

NUMERICAL SIMULATION OF
WATERSHED HYDROLOGY

by

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Mathematical Simulation of Rainfall
Runoff Processes

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PREFACE

This research is part of a continuing effort at The University of Texas to develop methods for a numerical simulation of watersheds which will be both operational from the standpoint of engineering usage and realistic from the standpoint of simulating physical processes occurring in the watershed. It is felt that the simulation processes must be closely related to real physical processes if the method is to be useful in predicting the effect of changes that may occur in a particular watershed. It is hoped that in the future efforts can be made to correlate parameters used in the numerical simulation with physical observations made in specific watersheds, and to establish the correspondence between the physical processes occurring in the watershed and the numerical simulation of these processes.

The Stanford Watershed Model was studied as the first generalized watershed model that appeared to be generally available. It soon became apparent that it was difficult to obtain a clear understanding of the inner workings of the model. However, in making a detailed translation of the model into Fortran IV compiler language, a rather complete understanding of the model was achieved and is presented as a part of this report. As the study progressed, it became apparent that the model represented a tremendous breakthrough in hydrologic research. But as with most scientific breakthroughs, its greatest value was the demonstration of what could be done with continuous accounting modeling of a portion of the hydrologic cycle.

Two somewhat conflicting goals emerge as one builds a watershed model. First, the model must give engineering answers to the problem of hydrograph synthesis. The practicing engineer or hydrologist wants a design hydrograph whose accuracy is commensurate with the accuracy of the other variables in the problem. But the research hydrologist wants a model which as closely resembles natural processes as possible. Only with close modeling will he have a tool which will help to understand and explain the complex processes that occur in the natural watershed.

In this study emphasis was continually given to utilizing meaningful parameters for each process, and an attempt was made to model the various physical processes as closely as possible. Hence, evaporation occurs only during daylight hours, transpiration only from the root zone, soil moisture movement in the unsaturated zone is based on the unsaturated form of Darcy's law, etc.

Chapter 1 discusses the general runoff process, identifying the important physical processes. Standard methods of estimating the storm runoff and the hydrograph are reviewed in Chapter 2. An evaluation of the Stanford Watershed Model IV is given in Chapter 3. Chapter 4 contains a discussion of flow through unsaturated porous media, an area unfamiliar to most engineers. A modified watershed model is presented in Chapter 5 with applications reported in Chapter 6. Suggestions for areas of future research are given in Chapter 7.

ABSTRACT

The Stanford Watershed Model has been reviewed and used as a pattern for developing a new watershed simulation model. The new model incorporates considerable flexibility of input data and model time steps which were not available in the Stanford Model. New parameters describing the infiltration, evaporation, and soil water movement processes have been defined and related to physical properties of the watershed where possible. Application to two experimental watersheds are reported.

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Chapter 1

THE RUNOFF PHENOMENON

The runoff portion of the hydrologic cycle embraces those processes by which precipitation, after reaching the watershed with both time and areal distribution, is converted into streamflow at the basin outlet. Observation has led to satisfactory identification of these processes; however, mathematical models of the various processes are not well developed although much effort is currently being expended in this field. The interaction of the various processes is not well understood. In this chapter, each of the processes which affect the conversion of precipitation into streamflow will be examined in a qualitative way. Figure 1.1 identifies these various processes.

Interception Storage

Some of the water falling to the ground is intercepted by vegetation and by man made structures. This water is said to be in interception storage. Interception storage is characterized by fixed maximum volume (with regard to the time scale of one precipitation event, although it varies seasonally). This water never reaches the earth surface, but subsequently evaporates. The storage is filled by the earliest part of the precipitation. Some storage capacity is regained during periods of no rainfall within the storm. The volume of interception storage available is a function of the exposed surface area of intercepting surfaces. This area will change with the growing season, as well as with change in

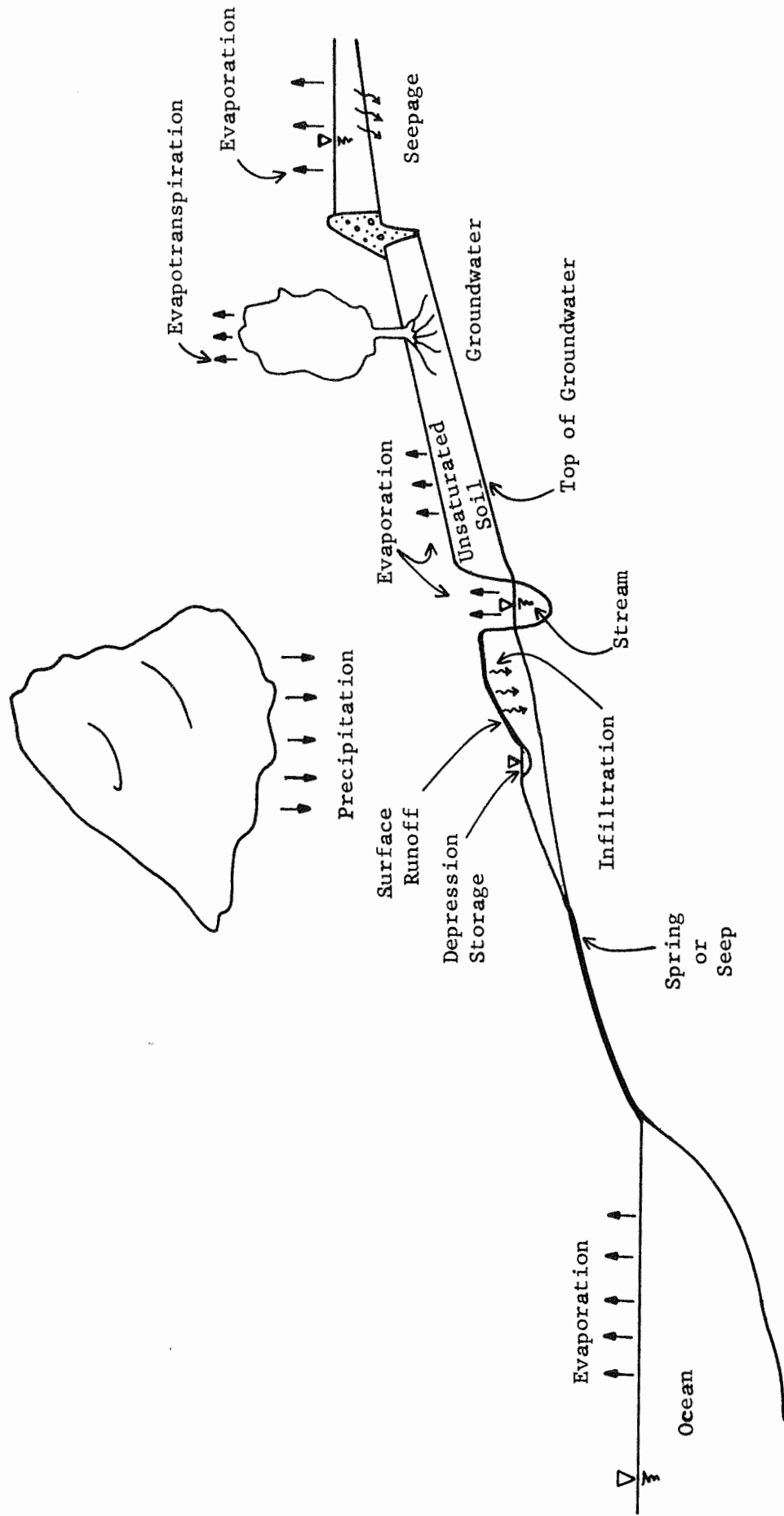


FIGURE 1.1 THE HYDROLOGIC CYCLE

land use.

Surface Depression Storage

This storage consists of cracks, crevices, depressions, and indentations in the earth's surface. If the watershed contains soils which exhibit shrinkage cracks upon drying, the volume of depression storage will decrease as the storm progresses. Water entering depression storage is removed by both the infiltration process and the evaporation process. The volume of surface depression storage available in a particular watershed depends on the nature of the watershed surface. Urban watersheds, in general, have much less depression storage volume than do agricultural lands, particularly agricultural lands where contour plowing and/or farm ponds are present. In agricultural watersheds, the condition of the land throughout the growing season will vary, causing a variation in the total depression storage volume.

Overland Flow and Overland Flow Storage

Overland flow, or sheet flow, is the flow of water at a small depth over the earth's surface. Overland flow provides the conveyance from the point where the water falls to the smallest "channel." It is essentially a two dimensional flow problem; the flow may be either laminar or turbulent. This process has been studied in detail both in theory and experimentally. Overland flow storage is water in transit in the overland flow process. It is a function of the length of the overland flow reach and the flow rate. Overland flow rate is a function

of the length, slope, and roughness of the reach.

Evaporation

The process of conversion from liquid or solid state to vapor state occurs continuously at all water-air interfaces. Evaporation is responsible for removal of water from interception storage, from surface depression storage, from both overland flow storage and stream storage, and from the water contained in the soil structure. Water is also removed from plants by transpiration, a form of evaporation.

Evaporation is inversely related to the partial pressure (or humidity) and directly to available energy. Wind may prevent the air adjacent to an air-water interface from becoming saturated, and hence may increase evaporation. In general evaporation increases with increased temperature and wind.

Evapotranspiration varies widely with the plant species, as well as with the temperature.

Infiltration

Infiltration is the entry of water into the pore spaces of the soil. In general, water will infiltrate more rapidly into a dry soil than a wet one, and more rapidly into a sand than a silt. Infiltration rates tend to decrease as the volume of water infiltrated increases. Many empirical formula have been proposed for describing either the instantaneous infiltration rate, or the average infiltration rate. Infiltration occurs continuously from depression storage and overland flow storage, provided water is available for infiltration.

Soil Water Storage and Flow

Water stored in the soil is either in the saturated zone or the unsaturated zone. Storage in and flow through the saturated zone is probably the best understood portion of the hydrologic cycle. Flow is proportional to the driving force and inversely proportional to a resistance factor. Storage consists of the pore space of the soil matrix. Storage in the upper part of the unsaturated zone, commonly called the root zone, has always been of concern to agriculturists. In addition to saturation, these people define two other terms to describe the amount of water in storage in the root zone. Field capacity refers to that volume of water held by capillary forces in the pore spaces against the force of gravity. Wilting point is that volume of water present in the soil when permanent wilting of plants takes place. Water available for plant use is that between field capacity and wilting point. Depth of the root zone may vary; twenty-four to thirty inches is often cited as the range.

Flow through the unsaturated zone has only recently received attention from investigators. Since this process is also the topic of this investigation, discussion of the factors involved will be delayed until Chapter 4.

Stream Flow and Stream Storage

Stream flow is differentiated from overland flow by the presence of a width of stream, i.e., the flow is three dimensional in the stream. The hydraulics of open channel flow are well understood for streams of

regular cross section. When inflow becomes a function of length, even these solutions are difficult. With the additional complexity of a natural channel, most investigators have turned to approximations. Best known of these is the unit hydrograph, which assumes a linear flow system. Another commonly used method is channel routing, where the continuity equation is combined with a channel equation. The channel equation describes the relation between the rate of inflow to a reach of the stream, the outflow from this reach, and the storage within the reach. Adequate channel equations do not exist, and approximations must be used.

Each of the processes sketched briefly here has been the subject of investigators in many different fields. An extensive list of references will be found in Chow (1964).

Chapter 2

METHODS OF ESTIMATING STORM RUNOFF

Many engineering design problems require an estimate of the response of a watershed to a "design" storm. Some of the more widely known methods are reviewed briefly in this chapter.

Methods of Estimating Runoff Volume

Soil Conservation Service Method: This method, developed by the Soil Conservation Service as a guide for their field personnel, is probably the most complete, comprehensive method available. It is based on observations made in many parts of the United States. The factors considered are soil type, land use, and land treatment. These are combined to give an index number, which is used in an equation together with rainfall to predict runoff volume. A table is provided for shifting the index to either a dry antecedent moisture condition (Condition I) or near saturated antecedent moisture condition (Condition III). The runoff equation is

$$Q = \frac{(P - 0.2 S)^2}{P + 0.8 S} \quad (2.1)$$

where Q is the direct runoff, in inches

P is storm rainfall, in inches, and

S is the maximum potential difference between P and Q
in inches at time of storm's beginning.

The value of S can be estimated by the relation

$$\text{Index Number} = \frac{1000}{10 + S} . \quad (2.2)$$

Coaxial Correlation: This method, proposed by Kohler and Linsley (1951), provides a graphical means of determining runoff from rainfall. The parameters used are antecedent precipitation index (API), the date of the storm, the amount of rainfall, and the duration of the storm. The API is an index to soil moisture, and hence, to infiltration. The time of year as reflected by the date is included to reflect the condition of the cover on the watershed, the likelihood of snow, and the condition of the soil surface. Duration provides a measure of average rainfall intensity. Recorded values of rainfall and concurrent storm runoff are required to derive the coaxial plot--in addition to a great deal of patience. The plot thus derived is applicable only to that particular basin. They have been applied, however, with good results to similar basins in the immediate vicinity. This method makes no attempt to model the rainfall runoff process, but rather represents a statistical approach.

Methods of Hydrograph Estimation, The Unit Hydrograph

The most popular method in use today is probably the unit hydrograph proposed by Sherman in 1932. The basic assumption made by Sherman was that the shape of the hydrograph was completely determined, except for a vertical scale factor, by the basin characteristics. Thus, a "characteristic" hydrograph of rainfall excess could be derived for any basin. By changing the ordinates of this graph, the area under the

curve, corresponding to runoff, could be made equal to one inch of excess rainfall over the basin. Hence the name, "unit hydrograph." The hydrograph for any other storm of similar duration and areal distribution can be found by adjusting the scale of the ordinate until the runoff volume is correct. It has been pointed out by many investigators that Sherman's assumption is equivalent to assuming a linear response by the basin. However, the natural channel is a nonlinear device. In spite of this departure from basic assumption, the unit hydrograph continues to be used with seemingly satisfactory results by most engineers.

Methods of Estimating Peak Flow

Chow (1962) has described many methods which have been used to estimate peak flow rates for use in determining required waterway openings. Only two methods will be described here, the Rational Method, and a method due to the Soil Conservation Service.

Rational Method: The origin of this widely used method is not known. Chow (1964) traces it to Kuichling in 1889 in American literature and to Lloyd-Davis in English writings. This method assumes that, for a constant rainfall of sufficient duration, called the time of concentration, the runoff rate becomes a fixed fraction of the rainfall rate, or

$$Q = C I A \quad (2.3)$$

where Q is the runoff rate in c. f. s.,

I is the rainfall rate in inches/hour,

A is the area in acres, and

C is a runoff factor, varying from zero (no runoff) to one (impervious area).

When used for design work, it is necessary to choose the proper value of both the coefficient C and the Intensity, I. The intensity is normally determined by assuming the peak flow will occur when runoff is produced on all the area. Thus, the "time of concentration" (the time necessary for water from the most hydraulically remote part of the watershed to reach the outlet) determines the rainfall duration. This in turn, is used in an intensity-duration relation to determine the value of I. It is sometimes possible to obtain higher rates of runoff by using only a portion of the basin when estimating time of concentration. The shorter time of concentration will yield a higher intensity, which may more than compensate for the loss of area.

Soil Conservation Service: This method is not described because of special merit, but rather as typical of current developments. As described by Kent (1968), the method utilizes the excess runoff determined by the Soil Conservation Service method described earlier in this chapter. It is somewhat similar to the Rational Method, in that a basin lag time is determined from a time of concentration. A chart is provided so that lag time can be estimated from the index number used in the determination of excess rainfall. Thus, the retardedness of the land surface is reflected in the value of the lag time. Other factors involved in the lag time are basin area and average basin slope. Two standard time distributions of rainfall are used, one for the west coast of the United States, the Hawaiian Islands and Alaska; and the other for the remainder of the United States, Puerto Rico, and the Virgin Islands. Both distributions produce a low intensity rainfall

during the first ten hours of a twenty-four hour period followed by a period of a very heavy rainfall, and finally another period of low intensity rainfall. The rainfall corresponds approximately to the twenty-five year frequency as shown in the Rainfall Atlases of the United States Weather Bureau. A triangular shaped hydrograph is assumed, with the peak runoff occurring at a time equal to the basin lag time plus one-half the storm duration. Peak discharge may be computed

$$Q_p = \frac{K A Q}{T_p} \quad (2.4)$$

where Q_p is the estimated peak rate of discharge

A is the area

Q is the storm runoff

K is a conversion factor, and

T_p is time to peak.

Equation 2.4 is for a uniform rain. In order to adapt this to the nonuniform standard rainfall, the equation was modified to

$$\Delta q_p = \frac{484 A \Delta Q}{\frac{\Delta D}{2} + L} \quad (2.5)$$

where A is the area, in square miles,

ΔQ is incremental runoff volume, inches

ΔD is an incremental time, in hours

L is lag, in hours, and

Δq_p is the incremental peak discharge, in c. f. s., i.e. the peak of a triangular hydrograph containing Q volume of runoff.

These incremental hydrographs are combined linearly to produce the design hydrograph. The whole process has been programmed for a digital computer, and charts are included which allow reading of the peak runoff rate directly as a function of drainage area, slope, and basin index number.

Chapter 3

THE STANFORD WATERSHED MODEL IV

The processes discussed in Chapter 1 are interrelated, and to some degree simultaneous in action. Adding to this complexity is the fact that observations are not generally available from controlled experiments, but rather, from random events in nature. This lack of controlled experiments is one of the major obstacles to hydrologic research. The result has been a great many empirical runoff relations. A few of these were discussed in the last chapter.

With the advent of the high speed digital computer, it has become feasible to construct a mathematical model of the entire runoff process. At least three such models have been reported in the literature. Apparently the first embryonic attempt began in 1957 when Professor Ray K. Linsley of Stanford University attempted a very simple rainfall-runoff study with one of his graduate classes in hydrology. From this beginning and with the aid of many other people, notably Dr. Norman H. Crawford, the model to be discussed in this chapter has evolved. This model is not static; Model VI is being tested at Stanford.

A second model, reported by Rockwood (1958), and developed by the Corps of Engineers, and the Portland River Forecast Center for use on the Columbia River Basin, is also undergoing evolution, Kuehl (1967), Schermerhorn and Kuehl (1968), Rockwood (1968). This model lacks detailed simulation of the natural processes; it is in fact, essentially

a flood routing model.

A third model has been reported by Boughton (1966) in Australia. This model was constructed with the Stanford model as a guide, and has as its distinguishing feature the use of daily rainfall data, rather than data from a continuous recorder. This, of course, has necessitated a longer scale of time for the model, with the resultant loss of detail. Other models have been reported by Holtan and Lopez (1969), by Dawdy (1969), and by Ayers and Balek (1968).

In addition to the work currently underway at Hydrocomp, Palo Alto, California, on the Stanford Model, and the work reported herein, Dr. L. Douglas James, of the University of Kentucky, has used a translation of the Stanford Watershed Model II, with some modifications from Model IV included. The computer programs written at Stanford were all in SUBALGOL, an extinct version of Burrough's early compiler. This compiler language barrier has, no doubt, limited the availability of the Stanford models to other researchers. Model IV, with the exception of the snowmelt subroutine, has been translated into Fortran IV by the author.

Some terms peculiar to the model need to be defined before proceeding to a discussion of the model and of the computer program. A parameter is a constant value associated with a particular basin, e.g., the recession constant for groundwater flow. A variable is the physical quantity determined by the model, e.g., the total evaporation. A flowpoint is the point on a stream where the hydrograph is to be computed by the model. A basin may contain more than one flowpoint. A

segment is the working unit of area for application of the runoff processes. A basin may consist of more than one segment.

The Stanford model is a continuous accounting model in the sense that the location of the water within the system is continuously known. The model, in fact, operates on a 15 minute cycle, i.e., adjustments in the location of water in the system are made each 15 minutes.

Figure 3.1 is a diagram of the watershed model as interpreted at the University of Texas. The model consists of storage units with flow between the units prescribed by relationships which approximate the physical phenomenon. Water entering the system in the form of precipitation is subjected, first of all, to the demands of interception storage. The total volume of interception storage is specified by the parameter EPXM. All available interception storage (SCEP) is filled, and the remainder of the precipitation (P3 in the program) proceeds to the surface. Interception storage is decreased each hour from 9:00 a.m. through 8:00 p.m. by the withdrawal of a volume sufficient to satisfy the hourly evapotranspiration potential (EPHRLI), which is calculated as 1/12 of the daily evapotranspiration potential, EP.

The soil is divided into four storage areas. The Upper Zone Storage area includes the depression storage and a shallow depth of the top soil. No physical limitation is given to this zone; however, it is intended to have small storage volume and a fast reaction time. The Lower Zone Storage area is much larger in volume, and apparently embraces the remainder of the unsaturated soil zone. Groundwater Storage is that storage active in causing base flow, while Deep Percolation is that

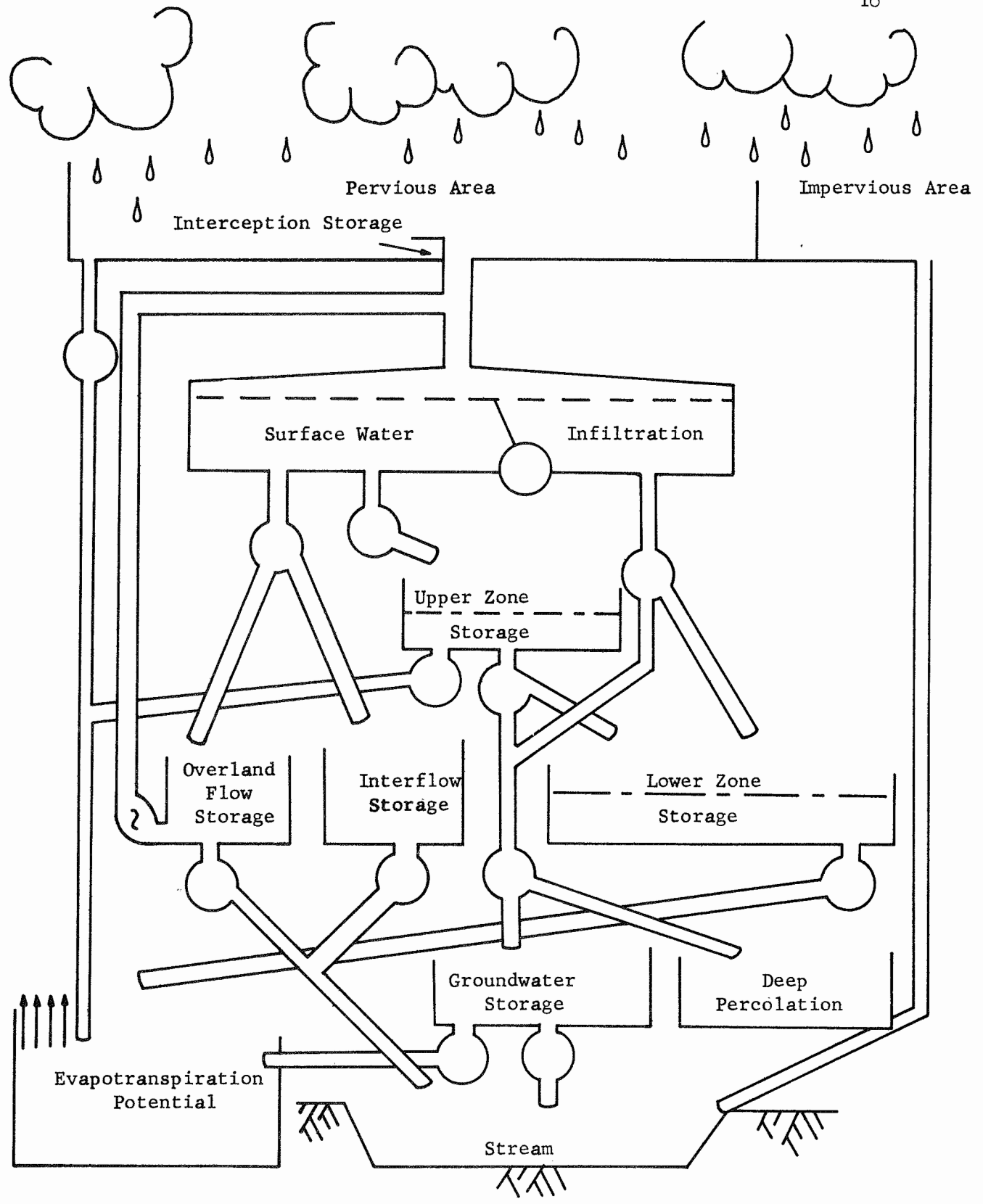


FIGURE 3.1 SCHEMATIC OF STANFORD WATERSHED MODEL IV

storage lost to the system. Both Upper Zone Storage (UZS) and Lower Zone Storage (LZS) have associated nominal storage values, UZSN and LZSN, respectively. Recommended values for LZSN are:

For Seasonal Rainfall. $4 + .25$ (MAR)

For Reasonably Uniform Rainfall. . . $4 + .125$ (MAR)

where MAR is mean annual rainfall. UZSN will vary from 0.06 LZSN for steep slopes to 0.14 LZSN for mild slopes.

In addition to the nominal storage parameters, the model requires the use of an infiltration index, CB, and an interflow index CC. CB is related to the soil type; however, this relationship has not been established. A range of 0.3 to 1.2 is suggested. The interflow index does not seem to have a physical meaning.

Water reaching the surface is divided between direct infiltration and surface phenomena. The portion going to each varies during a storm, and is a function of the amount to be divided and the volume of water in Lower Zone Storage. As shown in Figure 3.2, the lower zone storage rating ($D4F/CB$) is relatively constant when the Lower Zone Storage volume is high, and increases rapidly as the storage becomes less than the nominal. Figure 3.3 shows the manner in which the division between surface water and infiltration is made as a function of the amount of water available and the lower zone storage rating. Thus, early in a storm, Lower Zone Storage will be low, and the Lower Zone Storage rating will be high (Figure 3.2). Whether the intensity of the rainfall is high or low, a relatively large amount of infiltration will occur, with a somewhat smaller amount being associated

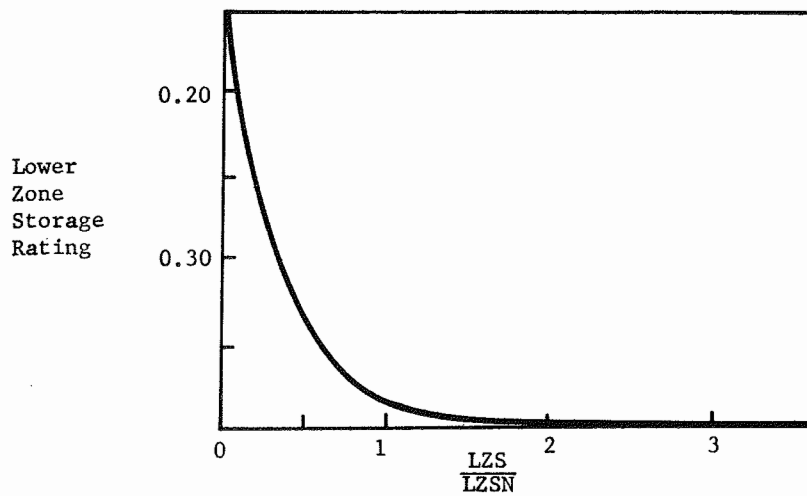


FIGURE 3.2 LOWER ZONE STORAGE RATING vs. LOWER ZONE STORAGE INDEX

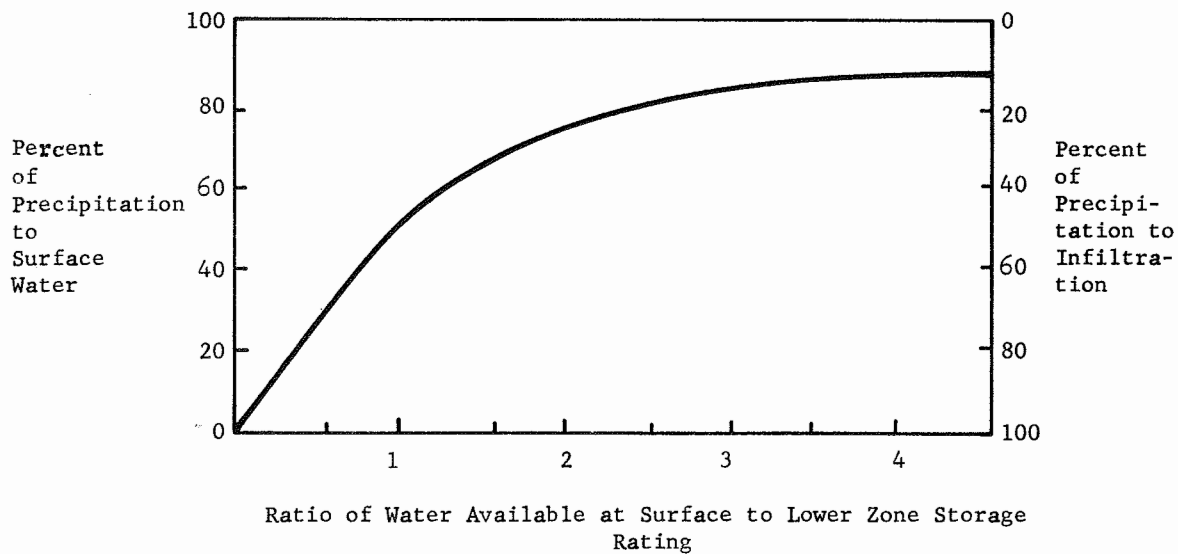


FIGURE 3.3 DIVISION BETWEEN SURFACE WATER AND INFILTRATION

with the larger intensity storm. The curve in Figure 3.2 has the following mathematical representation:

$$\text{Lower Zone Storage Rating} = 2^{-(2 + Z)} \quad (3.1)$$

where $Z = 4 \text{ LZS/LZSN}$ for $0 < \text{LZS/LZSN} < 1$

or $Z = 2(1 + \text{LZS/LZSN})$ for $1 \leq \text{LZS/LZSN} \leq 2$

or $Z = 6$ for $2 \leq \text{LZS/LZSN}$

Z is called LNRATHM in the watershed model, while the ration LZS/LZSN is called LNRAT. The equation for Figure 3.3 is

$$\text{Infiltration Percent} = [1 - (\text{Available Water/Lower Zone Storage Rating}) / (2 \cdot \text{CB})] \cdot 100 \quad (3.3)$$

for Available Water $< \text{CB} \cdot \text{Lower Zone Storage Rating}$ and

$$\text{Infiltration Percent} = [\text{CB} \cdot (\text{Lower Zone Storage Rating/Available Water}) / 2] \cdot 100 \quad (3.4)$$

for Available Water $\geq \text{CB} \cdot \text{Lower Zone Storage Rating}$.

Surface water is next subjected to division between Overland Flow Storage, Interflow Storage, and Upper Zone Storage. The division between the first two and the latter is controlled by the existing Upper Zone Storage, as shown in Figure 3.4.

This curve is represented mathematically by

$$\text{Percent to UZS} = [1 - 1/2 (\text{UZS/UZSN}) \cdot (1/(1 + \text{UZI}))^{\text{UZI}}] 100 \quad (3.5)$$

for $0 \leq \text{UZS/UZSN} \leq 2$, and with

$$\text{UZI} = 2 \left| 1/2 (\text{UZS/UZSN} - 1) \right| + 1 \quad (3.6)$$

$$\text{or by Percent to UZS} = [(1/(1 + \text{UZI}))^{\text{UZI}}] 100 \quad (3.7)$$

$$\text{for } \text{UZS/UZSN} > 2, \text{ and with } \text{UZI} = 2 \left| \text{UZS/UZSN} - 2 \right| + 1. \quad (3.8)$$

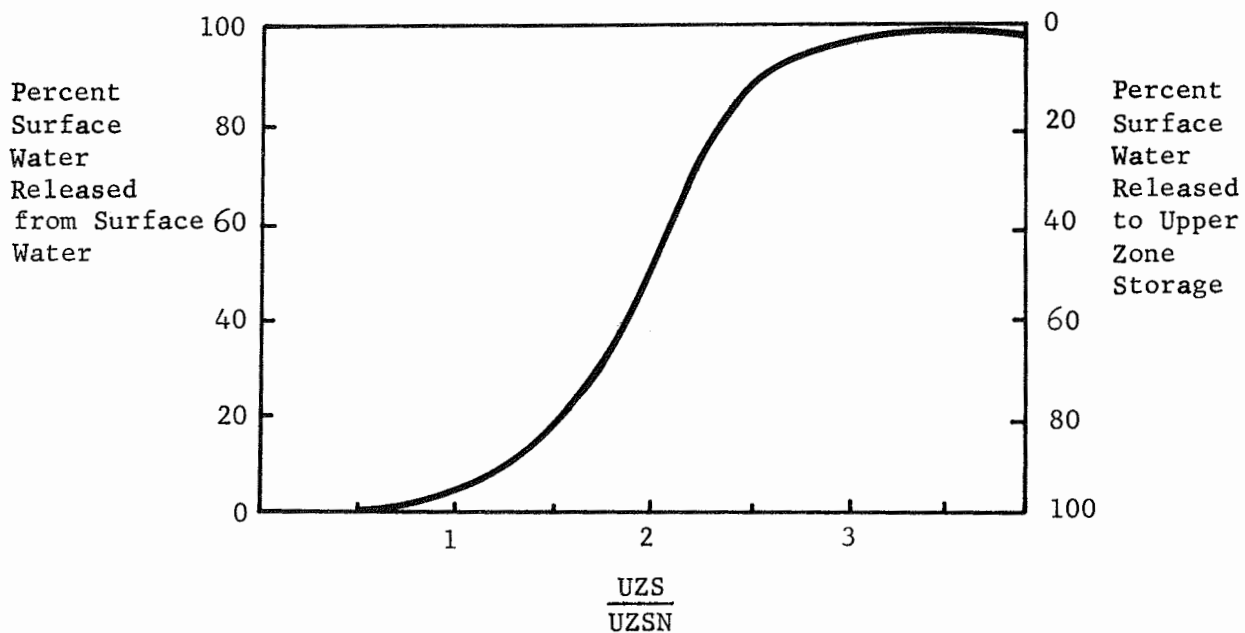


FIGURE 3.4 DIVISION OF WATER BETWEEN UPPER ZONE STORAGE, AND OVERLAND FLOW STORAGE AND INTERFLOW STORAGE

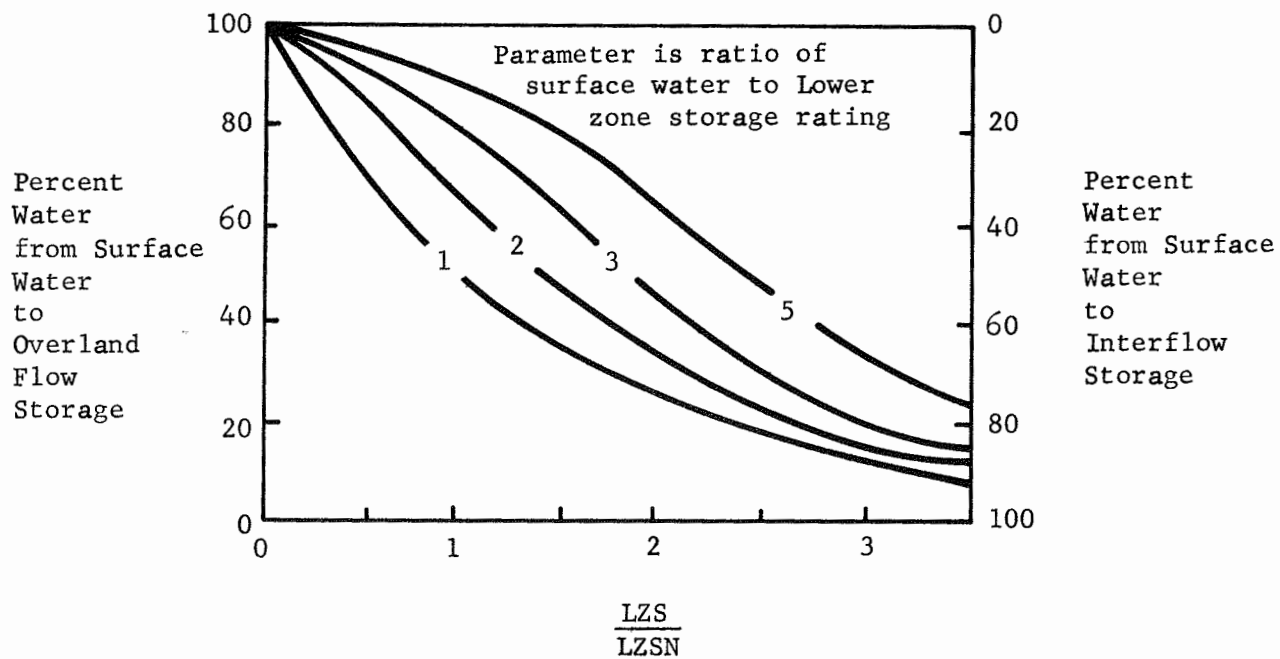


FIGURE 3.5 DIVISION OF WATER BETWEEN OVERLAND FLOW STORAGE AND INTERFLOW STORAGE

The water designated for Overland Flow Storage and for Interflow Storage is divided as shown in Figure 3.5 where both CB and CC equal 1. Decreasing CB causes the curves to drop more rapidly, while decreasing CC has the opposite effect.

Water which infiltrates is also divided three ways. An amount dependent on the volume in Lower Zone Storage is diverted to Lower Zone Storage. This relation is shown in Figure 3.6, where the curve has the mathematical form:

$$\text{Percent to LZS} = \left[\frac{1}{1 + \text{LZI}} \right]^{\text{LZI}} 100 \quad (3.9)$$

for $\text{LZS}/\text{LZSN} \geq 1$, or,

$$\text{Percent to LZS} = \left[1 - \frac{1}{1 + \text{LZI}} \right]^{\text{LZI}} (\text{LZS}/\text{LZSN}) 100 \quad (3.10)$$

for $\text{LZS}/\text{LZSN} < 1$,

$$\text{where, in either case, } \text{LZI} = 1.5 \left| \text{LZS}/\text{LZSN} - 1 \right| + 1. \quad (3.11)$$

A portion of the water diverted to Overland Flow Storage is released as surface runoff to the stream. The amount of overland flow is based on the empirical relation

$$y = D(1 + 0.6(D/D_e)^3)/L \quad (3.12)$$

for the rising side of the overland flow hydrograph, and

$$y = 1.6 D/L \quad (3.13)$$

for the recession side, where

y is the depth of flow at the outfall, in ft.,

D is the volume of storage in ft.³/ft.,

L is the length of overland flow, and

D_e is the volume of storage at the equilibrium condition corresponding to the current inflow rate η .

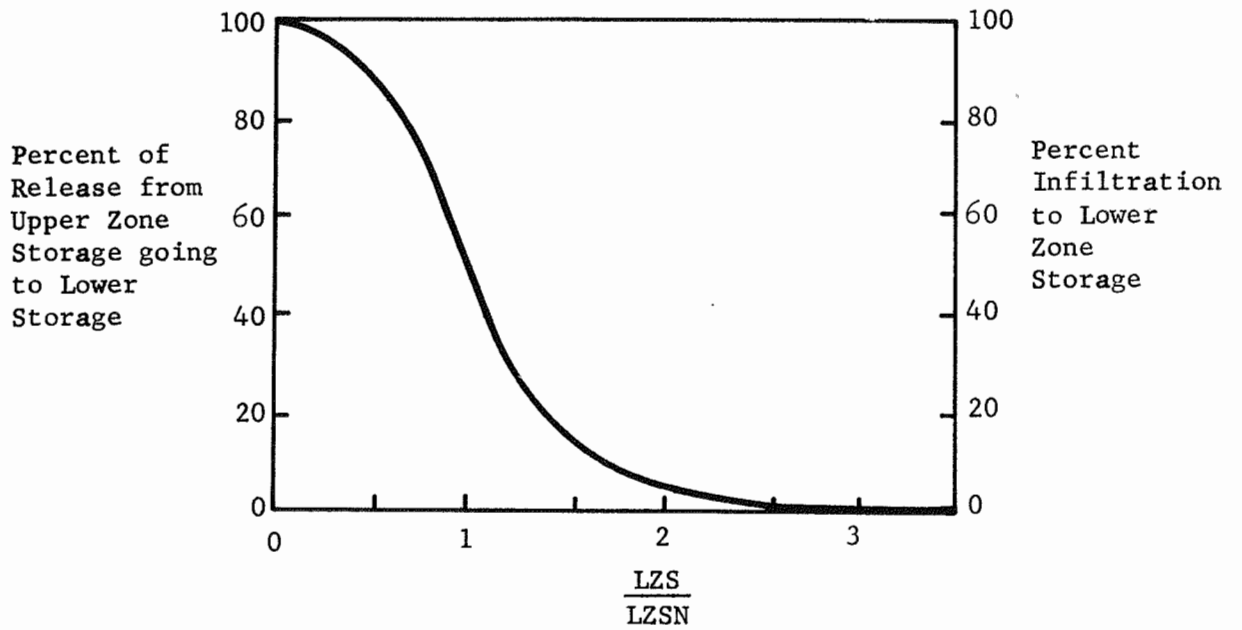


FIGURE 3.6 FLOW INTO LOWER ZONE STORAGE FROM UPPER ZONE STORAGE AND FROM INFILTRATION

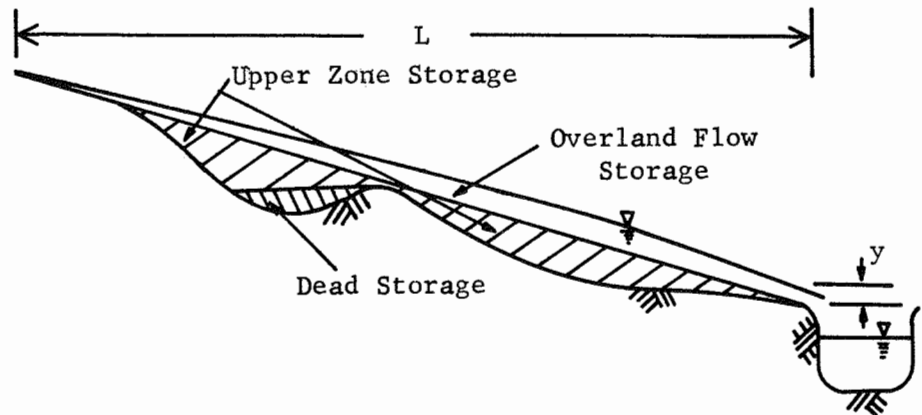


FIGURE 3.7 THE ROLE OF DEAD SURFACE STORAGE IN OVERLAND FLOW

For two dimensional flow, Manning's equation may be written as

$$q = 1.486/n \cdot y^{5/3} \cdot S^{1/2} \quad (3.14)$$

where

q is the flow in cfs per foot of width,

n is Manning's roughness factor, and

S is the slope of the energy gradeline, assumed equal to the bed slope.

When an equilibrium condition is obtained on the slope,

$$q = \eta L \quad (3.15)$$

and

$$q(x) = \eta x. \quad (3.16)$$

The volume of water in Overland Flow Storage at Equilibrium may be found by integrating the expression

$$D_e = \int_0^L y \, dx. \quad (3.17)$$

Solving Equation (3.13) for y , with the value of $q(x)$ from (3.15) used, this reduces to

$$\begin{aligned} D_e &= (n\eta / (1.486 S^{1/2}))^{3/5} \int_0^L x^{3/5} \, dx \\ &= (n\eta / (1.486 S^{1/2}))^{3/5} \cdot 5/8 L^{8/5}. \end{aligned} \quad (3.18)$$

With the current inflow rate known, this equation can be solved for D_e , which may be substituted into (3.12) to yield a value of the depth of flow, y , at the outfall. This value of y may then be used in (3.13) to calculate the runoff rate, q .

There is some problem in the model concerning what to use for the inflow rate, η . It is not the total rainfall, since a portion of this is removed by interception, and another portion by infiltration. Still

more is used to replenish Upper Zone Storage, and a part is allocated to Interflow Storage. In the watershed, infiltration occurs from Overland Flow Storage, and the "dead" Upper Zone Storage. Consequently, the "dead" Upper Zone Storage must be replenished from overland flow. This difference is illustrated in Figure 3.7, where a part of Upper Zone Storage is shown as "dead" surface storage. No physical counterpart has been discovered for that portion assigned to Interflow Storage. From Figure 3.1, it may be noted that the water subject to the division between Surface Water and Infiltration consists of the precipitation for the quarter hour period, less that trapped by interception and increased by the amount in Overland Flow Storage at the beginning of the time period (RES). Assuming that none of this volume is assigned to Infiltration, Upper Zone Storage, or to Interflow Storage, the portion of the precipitation occurring during this period which reaches Overland Flow Storage will be

$$\eta = RX - RES \quad (3.19)$$

where RX is the volume assigned to Overland Flow Storage at the end of the time period.

The model does not keep the initial volume in Overland Flow Storage separate; however, the foregoing reasoning seems to make plausible the use of (3.19) for defining η so long as $RX > RES$, i.e., the rising side of the overland flow hydrograph.

Water is released from Upper Zone Storage to the lower zones (Lower Zone Storage, Groundwater Storage, and Deep Percolation) only when the Upper Zone Storage is relatively fuller than the Lower Zone

Storage, i.e., when

$$(UZS/UZSN)/(LZS/LZSN) > 1 . \quad (3.20)$$

These releases are made on an hourly basis. The percent of Upper Zone Storage releases as a function of the two ratios UZS/UZSN and LZS/LZSN is shown in Figure 3.8. Mathematically, the relation is

$$\text{Percent UZS released} = \left[0.003 \cdot CB(UZS/UZSN)^{-1} (UZS/UZSN - LZS/LZSN)^3 \right] 100 \quad (3.21)$$

The water thus released from the Upper Zone Storage is divided between Lower Zone Storage and lower zones (Groundwater Storage and Deep Percolation) as shown in Figure 3.6.

The volume of water (INTF) released each fifteen minutes from Interflow Storage to become a part of the streamflow is determined by

$$\text{INTF} = \text{LIRC4} \cdot \text{SRGX} \quad (3.22)$$

where LIRC4 is a quarter hourly interflow recession constant

calculated from a daily interflow recession parameter (IRC).

In a similar manner, the contribution from groundwater (GWF) is calculated from

$$\text{GWF} = \text{LKK4} \cdot (1 + \text{KV} \cdot \text{GWS}) \cdot \text{SGW} \quad (3.23)$$

in which

LKK4 is a quarter hourly groundwater recession constant calculated

from a daily groundwater recession parameter (KK24),

KV is a parameter for the variable component of groundwater recession,

GWS is a groundwater slope index, and

SGW is the current groundwater storage.

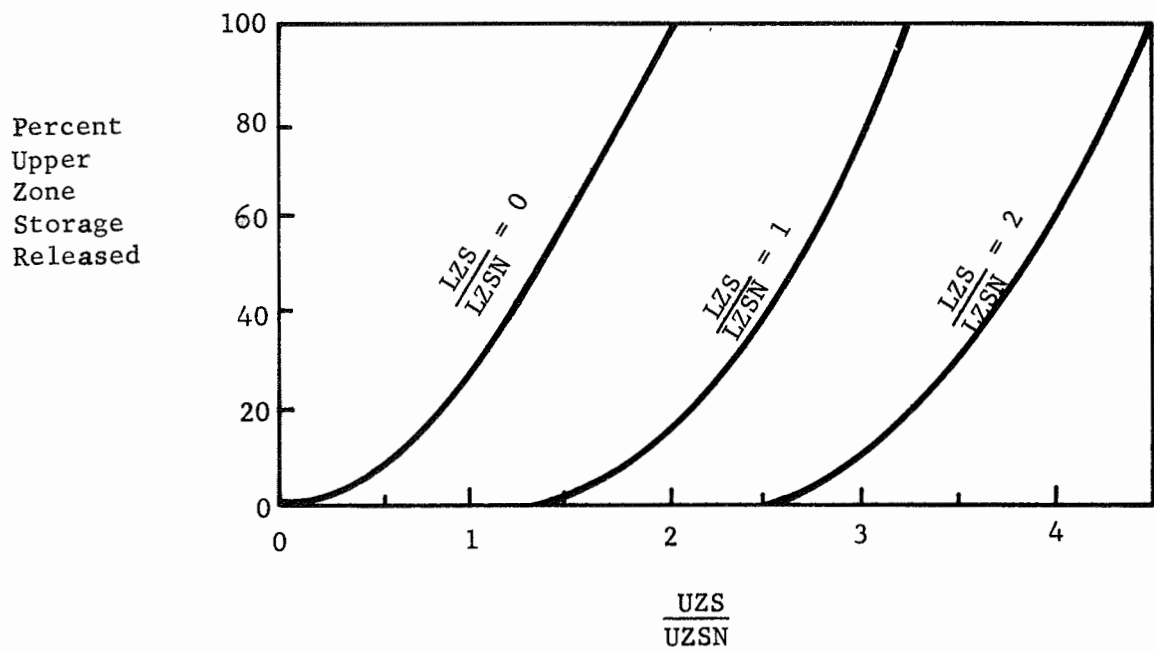


FIGURE 3.8 UPPER ZONE STORAGE RELEASES TO LOWER ZONE STORAGE

As mentioned earlier, water is removed hourly (9:00 a.m. through 8:00 p.m.) from Interception Storage to satisfy the hourly evapotranspiration potential. When the volume available from Interception Storage is less than the hourly evapotranspiration potential, the balance of the potential is removed from Upper Zone Storage. If the hourly evapotranspiration potential is not yet satisfied, the balance is accumulated as the variable REP. At 9:00 p.m. an amount LOS is removed from Groundwater Storage determined by

$$\text{LOS} = \text{REP} \cdot \text{PA} \cdot \text{SGW} \cdot \text{K24EL} \quad (3.24)$$

where PA is the fraction of the total area which is pervious; hence,

REP·PA is the volume of the day's evapotranspiration potential which is unsatisfied,

and K24EL is the fraction of the total area from which evapotranspiration takes place directly, i.e., that part of the watershed where the root zone extends into the groundwater table.

