## On the use of the Material Point Method for large rotation problems

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14th June 2018

# (implicit) Material Point Method focus at Durham University 

seabed ploughing (Cortis)<br>screwpile installation (Wang)

overcoming volumetric locking (CMAME, 2018)
IGA-hased MPM (Ghaffari-Motlagh)
B-spline representation \& enforcement of boundaries (Bing)
generalised interpolation \& gradient plasticity (Charlton)

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## Background

University

- Designing foundations for offshore wind turbines is challenging because of the complex dynamic mechanical loading environment;
- monopiles are currently the most commonly used foundation in the offshore wind market due to their ease of installation;
- this research is part of a larger UK
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Screwpiles for wind energy foundation systems

- The research aims to make screw piles a more attractive foundation (or anchoring) option for offshore wind farms;
- installation torque in different seabed conditions is a key question;
- computational modelling of screw pile installation is Durham's focus;
- challenging problem: truly 3D, large deformation, non-linear material behaviour -



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## Implicit material point formulation

University
governing equations
governing equation of elasticity

$$
\nabla \sigma_{i j}+f_{i}^{b}=0 \quad \text { in } \quad \Omega
$$

subject to the following

$$
u_{i}=g_{i} \quad \text { on } \quad \partial \Omega_{D} \quad \text { and } \quad \sigma_{i j} n_{j}=t_{i} \quad \text { on } \quad \partial \Omega_{N}
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where $g_{i}$ and $t_{i}$ are the Dirichlet and Neumann boundary conditions
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$$
\int_{\varphi_{t}(E)}\left[\nabla S_{v p}\right]^{T}\{\sigma\} \mathrm{d} v-\int_{\varphi_{t}(E)}\left[S_{v p}\right]^{T}\{b\} \mathrm{d} v-\int_{\varphi_{t}\left(\partial \Omega_{N}\right)}\left[S_{v p}\right]^{T}\{t\} \mathrm{d} s=\{0\}
$$

## Implicit material point formulation

University
finite deformation mechanics
linear isotropic relationship is assumed between elastic logarithmic strains and Kirchhoff stresses

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\tau_{i j}=D_{i j k l}^{\mathrm{e}} \varepsilon_{k l}^{\mathrm{e}} \quad \text { where } \quad \varepsilon_{i j}^{\mathrm{e}}=\frac{1}{2} \ln \left(F_{i k}^{\mathrm{e}} F_{j k}^{\mathrm{e}}\right)
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and the deformation gradient is obtained as

the Cauchy stress is recovered using

the adopted stress and strain measures provide the most straightforward way of implementing large strain elasto-plasticity

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## Implicit material point formulation

finite deformation mechanics: deformation gradient update

A point of departure of implicit MP methods from conventional finite elements is the calculation of the deformation gradient

$$
F_{i j}=\Delta F_{i k} F_{k j}^{n} \quad \text { where } \quad \Delta F_{i j}=\delta_{i j}+\frac{\partial \Delta u_{i}}{\partial \tilde{X}_{j}}
$$

and $\tilde{X}_{i}=x_{i}-\Delta u_{i}$ are the coordinates at the start of the loadstep.

However, equilibrium is satisfied in the updated frame, requiring mapping of the shape function derivatives


Note that the spatial derivatives are needed to integrate the stiffness and internal force contribution of a material point in an updated Lagrangian formulation.

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$$

Note that the spatial derivatives are needed to integrate the stiffness and internal force contribution of a material point in an updated Lagrangian formulation.

## Numerics \& implicit implementation

University
basis functions


$$
\begin{array}{lr}
S_{v p}=1+\left(\tilde{X}_{p}-\tilde{X}_{v}\right) / h & -h<\tilde{X}_{p}-\tilde{X}_{v} \leq 0 \\
S_{v p}=1-\left(\tilde{X}_{p}-\tilde{X}_{v}\right) / h & 0<\tilde{X}_{p}-\tilde{X}_{v} \leq h,
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## Numerics \& implicit implementation

University non-linear solution
fully implicit Newton process used to solve the non-linear equation

$$
\left\{f^{o o b f}\right\}=\left\{f^{\text {int }}\right\}+\left\{f^{e x t}\right\}=\{0\}
$$

where

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\begin{aligned}
& \left\{f^{i n t}\right\}=\boldsymbol{A}_{\forall p}\left(\left[\nabla S_{v p}\right]^{T}\left\{\sigma_{p}\right\} V_{p}\right) \text { and } \\
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global consistent tangent determined analytically for optimal convergence

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$$

global consistent tangent determined analytically for optimal convergence (linearisation of the internal force with respect to the unknown displacements)

## Numerics \& implicit implementation

 computational procedureFor each loadstep:

1. assemble the internal force stiffness contribution of all material points;
2. increment the external tractions
and/or body forces in and solve for
the nodal displacements within a
loadstep using the Newton process;
3. update material point positions,
stresses, volumes, domains, etc.;
4. reset or replace the background grid.


## Numerics \& implicit implementation

University computational procedure

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## Numerics \& implicit implementation

University

For each loadstep:


## Numerical examples

simple stretch (validation)
University


- $l_{0}=2, h=1$
- $E=10^{3}, \nu=0$
- von Mises, $\rho_{y}=400$
- $2^{2} \mathrm{MPs} /$ element
- plane strain
- moving mesh, edge displacement $u / l_{0}=2$


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## Numerical examples



- $l_{0}=2, h=1$
- $E=10^{3}, \nu=0.4$
- elastic behaviour
- $2^{2} \& 8^{2} \mathrm{MPs} / \mathrm{e}$
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- moving mesh, corner displacement
$\Delta x, \Delta y=4$


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## Numerical examples

 doughnut twist


- $R_{o}=10, R_{i}=5$
- $E=10^{6}, \nu=0$
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fixed outer boundary and incremental rotation $\Delta \alpha$ on
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University
doughnut twist


Will Coombs (Durham)

## Observations

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- A unified implicit computational framework for sMPM and CPDIs has been developed;
- moving mesh concept extended to include rotational deformation;
- CPDI approaches reduce the instabilities inherent in material point methods; but

- only the sMPM and CPDII approaches obtain physically meaningful solutions for large rotational problems;
- CPDI2q faces issues due to distortion of particle domains; and
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## Acknowledgements

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The research presented is the work of $\operatorname{Dr}$ Lei Wang supported by the Engineering and Physical Sciences Research Council (EPSRC) grant EP/N006054/1: Screw piles for wind energy foundation systems.

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