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0 A new enri
hed dual boundary element method for fracture in anisotropic materials

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Introduction 0.1

Fracture mechanics has been studied for over than 50 years, but it is still receiving great attention from the scientific community. One of the reasons is the appearan
e of a range of new materials, su
h as omposite, piezoele
tri
, magnetoele
troelasti materials, just to ite a few examples. All these materials have a ommon hara
teristi
: they present anisotropi behaviour, so material response to a given excitation is also conditioned to the crystal arrangements of this material (for more about crystal arrangements see $[1]$ for instance). Anisotropic materials have been widely employed in the aerospa
e and automobile industries, wind power generators, sporting goods, and also as piezoelectric sensors and a
tuators.

Analytical formulations to deal with anisotropic material in fracture mechanics problems are very limited $[2, 3]$. Hence, fracture mechanics problems are solved using numeri
al methods, su
h as the Finite Element Method (FEM) [4] and the Boundary Element Method (BEM) [5]. It has been shown that BEM is more accurate and effective than FEM. However, in the last 15 years, a new paradigm has been established for the FEM, where the partition of unity [6] was applied to describe the discontinuity due to the presence of the crack, where the crack is then modelled through additional degrees of freedom in the elements ontaining the ra
k. This approa
h was named Extended Finite Element Method $(X-FEM)$ [7] and it has been shown that it can provide similar accuracy to the one found in BEM models [8].

In this work we develop an alternative BEM formulation for anisotropi fracture problems using the partition of unity method (PUM). This idea has already been discussed by [9] for isotropic materials, and our objective is to generalise this approa
h for anisotropi materials. To this end, the PUM is implemented in a dual BEM ontext. The enri
hment fun
tions are derived in terms of the Stroh formalism [10] and further details on the implementation of the X-BEM are dis
ussed.

Furthermore, we ompare the results of the X-BEM with those obtained by alternative numeri
al te
hniques: dual BEM implemented in ombination with discontinuous quarter-point elements $[11, 12]$ and X-FEM with anisotropic enrichment functions [8].

0.2 Constitutive equations

Consider an anisotropic elastic domain Ω , then the static equilibrium equations in the presence of body forces **b** are defined as

$$
\sigma_{ij,j} + b_i = 0 \tag{0.1}
$$

Symmetry holds for the stress and strain tensors σ and ε , respectively,

$$
\sigma_{ij} = \sigma_{ji} \tag{0.2}
$$

$$
\varepsilon_{ij} = \varepsilon_{ji},\tag{0.3}
$$

where

$$
\varepsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})
$$
\n(0.4)

and u_i represent the displacements.

The linear onstitutive equations are given by the generalized Hooke's law

$$
\sigma_{ij} = C_{ijkl} \varepsilon_{kl} \tag{0.5}
$$

where C_{ijkl} define the material constants tensor, satisfying the following symmetry relations

$$
C_{ijkl} = C_{jikl} = C_{ijlk} = C_{klij}
$$
\n
$$
(0.6)
$$

0.3 The Dual Boundary Element Method (DBEM)

The boundary element method (BEM) has been established as a referen
e numerical method when dealing with linear elastic fracture mechanics problems [5]. BEM is known to be more accurate and robust than domain discretisation methods such as the more popular finite element method (FEM).

The dual BEM (DBEM) is the usual choice when dealing with fracture mechanics problems. Hong and Chen [13] presented the idea of the combined use of a BIE and its derivative. This was used for the first time in a fracture mechanics context when [14] presented the DBEM. It can be summarised by a displa
ement boundary integral equation (DBIE)

$$
c_{ij}(\boldsymbol{\xi})u_j(\boldsymbol{\xi}) + \int_{\Gamma} p_{ij}^*(\boldsymbol{x}, \boldsymbol{\xi})u_j(\boldsymbol{x})d\Gamma(\boldsymbol{x}) = \int_{\Gamma} u_{ij}^*(\boldsymbol{x}, \boldsymbol{\xi})p_j(\boldsymbol{x})d\Gamma(\boldsymbol{x}) \qquad (0.7)
$$

