Where both critical dimensions are shown, the values of bearing stress and deformation obtained by both methods of testing have been combined. In table 5, however, both dimensions recorded were determined by tensile loading.

In table 6, the critical dimensions are summarized according to diameter of bolt, thickness of compreg, method of loading, and grain direction. Average values of bearing stresses at proportional limit and ultimate are summarized in a similar manner in table 7, and average deformations at both proportional limit and ultimate in table 8.

## Analysis of Results

### Bolt-bearing Strength

Proportional limit loads were determined from load-deformation curves (fig. 10), which were obtained for all bolt-bearing tests. The rate of loading was such that at least 10 readings were obtained within the straight-line portion of the load-deformation curve. The departure of the curve from a straight line was usually so gradual that the exact limit of proportionality was not clearly defined. Wherever such uncertainty existed, the lowest probable value was selected; hence, the values presented in column 8, tables 2 through 5, are conservative.

Most specimens exhibited definite ultimate loads as illustrated by the load-deformation curve in figure 10. The curves for a limited number of tests became horizontal, but did not begin to decline immediately. Occasionally, after the curve had become horizontal or had even begun to decline, the load again increased. This behavior was attributed to lack of complete freedom of the specimen to deform, and loads thus attained were disregarded in recording the ultimate load. In those tests in which the ultimate load was not clearly defined, it was assumed to be the first load at which the curve remained horizontal or declined during an increment of deformation of at least 0.004 inch. The ultimate loads and the corresponding deformations determined in this manner agreed closely with clearly defined ultimate loads obtained for similar specimens.

In this investigation the determination of critical dimensions (edge clearance and end margin) was based on ultimate loads. Specimens in which the dimension under study was inadequate failed in some manner other than bearing, and usually at loads less than the true ultimate bearing load. The critical dimension was considered to have been attained when any further increase failed to produce an increase in the ultimate load. In general, all specimens in which the dimension was equal to or greater than the critical value, thus determined, failed in bearing.

Typical failures of specimens for each of the four kinds of bolt-bearing tests made in this investigation are shown in figures 11 through 14. The magnitude of the dimension under study and the loads at proportional limit and ultimate are shown for each specimen. The proportional limit loads in each of these four series were approximately the same for all specimens; hence, all were averaged in determining the bearing stress at proportional limit. The ultimate load, however, increased progressively with increases in the dimension until bearing failure was attained, after which they were practically uniform. Only these uniform values were averaged in determining the ultimate bearing stress.

The bearing area of each specimen was obtained by multiplying the measured thickness of the compreg by the measured diameter of the bolt. The unit bearing stress at the proportional limit or ultimate was found by dividing the appropriate load by the bearing area. This method is based on the assumption that the load is uniformly distributed over the projected area of the bolt.

In general, the proportional limit values obtained under tensile loading were from 10 to 35 percent lower than those obtained under compressive loading. The values of ultimate bearing stress, however, were about the same as those obtained under compressive loading. The average ultimate bearing stress of cross-banded compreg under 1/4-inch aircraft bolts was approximately 34,000 pounds per square inch; under 1/2-inch bolts, 30,000 pounds per square inch; and under 1-inch bolts, 28,000 pounds per square inch. The average proportional limit stresses under compressive loading were approximately 22,500, 18,000, and 15,000 pounds per square inch, respectively. The average proportional limit stresses under tensile loading were approximately 17,500 and 14,500 pounds per square inch for 1/4- and 1/2-inch bolts, respectively.

The average values for bearing stress at both proportional limit and ultimate, shown in tables 2 through 5, appeared to be unaffected by the grain direction of the face plies, although there was slightly less variation among individual values for angles of 45°, than for angles of 0° and 90°. There was a definite reduction in both proportional limit and ultimate bearing stress with an increase in the diameter of bolt. For any one diameter of bolt there was no consistent relationship between these stresses and the thickness of compreg. The ratio of the average proportional limit to average ultimate varied from 0.39 to 0.78 (average 0.60) for compressive and modified compressive loading, and from 0.40 to 0.60 (average 0.50) for tensile loading.

The results obtained in this investigation do not indicate any definite relationships between bolt-bearing strengths and average compressive strengths. Columns 20 and 21 in tables 2 through 5 show the computed ratios of bolt-bearing stress to compressive stress at both the proportional limit and ultimate. Both ratios vary over a wide range and exhibit no consistent trends. It is possible that more definite relationships would have been apparent had the materials studied provided a wider range in bearing and compressive strength.

#### Edge Clearance and End Margin

The critical dimensions determined by these tests under both tensile and compressive loading, recorded in table 6, were influenced but little by the thickness of the compreg within the range from 1/4 to 1 inch, but varied directly with the diameter of the bolt. It is convenient, therefore, to express these dimensions in terms of diameter, "D".

An edge clearance of 2D was adequate under compressive loading regardless of the angle between the direction of loading and the grain of the face plies. An edge clearance of 3D was adequate under tensile loading parallel to the grain of the face plies, 2-1/2D at an angle of 45°, and 4D at an angle of 90°.

Since exploratory tests had indicated that the end margin did not affect the proportional limit and ultimate bearing loads under compressive loading, this dimension was not determined.

An end margin of 4D was adequate under modified compressive loading parallel to the grain of the face plies and 3D at angles of 45° and 90°. An end margin of 5D was adequate under tensile loading parallel to the grain of the face plies, 3D at an angle of 45° and 4D at an angle of 90°. Where both methods of loading were employed, end margins determined under modified compressive loading, although slightly smaller, agreed reasonably well with those obtained under tensile loading. This agreement justified the use of modified compressive loading to determine critical end margins for tensile loading in the heavier combinations of diameter and thickness, for which tests under tensile loading were impracticable.

Wherever a difference in critical dimensions occurred between specimens of different grain orientation, the material having the grain of the face plies at an angle of  $45^\circ$  to the direction of the loading yielded the most consistent values.

#### Limit of L/D Ratio for Steel Aircraft Bolt in Cross-banded Compreg

It was planned originally to test 1-inch compreg under 1/4-inch steel aircraft bolts under compressive loading for all three grain directions. It was found, however, that the ultimate bearing loads for such combinations exceeded the double-shear strength of the bolts. One of the sheared bolts is shown in figure 15.

A supplementary edge-clearance series of four 1-inch compreg specimens was tested with 3/8-inch bolts under compressive loading parallel to the grain of the face plies to determine more closely the limiting ratio of bearing length to diameter. The average ultimate bearing load obtained was only 75 percent of the calculated ultimate shearing strength of a 3/8-inch aircraft bolt and there was no indication of shearing failures in the 3/8-inch aircraft bolts. It was concluded, therefore, that the strength of a bolted connection in compreg in which the ratio of bearing length to bolt diameter (L/D) is 4 or more is limited by the shearing strength of the bolt rather than by the bearing strength of the compreg.

#### Deformations

The average, maximum, and minimum deformations for each combination of type of loading, diameter of bolt, and thickness of compreg are shown in columns 9 and 11 of tables 2 through 5. A summary of all average deformations is presented in table 8. In general, the deformations at both the proportional limit and ultimate increased with an increase in the thickness of compreg, but did not change consistently with an increase in the diameter of the bolt. For most combinations of diameter and thickness, the deformations were greater for compressive loading. On the basis of these tests, it could be expected that the deformation would not exceed 0.015 inch under normal working loads and 0.075 inch at the ultimate.

#### Conclusions

The ultimate bolt-bearing stress of cross-banded compreg as determined by these tests was approximately 34,000 pounds per square inch under 1/4-inch steel aircraft bolts, 30,000 pounds per square inch under 1/2-inch bolts, and 28,000 pounds per square inch under 1-inch bolts. These are average values and apply to either compressive or tensile loading. With

one exception, the minimum values were at least 83 percent of these averages. The average proportional limit stresses were about 50 and 60 percent of the average ultimate stresses for tensile and compressive loading, respectively.

The grain direction of the face plies has no apparent effect on either the proportional limit or the ultimate bearing stress of the compreg. The bearing stress decreased as the diameter of the bolt increased. The thickness of the compreg showed no consistent effect on the bearing stress.

An edge clearance equal to twice the diameter ( $2D$ ) of the bolt was adequate to develop the bearing capacity of the compreg under compressive loading for  $L/D$  (ratio of bearing length to diameter) values from  $1/2$  to  $2$ , regardless of the orientation of the grain of the compreg.

An edge clearance of  $3D$  was ample to develop the full bearing capacity of the compreg under tensile loading parallel to the grain of the face plies,  $2-1/2D$  at an angle of  $45^\circ$ , and  $4D$  at an angle of  $90^\circ$ . An end margin of  $5D$  was ample under tensile loading parallel to the grain,  $3D$  at an angle of  $45^\circ$ , and  $4D$  at an angle of  $90^\circ$ .

At an  $L/D$  ratio of about  $4$ , compreg developed the double shear strength of the steel aircraft bolt.

No definite relationship between bolt-bearing strength and compressive strength was indicated by these tests.

Bolt-bearing deformations not exceeding  $0.015$  inch under normal working loads and  $0.075$  inch under ultimate loads can be expected on the basis of this study.

The most consistent critical dimensions and the most consistent bearing stress values were obtained in the tests of compreg having face grain at an angle of  $45^\circ$  to the direction of loading.

Table 1.--Physical data pertaining to laboratory-made, cross-banded yellow birch  
compreg used for bolt-bearing tests.

Panel number:	Dimensions	Number of	Date pressed	Pressure	Time of pressing	Resin content	Specific gravity
:	Width: Length: Thickness	plies	:	psi.	minutes	percent	:
:	Nominal: Actual : average	:	:	:	:	:	:
(1)	(2) : (3) : (4) : (5)	(6)	(7)	(8)	(9)	(10)	(11)
	Inches: Inches: Inch			Psi.	Minutes	Percent	
1	: 29.5 : 29.5 : 1/4 : 0.27	: 7	: 1/22/43	: 1,300	: 25	: 29.3	: 1.308
2	: 29.5 : 29.5 : 1/2 : .49	: 13	: 1/22/43	: 1,300	: 30	: 29.3	: 1.295
3	: 29.5 : 29.5 : 1 : .95	: 25	: 1/22/43	: 1,300	: 50	: 29.3	: 1.272
4	: 24 : 24 : 1/4 : .25	: 7	: 5/7/43	: 1,800	: 25	: 26	: 1.369
5	: 24 : 24 : 1/4 : .26	: 7	: 5/8/43	: 1,800	: 25	: 26	: 1.362
6	: 24 : 24 : 1/4 : .26	: 7	: 5/10/43	: 1,800	: 25	: 26	: 1.375
7	: 24 : 24 : 1/2 : .47	: 13	: 5/7/43	: 1,800	: 30	: 26	: 1.368
8	: 24 : 24 : 1/2 : .47	: 13	: 5/8/43	: 1,800	: 30	: 26	: 1.373
9	: 24 : 24 : 1/2 : .48	: 13	: 5/10/43	: 1,800	: 30	: 26	: 1.376
10	: 24 : 24 : 1 : .92	: 25	: 5/7/43	: 1,800	: 60	: 26	: 1.375
11	: 24 : 24 : 1 : .89	: 25	: 5/8/43	: 1,800	: 60	: 26	: 1.359
12	: 24 : 24 : 1 : .88	: 25	: 5/10/43	: 1,800	: 60	: 26	: 1.368
17	: 24 : 24 : 1/4 : .26	: 7	: 5/19/43	: 1,800	: 25	: 26	: 1.367
18	: 24 : 24 : 1/2 : .46	: 13	: 5/19/43	: 1,800	: 30	: 26	: 1.379
19	: 24 : 24 : 1/2 : .46	: 13	: 5/19/43	: 1,800	: 30	: 26	: 1.362

<sup>1</sup>Panels 13 through 16 were used for other purposes.