Also at 9:00 p.m., water is removed from Lower Zone Storage according to whether the accumulated unsatisfied evapotranspiration (REP) is greater than or less than $K3 \cdot \text{LZS}/\text{LZSN}$. K3 is an input parameter determining the value of LZS/LZSN at which the evapotranspiration from Lower Zone Storage becomes limited by the unsatisfied evapotranspiration potential. The relation is shown in Figure 3.9, and has the mathematical description

$$\text{AETR} = \text{REP} \left(1 - 1/2 \cdot \text{REP} (\text{K3} \cdot \text{LZS}/\text{LZSN})^{-1} \right) \quad (3.25)$$

for $\text{REP} < \text{K3} \cdot \text{LZS}/\text{LZSN}$, and

$$\text{AETR} = 1/2 \cdot \text{K3} \cdot \text{LZS}/\text{LZSN} \quad (3.26)$$

for $REP > K3 \cdot LZS / LZSN$, where

AETR is the volume of water removed from Lower Zone Storage in order to satisfy the daily evapotranspiration potential.

The computed runoff is based on a unit area whose composition is the same as that of the total segment, i.e., the same fraction of impervious area. The total runoff (R) is obtained by multiplying by

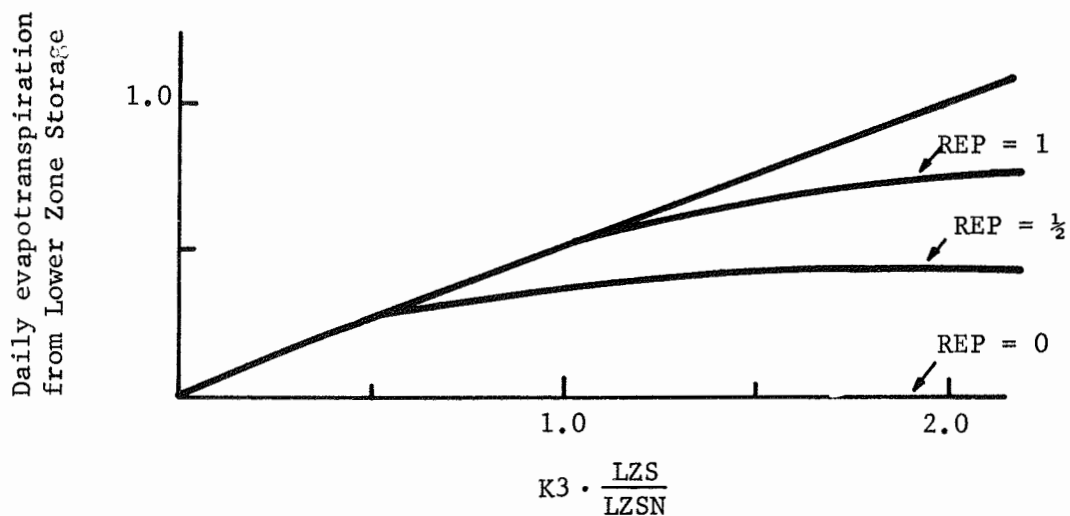
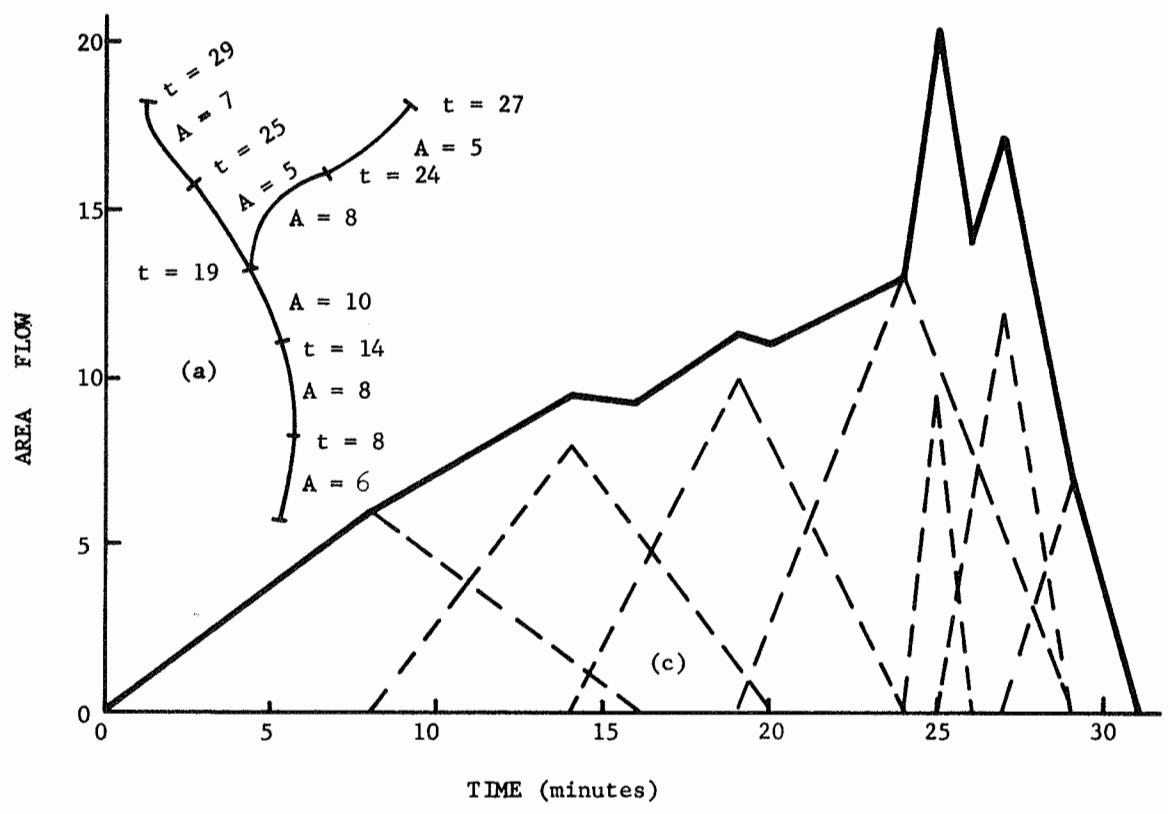
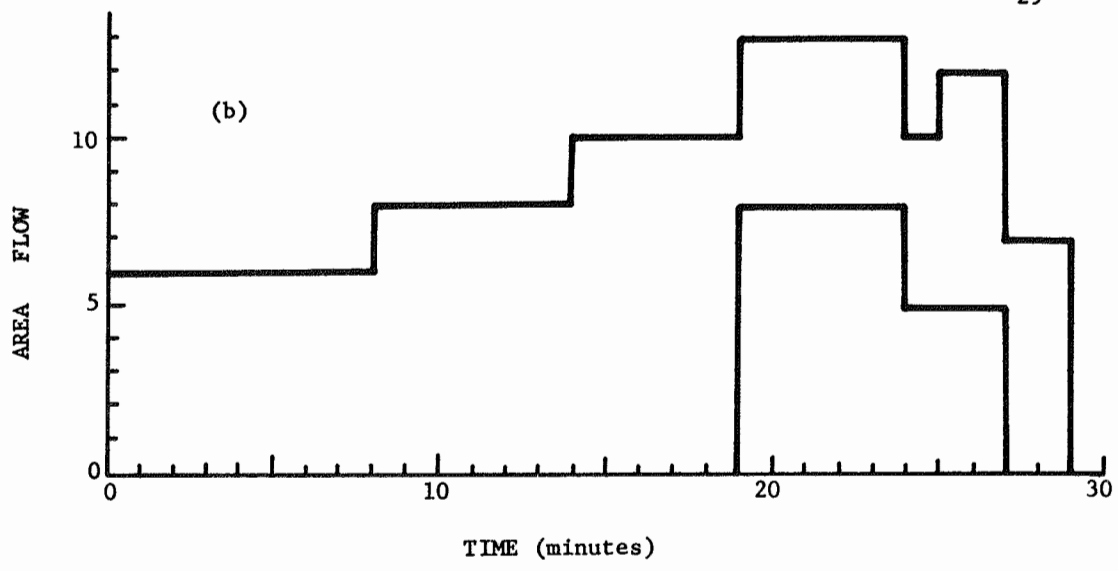


FIGURE 3.9 EVAPORATION FROM LOWER ZONE STORAGE

the total area. The time distribution of the runoff is accomplished by distributing the water forward in time according to a distribution graph determined by three of the input parameters shown in Table 3.1. The method of deriving these values is similar to that used for determining an instantaneous unit hydrograph. The channels within the segment are divided into reaches, Figure 3.10. The time of travel through each reach is estimated, and cumulative times to each point



Time	0-4	4-8	8-12	12-16	16-20	20-24	24-28	28-32
Area	3	8.6	13.8	18.3	19.6	24.0	32.2	10.2
Percent	2.3	6.6	10.7	14.1	15.1	18.5	24.8	7.9

FIGURE 3.10 DERIVATION OF THE ROUTING PARAMETERS

noted, i.e., the time of travel from the point to the outlet. Finally a contributing area is assigned each reach. A histogram is determined by plotting the contributory area against the travel time, as shown in Figure 3.10, (b). The histogram is the response of the watershed to an instantaneous rainfall event. To account for finite duration of rainfall, the response from each reach is assumed to vary in a linear manner from zero at the beginning of response (the travel time for the downstream end of the reach) to a maximum at the time when all of the reach is contributing (the travel time for the upstream end of the reach) and decreases linearly to zero in an equal amount of time. Figure 3.10 (c) shows each of the rectangles of Figure 3.10 (b) replaced by an isosceles triangle. These triangles are then combined to give the heavy line shown. A suitable time increment is chosen--in this case, four minutes, and the channel delay histogram determined as shown in the table in Figure 3.10.

Channel storage and attenuation is accomplished after the outflow hydrograph has been computed. An hourly "stream channel storage recession parameter" is used for this purpose.

The user of the model is required to furnish numerical values for the 16 parameters shown in Table 3.1, in addition to the initial conditions in the watershed. Part of these parameters are, at present, poorly defined. It is usually necessary to make several trials, using different values of the parameters, in order to determine the best set of parameter values. The model computes two sets of statistical data, based on the simulated flow and the actual flow. The first set of data consists of a frequency table based on the recorded flows. The

TABLE 3.1
HYDROLOGICAL PARAMETERS REQUIRED BY STANFORD WATERSHED MODEL IV

Variable	Explanation
CB	Infiltration Index
CC	Interflow Index
EPXM	Maximum Value of Interception Storage
ETL	Evaporation from Stream Surfaces
IRC	Interflow Runoff Recession
KK24	Basic Groundwater Recession Rate
KS1	Stream Channel Storage Recession Parameter
KV	Variable Component of Groundwater Recession
K24EL	Evapotranspiration from Groundwater
K24L	Portion of Groundwater Recharge Assigned to Deep Percolation
K3	Actual Evaporation Loss Index
L	Overland Flow Length
LZSN	Nominal Lower Zone Storage
NN	Manning's n for Overland Flow
SS	Overland Flow Slope
UZSN	Nominal Upper Zone Storage

difference between the simulated mean daily flow and the recorded mean daily flow is called the error (ERR) and for each frequency class the standard deviation of the error is found from

$$SQ_j = \frac{\sum_{i=1}^{n_j} ERR_j^2 - (\sum_{L=1}^{n_j} ERR_i)^2/n_j}{n_j - 1}^{1/2} \quad (3.31)$$

where SQ_j is the standard deviation for the j^{th} class and n_j is the number of events in the j^{th} class.

The model also computes the daily correlation coefficient between the simulated and recorded mean daily flows.

$$CORCO = \frac{\sum_{i=1}^N (FLO_i - \overline{FLO}) (DR_i - \overline{DR})}{\left(\sum_{i=1}^N (FLO_i - \overline{FLO})^2 \right)^{1/2} \left(\sum_{i=1}^N (DR_i - \overline{DR})^2 \right)^{1/2}} \quad (3.32)$$

where

CORCO is the daily correlation coefficient

FLO_i is the recorded mean daily flow on the i^{th} day

\overline{FLO} is the average recorded mean daily flow

DR_i is the simulated mean daily flow on the i^{th} day

\overline{DR} is the average simulated mean daily flow

N is the number of days in the year

The daily correlation coefficient gives a good overall measure of the performance of the model. However, in areas where the number of runoff events is few, the coefficient may be deceptively high, since it is strongly influenced by a large number of near zero events. In all cases, the standard deviation by classes indicates the degree to which the model

is capable of reproducing the recorded flows in the different ranges of flow.

Precipitation input is divided into two groups; that data from recording gauges (Pl), and that from non-recording gauges (PREC). Only one recording gauge may be used for each segment. The time distribution of the recording gauge is applied to the non-recording gauges to approximate the time distribution of the storm over all the watershed. Each gauge is assigned a weighting factor on the basis of the fraction of the area it is assumed to represent. These factors may be determined by a Thiessen network technique, or in any other manner desired by the user. The sum of the weights must equal one.

A single figure is calculated from the recording gauge record and the non-recording gauge records, which is the weighted precipitation for the segment. This data is stored on magnetic tape, and it is not necessary to repeat the calculation for subsequent runs.

The normal run uses hourly precipitation data from the recording gauge. However, an option is provided (DCS(11)) so that the data may be on a 15 minute basis. To exercise this option, four fifteen minute rates are written in the form

WWXXXYZZZ

where WW is the first quarter hour rate in hundredths of an inch with the decimal omitted, XXX represents the second quarter hour, etc. The program will unpack this data, calculate a 15 minute rainfall rate for the basin, using the non-recording gauges, and re-pack the data before storing on tape.

Chapter 4

UNSATURATED FLOW

When water falls to the ground it becomes subject to the processes of evaporation, infiltration, and runoff, as discussed in Chapter 1. The two processes, surface runoff and infiltration, are interrelated, each being parts of a continuity equation which could be written at the surface. In this chapter, surface runoff will be ignored, and a mathematical model formulated to describe the distribution of water in one dimensional space and time within a column of soil.

Physics of Unsaturated Flow

Water exists as solid, liquid, and gas within the soil. While movement is limited to the latter two states, the presence of ice has a profound effect on the movement of water vapor. The actual mechanics of the flow are discussed at length by Remson (1962). Only the necessary salient features of that discussion will be summarized here.

Motion occurs because of unbalanced forces, and hence, it seems natural to examine the forces acting on water within the soil. These forces are: (1) gravitational force in the vertical direction, and the following forces which may act in any direction, (2) so-called capillarity forces, (3) chemical and osmotic forces, and (4) forces due to partial pressure gradients acting on the water vapor. Chemical and osmotic forces cause relatively slow motion in most soils and will not be included. The volume of water moved in the vapor state is small in

comparison to that moved in the liquid state so long as the liquid state is continuous.

Several investigators, beginning with Richards (1931) have shown that Darcy's equation for flow in a saturated porous media,

$$V = - K \frac{\partial h}{\partial y}, \quad (4.1)$$

where V is the bulk velocity in the y direction, (i.e., the flow rate through a unit area, ignoring the area of the soil particles),

K is the permeability of the soil in L/T units,

h is the hydraulic head, usually consisting of a pressure term and a gravitational term, i.e., $p/\gamma + y$,

y is the direction of the flow,

may be used in unsaturated flow. Indeed, Remson (1962) suggests using the equation for flow in both the vapor and liquid state. When h is viewed as the total potential causing motion, Remson's suggestion seems appropriate.

In unsaturated flow, the pressure term is negative due to the "capillary forces"--actually surface tension effects. It is customary to speak of soil moisture tension rather than pressure, and frequently the negative sign is omitted. Considering only the flow in the liquid state, the total head, h , may be defined as

$$h = \psi + y \quad (4.2)$$

where ψ is the soil moisture tension potential, or capillary potential with units of length, and

y is the vertical distance from an arbitrary reference plane, positive upward.

The capillary potential is negative by this definition. .

Equation for Unsaturated Flow

Figure 4.1 shows a parallelepiped of the soil matrix. Flow is assumed to occur through only the top and bottom surfaces, which have unit area. Applying the continuity principle to the element

$$\begin{aligned} \rho V - \rho \left(V + \frac{\partial V}{\partial y} \cdot \delta y \right) &= \frac{\partial(\rho S \delta y)}{\partial t} \\ - \frac{\partial V}{\partial y} \delta y &= \delta y \frac{\partial S}{\partial t} \\ - \frac{\partial V}{\partial y} &= \frac{\partial S}{\partial t} \end{aligned} \quad (4.3)$$

where S is the volume of soilwater per unit soil volume. Substituting 4.2 into 4.1 yields the unsaturated flow form of Darcy's equation

$$V = - K \frac{\partial(\psi + y)}{\partial y} . \quad (4.4)$$

When this expression of velocity is used in the continuity equation, there results the basic partial differential equation for flow through partially saturated porous media due to capillary and gravitation potentials

$$\frac{\partial S}{\partial t} = \frac{\partial}{\partial y} \left[K \frac{\partial(\psi + y)}{\partial y} \right]$$

or

$$\frac{\partial S}{\partial t} = \frac{\partial}{\partial y} \left[K \left(\frac{\partial \psi}{\partial y} + 1 \right) \right] \quad (4.5)$$

The soil moisture, S , the permeability, K , and the capillary potential are related, and the relationship is a soil property which does not change as long as the geometry of the soil matrix does not change.

Typical curves of these relations are shown in Figure 4.2. Unfortunately,

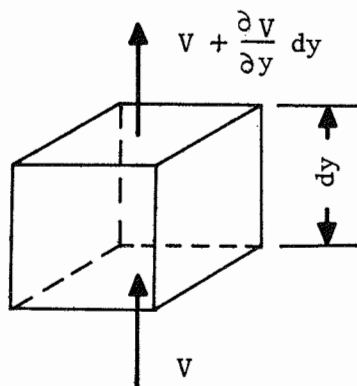


FIGURE 4.1 CONTINUITY FOR UNSATURATED ONE-DIMENSIONAL FLOW

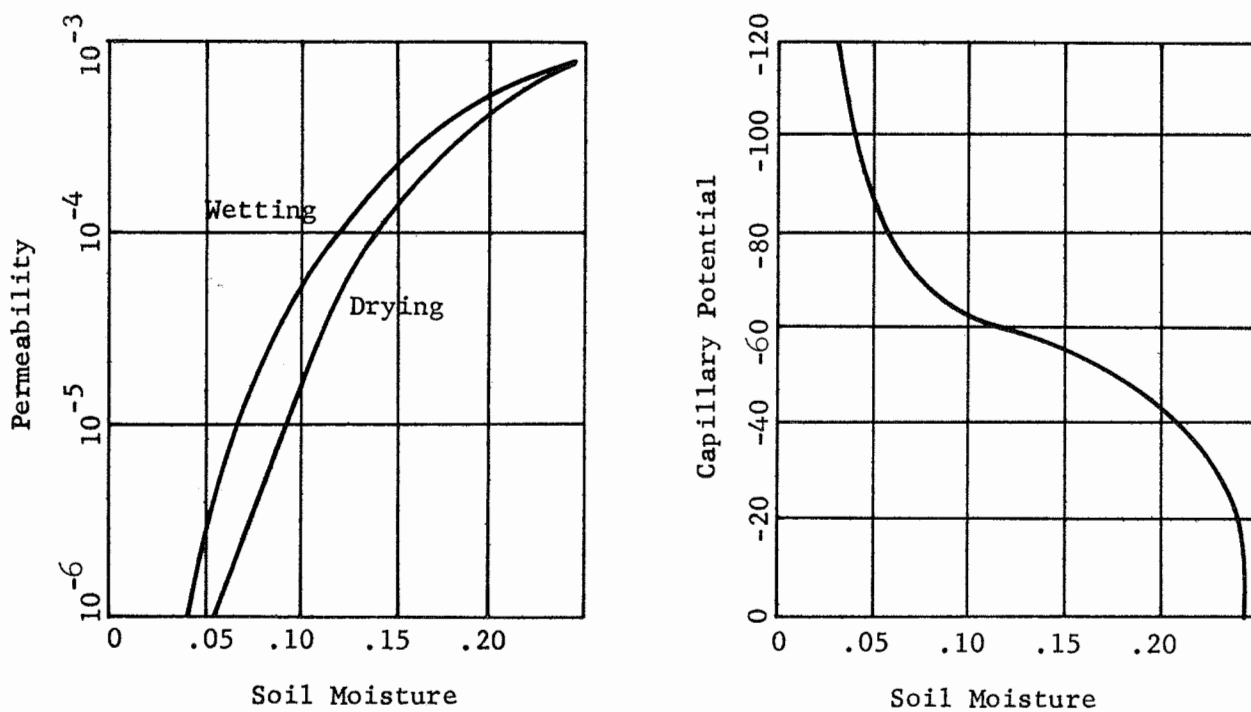


FIGURE 4.2 TYPICAL RELATION BETWEEN SOIL MOISTURE, PERMEABILITY AND CAPILLARY POTENTIAL

there is a hysteresis effect in both the soil moisture-permeability relation and the soil moisture-capillary potential relation. Recently, the suggestion of Poulouvasilis (1962) was implemented by Ibrahim and Brutsert (1968). Poulouvasilis had shown a method he called the "independent domain concept" to be applicable to the hysteresis problem. Essentially, this method reduces both the soil moisture-permeability and the soil moisture-capillary potential functions to single valued functions. Consequently, the analysis will be based on the assumption of a single valued, laboratory determined relationship for both permeability and capillary potential as a function of soil moisture.

If these two relationships could be expressed in a mathematical form, the next step would be to choose one variable as the dependent variable and express the other two variables as a function of this variable. Rubin (1966) has recognized solutions using both the soil moisture and the capillary potential as the dependent variable. He has suggested the use of a combination of these properties. While any of the three variables could be chosen, there is an advantage in choosing the capillary potential as the basic variable. The advantage consists of being able to express both permeability and soil moisture as uniquely defined functions of capillary potential. There are certain soils for which the soil moisture and the permeability do not change over some range of values of the capillary potential function. See Figure 4.3. Such soils have an air entry value different from zero, i.e., these soils will sustain some tension before the soil moisture is decreased below saturation.

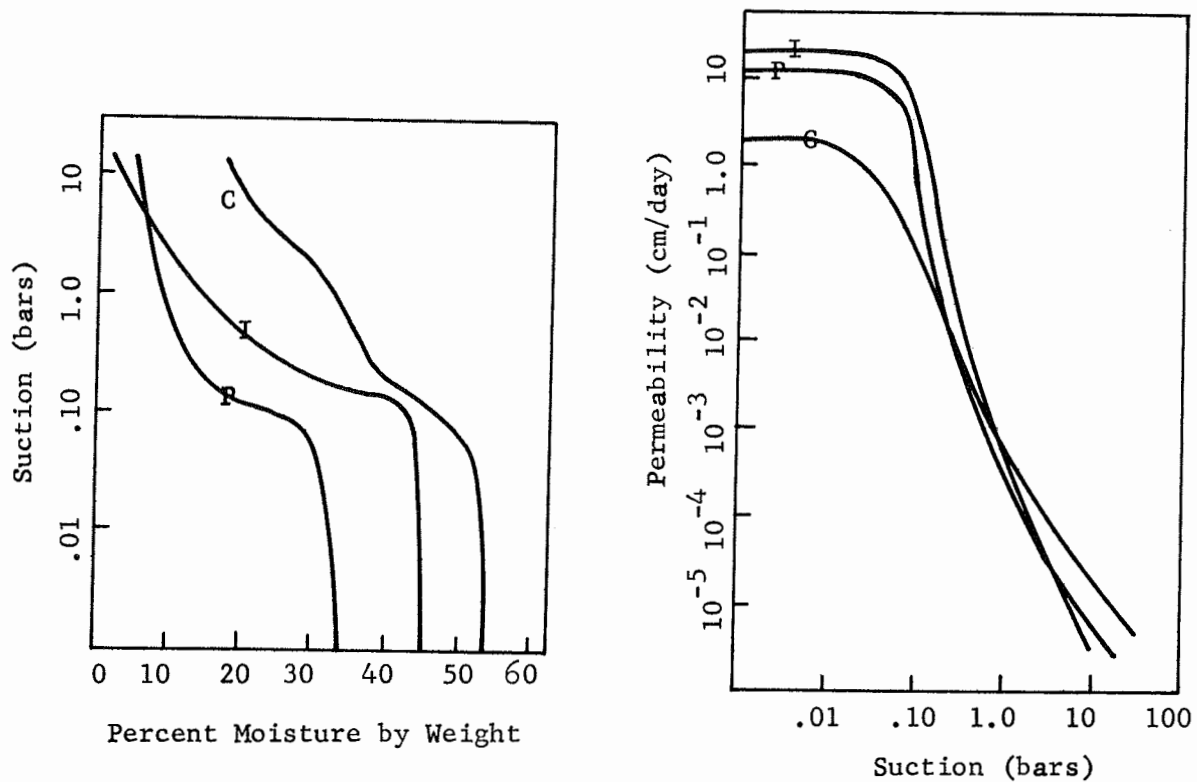


FIGURE 4.3 VARIATION OF PERMEABILITY AND WATER CONTENT WITH CAPILLARY POTENTIAL FOR

- I Indio Loam
- P Pachappa Sandy Loam
- C Chino Clay

(After Gardner (1958))

Then

$$K = K(\psi), \text{ and}$$

$$S = S(\psi).$$

Using the chain rule, 4.5 may be rewritten

$$\frac{\partial S}{\partial \psi} \frac{\partial \psi}{\partial t} = \frac{\partial}{\partial y} \left[K \left(\frac{\partial \psi}{\partial y} + 1 \right) \right] \quad (4.6)$$

$$\frac{\partial \psi}{\partial t} = \frac{d\psi}{dS} \left[\frac{\partial K}{\partial y} \left(\frac{\partial \psi}{\partial y} + 1 \right) + K \frac{\partial^2 \psi}{\partial y^2} \right] \quad (4.7)$$

$$\frac{\partial \psi}{\partial t} = \left[\frac{d\psi}{dS} \frac{dK}{d\psi} \frac{\partial \psi}{\partial y} \left(\frac{\partial \psi}{\partial y} + 1 \right) + K \frac{\partial^2 \psi}{\partial y^2} \right] \quad (4.8)$$

Equation 4.8 is a quasi-linear second order parabolic partial differential equation. Boundary conditions are of the form

$$\text{for } t = 0, \psi(y, 0) = \psi(y)$$

$$\text{for } t > 0, \psi(0, t) = G_1(t)$$

$$\psi(Y, t) = G_2(t)$$

where Y is the thickness of the soil column. Phillip (1957) has obtained a series solution for the special case of $\psi(y)$, $G_1(t)$, and $G_2(t)$ all constant. Other investigators have resorted to a finite difference representation of the partial differential equation.

In general these investigators have been interested in some special aspect of the unsaturated flow phenomena. Gardner and Fireman (1958) and Liakopoulos (1966) investigated evaporation from the soil surface. Gardner and Fireman included a saturated zone (a water table). Gardner (1959), Nielsen, et al. (1961) and Biswas, et al. (1966) all considered the redistribution of water within the soil column. Youngs (1957) considered the soil water profile with the surface maintained at saturation. Rubin (1966) studied infiltration into initially air dry Rehovot sand. He

recognized three types of infiltration:

- (a) Infiltration controlled by the rate of rainfall; occurs only when rainfall rate is less than saturated permeability of soil.
- (b) Preponding which occurs immediately when rainfall rate exceeds saturated permeability and lasts until ponding occurs.
- (c) Surface ponding when a saturated zone occurs at the soil surface.

For a given soil, the time to incipient ponding (i.e., the time of preponding) decreases as the rainfall intensity increases and increases with increasing initial dryness of the soil. Figure 4.4 shows the infiltration rate as a function of time for a hypothetical soil.

The finite difference solutions have used various representations of the derivatives. A common approximation of the first derivative

$$\frac{\partial \psi}{\partial y} \approx \frac{\psi_2 - \psi_1}{\Delta y} \quad (4.9)$$

is sometimes used. Here Δy is the distance between the points 1 and 2. The approximation possesses various degrees of error depending on the position of the point within the interval where the derivative is to be evaluated. Thus, if the derivative is to be evaluated at the midpoint of the interval, the error has order $(\Delta y)^2$. However, use of 4.9 to evaluate the derivative at either end of the interval may introduce errors of the order of magnitude of Δy . Since in finite difference schemes, the Δy value is normally less than unity, evaluation at the midpoint is usually more accurate. The difficulty of using 4.4 in finite difference form to evaluate the infiltration rate may now be examined.

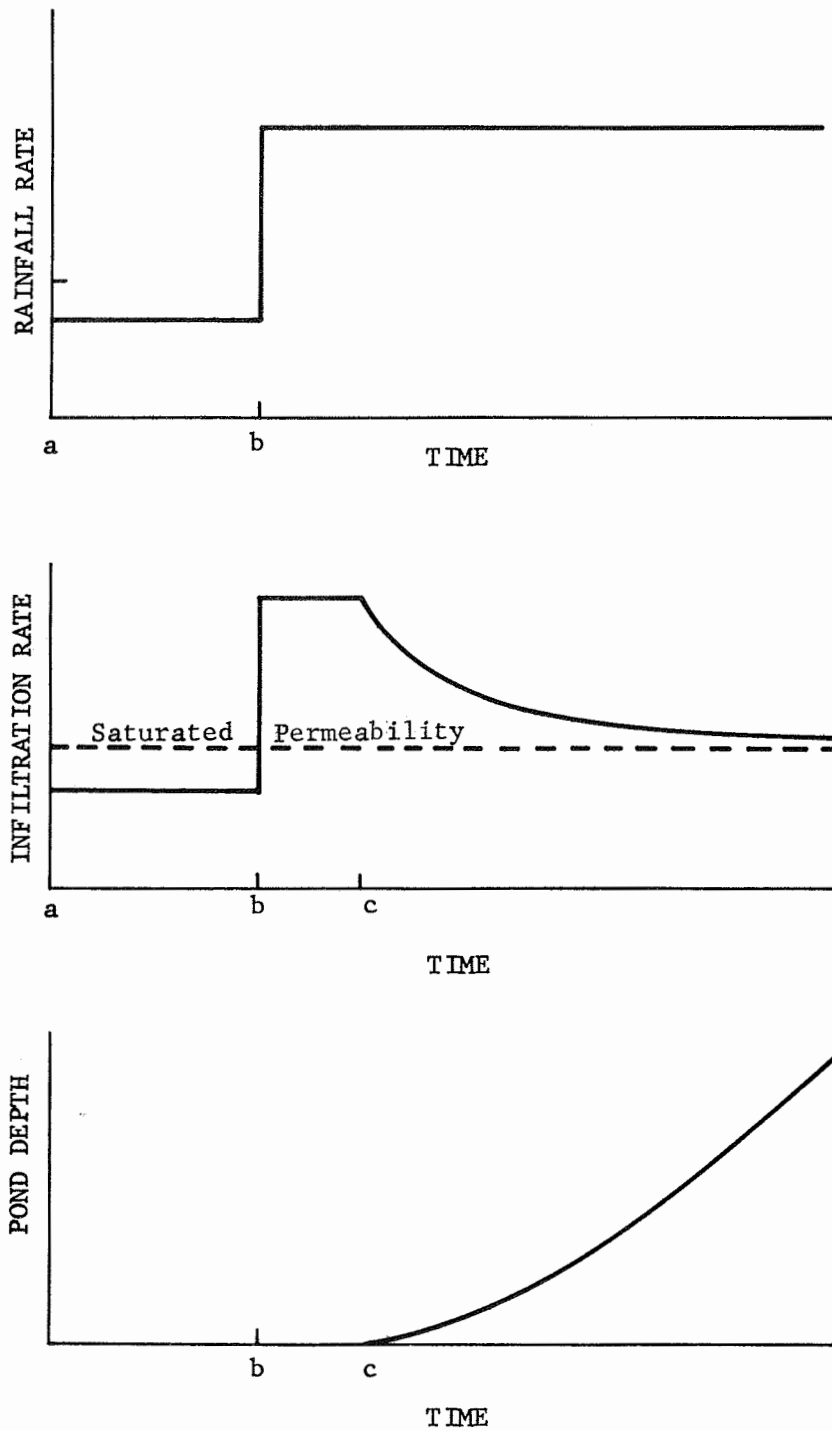


FIGURE 4.4 INFILTRATION RATES FOR THE THREE RAINFALL CONDITIONS

- a - b Supply control
- b - c Incipient ponding
- c - Ponding

Substituting 4.9 into 4.4 with the evaluation occurring at the surface.

$$[V]_{y=0} = -[K]_{y=0} \left(\frac{\psi_1 - \psi_0}{\Delta y} + 1 \right) \quad (4.10)$$

where the subscripts 0 and 1 on the ψ term indicate the value of ψ at the surface and at a distance Δy below the surface respectively. When 4.10 is used to describe the infiltration rate, the error is of the order Δy . Since Δy is necessarily finite, it is impossible to predict precisely the infiltration rate from finite difference analysis.

However, the use of a very small value for Δy gives a good approximation. More serious objections to the use of 4.4 to describe infiltration rates are based on the inadequacy of that equation itself. In particular, two phenomena which may strongly influence the infiltration rate are the rainfall impact effect and the flow of air out of the soil as it is being replaced by the water. The effect of the displaced air is probably very small except for extreme rainfall rates. Extreme rainfall rates may produce almost instantaneous flooding of the surface, trapping large quantities of air within the soil horizon. The effects of this air on the infiltration rate has not been studied.

The energy of the raindrop at impact causes some penetration of the water into the soil matrix, a phenomenon which might be termed "forced infiltration." This phenomenon has been observed, primarily by those interested in the erosion effects of such impacts.

The inclusion of either of these phenomenon in a mathematical model may be extremely difficult until more theoretical work has been accomplished. The movement of air and water simultaneously within the soil matrix will involve the concepts of two phase flow systems. Some

restructuring of the soil matrix occurs due to the impact of the raindrop; the problem of describing this mathematically seems insurmountable.

Movement of water beneath the soil surface may be described by 4.10

$$[V]_{y=\alpha} = - [K]_{y=\alpha} \left(\frac{\psi_i - \psi_{i-1}}{\Delta y} + 1 \right) \quad (4.11)$$

where the point y equal α is between the nodes i and $i-1$. The velocity V may be viewed as the velocity of the water leaving the area of node $i-1$ and entering the area of node i . This equation will be used in Chapter 5 to describe the movement of water after infiltration has occurred.

Related Investigations

Some time was spent during this investigation studying the behavior of a finite difference model of a soil column. The model was obtained by substituting 4.11 in the continuity equation, 4.3. The resulting equation related the values of the capillary potential at three node points at one time step to the values at the same three nodes at the previous time step. The resulting system of nonlinear algebraic equations were solved by an iterative technique which assured the use of the proper values of permeability and soil moisture when the solution was reached. Boundary conditions at the surface consisted of a period of no flow at the surface, a period of rainfall, followed by a period of evaporation. An impervious layer was assumed to exist at the bottom, with the water removed from any saturated zone formed at the bottom at a rate proportional to the thickness of the saturated zone. As may be seen from 4.8, and as noted by Rubin (1966), the finite difference equations are inadequate for saturated flow because of the presence of the $d\psi/dS$ term

which is undefined in the saturated zone. Consequently the location of the interface between a saturated zone and an unsaturated zone must be obtained as part of the solution, with the proper form of Darcy's law applied in each zone. Finite difference solutions provide values of the dependent variable only at the node points. When the interface occurred between nodes, some interpolation scheme had to be used to locate the interface. Both linear and quadratic interpolation were tried. However, the soil moisture appears to approach the saturated value in an asymptotic manner, producing large errors in the interpolation process. The result of these difficulties within the computer model was instability of the location of the interface. This led to abandonment of this line of inquiry. Some method which does not require differing forms of Darcy's law for the saturated and unsaturated zones will avoid these difficulties. Rubin (1966) asserts his combination of permeability, soil moisture and capillary potential accomplishes this feat. Almost certainly such a model can be used to ascertain an infiltration model somewhat better than the one presented in Chapter 5. However, such a study was considered outside the scope of the present investigation.

Chapter 5

A MODIFIED WATERSHED MODEL

The Stanford Watershed Model, as described in Chapter 3, has not received widespread use to date. This is due, in part, to the computer language used. A second cause is the inability to associate the computer program directly with the steps in the physical process. A third restriction on the use of the Stanford Model is the inability to deal with time units shorter than one hour^{*}. On small watersheds this is a serious limitation. In this chapter a modified watershed model is presented which attempts to deal with these last two problems. Extensive use has been made of the processes of the Stanford Model, particularly relating to overland flow and stream routing. The material reviewed in Chapter 4 has been used as a guide to the unsaturated flow process. A schematic of the watershed model is shown in Figure 5.1; each phase will be discussed in detail.

The computer program which has been written to implement the modified watershed model is given in Appendix 1. It has been written to accommodate rainfall measured at irregular intervals in either accumulated form as from a weighing gauge or as a rate from a tipping bucket gauge. Either of these records is converted to a record with the average rate for the specified time period. This period may be as short as one minute,

* Although the model operates on a 15 minute cycle, the routing period must be in units of whole hours, and this effectively removed the quarter-hour variations.

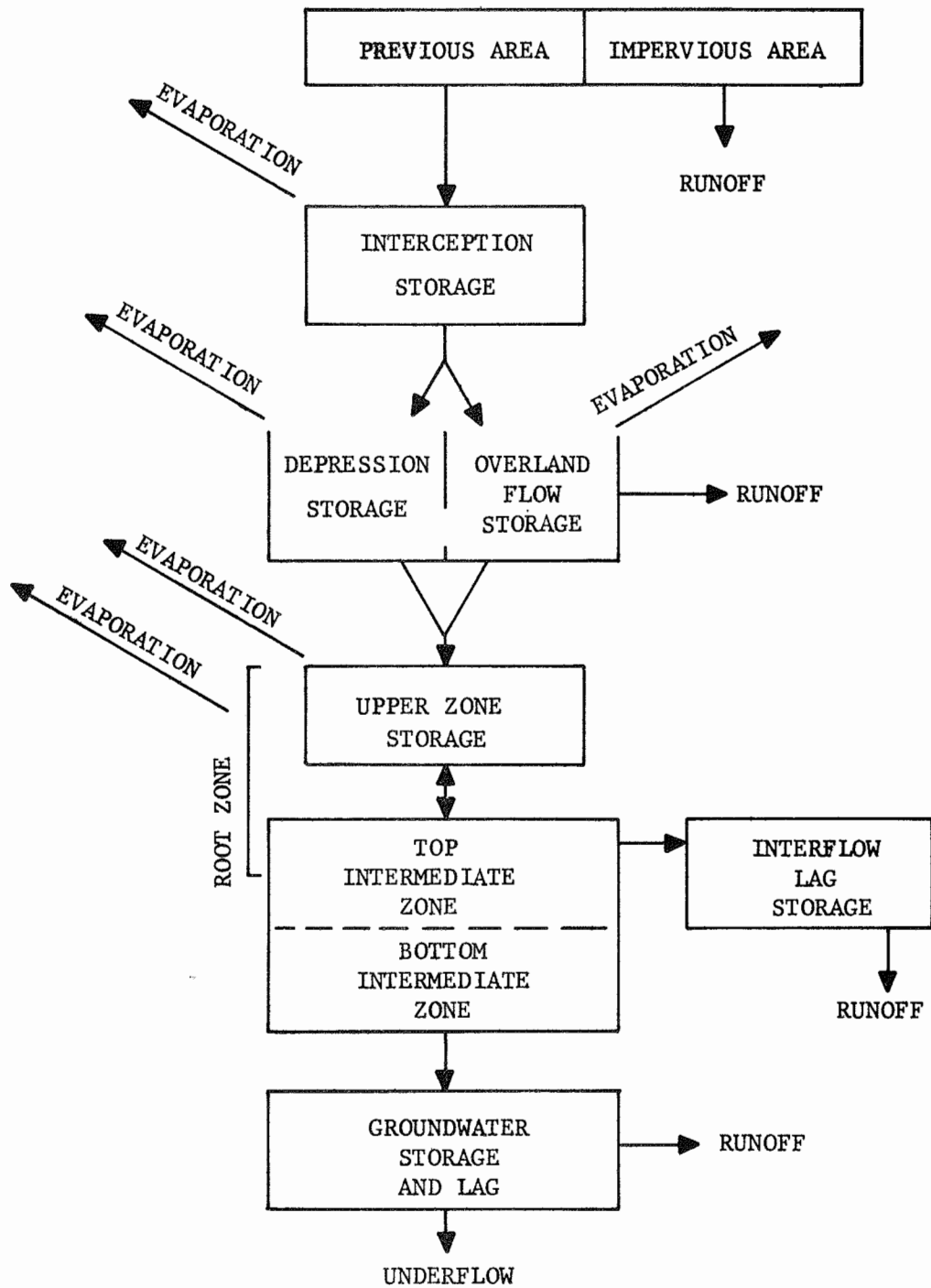


FIGURE 5.1 SCHEMATIC OF UNIVERSITY OF TEXAS WATERSHED MODEL

or as long as one day. A similar record is prepared from the measured streamflow when this is available. The maximum length of the basic accounting cycle is set at fifteen minutes. A full discussion of the input variables and the output available is found in Appendix 1.

Interception Storage

All precipitation falling on the pervious area is routed through interception storage. The volume of water in interception storage in inches at any time is limited by an input parameter, VINSTM, which may vary monthly. This parameter may be estimated as a function of the type and density of material above ground. In agricultural areas, the maximum interception storage may be very small early in the year, increasing to a maximum as the crops mature in the fall, and then drop sharply as the fields are prepared for the winter. On the other hand, in forest or urban areas, the maximum volume may be relatively constant over the annual cycle.

Surface Storage

Water passing through interception storage arrives at the ground surface. It may fall into one of two types of storage: depression storage or overland flow storage. Depression storage, or dead storage, is that volume of storage within depressions of all sizes in the watershed. A depression is defined as a hole or low area with no outlet. Within the pervious area of the watershed, three types of areas are delineated as shown in Figure 5.2. These areas are characterized as (A) areas from which overland flow proceeds directly to the stream: (B)

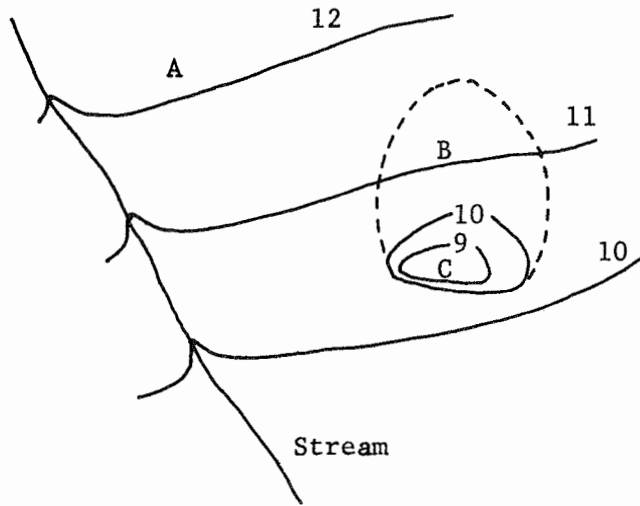


FIGURE 5.2 THREE TYPES OF AREAS WITHIN WATERSHED
 A) OVERLAND FLOW DIRECTLY TO STREAM
 B) OVERLAND FLOW TO DEPRESSION STORAGE
 C) DEPRESSION STORAGE

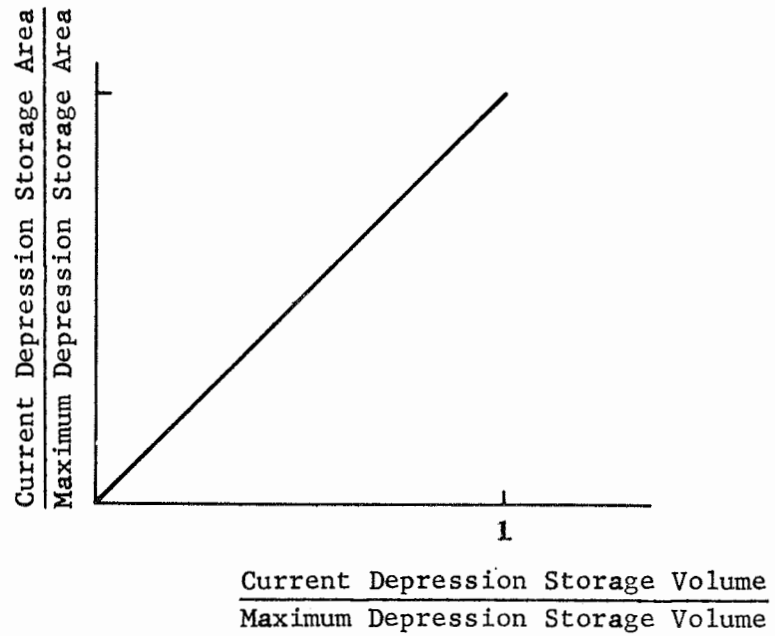


FIGURE 5.3 ASSUMED RELATION BETWEEN SURFACE AREA AND VOLUME OF DEPRESSION STORAGE

areas from which overland flow proceeds into a depression; and (C) areas of depression storage. The areas of depression storage vary from the hole caused by a woman's shoe to farm ponds and other flow retarding structures. The surface area of the water in depression storage at any time is assumed to vary linearly with the volume of depression storage, Figure 5.3. Rainfall reaching the land surface is divided between overland flow storage (OLS) and depression storage (DS) on the basis of the surface area of each type of storage. Overland flow storage is further divided between that flow going directly to the stream and the flow going into depression storage. Infiltration and evaporation, also surface phenomena, are based on the surface area of each type of storage.

The overland flow model used is taken from the Stanford Model. It is a quasi-turbulent form of Izzard's overland flow equations

$$OLF = \frac{64200 S^{0.5}}{nL} \left(\frac{D}{L}\right)^{5/3} \left(1 + 0.6 \left(\frac{D}{D_e}\right)^{3/5}\right) \quad (5.1)$$

where OLF is the rate of runoff in inches per hour per unit area,

n is the Manning roughness factor for overland flow (ROUGH),

L is the length of the overland flow reach in feet (VLENGTH),

S is the slope of the overland flow surface (SLOPE),

D is the storage in ft.³/ft. and is defined as the average of the storage at the beginning of the time period and the storage at the end of the time period (OFSAVG),

D_e is the storage in ft.³/ft. for an equilibrium condition (OFSEQU), and is defined by

$$OFSEQU = \frac{0.000818 i^{0.6} n^{0.6} L^{1.6}}{s^{0.3}} \quad (5.2)$$

where i is the precipitation rate, in inches per hour (XI). This model is based on the following empirical relationship between outflow depth and storage

$$y = \frac{D}{L} \left[1 + 0.6 \left(\frac{D}{D_e} \right)^3 \right] \quad (5.3)$$

where y is the depth, in feet, at the lower edge of the flow plane.

These equations were developed by Linsley and Crawford (1966), and plots showing quite good agreement with a finite difference solution of the partial differential equation of varied flow were presented.

Overland flow generated on that portion of the watershed classified as contributing directly to streamflow is added to the variable R_0 . That part of overland flow originating on the area contributing to depression storage is added to depression storage. Some of the smaller depression storage areas will fill rapidly and surface runoff from areas originally designated as depression areas will commence when these areas are filled. For small indentions of the soil this will occur early in the storm, while for larger storage areas the runoff will not occur until much later. A typical relationship between area producing runoff through depression storage and the volume in depression storage is shown in Figure 5.4. The curve is assumed to be a parabolic with vertex at the origin and may be written as

$$X_a = \left(\frac{1}{C} \frac{DS}{DSMAX} \right)^{0.5} \quad (5.4)$$

where X_a is the fraction of the pervious area which originally was designated as depression storage, or an area contributing flow to depression storage, but is now producing runoff,

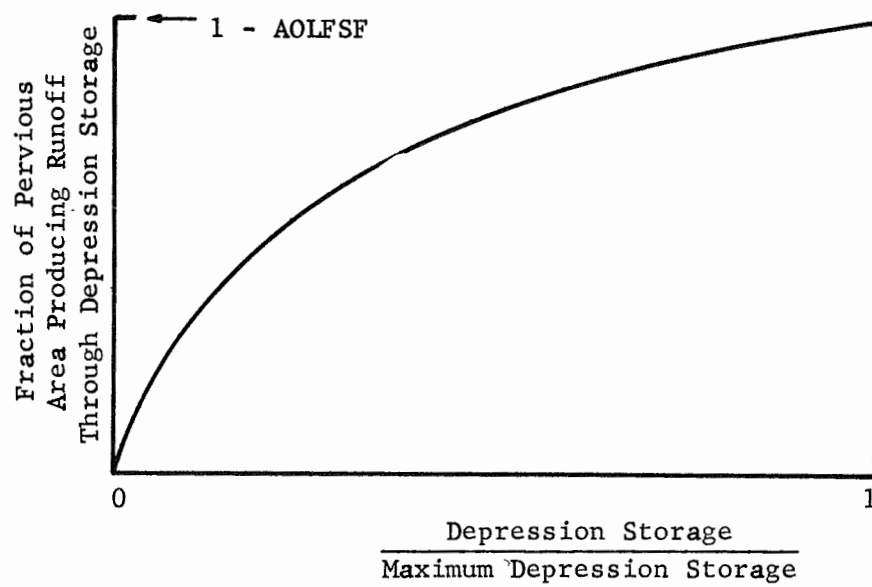


FIGURE 5.4 ASSUMED RELATION BETWEEN VOLUME OF DEPRESSION STORAGE, AND SURFACE RUNOFF FROM DEPRESSION STORAGE

DS is the actual depression storage volume, in inches of depth,
DSMAX is the maximum depression storage volume in inches of
depth, and

C is a constant, normally with a value of 1.