and a traction boundary integral equation (TBIE), obtained by the differentiation of (0.7) and further substitution in (0.5)

$$
c_{ij}(\boldsymbol{\xi})p_j(\boldsymbol{\xi}) + N_r \int_{\Gamma} s^*_{rij}(\boldsymbol{x}, \boldsymbol{\xi}) u_j(\boldsymbol{x}) d\Gamma(\boldsymbol{x}) = N_r \int_{\Gamma} d^*_{rij}(\boldsymbol{x}, \boldsymbol{\xi}) p_j(\boldsymbol{x}) d\Gamma(\boldsymbol{x}) \quad (0.8)
$$

where Γ represents all the boundaries (including crack boundaries) of domain Ω ; N_r is the outward unit normal to the boundary at the collocation point ξ ; c_{ij} is the free term deriving from the Cauchy Principal Value integration of the strongly singular kernels p_{ij}^* ; u_{ij}^* and p_{ij}^* are the displacement and traction fundamental solutions; d_{rij}^* and s_{rij}^* follow from differentiation and substitution into the generalised Hooke's law of u_{ij}^* and p_{ij}^* , respectively. The kernels u_{ij}^* and p_{ij}^* are given by

$$
u_{ij}^* = -\frac{1}{\pi} \Re \left\{ A_{jm} Q_{mi} \, \ln(z_m^x - z_m^{\xi}) \right\} \tag{0.9}
$$

$$
p_{ij}^* = \frac{1}{\pi} \Re \left\{ B_{jm} Q_{mi} \frac{\mu_m n_1 - n_2}{z_m^x - z_m^{\xi}} \right\}
$$
 (0.10)

where $\mathbf{n} = (n_1, n_2)$ is the unit normal at the observation point; $z_m^x = x_1 + z_2^x$ $\mu_m x_2, z_m^{\xi} = \xi_1 + \mu_m \xi_2$ are evaluated at the observation and collocation points, respectively; \Re denotes the real part ; $\mathbf{Q} = \mathbf{A}^{-1} (\mathbf{L}^{-1} + \overline{\mathbf{L}}^{-1})^{-1}$ with $\mathbf{L} = i \mathbf{A} \mathbf{B}^{-1}$ and L represents the complex conjugate of L .

The matrices **A**, **B** and constant μ_m are parameters from the Stroh formalism and can be obtained through the following eigenvalue problem

$$
\left(\begin{array}{c|c}\n-C_{2ij2}^{-1}C_{2ij1} & -C_{2ij2}^{-1} \\
\hline\nC_{1ij1} - C_{2ij1}^T C_{2ij2}^{-1} C_{2ij1} & -C_{2ij1}^T C_{2ij2}^{-1}\n\end{array}\right) \left(\begin{array}{c} A_m \\
\hline\nB_m\n\end{array}\right) = \mu_m \left(\begin{array}{c} A_m \\
\hline\nB_m\n\end{array}\right) \tag{0.11}
$$

where there is no summation on the m index.

0.3.1 Extended Boundary Element Method (X-BEM)

The extended boundary element method $(X-BEM)$ was first proposed by Simpson and Trevelyan [9] for fracture mechanics problems in isotropic materials. The main idea is the same as for the X-FEM, to model the asymptotic behaviour of the displacements around the crack tips by introducing new degrees of freedom. The displacements $\mathbf{u}^h(\mathbf{x})$ are thus redefined as

$$
\mathbf{u}^{h}(\mathbf{x}) = \sum_{i \in \mathcal{N}} N_{i}(\mathbf{x}) \mathbf{u}_{i} + \sum_{k \in \mathcal{N}^{CT}} N_{k}(\mathbf{x}) \sum_{\alpha} F_{\alpha}(\mathbf{x}) \mathbf{a}_{k}^{\alpha}
$$
(0.12)