<sup>2</sup>Based on weight of veneer.

<sup>3</sup>Based on weight and volume at time of test.

Table 2.--Bolt-bearing and compressive strength and related properties for laboratory-made, cross-banded, yellow birch compress. Bolt-bearing tests were made under compressive and modified compressive loading. All tests were made with the load parallel to the grain of the face plies.

Nominal bolt diameter	Panel number	Bolt bearing	Bolt bearing										Compression		Ratio of bolt-bearing stress to average compressive stress		Critical diameter								
			(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)		(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)
			Total tested	Tested for proportional limit	Ultimate	Proportional limit	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation	Stress deformation
1/4	1	1/4	11	9	7	Minimum Average Maximum	19,170 22,600 26,080	0.00405 0.00898 0.00890	22,040 34,810 38,190	0.0290 0.0571 0.0300	0.6575	20	Minimum Average Maximum	6,700 9,850 11,460	0.00285 0.00415 0.00848	20,610 24,190 29,490	0.407	1.95 2.21 2.85	1.57 1.44 1.96	4					
1/4	2	1/2	11	9	6	Minimum Average Maximum	16,960 20,690 24,300	0.00940 0.00794 0.1125	30,620 34,240 36,710	0.0300 0.0350 0.0400	0.604	25	Minimum Average Maximum	5,960 10,060 11,340	0.00373 0.00412 0.00463	18,660 20,440 22,480	0.492	2.00 2.04 2.57	1.69 1.67 1.60	4					
1/4	3	1	4	4	1/2	Minimum Average Maximum	12,770 13,420 15,720	0.00725 0.00913 0.1095	32,040 34,070 36,020	0.0180 0.0180 0.0180	0.394	10	Minimum Average Maximum	7,130 8,040 8,840	0.00258 0.00310 0.00350	19,120 20,480 22,840	0.392	2.50 2.59 2.76	1.57 1.66 1.76	2					
3/8	3	1	4	3	2	Minimum Average Maximum	15,660 16,570 17,140	0.00865 0.1083 0.1295	29,920 31,160 36,410	0.0500 0.0586 0.0675	0.500	10	Minimum Average Maximum	7,130 9,850 8,840	0.00258 0.00415 0.00350	19,120 20,480 22,840	0.392	2.50 2.59 2.76	1.57 1.66 1.76	2					
1/2	1	1/4	10	8	6	Minimum Average Maximum	13,910 16,130 19,860	0.00291 0.00561 0.00820	22,880 26,910 29,350	0.0190 0.0250 0.0350	0.599	20	Minimum Average Maximum	6,700 9,850 11,460	0.00285 0.00415 0.00848	20,610 24,190 29,490	0.407	1.95 2.21 2.85	1.57 1.44 1.96	3					
1/2	2	1/2	25	22	14	Minimum Average Maximum	14,460 16,960 19,410	0.00520 0.00814 0.00715	25,710 27,670 29,260	0.0290 0.0328 0.0400	0.614	25	Minimum Average Maximum	8,960 10,060 11,340	0.00373 0.00412 0.00463	18,660 20,440 22,480	0.492	2.30 2.44 2.57	1.44 1.69 1.93	4					
1/2	3	1	12	12	7	Minimum Average Maximum	13,690 15,740 18,140	0.00690 0.01015 0.1570	26,410 29,180 31,780	0.0390 0.0471 0.0590	0.580	10	Minimum Average Maximum	7,130 8,040 8,840	0.00258 0.00310 0.00350	19,120 20,480 22,840	0.392	2.50 2.59 2.76	1.57 1.66 1.76	2					
1	2	1/2	12	9	2	Minimum Average Maximum	12,150 13,720 15,020	0.00490 0.00749 0.1070	27,080 27,300 27,530	0.0290 0.0400 0.0590	0.502	25	Minimum Average Maximum	6,960 10,060 11,340	0.00373 0.00412 0.00463	18,660 20,440 22,480	0.492	2.30 2.44 2.57	1.44 1.69 1.93	4					
1	3	1/2	4	4	3	Minimum Average Maximum	15,040 16,060 17,000	0.00600 0.00963 0.1100	26,900 28,190 28,920	0.0400 0.0437 0.0460	0.570	26	Minimum Average Maximum	6,700 9,850 11,460	0.00285 0.00415 0.00848	20,610 24,190 29,490	0.407	2.30 2.44 2.57	1.44 1.69 1.93	4					
1	5	1	22	17	11	Minimum Average Maximum	11,590 14,700 17,160	0.00990 0.00879 0.10530	25,710 27,160 28,990	0.0290 0.0490 0.0790	0.541	10	Minimum Average Maximum	7,130 8,040 8,840	0.00258 0.00310 0.00350	19,120 20,480 22,840	0.392	2.50 2.59 2.76	1.57 1.66 1.76	3					

<sup>1</sup>Column 8 divided by column 15.

<sup>2</sup>Column 10 divided by column 17.

<sup>3</sup>Load margin for tensile loading determined by modified compressive loading.

<sup>4</sup>Bolt-bearing load exceeded double shear strength of 1/4-inch bolt (Fig. 15).



Table 3.—Bolt-bearing and compressive strength and related properties for laboratory-made, cross-banded, yellow birch composites. Bolt-bearing tests were made under compressive and modified compressive loading, with the load at an angle of 45° to the grain of the face plies. Compression tests were made with the load parallel to the grain of the face plies.

Nominal bolt diameter, inches	Nominal panel thickness, inches	Panel number	Bolt bearing				Compression										Critical dimensions						
			Number of specimens		Ultimate	Variation	Proportional limit		Ratio of ultimate stress to ultimate stress	Ultimate stress	Ratio of ultimate stress to average compressive stress	Proportional limit	Ratio of ultimate stress to average compressive stress	Proportional limit	Ratio of ultimate stress to average compressive stress	Ultimate stress		Ratio of ultimate stress to average compressive stress	Proportional limit	Ratio of ultimate stress to average compressive stress			
			Total tested	Tested for proportional limit			Stress	Deformation													Stress	Deformation	Stress
1/4	1/4	5	10	9	7	29,280	0.0250	0.0276	0.696	29,280	0.0250	0.0276	0.696	29,280	0.0250	0.0276	0.696	29,280	0.0250	0.0276	0.696	2	3
1/4	1/2	8	10	10	7	31,370	0.0250	0.0250	0.653	31,370	0.0250	0.0250	0.653	31,370	0.0250	0.0250	0.653	31,370	0.0250	0.0250	0.653	2	3
1/2	1/4	5	10	10	7	26,850	0.0200	0.0200	0.652	26,850	0.0200	0.0200	0.652	26,850	0.0200	0.0200	0.652	26,850	0.0200	0.0200	0.652	2	3
1/2	1/2	8	10	8	7	27,980	0.0300	0.0352	0.650	27,980	0.0300	0.0352	0.650	27,980	0.0300	0.0352	0.650	27,980	0.0300	0.0352	0.650	2	3
1/2	1	10	5	5	4	33,600	0.0400	0.0438	0.498	33,600	0.0400	0.0438	0.498	33,600	0.0400	0.0438	0.498	33,600	0.0400	0.0438	0.498	—	3
1/2	1	11	5	5	3	34,020	0.0363	0.0400	0.549	34,020	0.0363	0.0400	0.549	34,020	0.0363	0.0400	0.549	34,020	0.0363	0.0400	0.549	2	—
1	1/2	9	11	11	5	26,200	0.0220	0.0220	0.542	26,200	0.0220	0.0220	0.542	26,200	0.0220	0.0220	0.542	26,200	0.0220	0.0220	0.542	2	3
1	1	10	5	5	4	29,670	0.0475	0.0500	0.582	29,670	0.0475	0.0500	0.582	29,670	0.0475	0.0500	0.582	29,670	0.0475	0.0500	0.582	—	2-1/2
1	1	11	6	6	1	26,400	0.0300	0.0300	0.527	26,400	0.0300	0.0300	0.527	26,400	0.0300	0.0300	0.527	26,400	0.0300	0.0300	0.527	2	—

<sup>1</sup>Column 8 divided by column 15.

<sup>2</sup>Column 10 divided by column 17.

<sup>3</sup>End margin for tensile loading determined by modified compressive loading.

Table 4. Bolt-bearing and compressive strength and related properties for laboratory-made, cross-banded, yellow birch composites. Bolt-bearing tests were made under compressive and modified compressive loading, with the load at an angle of 30° to the grain of the face plies. Compression tests were made with the load parallel to the grain of the face plies.