The actual shape of this relation will vary from one basin to the next. However, it is felt that the parabola is adequate for most basins, based on the following preliminary investigation. The depression storage of a basin is the sum of the storage of many different depressions. Figure 5.5 shows several normalized volume-area distribution curves which might characterize different basins. The area under any curve is one, i.e., all of the depression storage within the basin. The basins labeled a and b have a significant proportion of their depression storage in shallow depressions. Basin b₁ has a uniform distribution--equal volume in shallow, intermediate, and deep basins; while both c and d possess considerable amounts of storage in deep depressions. If one assumes that the area producing runoff into a depression is approximately linearly related to the area of the depression storage (that is, the surface area of the depression when full), the relation between fraction of depression storage volume (DS/DSMAX) and the fraction of area which was originally classified as depression storage oriented but is now producing runoff to the stream may be derived. This relation is shown in Figure 5.6. The relation of equation 5.4 is also shown. The fit is reasonably good, except for that portion of the curve generated by deep ponds, i.e., basins with the characteristics of c or d. However, type d basins can be fitted with equation 5.4 provided C is chosen so

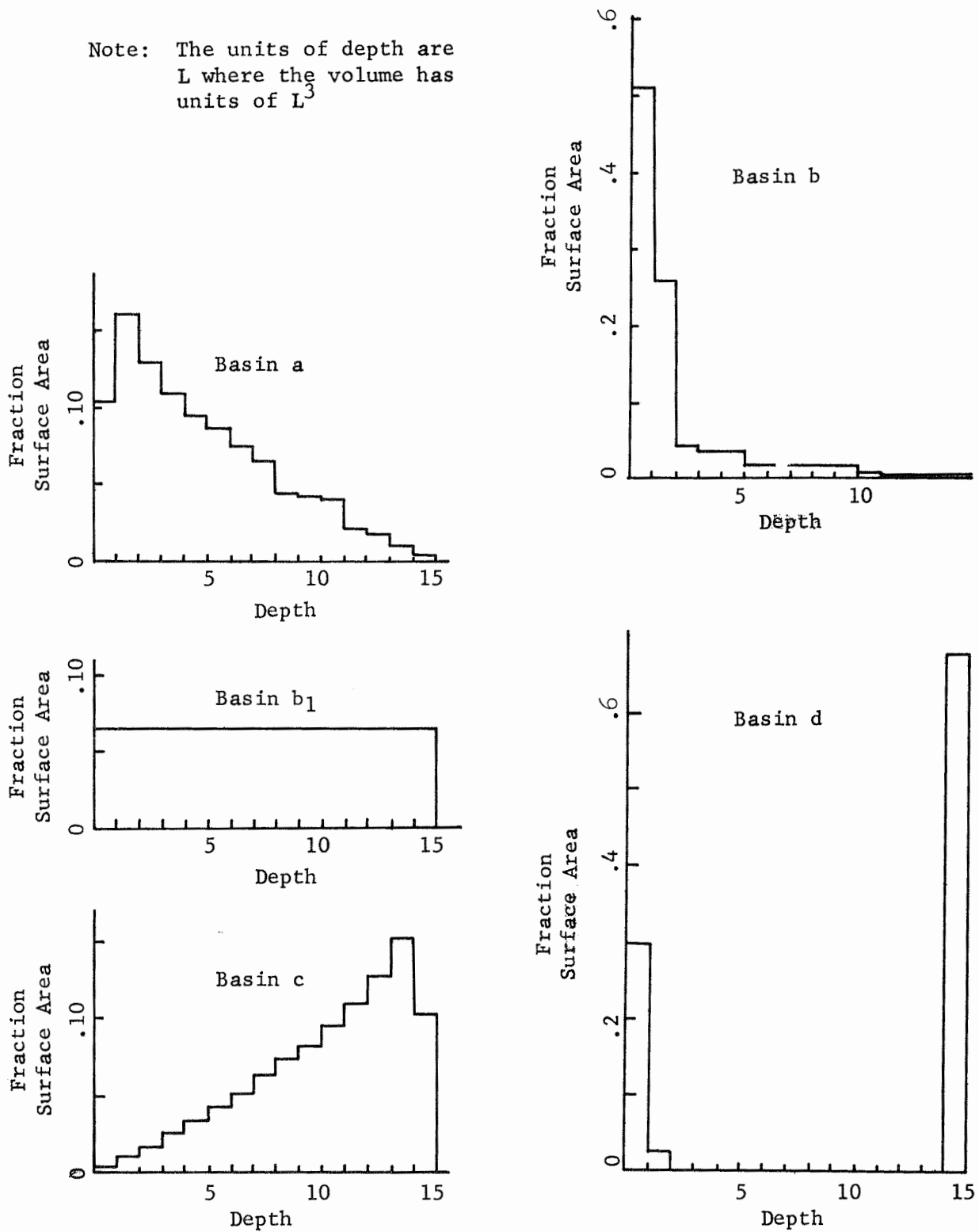


FIGURE 5.5 NORMALIZED AREA-VOLUME HISTOGRAMS

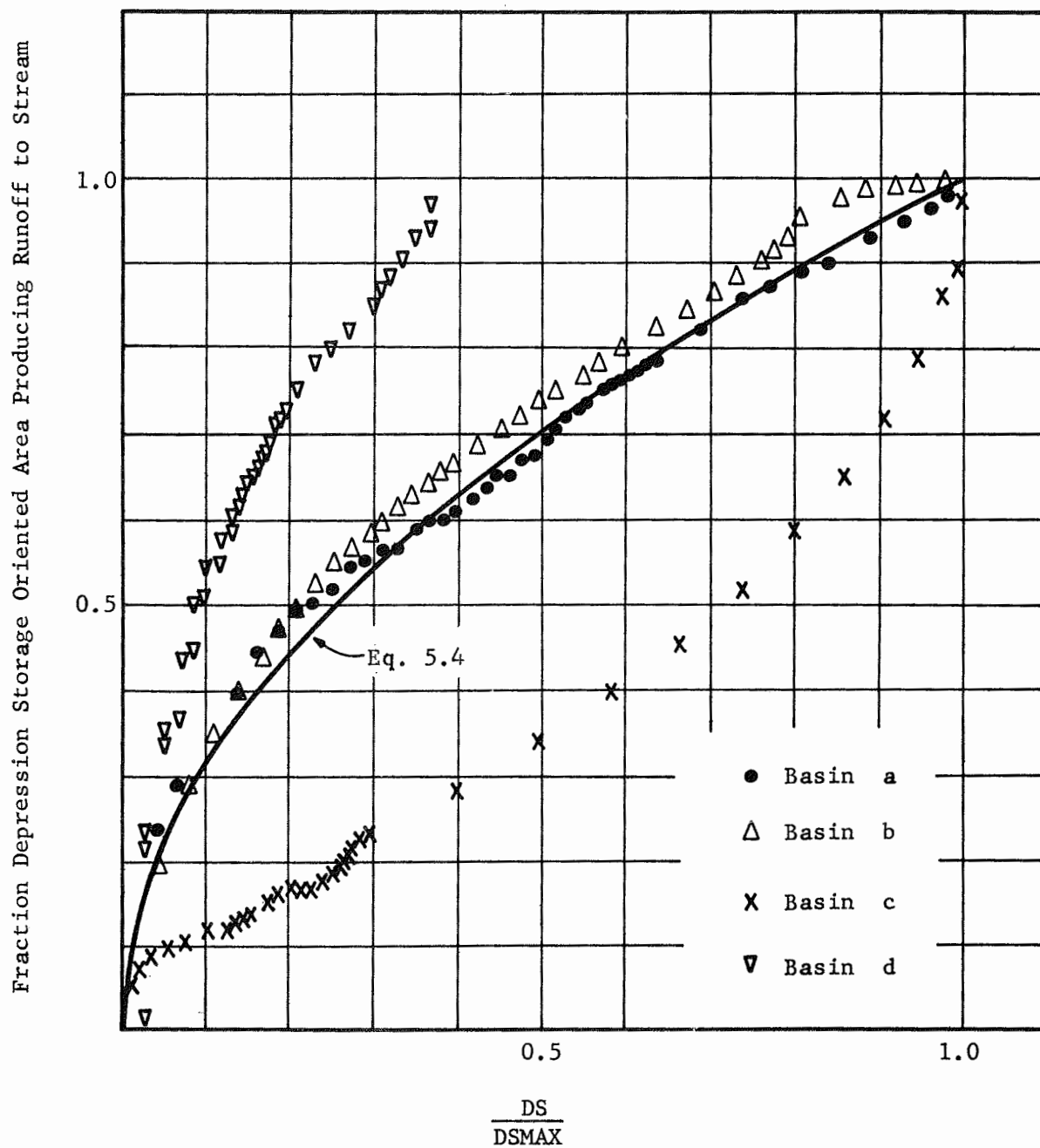


FIGURE 5.6 INFLUENCE OF DEPRESSION STORAGE CHARACTERISTICS ON FRACTION OF AREA PRODUCING RUNOFF

as to cause the curve to pass through the point defined by subtracting the pond storage from the total basin depression storage, and subtracting the pond area (and associated drainage area) from the total area. It is doubtful if natural basins of type c exist, although this might be the distribution for a basin after several "stock pond" type improvements have been made. In such a case, equation 5.4 should probably be replaced by an exponential curve of the form

$$X_a = C \left(\frac{DS}{DSMAX} \right)^z \quad (5.5)$$

where C and z are parameters needed to obtain adequate fit.

Infiltration

The infiltration rate was given in Chapter 4 as being dependent on (a) the available water, i.e., the rainfall rate; or (b) the ability of the soil to conduct the water through the soil in its unsaturated condition, i.e., Rubin's preponding condition, or (c) the ponded depth, the thickness of the saturated zone, and the saturated permeability. Ponding of water occurs only for the third condition. The first is characterized by a rainfall rate less than the saturated permeability of the soil, and the second by a rainfall rate greater than the saturated permeability. Equation 4.4 may be rewritten as

$$V_o = -(K)_{y=0} \left[\left(\frac{d\psi}{dS} \right) \left(\frac{\partial S}{\partial y} \right)_{y=0} + 1 \right] \quad (5.6)$$

where V_o is the infiltration rate. This equation is applicable to both conditions (a) and (b) and $\partial S/\partial y$ should be viewed as the dependent variable.

Referring to Figure 5.7, the infiltration rate of ponded rainfall is given by

$$V_o = -K_{sat} \left[\frac{POND - \psi_{ae}}{SATDPH} + 1 \right] \quad (5.7)$$

where POND and SATDPH have the meanings shown in the figure, and

ψ_{ae} is the value of soil tension at air entry.

Holtan (1961) proposed an infiltration equation

$$f = a F_p^n + f_c \quad (5.8)$$

where f is the infiltration rate when supply is not the limiting factor,

f_c is the constant infiltration rate, the saturated permeability of the soil,

F_p is the total volume which can be infiltrated before a constant rate of infiltration is reached,

a and n are constants.

Holtan recommends a be taken as 0.62 and n as 1.387. F_p is a measure of the voids remaining in the soil column at any time, and hence is also a measure of the water in storage in the column. Since Holtan's equation is for the period when rainfall excess exists, i.e., ponding,

$$-[K]_{y=0} = -K_{sat} = -f_c \quad (5.9)$$

where the minus sign is a matter of convention. Then to the extent that

$$K_{sat} \left[\frac{POND - \psi_{ae}}{SATDPH} \right] = a F_p^n \quad (5.10)$$

Holtan's equation approximates equation 5.7. Holtan found from experimental data that F_p could be expressed as

$$[F_p]_0 = K S_0 \quad (5.11)$$

where the zero subscript indicates evaluation at time zero,

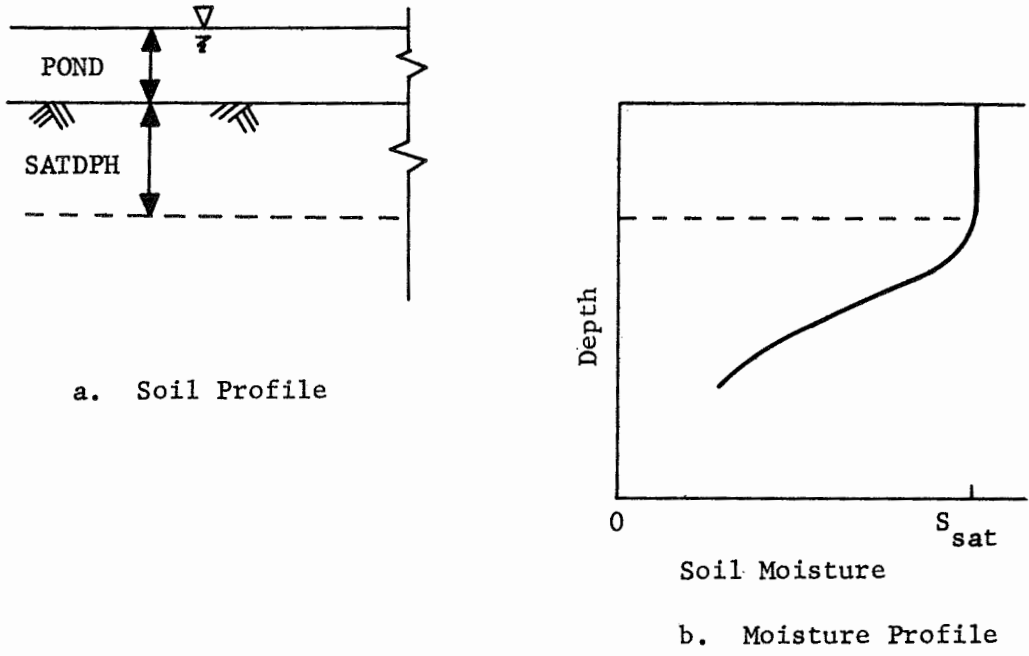


FIGURE 5.7 INFILTRATION WITH PONDED RAINFALL

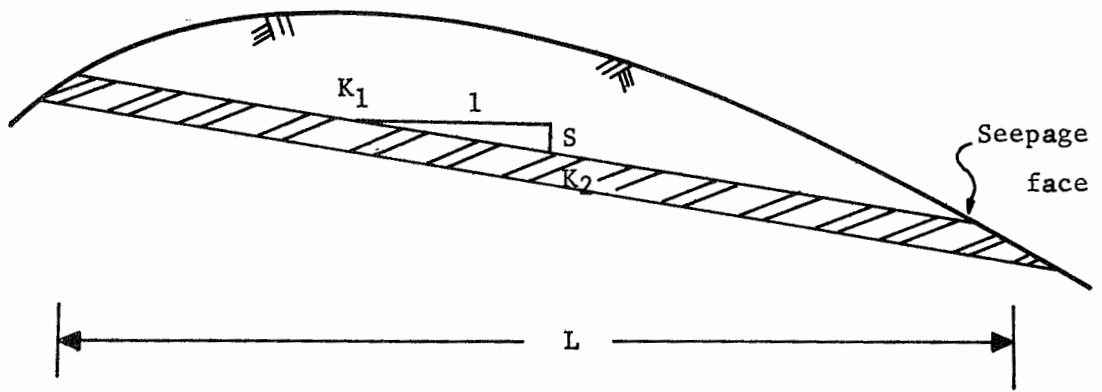


FIGURE 5.8 IDEALIZED INTERFLOW MODEL

K is a vegetative factor given in Table 5.1, and S is the available pore space in the 0-21 inch depth.

TABLE 5.1
HOLTAN'S VEGETATIVE FACTOR

<u>Cover</u>	<u>Value of Vegetative Factor K</u>
Bluegrass	1.00
Crabgrass and Alfalfa	.70
Lespedeza and Timothy	.45
Alfalfa	.35
Weeds	.30

Holtan's equation is particularly suited to a continuous accounting type watershed model, since the independent variable is the unsaturated pore space within the top layer of soil. Defining this layer to be the Upper Zone, equation 5.8 may be written

$$\begin{aligned}
 f &= a(\text{UZST} - \text{UZS})^n + \text{SATPRM} \\
 &= C1 (\text{UZST} - \text{UZS})^{C2} + \text{SATPRM}
 \end{aligned}
 \tag{5.12}$$

where UZST is the total pore space in the Upper Zone, porosity times thickness in inches³/inch².

UZS is the current volume of water in the Upper Zone in inches³/inch².

SATPRM is the saturated permeability of the Upper Zone, in inches per hour.

C1 and C2 are input constants, corresponding to Holtan's a · K and n.

No allowance was made by Holtan for water leaving the Upper Zone, i.e., the volume of the pore space remaining at any time t after the storm began was computed as the pore space available at t_0 less the volume of water infiltrated from t_0 to time t . The watershed model will reflect more accurately the actual volume of water in storage within the Upper Zone. Consequently, the Upper Zone can be thinner than Holtan's twenty-one inches. In agricultural lands, the depth to which cultivation takes place seems a reasonable thickness, while in uncultivated areas, the thickness may vary from one-half inch for very tight soils to several inches for sandy soils. The presence of any less permeable zone should terminate the Upper Zone. Equation 5.12 gives the infiltration rate for the period when supply is not the limiting factor, i.e., the potential infiltration. When the rainfall rate is less than this potential infiltration, all of the rainfall is infiltrated. This decreases the pore space available, and consequently decreases the infiltration potential for the next time period. Rubin (1966) pointed out that for a uniform supply less than the potential infiltration rate, the soil column eventually reaches an equilibrium state in its upper reaches where the gradient $\frac{\partial[K(\psi + y)]}{\partial y}$ is just adequate to transfer the supply. This may also occur in the model, as the flow into the Upper Zone may be exactly equal to the flow out of the Upper Zone. Holtan's equation, while not exact, does seem to offer a good approximation to the infiltration process, both in the situation of infiltration limited by supply, and infiltration limited by the soil condition.

Flow Through The Unsaturated Zone

Ideally, one would describe flow through the unsaturated zone using some form of equation 4.4. However, in a watershed model this will have to be approximated. In the continuous accounting model, the volume of water within each interval of depth can be obtained. However, as the number of these intervals increases, the time to perform the calculations also increases, and soon reaches an economic limit. The number of such zones must be limited. The independent variables in equation 4.4 may be written as functions of the soil moisture, S.

$$\psi = f(S) \quad (5.13a)$$

$$K = g(S) \quad (5.13b)$$

Solution of equation 4.4 must yield

$$V = h(S) \quad (5.14)$$

Several forms of the functions f(S) and g(S) have been proposed.

Ibrahim and Brutsaert (1968) proposed

$$g(S) = K_{sat} \left(\frac{S - S^*}{S_{sat} - S^*} \right)^n \quad (5.15)$$

where n varies from 2 to 5 and depends on the pore size distribution,

assuming the smaller value for pore size distributions with

small variances,

S^* is the soil moisture at which the permeability is "negligibly small," and

K_{sat} is the saturated permeability.

An attempt was made to fit the data for the soil described by Watson (1967), for Geary Silt Loam, and for Sarpy Loam by a least squares technique. In

each case, the value of S^* needed to minimize the square of the error, E , using the model

$$g(S) = K_i = K_{\text{sat}} \left(\frac{S_i - S^*}{S_{\text{sat}} - S^*} \right)^n + E_i \quad (5.16)$$

became greater than the smallest value of S . This required raising a negative number to a power. The definition of S^* seems arbitrary; a strict interpretation of the definition would require S^* to be zero for each soil.

Gardner (1958) proposed

$$g(S) = K = \frac{a}{\psi^\tau + b}$$

which was subsequently modified to

$$g(S) = K = \frac{K_{\text{sat}}}{\left(\frac{\psi}{\psi^*} \right)^\tau + b} \quad (5.17)$$

by King (1965) to obtain dimensional homogeneity. The constant b is approximately 1 for soils with an air entry capillary potential of zero. An estimate of τ may be obtained by evaluating $\frac{d(\log K)}{d(\log \psi)}$ as ψ approaches $-\infty$. King shows good agreement for four soils, ranging from a sand through a clay. King also gives

$$f(S) = e \left(\frac{\cosh \left[\left(\frac{\psi}{\psi^*} \right)^\beta + \alpha \right] - \gamma}{\cosh \left[\left(\frac{\psi}{\psi^*} \right)^\beta + \alpha \right] + \gamma} \right) \quad (5.18)$$

where e is the porosity of the soil, and

ψ^* , β , γ , and α are "parameters depending on the liquid, the soil, and the capillary pressure history."

The following restrictions on the parameters exist:

$$\psi^* > 0$$

$$\beta < 0$$

$$\alpha \geq 0$$

$$0 < \gamma < \cosh \alpha$$

The complexity of equation 5.17 and 5.18 makes curve fitting a prohibitive task.

A somewhat simpler set of functions has been used in the model

$$K = A_1 (S + B_1)^{C_1} + D_1 \quad (5.19)$$

and

$$-\psi = A_2 (S + B_2)^{C_2} + D_2 \quad (5.20)$$

Values of the coefficients for the various soils tested are shown in Table 5.2 (a and b), which may be used as a guide in assigning the coefficients. The program moves moisture from one zone to another using equation 4.4 rewritten as

$$V = K \left(\frac{\partial \psi}{\partial y} + 1 \right) \quad (5.21)$$

The value of permeability is the weighted average of the permeability in each zone as determined by equation 5.19. The value of $\frac{\partial \psi}{\partial y}$ is found from

$$\frac{\partial \psi}{\partial y} = \frac{\psi_1 - \psi_2}{y_1 - y_2} \quad (5.22)$$

where subscripts 1 and 2 refer to the two zones. The value of ψ is found using equation 5.20. The value of S for each zone is found by dividing the total moisture in the zone by the zone thickness.

Interflow

The phenomenon of interflow, while frequently observed in nature has not been studied in detail. A simplified interflow model is shown in Figure 5.8. The occurrence of water at the seepage face is dependent on the slope of the less permeable zone, S , the permeability of this

Description and Source	Permeability							R ²
	A	B	C	D	Max. R.E.	Avg. R.E.		
Geary Silt Loam Hanks and Bowers (1962)	797.7	-0.114	7.75	$2.9 \cdot 10^{-9}$	0.400	0.139	0.949	
Yolo Light Clay Phillip (1957), Gardner (1958)	3.37	-0.103	6.01	$-3.0 \cdot 10^{-7}$	0.372	0.147	0.936	
Watson (1967)	206.8	0.002	4.06	$-6.9 \cdot 10^{-4}$	0.231	0.091	0.919	
Sarpy Loam Hanks and Bowers (1962)	1677.2	0.044	8.70	$-1.6 \cdot 10^{-6}$	0.652	0.284	0.936	
Del Monte Sand (drying) Liakopoulos (1966)	1.689	0.026	2.45	$2.4 \cdot 10^{-3}$	0.233	0.079	0.899	
Del Monte Sand (wetting) Liskopoulos (1966)	1.983	-0.00585	2.66	$2.2 \cdot 10^{-5}$	0.312	0.188	0.915	
Vachaud (1966)	117.32	-0.1185	5.69	$7.1 \cdot 10^{-4}$	0.187	0.068	0.954	
Remson (1965)	$1.6 \cdot 10^4$	0.087	25.16	$1.0 \cdot 10^{-9}$	1.790	1.179	0.883	

TABLE 5.2a COEFFICIENTS FOR PERMEABILITY AND CAPILLARY POTENTIAL APPROXIMATIONS

Description and Source	Capillary Potential							R ²
	A	B	C	D	Max. R.E.	Avg. R.E.		
Geary Silt Loam	0.061	0.063	-7.67	-8.11	0.014	0.004	0.963	
Yolo Light Clay	Data given in equation form: $\psi = -7.87 \cdot 10^{-7} / 12.2 \cdot K - 1$							0.5
Watson	-47.6	-0.047	-0.033	33.74	0.008	0.001	0.923	
Sarpy Loam	26.20	.7745	-23.91	0.965	2.68	0.753	0.898	
Del Monte Sand (drying)	225.3	1.0	-11.35	3.64	0.59	0.239	0.900	
Del Monte Sand (wetting)	4.8788	-0.0219	-9.8626	-2.888	0.25	0.119	0.911	
Vachaud	9.937	0.6555	-21.28	1.207	1.765	0.478	0.894	
Remson	249.76	0.816	-30.89	-2.653	0.480	0.167	0.909	

TABLE 5.2b COEFFICIENTS FOR PERMEABILITY AND CAPILLARY POTENTIAL APPROXIMATIONS

zone in relation to the permeability of the zone above it, the depth to the less permeable zone, and the area contributing water to the interflow process, i.e., the length L in Figure 5.8. These variables and perhaps others combine to change a portion of the infiltration hydrograph into the interflow hydrograph. The simplest mathematical formulation of the process would seem to be one which determines the volume of the interflow and produces the interflow hydrograph by lagging this volume. This method is used in the watershed model. The volume of interflow from the top level of the Intermediate Zone is determined by

$$VINFLO = C_{16} (TIZS - C_{10} \cdot VINST) \Delta t \quad (5.23)$$

where VINFLO is the volume of water added to the interflow process during the time period Δt ,

TIZS is the volume of water stored in the top half of the Intermediate Zone,

VINST is the total volume of the Intermediate Zone, and

C_{10} , C_{16} are the input parameters which must reflect the difference in permeability between the two zones.

When the quantity

$$TIZS - C_{10} \cdot VINST \leq 0,$$

VINFLO is set to zero. C_{10} is seen to be a parameter which sets a lower limit on the volume of water which must be present before interflow can occur. The interflow hydrograph shape is determined by a lag function

$$X = (C_{14} \cdot DELINF + C_{15}) \Delta t \quad (5.24)$$

where X is the volume of water discharged to the stream from

interflow storage in time Δt ,

DELINF is the volume of water in interflow storage, and C_{14} , C_{15} are input parameters.

No rationale can be given for Equation 5.24. Probably the parameter C_{15} can be taken as zero. By analogy to routing equations for stream, the outflow should be related to the volume in storage.

Groundwater Flow

Much of what was said about the difficulties of mathematically modeling the interflow process also applies to the flow of groundwater. The watershed model makes provision for groundwater flow to occur into the stream and also to flow out of the basin. The latter is termed "underflow," and is determined as a function of the water stored in the saturated portion of the soil profile

$$Y = C_{13} \cdot GWS \cdot \Delta t \quad (5.25)$$

where Y is the volume of water leaving the basin as underflow in the period Δt ,

GWS is the volume of water in groundwater storage, and

C_{13} is an input parameter.

The flow into the stream is determined in a similar manner

$$X = (GWS - C_{11}) \cdot C_{12} \cdot \Delta t \quad (5.26)$$

where X is the volume of water flowing from the groundwater system into the stream during time Δt ,

C_{11} is the volume of water in groundwater storage below which no flow into the stream occurs. (A slight modification here

would allow influent streams to be modeled.)

C_{12} is an input parameter.

For the base flow period of the stream hydrograph, equation 5.26 is analogous to the commonly used decay equation

$$q_i = k \cdot q_{i-1} = k^i \cdot q_0 \quad (5.27)$$

where q_i is the flow for the i^{th} time period,

q_{i-1} is the flow for the $(i-1)^{\text{st}}$ time period,

q_0 is the flow at the beginning of the first time period, and

k is the recession constant.

For equation 5.26,

$$k = \frac{1}{1 - C_{12} \frac{GWS + C_{11}}{GWS - C_{11}} \Delta t} \quad (5.28)$$

It is seen that k is not constant, but approaches zero as the groundwater storage approaches the input parameter C_{11} . The result of the variable recession constant is a steeper and shorter recession than given by equation 5.27. This is further accentuated by the removal of a volume of water from groundwater storage for underflow.

Evaporation and Evapotranspiration

The evaporation process is highly dependent on the energy received in the form of sunlight. The amount of energy received is reflected in both the total evaporation and in the time distribution of the evaporation. Other important climatic factors in the evaporation process are wind and humidity. The average effect of all three factors will be reflected in the amount of evaporation from an evaporation pan. Accordingly, the model makes use of monthly pan evaporation to establish potential monthly

evaporation for the basin. An average daily potential is determined from the monthly potential, using a second order interpolating equation, and finally an instantaneous potential evaporation rate is determined. Potential evaporation is assumed to occur from thirty minutes past sunrise until one hour after sunset, and to reach a peak when three-fourths of this time has elapsed. A skewed sine curve fitted to these three points is used for estimating the instantaneous potential evaporation rate. The time distribution of the evaporation potential agrees well with data reported by van Bavel (1966) as shown in Figure 5.9.

Evaporation occurs first from Interception Storage. If there is insufficient water stored in Interception Storage to satisfy the potential evaporation for this period, additional water is removed, in order, from Overland Flow Storage, Depression Storage, and Upper Zone Storage. Evaporation is assumed to occur at the potential rate from Overland Flow Storage and from Depression Storage. It will also occur at the potential rate from the Upper Zone Storage when the Upper Zone is saturated. However, as the water content of the Upper Zone decreases, the evaporation also decreases because of the difficulty of the soil to deliver the necessary moisture to the surface. This is approximated in the model by using

$$E = \text{EVAPOT} \cdot \left(\frac{\text{UZS} - \text{UZMIN}}{\text{UZST} - \text{UZMIN}} \right)^\alpha \quad (5.29)$$

where E is the actual evaporation rate from the soil surface,

EVAPOT is the potential evaporation,

UZS is the current volume of water stored in the Upper Zone,

UZMIN is the minimum volume allowed in the Upper Zone (evaporation

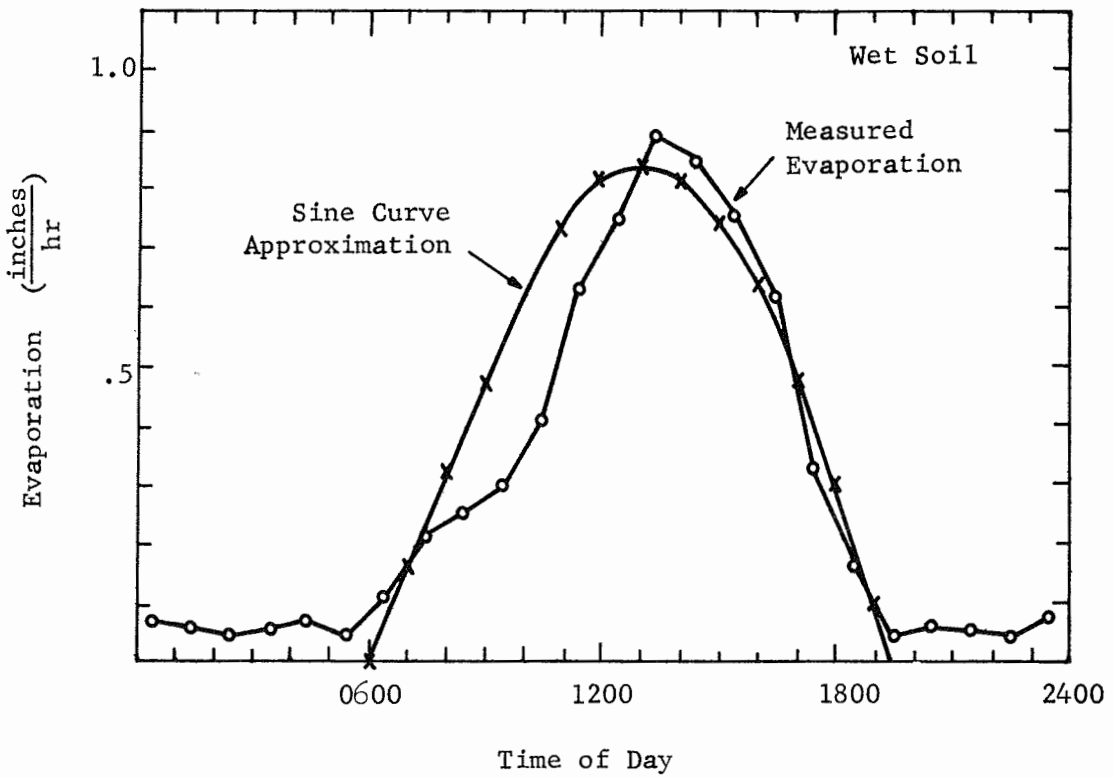
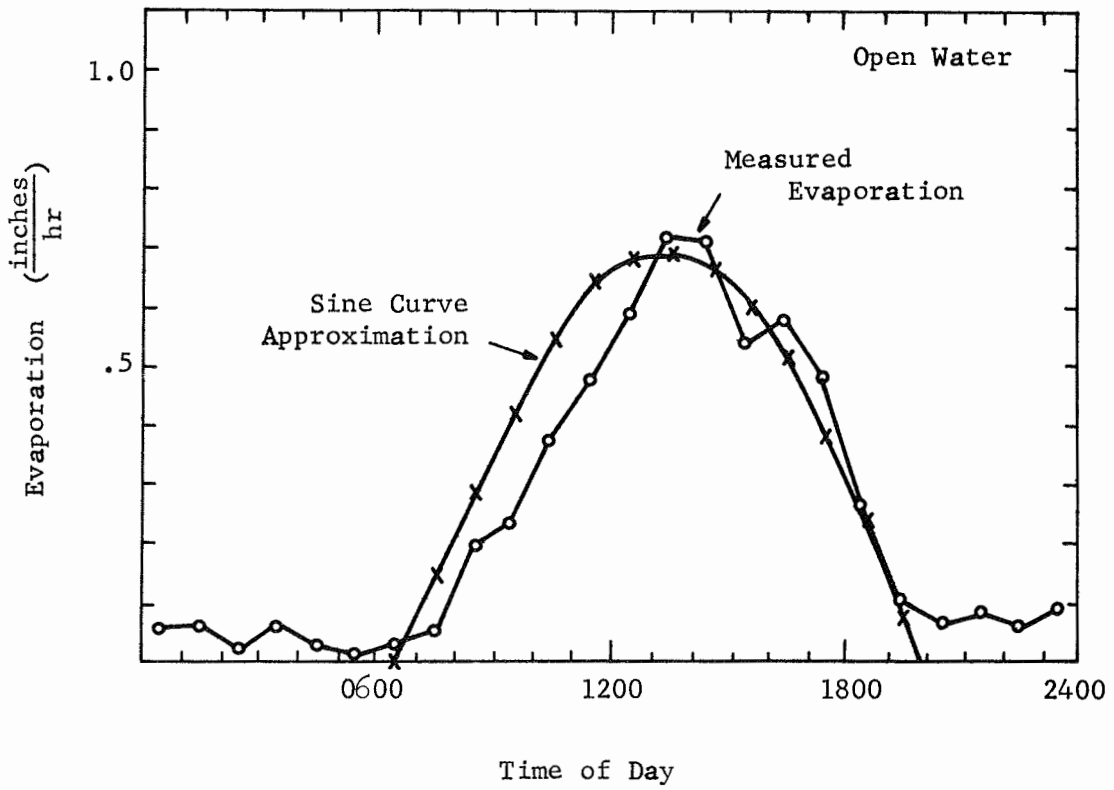


FIGURE 5.9 SINE CURVE APPROXIMATION OF van Bavel's (1966) EVAPORATION MEASUREMENTS

will not remove the chemically bonded water from the soil particles),

UZST is the total volume allowed in the Upper Zone, and

α is a parameter describing the soil's ability to deliver water to the surface.

The ratio, $E/EVAPOT$ is shown plotted against the moisture values for various levels of the parameter α in Figure 5.10.

Transpiration of water from the soil by plant life seems to be independent of the soil moisture content when the content is above the wilting point. Consequently, during the months when transpiration is a factor, water is removed from the root zone (RTZONE) in addition to the evaporation. This removal is limited by the wilting point moisture content, assumed equal to the minimum volume for each zone. The root zone thickness does not necessarily coincide with the Upper Zone--for deep rooted trees and such crops as alfalfa it should be larger. The monthly evaporation as measured by the pan evaporation is augmented by the monthly consumptive use (TRANPO).

Stream Flow Routing

The runoff for any time period as calculated by the model thus far described must be routed through the basin in order to produce the hydrograph of stream flow. As in the Stanford Model, a distribution graph technique is used. (Figure 3.10) The flow generated during this period is distributed according to input parameters into future runoff intervals,

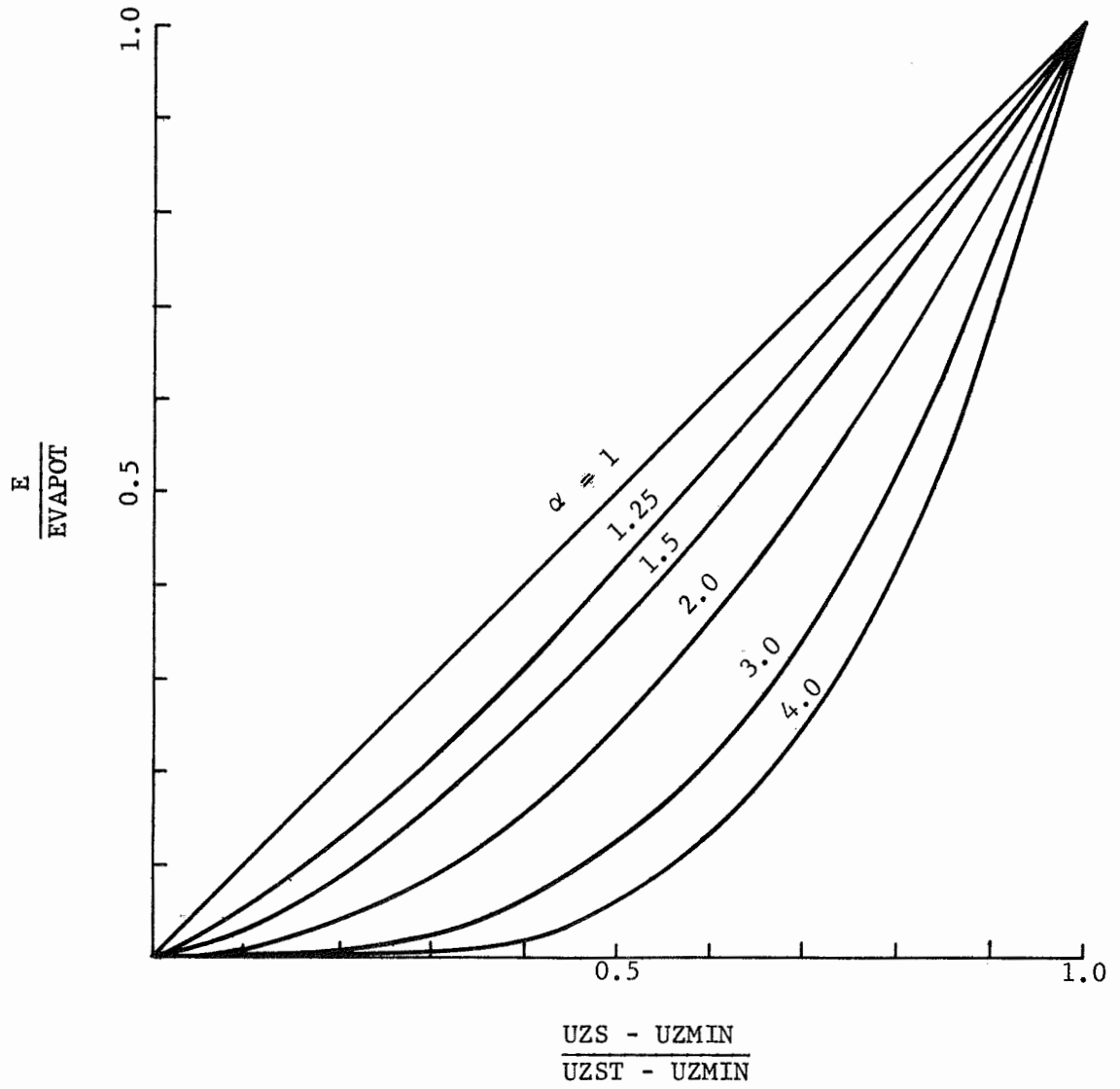


FIGURE 5.10 VARIATION OF TRANSPIRATION WITH MOISTURE CONTENT

i.e., the flow reaching the stream during the present time interval will appear at the gauging point distributed in time.

Watersheds with Multiple Segments

The program will simulate a watershed composed of several sub-watersheds or segments. The order in which the segments are simulated must be from upstream to downstream, and the precipitation tape must be prepared in this same sequence. The program processes the segments sequentially; segment numbering must conform to the above usage.

Program Options

The program written to implement this model provides several options which the user may exercise to obtain output which meets his needs. The program will provide detailed storm analysis, printing the stream flow for each time period, or plotting this data on the printer. Options are also available for statistical analyses of several types when measured stream flow is supplied. The correlation coefficient for each storm is provided with the detailed storm analysis. The correlation coefficient is defined as

$$\text{CORRE} = \frac{\sum X_i Y_i - (\sum X_i \sum Y_i)/n}{\left[(\sum X_i^2 - (\sum X_i)^2/n) \cdot (\sum Y_i^2 - (\sum Y_i)^2/n) \right]^{0.5}} \quad (5.30)$$

The correlation coefficient, defined in the same way, is also provided for the total yearly period. However, this coefficient can be extremely

deceiving since each of the time intervals is counted. If the flow rate is zero for a large portion of the time, the correlation coefficient will be unrealistically high. A second measure of the goodness of fit is provided by classifying the flows into several flow ranges, and analyzing each range individually. The ranges are the same as used by the Stanford Model; the logarithm of the interval size is constant. Twenty-five ranges are provided; for each class the total number of cases in that class, the average error of these events, the average absolute error, and, when possible, the standard deviation of the errors is presented. This table allows the user to judge whether the model is doing an equally good job on all magnitude of flows. A third statistic which is provided is the coefficient of weighted absolute errors defined by

$$\text{COEFF} = \frac{\sum A_i |X_i|}{\sum A_j f_j \bar{x}} \quad (5.31)$$

where A_i is the weighting factor, defined by

$$A_i = \ln \left(\frac{Y_1 + Y_2}{2} \right) + 1$$

Y_1 and Y_2 are the two end points of the flow interval as described above which contain the i th flow rate

X_i is the error, i.e., the difference between the simulated and the recorded flow for the i^{th} time period

f_j is the number of events in the j^{th} class

\bar{x} is the average flow for the time interval.

This coefficient eliminates the time intervals of no flow and provides a logarithmic weighting factor. In general, the smaller it becomes, the better the overall fit.

The adequacy of each part of the proposed model must be tested on existing basins. Two such tests have been performed and are reported in Chapter 6.

With further testing and with more analytical work performed, some aspects of the model are sure to change. For example, a two dimensional finite-difference analysis of the phenomenon of interflow will surely result in a better mathematical model to use in the simulation process. However, the model, as presented in this chapter, contains the basic framework to support improved mathematical statements of the various components of water transfer without destroying the model. This feature, it is believed, will make the basic model structure extremely usable and durable.

Chapter 6

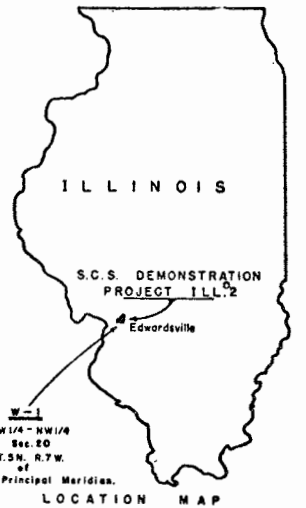
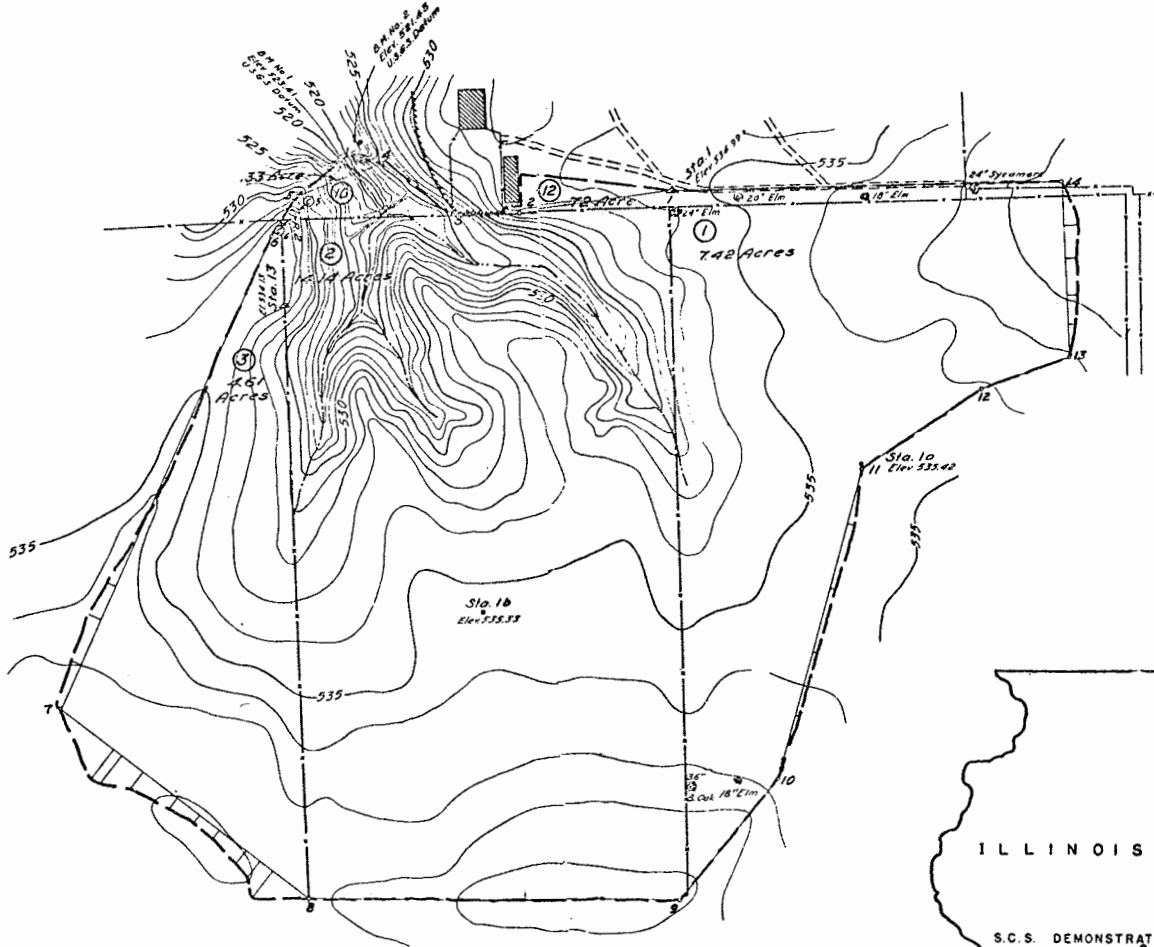
APPLICATIONS OF THE WATERSHED MODEL

The algorithms used to describe the various components of the hydrological cycle within the watershed model have been described in Chapter 5. The use of the model on two small watersheds is described in this chapter.

The Edwardsville, Illinois Watershed

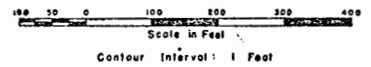
Holtan and Minshall (1968) have presented comprehensive precipitation and streamflow data for watersheds WI and WII at the Soil Conservation Service experimental area near Edwardsville, Illinois, for the period July, 1941, through June, 1943. The description of the watershed is complete enough to allow estimation of most of the parameters needed for the model. Watershed WI was chosen because it is the smaller (27.22 acres) and appears to be the more homogeneous. Figure 6.1 shows the contour map of the watershed, together with some land use data as furnished by the Agricultural Research Service. The precipitation records are from weighting type recorders; the watershed contains four recorders. There was little or no variation in the four records--so little, in fact, that only one recorder record is given by Holtan and Minshall for many of the storms. Consequently, a composite precipitation record was constructed relying heavily on recorders R9 and R11, Figure 6.2. Runoff was measured by a triangular weir; runoff rate was reported.

Several small plot infiltration studies were conducted within the experimental watershed. Rather complete descriptions of these are given,



CROPPING PLAN

CROP YEAR	Field No. 1 742 Acres	Field No. 2 14.14 Acres	Field No. 3 4.61 Acres	Field No. 10 0.33 Acres	Field No. 12 0.72 Acres
1937	Corn	Oats	Corn	Permanent Pasture	Permanent Pasture
1938	Corn	Alfalfa	Wheat Sweet Clover		
1939	Wheat Sweet Clover	Alfalfa	Corn	↓	↓
1940	Corn	Alfalfa	Wheat Sweet Clover	↑	↑
1941	Wheat Sweet Clover	Alfalfa	Corn	↑	↑
1942	Corn	Alfalfa	Wheat Sweet Clover	↑	↑
1943					



WATERSHED CHARACTERISTIC

1. Size..... 27.22 Acres 0.0425 Sq. miles
2. Range in Elevation (approx. M.S.L.) from 520.0. ft. to 540.0. ft.
3. Prevailing Land Slope..... 1.0 %
4. Range in Land Slopes from 0.2 % to 10 %
5. Length of Principal Waterway..... 785. ft.
6. Average Slope of Principal Waterway..... 1.8 %
7. Total Number of Waterways..... 3
8. Number of Acres per Waterway..... 9.07
9. Total Length of Waterways..... 1650. feet.
10. Drainage Density (Length of waterways per acre)..... 61. ft/acre
11. Form Factor A/L^2 0.77

LEGEND

- Field Boundary.....
- Field Number..... 12
- Runoff Measuring Station.....
- Diversion Terrace.....
- Drop Structure.....
- Rainfall Measuring Station and Number..... 0.22
- Secondary Traverse Station..... 0.76
- Watershed Boundary Traverse Station..... 0.8
- Camera Station and View Number..... 50
- Watershed Boundary.....

NOTE: All other Symbols are those adopted by the U.S. Board of Surveys and Maps.