where $\mathcal N$ and $\mathcal N^{\mathcal{CT}}$ are the sets of all nodes and the enriched nodes, respectively, N_i is the standard Lagrangian shape function associated with node i, \mathbf{u}_i is the vector of nodal degrees of freedom, and \mathbf{a}_{k}^{α} represents the amplitudes of the enri
hed basis fun
tions whi
h apture the asymptoti behaviour around the crack tips. In elastic materials, \mathbf{a}_k^{α} is an 8-component vector for two-dimensional problems, since only two nodal variables (u_1, u_2) and four enrichment functions are needed to describe all the possible deformation states in the vicinity of the crack-tip [8].

In this work, we use the anisotropic enrichment functions obtained by [8] for the X-FEM:

$$
F_l(r,\theta) = \sqrt{r} \begin{pmatrix} \Re\{A_{11}B_{11}^{-1}\beta_1 + A_{12}B_{21}^{-1}\beta_2\} \\ \Re\{A_{11}B_{12}^{-1}\beta_1 + A_{12}B_{22}^{-1}\beta_2\} \\ \Re\{A_{21}B_{11}^{-1}\beta_1 + A_{22}B_{21}^{-1}\beta_2\} \\ \Re\{A_{21}B_{12}^{-1}\beta_1 + A_{22}B_{22}^{-1}\beta_2\} \end{pmatrix}
$$
(0.13)

where $\beta_m = \sqrt{\cos \theta + \mu_m \sin \theta}$, r is the distance between the crack tip and an arbitrary position, θ is the orientation measured from a coordinate system centred at the crack tip.

Let us emphasise that the anisotropic enrichment functions can also be used for isotropic materials, since this is a degenerated case from anisotropic materials. For more details please refer to reference [8].

The X-BEM formulation is similar to the one used by Simpson and Trevelyan [9] for isotropic materials. The extended DBIE and the TBIE can be restated as

$$
c_{ij}(\xi)u_j(\xi) + \int_{\Gamma} p_{ij}^*(x,\xi)u_j(x)d\Gamma(x) + \int_{\Gamma_c} p_{ij}^*(x,\xi)F_{\alpha}(x)a_k^{\alpha}d\Gamma =
$$

$$
\int_{\Gamma} u_{ij}^*(x,\xi)p_j(x)d\Gamma(x)
$$

$$
c_{ij}(\xi)p_j(\xi) + N_r \int_{\Gamma} s_{rij}^*(x,\xi)u_j(x)d\Gamma(x) + N_r \int_{\Gamma_c} s_{rij}^*(x,\xi)F_{\alpha}(x)a_k^{\alpha}d\Gamma =
$$

$$
N_r \int_{\Gamma} d_{rij}^*(x,\xi)p_j(x)d\Gamma(x)
$$

(0.15)

where $\Gamma_c = \Gamma_+ \cup \Gamma_-$ stands for the crack surfaces Γ_+ and Γ_- . In this work, only the element containing the crack tip receives the enrichment function. Let us re
all that strongly singular and hypersingular terms arise from the integration of the p_{ij}^* , d_{rij}^* and s_{rij}^* kernels and they are regularised in the same way as shown in $[15]$.

The enrichment functions add new degrees of freedom in the BEM formulation, which causes the linear system of equations to become indeterminate. An easy form to overcome this issue is to include additional collocation points. Each new collocation point will provide 2 new equations (in x_1 and x_2 directions). In this work, 3 nodes are enriched in each crack face, which will add $3 \times 4 \times 2 \times 2 = 48$ new degrees of freedom to the problem. Therefore, 24 extra ollo
ation points are ne
essary to solve the linear system of equations.

Numerical results 0.4

In this section we show the crack opening displacement (COD) for anisotropic materials in two fracture mechanics examples. The numerical results are compared with the results from the extended BEM, the dual BEM with quarterpoint elements $[11]$ and the X-FEM.

The BEM formulation presented in [11] has the particularity of modelling only one crack surface, thus obtaining the COD straight from the solution of the linear system of equations.