Nominal panel diameter: thickness	Panel number	Bolt bearing										Compression					Ratio of bolt-bearing stress to average compressive stress			Critical dimensions						
		(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)		(19)	(20)	(21)	(22)	(23)	
Inch	Inch	Number of specimens		Variation		Proportional limit		Ultimate		Ratio of number of plies or proportional limit to tensile ultimate		Stress: Deformation		Stress: Deformation		Stress: Deformation		Stress: Deformation		Stress: Deformation		Stress: Deformation		Bolt diameter	Bolt diameter	
		Tested	Not tested	Minimum	Average	Maximum	Minimum	Average	Maximum	Minimum	Average	Maximum	Minimum	Average	Maximum	Minimum	Average	Maximum	Minimum	Average	Maximum	Minimum	Average			Maximum
1/4	17	5	5	4	4	23,020	24,470	25,000	23,400	0.0200	0.780	14	Minimum	10,020	10,034	10,038	22,390	22,390	22,390	22,390	2,867	2,867	2,867	2.11	2.14	—
1/4	7	6	6	5	5	23,100	24,300	25,000	23,700	0.0204	0.701	26	Average	10,890	0.0370	25,730	25,730	25,730	25,730	2,946	2,946	2,946	2.24	1.22	1-1/2	
1/2	17	10	10	5	5	23,100	24,300	25,000	23,700	0.0200	0.560	14	Maximum	11,330	0.0388	27,220	27,220	27,220	27,220	3,051	3,051	3,051	2.40	1.29	—	
1/2	19	5	5	3	3	23,100	24,300	25,000	23,700	0.0200	0.639	24	Minimum	8,740	0.0347	19,780	19,780	19,780	19,780	2,332	2,332	2,332	2.13	1.42	1-1/2	
1/2	3	4	4	2	2	23,100	24,300	25,000	23,700	0.0200	0.535	10	Average	10,640	0.0385	22,900	22,900	22,900	22,900	2,622	2,622	2,622	2.24	1.52	—	
1/2	1	12	5	4	4	23,100	24,300	25,000	23,700	0.0200	0.569	24	Maximum	12,760	0.0440	24,900	24,900	24,900	24,900	3,160	3,160	3,160	2.43	1.60	—	
1	8	2	2	1	1	23,100	24,300	25,000	23,700	0.0200	0.598	23	Minimum	10,020	0.0338	22,390	22,390	22,390	22,390	2,867	2,867	2,867	1.32	1.12	1-1/2	
1	9	2	2	0	0	23,100	24,300	25,000	23,700	0.0200	0.562	24	Average	10,890	0.0370	25,730	25,730	25,730	25,730	2,946	2,946	2,946	1.62	1.22	—	
1	12	10	10	4	4	23,100	24,300	25,000	23,700	0.0200	0.443	5	Maximum	11,330	0.0388	27,220	27,220	27,220	27,220	3,051	3,051	3,051	1.81	1.40	1-1/2	
						23,100	24,300	25,000	23,700	0.0200	0.535	10	Minimum	8,740	0.0347	19,780	19,780	19,780	19,780	2,332	2,332	2,332	1.82	1.40	—	
						23,100	24,300	25,000	23,700	0.0200	0.569	24	Average	10,640	0.0385	22,900	22,900	22,900	22,900	2,622	2,622	2,622	1.87	1.41	1-3/4	
						23,100	24,300	25,000	23,700	0.0200	0.598	23	Maximum	12,760	0.0440	24,900	24,900	24,900	24,900	3,160	3,160	3,160	2.00	1.47	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Minimum	7,130	0.0288	19,120	19,120	19,120	19,120	2,520	2,520	2,520	1.95	1.51	—	
						23,100	24,300	25,000	23,700	0.0200	0.598	23	Average	8,480	0.0350	22,840	22,840	22,840	22,840	2,793	2,793	2,793	2.07	1.52	2	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Maximum	9,700	0.0336	22,680	22,680	22,680	22,680	2,749	2,749	2,749	2.24	1.36	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Average	10,200	0.0355	23,070	23,070	23,070	23,070	2,889	2,889	2,889	1.90	1.48	3	
						23,100	24,300	25,000	23,700	0.0200	0.598	23	Maximum	10,690	0.0388	23,890	23,890	23,890	23,890	3,019	3,019	3,019	2.13	1.59	—	
						23,100	24,300	25,000	23,700	0.0200	0.598	23	Minimum	8,990	0.0301	22,100	22,100	22,100	22,100	2,666	2,666	2,666	1.95	1.17	3	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Average	10,170	0.0355	23,430	23,430	23,430	23,430	2,870	2,870	2,870	1.51	1.22	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Maximum	11,160	0.0391	25,110	25,110	25,110	25,110	3,073	3,073	3,073	1.67	1.17	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Minimum	9,360	0.0290	22,420	22,420	22,420	22,420	2,659	2,659	2,659	1.13	—	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Average	11,120	0.0358	24,140	24,140	24,140	24,140	3,111	3,111	3,111	1.18	—	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Maximum	12,760	0.0484	26,360	26,360	26,360	26,360	3,552	3,552	3,552	1.23	—	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Minimum	9,600	0.0368	19,580	19,580	19,580	19,580	2,310	2,310	2,310	1.24	1.29	1-1/2	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Average	10,490	0.0409	21,450	21,450	21,450	21,450	2,568	2,568	2,568	1.49	1.30	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Maximum	12,230	0.0485	23,290	23,290	23,290	23,290	2,824	2,824	2,824	1.65	1.31	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Minimum	9,700	0.0336	22,680	22,680	22,680	22,680	2,749	2,749	2,749	1.23	1.35	—	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Average	10,200	0.0355	23,070	23,070	23,070	23,070	2,889	2,889	2,889	1.37	1.37	2	
						23,100	24,300	25,000	23,700	0.0200	0.562	24	Maximum	10,690	0.0388	23,890	23,890	23,890	23,890	3,019	3,019	3,019	1.46	1.38	3	

<sup>1</sup>Column 8 divided by column 15.

<sup>2</sup>Column 10 divided by column 17.

<sup>3</sup>End margin for tensile loading determined by modified compressive loading.

Table 5. Bolt-bearing and compressive strength and ultimate penetration for laboratory-made, cross-banded, yellow birch plywood. Bolt-bearing tests were made under tensile loading with load at angles of 0°, 45°, 90°, and 135° to the grain of the face plies. Compression tests were made parallel to the grain of the face plies.

Bolt diameter	Bolt thread	Bolt diameter	Bolt thread	Number of specimens tested	Number of specimens tested for proportional limit	Number of specimens tested for ultimate	Bolt-bearing			Compression			Ratio of bolt-bearing stress to bearing stress			Critical dimensions						
							Proportional limit	Ultimate	Ratio of bolt-bearing stress to bearing stress	Proportional limit	Ultimate	Ratio of bolt-bearing stress to bearing stress	Proportional limit	Ultimate	Ratio of bolt-bearing stress to bearing stress							
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)	(20)	(21)	(22)	(23)
1/4	1/4	1/4	10	6	7	Minimum 11,560 Average 20,300 Maximum 23,600	24,000 27,500 31,500	0.465 0.730 0.930	0.0164	0.599	6	Minimum 10,400 Average 11,770 Maximum 12,600	0.0058 0.0051 0.0050	22,170 24,800 26,800	0.474	2,269 2,775 3,465	1.53 1.71 2.02	1.24 1.27 1.47	2-1/2	4		
1/4	1/2	2	5	5	2	Minimum 13,200 Average 18,300 Maximum 23,500	24,000 27,500 31,500	0.55 0.67 0.84	0.0164	0.403	25	Minimum 8,980 Average 10,560 Maximum 11,500	0.0073 0.0062 0.0045	18,600 21,400 24,400	0.492	2,300 2,495 2,570	1.39 1.45 1.31	1.76 1.77 1.70	---	5		
1/4	1/2	7	6	6	4	Minimum 15,400 Average 16,770 Maximum 18,750	22,000 24,000 27,500	0.723 0.761 0.795	0.0164	0.466	26	Minimum 8,700 Average 10,600 Maximum 12,760	0.0047 0.0045 0.0040	19,700 22,900 24,900	0.473	2,332 2,628 3,160	1.42 1.55 1.73	1.44 1.51 1.60	3	---		
1/2	1/4	4	10	10	6	Minimum 13,200 Average 14,650 Maximum 16,500	24,000 27,500 31,500	0.55 0.57 0.62	0.0164	0.516	6	Minimum 10,400 Average 11,770 Maximum 12,600	0.0058 0.0051 0.0050	22,170 24,800 26,800	0.474	2,269 2,775 3,465	1.19 1.24 1.39	1.08 1.24 1.20	2	3-1/2		
1/2	1/2	7	10	10	7	Minimum 14,900 Average 16,700 Maximum 18,600	24,000 27,500 31,500	0.627 0.671 0.722	0.0164	0.518	26	Minimum 8,700 Average 10,400 Maximum 12,760	0.0047 0.0045 0.0040	19,700 22,900 24,900	0.473	2,332 2,628 3,160	1.24 1.46 1.72	1.26 1.33 1.43	2-1/2	3-1/2		
1/4	1/4	6	9	9	7	Minimum 19,200 Average 21,500 Maximum 23,300	24,000 27,500 31,500	0.805 0.850 0.901	0.0164	0.622	10	Minimum 10,650 Average 11,810 Maximum 12,500	0.0057 0.0058 0.0050	21,600 22,920 23,920	0.515	2,046 3,046 3,228	1.64 1.73 1.79	1.34 1.43 1.53	2-1/2	3		
1/4	1/2	18	10	10	9	Minimum 14,400 Average 16,200 Maximum 18,200	24,000 27,500 31,500	0.60 0.64 0.68	0.0164	0.490	62	Minimum 8,850 Average 10,180 Maximum 11,520	0.0052 0.0047 0.0045	19,900 22,900 25,500	0.463	2,271 2,693 3,009	1.47 1.61 1.80	1.31 1.38 1.44	2-1/2	3		
1/2	1/4	6	9	9	8	Minimum 13,400 Average 14,700 Maximum 15,700	24,000 27,500 31,500	0.67 0.71 0.74	0.0164	0.526	10	Minimum 10,650 Average 11,810 Maximum 12,500	0.0057 0.0058 0.0050	21,600 22,920 23,920	0.515	2,046 3,046 3,228	1.14 1.25 1.31	1.09 1.23 1.31	2-1/2	2-1/2		
1/2	1/2	18	9	9	8	Minimum 12,500 Average 13,800 Maximum 15,700	24,000 27,500 31,500	0.516 0.57 0.59	0.0164	0.467	62	Minimum 8,850 Average 10,180 Maximum 11,520	0.0052 0.0047 0.0045	19,900 22,900 25,500	0.463	2,271 2,693 3,009	1.22 1.37 1.55	1.22 1.36 1.53	2-1/2	2-1/2		
1/4	1/4	17	10	10	5	Minimum 19,200 Average 21,500 Maximum 23,300	24,000 27,500 31,500	0.805 0.850 0.901	0.0164	0.593	14	Minimum 10,020 Average 10,890 Maximum 11,330	0.0058 0.0050 0.0045	22,300 25,730 27,820	0.423	2,067 2,941 3,094	1.43 1.64 1.94	1.14 1.27 1.44	3-1/2	3		
1/4	1/2	19	10	10	5	Minimum 24,700 Average 25,650 Maximum 26,500	24,000 27,500 31,500	0.779 0.807 0.836	0.0164	0.536	28	Minimum 9,600 Average 10,490 Maximum 12,230	0.0056 0.0049 0.0045	19,500 21,450 23,250	0.469	2,310 2,564 2,628	1.40 1.49 1.58	1.63 1.64 2.02	4	4		
1/2	1/4	17	8	6	3	Minimum 11,900 Average 12,850 Maximum 13,800	24,000 27,500 31,500	0.50 0.54 0.57	0.0164	0.469	14	Minimum 10,020 Average 10,890 Maximum 11,330	0.0058 0.0050 0.0045	22,300 25,730 27,820	0.423	2,067 2,941 3,094	1.06 1.13 1.34	1.00 1.10 1.18	3	4		
1/2	1/2	19	7	7	3	Minimum 13,400 Average 14,100 Maximum 14,870	24,000 27,500 31,500	0.565 0.605 0.625	0.0164	0.473	28	Minimum 9,600 Average 10,490 Maximum 12,230	0.0056 0.0049 0.0045	19,500 21,450 23,250	0.469	2,310 2,564 2,628	1.28 1.35 1.42	1.28 1.30 1.40	4	---		

<sup>1</sup>Column 8 divided by column 15.