FIGURE 6.1 DATA FOR WATERSHED WI

VIEW NEGATIVE NUMBER
View No. 5 Negative No. III - R.S. - 30
View No. 6 Negative No. III - R.S. - 31

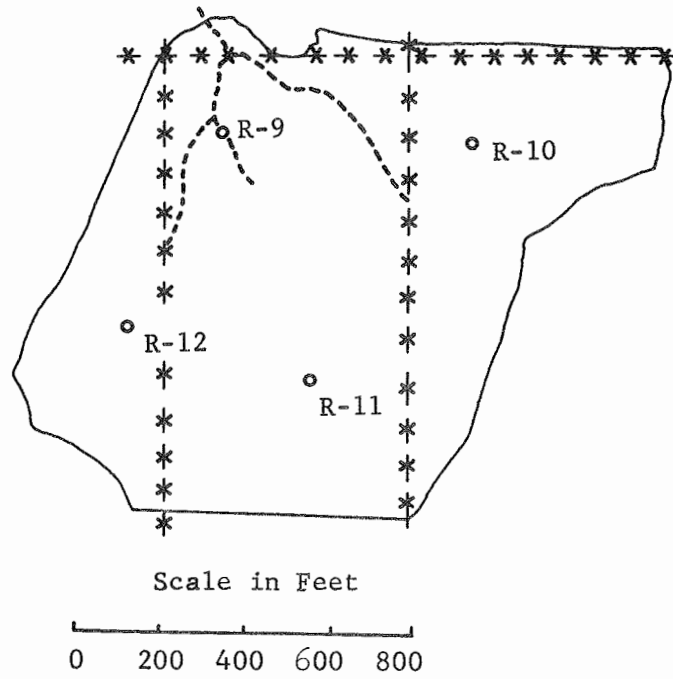


FIGURE 6.2 LOCATION OF RECORDING RAIN GAUGES IN WATERSHED WI

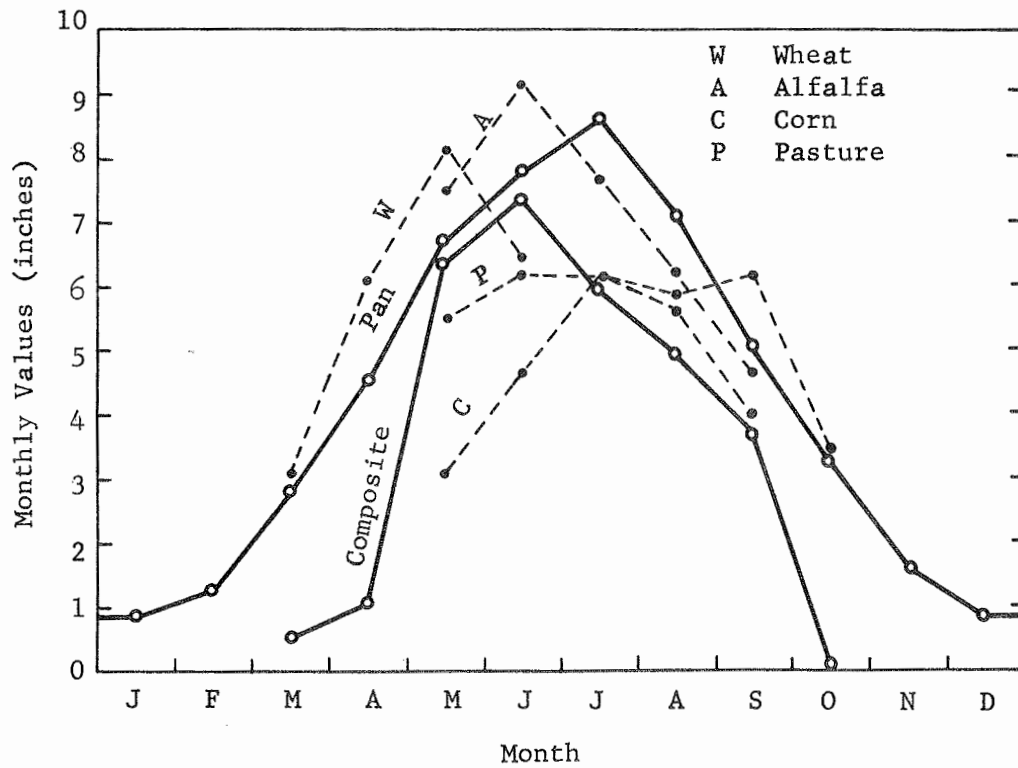


FIGURE 6.3 MONTHLY VARIATION OF PAN EVAPORATION AND CONSUMPTIVE USE

together with soil types and depths. As a result of these experiments, Holtan and Minshall divided the watershed into four areas, based on the average permeability in a three-hour test as shown in Table 6.1.

Pan evaporation rates were secured from the United States Weather Bureau records. Monthly values are shown on Figure 6.3. As shown on Figure 6.1, crops for 1942 consisted of corn, alfalfa, wheat and sweet clover, in addition to a small amount of permanent pasture. The consumptive use of these crops was estimated by the Blaney-Criddle method (Chow, 1964). These monthly values are plotted on Figure 6.3. Also shown on Figure 6.3 is the monthly transpiration potential used by the model.

TABLE 6.1

DISTRIBUTION OF EQUILIBRIUM INFILTRATION RATES FOR WATERSHED WI

<u>Percent of Total Area</u>	<u>Infiltration Rate (inch/hour)</u>
12.06	0 - 0.53
20.99	0.53 - 0.67
19.63	0.67 - 0.83
47.32	over 0.83

Holtan and Minshall indicate no groundwater contribution to streamflow from WI. There was some, however, from WII, of which WI is a part. It seems logical then to suppose that some amount of "underflow" was occurring.

Selecting the Parameters

One goal of the watershed model is to have physically meaningful and physically determined parameters. The parameters and the basis of the

selected value will be discussed.

Impervious Area: Figure 6.1 shows no impervious area, assume a value of zero.

Division of Pervious Area: No benching or contouring is evident from Figure 6.1. Consequently 0.8 of the total area was used as producing runoff directly to the stream, 0.15 as producing runoff directly to depression storage, and 0.05 as depression storage, except for the months of October and May. Values for October were 0.4, 0.5, and 0.1 while 0.3, 0.55, and 0.15 were used for May. These changes were necessary to account for the land preparation prior to planting winter wheat on 4.61 acres in October and corn on 7.42 acres in May.

Infiltration Parameters: From equation 5.12, three parameters are needed in the infiltration equation: C_1 , C_2 , and $SATPRM$. C_1 was found using Holtan's recommended value of 0.62 times a vegetative factor of 0.35 from Table 5.1. C_2 is also Holtan's recommended value. The saturated permeability was originally taken as 0.15 inches/hour. However, after studying the storms of October 30-31, 1941, when the soil was saturated, this value was revised to 0.05.

Unsaturated Permeability and Capillary Potential Parameters: At the present stage of development, these parameters are difficult to assign. Holtan and Minshall give soil horizon descriptions at four sites within WI. Bogota silt loam is the predominate soil type, which they describe as "highly permeable." Chow (1964), quoting from the United States Soil Conservation Service, classifies the Bogota series as group C--slow infiltration rates generally with impeding layer.

The soils listed in Table 5.2, for which the parameters have been found by a curve fitting technique, are not well described in the literature from which the data were taken. In fact, all are laboratory results from reconstituted soils. The values of Geary Silt Loam were used initially. However, the value of saturated permeability as computed by equation 5.19 did not agree with the value used in the infiltration equation. Reference to the table in Chow (1964) showed that this soil is in group B, a group with somewhat more permeability than group C. Consequently, the parameters were changed to describe a less permeable soil. Final values are given in Table 6.2.

Routing Parameters: A basic time interval of 10 minutes was chosen as corresponding reasonably well with the size of the watershed without causing the program to consume an excess amount of time for execution. The contour map, Figure 6.1, was examined, and the following velocities assigned:

areas below el. 530	1.63 ft/sec
areas between el. 530 and 535	1.13 ft/sec
areas above el. 535	0.67 ft/sec

Travel times and areas were estimated as shown in Figure 6.4 in which the travel times in minutes are underlined and the contributing areas, in acres, are indicated along the stream reach. The time distribution of runoff corresponding to an instantaneous rainfall is shown in Figure 6.5. The resulting routing elements were

0.12 0.42 0.40 0.06.

TABLE 6.2 PARAMETER VALUES

Parameter	Values
XLAT	38.8
XLONG	90.0
CORTZ	0
Monthly Pan Evaporation	0.84 1.28 2.72 4.54 6.64 7.80 8.70 7.16 5.10 3.32 1.63 0.84
TRANPO	0.0 0.0 0.52 1.03 6.28 7.33 5.91 4.97 3.66 0.31 0.0 0.0
NEWVAL	0 0 0 0 1 1 0 0 0 1 1 0
KSEGMT	1
JRINT	10
ISEG1	0
ISEG2	0
NRELEM	4
LAG	0
C	0.12 0.42 0.40 0.06
PANFAC	0.8
ALPHA	3.5
RTZONE	15.
ATOTAL	27.22
APERVS	1.0
AOLFSF	Oct: 0.4, Nov-Apr: 0.8, May: 0.55, Jun-Sep: 0.8
AOLFDS	Oct: 0.5, Nov-Apr: 0.15, May: 0.25, Jun-Sep: 0.15
DSMAX	0.25
VINSTM	0.005
DEPTHI	12.
DEPTHU	0.75
CHANLG	0.55
C1	0.20
C2	1.387

TABLE 6.2 (continued)

Parameter	Values
C3	186.
C4	-0.114
C5	7.745
C6	$0.2026 \cdot 10^{-6}$
C7	-10.
C8	0.2
C9	-3.0
C10	0.9
C11	0
C12	0
C13	0.001
C14	0.995
C15	0
ROUGH	0.065
SLOPE	0.0175
VLENGH	75.
SATPRM	0.05
UZST	4.14
VIZST	67.44
C16	0.001
SMMIN	0.15

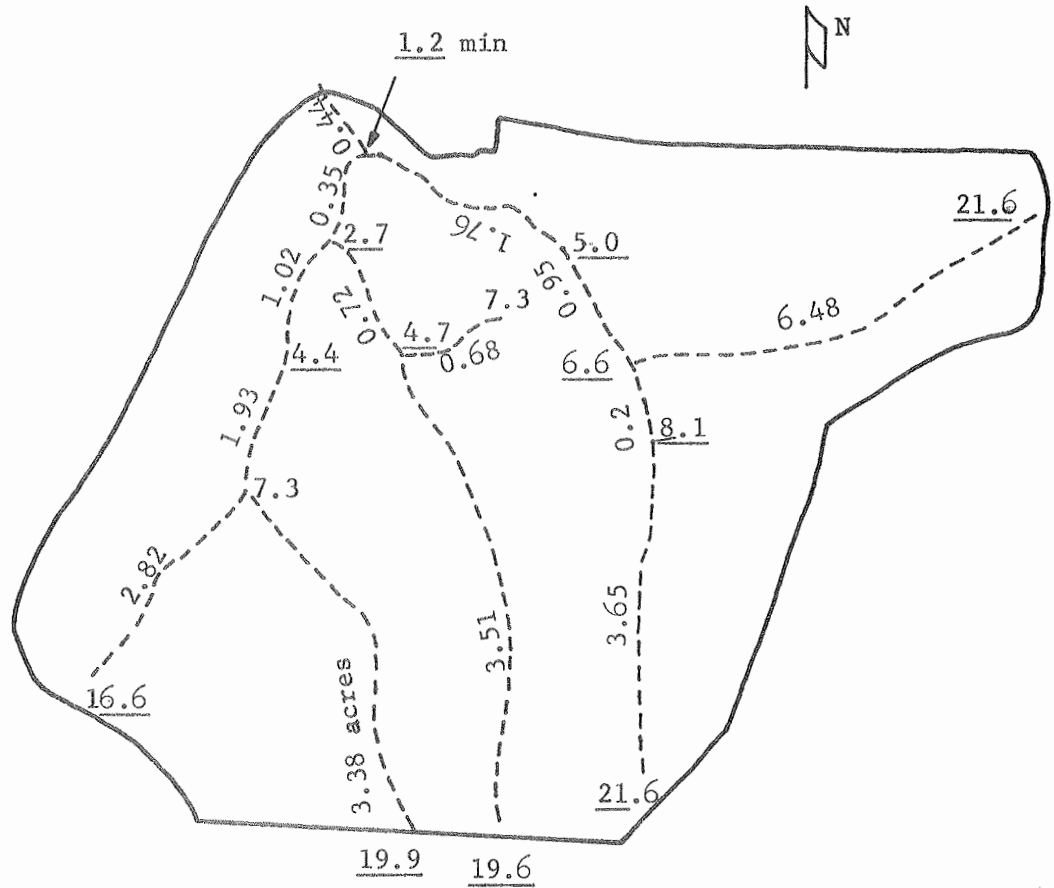


FIGURE 6.4 TRAVEL TIMES AND CONTRIBUTING AREAS FOR WATERSHED WI

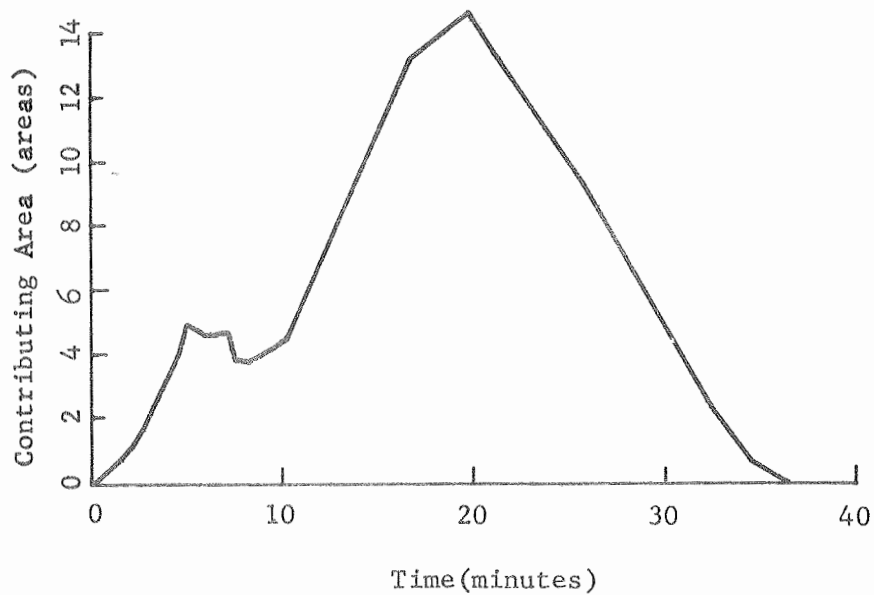


FIGURE 6.5 CONTRIBUTING AREAS OF WATERSHED WI TO AN INSTANTANEOUS INPUT

Upper Zone: A depth of nine inches was chosen for the Upper Zone Storage.

This figure was selected as the probable depth of cultivation during the 1940 era. A maximum moisture content of 0.46 (by volume) was used because of the silt loam classification. A wilting point of 0.15 was used.

Intermediate Zone: No data is available on the soil below about 60

inches. An Intermediate Zone depth of 12 feet was selected as being adequate to prevent any groundwater flow being significant in the model. The same soil moisture limits were used in this zone as in the Upper Zone.

Interflow: The soil descriptions given by Holtan and Minshall indicate a restrictive clay layer at the 18 inch level on the portion of the watershed above elevation 530. Quite probably interflow does occur due to this clay layer. Transfer to interflow from the top portion of the Intermediate Zone is allowed when this zone is 0.9 full. The amount transferred is 0.001 of that volume above 0.9. The volumetric recession constant for the interflow storage is 0.99.

Results of Computer Run

The results of this choice of parameters is presented in Tables 6.3 and 6.4 and in the hydrographs of Figure 6.6 through 6.9. The hydrographs presented were chosen before the computer run as typical of the various conditions encountered by the model.

The model has not been as successful as was expected using the parameters assigned on the basis of available data. The most serious discrepancy is in generating the proper volume of runoff for each storm.

TABLE 6.3 SUMMARY OF MONTHLY SIMULATED AND MEASURED VALUES

Month	Rainfall (inches)	Measured Runoff (inches)	Simulated Runoff (inches)	Simulated Evaporation (inches)
Oct	8.10	1.01	1.04	0.10
Nov	2.87	0.77	0.24	0
Dec	1.59	T	0	0
Jan	1.23	0.01	0	0
Feb	2.59	0.90	0.03	0
Mar	2.01	0.10	0.01	0.43
Apr	2.22	0.34	0.03	0.83
May	5.25	0.10	0.57	5.09
Jun	7.56	1.31	2.16	6.00
Jul	8.20	3.46	3.57	4.75
Aug	3.49	0.46	0.94	3.94
Sep	1.43	T	0.01	2.91
Annual	46.54	8.58	8.59	24.04

TABLE 6.4 COMPARISON OF SIMULATED AND MEASURED VALUES FOR EACH STORM

Date	Rainfall (inches)	Runoff (inches)		Correlation Coefficient
		Measured	Simulated	
10- 2-41	0.30			
10- 4-41	0.20			
10- 5-41	0.36	0.0084	0.0404	0.9484
10- 7-41	0.24		0.0007	0
10- 9-41	0.86	0.0025	0.1996	0.7560
10-14-41	0.42	0.0103	0.0490	0.9453
10-17-41	1.44	0.1729	0.2302	0.8557
10-20-41				
10-22-41	1.89	0.3322	0.3866	0.8020
10-26-41	0.37	0.0031	0	0
10-30-41				
10-31-41	2.02	0.3898	0.0941	0.8701
11- 5-41				
11- 6-41	2.04	0.7495	0.2308	0.8747
11-10-41	0.01			
11-17-41	0.01			
11-19-41				
11-20-41	0.29			
11-22-41	0.35	0.0031		
11-23-41	0.05			
11-24-41	0.12	0.0255		
12-12-41	0.30			
12-22-41	0.28			
12-23-41	0.44	0.0007		
12-25-41	0.28	0.0050		
12-26-41	0.29			
1- 1-42	0.28			
1-18-42	0.05			
1-30-42				
1-31-42	0.90	0.0102		
2- 3-42				
2- 6-42	1.40	0.5999	0.0271	0.7963
2- 9-42	0.21			
2-15-42				
2-16-42	0.55	0.0851		
2-23-42	0.35			
2-26-42	0.05			
2-27-42		0.0157		
2-28-42		0.0082		

TABLE 6.4 (continued)

Date	Rainfall (inches)	Runoff (inches)		Correlation Coefficient
		Measured	Simulated	
3- 4-42	0.03			
3- 5-42				
3- 7-42	0.71	0.1365		
3- 8-42				
3-12-42	0.65	0.0874	0.0073	0.4577
3-13-42				
3-20-42	0.07			
3-25-42	0.25			
3-26-42				
3-27-42	0.23			
3-30-42	0.07			
4- 6-42	1.16	0.0199	0.0215	0.9712
4- 7-42				
4- 8-42	1.17	0.3211	0.0019	0.5257
4-10-42				
4-24-42	0.02			
5- 2-42	0.17			
5- 3-42	1.05	0.0107	0.1629	0.3763
5- 5-42	1.11	0.0465	0.2928	0.8658
5- 6-42				
5-11-42	0.03			
5-13-42	0.60	0.0047	0.0792	0.6288
5-15-42	0.96	0.0288	0.1080	0.9270
5-17-42	0.10			
5-18-42	0.59	0.0098		
5-23-42	0.16			
5-26-42	0.07			
5-31-42	0.41		0.0002	0
6- 1-42	0.59	0.0058	0.1051	0.7161
6- 6-42	0.12			
6- 9-42	0.31			
6-10-42	0.07			
6-11-42	0.18			
6-13-42	1.35	0.1442	0.5503	0.9823
6-15-42	0.31			
6-18-42	0.52	0.0033	0.0322	0.0768
6-19-42	0.10			
6-20-42	0.03			
6-21-42	1.69	0.0275	0.6521	0.9685
6-25-42	0.50			
6-26-42	1.79	0.8281	0.8110	0.9064

TABLE 6.4 (continued)

Date	Rainfall (inches)	Runoff (inches)		Correlation Coefficient
		Measured	Simulated	
7- 7-42				
7-10-42	6.97	3.2982	3.3998	0.9164
7-12-42	0.45	0.0703	0.1111	0.9463
7-14-42	0.51	0.0598	0.0274	0.5551
7-19-42	0.09			
7-21-42	0.63	0.0174	0.0662	0.9378
7-26-42	0.31			
8- 2-42	0.18			
8- 3-42	0.20			
8- 6-42				
8- 7-42	2.21	0.2157	0.5390	0.9014
8-14-42				
8-15-42	0.90	0.0024		
9- 7-42				
9- 8-42	0.82	0.0020	0.0027	0.1332
9-19-42	0.46			
9-26-42	0.15			

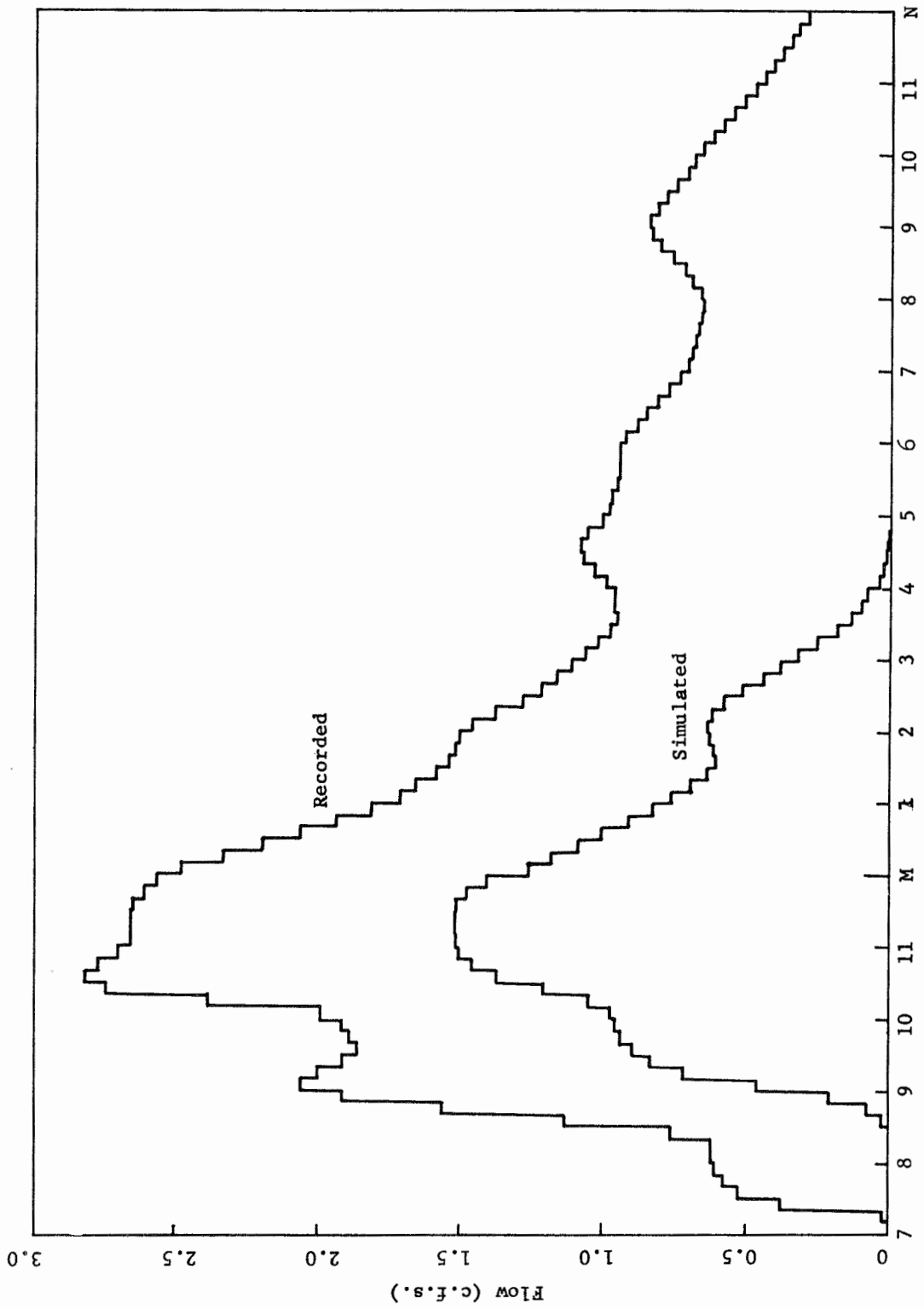


FIGURE 6.6 COMPARISON OF MEASURED AND SIMULATED FLOW FOR STORM OF NOV. 5-6, 1941

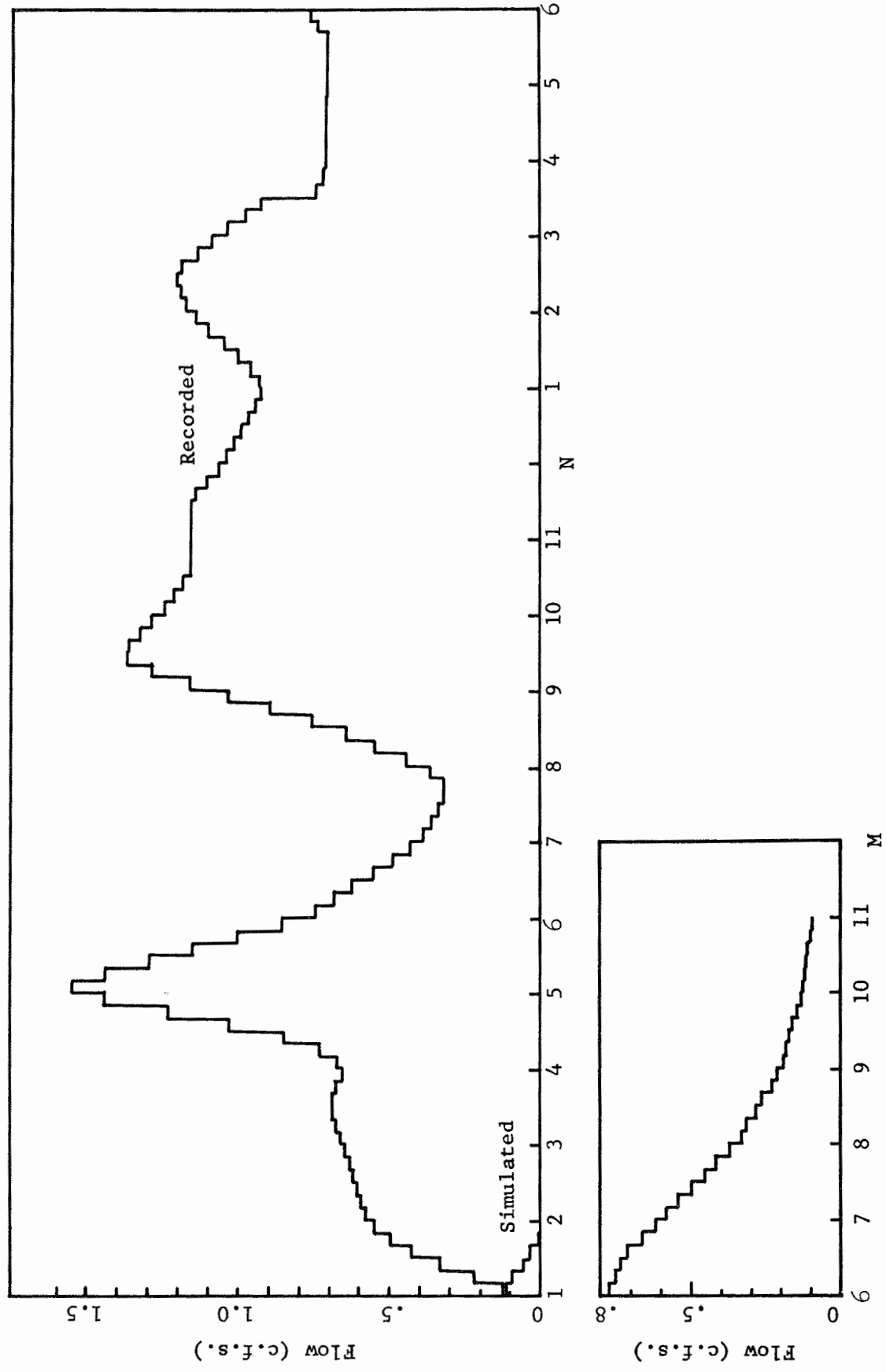


FIGURE 6,7 COMPARISON OF MEASURED AND SIMULATED FLOW FOR STORM OF FEB. 6, 1942

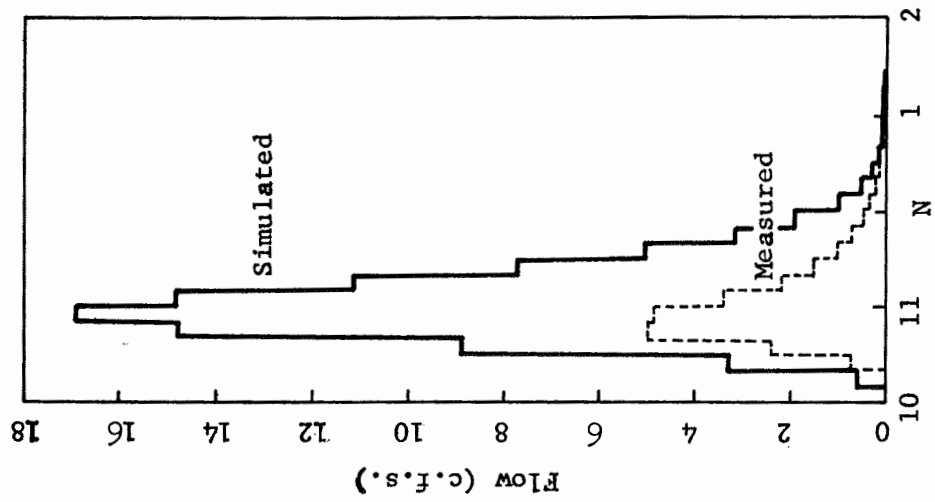


FIGURE 6.8 COMPARISON OF MEASURED AND SIMULATED FLOW FOR STORM OF JUNE 13, 1942

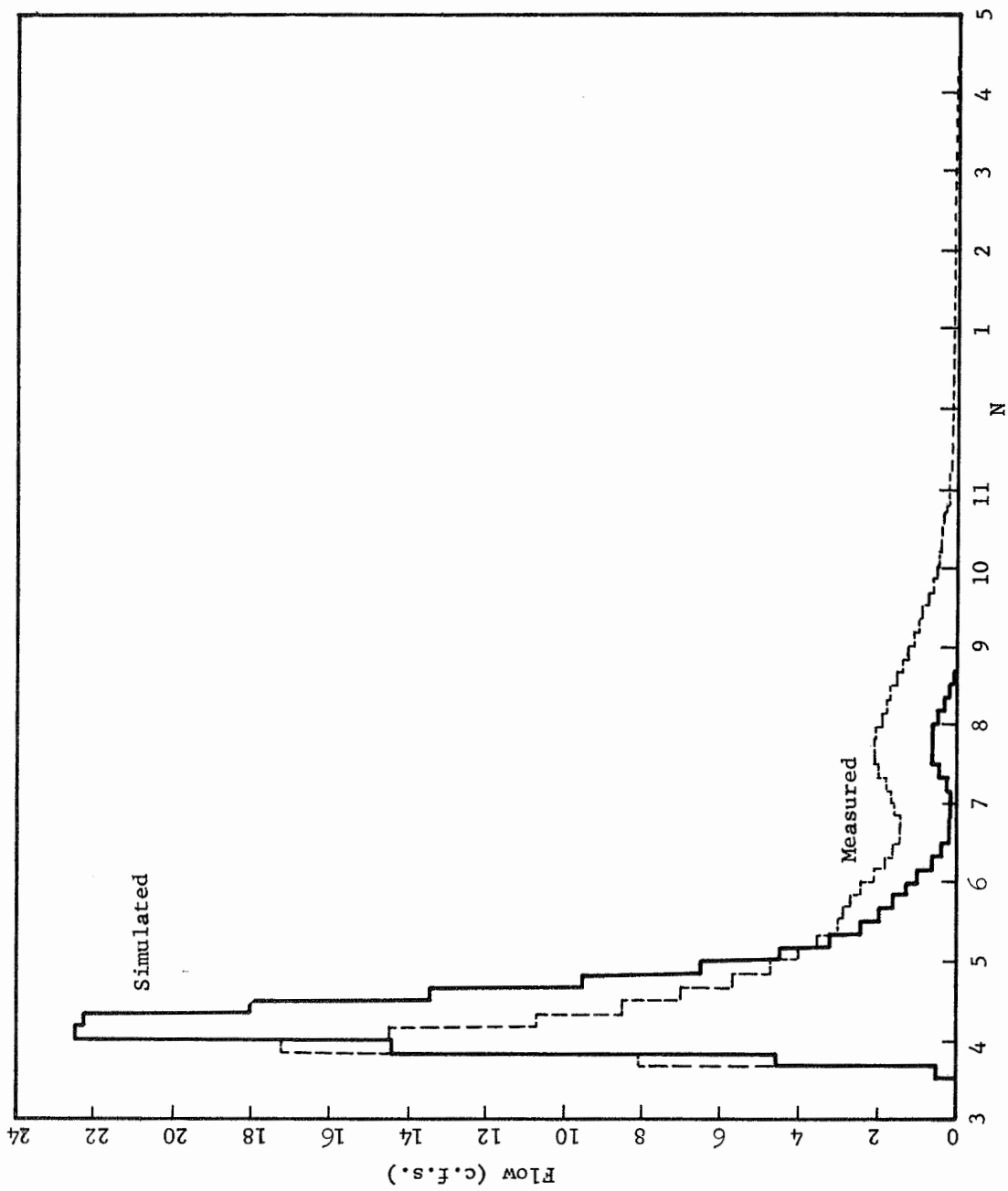


FIGURE 6.9 COMPARISON OF MEASURED AND SIMULATED FLOW FOR STORM OF JUNE 26, 1942

Part of this failure is attributable to the lack of a frozen soil facility within the model. Holtan and Minshall state that infiltrometer tests were discontinued during the winter because the soil was saturated. Figure 6.10 shows the daily maximum and minimum temperatures recorded at Greenville, Illinois, for the period November, 1941, through March, 1942. Greenville is about 30 miles east of Edwardsville. Clearly the ground was frozen throughout much of February. Fluker (1958) found soil temperature extremes to lag air temperature extremes by one month. Quite possibly the period of frozen ground extended well into March. If the ground is frozen and near saturation, virtually no infiltration, soil moisture movement, or evaporation will occur. The complexity of including the affects of temperature in the model are much greater than simply the problem of securing the daily temperatures (although this is a data preparation chore not to be overlooked). Rather the problem is in modeling the head flow process. The thermal characteristics of soils vary with the soil type and with moisture content. Also, the soil moisture freezes from the surface down; there may be no infiltration or evaporation at the surface because it is frozen while moisture transfer occurs a few inches beneath the surface below the frost line. Almost certainly some simple model such as "no evaporation, infiltration, or soil moisture movement while the air temperature is below 30^o F." would improve the performance of the model, particularly during February.

Application to Mukewater Creek Watershed

Coskun and Moore (1969) reported the results of application of the Stanford Model to the Mukewater Creek watershed in central Texas. Using

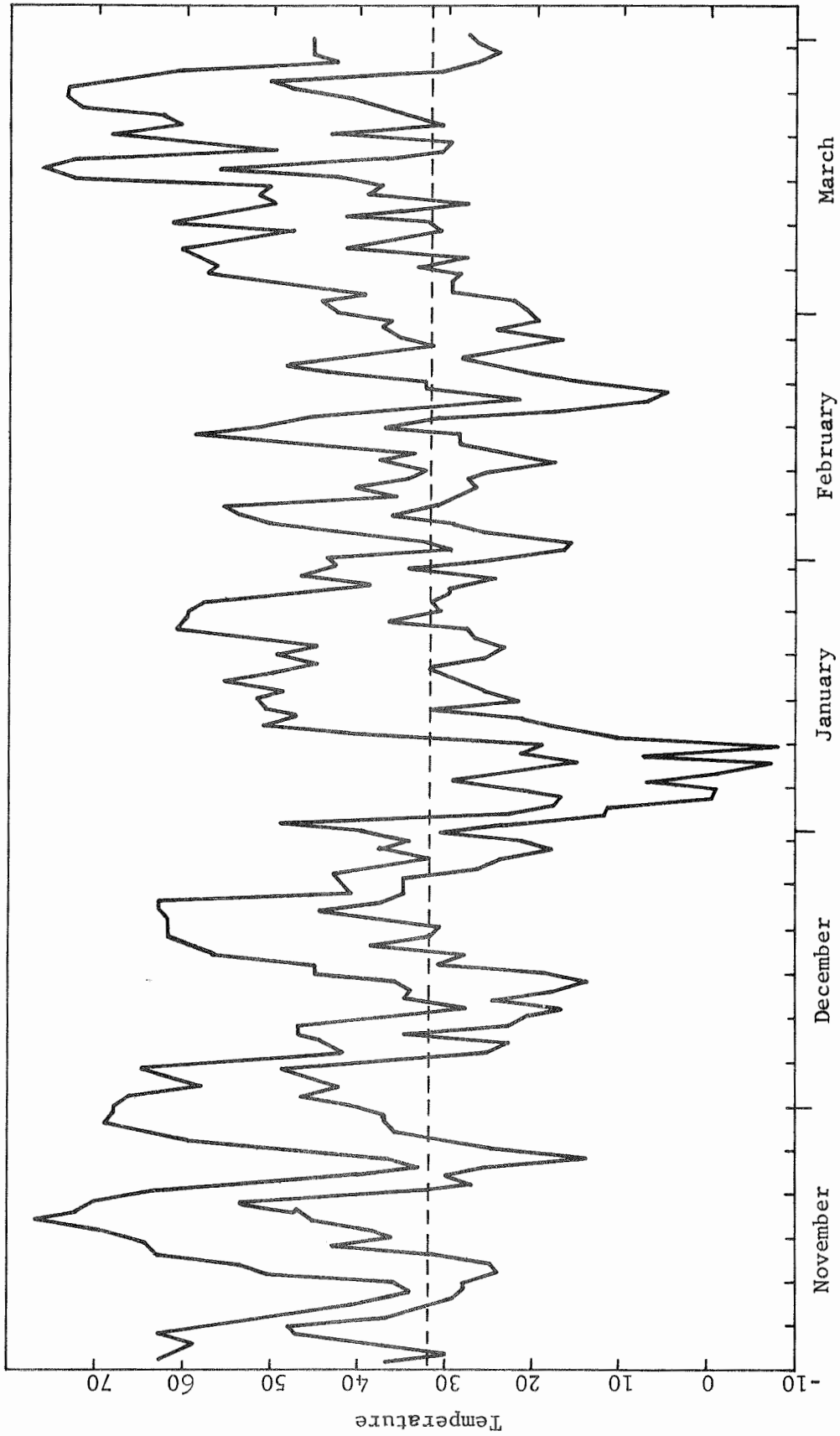


FIGURE 6.10 DAILY MAXIMUM MINIMUM TEMPERATURES AT GREENVILLE, ILLINOIS, 1941-1942

the rainfall data from their study, a similar application has been made with the proposed model.

Mukewater Creek is one of numerous small watersheds which have been studied in detail by the United States Geological Survey in co-operation with the Texas Water Development Board, and the United States Soil Conservation Service. Sauer (1965) has given the physiographic features as well as a summary of the hydrologic data for the period 1952-1960. Figure 6.11 shows the location of the watershed as well as the location of the rain gauges. The watershed contains 70.4 square miles above the stream gauging station at Trickham, Texas. About one half the watershed area is devoted to farming and the other half to ranching. Sauer reported the existence of 211 stock ponds by March, 1962. These controlled 20 percent of the drainage area. Six flood retarding structures were built between 1960-1965. These control almost 40 percent of the drainage area. Sauer concluded that areal distribution of the rain was insignificant when correlating rainfall and runoff. Coskun and Moore utilized the rainfall records from four recording rain gauges to produce a single hourly rainfall record for use in their application of the Stanford Model.

While some attempt was made to assign parameter values from available information, the emphasis in this application has been on determining the "best" set of parameters. Those parameters which are common to both the Stanford Model and to this model were assigned the values used by Coskun and Moore. Other values were assigned on the basis of the experience with the Edwardsville watershed. Only mean daily flow values are available for comparison to simulated values, except for hydrographs

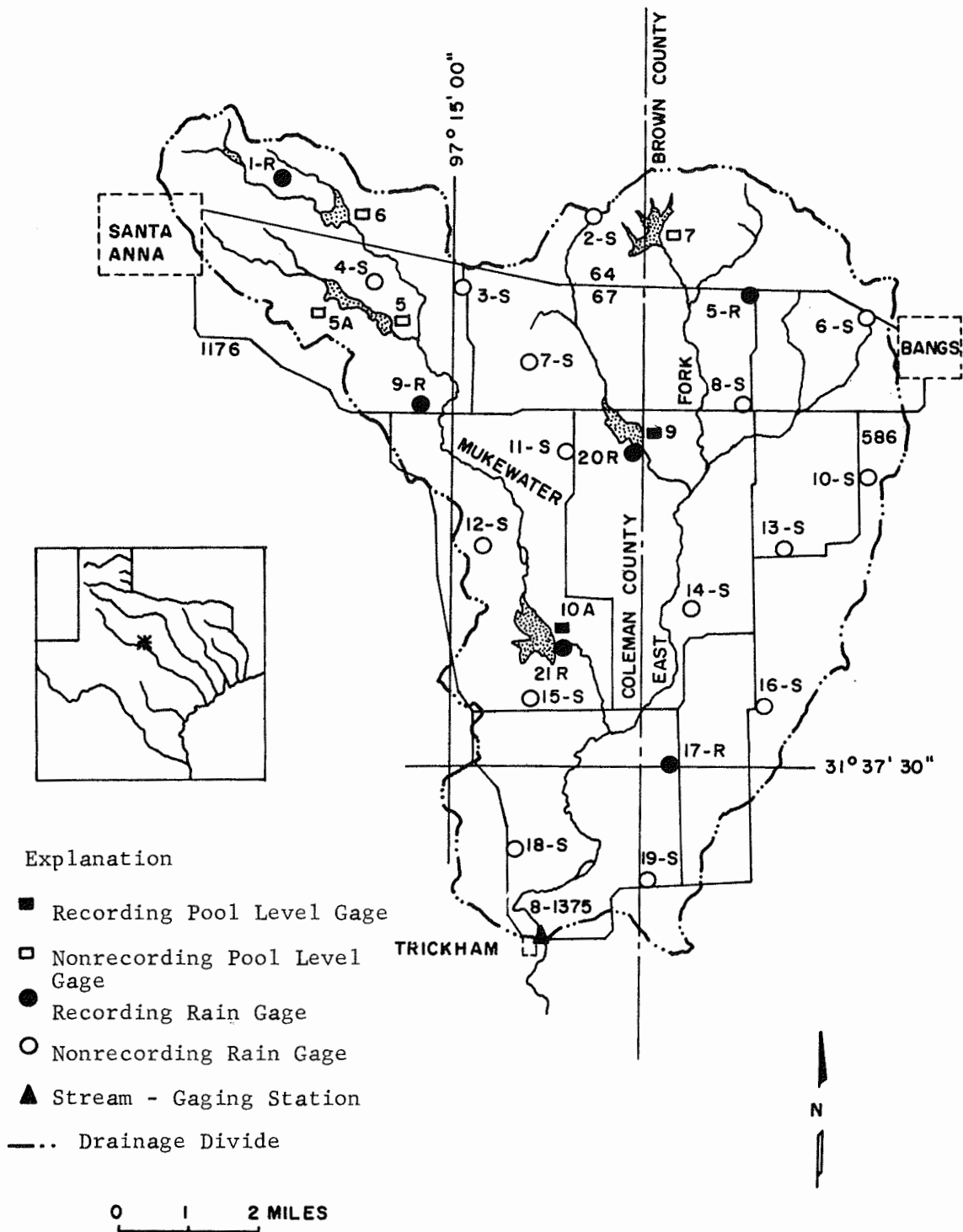


FIG. 6.11 MUKEWATER CREEK STUDY AREA

of the storms of May 11-13, 17-18, 1957 and February 22-23, 1958, which were reported by Coskun and Moore.

Results of the simulation are shown in Figures 6.12 through 6.19. Figures 6.12, 6.13, and 6.14 show the measured and simulated hydrographs for the three storms cited above. Figures 6.15 through 6.19 show a comparison of mean daily flow values for five of the more active months.