0.4.1 Double edge crack

Figure 0.1 illustrates a square plate $(h/w = 1)$ with two edge cracks of length a under a uniform loading σ . The size of the crack is defined by $a/w = 0.25$. The material is a four graphite-epoxy laminate, where the elastic properties are: $E_1 = 144.8 \text{ GPa}, E_2 = 11.7 \text{ GPa}, G_{12} = 9.66 \text{ GPa} \text{ and } \nu_{12} = 0.21. \text{ The COD}$ was obtained for two distinct fibre orientation, $\phi = 75^{\circ}$ and $\phi = 90^{\circ}$. Results for the X-FEM are obtained using topological and geometrical enrichment, with a fixed area of $r_e/a = 0.2$ and a mesh of 4525 elements. For more information about both adopted enrichment types please refer to [8] for instance.

Figure 0.1: Double edge crack problem.

For the dual BEM and the X-BEM a 10 ontinuous quadrati element per side mesh was used. The crack is discretised with 10 discontinuous elements. The dual BEM presents a quarter-point in the element at the crack tip, while the X-BEM presents an enri
hed element.

Results are given in Figures 0.2 and 0.3. The COD of the X-FEM models is smaller than the ones of the BEM models. The X-BEM COD is higher at the upper crack face than its equivalent using quarter point. In this case, the X-FEM results an be underestimated ompared to the BEM results. Nevertheless, the displacements around the crack tip have the same \sqrt{r} behaviour.

0.4.2 Slanted centred crack

A square plate $h/w = 1$ with a slanted centred crack of length 2a under a uniform loading is represented in Figure 0.4, where $\beta = 30^{\circ}$. The size of the crack is $a/w = 0.5$. The material is a glass-epoxy composite with the following

Figure 0.2: COD for the double edge crack - $\theta = 75^{\circ}$.

elastic properties: $E_1 = 48.26 \text{ GPa}, E_2 = 17.24 \text{ GPa}, G_{12} = 6.89 \text{ GPa}$ and $\nu_{12} = 0.29$. The fibre orientation angle ϕ assumes values of 90° and 120° in this example. The BEM meshes present 6 quadrati elements per side, and 10 discontinuous elements at the crack. The X-FEM mesh contain 2025 elements and the geometrical enrichment uses a fixed area of $r_e/a = 0.2$.

The results are shown in Figures 0.5 and 0.5 for $\dot{\phi} = 90$ and 120°, respectively. Here we present the results in terms of the u_2 displacement only, since it is difficult to get an accurate COD for the X-FEM model, which can be seen by the deformed shape of the COD. Both CODs from the BEM models result in symmetrical relative displacements. The COD from the extended BEM is considerably larger than the other referen
e solutions. One of the possible reasons is that the extended BEM model is underestimating the stiffness of the plate. resulting in a larger COD.

0.5 **Summary**

An enri
hed boundary element method was proposed in this work. This method uses the benefits of the partition of unit to model in a more general way the asymptoti displa
ements around the ra
k tip. Existing anisotropi enri
hment fun
tions for the X-FEM were employed at the proposed method. Some numeri
al examples were evaluated, and the results ompared to a well established BEM formulation and the X-FEM. The crack opening displacement was com-

Figure 0.3: COD for the double edge crack - $\theta = 90^\circ$.

Figure 0.4: Slanted centred crack problem.

pared with the quarter point BEM and the X-FEM. For a flat horizontal edge crack, both BEM models have the same behaviour around the crack tip, and the

Figure 0.5: COD for the slanted crack problem - $\phi = 90^{\circ}$.

Figure 0.6: COD for the slanted crack problem - $\phi = 120^{\circ}$.

X-BEM considers a larger COD at the end of the crack. In a slanted crack, the COD in the X-BEM model is larger than both BEM and X-FEM. The X-BEM ould be used an alternative to the dual BEM with quarter-point elements.

0.6 A
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