<sup>2</sup>Column 10 divided by column 17.

<sup>3</sup>Lightest specimen tested; failed in tension.

Z.M. 59357 F

Table 6.--Summary of critical dimensions for various grain angles

Diam-:Thick-:		Edge clearance			End margin		
eter: ness	of :	Compressive loading	Tensile loading	Modified	Tensile loading	compressive loading	
bolt: com-:	preg	Angle of grain of face plies to direction of load			Angle of grain of face plies to direction of load		
		0°	45°	90°	0°	45°	90°
Inch	Bolt diameters	Bolt diameters	Bolt diameters	Bolt diameters	Bolt diameters	Bolt diameters	Bolt diameters
1/4	1-1/2	2	1-1/2	2-1/2	3-1/2	4	3
1/4	1/2	2	1-1/2	2-1/2	4	4	3
1/4	1	2	--	--	--	--	3
3/8	1	--	--	--	--	4	--
1/2	1/4	1-1/2	2	2-1/2	3	3	3-1/2
1/2	1/2	1-3/4	2	2-1/2	4	4	3-1/2
1/2	1	2	2	2-1/2	--	3	2-1/2
1	1/2	1-1/2	2	2-1/2	2-1/2	3	3
1	1	1-1/2	2	2-1/2	3	3	3
					2-1/2	3	3
					3	2-1/2	3

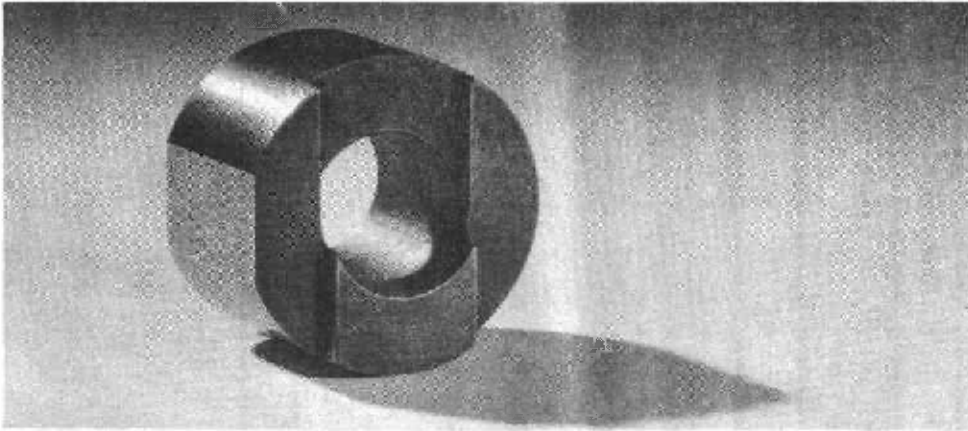
Table 7.--Summary of average bolt-bearing stresses at proportional limit and ultimate.

Diam-:Thick-: eter: of : bolt:	Stress at proportional limit			Stress at ultimate								
	Compressive loading <sup>1</sup>	Tensile loading	Angle of grain of face plies to direction of load	Compressive loading <sup>1</sup>	Tensile loading	Angle of grain of face plies to direction of load						
(1) : (2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)
Inch :Incl:	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.	Psi.
1/4 : 1/4	22,800:	21,390:	24,450:	20,100:	20,430:	17,810:	34,810:	30,730:	31,340:	33,530:	32,840:	32,780
1/4 : 1/2	20,590:	22,250:	24,350:	15,670:	16,350:	15,630:	34,240:	34,060:	34,710:	35,350:	36,300:	39,420
1/4 : 1	13,420:	--	--	--	--	--	34,030:	--	--	--	--	--
3/8 : 1	16,570:	--	--	--	--	--	33,160:	--	--	--	--	--
1/2 : 1/4	16,130:	18,500:	17,630:	14,630:	14,780:	12,960:	26,910:	28,360:	31,470:	28,360:	28,120:	27,610
1/2 : 1/2	16,980:	19,940:	19,580:	15,780:	13,880:	14,190:	27,670:	30,660:	30,660:	30,450:	29,700:	30,000
1/2 : 1	15,740:	17,850:	18,010:	--	--	--	29,120:	34,080:	32,560:	--	--	--
1 : 1/2	14,890:	15,910:	15,040:	--	--	--	27,730:	29,380:	27,610:	--	--	--
1 : 1	14,700:	16,160:	13,960:	--	--	--	27,160:	29,120:	31,520:	--	--	--

<sup>1</sup>Includes modified compressive loading.

<sup>2</sup>Bolt bent and sheared during test. High L/D ratio.





**Figure 1.--Recessed, hardened steel bushing used  
in bolt-bearing tests of compreg.**

Z M 56563 F

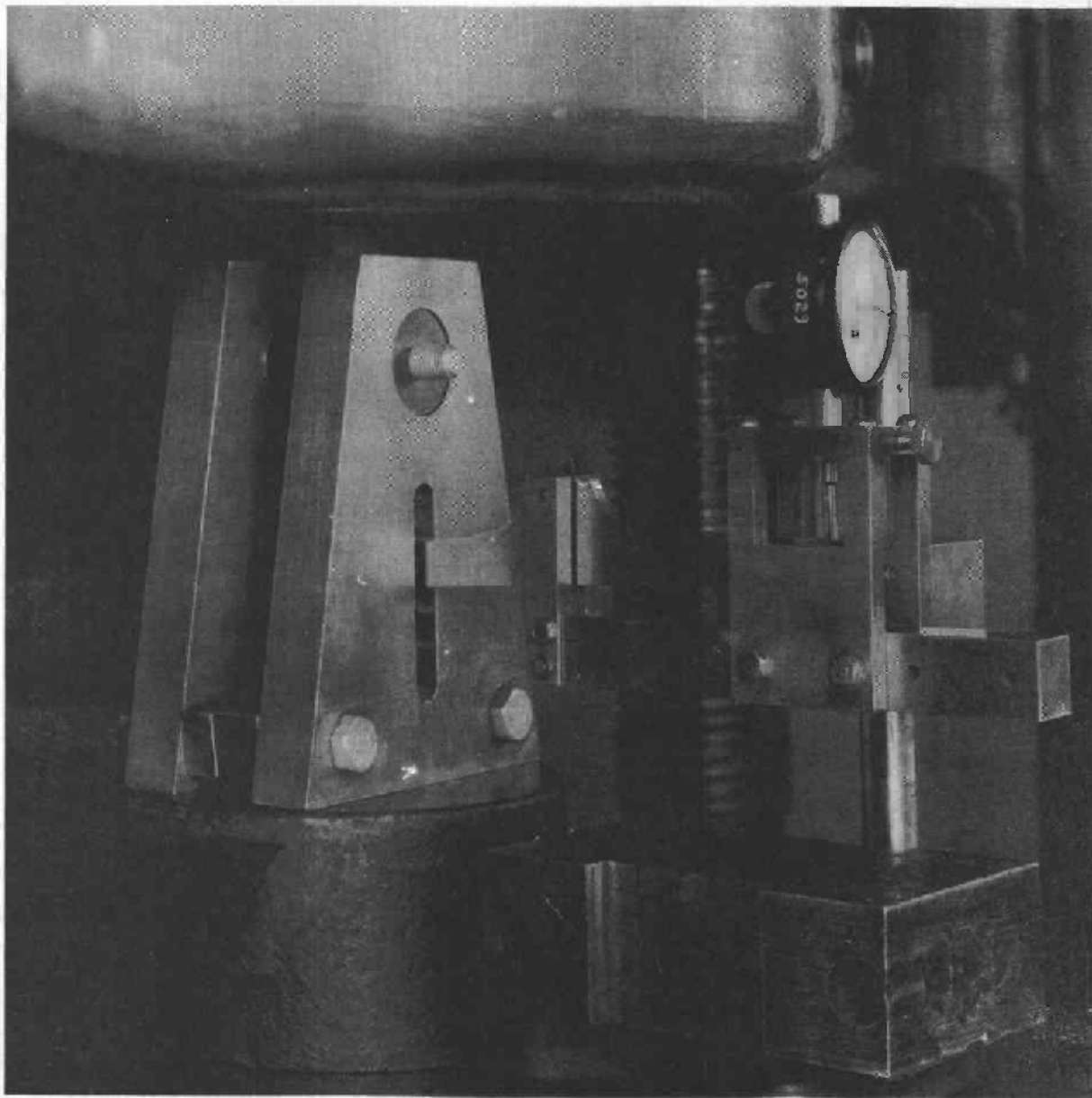


Figure 2.--Apparatus for bolt-bearing tests under compressive loading.