A direct comparison to the Stanford Model is difficult to make except on a statistical basis. Table 6.5 shows a comparison of the monthly runoff values as simulated by Coskun and Moore, those as simulated by this study, and the measured values. Also shown is the correlation coefficient for mean daily flows. As in the Edwardsville application, the mean daily flow series contains numerous zero values which enhance the correlation coefficient. Were it possible to choose some single measure of goodness of fit by which to judge a set of parameters, a computer program could be utilized to choose an optimum set of parameter values. However, the insensitivity of the correlation coefficient, even when the zero flow days have been eliminated, render it nearly useless for this purpose. Perhaps the most useful such measure is simply the sum of the absolute value of the errors. Such a sum has the serious drawback that all errors are weighted equally, i.e., a 10 second foot day error carries the same significance whether made on a day when the mean daily flow was 100 cfs or 1000 cfs. In the latter case, the error is of the same order of magnitude as the stream flow measurement inaccuracies, and obviously should not be considered significant. If a relative error is used instead, defined as the ratio of the error to the measured flow, an awkward situation prevails when the measured flow is zero and the simulated flow

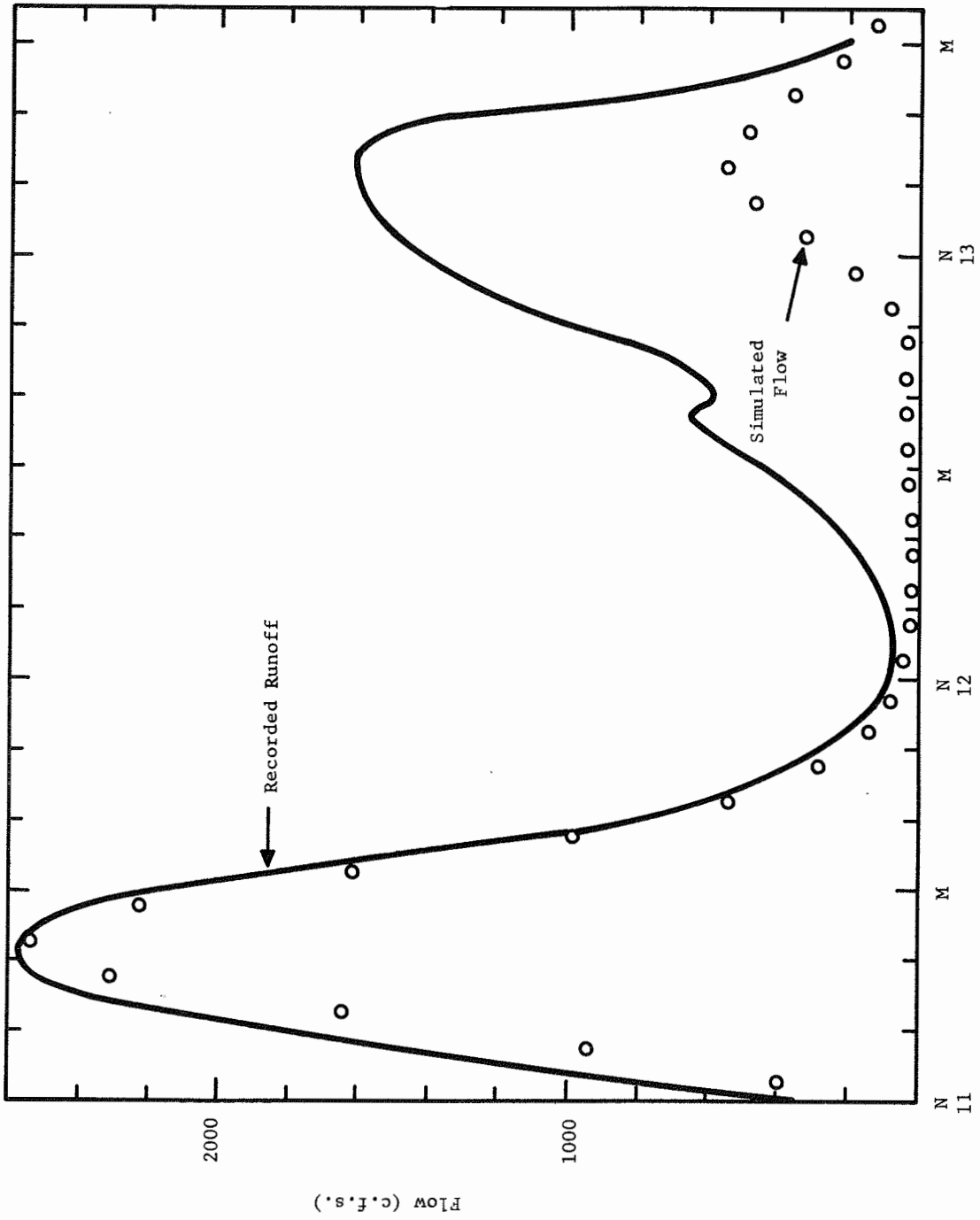


FIGURE 6.12 STORM OF MAY 11-13, 1957

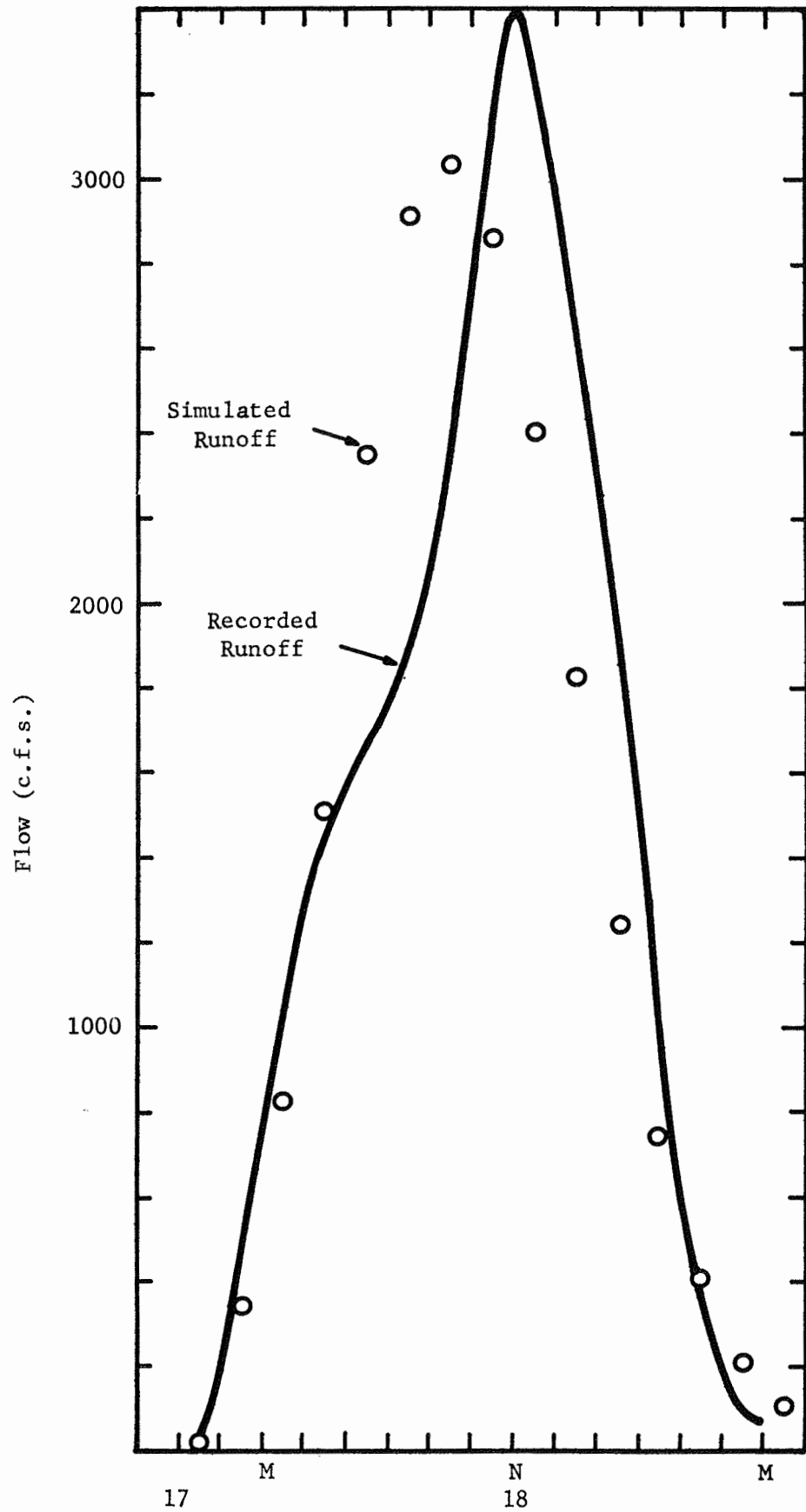


FIGURE 6.13 STORM OF MAY 17-18, 1957

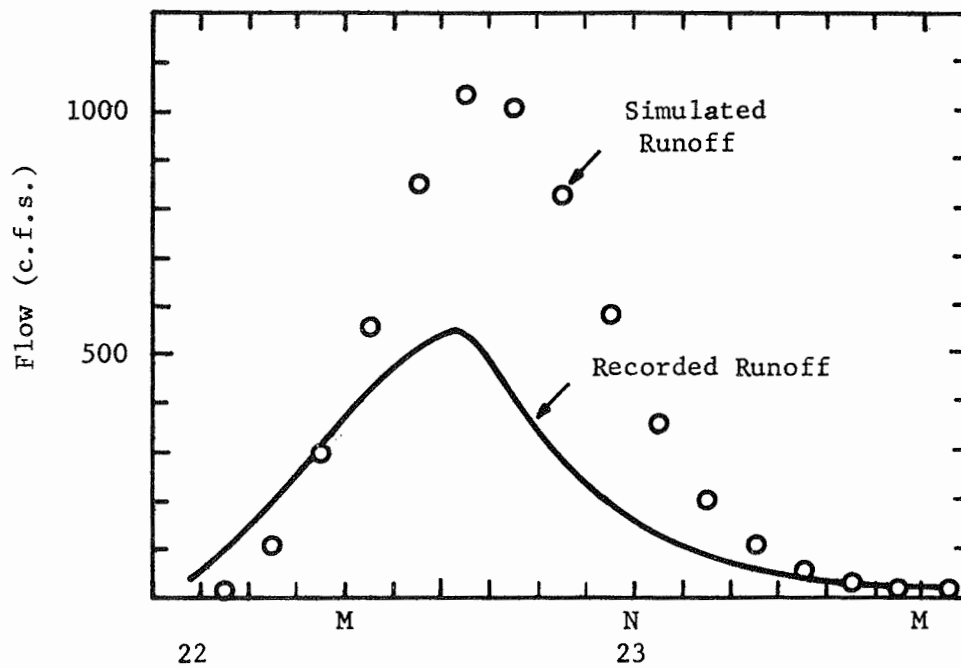


FIGURE 6.14 STORM OF FEBRUARY 22-23, 1958

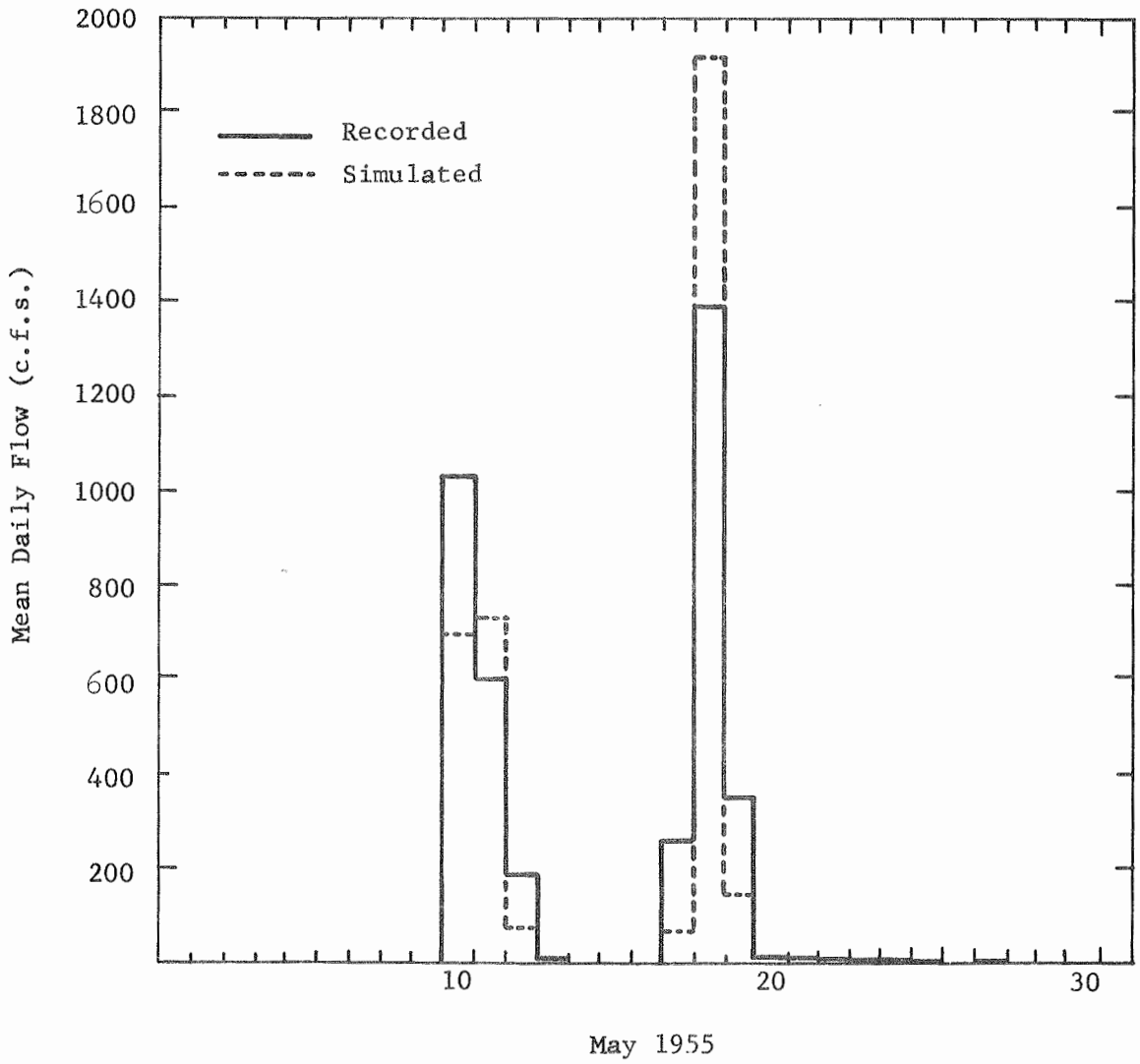
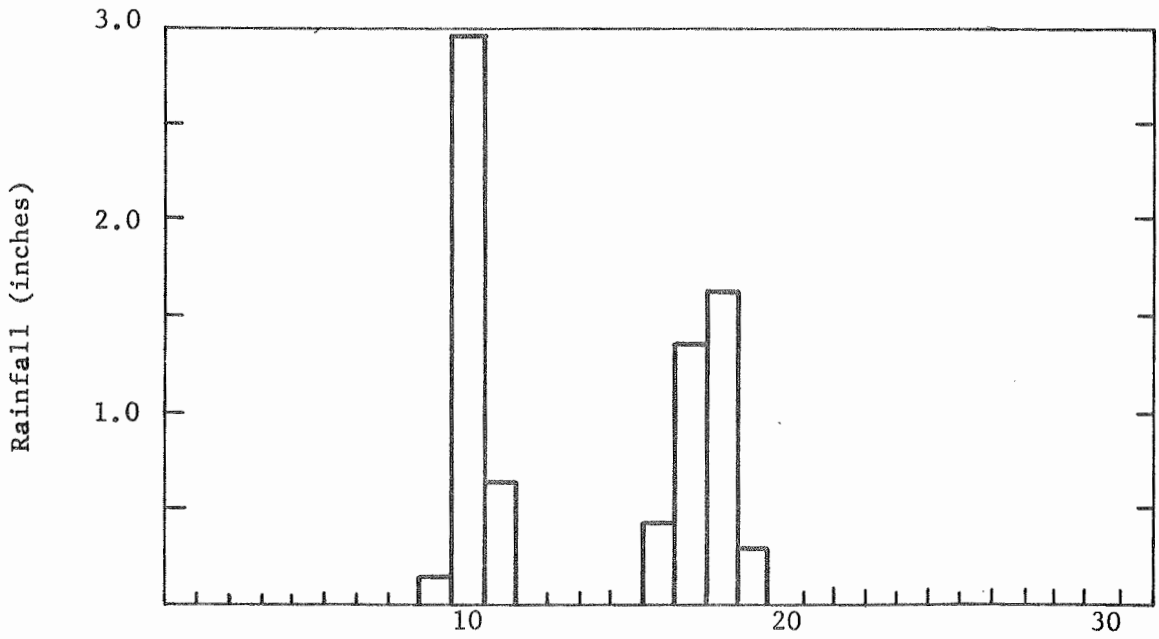


FIGURE 6.15 PRECIPITATION AND RUNOFF FOR MAY 1955

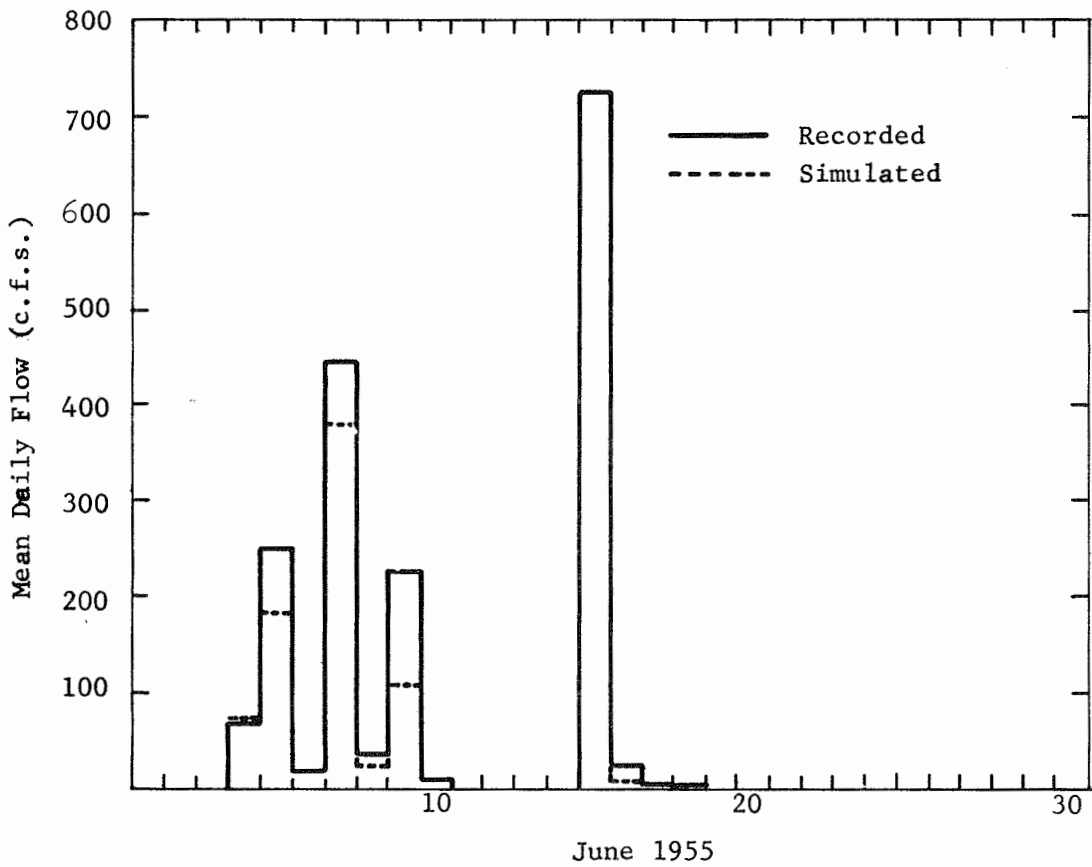
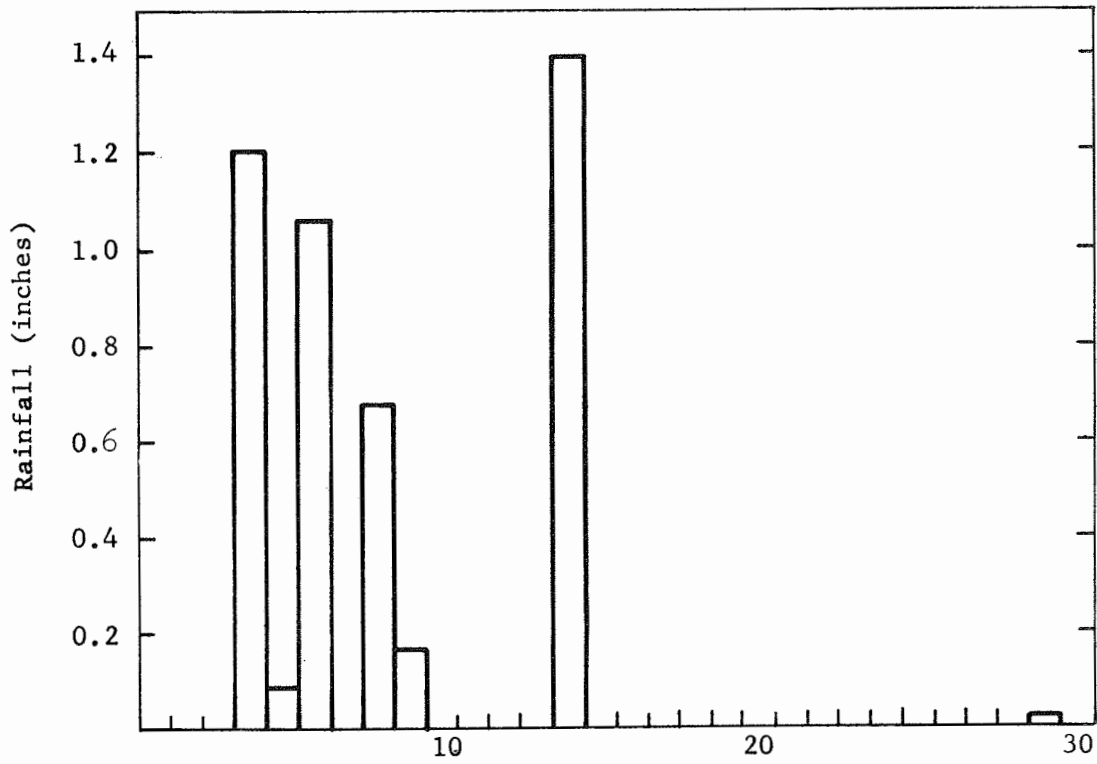


FIGURE 6.16 PRECIPITATION AND RUNOFF FOR JUNE 1955

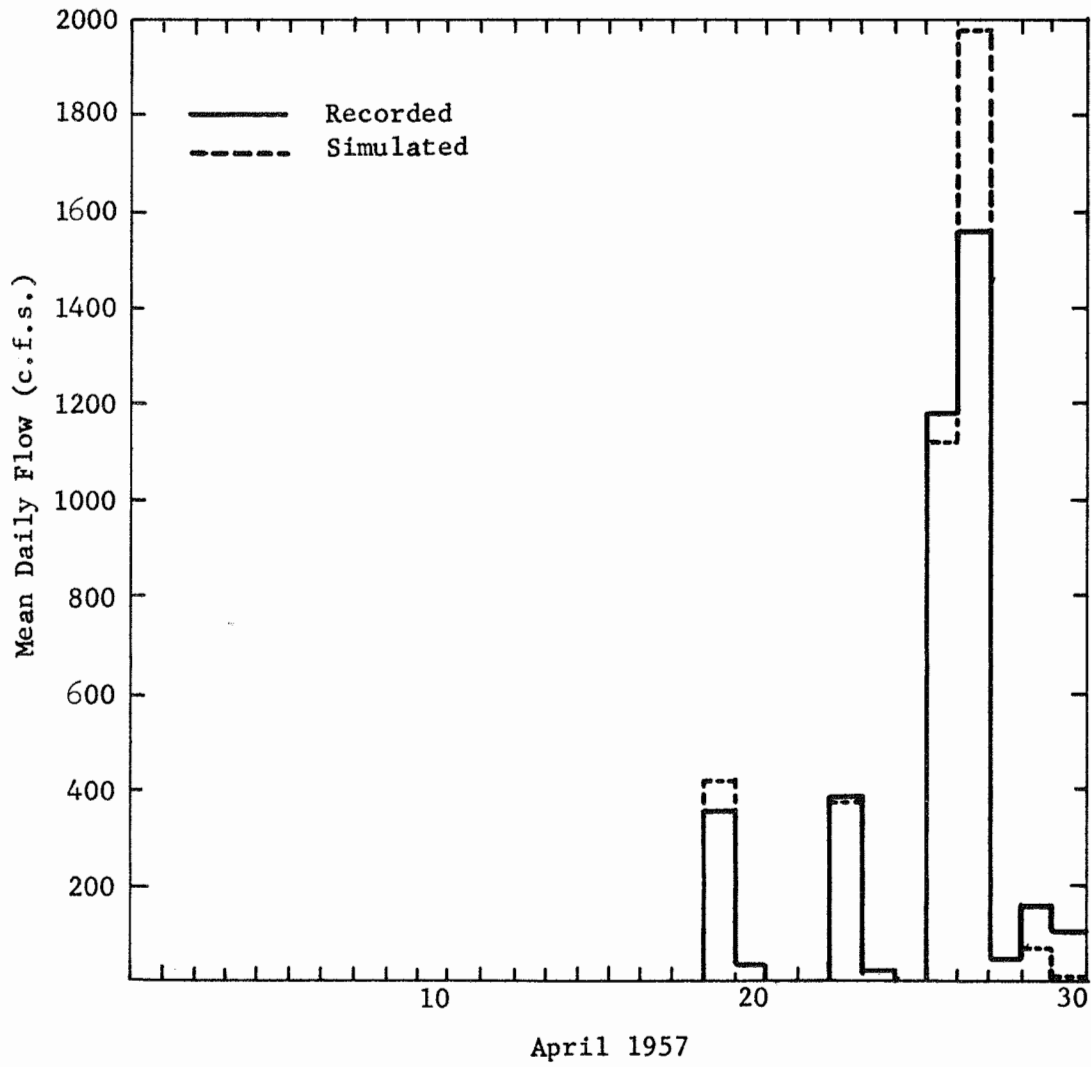
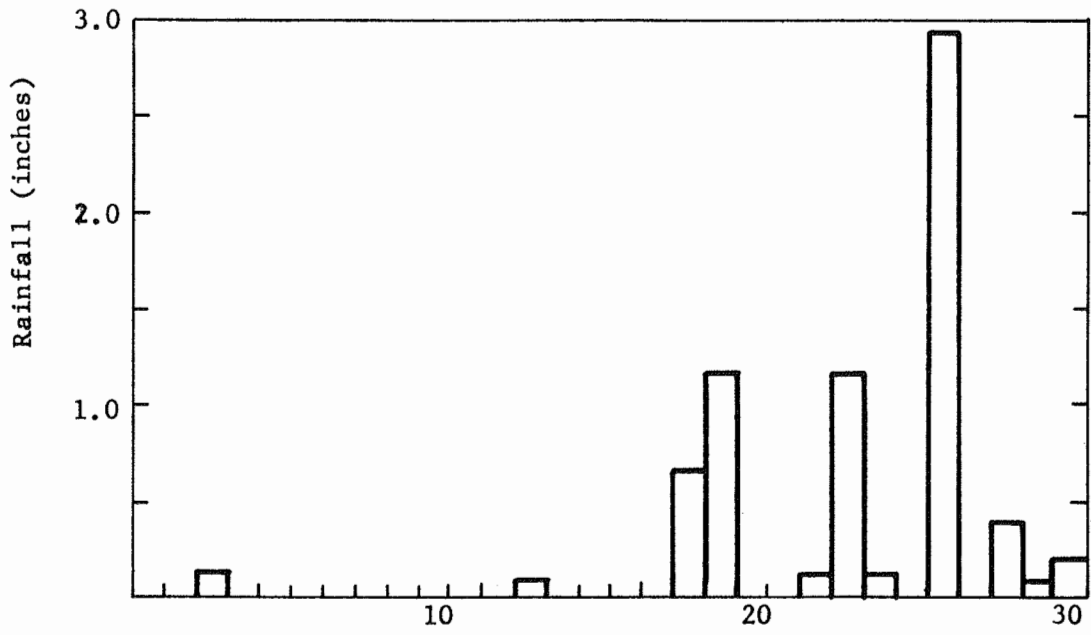


FIGURE 6.17 PRECIPITATION AND RUNOFF FOR APRIL 1957

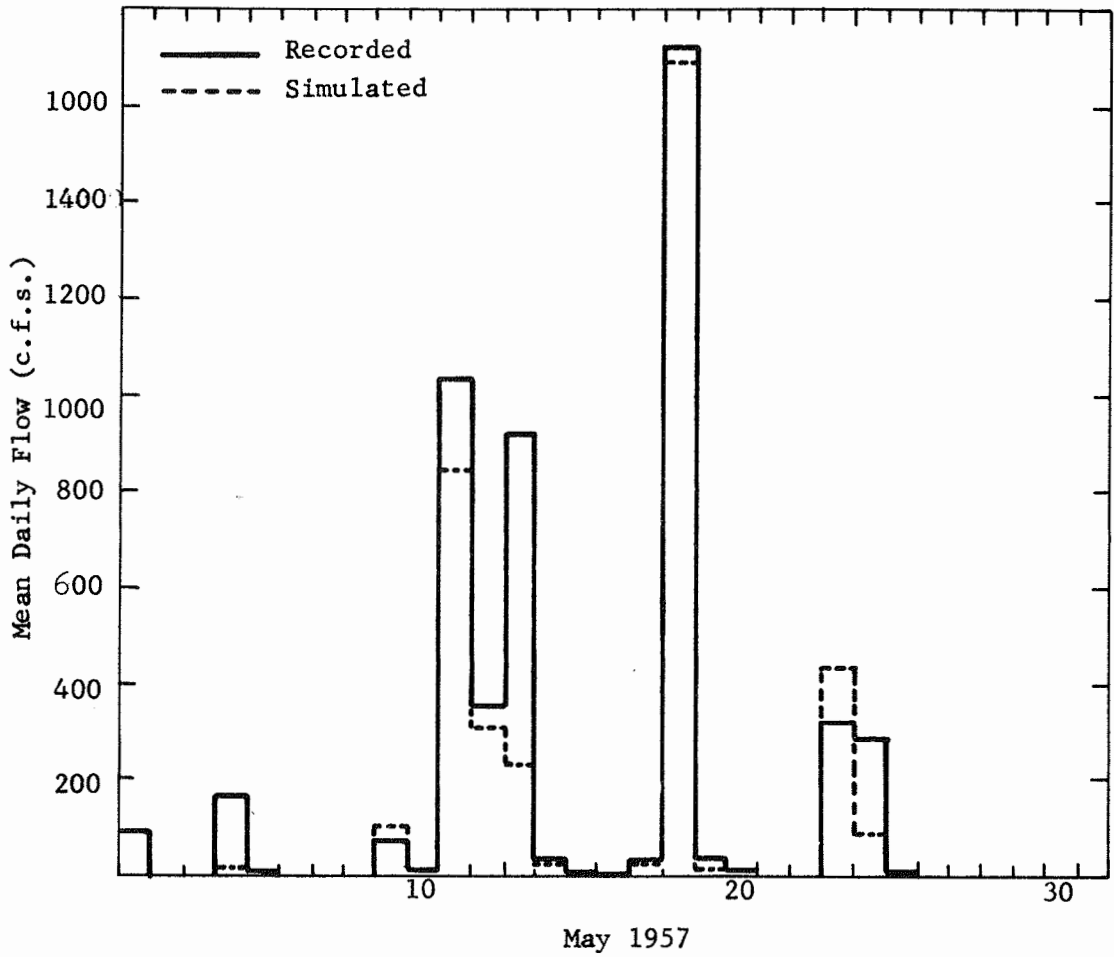
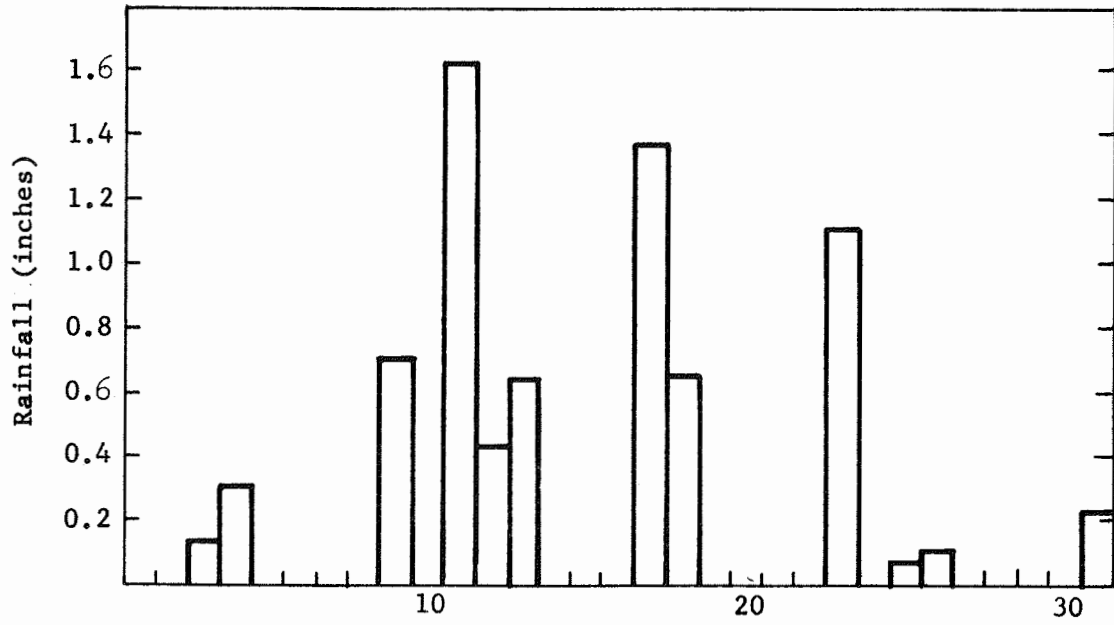


FIGURE 6.18 PRECIPITATION AND RUNOFF FOR MAY 1957

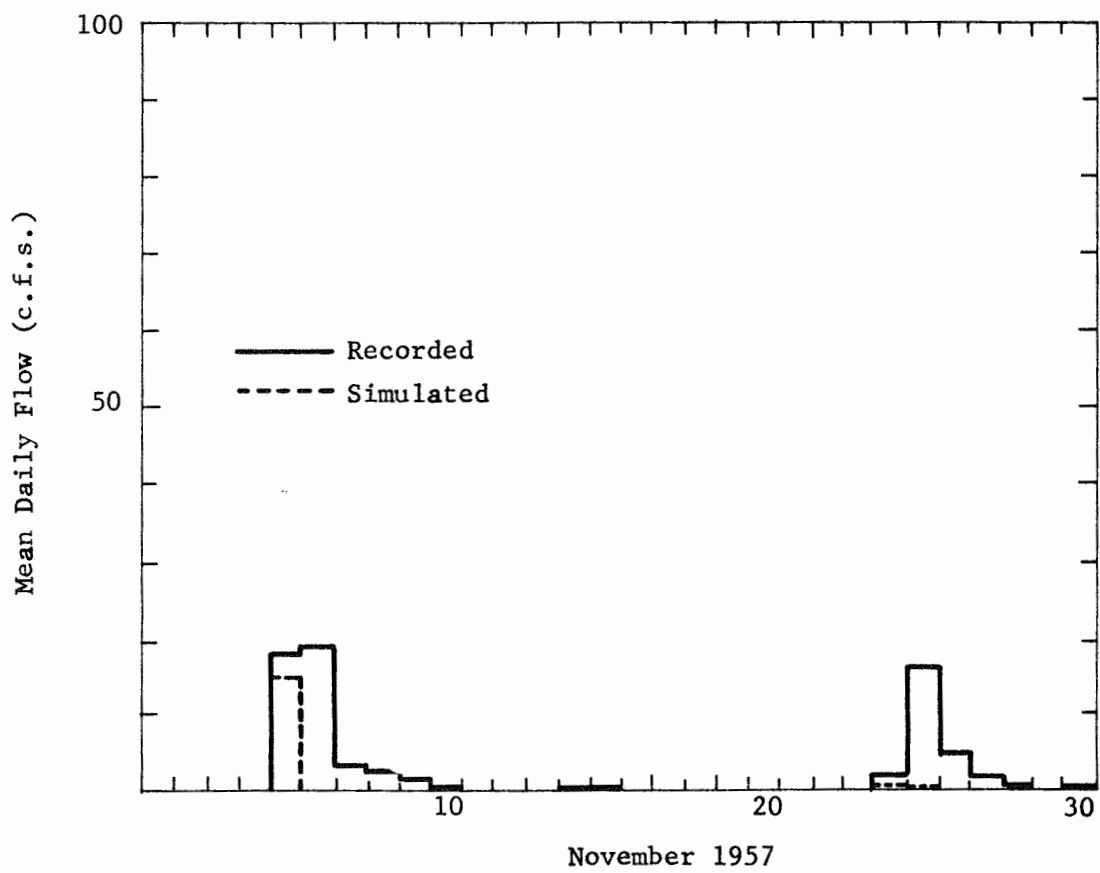
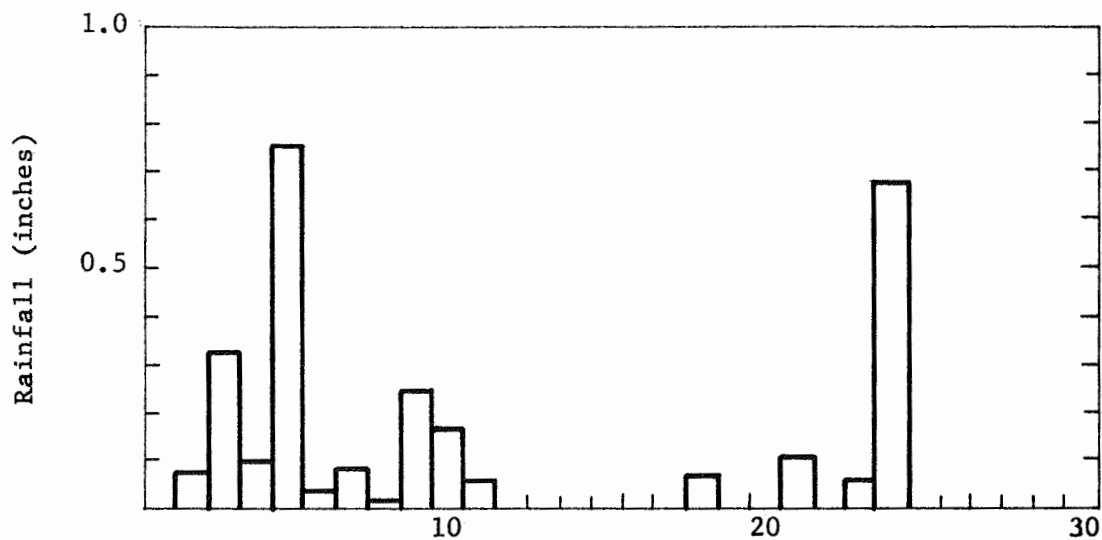


FIGURE 6.19 PRECIPITATION AND RUNOFF FOR NOVEMBER 1957

Water Year	Source	Oct	Nov	Dec	Jan	Feb	Mar	Apr	May	June	July	Aug	Sep	Annual	Correlation Coefficient
1953-1954	Recorded Stanford U. T.	0.50 0.60 .77					0.17 0.01 .03	0.25 0.18 .49	0.18 0.14 .17	0.02				1.10 0.97 1.46	0.9500 .9593
1954-1955	Recorded Stanford U. T.		0.05 0.02	0.01		0.03 0.01	0.03		2.03 1.80 1.92	0.95 0.42 0.80	0.91 1.27 .58	0.06 0.01 .01	0.49 0.53 .30	4.55 4.10 3.62	0.9318 .9434
1955-1956	Recorded Stanford U. T.							0.10 0.25	2.15 2.31					2.25 2.57	0.9970
1956-1957	Recorded Stanford U. T.	0.07 0.05 .06	0.05 0.02	0.02			0.08 0.07 .16	2.03 2.03 2.10	2.75 2.58 2.03	0.20 0.14 .30	0.01	0.01	0.01 0.02	5.19 4.96 4.65	0.9550 .9544
1957-1958	Recorded Stanford U. T.	0.12 0.18 .05	0.04 0.07 .10	0.03	0.01 0.04	0.17 0.13 .29	0.17 0.14 .06	0.04 0.09 .03	0.11 0.08 .27	0.07 0.11 .26		0.10 0.16 .14	0.03	0.83 1.06 1.11	0.9284 .7402
1958-1959	Recorded Stanford U. T.	0.01	0.01					0.01	0.01 0.04 .11	2.24 3.18 2.50	2.00 2.62 1.67	0.03	0.02	4.25 5.92 4.31	0.9766 .9792

TABLE 6.5 COMPARISON OF RECORDED AND SIMULATED RUNOFF VALUES IN INCHES

is nonzero. Both the annual correlation coefficient and the sum of the absolute value of the errors were used in arriving at the "optimal" set of parameters for Mukewater Creek.

Among those parameters which cannot be readily assigned numerical values from field data, the length of overland flow, the constants in the infiltration equation (C_1 , C_2 , and saturated permeability), and the thickness of the upper zone seem to be rather important. Some guidance can be obtained from the existing runoff data, and from current computer runs. Figure 6.20 shows the results of one such computer run. Consider a storm in which a considerable amount of rainfall fell in a short period. If no runoff was produced, essentially all the water infiltrated. The infiltration rate throughout the storm must be equal to or greater than the rainfall rate. In Figure 6.20, several of the isolated one hour storms which produced no runoff are shown by horizontal line segments. The left end of the line is plotted at the value of Upper Zone Storage which existed at the beginning of the storm. The line terminates at that value of Upper Zone Storage which would obtain if all of the rainfall were infiltrated. The infiltration curve (equation 5.12) must be an upper envelope for these line segments. Obviously, to obtain values of the Upper Zone Storage at the beginning of the storm requires the simulation of the entire period. However, unless large changes are made in the parameters from run to run, the values of Upper Zone Storage do not seem to vary over a very large range. When applied to the Mukewater Creek data, the envelope lacked the proper shape for small values of Upper Zone thickness.

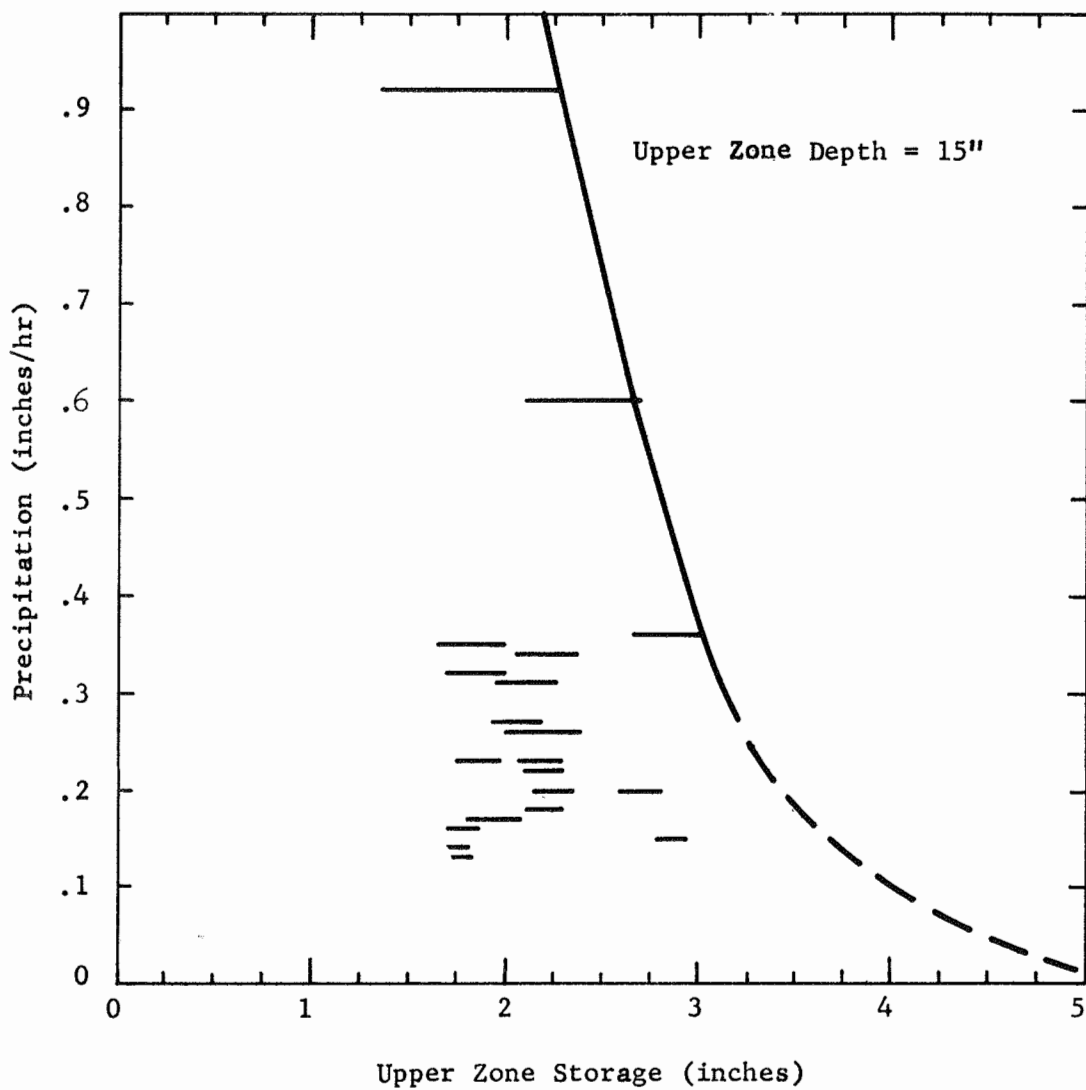


FIGURE 6.20 UPPER ZONE STORAGE FOR STORMS WITHOUT RUNOFF

After the parameters seem to be of the proper magnitude, further corrections can be determined from plots similar to Figure 6.21. The current values of C_1 , C_2 , Upper Zone thickness, and saturated permeability are used to plot the infiltration rate as a function of Upper Zone Storage. Each storm is then examined to determine whether a larger or smaller infiltration rate is required to obtain agreement between simulated and recorded values. In Figure 6.21, the results are indicated by an arrow showing the required direction of change for that storm. The results of such a plot may be that some segment of the infiltration curve must be shifted. Figure 6.22 shows the result of varying C_1 ; Figure 6.23 shows changes due to various values of C_2 ; while Figure 6.24 indicates the affects of depth of the Upper Zone when infiltration is not limited by rainfall. Such curves aid in choosing a new parameter value.

As stated earlier, Sauer reported no significant increase in correlation of runoff and rainfall when areal distribution of rainfall was considered. This conclusion was based on a comparison of recorded runoff and predicted runoff. To determine the predicted runoff, Sauer constructed a coaxial correlation graph using the weighted rainfall. To test the effects of areal distribution of rainfall, he utilized this coaxial correlation graph on each of the 19 segments and added the resulting runoff values. However, he did not compute API (antecedent precipitation index) values for each segment; instead he assumed a uniform value throughout the watershed. Possibly quite different results might be obtained if API were allowed to vary throughout the watershed. An examination of those storms for which this model gave poor results suggests some correlation with those storms which exhibited quite noticeable areal distribution. Quite likely, Mukewater Creek is too large to be treated as a homogeneous unit by the model.

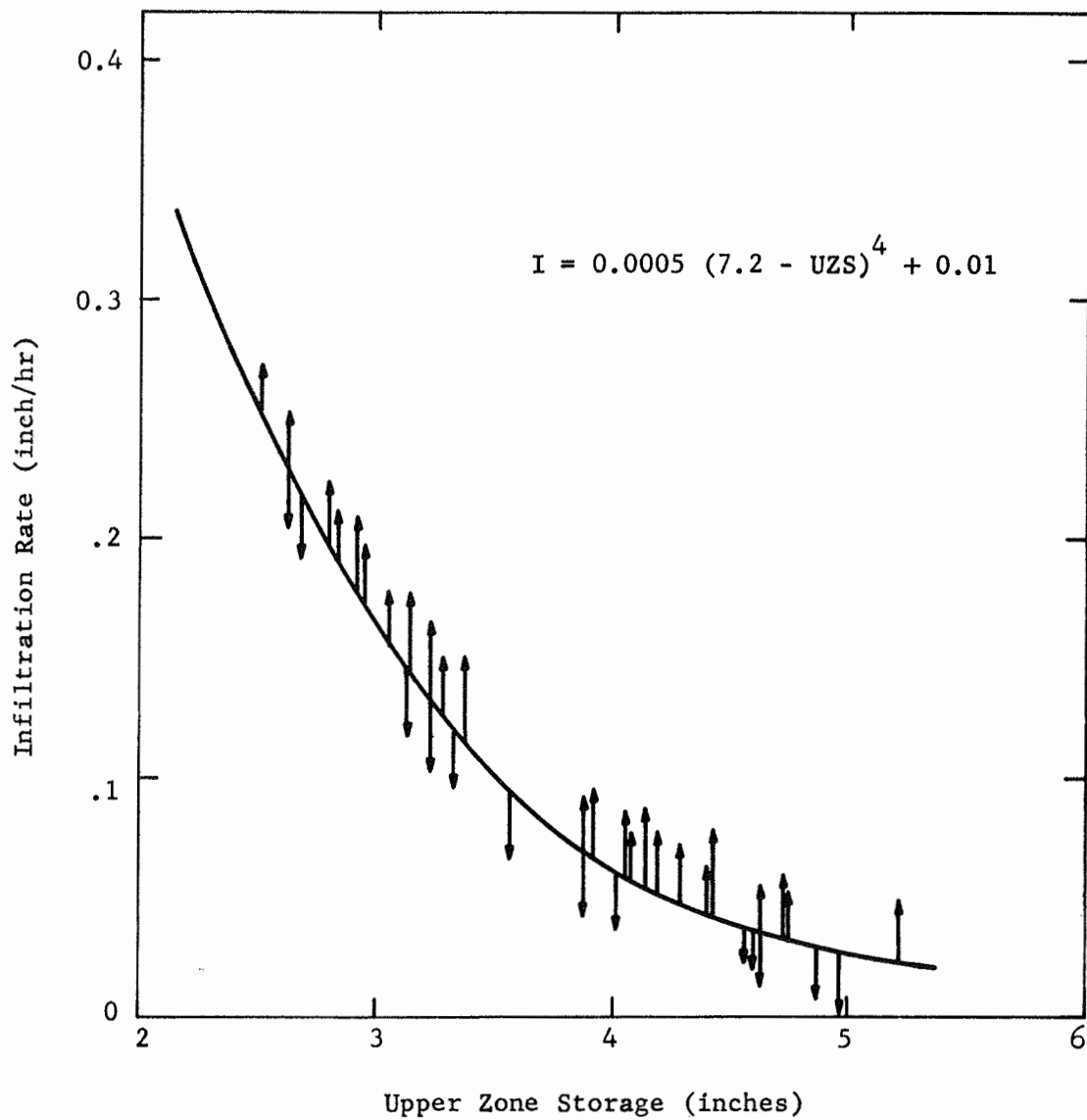


FIGURE 6.21 CHANGES NEEDED IN INFILTRATION RATE

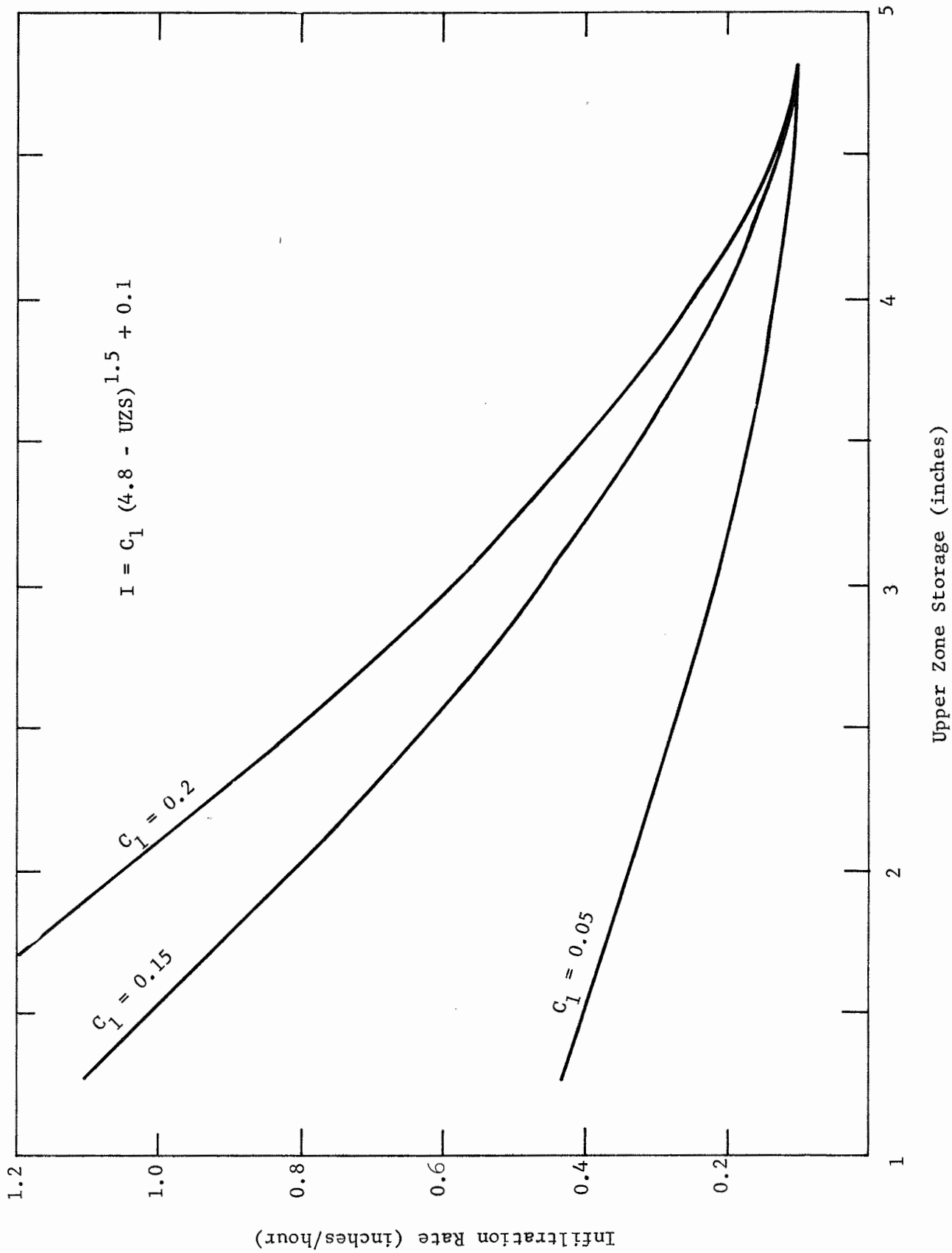


FIGURE 6.22 INFILTRATION RATE FOR VARIOUS VALUES OF C_1

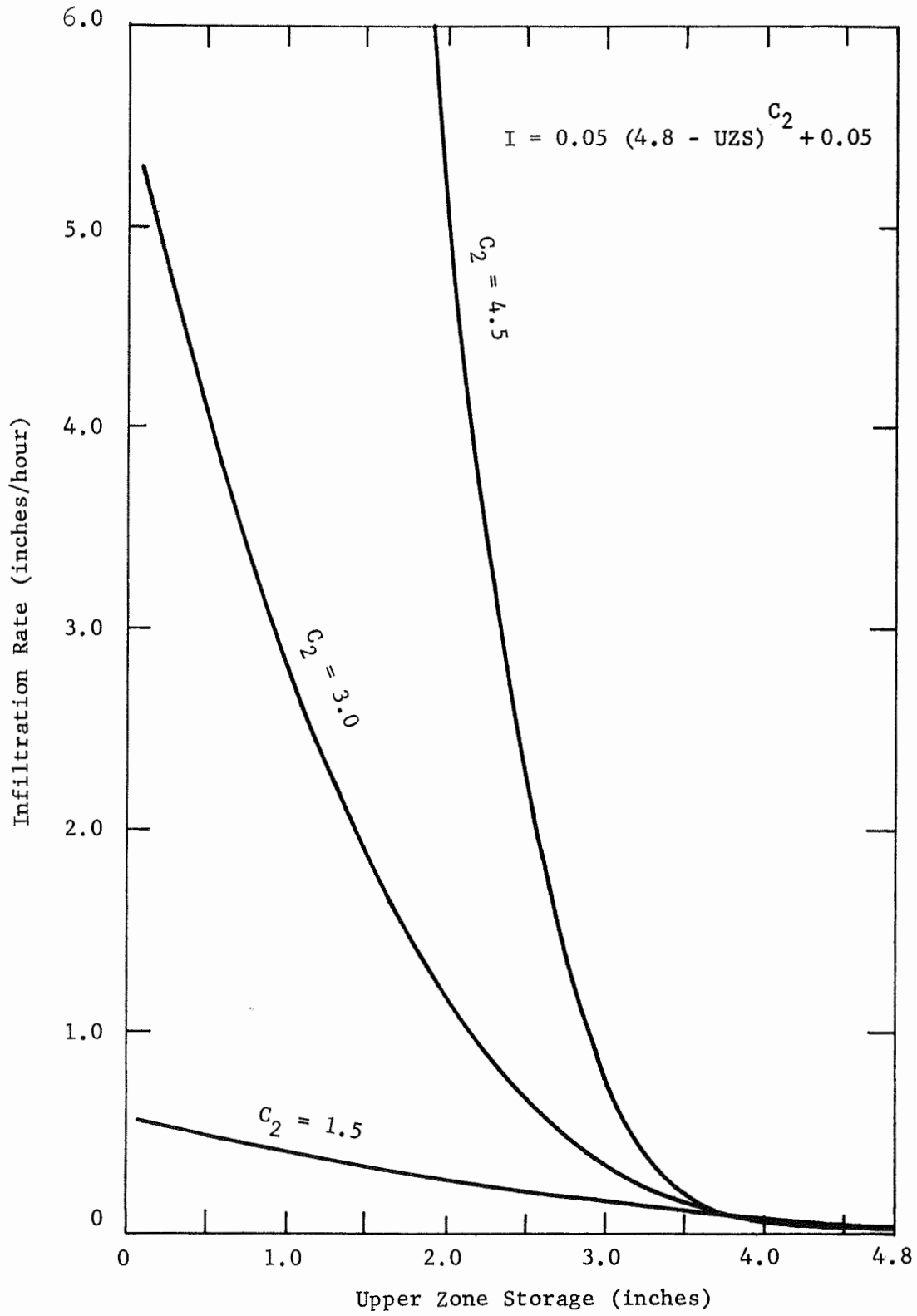


FIGURE 6.23 INFILTRATION RATE FOR VARIOUS VALUES OF C_2

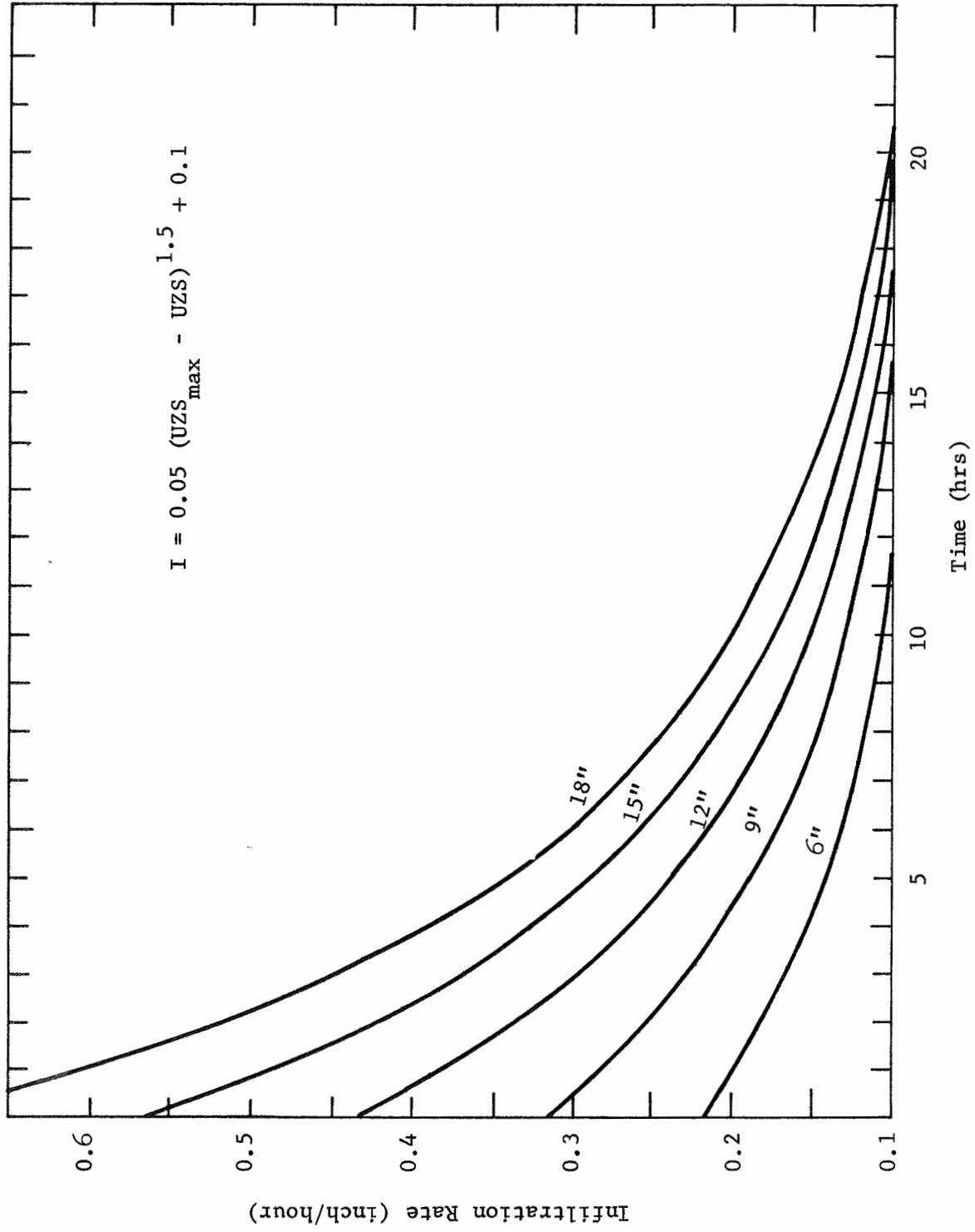


FIGURE 6.24 EFFECTS OF DEPTH OF UPPER ZONE ON INFILTRATION RATES

Chapter 7

CONCLUSIONS AND AREAS OF FUTURE RESEARCH

The validity of the various concepts incorporated into the model cannot be thoroughly tested by application to two watersheds. While the results of the application to the Edwardsville watershed were not as good as expected, those from the Mukewater Creek watershed are considerably more encouraging. Each of the 42 input parameters either has exact physical meaning or is an index of a physical quantity. The parameters in the latter category have been held to a minimum. There remain, however, many unsolved problems when modeling a watershed. Foremost of these problems is the spatial variation in most of the parameters. The Stanford Watershed Model attempted to solve this by assuming a linear distribution across the watershed. However, this is equivalent to assuming some average value of the parameter, such as has been done in the proposed model. The work reported by Holtan and Lopez (1969) uses three zones of differing characteristics to account for the spatial variation. Quite possible, as digital computers become larger, the watershed can be modeled in smaller segments, thereby achieving a greater degree of homogeneity in each subarea.

The simulation of the infiltration process needs further improvement. While Holtan's approximation is more closely related to the physical phenomenon than other empirical equations, research using finite difference methods to model the flow in both the saturated and the unsaturated zones should yield even better approximations.

The Mukewater Creek application indicates a serious deficiency in the manner in which the interflow is handled. The classical, but empirical recession constant has not been utilized because it lacks physical significance. The Stanford Model does use this method, and the results are quite apparent in the correlation coefficients for 1957-1958 (Table 6.5). This water year contained twice as many (109) days of recession type flow as did any other year. Were these portions of the hydrograph closely modeled, the correlation coefficient would improve considerably. Future research needs to be directed toward defining a physically meaningful interflow model.

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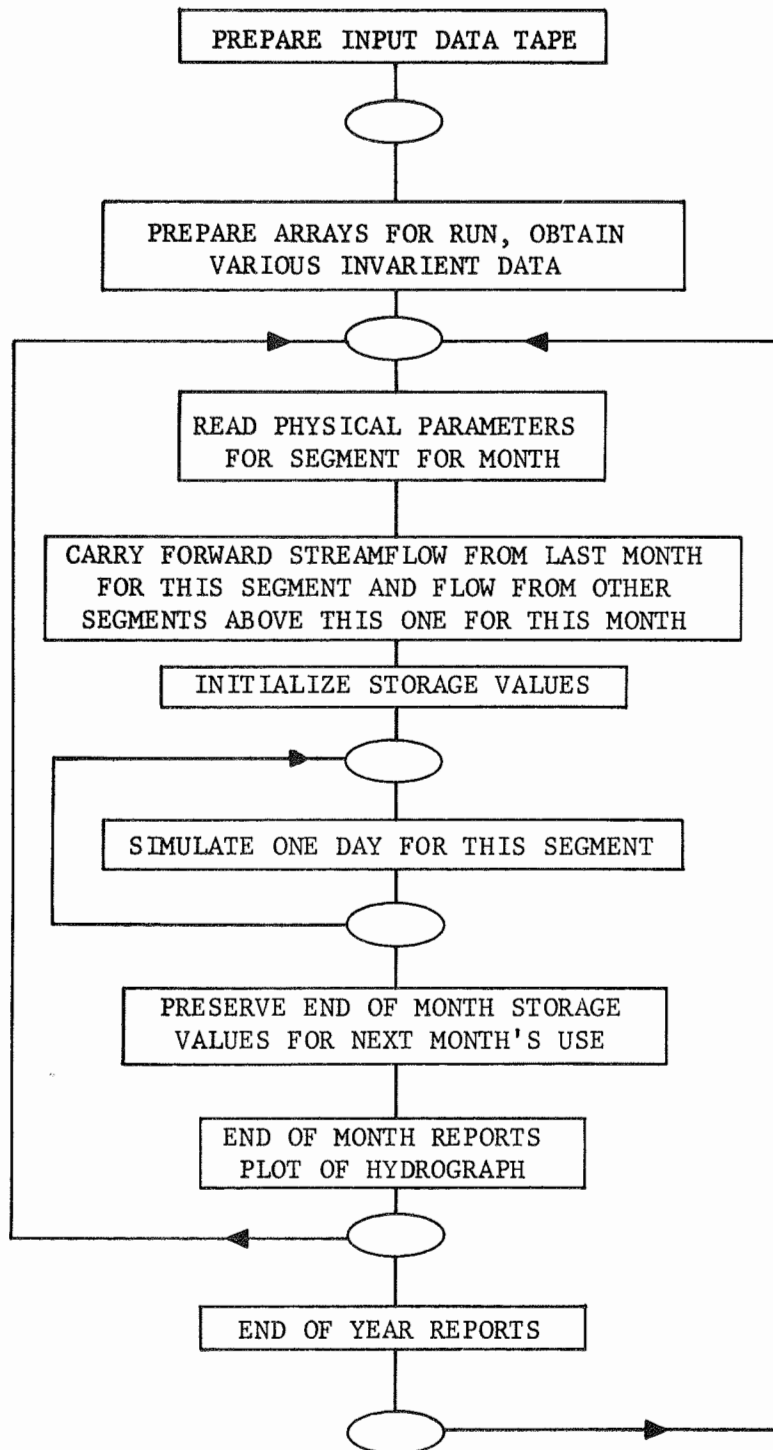
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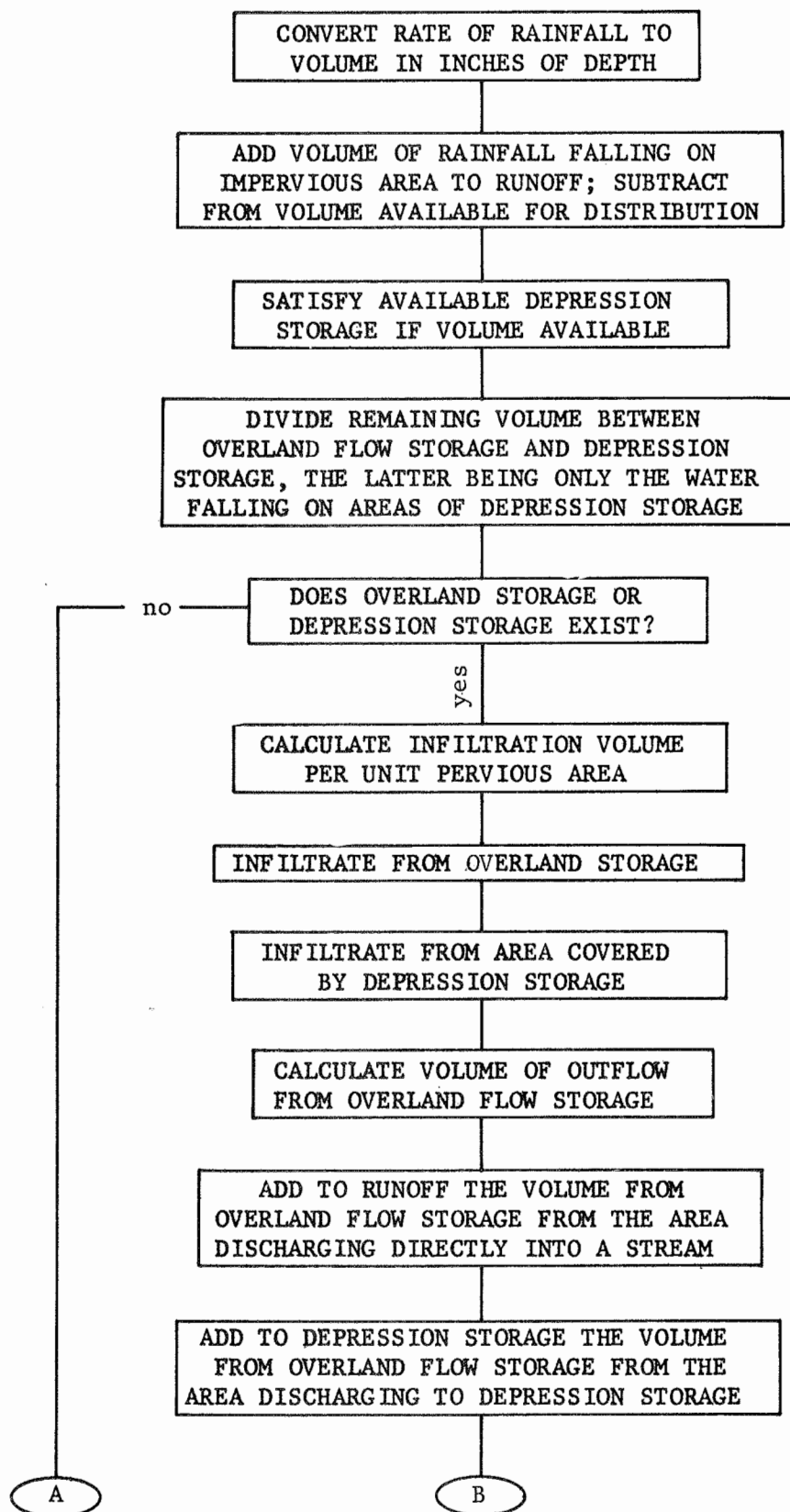
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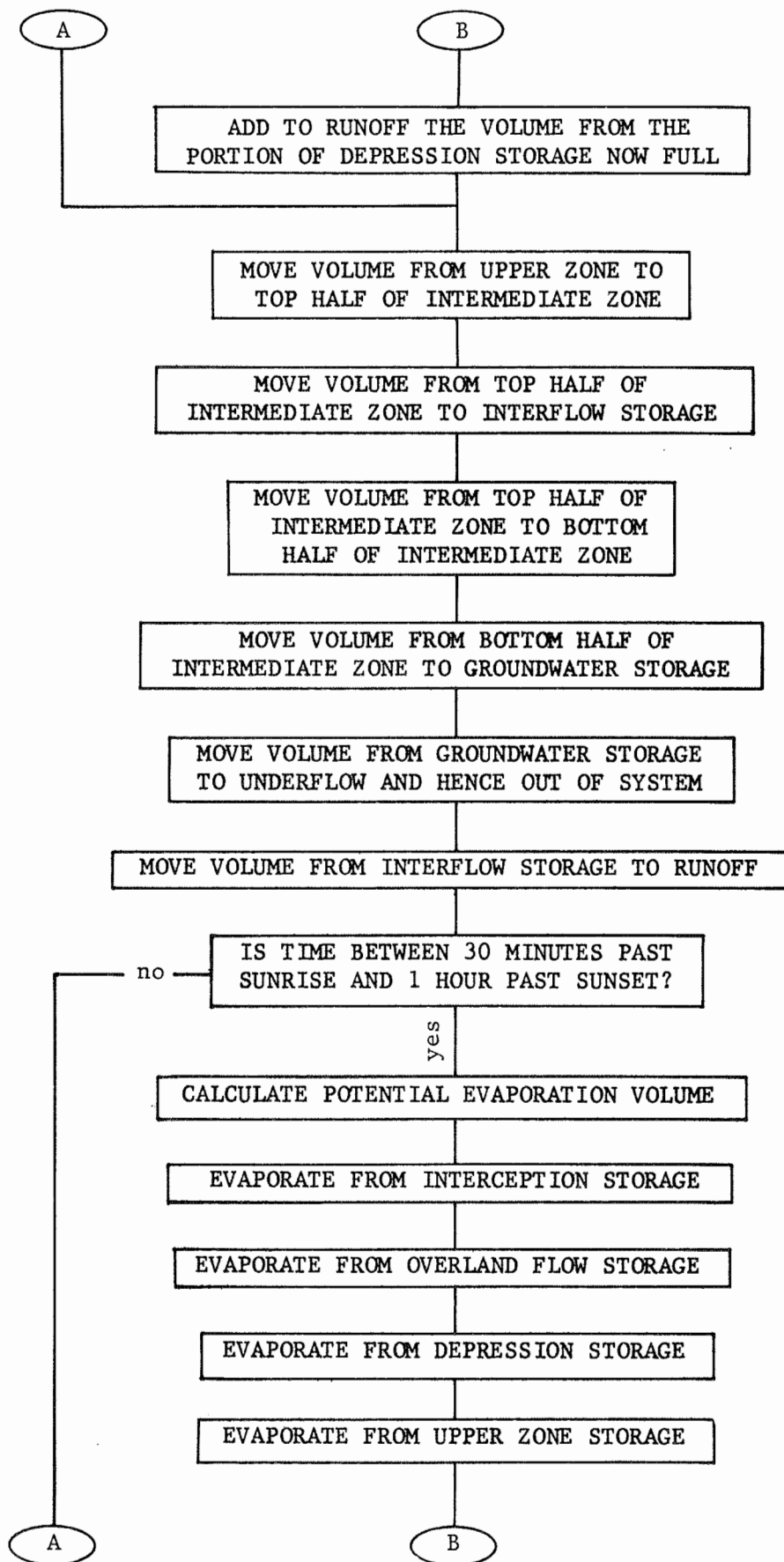
APPENDIX I

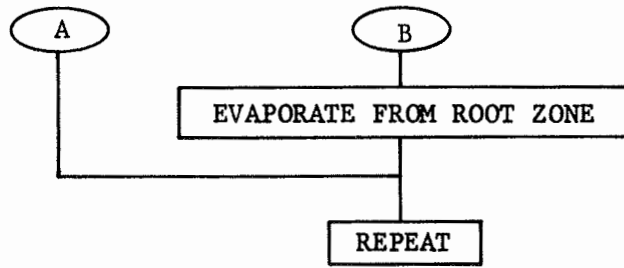
MAJOR STEPS IN COMPUTER PROGRAM



DETAILED FLOW CHART







1	DIMENSION	AD(38)	TWM	2
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3	DIMENSION	CARRYO(10,20),C(20),CHNLG(10)	TWM	4
4	DIMENSION	DELIN1(10), DS1(10), DAILYF(31)	TWM	5
5	DIMENSION	EVAP1(12), EVAP2(12), EVAP3(12)	TWM	6
6	DIMENSION	EOMINC(10,12), EOMSUR(10,12), EOMU7(10,12), EOMINT(10,12)	TWM	7
7	1)		TWM	8
8	DIMENSION	EOMGW(10,12), EOMINF(10,12)	TWM	9
9	DIMENSION	FLOINT(25)	TWM	10
10	DIMENSION	GWS1(10)	TWM	11
11	DIMENSION	ICASE(10,25), JPREC(10), IRECOD(10)	TWM	12
12	DIMENSION	JDPH(12)	TWM	13
13	DIMENSION	JPL0T(120)	TWM	14
14	DIMENSION	NAMSEG(19), NEWVAL(12)	TWM	15
15	DIMENSION	OLS1(10)	TWM	16
16	DIMENSION	PREC(1440)	TWM	17
17	DIMENSION	SIMFLO(1440)	TWM	18
18	DIMENSION	SWEIFL(10)	TWM	19
19	DIMENSION	SPAIN(10,12), SEVAP(10,12), SSF(10,12), SOLF(10,12)	TWM	20
20	DIMENSION	SGWF(10,12), SIF(10,12), SUF(10,12), SUMERR(10,25)	TWM	21
21	DIMENSION	SABSER(10,25), SERRSQ(10,25), SUMX(10), SUMY(10)	TWM	22
22	DIMENSION	REFCLO(1440)	TWM	23
23	DIMENSION	TIZS1(10),TRANPO(12)	TWM	24
24	DIMENSION	U7S1(10)	TWM	25
25	DIMENSION	VINST1(10)	TWM	26
26	DIMENSION	WFIGH(25)	TWM	27
27	DIMENSION	XX(10), XY(10)	TWM	28
28	DIMENSION	YY(10)	TWM	29
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30		1014,.0698,.1361,.1986,.2557,.3057,.3472,.3789,.3996,.4087,.4058,.3TWM	TWM	31
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		3,-.2208,-.2769,-.3253,-.3642,-.3917,-.4066,-.4080,-.3954/ DATA FLOINT/0.,1.,1.6,2.7,4.5,7.4,12.2,20.1,33.1,54.6,90.,148.4,24TWM	TWM	33
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		26315.5,59874.1,98715.8/ TWM	TWM	35
			TWM	36
			TWM	37


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C RECORD IS WRITTEN ON TAPE 3. IF DATA NEEDED FOR ANOTHER TWM
C SEGMENT, THIS IS WRITTEN ON TAPE 4. TWM
C***** TWM
C IF (ISTGAG) 110,110,100 TWM
C CALL DATAMG (PREC,IPREC,WFSG,IDAYS,MONTH,JYR,KSEGMT) TWM
C***** TWM
C DATAMG READS THE STORAGE GAUGE RECORD FROM CARDS AND THE TWM
C RECORDER RECORD FROM TAPE 3 AND MERGES THEM INTO THE RECORD TWM
C FOR THIS SEGMENT. THIS RECORD IS THEN WRITTEN ON TAPE 3. TWM
C***** TWM
C IF (IRECOD(KSEGMT)) 120,130,130 TWM
C READ 2280, JRINT TWM
C CALL DATAIN (RECFLD,MONTH,JYR,JRINT,0,KSEGMT,KSEGMT,IDAYS,0,ISEGMT) TWM
C***** TWM
C PRINT 2340, KSEGMT, MONTH, JYR TWM
C GO TO 20 TWM
C***** TWM
C PRECIPITATION TAPE COMPLETE TWM
C***** TWM
C REWIND 3 TWM
C REWIND 4 TWM
C IFIRST=1 TWM
C ISCRTH=1 TWM
C IOLD=4 TWM
C INEW=2 TWM
C READ 2290, IRUN TWM
C IF (IRUN-1) 150,160,150 TWM
C CALL EXIT TWM
C***** TWM
C BEGIN SIMULATION RUN TWM
C***** TWM
C UNDFLO=0. TWM
C READ(3) (IRECOD(I),I=1,ISEGMT) TWM
C***** TWM
C STATISTICAL CLASS DEFINITION TWM
C***** TWM

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124 DO 250 I=1,1440
125 SIMFLO(I)=0.
126 PREC(I)=0.
127 CONTINUE
250 *****
C ***** NEW CALENDER YEAR
C *****
C ***** IF (MONTH=13) 280,260,280
260 JDTD=0
128 MONTH=1
129 JYR=JYR+1
130 JDPY=365
131 *****
132 C ***** LEAP YEARS
C *****
C ***** IF (MOD(JYR,4)) 280,270,280
133 JDPY=366
134 IDAYS=JDPM(MONTH)
135 IF (MONTH=2) 310,290,310
136 IF (MOD(JYR,4)) 310,300,310
137 IDAYS=29
138 *****
C ***** RUN EACH SEGMENT THROUGH ONE MONTH
C *****
C ***** IF (NEWVAL(MONTH)+IFIRST) 330,330,320
139 READ 2310, KSEGMT,JRINT,ISEG1,ISEG2,NRELEM,LAG,(C(I),I=1,NRELEM)
140 ISI7E=1440/JRINT
141 IPREC=JPREC(KSEGMT)
142 YESDAY = CHNLG(KSEGMT)
143 DO 340 I=1,1440
144 SIMFLO(I)=0.
145 CONTINUE
146 *****
C *****
C ***** CONSTRUCT RUNOFF ARRAY FOR EACH DAY ON TAPE
C *****
C ***** DO 350 I=1,1440
147 *****

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TWM 133
TWM 134
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148 WRITE (INew) MONTH, I, JYR, KSEGMT, JPRINT, ISIZE, (SIMFLO(IA), IA=1, 1440) TWM 157
149 CONTINUE TWM 158
150 IF (NEWVAL(MONTH)+IFIRST) 430, 430, 360 TWM 159
151 READ 2320, PANFAC, ALPHA, RTZONE TWM 160
152 READ 2280, IOPT1, IOPT2, IOPT3, IOPT4 TWM 161
153 IF (IRECOD(KSEGMT)) 362, 363, 363 TWM 162
154 IOPT5 = 1 TWM 163
155 GO TO 364 TWM 164
156 IOPT5 = 0 TWM 165
157 CONTINUE TWM 166
158 IF (IOPT1-1) 380, 370, 380 TWM 167
159 READ 2320, BASE TWM 168
160 IF (IOPT3-1) 400, 390, 400 TWM 169
161 READ 2280, IFILM TWM 170
162 CONTINUE TWM 171
163 IF (IOPT3-1) 410, 420, 410 TWM 172
164 IF (IOPT4-1) 430, 420, 430 TWM 173
165 READ 2330, LTR, (NAMSEG(I), I=1, 19) TWM 174
166 CONTINUE TWM 175
167 IF (JYR-IYR) 450, 440, 450 TWM 176
168 IF (MONTH-10) 450, 750, 450 TWM 177
169 IF (ISEG1) 680, 680, 460 TWM 178
170 CONTINUE TWM 179
171 C***** TWM
172 C ASSIMULATE RUNOFF FROM UPSTREAM SEGMENT(S) INTO RUNOFF ARRAY TWM
173 C ON TAPE TWM
174 C FIRST SEGMENT TWM
175 C***** TWM
176 CALL TPSWCH (IOLD, INEW, ISEGMT, KSEGMT, IDAYS, SIMFLO) TWM
177 REWIND IOLD TWM
178 READ (IOLD) MONTH1, IA, JYR1, JSEGMT, IPRINT, ISIZE, (SIMFLO(IB), IB=1, 1440) TWM 180
179 TWM 181
180 TWM 182
181 TWM 183
182 CALL TAPCHK(1, IA, MONTH, MONTH1, JYR, JYR1, 1) TWM
183 IF (JSEGMT-ISEG1) 470, 480, 470 TWM
184 TWM 184
185 DO 510 I=1, IDAYS TWM
186 TWM 185
187 TWM 186

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177 IF (IRINT-JRINT) 490,500,490 TWM 187
178 CALL SHIFTT (JRINT,IRINT,SIMFLO) TWM 188
179 WRITE (INEW) MONTH1,I,JYR,KSEGMT,IRINT,ISIZE,(SIMFLO(IB),IB=1,1440)TWM 189
180 . 1) READ (IOLD)MONTH1,IA,JYR1,JSEGMT,IRINT,ISIZE,(SIMFLO(IB),IB=1,1440)TWM 190
181 1) CALL TAPCHK(I+1,IA,MONTH,MONTH1,JYR,JYR1,2) TWM 191
182 CONTINUE TWM 192
183 C***** TWM 193
184 C SECOND SEGMENT TWM 194
185 C***** TWM
186 IF (ISEG2) 680,680,520 TWM
187 CALL TPSWCH (IOLD,INEW,1,KSEGMT,IDAYS,SIMFLO) TWM
188 REWIND ISCRTH TWM
189 READ (IOLD) MONTH1,IA,JYR1,JSEGMT,IRINT,ISIZE,(SIMFLO(IB),IB=1,144)TWM 195
190 10) WRITE (INEW) MONTH1,IA,JYR1,JSEGMT,IRINT,ISIZE,(SIMFLO(IB),IB=1,14)TWM 196
191 140) IF (JSEGMT-ISEG2) 550,540,550 TWM 197
192 WRITE (ISCRTH) MONTH1,IA,JYR1,JSEGMT,IRINT,ISIZE,(SIMFLO(IB),IB=1, TWM 198
193 11440) TWM 199
194 IF (JSEGMT+1-KSEGMT) 530,560,530 TWM 200
195 IF (IA-JDPM(MONTH1)) 530,570,530 TWM 201
196 REWIND ISCRTH TWM 202
197 DO 610 I=1,IDAYS TWM 203
198 READ (IOLD) MONTH1,IA,JYR1,JSEGMT,IRINT,ISIZE,(SIMFLO(IB),IB=1,144)TWM 204
199 10) CALL TAPCHK(I,IA,MONTH,MONTH1,JYR,JYR1,3) TWM 205
200 READ (ISCRTH) MONTH1,IA,JYR1,JSEGMT,IRINT1,JSIZE,(PREC(IB),IB=1,14)TWM 206
201 140) TWM 207
202 CALL TAPCHK(I,IA,MONTH,MONTH1,JYR,JYR1,4) TWM 208
203 IF (IRINT1-JRINT) 580,590,580 TWM 209
204 CALL SHIFTT (JRINT,IRINT1,PREC) TWM 210
205 CONTINUE TWM 211
206 DO 600 II=1,ISIZE TWM 212
207 SIMFLO(II)=SIMFLO(II)+PREC(II) TWM 213
208 TWM 214
209 TWM 215
210 TWM 216
211 TWM 217
212 TWM 218
213 TWM 219

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203          CONTINUE                                TWM 220
204          WRITE (INEW) MONTH,I,JYR,KSEGMT,JRINT,ISIZE,(SIMFLO(IB),IB=1,1440) TWM 221
205          CONTINUE                                TWM 222
C*****
C          LAG FLOW THROUGH THIS SEGMENT          TWM
C*****
206          IF (LAG) 680,680,620                    TWM 223
207          DO 630 I=1,1440                          TWM 224
208          PREC(I)=0.                                TWM 225
209          IT=LAG+1                                   TWM 226
210          CALL TPSWCH (IOLD,INEW,ISEGMT,KSEGMT,IDAYS,SIMFLO) TWM 227
211          READ (IOLD) MONTH1,IA,JYR1,JSEGMT,JRINT,ISIZE,(SIMFLO(IB),IB=1,1440) TWM 228
10)
212          DO 650 II=IT,ISIZE                          TWM 229
213          IA=ISIZE+IT-II                             TWM 230
214          TRS(IA)=SIMFLO(IA)                        TWM 231
215          SIMFLO(IA)=SIMFLO(IA-LAG)                TWM 232
216          CONTINUE                                  TWM 233
217          DO 660 II=1,LAG                             TWM 234
218          SIMFLO(II)=PREC(II)                       TWM 235
219          PREC(II)=TRS(ISIZE-LAG+II)               TWM 236
220          CONTINUE                                  TWM 237
221          WRITE (INEW) MONTH,IA,JYR,KSEGMT,JRINT,ISIZE,(SIMFLO(IB),IB=1,1440) TWM 238
1)
222          IF (IA-IDAYS) 640,670,670                 TWM 239
223          CONTINUE                                  TWM 240
C*****
C          ADD FLOW GENERATED LAST MONTH, BUT NOT YET PAST STREAM GAUGING TWM
C          STATION.                                  TWM
C*****
224          II=0                                       TWM 243
225          IA=0                                       TWM 244
226          JSIZE = NRELEM                             TWM 245
227          II=II+1                                   TWM 246
228          CALL TPSWCH (IOLD,INEW,ISEGMT,KSEGMT,IDAYS,SIMFLO) TWM 247
229          READ (IOLD) MONTH1,I,JYR1,JSEGMT,JRINT,ISIZE,(SIMFLO(IB),IB=1,1440) TWM 248

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256 790 READ 2320, ATOTAL, APERVS, AOLFSF, AOLFDS, DSMAX, VINSTM, DEPTH1, DEPTHU, TWM
1CHANLG, C1, C2, C3, C4, C5, C6, C7, C8, C9, C10, C11, C12, C13, C14, C15, ROUGH, SL TWM
20PE, VLENGH, SATPRM, UZST, VI7ST, C16, SMMIN TWM
IFIRST=0 TWM
DEPTHU=DEPTHU*12. TWM
DEPTH1=DEPTH1*12. TWM
X=-C4 TWM
U7MIN=DEPTHU*AMAX1(SMMIN, X) TWM
VIZMIN=.5*DEPTH1*AMAX1(SMMIN, X) TWM
IF (ATOTAL) 800, 810, 810 TWM
ATOTAL=-640.*ATOTAL TWM
C***** TWM
C PRINT PARAMETERS TWM
C***** TWM
810 PRINT 2970, MONTH, JYR TWM
265 PRINT 2980, KSEGMT, JRINT, IT TWM
266 PRINT 2840, ATOTAL, APERVS, AOLFSF, AOLFDS, DSMAX, VINSTM, DEPTH1, DEPTHU TWM
267 PRINT 2840, ATOTAL, APERVS, AOLFSF, AOLFDS, DSMAX, VINSTM, DEPTH1, DEPTHU TWM
1, CHANLG, RTZONE TWM
268 PRINT 2960, ROUGH, SLOPE, VLENGH, SATPRM, UZST, VI7ST TWM
269 PRINT 3020, U7MIN, VIZMIN TWM
270 PRINT 2850, C1, U7ST, C2, SATPRM TWM
271 PRINT 2860, C3, C4, C5, C6 TWM
272 PRINT 2870, C7, C8, C9 TWM
273 PRINT 2880, C10 TWM
274 PRINT 2890, C11 TWM
275 PRINT 2900 TWM
276 PRINT 2910, C12 TWM
277 PRINT 2920, C13 TWM
278 PRINT 2930, C14 TWM
279 PRINT 2950, C16 TWM
280 PRINT 2940, C15 TWM
281 IF (JYR-IYR) 840, 820, 840 TWM
282 IF (MONTH-10) 840, 830, 840 TWM
820 ***** TWM
C***** TWM
C READ INITIAL VALUES TWM
C***** TWM

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283      830  READ 2320, T1ZS1(KSEGMT),R1ZS1(KSEGMT),OLS1(KSEGMT),DS1(KSEGMT),UZTWM
          1S1(KSEGMT),GWS1(KSEGMT),DELIN1(KSEGMT),VINST1(KSEGMT)
C*****
C      INITIALIZE STORAGE
C*****
840  VINSTG=VINST1(KSEGMT)
      OLS=OLS1(KSEGMT)
      DS=DS1(KSEGMT)
      U7S=U7S1(KSEGMT)
      T1ZS=T1ZS1(KSEGMT)
      R1ZS=R1ZS1(KSEGMT)
      GWS=GWS1(KSEFGMT)
      DELINF=DELIN1(KSEGMT)
C*****
C      CHECK INITIAL STORAGE VALUES AGAINST MINIMUM/MAXIMUM VALUES
C*****
292  850  IF (DS-DSMAX) 860,860,850
293  PRINT 3030, DS,DSMAX
294  DS=DSMAX
295  860  IF (U7S-U7ST) 880,880,870
296  PRINT 2990, U7S,U7ST
297  U7S=U7ST
298  880  IF (U7S-U7MIN) 890,900,900
299  PRINT 2990, U7S,U7MIN
300  U7S=U7MIN
301  900  IF (T1ZS-V1ZMIN) 910,920,920
302  PRINT 3000, T1ZS,V1ZMIN
303  T1ZS=V1ZMIN
304  920  IF (T1ZS-V1ZST) 940,940,930
305  PRINT 3000, T1ZS,V1ZST
306  T1ZS=V1ZST
307  940  IF (B1ZS-V17MIN) 950,960,960
308  PRINT 3010, B1ZS,V1ZMIN
309  B1ZS=V1ZMIN
310  960  IF (B1ZS-V1ZST) 980,980,970
311  PRINT 3010, B1ZS,V1ZST
          309  TWM
          310  TWM
          311  TWM
          312  TWM
          313  TWM
          314  TWM
          315  TWM
          316  TWM
          317  TWM
          318  TWM
          319  TWM
          320  TWM
          321  TWM
          322  TWM
          323  TWM
          324  TWM
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          328  TWM
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          336  TWM
          337  TWM
          338  TWM

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312 BIZSVIZST
313 CONTINUE
C*****
C      INITIALIZE STATISTICAL VARIABLES
C*****
314 SSRAIN=0.
315 SSEVAP=0.
316 SSSF=0.
317 SSOLF=0.
318 SSGWF=0.
319 SSIF=0.
320 SSUF=0.
321 DO 990 I=1,1440
322 TRS(I)=0.
323 CONTINUE
990 CONTINUE
C*****
C      DAY LOOP
C*****
324 REWIND 1
325 CALL TPSWCH (IOLD,INEW,ISEGMT,KSEGMT,IDAYS,SIMFLO)
326 DO 1530 IDAY=1, IDAYS
327 READ(3)MONTH1,IAI, JYR1,KSEGMT,IT,ISIZE,(PREC(IB),IB=1,1440)
328 CALL TAPCHK(IDAY,IAI, MONTH,MONTH1,JYR,JYR1,5)
329 READ (IOLD) MONTH1,IA,JYR1,JSEGMT,JRINT,JSIZE,(SIMFLO(IB),IB=1,144
10)
330 CALL TAPCHK(IDAY,IA, MONTH,MONTH1,JYR,JYR1,6)
C*****
C      ADD FLOW GENERATED YESTERDAY
C*****
331 DO 1000 I=1,JSIZE
332 SIMFLO(I)=TRS(I)+SIMFLO(I)
333 TRS(I)=0.
334 CONTINUE
335 IA=JSIZE+1
336 DO 1010 I=IA,1440
337 TRS(I-IA+1)=TRS(I)

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TWM 339
TWM 340TWM 341
TWM 342
TWM 343TWM 344
TWM 345
TWM 346TWM 347
TWM 348
TWM 349TWM 350
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TWMTWM 354
TWM 355
TWM 356TWM 357
TWM 358
TWM 359TWM 360
TWM 361
TWMTWM 362
TWM 363
TWM 364TWM 365
TWM 366
TWM 367

TWM 368

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338 TRS(I)=0. TWM 369
339 CONTINUE TWM 370
C***** INITIALIZE DAILY FLOW SUMS TWM
C***** SDAYFL=0. TWM 371
C***** RO=0. TWM 372
342 DELT=FLOAT(IT)/60. TWM 373
343 X=IDAY TWM 374
C***** POTENTIAL EVAPORATION FOR THIS DAY TWM
C***** (EVAP1(MONTH)*X*EVAP2(MONTH)*X*EVAP3(MONTH)) TWM 375
C***** POTEVP= TWM
C***** DETERMINE SUNRISE AND SUNSET, EVAPORATION IS ASSUMED TO OCCUR TWM
C***** FROM 30 MINUTES AFTER SUNRISE (TR), UNTIL 1 HOUR AFTER TWM
C***** SUNSET (TE), WITH THE MAXIMUM RATE OCCURRING AT A TIME (TH) TWM
C***** 3/4 FROM THE BEGINNING TO THE END. TWM
345 X=FLOAT(JDJD)*X TWM 376
346 JAD=FIX(X)/10+1 TWM 377
347 DELTA=AD(JAD)*(X=FLOAT(10*JAD-10))/10*(AD(JAD*1)-AD(JAD)) TWM 378
348 COSH=(-SIN(XLAT)*SIN(DELTA)-.0145)/(COS(XLAT)*COS(DELTA)) TWM 379
349 H=ARCOS(COSH)*3.82044 TWM 380
350 IF (FIX(X)-260) 1020,1030,1030 TWM 381
351 ET=12.+(9.36-.0467*X*.9*SIN(.04*(X-14.)))/60. TWM 382
352 GO TO 1040 TWM 383
353 ET=12.+(.15.3*.04*X*.6*SIN(.0308*(X-360.)))/60. TWM 384
354 X=AMOD(XLONG,15.) TWM 385
355 IF (X-7.5) 1050,1050,1060 TWM 386
356 CORREC=X/15. TWM 387
357 GO TO 1070 TWM 388
358 CORREC=X/15.-1. TWM 389
359 TB=ET+H+CORREC+.5+CORTZ TWM 390
360 TE=ET+H+CORREC+1.+CORTZ TWM 391
361 TH=.75*(TE-TB)+TB TWM 392
362 E4=SIN(.2618*(TE-TH+6.)) TWM 393
363 E5=SIN(.2618*(TB-TH+6.)) TWM 394

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364 E2=(E4-E5)/(TE-TB) TWM 395
365 E3=(TE+E5-TB+E4)/(TE-TB) TWM 396
366 E1=POTEVP/(3.82*(COS((TB-TH*6.))-COS(.2618*(TE+TH*6.)))+.5 TWM 397
      1*E2*(TB+TR-TE)+E3*(TB-TE)) TWM 398
C***** TWM
C BASIC TIME LOOP TWM
C***** TWM
C DO 1520 I=1,ISIZE TWM
367 RAIN=PREC(I)*DELT TWM 399
368 SSRAIN=SSRAIN+RAIN TWM 400
369 IF (RAIN*.0001) 1090,1090,1080 TWM 401
370 C***** TWM 402
C PERVIOUS - IMPERVIOUS SEPARATION TWM
C***** TWM
C X=(1.-APERVS)*RAIN TWM
371 CALL ADJUST (RAIN,RO,X) TWM 403
372 C***** TWM 404
C INTERCEPTION TWM
C***** TWM
C CALL ADJUST (RAIN,VINSTG,AMIN1(RAIN,VINSTG-VINSTG)) TWM
373 C***** TWM 405
C OVERLAND FLOW AND DEPRESSION STORAGE TWM
C***** TWM
374 1090 X1=(AOLFSF+AOLFDS)*RAIN TWM 406
375 OLSI=OLS TWM 407
C***** TWM
C RAIN = OVERLAND STORAGE TWM
C***** TWM
C CALL ADJUST (RAIN,OLS,X1) TWM
376 DS=DS+RAIN TWM 408
377 IF (DS*.0001) 1100,1100,1110 TWM 409
378 IF (OLS*.0001) 1210,1210,1110 TWM 410
379 C***** TWM 411
C INFILTRATION TWM
C***** TWM
380 1110 IF (U7ST -U7S) 1112, 1112, 1114 TWM 412

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381      SFEED = DELT * SATPRM
382      GO TO 1116
383      SFEED = DELT * SATPRM
384      SFEED=DELT*(C1*(U7ST-U7S))*C2*(SATPRM)
385      X=(1.-AOLFSF-AOLFDS)*DS/DSMAX
386      IF (DS/DSMAX-1.) 1130,1130,1120
387      X=1.-AOLFSF-AOLFDS
388      Y=UZST-UZS
C*****
C      INFILTRATE FROM OVERLAND FLOW STORAGE
C*****
C      X4 = AMIN1(SEEP*(1.-X),OLS,Y*(1.-X))
C      CALL ADJUST (OLS, UZS, X4)
C      IF (SEEP*(1.-X) - X4) 1132, 1132, 1131
C      X4 = AMIN1((SEEP*(1.-X)-X4),OLS,(VIZST-TIZS))
C      CALL ADJUST (OLS, TIZS, X4)
C*****
C      INFILTRATE FROM DEPRESSION STORAGE
C*****
C      X4 = AMIN1(X*SEEP,DS,Y*X)
C      CALL ADJUST (DS, UZS, X4)
C      IF (X*SEEP - X4) 1134, 1134, 1133
C      X4 = AMIN1((X*SEEP-X4),DS,(V17ST-T17S))
C      CALL ADJUST ( DS, T17S, X4)
C*****
C      CALCULATE OVERLAND FLOW
C*****
C      IF (OLS1) 1140, 1140, 1150
C      IF (OLS) 1210,1210,1150
C      OFSAVG=.5*(OLS1+OLS)
C      OFSEQU=.00018*(X)*ROUGH*VLENGH)*.6*(VLENGH/SLOPE)*.3
C      IF (OFSAVG-OFSEQU) 1170,1170,1160
C      OFSEQU=OFSAVG
C      OLF=64200.*SLOPE*.5*(OFSAVG*(1.+*.6*(OFSAVG/OFSEQU))*3)/VLENGH)*.1
C      1.67/(ROUGH*VLENGH)*DELT

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TWM 413
 TWM 414
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 TWM 436
 TWM 437
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C*****
C      OVERLAND FLOW INTO STREAM
C*****
C      CALL ADJUST (OLS,RO,AMIN1(OLS+AOLFSF/(AOLFSF+AOLFDS),AOLFSF+OLF))
C*****
C      OVERLAND FLOW INTO DEPRESSION STORAGE
C*****
C      CALL ADJUST (OLS,DS,AMIN1(AOLFDS+OLF,AOLFDS/(AOLFDS+AOLFDS)+OLS))
C*****
C      RUNOFF FROM DEPRESSION STORAGE
C*****
C      IF (DS/DSMAX-1.) 1190,1180,1180
1180      X=1.-AOLFSF
408      GO TO 1200
1190      X=(1.-AOLFSF)*(DS/DSMAX)**.5
409      CONTINUE
410
411      CALL ADJUST (DS,RO,AMIN1(X*OLF,DS))
412      SSOLF=SSOLF+(X+AOLFSF)*OLF
413
414      C*****
C      UPPER ZONE TO TOP INTERMEDIATE ZONE
C*****
C*****
1210      CONTINUE
415
416      XK=C3*((UZS/DEPTHU+TI7S/(.5*DEPTHI))*+.5+C4)**C5+C6
417      IF (XK) 1211, 1212, 1212
418      XK = 0.
419      PSI1=-C7*(UZS/DEPTHU+C8)**C9
420      PSI2=-C7*(TI7S/(.5*DEPTHI)+C8)**C9
421      X=XK*((PSI1-PSI2)/(.5*(.5*DEPTHI+DEPTHU))*1.)+DELT
422      IF (X) 1220,1280,1250
423      X=-X
424      IF (X=(TI7S-VIZMIN)) 1240,1240,1230
425      X=TI7S-VIZMIN
426      CALL ADJUST(TI7S,UZS,AMIN1(X,UZST-U7S))
427      GO TO 1280
428      CONTINUE
429      IF (X=(UZS-UZMIN)) 1270,1270,1260

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430 1260 X=UZS-UZMIN
431 1270 CALL ADJUST (UZS,TIZS,AMIN1(X,VIZST-TIZS))
C*****
C INTERFLOW
C*****
432 1280 IF (TIZS/VIZST-C10) 1300,1300,1290
433 1290 VINFL0=C16*(TIZS-C10*VIZST)*DFLT
434 CALL ADJUST (TIZS,DELINF,AMIN1(VINFLO,TIZS-VIZMIN))
C*****
C MOVE FROM TOP TO BOTTOM IN INTERMEDIATE ZONE.
C*****
C CONTINUE
1300 XK=C3*((TIZS-BIZS)/DEPTHI+C4)**C5+C6
IF (XK) 1301, 1302, 1302
1301 XK = 0.
1302 PSI1=-C7*(BIZS/(.5*DEPTHI)+C8)**C9
X=XK*((PSI2-PSI1)/(1.5*DEPTHI)+1.)*DELT
IF (X) 1310,1360,1340
1310 X="X
IF (X-(BIZS-VIZMIN)) 1320,1330,1330
1320 X=BIZS-VIZMIN
1330 CALL ADJUST (BIZS,TIZS,X)
GO TO 1370
1340 CONTINUE
IF (X-(TIZS-VIZMIN)) 1360,1360,1350
1350 X=TIZS-VIZMIN
1360 CALL ADJUST (TIZS,BIZS,X)
1370 CONTINUE
C*****
C MOVE FROM BOTTOM OF INTERMEDIATE ZONE TO GROUNDWATER ZONE.
C*****
X=SATPRM*(1.-PSI1/(.25*DEPTHI))*DELT
IF (X-(BIZS-VIZMIN)) 1390,1390,1380
1380 X=BIZS-VIZMIN
1390 CALL ADJUST (BIZS,GWS,X)
C*****