Z M 51671 F



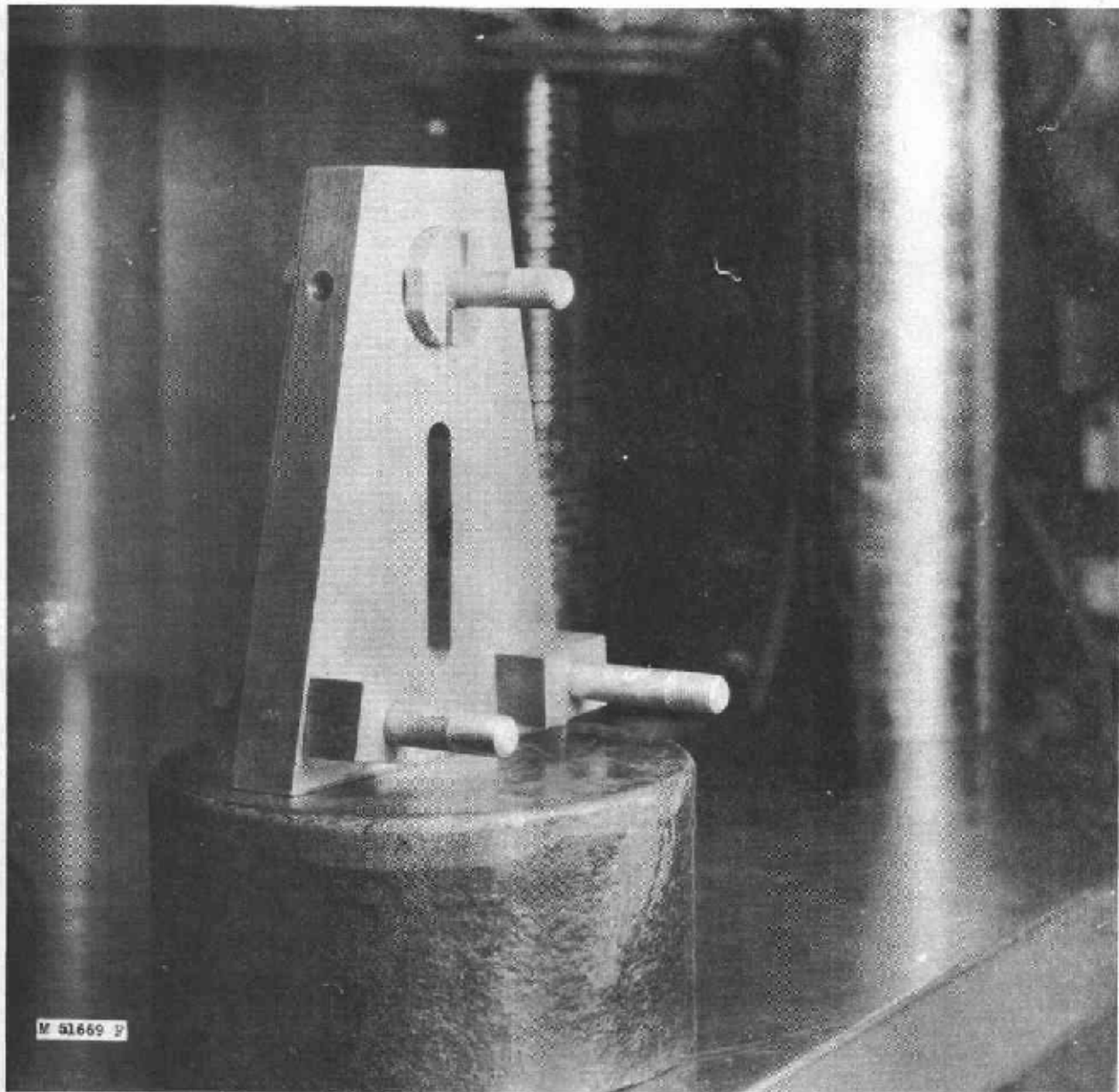


Figure 3.--Inside view of apparatus shown in figure 2 for bolt-bearing tests under compressive loading.

Z M 51669 F

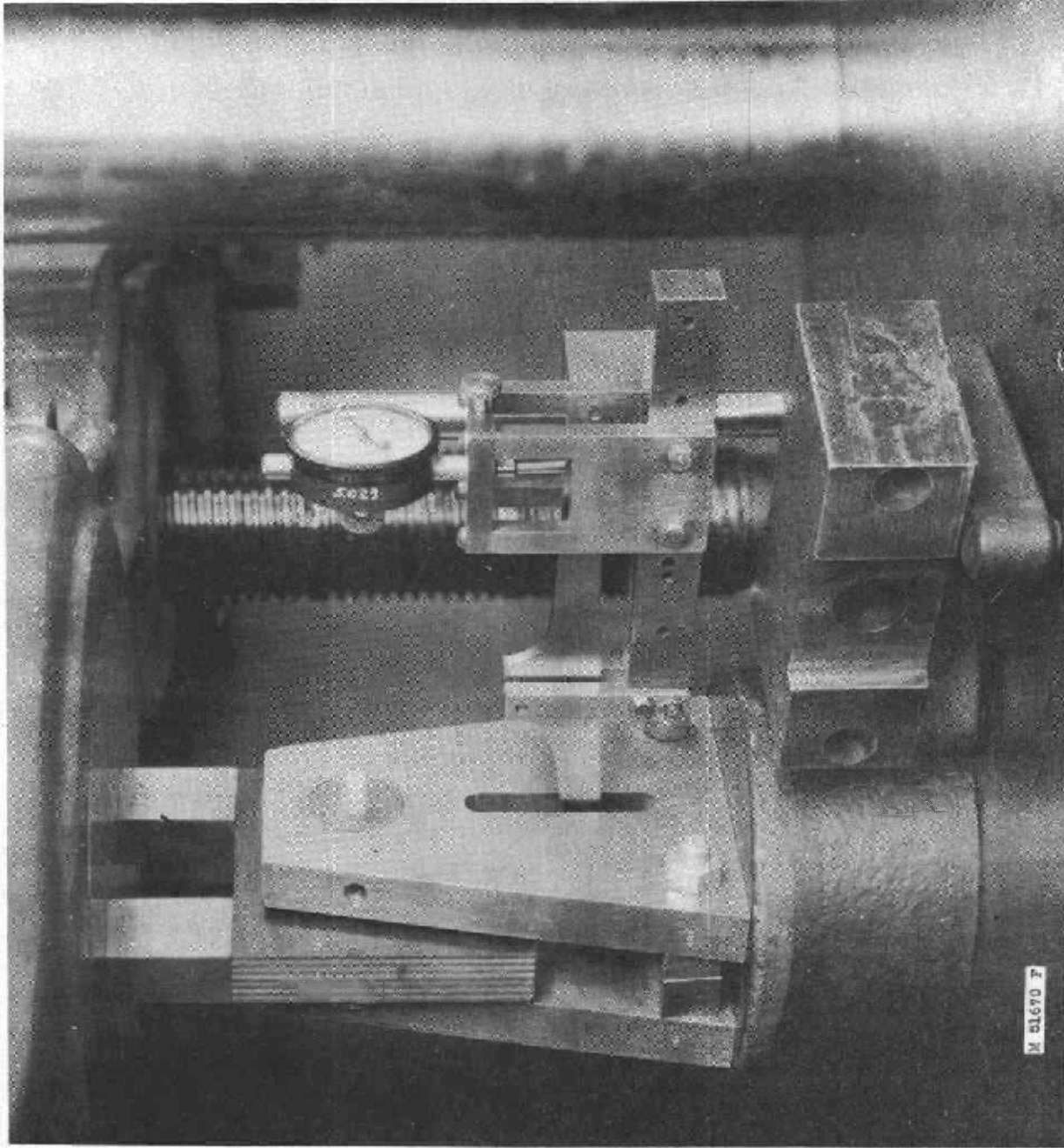


Figure 4.--Apparatus for bolt-bearing tests under modified compressive loading.

Z M 51670 F

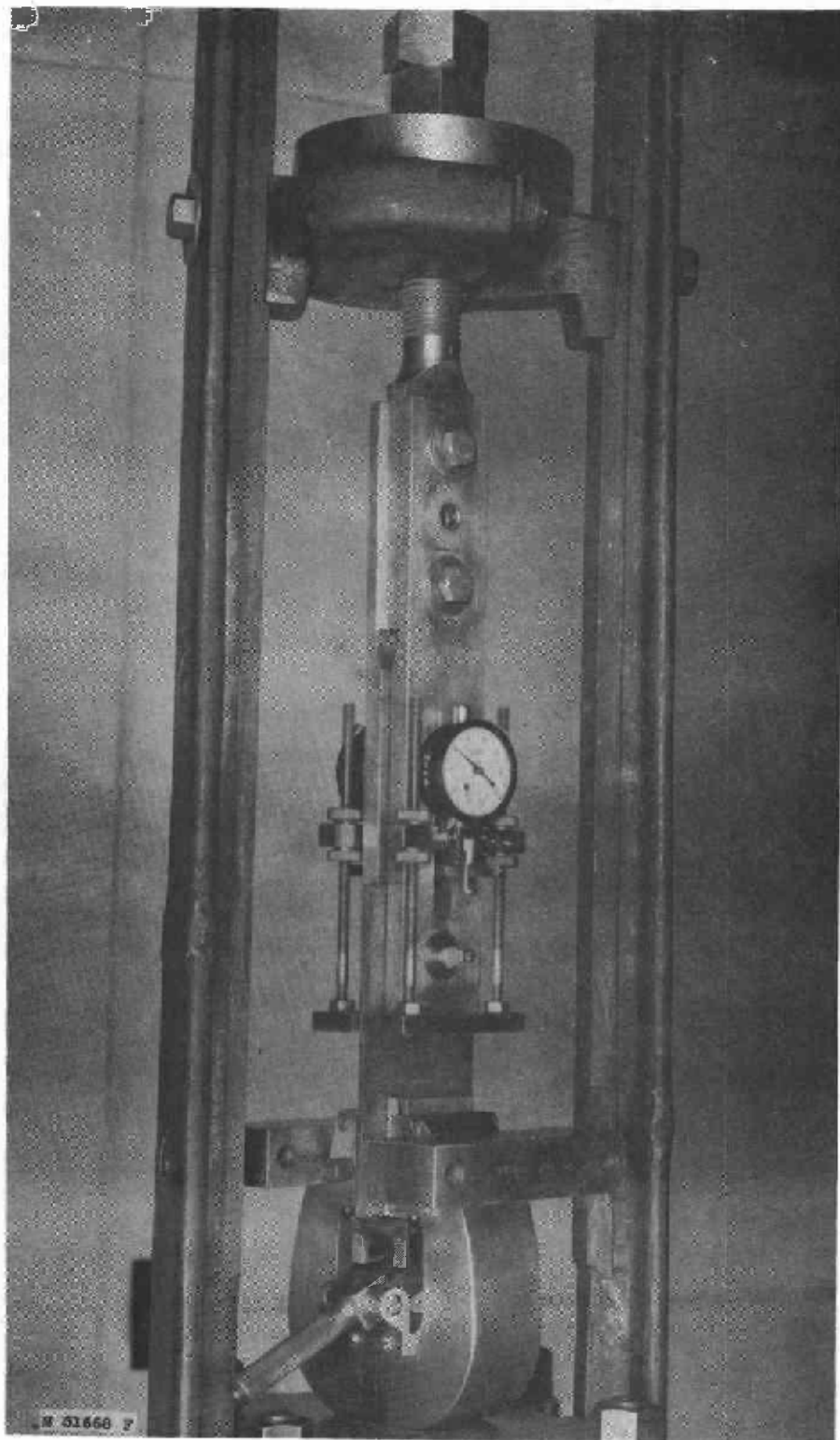


Figure 5.--Apparatus for bolt-bearing tests under tensile loading.