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TWM 463
TWM 464
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TWM 486
TWM 487
TWM 488
TWM

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456 C***** OUTFLOW FROM GROUNDWATER TO STREAM TWM
457 C***** Y=C13*GWS*DELT TWM
458 C***** IF (GWS-C11) 1410,1410,1400 TWM
459 C***** X=(GWS-C11)*C12*DELT TWM
460 C***** SSGWF=SSGWF*X TWM
461 C***** CALL ADJUST (GWS,RO,X) TWM
462 C***** UNDERFLOW TWM
463 C***** CALL ADJUST (GWS,UNDFLO,Y) TWM
464 C***** SSUF=SSUF+Y TWM
465 C***** RUNOFF FROM INTERFLOW TWM
466 C***** IF (DELINF-.00001) 1430,1430,1420 TWM
467 C***** X=(C14*DELINF+C15)*DELT TWM
468 C***** CALL ADJUST (DELINF,RO,X) TWM
469 C***** SSIF=SSIF*X TWM
470 C***** EVAPORATION FROM STORAGE TWM
471 C***** HOUR=FLOAT(I*IT)/60. TWM
472 C***** IF (HOUR-TB) 1465,1465,1440 TWM
473 C***** IF (HOUR-TE) 1450,1465,1465 TWM
474 C***** EVAPOT=-E1*(SIN(.2618*(HOUR-TH+6.))-E2*HOUR-E3)*DELT*PANFAC TWM
475 C***** SSEVAP = SSEVAP + ABS(EVAPOT) TWM
476 C***** CONSUM = TRANPO(MONTH) + ABS(EVAPOT) + EVAPOT TWM
477 C***** EVAPORATION FROM INTERCEPTION STORAGE TWM
478 C***** CALL ADJUST (VINSTG,EVAPOT,AMIN1(VINSTG,ABS(EVAPOT))) TWM
479 C***** EVAPORATION FROM OVERLAND STORAGE TWM
480 C***** EVAPORATION FROM OVERLAND STORAGE TWM
481 C***** EVAPORATION FROM OVERLAND STORAGE TWM
482 C***** EVAPORATION FROM OVERLAND STORAGE TWM
483 C***** EVAPORATION FROM OVERLAND STORAGE TWM
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502 C***** EVAPORATION FROM OVERLAND STORAGE TWM
503 C***** EVAPORATION FROM OVERLAND STORAGE TWM
504 C***** EVAPORATION FROM OVERLAND STORAGE TWM
505 C***** EVAPORATION FROM OVERLAND STORAGE TWM
506 C***** EVAPORATION FROM OVERLAND STORAGE TWM

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C      EVAPORATION FROM DEPRESSION STORAGE
C*****
C      CALL ADJUST (DS, EVAPOT, AMIN1(DS, ABS(EVAPOT))*DS/DSMAX*(1.-AOLFSF-AOTW
507 TWM
508 TWM
C*****
C      EVAPORATION FROM UPPER ZONE STORAGE
C*****
C      CALL ADJUST(UZS, EVAPOT, AMIN1(ABS(EVAPOT))*((UZS-UZMIN)/(UZST-UZMIN))
509 TWM
510 TWM
511 TWM
512 TWM
513 TWM
514 TWM
515 TWM
516 TWM
517 TWM
518 TWM
C*****
C      END OF MOVEMENT OF WATER FOR THIS CYCLE. IS THIS END OF JRINT
TWM
C      PERIOD
TWM
C*****
C      SSEVAP = SSEVAP + EVAPOT
519 TWM
520 TWM
C*****
C      CALCULATIONS FOR THIS INTERVAL COMPLETE. LAG RUNOFF THROUGH
TWM
C      HISTOGRAM
TWM
C*****
C      SDAYFL=SDAYFL*RO
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM
C*****
C      SSSF=SSSF+RO
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM
C*****
C      DO 1510 J=1, NRELEM
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM
C*****
C      IF (II-ISIZE) 1490, 1490, 1500
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM
C*****
C      SIMFLO(II)=C(J)*RO*ATOTAL+SIMFLO(II)
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM
C*****
C      GO TO 1510
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM
C*****
C      TRS(II-ISIZE)=C(J)*RO*ATOTAL+TRS(II-ISIZE)
521 TWM
522 TWM
523 TWM
524 TWM
525 TWM
526 TWM
527 TWM
528 TWM
529 TWM

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493 1510 CONTINUE                                TWM 530
494   RO=0.                                       TWM 531
C*****
C*****
C*****
C*****
495 1520 CONTINUE                                TWM 532
C*****
C***** CHANNEL LAG                             TWM
C*****
C***** SIMFLO(1)=(1.-CHANLG)*SIMFLO(1)+YESDAY TWM
496   YESDAY=CHANLG*SIMFLO(JSIZE)               TWM
497   DO 1560 IA=2,JSIZE                          TWM
498     SIMFLO(IA)=SIMFLO(IA)*(1.-CHANLG)+CHANLG*SIMFLO(IA-1) TWM
499   CONTINUE                                    TWM
500
501 C*****
502 C***** CONVERT SDAYFL FROM INCHES PER DAY TO CFS. ATOTAL IS IN ACRES TWM
C*****
C***** DAILYF(IDAY)=SDAYFL*ATOTAL*.042014     TWM
501   WRITE (INew) MONTH, IDAY, JYR, KSEGMT, Jrint, Jsize, (SIMFLO(IB), IB=1, 14 TWM
502   140)                                         TWM
C*****
C***** END OF DAY LOOP                          TWM
C*****
503 1530 CONTINUE                                TWM 541
504   PRINT 2970, MONTH, JYR                      TWM 542
C*****
C***** PRESERVE ALL FLOWS WHICH ORIGINATE THIS MONTH, BUT DO NOT TWM
C***** APPEAR AS STREAM FLOW UNTIL NEXT MONTH TWM
C*****
505   DO 1550 I = 1, NRELEM                       TWM
506     CARRY0(KSEGMT, I)=TRS(I)                 TWM
507   CONTINUE                                    TWM
C*****
508 C***** STORE END OF MONTH STORAGE VALUES FOR NEXT MONTHS STARING TWM
509 C***** VALUES.                              TWM
C*****

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508 BIZS1(KSEGMT)=BIZS TMM 546
509 DELIN1(KSEGMT)=DELINF TMM 547
510 DS1(KSEGMT)=DS TMM 548
511 GWS1(KSEGMT)=GWS TMM 549
512 OLS1(KSEGMT)=OLS TMM 550
513 TIZS1(KSEGMT)=TIZS TMM 551
514 UZS1(KSEGMT)=UZS TMM 552
515 VINST1(KSEGMT)=VINSTG TMM 553
516 SRAIN(KSEGMT,MONTH)=SSRAIN TMM 554
517 SEVAP(KSEGMT,MONTH)=SSEVAP TMM 555
518 SSF(KSEGMT,MONTH)=SSSF TMM 556
519 SOLF(KSEGMT,MONTH)=SSOLF TMM 557
520 SGWF(KSEGMT,MONTH)=SSGWF TMM 558
521 SIF(KSEGMT,MONTH)=SSIF TMM 559
522 SUF(KSEGMT,MONTH)=SSUF TMM 560
523 EOMINC(KSEGMT,MONTH)=VINSTG TMM 561
524 EOMSUR(KSEGMT,MONTH)=DS+OLS TMM 562
525 EOMUZ(KSEGMT,MONTH)=UZS TMM 563
526 EOMINT(KSEGMT,MONTH)=TIZS+BIZ TMM 564
527 EOMGW(KSEGMT,MONTH)=GWS TMM 565
528 EOMINF(KSEGMT,MONTH)=DELINF TMM 566
529 CHNLG(KSEGMT) = YESDAY TMM 567
C*****
C END OF MONTH REPORTS TMM
C*****
530 PRINT 2810, KSEGMT, MONTH, JYR TMM 568
531 PRINT 2820, (DAILYF(I), I=1, IDAYS) TMM 569
532 Y1=60.5/FLOAT(JRINT) TMM 570
533 X11=0. TMM 571
534 X12=0. TMM 572
535 DO 1580 I=1,1440 TMM 573
536 RECFLO(I)=0. TMM 574
537 CALL TPSWCH (IOLD, INEW, ISEGMT, KSEGMT, IDAYS, SIMFLO) TMM 575
538 DO 2090 II=1, IDAYS TMM 576
539 IFLAG=0 TMM 577
540 READ (IOLD) MONTH1, IAI, JYR1, JSEGMT, JRINT, JSIZE, (SIMFLO(IB), IB=1, 14) TMM 578

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140)
541 CALL TAPCHK(II, IAI, MONTH, MONTH1, JYR, JYR1, 7)
542 IF (IOPT5) 1610, 1620, 1610
543 CONTINUE
544 READ(3)MONTH1, IAI, JYR1, KSEGMT, JRINT, JSIZE, (RECFL0(IB), IB=1, 1440)
545 CALL TAPCHK(II, IAI, MONTH, MONTH1, JYR, JYR1, 8)
546 CONTINUE
547 IF (IOPT1-1) 1590, 1600, 1590
548 IF (IOPT3-1) 1960, 1600, 1960
549 CONTINUE
550 DO 1640 I=1, JSIZE
C*****
C CONVERT SIMULATED FLOW TO C.F.S.
C*****
551 X=SIMFLO(I)*Y1
552 X11=X11+X
553 X12=X12+RECFL0(I)
554 TRS(I)=0.
555 IF (X-BASE) 1640, 1630, 1630
556 IFLAG#1
557 TRS(I)=X
558 CONTINUE
559 IF (IFLAG=1) 2000, 1650, 1650
560 KFIRST#1
561 KSECND=0
562 X1#0.
563 X2#0.
564 DO 1950 I=1, JSIZE
565 IF (TRS(I)) 1670, 1660, 1670
566 IF (RECFL0(I)) 1670, 1740, 1670
567 IF (I-1) 1680, 1730, 1680
568 IF (I-JSIZE) 1700, 1690, 1700
569 KSECND=JSIZE
570 GO TO 1780
571 CONTINUE
572 IF (TRS(I-1)) 1730, 1710, 1730
TWM 579
TWM 580
TWM 581
TWM 582
TWM 583
TWM 584
TWM 585
TWM 586
TWM 587
TWM 588
TWM 589
TWM 590
TWM 591
TWM 592
TWM 593
TWM 594
TWM 595
TWM 596
TWM 597
TWM 598
TWM 599
TWM 600
TWM 601
TWM 602
TWM 603
TWM 604
TWM 605
TWM 606
TWM 607
TWM 608
TWM 609
TWM 610
TWM 611

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573 1710 IF (RECFL0(I-1)) 1730,1720,1730
574 1720 KFIRST=I
575 1730 KSECND=I
576 1740 GO TO 1950
577 1750 IF (I-1) 1750,1770,1750
578 1750 IF (TRS(I-1)) 1780,1760,1780
579 1760 IF (RECFL0(I-1)) 1780,1770,1780
580 1770 KFIRST=I
581 1780 KSECND=I
582 1780 GO TO 1950
583 1780 IDAY=IAI
584 1790 IF (IOPT1-1) 1840,1790,1840
585 1790 IHOURS=(JPRINT*KFIRST)/60
586 1790 IMIN=MOD(JPRINT*KFIRST,60)
587 1790 PRINT 2780, KSEGMT, MONTH, IDAY, JPRINT, IHOURS, IMIN
588 1790 JFIRST=KFIRST
589 1790 X1=0.
590 1790 X2=0.
591 1790 X1Y1 = 0.
592 1790 SX1X1 = 0.
593 1790 SX2X2 = 0.
594 1790 DO 1800 IK=KFIRST,KSECND
595 1790 X2=X2+RECFL0(IK)
596 1790 X1=X1+SIMFLO(IK)
597 1790 X1Y1 = X1Y1 + RECFL0(IK) + TRS(IK)
598 1790 SX2X2 = SX2X2 + TRS(IK) + TRS(IK)
599 1790 SX1X1 = SX1X1 + RECFL0(IK) + RECFL0(IK)
600 1790 XN = KSECND - KFIRST
601 1800 CONTINUE
602 1800 XN = KSECND - KFIRST
603 1810 JJ=MIN0(KSECND,KFIRST+11)
604 1810 PRINT 2790, (TRS(IK),IK=KFIRST,JJ)
605 1810 PRINT 2800, (RECFL0(IK),IK=KFIRST,JJ)
606 1820 IF (JJ-KSECND) 1820,1830,1830
607 1820 KFIRST=JJ+1
608 1810 GO TO 1810
TWM 612
TWM 613
TWM 614
TWM 615
TWM 616
TWM 617
TWM 618
TWM 619
TWM 620
TWM 621
TWM 622
TWM 623
TWM 624
TWM 625
TWM 626
TWM 627
TWM 628
TWM 629
TWM 630
TWM 631
TWM 632
TWM 633
TWM 634
TWM 635
TWM 636
TWM A636
TWM 637
TWM 638
TWM 639
TWM 640
TWM 641
TWM 642
TWM 643
TWM 644
TWM 645
TWM 646

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609 1830 KFIRST=KSECND*1
610 X1=X1*Y1*FLOAT(JRINT)/(60.5*AOTAL)
611 X2 = X2*FLOAT(JRINT)/(60.5*AOTAL)
612 PRINT 2450, X2,X1
613 IF (XU * X2) 1835, 1835, 1834
614 1834 CORRCO = (X1Y1 - X1 * X2/XN)/((SX1X1 - X1*X1/XN)*(SX2X2 - X2*X2/XN)
1
615 PRINT 2455, CORRCO
616 X1=0.
617 X2=0.
618 PRINT 2830
619 IF (IOPT3-1) 1950,1850,1950
C*****
C PLOT INDIVIDUAL STORMS
C*****
1850 FLOMAX=0.
KFIRST=JFIRST
DO 1860 IA=KFIRST,JJ
FLOMAX=AMAX1(FLOMAX,TRS(IA),RECFL0(IA))
CONTINUE
1860 J=JJ-KFIRST+1
LINE=0
YP=FLOMAX*.25
XP=FLOMAX*.5
ZP=FLOMAX*.75
PRINT 2390, YP,XP,ZP,FLOMAX
PRINT 2400
DO 1940 IA=KFIRST,JJ
IHOURS=(JRINT*IA)/60
IMIN=MOD(JRINT*IA,60)
DO 1870 IR=1,120
JPLOT(IR)=BLANK
IR=(RECFL0(IA)/FLOMAX)*120.+ .5
IS=(TRS(IA)/FLOMAX)*120.+ .5
IF (IR) 1892, 1892, 1891
1891 JPLOT(IR) = R
TWM 647
TWM 648
TWM 649
TWM 650
TWM 651
TWM 652
TWM 653
TWM 654
TWM 655
TWM 656
TWM 657
TWM 658
TWM 659
TWM 660
TWM 661
TWM 662
TWM 663
TWM 664
TWM 665
TWM 666
TWM 667
TWM 668
TWM 669
TWM 670
TWM 671
TWM 672
TWM 673
TWM 674
TWM 675
TWM 676
TWM 677
TWM 678

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641 1892 IF (IS) 1900, 1900, 1893
642 1893 JPLOT(IS)=S
643 IF (IR - IS) 1900, 1880, 1900
644 1880 IF (IR) 1900, 1900, 1885
645 1885 JPLOT(IR) = B
646 1900 IF (MOD(LINE,5)) 1910,1920,1910
647 1910 PRINT 2410, (JPLOT(IX),IX=1,120)
648 GO TO 1930
649 1920 PRINT 2420, IHOURS,IMIN,(JPLOT(IX),IX=1,120)
650 1930 LINE=LINE+1
651 1940 CONTINUE
652 PRINT 2430
653 PRINT 2440, YP,XP,ZP,FLOMAX
654 1950 CONTINUE
C*****
C PLOT HYDROGRAPH FOR MONTH
C*****
655 1960 IF (IOPT4=1) 1990,1970,1990
656 1970 FLOMAX=0.
657 DO 1980 I=1,J
658 PREC(I)=FLOAT(I+JRINT)
659 TRS(I)=SIMFLO(I)*Y
660 FLOMAX=AMAX1(FLOMAX,TRS(I))
661 CONTINUE
662 FLOMAX=FLOAT(IFIX(FLOMAX)/25+1)*25.
C*****
C CALL FLOWPLT(PREC,SIMFLO,RECFL0,FLOMAX,JRINT,J,JYR,1,J,MONTH,
C NAMSEG, LTR, 120.)
663 1990 IF (IOPT2=1) 2090,2000,2090
664 2000 IF (IOPT5) 2020,2010,2020
C*****
665 2010 PRINT 2270
666 GO TO 2090
667 2020 CONTINUE
668 DO 2080 I=1,JSIZE
669 X=SIMFLO(I) * 60.5/FLOAT(JRINT)

```

TWM 679
TWM 680
TWM 681
TWM 682
TWM 683
TWM 684
TWM 685
TWM 686
TWM 687
TWM 688
TWM 689
TWM 690
TWM 691
TWM 692

TWM 693
TWM 694
TWM 695
TWM 696
TWM 697
TWM 698
TWM 699
TWM 700

TWM 701
TWM 702

TWM 703
TWM 704
TWM 705
TWM 706
TWM 707


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670 Y=RECFL0(I)
671 SUMX(KSEGMT)=SUMX(KSEGMT)*X
672 SUMY(KSEGMT)=SUMY(KSEGMT)*Y
673 XX(KSEGMT)=XX(KSEGMT)*X*X
674 YY(KSEGMT)=YY(KSEGMT)*Y*Y
675 XY(KSEGMT)=XY(KSEGMT)*X*Y
676 ERR=X-Y
677 IF (ABS(ERR)-.00001) 2030,2030,2040
678 ERR=0.
679 IF (Y-1.) 2050,2050,2060
680 IND=1
681 GO TO 2070
682 IND=2.*ALOG(Y)+2.
683 CONTINUE
684 ICASE(KSEGMT,IND)=ICASE(KSEGMT,IND)+1
685 SUMERR(KSEGMT,IND)=SUMERR(KSEGMT,IND)+ERR
686 SABSER(KSEGMT,IND)=SABSER(KSEGMT,IND)+ABS(ERR)
687 SERRSQ(KSEGMT,IND)=SERRSQ(KSEGMT,IND)+ERR*ERR
688 SWEIFL(KSEGMT)=SWEIFL(KSEGMT)+ARS(ERR)*WEIGH(IND)
689 CONTINUE
690 CONTINUE
691 X11=X11*FLOAT(JRINT)/(60.5*ATOTAL)
692 X12=X12*FLOAT(JRINT)/(60.5*ATOTAL)
693 PRINT 2460, SSRAIN,X12,X11
694 IF (MONTH-9) 220,2100,220
C*****
C YEAR-END REPORTS
C*****
2100 DO 2240 I=1,ISEGMT
695 SSRAIN=0.
696 SSEVAP=0.
697 SSSF=0.
698 SSOLF=0.
699 SSGWF=0.
700 SSIF=0.
701 SSUF=0.
702
TWM 708
TWM 709
TWM 710
TWM 711
TWM 712
TWM 713
TWM 714
TWM 715
TWM 716
TWM 717
TWM 718
TWM 719
TWM 720
TWM 721
TWM 722
TWM 723
TWM 724
TWM 725
TWM 726
TWM 727
TWM 728
TWM 729
TWM 730
TWM 731
TWM 732
TWM 733
TWM 734
TWM 735
TWM 736
TWM 737
TWM 738
TWM 739
TWM 740

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703 DO 2110 J=1,12
704   SSRAIN=SSRAIN+SRAIN(I,J)
705   SSEVAP=SSEVAP+SEVAP(I,J)
706   SSSF=SSSF+SSF(I,J)
707   SSOLF=SSOLF+SOLF(I,J)
708   SSGWF=SSGWF+SGWF(I,J)
709   SSIF=SSIF+SIF(I,J)
710   SSUF=SSUF+SUF(I,J)
711   CONTINUE
712   AVINFL=SSSF/FLOAT(1440+JDPY/JRINT)
713   PRINT 2490, I,JYR
714   PRINT 2500
715   PRINT 2510,(SRAIN(I,J),J=10,12),(SRAIN(I,J),J=1,9),SSRAIN
716   PRINT 2520
717   PRINT 2530,(SEVAP(I,J),J=10,12),(SEVAP(I,J),J=1,9),SSEVAP
718   PRINT 2520
719   PRINT 2540,(SSF(I,J),J=10,12),( SSF(I,J),J=1,9), SSSF
720   PRINT 2520
721   X=ATOTAL/12.
722   DO 2120 K=1,12
723     SSF(I,K)=SSF(I,K)*X
724   CONTINUE
725   SSSF=SSSF*X
726   PRINT 2550,(SSF(I,J),J=10,12),(SSF(I,J),J=1,9),SSSF
727   X=.504166667/FLOAT(IDAYS)
728   DO 2130 K=1,12
729     SSF(I,K)=SSF(I,K)*X
730   CONTINUE
731   SSSF=SSSF*X
732   PRINT 2550,(SSF(I,J),J=10,12),(SSF(I,J),J=1,9),SSSF
733   PRINT 2570
734   PRINT 2580,(SOLF(I,J),J=10,12),(SOLF(I,J),J=1,9),SSOLF
735   PRINT 2590
736   PRINT 2600,(SGWF(I,J),J=10,12),(SGWF(I,J),J=1,9),SSGWF
737   PRINT 2610,(SIF(I,J),J=10,12),(SIF(I,J),J=1,9),SSIF
738   PRINT 2520
741 TWM
742 TWM
743 TWM
744 TWM
745 TWM
746 TWM
747 TWM
748 TWM
749 TWM
750 TWM
751 TWM
752 TWM
753 TWM
754 TWM
755 TWM
756 TWM
757 TWM
758 TWM
759 TWM
760 TWM
761 TWM
762 TWM
763 TWM
764 TWM
765 TWM
766 TWM
767 TWM
768 TWM
769 TWM
770 TWM
771 TWM
772 TWM
773 TWM
774 TWM
775 TWM
776 TWM

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739 PRINT 2620, (SUF(I,J),J=10,12),(SUF(I,J),J=1,9),SSUF TWM 777
740 PRINT 2520 TWM 778
741 PRINT 2630 TWM 779
742 PRINT 2640,(EOMINC(I,J),J=10,12),(EOMINC(I,J),J=1,9) TWM 780
743 PRINT 2650,(EOMSUR(I,J),J=10,12),(EOMSUR(I,J),J=1,9) TWM 781
744 PRINT 2660,(EOMUZ(I,J),J=10,12),(EOMU7(I,J),J=1,9) TWM 782
745 PRINT 2670 TWM 783
746 PRINT 2680,(EOMINT(I,J),J=10,12),(EOMINT(I,J),J=1,9) TWM 784
747 PRINT 2690,(EOMGW(I,J),J=10,12),(EOMGW(I,J),J=1,9) TWM 785
748 PRINT 2700,(EOMINF(I,J),J=10,12),(EOMINF(I,J),J=1,9) TWM 786
749 IF (IOPT2-1) 2230,2140,2230 TWM 787

C*****
C CONVERT FLOWS FROM INCHES PER TIME INTERVAL TO C.F.S. TWM
C***** TWM
2140 CONTINUE TWM 788
PRINT 2490, I, JYR TWM 789
PRINT 2710 TWM 790
PRINT 2720, JRINT TWM 791
PRINT 2730 TWM 792
DO 2180 J=1,24 TWM 793
IF (ICASE(I,J)-1) 2160,2170,2170 TWM 794
PRINT 2740, FLOINT(J),FLOINT(J+1),ICASE(I,J),SUMERR(I,J),SARSER(I, TWM 795
1J) TWM 796
GO TO 2180 TWM 797
2170 X=ICASE(I,J) TWM 798
AVGERR=SUMERR(I,J)/X TWM 799
AVABER=SARSER(I,J)/X TWM 800
STDERR=SQRT((SERRSQ(I,J)-SUMERR(I,J)*SUMERR(I,J)/X)/(X-1.)) TWM 801
PRINT 2740, FLOINT(J),FLOINT(J+1),ICASE(I,J),AVGERR,AVABER,STDERR TWM 802
CONTINUE TWM 803
2180 IF (ICASE(I,25)) 2200,2190,2200 TWM 804
PRINT 2750, FLOINT(25),ICASE(I,25) TWM 805
GO TO 2210 TWM 806
2200 X=ICASE(I,25) TWM 807
AVGERR=SUMERR(I,25)/X TWM 808
AVABER=SARSER(I,25)/X TWM 809

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771 STDERR=SQRT((SERRSQ(I,25)-SUMERR(I,25)*SUMERR(I,25)/X)/(X-1.))
772 PRINT 2750, FLOINT(25), ICASE(I,25), AVGERR, AVABER, STDERR
773 X=0.
774 DO 2220 J=1,25
775 X=X+WEIGH(J)*FLOAT(ICASE(I,J))
776 CONTINUE
777 COEFF=SWEIFL(I)/(X*AVINFL*ATOTAL*60.5/FLOAT(JRINT))
778 PRINT 2760, COEFF
779 X=JDPY*1440/JRINT
780 CORRCO=(XY(I)-SUMY(I)*SUMY(I)/X)/SQRT((XX(I)-SUMX(I)*SUMX(I)/X)*
1Y(I)-SUMY(I)*SUMY(I)/X))
PRINT 2770, CORRCO
781 CONTINUE
782 CONTINUE
783 CONTINUE
784 DO 2250 I=1, ISEGMT
785 SUMX(I)=0.
786 SUMY(I)=0.
787 XX(I)=0.
788 YY(I)=0.
789 XY(I)=0.
790 SWEIFL(I)=0.
791 DO 2250 J=1,25
792 ICASE(I,J)=0
793 SUMERR(I,J)=0.
794 SABSER(I,J)=0.
795 SERRSQ(I,J)=0.
796 CONTINUE
797 GO TO 220
C
798 FORMAT (/1H 12F10.5)
799 FORMAT (89H0STATISTICAL ANALYSIS CANNOT BE PERFORMED WITHOUT RECORD
1DED STREAMFLOW. OPTION 2 IGNORED.)
800 FORMAT (20I4)
801 FORMAT (2I4,F4.3)
802 FORMAT (12F6.0)
803 FORMAT (12,14,2(12,4X),12,14,14F4.0)

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TWM R10
TWM R11
TWM R12
TWM R13
TWM R14
TWM R15
TWM R16
TWM R17
TWM R18
TWM R19
TWM R20
TWM R21
TWM R22
TWM R23
TWM R24
TWM R25
TWM R26
TWM R27
TWM R28
TWM R29
TWM R30
TWM R31
TWM R32
TWM R33
TWM R34
TWM R35
TWM R36
TWM R37
TWM R38
TWM R39
TWM R40
TWM R41
TWM R42
TWM R43
TWM R44

804	2320	809	2370	FORMAT (8F10.0)	TWM	R45
805	2330	810	2380	FORMAT (1P,2X,19A4)	TWM	R46
806	2340	811	2390	FORMAT (1H 38HPRECIPITATION TAPE FOR SEGMENT NUMBER 13,17H COMPLETE 1E FOR THE 12,12MTH MONTH, 19 12, 1M.)	TWM	R47
807	2350	812	2400	FORMAT (1H 12,F9.2, 13F9.4)	TWM	R48
808	2360	813	2410	FORMAT (4H0DAY2X,7HRAIN= 2X,6HINTER=1X,16H0VFRLAND STORAGE1X,18HDTWM 1EPRESSION STORAGE3X,13HU. Z. STORAGE3X,16HT. J. 7. STORAGE1X,17HINTWM 2TERFLOW STORAGE1X,9HPOTENTIAL1X,12HUNSAT EVAP0=)	TWM	R49
		814	2420	FORMAT (1H 6X,6HFALL 7HCEPTION4X,2HIN4(6X,3HOUT7X,2HIN),6X,3HOUT, TWM 13X,9HEVAPORAT.1X,11HRAATION POT./)	TWM	R50
		815	2430	FORMAT (1H 12,9X,F9.6,3(4X,F9.6,5X),3F9.6)	TWM	R51
		816	2440	FORMAT (1H17HTIME 024X,1X,F8.3,3(22X,F8.3))	TWM	R52
		817	2450	FORMAT (7H ENDING123(1H.))	TWM	R53
		818	2460	FORMAT (1H 6X,1H,120A1,2X,1H.)	TWM	R54
		819	2470	FORMAT (1H 1X,12,1X,12,1H,120A1,2X,1H.)	TWM	R55
		820	2480	FORMAT (1H 6X,123(1H.))	TWM	R56
		821	2490	FORMAT (1H 6X,1H0.25X,F8.3,3(22X,F8.3)////)	TWM	R57
		822	2500	FORMAT (1H 17HRECORDED FLOW IS F8.4,20H, SIMULATED FLOW IS F8.4)	TWM	R58
		823	2510	FORMAT (1H 46HTHE CORRELATION COEFFICIENT FOR THIS STORM IS F7.4)	TWM	R59
		824	2520	FORMAT (1H 17HPRECIPITATION IS F8.4,18HRECORDED FLOW IS F8.4,18HSTWM 1IMULATED FLOW IS F8.4)	TWM	R60
		825	2530	FORMAT (1H 10I10)	TWM	R61
		826	2540	FORMAT (1H 2I10,F10.4)	TWM	R62
		827	2550	FORMAT (1H144X,15HSEGMENT NUMBER 13,5X,23HYEAR ENDING SEP. 30, 19ITWM 12)	TWM	R63
		828	2560	FORMAT (1H015X,3H0CT6X,3HNOV6X,3HDEC6X,3HJAN6X,3HFEB6X,3HMAR6X,3HATWM 1PR6X,3HMAY6X,3HJUN6X,3HJUL6X,3HAUG6X,3HSEP 9X,6HANNUAL)	TWM	R64
		829	2570	FORMAT (1H 8HRAINFALL7X,12(F4.2,5X), 3X,F6.2)	TWM	R65
		830	2580	FORMAT (1H 2X,8H(INCHES))	TWM	R66
		831	2590	FORMAT (1H 11HEVAPORATION4X,12(F4.2,5X), 3X,F6.2)	TWM	R67
		832		FORMAT (1H 10HSTREAMFLOW5X,12(F4.2,5X), 3X,F6.2)	TWM	R68
				FORMAT (1H 3X,7HACRE=FT4X,12(F6.1,3X),1X,F8.1)	TWM	R69
				FORMAT (1H 2X,10HC.F.S.=DAY3X,12(F6.1,3X),F8.1)	TWM	R70
				FORMAT (1H 8H0VFRLAND)	TWM	R71
				FORMAT (1H 3X,10HFLOW(INCH)2X,12(F4.2,5X), 3X,F6.2)	TWM	R72
				FORMAT (1H 11HGROUNDWATER)	TWM	R73

833 2600 FORMAT (1H 3X,10HFLOW(INCH)2X,12(F4.2,5X), 3X,F6.2) TWM R81
 834 2610 FORMAT (1H 9HINTERFLOW6X,12(F4.2,5X), 3XF6.2) TWM R82
 835 2620 FORMAT (1H 9HUNDERFLOW6X,12(F4.2,5X), 3X,F6.2) TWM R83
 836 2630 FORMAT (1H050HEND OF MONTH VALUES OF STORAGE VOLUMES, IN INCHES.) TWM R84
 837 2640 FORMAT (1H 12HINTERCEPTION3X,12(F4.2,5X)) TWM R85
 838 2650 FORMAT (1H 7HSURFACE8X,12(F4.2,5X)) TWM R86
 839 2660 FORMAT (1H 10HUPPER ZONE 5X,12(F4.2,5X)) TWM R87
 840 2670 FORMAT (1H 12HINTERMEDIATE) TWM R88
 841 2680 FORMAT (1H 7X,4HZONE3X,12(F5.2,4X)) TWM R89
 842 2690 FORMAT (1H 11HGROUNDWATER4X,12(F6.2,3X)) TWM R90
 843 2700 FORMAT (1H 9HINTERFLOW6X,12(F4.2,5X)) TWM R91
 844 2710 FORMAT (1H055X,20HSTATISTICAL ANALYSIS) TWM R92
 845 2720 FORMAT (1H024HBASIC TIME INCREMENT IS 14,9H MINUTES.) TWM R93
 846 2730 FORMAT (1H03X,13HFLOW INTERVAL8X,5HCASES3X,10HAVG. ERROR3X,15HAVG. TWM R94
 1 ABS. ERROR2X,14HSTANDARD ERROR) TWM R95
 847 2740 FORMAT (1H F8.1,3H = F8.1,5X,F8.1,7X,F8.1,9X,F9.2) TWM R96
 848 2750 FORMAT (1H F8.1,3H = 13X,18,5X,F8.1,7X,F8.1,9X,F9.2) TWM R97
 849 2760 FORMAT (1H047HTHE COEFFICIENT OF WEIGHTED ABSOLUTE ERRORS IS F15.7 TWM R98
 1) TWM R99
 850 2770 FORMAT (1H058HTHE CORRELATION COEFFICIENT OF THE INTERVAL FLOWS IS TWM 900
 1 R = F6.4) TWM 901
 851 2780 FORMAT (1H015HSEGMNT NUMBER I3,5X,I2,10HTH MONTH, I2,43HTH DAY, TWM 902
 1TIME INTERVAL BETWEEN READINGS IS I4,39H MINUTES. FLOWS ARE IN C.TWM 903
 2F.S. STARTING/3H AT I2,9H HOURS, I2,9H MINUTES.) TWM 904
 852 2790 FORMAT (1H 23X,12F9.3) TWM 905
 853 2800 FORMAT (1H 23X,12F9.3,/) TWM 906
 854 2810 FORMAT (1H045HSIMULATED MEAN DAILY FLOW FOR SEGMENT NUMBER I3,5H FTWM 907
 1OR I2,12HTH MONTH, 19I2,10H IN C.F.S.) TWM 908
 855 2820 FORMAT (1H 13F10.1) TWM 909
 856 2830 FORMAT (1H0) TWM 910
 857 2840 FORMAT (14H TOTAL AREA = F6.1,6H ACRES5X,25HFRACTION PERVIOUS AREATWM 911
 1 = F5.3,5X,/68H FRACTION OF PERVIOUS AREA CONTRIBUTING RUNOFF DIRE TWM 912
 2CTLY TO STREAM = F5.3,/71H FRACTION OF PERVIOUS AREA CONTRIBUING TWM 913
 3RUNOFF TO DEPRESSION STORAGE = F5.3,5X,29HMAXIMUM DEPRESSION STORATWM 914
 4GE = F6.2,14H INCH PER UNIT/23H AREA OF PERVIOUS AREA,5X,31HMAXIMUTWM 915
 5M INTERCEPTION STORAGE = F6.2,6H INCH,5X,29HDEPTH OF INTERMEDIATE TWM 916

6ZONE = F6.1,4H IN./24H UPPER ZONE THICKNESS = F8.3,4H IN.5X,14HCHATTW 917
 7NNEL LAG = F4.2,5X,22HROOT ZONE THICKNESS = F5.2,7H INCHES) TWM 918
 1E)*F8.4,3H + F8.3) FORMAT (21H INFILTRATION RATE = F8.2,4H * (F8.4,19H - SOIL MOISTUR TWM 919
 1E)*F8.4,3H + F8.3) FORMAT (28H UNSATURATED PERMEABILITY = F8.2,19H *(SOIL MOISTURE + TWM 920
 1E)*F8.4,3H + F8.3) FFORMAT (26H SOIL MOISTURE TENSION = -E11.4,19H *(SOIL MOISTURE + FTWM 921
 1E)*F8.4,3H + F8.3) FFORMAT (89H MINIMUM VALUE OF STORAGE IN TOP INTERMEDIATE ZONE FOR TWM 922
 1E)*F8.4,3H + F8.3) FFORMAT (73H MINIMUM VALUE OF STORAGE IN GROUNDWATER STORAGE FOR OUT TWM 923
 1E)*F8.4,3H + F8.3) FFORMAT (38H DAILY RECESSON CONSTANTS ARE FROM15X,2HTO) TWM 924
 1E)*F8.4,3H + F8.3) FFORMAT (33X,11HGROUNDWATER7X,6HSTREAM8X,FR.2) TWM 925
 1E)*F8.4,3H + F8.3) FFORMAT (33X,11HGROUNDWATER6X,9HUNDERFLOW6X,F8.2) TWM 926
 1E)*F8.4,3H + F8.3) FFORMAT (34X,9HINTERFLOW8X,6HSTREAM8X,F8.2) TWM 927
 1E)*F8.4,3H + F8.3) FFORMAT (24H INTERFLOW BASE FLOW IS F8.2) TWM 928
 1E)*F8.4,3H + F8.3) FFORMAT (31X,15HTOP INTER.ZONE 4X,9HINTERFLOW6X,F8.2) TWM 929
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 930
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 931
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 932
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 933
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 934
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 935
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 936
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 937
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 938
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 939
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 940
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 941
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 942
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 943
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 944
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 945
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 946
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 947
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 948
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 949
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 950
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 951
 1E)*F8.4,3H + F8.3) FFORMAT (29H MANNINGS ROUGHNESS FACTOR = F6.4,5X,22HOVERLAND FLOW STWM 952

878

END

TWM 953

VARIABLE	INDEX STATEMENT NUMBER									
ABS	471,	472,	473,	474,	475,	677,	686,	688,		
AD	1,	29,	347,	347,	347,					
ADJUST	372,	373,	376,	390,	393,	395,	398,	406,		
	407,	413,	426,	431,	434,	445,	450,	455,		
	460,	461,	465,	473,	474,	475,	478,	480,		
	481,									
ALOG	79,	682,								
ALPHA	151,	475,								
AMAX1	261,	262,	623,	660,						
AMIN1	373,	389,	392,	394,	397,	406,	407,	413,		
	426,	431,	434,	473,	474,	475,	478,	480,		
	481,									
AMOD	354,									
AOLFDS	256,	267,	374,	385,	387,	406,	407,	407,		
	407,	474,	475,							
AOLFSP	256,	267,	374,	385,	387,	406,	406,	406,		
	407,	409,	411,	414,	474,	475,				
APERVS	256,	267,	371,							
ARCOS	349,									
ATOTAL	256,	263,	264,	264,	267,	490,	492,	501,		
	610,	611,	691,	692,	721,	777,				
AVABER	761,	763,	770,	772,						

AVGERR	760,	763,	769,	772,			
AVINFL	712,	777,					
B	28,	645,					
BASE	159,	555,					
BYZS	289,	307,	308,	309,	310,	311,	312,
	439,	443,	444,	445,	450,	453,	454,
	508,	526,					455,
BYZS1	2,	283,	289,	508,			
BLANK	28,	636,					
C	3,	140,	490,	492,			
CALL	59,	61,	64,	75,	171,	174,	178,
	184,	195,	197,	199,	210,	228,	328,
	330,	372,	373,	376,	390,	393,	398,
	406,	407,	413,	426,	431,	434,	450,
	455,	460,	461,	465,	473,	474,	475,
	480,	481,	537,	541,	545,		478,
CARRYO	3,	234,	506,				
CHANLG	256,	267,	496,	497,	499,	499,	
CHNLG	3,	102,	143,	529,			
COEFF	777,	778,					
COMMON	31,						
CONSUM	472,	476,	476,	478,	480,	481,	

CORRCO	614,	615,	780,	781,
CORREC	356,	358,	359,	360,
CORTZ	82,	359,	360,	
COS	348,	348,	366,	366,
COSH	348,	349,		
C1	256,	270,	384,	
C10	256,	273,	432,	433,
C11	256,	274,	457,	458,
C12	256,	276,	458,	
C13	256,	277,	456,	
C14	256,	278,	464,	
C15	256,	280,	464,	
C16	256,	279,	433,	
C2	256,	270,	384,	
C3	256,	271,	416,	436,
C4	256,	260,	271,	416, 436,
C5	256,	271,	416,	436,
C6	256,	271,	416,	436,

C7	256,	272,	419,	420,	439,
C8	256,	272,	419,	420,	439,
C9	256,	272,	419,	420,	439,
DAILYF	4,	501,	531,		
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DELINI	4,	283,	291,	509,	
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	440,	452,	456,	458,	464, 470,
DELTA	347,	348,	348,		
DEPTHI	256,	259,	259,	262,	267, 416, 420, 421,
	436,	439,	440,	452,	
DEPTHU	256,	258,	258,	261,	267, 416, 419, 421,
	477,	480,	481,		
DO	78,	86,	92,	101,	104, 111, 124, 144,
	147,	176,	193,	201,	212, 217, 233, 233,
	321,	326,	331,	336,	367, 487, 498, 505,
	535,	538,	550,	564,	594, 622, 632, 635,
	657,	668,	695,	703,	722, 728, 755, 774,
	784,	791,			

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DSMAX	256, 411,	267, 474,	292,	293,	294,	385,	386,	408,
DS1	4,	283,	286,	510,				
EOMGW	7,	527,	747,	747,				
EOMINC	6,	523,	742,	742,				
EOMINF	7,	528,	748,	748,				
EOMINT	6,	526,	746,	746,				
EOMSUR	6,	524,	743,	743,				
EOMUZ	6,	525,	744,	744,				
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ET	351,	353,	359,	360,				
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EVAP2	5,	94,	344,					
EVAP3	5,	95,	344,					
EXIT	75,							

E1	366.	470.										
E2	364.	366.	470.									
E3	365.	366.	470.									
E4	362.	364.	365.									
E5	363.	364.	365.									
FLOAT	88.	342.	345.	347.	467.	532.	610.	611.				
	658.	662.	669.	691.	692.	712.	727.	775.				
	777.											
FLOINT	8.	30.	79.	79.	757.	757.	763.	763.				
	766.	772.										
FLOMAX	620.	623.	623.	627.	628.	629.	630.	637.				
	638.	653.	654.	660.	660.	662.	662.					
GO	66.	155.	244.	352.	357.	382.	410.	427.				
	446.	479.	491.	570.	576.	582.	608.	648.				
	666.	681.	758.	767.	877.							
GWS	290.	455.	456.	457.	458.	460.	461.	511.				
	527.											
GWS1	9.	283.	290.	511.								
H	28.	28.	28.	28.	349.	359.	360.					
HOUR	467.	468.	469.	470.	470.							
I	35.	35.	36.	36.	39.	39.	40.	40.				
	44.	44.	44.	44.	77.	78.	79.	79.				

84,	85,	86,	87,	87,	87,	87,
88,	88,	93,	93,	93,	93,	93,
94,	94,	95,	95,	95,	95,	97,
97,	102,	105,	106,	107,	107,	108,
109,	112,	114,	115,	124,	124,	125,
126,	140,	145,	147,	148,	148,	165,
165,	179,	181,	195,	197,	197,	204,
207,	229,	234,	234,	234,	234,	321,
322,	332,	332,	333,	336,	336,	337,
337,	367,	467,	483,	488,	488,	505,
506,	531,	535,	536,	550,	550,	551,
553,	557,	565,	566,	567,	567,	568,
572,	574,	577,	578,	579,	579,	580,
581,	658,	659,	659,	660,	660,	668,
669,	695,	705,	706,	707,	707,	708,
709,	713,	718,	717,	717,	717,	719,
719,	723,	726,	729,	729,	729,	732,
732,	734,	736,	737,	737,	737,	739,
739,	742,	743,	744,	744,	744,	746,
746,	747,	748,	751,	756,	756,	757,
757,	759,	760,	762,	762,	762,	762,
763,	766,	769,	770,	771,	771,	771,
771,	775,	780,	780,	780,	780,	780,
780,	780,	780,	784,	785,	785,	786,
787,	789,	792,	793,	794,	794,	795,
148,	173,	180,	181,	186,	186,	187,
189,	194,	196,	197,	211,	211,	213,
214,	215,	221,	222,	225,	225,	232,
234,	241,	329,	330,	335,	335,	336,
337,	499,	499,	622,	623,	623,	623,
632,	634,	638,				
IA						
148,	173,	180,	181,	186,	186,	187,
189,	194,	196,	197,	211,	211,	213,
214,	215,	221,	222,	225,	225,	232,
234,	241,	329,	330,	335,	335,	336,
337,	499,	499,	622,	623,	623,	623,
632,	634,	638,				
IABS	59,					
327,	328,	540,	541,	544,	545,	583,
IAI						

IB	173,	179,	179,	180,	180,	180,	186,	186,
	187,	189,	189,	194,	194,	194,	196,	196,
	204,	211,	211,	221,	221,	221,	229,	229,
	230,	236,	236,	237,	237,	238,	241,	241,
	242,	327,	327,	329,	329,	329,	502,	502,
	540,	544,	544,	635,	635,	636,		
ICASE	10,	684,	684,	756,	756,	757,	759,	763,
	765,	768,	772,	775,	775,	792,		
IDAY	326,	330,	343,	501,	501,	502,	583,	587,
IDAYS	54,	59,	61,	64,	64,	135,	138,	147,
	171,	184,	193,	210,	210,	222,	228,	240,
	325,	531,	537,	538,	538,	727,		
IF	37,	47,	48,	55,	55,	56,	60,	62,
	74,	122,	128,	133,	133,	136,	137,	139,
	150,	158,	160,	163,	163,	164,	167,	168,
	169,	177,	183,	188,	188,	190,	191,	198,
	206,	231,	238,	240,	240,	248,	251,	255,
	263,	282,	292,	295,	295,	298,	301,	304,
	307,	350,	355,	370,	370,	378,	379,	380,
	386,	396,	399,	400,	400,	403,	408,	417,
	422,	429,	432,	437,	437,	441,	443,	448,
	453,	463,	468,	469,	469,	477,	483,	486,
	489,	547,	548,	555,	555,	559,	565,	566,
	567,	572,	573,	577,	577,	578,	579,	584,
	606,	619,	639,	641,	641,	643,	644,	646,
	655,	664,	677,	679,	679,	694,	749,	756,
IFILM	161,							
IFIRST	69,	139,	150,	255,	257,			

IFIX	346,	350,	662,	
IFLAG	539,	556,	559,	
Ihour	31,			
Ihours	585,	587,	633,	649,
IY	201,	202,	202,	202,
	218,	219,	224,	227,
	240,	243,	488,	489,
	492,	538,	541,	545,
IK	594,	595,	596,	597,
	599,	604,	604,	605,
IMIN	586,	587,	634,	649,
IND	680,	682,	684,	685,
	687,	687,	688,	686,
INew	72,	118,	148,	171,
	210,	221,	228,	236,
IOLD	71,	119,	171,	172,
	194,	210,	211,	228,
	537,	540,		
IOPT1	152,	158,	547,	584,
IOPT2	152,	663,	749,	
IOPT3	152,	160,	163,	548,
IOPT4	152,	164,	655,	
				619,
				213,
				217,
				218,
				230,
				236,
				490,
				492,
				598,
				598,
				599,
				686,
				686,
				184,
				187,
				204,
				502,
				537,
				186,
				329,

JRINT	63,	140,	141,	148,	177,	178,	198,
	199,	211,	221,	229,	236,	246,	247,
	251,	329,	483,	502,	532,	540,	544,
	585,	587,	610,	611,	633,	634,	658,
	669,	692,	712,	753,	777,	779,	
JRINT1	241,						
JSEGMT	123,	175,	180,	186,	187,	188,	189,
	190,	196,	211,	229,	241,	242,	329,
	540,						
JSIZE	196,	230,	232,	233,	329,	331,	335,
	497,	540,	544,	550,	564,	568,	569,
	668,						
JYR	41,	50,	50,	56,	59,	61,	64,
	65,	121,	131,	131,	133,	137,	148,
	167,	179,	181,	195,	197,	204,	221,
	236,	281,	328,	330,	502,	504,	530,
	541,	713,	751,				
JYR1	173,	180,	181,	186,	187,	189,	194,
	195,	197,	211,	229,	241,	242,	327,
	328,	330,	540,	541,	544,	545,	
K	722,	723,	728,	729,	729,		
KFIRST	560,	580,	585,	586,	588,	594,	600,
	602,	604,	605,	607,	609,	621,	622,
	625,	632,					
KSECND	561,	575,	581,	594,	600,	602,	603,
	606,	609,					
KSEGMT	51,	53,	59,	59,	61,	62,	64,

64,	65,	140,	142,	143,	148,	153,	171,
179,	184,	190,	204,	210,	221,	228,	234,
236,	266,	283,	283,	283,	283,	283,	283,
283,	283,	284,	285,	286,	287,	288,	289,
290,	291,	325,	327,	502,	506,	508,	509,
510,	511,	512,	513,	514,	515,	516,	517,
518,	519,	520,	521,	522,	523,	524,	525,
526,	527,	528,	529,	530,	537,	544,	587,
671,	671,	672,	672,	673,	673,	674,	674,
675,	675,	684,	684,	685,	685,	686,	686,
687,	687,	688,	688,				

LAG 140, 206, 209, 215, 217, 219,

LINE 626, 646, 650, 650,

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LYR 33, 34, 34, 46, 121,

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59, 61, 64, 65, 98, 117, 117, 120,
122, 128, 130, 135, 136, 139, 148, 150,
168, 174, 181, 195, 197, 204, 221, 236,
255, 265, 282, 328, 330, 344, 344,
472, 502, 504, 516, 517, 518, 519, 520,

521,	522,	523,	524,	525,	526,	527,	528,
530,	541,	545,	587,	694,			
MONTH1	173,	174,	179,	180,	181,	186,	187,
	191,	194,	195,	196,	197,	211,	229,
	242,	327,	328,	329,	330,	540,	541,
	545,						544,
NAMSEG	13,	165,					
NEWVAL	13,	97,	139,	150,	253,		
NRRELEM	140,	140,	226,	231,	232,	238,	487,
OFSAVG	401,	403,	404,	405,	405,		505,
OFSEOU	402,	403,	404,	405,			
OLF	405,	406,	407,	413,	414,		
OLS	285,	375,	376,	379,	389,	390,	392,
	400,	401,	406,	406,	407,	407,	512,
OLSI	375,	399,	401,				524,
OLS1	14,	283,	285,	512,			
PANFAC	151,	470,					
POTEVP	344,	366,					
PREC	15,	59,	61,	126,	196,	199,	202,
	218,	219,	327,	368,	658,		208,
PRINT	34,	36,	40,	52,	65,	265,	266,
	268,	269,	270,	271,	272,	273,	274,
							275,

276,	277,	278,	279,	280,	293,	296,	299,
302,	305,	308,	311,	504,	530,	531,	587,
604,	605,	612,	615,	618,	630,	631,	647,
649,	652,	653,	665,	693,	713,	714,	715,
716,	717,	718,	719,	720,	726,	732,	733,
734,	735,	736,	737,	738,	739,	740,	741,
742,	743,	744,	745,	746,	747,	748,	751,
752,	753,	754,	757,	763,	766,	772,	778,
781,							
PSI1	419,	421,	439,	440,	452,		
PSI2	420,	421,	440,				
R	28,	640,					
RAIN	368,	369,	370,	371,	372,	373,	374,
	376,	377,					
READ	33,	35,	39,	51,	63,	73,	77,
	84,	85,	97,	140,	151,	152,	82,
	165,	173,	180,	186,	194,	196,	161,
	241,	256,	283,	327,	329,	540,	229,
						544,	
REFLO	21,	64,	536,	544,	553,	566,	579,
	595,	597,	599,	599,	605,	623,	670,
REWIND	42,	43,	67,	68,	118,	119,	185,
	192,	324,				172,	
RO	341,	372,	406,	413,	460,	465,	484,
	486,	490,	492,	494,			485,
ROUGH	256,	268,	402,	405,			
RTZONE	151,	267,	477,	480,	481,		

S	28,	642,							
SABSER	20,	114,	686,	686,	757,	761,	770,	794,	
SATPRM	256,	268,	270,	381,	383,	384,	452,		
SCRAP	31,	84,	87,	88,	88,	90,	90,	91,	
	91,	93,	93,	93,	94,	94,	95,	95,	
	95,								
SDAYFL	340,	484,	484,	501,					
SEEP	381,	383,	384,	389,	391,	392,	394,	396,	
	397,								
SERRSQ	20,	115,	687,	687,	762,	771,	795,		
SEVAP	18,	517,	705,	717,	717,				
SGWF	19,	520,	708,	736,	736,				
SHIFTT	178,	199,							
SIF	19,	521,	709,	737,	737,				
SIMFLO	16,	125,	145,	148,	171,	173,	178,	179,	
	180,	184,	186,	187,	189,	194,	202,	202,	
	204,	210,	211,	214,	215,	215,	218,	221,	
	228,	229,	234,	234,	236,	241,	242,	325,	
	329,	332,	332,	490,	490,	496,	496,	497,	
	499,	499,	499,	502,	537,	540,	551,	596,	
	659,	669,							
SIN	348,	348,	351,	353,	362,	363,	470,		
SLOPE	256,	268,	402,	405,					

SUMY	20.	106.	672.	672.	780.	780.	780.	786.
SWEIFL	17.	110.	688.	688.	777.	790.		
SX1X1	592.	599.	599.	614.				
SX2X2	593.	598.	598.	614.				
TAPCHK	174.	181.	195.	197.	328.	330.	541.	545.
TB	359. 366.	361. 366.	361. 366.	363. 468.	364.	365.	365.	366.
TE	360. 366.	361. 366.	362. 469.	364.	365.	365.	366.	366.
TH	361.	362.	363.	366.	366.	470.		
TIZS	288. 393. 431. 448.	301. 397. 431. 449.	302. 398. 432. 450.	303. 416. 433. 481.	304. 420. 434. 481.	305. 424. 434. 513.	306. 425. 436. 526.	392. 426. 445.
TIZS1	22.	283.	288.	513.				
TO	66. 446. 666.	155. 479. 681.	244. 491. 758.	352. 570. 767.	357. 576. 877.	382. 582.	410. 608.	427. 648.
TPSWCH	171.	184.	210.	228.	325.	537.		
TRANPO	22.	85.	87.	87.	472.			
TRS	31. 338.	214. 492.	219. 492.	322. 506.	332. 554.	333. 557.	337. 565.	337. 572.

578,	597,	598,	598,	604,	623,	638,	659,
660,							
UNDFLO	76,	461,					
UZMIN	261,	269,	298,	299,	300,	429,	430,
	475,	475,	478,	480,			475,
UZS	287,	295,	296,	297,	298,	299,	300,
	384,	388,	390,	395,	416,	419,	426,
	429,	430,	431,	475,	475,	475,	478,
	480,	480,	514,	525,			
UZST	256,	268,	270,	295,	296,	297,	380,
	388,	426,	475,				384,
UZS1	23,	283,	287,	514,			
VINFLO	433,	434,					
VINSTG	284,	373,	373,	473,	473,	515,	523,
VINSTM	256,	267,	373,				
VINST1	24,	283,	284,	515,			
VIZMIN	262,	269,	301,	302,	303,	307,	308,
	424,	425,	434,	443,	444,	448,	449,
	454,	481,					453,
VIZST	256,	268,	304,	305,	306,	310,	311,
	392,	397,	431,	432,	433,		312,
VLENGH	256,	268,	402,	402,	405,	405,	
WEIGH	25,	79,	81,	81,	81,	688,	775,

WFSG	51,	52,	61,											
WRITE	44,	148,	179,	187,	189,	204,	221,	236,						
	242,	502,												
X	260,	261,	262,	343,	344,	344,	344,	344,	345,					
	345,	346,	347,	350,	351,	351,	353,	353,	353,					
	354,	355,	356,	358,	371,	372,	385,	385,	387,					
	389,	389,	391,	392,	394,	394,	396,	396,	397,					
	409,	411,	413,	414,	421,	422,	423,	423,	423,					
	424,	425,	426,	429,	430,	431,	440,	440,	441,					
	442,	442,	443,	444,	445,	448,	449,	449,	450,					
	452,	453,	454,	455,	458,	459,	460,	460,	464,					
	465,	466,	551,	552,	555,	557,	669,	669,	671,					
	673,	673,	675,	676,	721,	723,	725,	725,	727,					
	729,	731,	759,	760,	761,	762,	762,	762,	768,					
	769,	770,	771,	771,	773,	775,	775,	775,	777,					
	779,	780,	780,	780,										
XK	416,	417,	418,	421,	436,	437,	438,	440,						
XLAT	82,	83,	83,	348,	348,									
XLONG	82,	354,												
XN	600,	602,	614,	614,	614,									
XP	628,	630,	653,											
XU	613,													
XX	26,	107,	673,	673,	780,	787,								
XY	26,	109,	675,	675,	780,	789,								
X1	374,	376,	402,	562,	589,	596,	596,	610,						

	610,	612,	614,	614,	614,	614,	616,
X1Y1	591,	597,	597,	614,			
X11	533,	552,	552,	691,	691,	693,	693,
X12	534,	553,	553,	692,	692,	693,	
X2	563,	590,	595,	595,	611,	612,	613,
	614,	614,	614,	617,			
X4	389,	390,	391,	392,	392,	394,	395,
	396,	397,	397,	398,			
Y	388,	389,	394,	456,	461,	462,	670,
	672,	674,	674,	675,	676,	679,	682,
YESDAY	143,	496,	497,	529,			
YP	627,	630,	653,				
YY	27,	108,	674,	674,	780,	788,	
Y1	532,	551,	610,				
ZP	629,	630,	653,				

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1      SUBROUTINE TPSWCH (IOLD,INEW,ISEGMT,KSEGMT,IDAYS,X)
2      DIMENSION X(1440)
3      ITEMP=IOLD
4      IOLD=INEW
5      INEW=ITEMP
6      REWIND IOLD
7      REWIND INEW
8      IF (ISEGMT-1) 20,10,20
9      RETURN
10     READ (IOLD) MONTH1,IA,JYR1,JSEGMT,JRINT,JSIZE,(X(I),I=1,1440)
11     WRITE (INEW) MONTH1,IA,JYR1,JSEGMT,JRINT,JSIZE,(X(I),I=1,1440)
12     IF (JSEGMT+1-KSEGMT) 20,30,50
13     IF (IA-IDAYS) 20,40,50
14     RETURN
15     PRINT 60, JSEGMT,KSEGMT,IA,IDAYS
16     CALL EXIT
17     FORMAT (1H039HTROUBLE IN TAPE MANIPULATION. JSEGMT ISI3,11H, KSEGMT
18     1T ISI3,8H, IA IS I3,14H, AND IDAYS ISI3)
END
TWM 954
TWM 955
TWM 956
TWM 957
TWM 958
TWM 959
TWM 960
TWM 961
TWM 962
TWM 963
TWM 964
TWM 965
TWM 966
TWM 967
TWM 968
TWM 969
TWM 970
TWM 971
TWM 972

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VARIABLE INDEX STATEMENT NUMBER

CALL	17,							
EXIT	17,							
I	10,	10,	11,	11,				
IA	10,	11,	13,	15,				
IDAYS	1,	13,	15,					
IF	8,	12,	13,					
INew	1,	4,	5,	7,	11,			
IOLD	1,	3,	4,	6,	10,			
ISEGMT	1,	8,						
ITEMP	3,	5,						
JRINT	10,	11,						
JSEGMT	10,	11,	12,	15,				
JSIZE	10,	11,						
JYR1	10,	11,						
KSEGMT	1,	12,	15,					
MONTH1	10,	11,						
PRINT	15,							
READ	10,							

RETURN 9, 14,

REWIND 6, 7,

TPSWCH 1,

WRITE 11,

X 1, 2, 10, 11,

1
2
3
4
5
SUBROUTINE ADJUST (X,Y,Z)
X=X-Z
Y=Y+Z
RETURN
END

TWM 973
TWM 974
TWM 975
TWM 976
TWM 977

INDEX STATEMENT NUMBER

VARIABLE

ADJUST	1,		
RETURN	4,		
X	1,	2,	2,
Y	1,	3,	3,
Z	1,	2,	3,

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1 SUBROUTINE DATAIN (RECORD, MONTH, NYEAR, INCRE, MASS, KSEGMT, IRECOD, IDATA, 978
1YS, IFLAG) TWM 979
2 COMMON MAT(16),SCRAP(31), I HOUR(15), TRS(1440) TWM 980
3 DIMENSION RECORD(1440) TWM 981
4 DATA MDFSM/1HS/ TWM 982
5 400 FORMAT (1H 3I3, 8(3X, 14, 2X, F6, 3)) TWM 983
6 I SIZE=1440/INCRE TWM 984
7 DO 1000 I=1, 1440 TWM 985
8 RECORD(I)=0. TWM 986
9 CONTINUE TWM 987
10 DELT=FLOAT(INCRE) TWM 988
11 JDAY=1 TWM 989
12 IF (IRECOD-KSEGMT) 1340, 1010, 1340 TWM 990
13 READ 1480, N, UNIT, (MA7(I), I=1, 16) TWM 991
C***** N IS THE NUMBER OF EVENTS PER CARD TO BE READ UNDER FORMAT MAT, TWM
C***** TWM
C***** TWM
14 1020 LMINUT=0 TWM 992
15 SAVE1=0 TWM 993
16 READ MAT, IYEAR, IMONTH, IDAY, (I HOUR(I), I=1, N), MDF TWM 994
C PRINT 400, IYEAR, IMONTH, IDAY, (I HOUR(I), I=1, N) TWM
17 1040 IF (JDAY-IDAY) 1050, 1080, 1050 TWM 995
18 1050 CONTINUE TWM 996
19 WRITE (3) MONTH, JDAY, NYEAR, KSEGMT, INCRE, I SIZE, (RECORD(IA), IA=1, 144 TWM 997
10) TWM 998
20 IF (IFLAG) 1055, 1060, 1055 TWM 999
21 1055 WRITE (4) MONTH, JDAY, NYEAR, KSEGMT, INCRE, I SIZE, (RECORD(IA), IA=1, 144 TWM 1000
10) TWM 1001
22 1060 JDAY=JDAY+1 TWM 1002
23 LMINUT=0 TWM 1003
24 SAVE1=0. TWM 1004
25 DO 1070 I=1, 1440 TWM 1005
26 RECORD(I)=0. TWM 1006
27 CONTINUE TWM 1007
28 IF (JDAY-IDAYS) 1040, 1040, 1080 TWM 1008
29 1080 IF (IMONTH-MONTH) 1090, 1100, 1090 TWM 1009

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30 1090 RETURN TWM 1010
31 1100 IF (MDF-MDFSM) 1110,1420,1110 TWM 1011
32 1110 JDAY=IDAY TWM 1012
C***** TWM
C CONVERT UNITS WHEN NECESSARY TWM
C***** TWM
33 IF (UNIT-1.) 1120,1140,1120 TWM 1013
34 DO 1130 I=1,N TWM 1014
35 SCRAP(I)=SCRAP(I)*UNIT TWM 1015
36 CONTINUE TWM 1016
C***** TWM
C IF THE DATA IS IN ACCUMULATED FORM, CONVERT TO RATE. TWM
C***** TWM
37 1140 IF (MASS - 1) 1600, 1150, 1600 TWM 1017
38 1150 IF (SCRAP(1)) 1160,1160,1170 TWM 1018
39 1160 SAVE1=0. TWM 1019
40 1170 DO 1230 I=1,N TWM 1020
41 IF (I-1) 1190,1190,1180 TWM 1021
42 IF (IHOURL(I)-IHOURL(I-1)) 1240,1240,1190 TWM 1022
43 CONTINUE TWM 1023
44 SAVE=SCRAP(I) TWM 1024
45 MINUT=(IHOURL(I)/100)*60+MOD(IHOURL(I),100) TWM 1025
46 IF (MINUT-LMINUT) 1200,1220,1210 TWM 1026
47 1200 IF (SAVE) 1210,1210,1330 TWM 1027
48 1210 SCRAP(I)=(SCRAP(I)-SAVE1)/FLOAT(MINUT-LMINUT)*60. TWM 1028
49 LMINUT=MINUT TWM 1029
50 1220 SAVE1=SAVE TWM 1030
51 CONTINUE TWM 1031
52 CONTINUE TWM 1032
53 DO 1320 I=1,N TWM 1033
54 MINUT=(IHOURL(I)/100)*60+MOD(IHOURL(I),100) TWM 1034
55 IF (SCRAP(I)) 1270,1260,1270 TWM 1035
56 1260 JMINUT=MINUT TWM 1036
57 JSAVE=MINUT/INCRE+1 TWM 1037
58 GO TO 1320 TWM 1038
59 1270 J=MINUT/INCRE+1 TWM 1039

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60 1280 IF (J-JSAVE) 1290,1310,1300 TWM 1040
61 1290 IF (J-JSAVE) 1330,1300,1300 TWM 1041
62 1300 NMINUT=JSAVE+INCR TWM 1042
63 RECORD(JSAVE)=RECORD(JSAVE)+SCRAP(I)*FLOAT(NMINUT-JMINUT)/DELT TWM 1043
64 JMINUT=NMINUT TWM 1044
65 JSAVE=JSAVE+1 TWM 1045
66 GO TO 1280 TWM 1046
67 1310 RECORD(J)=RECORD(J)+SCRAP(I)*FLOAT(MINUT-JMINUT)/DELT TWM 1047
68 JMINUT=MINUT TWM 1048
69 CONTINUE TWM 1049
70 GO TO 1030 TWM 1050
71 1330 PRINT 1490, I, IDAY, I HOUR(I), I MONTH, I YEAR, J, JDAY, JMINUT, JSAVE, LMINUT TWM 1051
72 1T, MINUT, N, NMINUT TWM 1052
73 PRINT 1500, DELT, SAVE, SAVE1, SCRAP(I), UNIT TWM 1053
CALL EXIT TWM 1054
C***** REORDER FOR THIS SEGMENT HAS ALREADY BEEN USED, AND IS ON TWM
C TAPE 4, FIND IT, AND PLACE ON TAPE 3 TWM
C***** REORDER FOR THIS SEGMENT HAS ALREADY BEEN USED, AND IS ON TWM
C TAPE 4, FIND IT, AND PLACE ON TAPE 3 TWM
74 1340 CONTINUE TWM 1055
75 NN=99999999 TWM 1056
76 WRITE (4) NN TWM 1057
77 REWIND 4 TWM 1058
78 1350 READ (4) JMONTH, KDAY, IYEAR, JSEGMT, JINCR, JSIZE, (RECORD(I), I=1, 1440) TWM 1059
1) TWM 1060
79 IF (JMONTH-MONTH) 1350, 1360, 1350 TWM 1061
80 1360 IF (JSEGMT-RECORD) 1350, 1370, 1350 TWM 1062
81 1370 IF (KDAY-JDAY) 1350, 1380, 1350 TWM 1063
82 1380 WRITE (3) MONTH, JDAY, NYEAR, KSEGMT, INCR, I SIZE, (RECORD(IA), IA=1, 144) TWM 1064
10) TWM 1065
83 JDAY=JDAY+1 TWM 1066
84 IF (JDAY-IDAYS) 1350, 1350, 1390 TWM 1067
85 1390 REWIND 4 TWM 1068
86 1400 READ (4) MM TWM 1069
87 IF (MM-NN) 1400, 1410, 1400 TWM 1070
88 1410 BACKSPACE 4 TWM 1071