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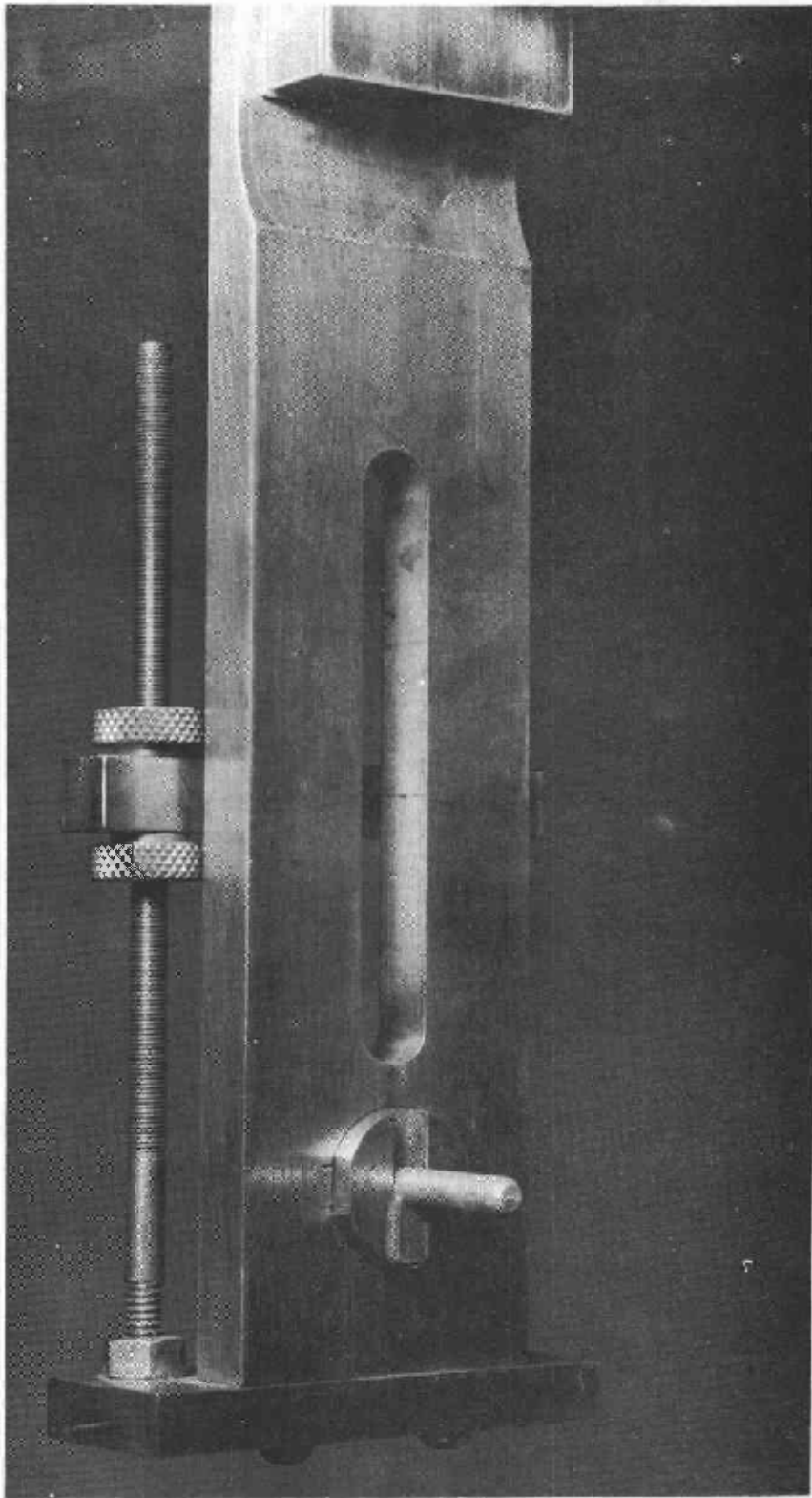


Figure 6.--Inside view of apparatus  
for bolt-bearing tests under ten-  
sile loading.

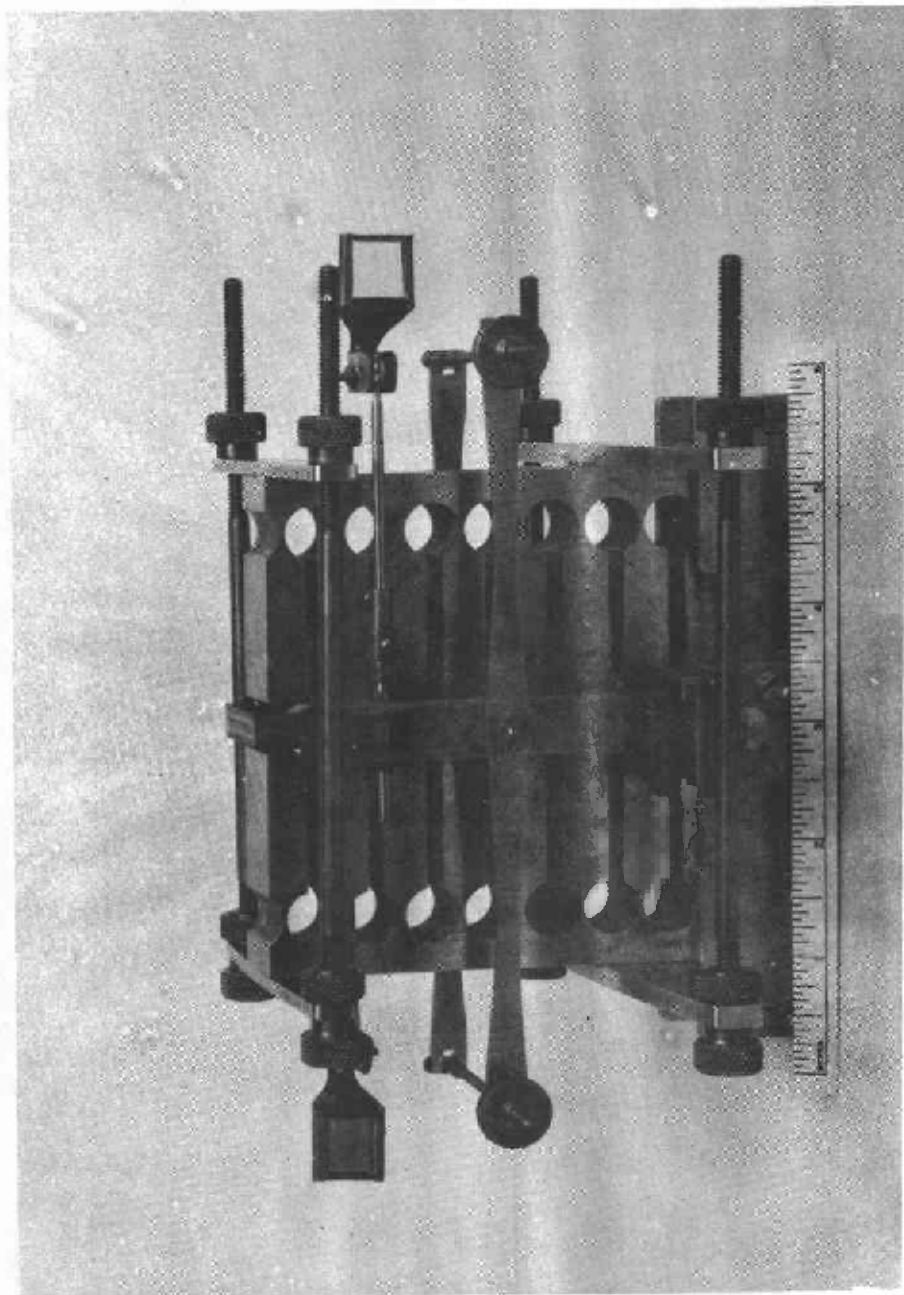


Figure 7.--Apparatus, including a 2-inch Martens' mirror com-  
pressometer, used in compression tests of laterally sup-  
ported 1/4- by 1- by 4-inch specimens.

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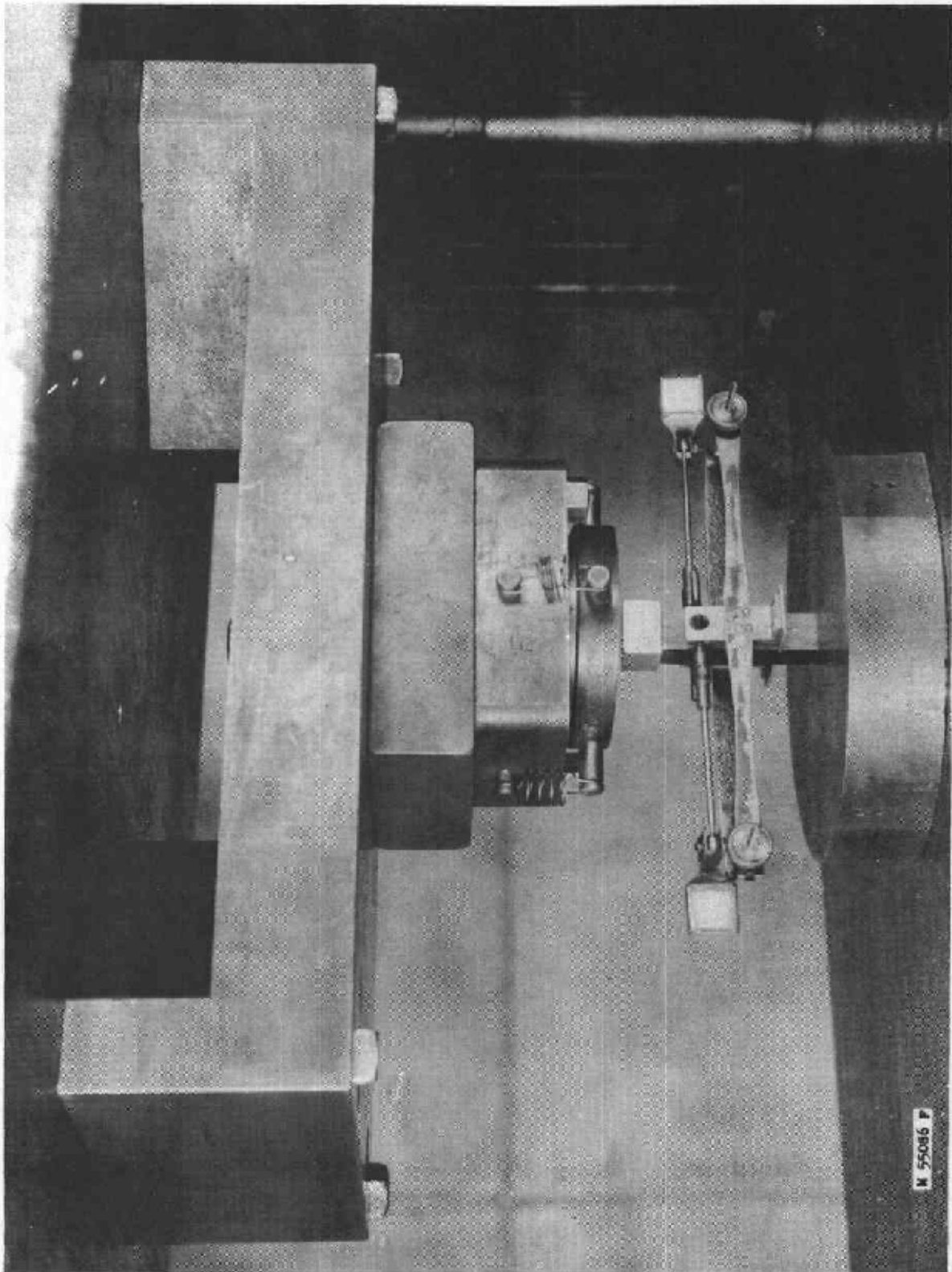


Figure 8.---Apparatus, including a 1-inch Martens' mirror compressometer, used for compression tests of 1/2- by 1/2- by 2-inch specimens.

Z M 55086 F

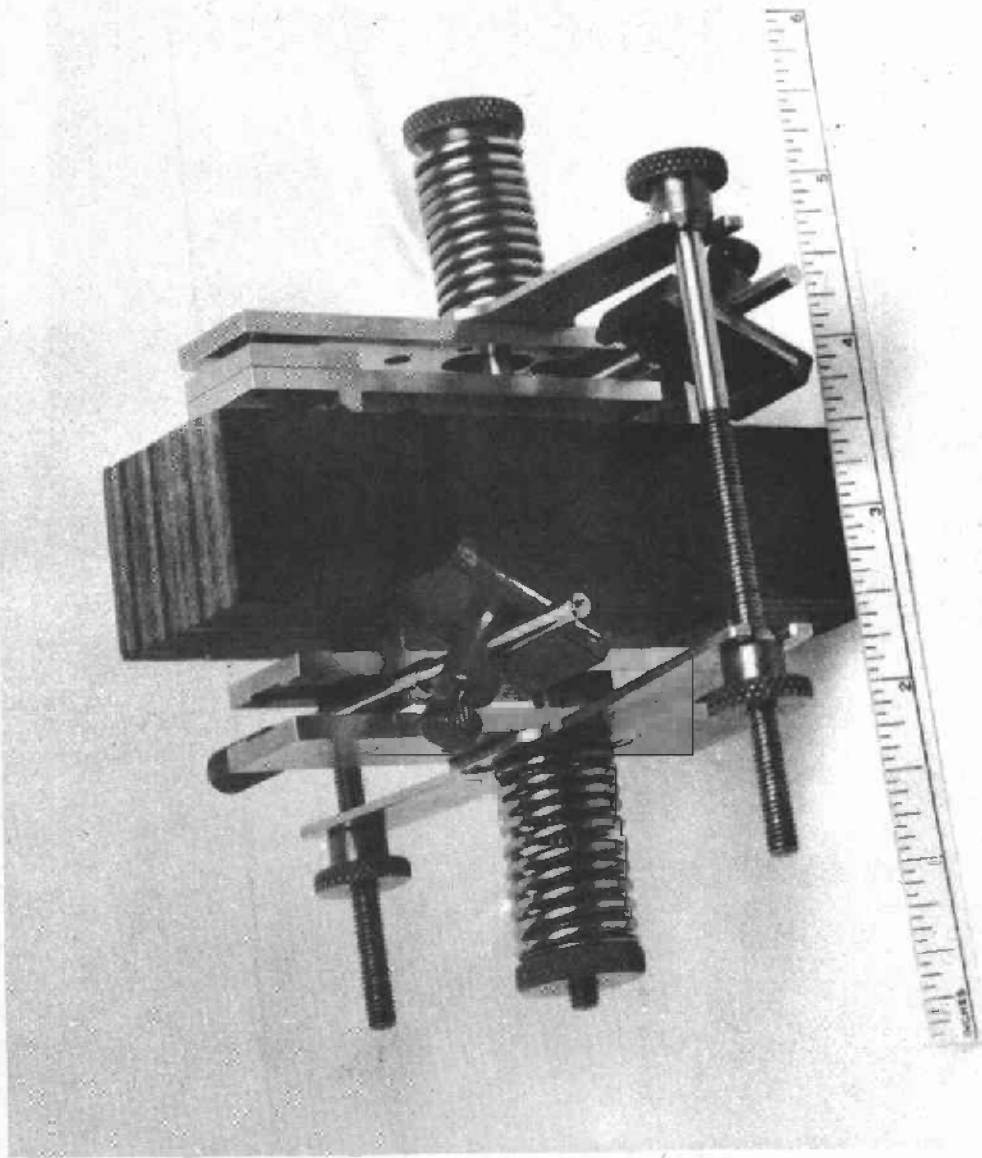


Figure 9.--Apparatus, including a roller type compression meter with averaging optical levers, used in compression tests of 1- by 1- by 4-inch specimens. Gage length = 2 inches.

Z M 55913 F

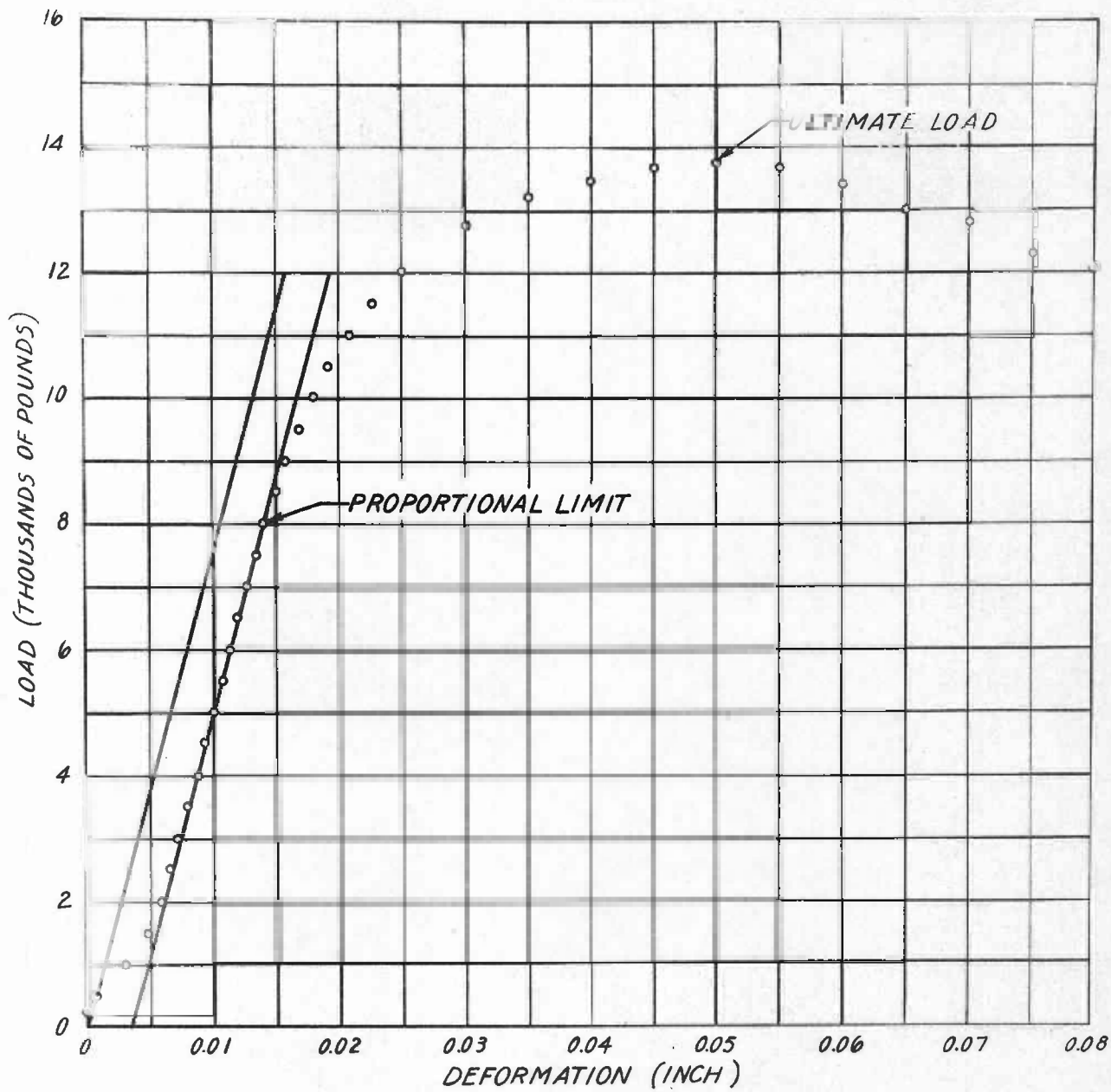


Figure 10.--Typical load-deformation curve for bolt-bearing test of 1-inch cross-banded compreg using 1/2-inch steel aircraft bolt under compressive loading parallel to the grain of the face plies.



EDGE CLEARANCE SERIES  
 COMPRESSIVE LOADING  
 0° TO FACE PLY

1/2 INCH STEEL AIRCRAFT BOLT

3/4 INCH LABORATORY-MADE CROSS-BANDED YELLOW BIRCH : SAWERS

EDGE CLEARANCE = 1/4 INCH TO CENTERLINE OF HOLE  
 PROPORTIONAL LIMIT LOAD = 1900 POUNDS  
 MAXIMUM LOAD = 2800 POUNDS

EDGE CLEARANCE = 1/2 INCH TO CENTERLINE OF HOLE  
 PROPORTIONAL LIMIT LOAD = 2000 POUNDS  
 MAXIMUM LOAD = 3095 POUNDS



EDGE CLEARANCE = 3/4 INCH TO CENTERLINE OF HOLE  
 PROPORTIONAL LIMIT LOAD = 2700 POUNDS  
 MAXIMUM LOAD = 3500 POUNDS

EDGE CLEARANCE = 1 INCH TO CENTERLINE OF HOLE  
 PROPORTIONAL LIMIT LOAD = 2600 POUNDS  
 MAXIMUM LOAD = 3450 POUNDS

EDGE CLEARANCE = 1 1/4 INCH TO CENTERLINE OF HOLE  
 PROPORTIONAL LIMIT LOAD = 2100 POUNDS  
 MAXIMUM LOAD = 3050 POUNDS

Figure 11.--Edge-clearance series tested under compressive loading parallel to grain of face ply. One-half inch steel aircraft bolt on 1/4-inch, Laboratory-made, cross-banded yellow birch compreg.

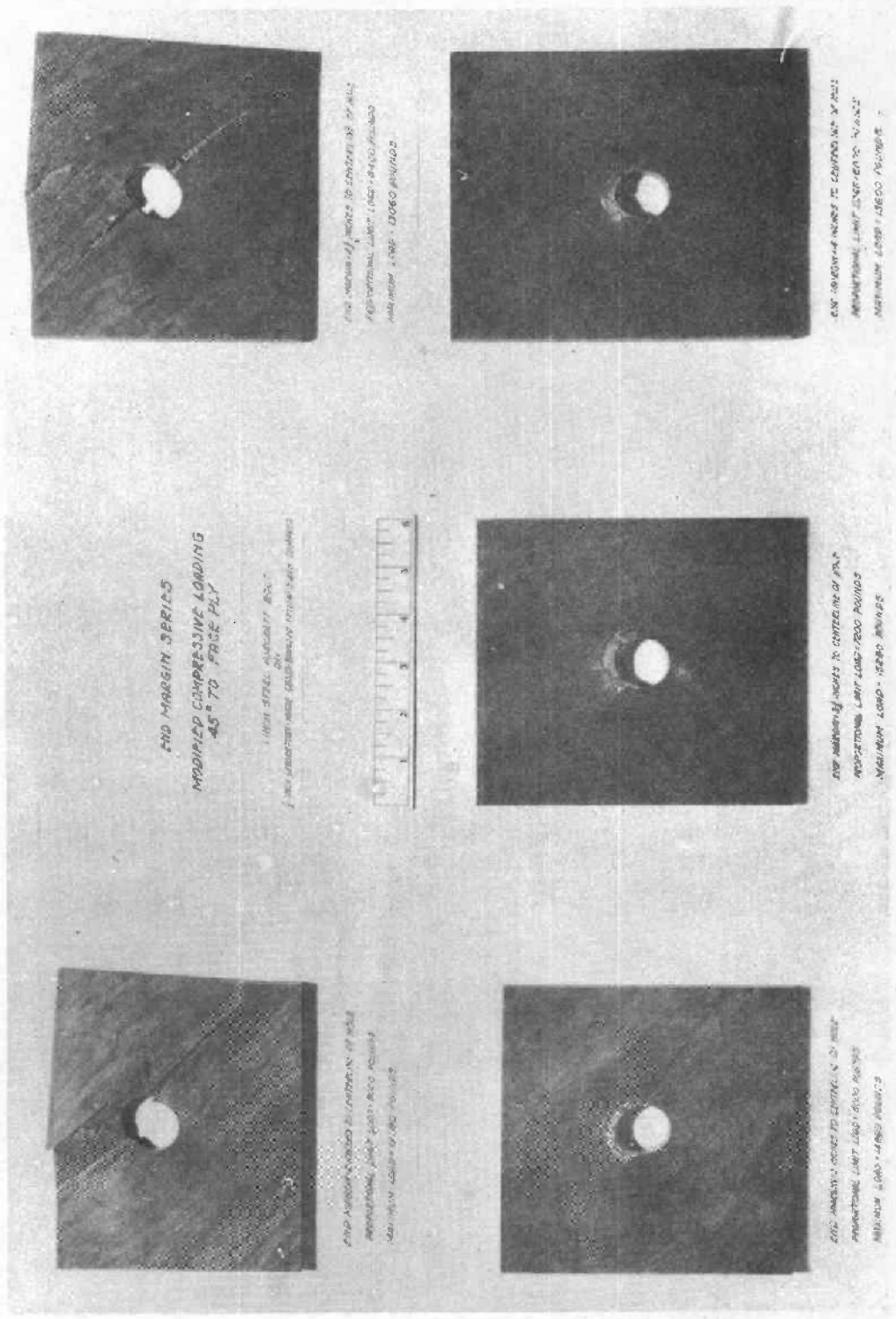


Figure 12.--End-margin series tested under modified compressive loading at an angle of 45° to the grain of the face ply. One-inch steel aircraft bolt on 1/2-inch, Laboratory-made, cross-banded yellow birch compreg.

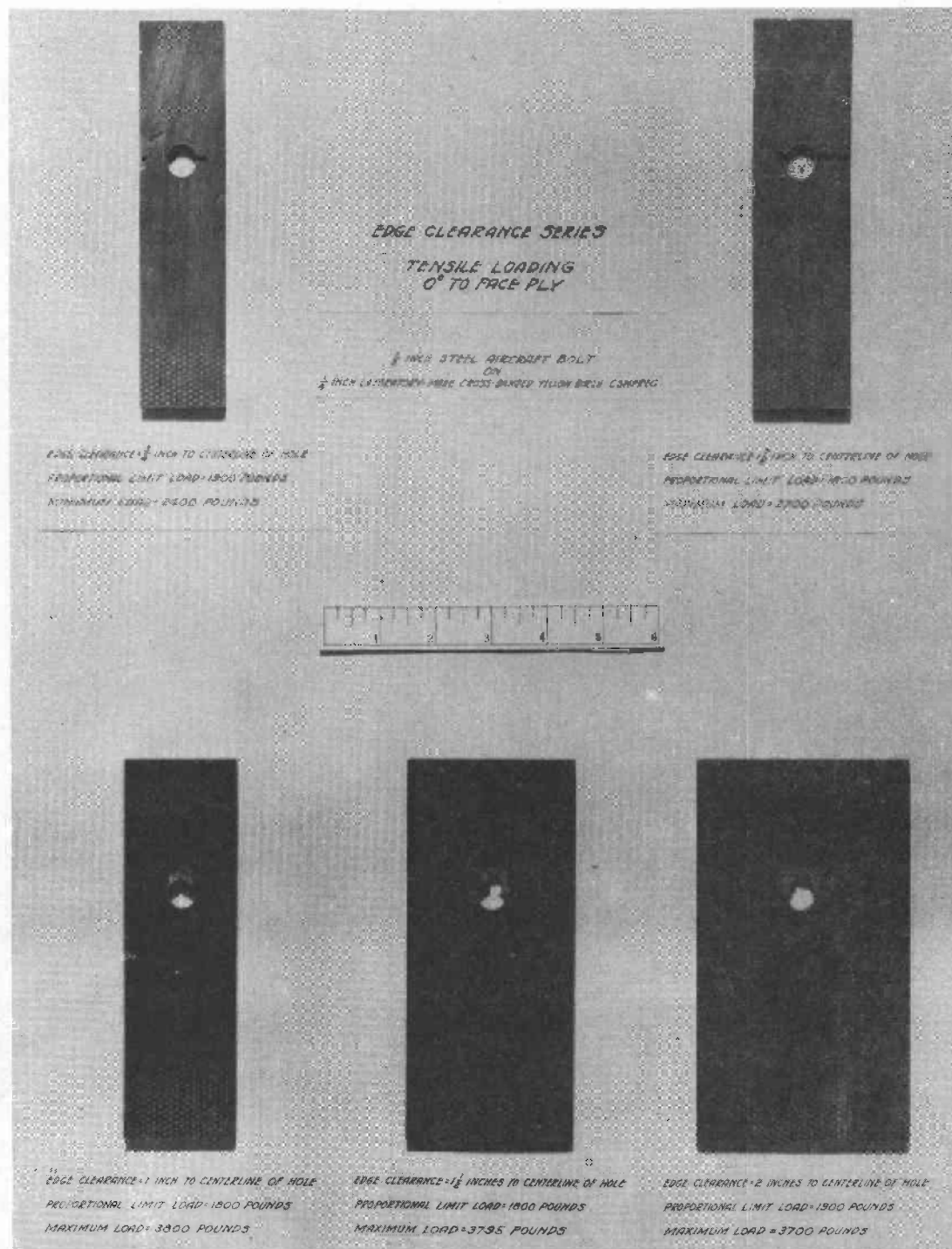


Figure 13.--Edge-clearance series tested under tensile loading parallel to the grain of the face ply. One-half-inch steel aircraft bolt on 1/4-inch, Laboratory-made, cross-banded yellow birch compreg.

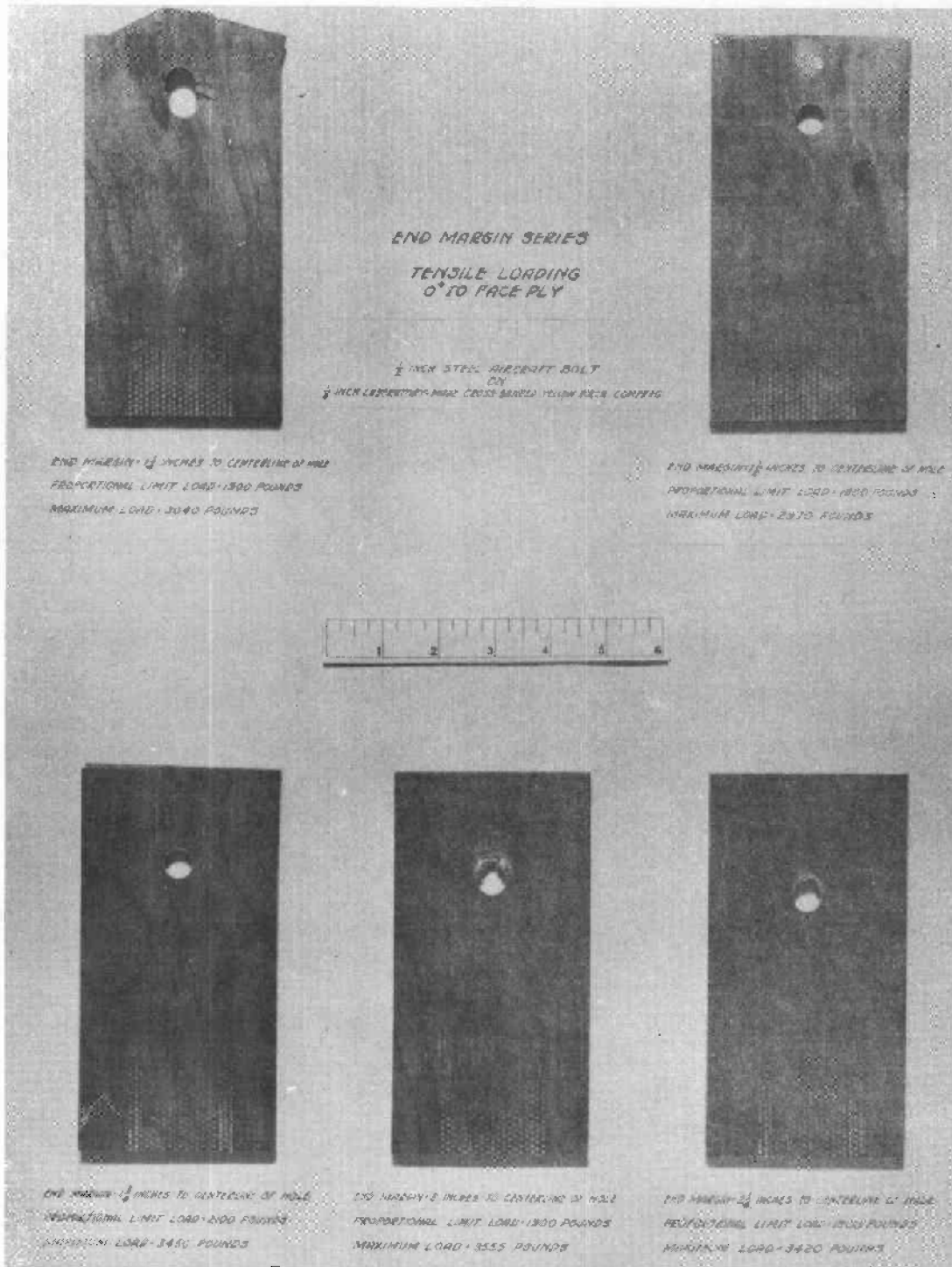


Figure 14.--End-margin series tested under tensile loading parallel to the grain of the face ply. One-half-inch steel aircraft bolt on 1/4-inch, Laboratory-made, cross-banded yellow birch compreg.



Figure 15.--Steel aircraft bolt sheared during bolt-bearing test of 1-inch cross-banded compreg under compressive loading. Ultimate load, 7,480 pounds; bolt diameter, 0.246 inch; bearing length, .949 inch.

Z M 60750 F