```

```

89          RETURN
C*****
C          DATA CARD IS FOR MEAN DAILY MEASUREMENT
C*****
90      DO 1460 I=1,N
91          JDAY=I*HOUR(I)
92          IF (JDAY) 1430,1470,1430
93      1430  X=SCRAP(I)*UNIT
94          DO 1440 II=1,ISIZE
95      1440  RECORD(II)=X
96          WRITE (3) MONTH,JDAY,NYEAR,KSEGMT,INCRE,ISIZE,(RECORD(IA),IA=1,144
10)
97          IF (IFLAG) 1450,1460,1450
98      1450  WRITE (4) MONTH,JDAY,NYEAR,KSEGMT,INCRE,ISIZE,(RECORD(IA),IA=1,144
10)
99      1460  CONTINUE
100         JDAY=JDAY+1
101         GO TO 1020
102         JDAY=I*HOUR(I-1)+1
103         GO TO 1020
C
104      1480  FORMAT (14,F6.0,16A4)
105      1490  FORMAT (1H 12I10)
106      1500  FORMAT (1H 5E20.6)
107      1600  I = 1
108      1605  MINUT = (I*HOUR(I)/100)*60 + MOD(I*HOUR(I),100)
109      1610  IF (FLOAT(MINUT)+SCRAP(I)) 1710, 1710, 1610
110      1610  IF (SAVE1 + SCRAP(I)) 1705, 1705, 1620
111      1620  ISTART = LMINUT + 1
112          L = 0
113          X = FLOAT(MINUT - LMINUT)
114          IF (X) 1330, 1705, 1650
115      1650  CONTINUE
116          DO 1700 K = ISTART, MINUT
117          L = L + 1
118          J = K / INCRE + 1
TWM 1072
TWM
TWM
TWM 1073
TWM 1074
TWM 1075
TWM 1076
TWM 1077
TWM 1078
TWM 1079
TWM 1080
TWM 1081
TWM 1082
TWM 1083
TWM 1084
TWM 1085
TWM 1086
TWM 1087
TWM 1088
TWM
TWM 1089
TWM 1090
TWM 1091
TWM 1092
TWM 1093
TWM 1094
TWM 1095
TWM 1096
TWM 1097
TWM 1098
TWM 1099
TWM 1100
TWM 1101
TWM 1102
TWM 1103

```

```

119 1700 RECORD(J) = RECORD(J) + ((SCRAP(I) * SAVE1) + FLOAT(L)/X * SAVE1) /
      DELT
120 1705 SAVE1 = SCRAP(I)
121 1710 LMINUT = MINUT
122 1710 I = I + 1
123 1710 IF (I-N) 1605, 1605, 1030
124 1710 END
      TWM 1104
      TWM 1105
      TWM 1106
      TWM 1107
      TWM 1108
      TWM 1109
      TWM 1110

```


JSAVE	57, 71,	60,	61,	62,	63,	63,	65,	65,
JSEGM	78,	80,						
JSIZE	78,							
K	116,	118,						
KDAY	78,	81,						
KSEGM	1,	12,	19,	21,	82,	96,	98,	
L	112,	117,	117,	119,				
LMINUT	14, 121,	23,	46,	48,	49,	71,	111,	113,
MASS	1,	37,						
MAT	2,	13,	16,					
MDF	16,	31,						
MDFSM	5,	31,						
MINUT	45, 67,	46, 68,	48, 71,	49, 108,	54, 109,	56, 113,	57, 116,	59, 121,
MM	86,	87,						
MOD	45,	54,	108,					
MONTH	1,	19,	21,	29,	79,	82,	96,	98,
N	13,	16,	34,	40,	53,	71,	90,	123,

NMINUT	62.	63.	64.	71.
NN	75.	76.	87.	
NYEAR	1.	19.	21.	82.
PRINT	71.	72.	96.	98.
READ	13.	16.	78.	86.
RECOD	80.			
RECORD	1.	3.	8.	19.
	67.	67.	78.	82.
	119.			
			21.	26.
			95.	96.
			63.	63.
			98.	119.
RETURN	30.	89.		
REWIND	77.	85.		
SAVE	44.	47.	50.	72.
SAVE1	15.	24.	39.	48.
	119.	120.	50.	72.
			110.	119.
SCRAP	2.	16.	35.	38.
	55.	63.	67.	72.
	120.			
			44.	48.
			109.	110.
			110.	119.
TO	58.	66.	70.	101.
			106.	
TRS	2.			
UNIT	13.	33.	35.	72.
			93.	93.
WRITE	19.	21.	76.	82.
			96.	98.

X 93, 95, 113, 114, 119,

```

1  SUBROUTINE DATAMG (RECORD,IPREC,WFSG,IDAYS,MONTH,JYR,KSEGMT)
2  DIMENSION RECORD(1440)
3  COMMON MAT(16),SCRAP(31),Ihour(15),TRS(1440)
4  REWIND 1
5  READ 200, LOOK,(MAT(I),I=1,16)
6  READ MAT,(SCRAP(I),I=1,16)
7  DO 40 I=1,16
8  BACKSPACE 3
9  CONTINUE
10
11 C*****
12 C*****
13 C*****
14 C*****
15 C*****
16 C*****
17 C*****
18 C*****
19 C*****
20 C*****
21 C*****
22 C*****
23 C*****
24 C*****
25 C*****
26 C*****
27 C*****
28 C*****

```

MONTH1, IDAY1, IYR1, JSEGMT, JRINT, JSIZE, (RECORD(I), I=1, 1440)
 MERGE RECORD FROM MIDNIGHT OF FIRST TO OBSERVATION TIME ON
 FIRST
 LOOK=(LOOK/100)*60+MOD(LOOK,100)
 K=LOOK/IPREC
 J=K+1
 SUM=0.
 DO 20 I=1,K
 SUM=SUM+RECORD(I)
 CONTINUE
 IF (SUM-.0001) 50,50,30
 SUM=(SUM*FLOAT(IPREC)/60)
 X=(SCRAP(1)/SUM-1.)*WFSG+1.
 DO 40 I=1,K
 RECORD(I)=RECORD(I)*X
 CONTINUE
 MERGE RECORD FROM OBSERVATION TIME ON FIRST DAY THROUGH
 OBSERVATION TIME ON LAST DAY.
 K=1440/IPREC
 NEXI1=IDAYS-1
 DO 130 I=2,NEXI1
 RFAD(3) MONTH1, IDAY1, IYR1, JSEGMT, JRINT, JSIZE, (TRS(IA), IA=1, 1440)
 SUM=0.

TWM 1111
 TWM 1112
 TWM 1113
 TWM 1114
 TWM 1115
 TWM 1116
 TWM 1117
 TWM 1118
 TWM 1119
 TWM 1120
 TWM 1121
 TWM 1122
 TWM 1123
 TWM 1124
 TWM 1125
 TWM 1126
 TWM 1127
 TWM 1128
 TWM 1129
 TWM 1130
 TWM 1131
 TWM 1132
 TWM 1133
 TWM 1134
 TWM 1135
 TWM 1136
 TWM 1137
 TWM 1138

```

29          L=J-1
30          DO 60 M=J,K
31             SUM=SUM+RECORD(M)
32             CONTINUE
33          DO 70 M=1,L
34             SUM=SUM+TRS(M)
35             CONTINUE
36          IF (SUM-.0001) 110,110,80
37          SUM=(SUM*FLOAT(IPREC)/60)
38          X=(SCRAP(I)/SUM-1.)*WFSG+1.
39          DO 90 M=J,K
40             RECORD(M)=RECORD(M)*X
41             CONTINUE
42          DO 100 M=1,L
43             TRS(M)=TRS(M)*X
44             CONTINUE
45          IR=I-1
46          WRITE (1) MONTH,IB,JYR,KSEGMT,IPREC,K,(RECORD(IA),IA=1,1440)
47          DO 120 M=J,K
48             RECORD(M-J+1)=TRS(M)
49             CONTINUE
50          CONTINUE
C*****
C          MERGE RECORD FROM OBSERVATION TIME ON LAST DAY TO MIDNIGHT ON
C          LAST DAY.
C*****
51          SUM=0.
52          DO 140 I=J,K
53             SUM=SUM+RECORD(I)
54             CONTINUE
55          IF (SUM-.0001) 170,170,150
56          SUM=(SUM*FLOAT(IPREC)/60)
57          X=(SCRAP(IDAYS)/SUM-1.)*WFSG+1.
58          DO 160 I=J,L
59             RECORD(I)=RECORD(I)*X
60          CONTINUE

```

```

TMM 1139
TMM 1140
TMM 1141
TMM 1142
TMM 1143
TMM 1144
TMM 1145
TMM 1146
TMM 1147
TMM 1148
TMM 1149
TMM 1150
TMM 1151
TMM 1152
TMM 1153
TMM 1154
TMM 1155
TMM 1156
TMM 1157
TMM 1158
TMM 1159
TMM 1160
TMM 1161
TMM 1162
TMM 1163
TMM 1164
TMM 1165
TMM 1166
TMM 1167
TMM 1168
TMM 1169
TMM 1170

```

```

61 170 WRITE (1) MONTH, IDAYS, JYR, KSEGMT, IPREC, K, (RECORD(IA), IA=1, 1440)
62 REWIND 1
63 DC 180 I=1, IDAYS
64 BACKSPACE 3
65 180 CONTINUE
66 DC 190 I=1, IDAYS
67 READ (1) MONTH, IDAYS, JYR, KSEGMT, IPREC, K, (RECORD(IA), IA=1, 1440)
68 WRITE (3) MONTH, IDAYS, JYR, KSEGMT, IPREC, K, (RECORD(IA), IA=1, 1440)
69 190 CONTINUE
70 RETURN
C
71 200 FORMAT (I4, 16A4)
72 END
TWM 1171
TWM 1172
TWM 1173
TWM 1174
TWM 1175
TWM 1176
TWM 1177
TWM 1178
TWM 1179
TWM 1180
TWM 1181
TWM 1182

```

INDEX STATEMENT NUMBER

VARIABLE

COMMON	3,								
DATAMG	1,								
DO	7, 47,	15, 52,	21, 58,	26, 63,	30, 66,	33,	39,	42,	
FLOAT	19,	37,	56,						
I	5, 16, 53,	5, 21, 58,	6, 22, 59,	6, 22, 59,	7, 26, 63,	10, 38, 66,	10, 45,	15, 52,	
IA	27, 68,	27, 68,	46,	46,	61,	61,	67,	67,	
IB	45,	46,							
IDAYS	1, 67,	6, 68,	7,	25,	57,	61,	63,	66,	
IDAY1	10,	27,							
IF	18,	36,	55,						
IMOUR	3,								
IPREC	1, 67,	12, 68,	19,	24,	37,	46,	56,	61,	
IYR1	10,	27,							
J	13,	29,	30,	39,	47,	48,	52,	58,	
JRINT	10,	27,							

REWIND	4,	62,					
SCRAP	3,	6,	20,	38,	57,		
SUM	14,	16,	16,	18,	19,	19,	20,
	31,	31,	34,	34,	36,	37,	37,
	51,	53,	53,	55,	56,	56,	57,
TRS	3,	27,	34,	43,	43,	48,	
WFSG	1,	20,	38,	57,			
WRITE	46,	61,	68,				
X	20,	22,	38,	40,	43,	57,	59,

```

1 SUBROUTINE SHIFTT (JRINT,LRINT,REC1)
2 DIMENSION REC1(1440)
3 COMMON MAT(16),SCRAP(31),Ihour(15),TRS(1440)
4 *****
5 C*****
6 C *****
7 C *****
8 C *****
9 C *****
10 C *****
11 C *****
12 C *****
13 C *****
14 C *****
15 C *****
16 C *****
17 C *****
18 C *****
19 C *****
20 C *****
21 C *****
22 C *****
23 C *****

```

SHIFTT CHANGES THE DATA IN REC1 FROM A TIME PASE OF LRINT
 MINUTES TO JRINT MINUTES. TRS IS USES AS A SCRATCH ARRAY
 THE DATA IS IN REC1 , AND WILL BE RETURNED IN REC1.

```

4 DO 10 I=1,1440
5 TRS(I)=REC1(I)
6 REC1(I)=0.
7 CONTINUE
8 L=1
9 J=1
10 LTIME=LRINT
11 JTIME=JRINT
12 IF (LTIME-JTIME) 30,50,40
13 LTIME=LTIME+LRINT
14 GO TO 20
15 JTIME=JTIME+JRINT
16 GO TO 20
17 ICYCLF=LTIME/MINO(JRINT,LRINT)
18 IC=(1440/LTIME)

```

IC IS THE NUMBER OF CYCLES IN THE DAY , WHERE A CYCLE IS
 SHORTEST PERIOD FOR WHICH A*JRINT = R*IRINT(ISEG) FOR A AND B
 BOTH INTEGERS.

```

19 IB=LRINT/JRINT
20 IF (IB-1) 120,60,60

```

CONDENSE RECORD TO SHORTER TIME BASE

```

21 J=1
22 JTIME=JRINT
23 DO 110 IA=1,IC

```

TWM 1183
TWM 1184
TWM 1185

TWM 1186
TWM 1187
TWM 1188
TWM 1189
TWM 1190
TWM 1191
TWM 1192
TWM 1193
TWM 1194
TWM 1195
TWM 1196
TWM 1197
TWM 1198
TWM 1199
TWM 1200

TWM 1201
TWM 1202

TWM 1203
TWM 1204
TWM 1205

```

24     LTIME=JTIME-JRINT*LRINT
25     L=LTIME/LRINT
26     DO 100 ID=1,ICYCLE
27     IF (JTIME-LTIME) 70,70,80
28     REC1(J)=REC1(J)+TRS(L)
29     GO TO 90
30     REC1(J)=REC1(J)+FLOAT(LTIME-JTIME+JRINT)/FLOAT(JRINT)*TRS(L)
31     REC1(J)=REC1(J)+FLOAT(JTIME-LTIME)/FLOAT(JRINT)*TRS(L+1)
32     L=L+1
33     LTIME=LTIME+LRINT
34     J=J+1
35     JTIME=JTIME+JRINT
36     CONTINUE
37     CONTINUE
38     RETURN
C*****
C     EXPAND RECORD TO LONGER TIME BASE
C*****
120    L=1
39     LTIME=LRINT
40     DO 170 IA=1,IC
41     JTIME=LTIME+LRINT+JRINT
42     J=JTIME/JRINT
43     DO 170 ID=1,ICYCLE
44     IF (LTIME-JTIME) 130,140,150
45     REC1(J)=REC1(J)+FLOAT(LTIME-JTIME+JRINT)/FLOAT(JRINT)*TRS(L)
46     GO TO 160
47     REC1(J)=REC1(J)+FLOAT(LRINT)/FLOAT(JRINT)*TRS(L)
48     GO TO 160
49     REC1(J)=REC1(J)+FLOAT(JTIME-LTIME+LRINT)/FLOAT(JRINT)*TRS(L)
50     REC1(J+1)=REC1(J+1)+FLOAT(LTIME-JTIME)/FLOAT(JRINT)*TRS(L)
51     J=J+1
52     JTIME=JTIME+JRINT
53     L=L+1
54     LTIME=LTIME+LRINT
55     CONTINUE
56     170

```

```

TWM 1206
TWM 1207
TWM 1208
TWM 1209
TWM 1210
TWM 1211
TWM 1212
TWM 1213
TWM 1214
TWM 1215
TWM 1216
TWM 1217
TWM 1218
TWM 1219
TWM 1220
TWM 1221
TWM 1222
TWM 1223
TWM 1224
TWM 1225
TWM 1226
TWM 1227
TWM 1228
TWM 1229
TWM 1230
TWM 1231
TWM 1232
TWM 1233
TWM 1234
TWM 1235
TWM 1236
TWM 1237
TWM 1238

```

57
58

RETURN
END

TWM 1239
-TWM 1240

VARIABLE INDEX STATEMENT NUMBER

COMMON	3,							
DO	4,	23,	26,	41,	44,			
FLOAT	30,	30,	31,	31,	46,	46,	48,	48,
	50,	50,	51,	51,				
GO	14,	16,	29,	47,	49,			
I	4,	5,	5,	6,				
IA	23,	41,						
IB	19,	20,						
IC	18,	23,	41,					
ICYCLE	17,	26,	44,					
ID	26,	44,						
IF	12,	20,	27,	45,				
IHOURL	3,							
J	9,	21,	28,	28,	30,	30,	31,	31,
	34,	34,	43,	46,	46,	48,	48,	50,
	50,	51,	51,	52,	52,			
JRINT	1,	11,	15,	17,	19,	22,	24,	30,
	30,	31,	35,	42,	43,	46,	46,	48,
	50,	51,	53,					
JTIME	11,	12,	15,	15,	22,	24,	27,	30,
	31,	35,	35,	42,	43,	45,	46,	50,

51,	53,	53,							
L	8,	25,	30,	31,	32,	32,	32,	39,	
	46,	48,	51,	54,	54,				
LRINT	1,	10,	17,	19,	24,	25,	25,	33,	
	40,	42,	48,	50,	55,				
LTIME	10,	12,	13,	13,	17,	18,	24,	25,	
	27,	30,	31,	33,	33,	40,	42,	45,	
	46,	50,	51,	55,	55,				
MAT	3,								
MIND	17,								
REC1	1,	2,	5,	6,	28,	28,	30,	30,	
	31,	31,	46,	46,	48,	48,	50,	50,	
	51,	51,							
RETURN	38,	57,							
SCRAP	3,								
SHIFTT	1,								
TO	14,	16,	29,	47,	49,				
TRS	3,	5,	28,	30,	31,	46,	48,	50,	
	51,								

ERROR IN I/O FROM COOP

```

1      SUBROUTINE TAPCHK(IDAY, IDAY1, MONTH, MONTH1, JYR, JYR1, MSG)
2      IF (IDAY-IDAY1) 50, 10, 50
3      IF (MONTH - MONTH1) 50, 20, 50
4      IF (JYR - JYR1) 50, 30, 50
5      RETURN
6      FORMAT (1H011HINSTEAD OF 12,1H/I2,1H/I2,1H/I2,10X)
7      1,8HMESSAGE 13)
8      PRINT 40,IDAY,MONTH,JYR,IDAY1,MONTH1,JYR1,MSG
9      CALL EXIT
      END
      TWM 1242
      TWM 1243
      TWM 1244
      TWM 1245
      TWM 1246
      TWM 1247
      TWM 1248
      TWM 1249
      TWM 1250
      TWM 1251

```


VARIABLE	INDEX STATEMENT NUMBER
CALL	8,
EXIT	8,
IDAY	1, 2, 7,
IDAY1	1, 2, 7,
IF	2, 3, 4,
JYR	1, 4, 7,
JYR1	1, 4, 7,
MONTH	1, 3, 7,
MONTH1	1, 3, 7,
MSG	1, 7,
PRINT	7,
RETURN	6,
TAPCHK	1,

I N P U T C A R D S

Card Number	Variables	Format	Comment Code
1	ISEGMT, ITAPE, IYR, LRY, MASS	5I4	
2A, 2B,.....	JPREC(1, 2,...ISEGMT)	20I4	
3A, 3B,.....	IREFOD(1, 2,...ISEGMT)	20I4	a
4	ISTGAG, KSEGMT, WFSG	2I4, F4.3	a
5	N, UNIT, MAT(1, 2,...16)	I4, F6.0, 16A4	a, b
6A, 6B,.....	IYEAR, IMONTH, IDAY, IHOURL(1), SCRAP(1), IHOURL(2), SCRAP(2) ...IHOURL(N), SCRAP(N), MDF	MAT	a, b
7	blank card to terminate Card 6 series		a, b
8	LOOK, MAT(1, 2,...16)	I4, 16A4	a, c
9A, 9B,.....	SCRAP(1, 2,...IDAYS)	MAT	a, c
10	JRINT	I4	a, d
11	N, UNIT, MAT(1, 2,...16)	I4, F6.0, 16A4	a, b, d
12A, 12B...	IYEAR, IMONTH, IDAY, IHOURL(1), SCRAP(1), IHOURL(2), SCRAP(2) ...IHOURL(N), SCRAP(N), MDF	MAT	a, b, d
13	blank card to terminate Card 12 series		a, b, d
14	IRUN	I4	
15	XLAT, XLONG, CORTZ	3F10.0	
16	SCRAP(2, 3,...13)	12F6.0	e
17	TRANPO(1, 2,...12)	12F6.0	e

Card Number	Variables	Format	Comment Code
18	NEWVAL(1, 2,...12)	12I4	k
19	KSEGMT, KRINT, ISEG1, ISEG2, NRELEM, LAG, C(1, 2,... NRELEM)	I2, I4, 2(I2, 4X), I2, I4, 14F4.0	
20	PANFAC, ALPHA, RTZONE	3F10.0	f
21	IOPT1, IOPT2, IOPT3, IOPT4	5I4	f
22	BASE	F10.0	f, g
23	IFILM	I4	f, h
24	LTR, NAMSEG(1, 2,...19)	I2, 2X, 19A4	f, i
25A	ATOTAL, APERVS, AOLFSF, AOLFDS, DSMAX, VINSTM, DEPTH1, DEPTHU	8F10.0	f
25B	CHANLG, C1, C2, C3, C4, C5, C6, C7	8F10.0	f
25C	C8, C9, C10, C11, C12, C13, C14, C15	8F10.0	f
25D	ROUGH, SLOPE, VLENGH, SATPRM, UZST, VIZST, C16, SMMIN	8F10.0	f
26	TIZS1, BIZS1, OLS1, DS1, UZS1, GWS1, DELIN1, VINST1	8F10.0	f, j

Comments:

- a. This card(s) should be omitted if ITAPE = 0
- b. This card(s) read by subroutine DATAIN and is repeated for each month
- c. This card(s) read by subroutine DATAMG and should be omitted when
ISTGAG \geq 0
- d. This card(s) should be omitted for IRECOD(KSEGMT) \geq 0
- e. This card contains the monthly pan evaporation
- f. Omit for NEWVAL(MONTH) = 0 except for October of the first water year

Comments continued:

- g. Omit for IOPT1 \neq 1
- h. Omit for IOPT3 \neq 1
- i. Omit when both IOPT1 and IOPT4 \neq 1
- j. Initial values for each segment
- k. NEWVAL must not be zero when more than one segment is involved

PREPARATION OF RAINFALL DATA

Rainfall data may be supplied in either an accumulated amount for the day (as read from a weighing type rain gauge) or in intensity.

The parameter MASS, read on card 1 signifies which type record is to be processed:

MASS = 1 for accumulated record

MASS \neq 1 for intensity type record.

The Parameter UNIT, read on card 5 is the constant needed to convert the rainfall record to units of inches per hour. When an accumulated type record is supplied with volume in inches and time in hours and minutes (military time), the value of UNIT is one.

The month, day, and year occupy the first three fields of the precipitation card. The time of the beginning of the precipitation followed by a zero are next. Time of observation and amount of observation alternate until the end of the card is reached. The last column of the card must be read, and must not contain a \$ symbol except for cards containing mean daily values (see section on Preparation of Streamflow Data). When a storm cannot be punched on one card, the following card must contain the month, day, and year information. The observation time and amount alternate across the card as for the first card.

When a storm extends past midnight, a value must be supplied at 2400 hours. The following day's amounts should start at zero for weighing type gauges.

A blank card terminates each month's data.

PREPARATION OF STREAMFLOW DATA

Subroutine DATAIN reads both precipitation and streamflow; consequently, the material in the preceding section is also true for streamflow data. The parameter UNIT is the number needed to convert the input data into units of cubic feet per second.

Mean daily flows may be read in by means of a special symbol, the \$ symbol, in the last field of the card. The first three fields must contain the month, the day corresponding to the first mean daily flow on this card, and the year. The remainder of the card to the last field may be filled with the day in the field normally occupied by the time and the day's mean daily flow values in the amount field. If the string of days for which mean daily flows are to be entered exceeds a single card, the next card must contain the same type information as the first. Cards containing mean daily flow and detailed streamflow may be mixed as desired. The special symbol differentiates between the two.

APPENDIX II

GLOSSARY OF MAIN PROGRAM VARIABLES

AD()	An array containing the apparent declination of the sun
ALPHA	The input variable determining the ability of the soil to deliver moisture to the surface for evaporation
AOLFDS	Area producing overland flow to Depression Storage as a fraction of the total pervious area in the watershed
AOLFST	Area producing overland flow to stream as a fraction of the total pervious area of the watershed
APERVS	Pervious area as a fraction of the total area in the watershed
ATOTAL	Total area in the watershed, negative if in square miles, positive if in acres
AVABER	Average absolute error
AVGERR	Average error
AVINFL	Average interval flow in inches per time interval
B	A symbol used in the on line plot operation
BASE	The value of stream flow, in c.f.s., below which everything is considered to be base flow; no storm analysis or plots are given for these low flows
BIZS	Bottom Intermediate Zone Storage, in inches
BIZS1	The value of Bottom Intermediate Zone Storage at the beginning of the month
BLANK	A symbol used in the on line plot operation
C()	An array containing the elements of the time delay histogram
CARRYO(,)	An array containing the flow generated in one month but which occurs as stream flow during the next month
CHANLG	Channel lag, the ratio of this routing interval's flow to the last routing interval's flow

CHNLG()	An array containing the flow lagged in the channel lagging operation during the last time increment of the day
COEFF	The coefficient of weighted absolute errors
CORRCO	The correlation coefficient for flow on a routing interval time base
CORREC	A correction for the rising and setting of the sun due to the location of the watershed with respect to the principal meridan
CORTZ	A correction for time zone when the basin is in the wrong time zone with respect to the actual location of the time zone boundaries
COSH	The cosine of the angle H in an equation for the rising and setting of the sun taken from page 403, <u>Explanatory Supplement to the Ephemeris</u>
C1, C2	Constants in the infiltration equation
C3, C4, C5	Constants in the equation for determining the unsaturated permeability
C7, C8, C9	Constants in the equation for defining the capillary potential
C10	The fraction of the Top Intermediate Zone which must be filled before transfer to Interflow Storage
C11, C12	Constants in the equation for flow from groundwater to the stream
C13	Constant in the equation for flow from groundwater to underflow
C14, C15	Constants in the equation used to lag flow through Interflow Storage
C16	Constant in the equation for transfer of water from Top Intermediate Zone to Interflow Storage
DAILYF()	Simulated mean daily flow
DELINF	The volume of water stored in Interflow Storage
DELIN1()	The volume of water in Interflow Storage at the beginning of the month

DELT	The time, in hours, of the basic operation cycle
DELTA	Angle of declination of the sun used in computing the time of sunrise and sunset, and is computed from AD()
DEPTHI	Thickness of Intermediate Zone, in feet
DEPTHU	Thickness of Upper Zone, in feet
DS	Depression Storage, in inches
DSMAX	Maximum Depression Storage, in inches
DS1()	The Depression Storage at the beginning of the month
EOMGW(,)	End of month Groundwater Storage
EOMINC(,)	End of month Interception Storage
EOMINF(,)	End of month Interflow Storage
EOMINT(,)	End of month Intermediate Zone Storage
EOMSUR(,)	End of month Surface Storage, the sum of Overland Flow Storage and Depression Storage
EOMUZ(,)	End of month Upper Zone Storage
ERR	The difference between simulated flow and recorded flow for the time interval
ET	The time of the day when the sun is directly overhead; used in computing sunrise and sunset
EVAPOT	Evaporation potential for period, in inches
EVAP1()	A constant in a parabolic interpolation equation for evaporation from monthly data
EVAP2()	See EVAP1()
EVAP3()	See EVAP1()
E1, E2, E3, E4, E5	Constants in evaporation rate equation from van Bavel's data
FLOINT()	Flow interval used in statistical analysis, based on logarithmic increments

FLOMAX	The maximum flow to be plotted
GWS	Groundwater Storage
GWS1()	The Groundwater Storage at the beginning of the month
H	One half of the daylight hours, used in computing sunrise and sunset
HOURL	The hour of the day, used in computing the evaporation potential rate
I	An index
IA	An index
IAI	Dummy variable used in place of the day of the month when reading data from tape or disks
IB	An index
ICASE(,)	The number of cases of flow within each class for each segment
IDAY	An index corresponding to days of the month
IDAYS	The number of days in the month being processed
IFILM	A flag used to signal use of film for plotting (IFILM = 1)
IFIRST	A flag used to indicate if the current month is October of the first year (IFIRST = 1)
IFLAG	A flag which is set to 1 if flow above BASE is encountered during the month in the simulated flow
IHOUR()	An array used to store on a temporary basis the time of occurrence of rainfall and runoff events being read from cards
IHOURS	The time of beginning of runoff from a storm
II	An index
IK	An index
IMIN	The minutes portion of the time

IND	An index used to classify the flow into the proper interval for statistical analysis
INEW, IOLD	Tape or disk numbers, alternates between units 2 and 4
IOPT1	If 1, print out simulated flow on a time base of JRINT for all flows above a base flow, BASE
IOPT2	If 1, print statistical data
IOPT3	If 1, plot storm hydrographs on Calcomp plotter
IOPT4	If 1, plot monthly stream flow
IOPT5	If 1, recorded streamflow available on tape 3
IPREC()	The time base, in minutes, to be used in preparing the precipitation tape
IR	Variable denoting position of the recorded flow in the on line plot routine
IRECOD()	A flag indicating whether there is recorded streamflow to read (+, 0, no streamflow data) and whether the rainfall data for this segment has already been read (IRECOD(KSEGMT) = KSEGMT, data not yet read in; IRECOD(KSEGMT) \neq KSEGMT, data for KSEGMT was read for segment IRECOD(KSEGMT) and is on tape 4)
IRINT	A dummy variable used in place of JRINT when reading tape or disk
IRINT1	The routing interval, in minutes, of the segment just upstream of the segment being processed
IRUN	A flag to indicate if a run is to be made (IRUN = 1)
IS	Variable denoting position of the simulated flow in the on line plot routine
ISCRTH	The scratch tape number, specified as unit 1
ISEGMT	The number of segments to be processed each month
ISEG1	Segment number of upstream segment
ISEG2	Segment number of upstream segment
ISIZE	The number of IPREC time units in the month

ISTGAG	The number of storage gauges in the segment
IT	The time period, in minutes, of the basic cycle
ITAPE	If ITAPE is 1, the precipitation tape is to be prepared
IYR	The first year of the first water year to be processed
J	An index
JAD	An index
JDPM()	An array containing the days of the months
JDPY	Days per year
JDTD	Days to Date
JFIRST	A dummy variable used to protect the value of KFIRST during the printing of the individual storms
JJ	The number of values of simulated flow to be printed on one line
JPLOT()	The array used to store the symbols for printing the on line plot
JPREC()	The time base, in minutes, to be used in preparing the precipitation tape
JRINT	The time period, in minutes, to be used in simulating the streamflow, hence the routing interval
JRINT1	A dummy variable used in place of JRINT when reading tape or disc
JSEGMT	The counter for the number of segments which have been processed
JSIZE	A dummy variable used in place of ISIZE when reading tape or disks
JYR	The year being processed
JYR1	A dummy variable used in place of JYR when reading tape or disks
K	An index

KFIRST	The location in the SIMFLO array of the first element of a string of elements characterized by magnitudes greater than BASE
KSECND	The location of the last element in the string described under KFIRST
KSEGMT	The number of the segment being processed
LAG	The time of flow through the segment in units of Jrint minutes
LINE	The counter of the number of lines printed in the on line plot
LOOP	The number of basic cycles in a day
LTR	The number of letters to be plotted as the name of the segment
LYR	The second year of the last water year to be processed
MASS	Equal 1 if precipitation data is from weighing gauge and hence is in form of sum of rain to date
MAT()	A variable format used to read in data
MONTH	The month being processed, January equal 1
MONTH1	A dummy variable used in place of MONTH when reading tape or disks
NAMSEG()	The array containing the name of the segment to be plotted under the plot of stream flow
NEWVAL()	An array used to control the reading of new data. When NEWVAL is zero for a particular month, the physical parameters from the last month will be used. Can be used only when one segment is being modeled
NRELEM	The number of routing elements, C
OFSAVG	Average Overland Flow Storage for the period
OFSEQU	Equilibrium Overland Flow Storage
OLF	Overland Flow, in inches
OLS	Overland Flow Storage, in inches

OLSI	Overland Flow Storage at beginning of current period, in inches
OLSI()	Overland Flow Storage at the beginning of the month
PANFAC	A coefficient to be multiplied by the monthly pan evaporation to obtain monthly evaporation potential
POTEVP	The potential evaporation for a day
PREC()	The array containing the precipitation data, sometimes used for scratch storage
PSI1, PSI2	The capillary potential in the various soil zones
R	The symbol used for recorded flow in the on line plot routine
RAIN	The volume of water falling in the time period, in inches
RECFLO()	The recorded stream flow, may be any units, convert with UNIT, of the subroutine DATAIN
RO	The volume of runoff generated in the segment during the current time period
ROUGH	Manning's roughness factor for overland flow
RTZONE	The thickness of the root zone in inches
S	The symbol used for simulated flow in the on line plot routine
SABSER(,)	The sum of the absolute value of the error for the segment for the flow interval
SATPRM	The saturated permeability of the soil, in inches per hour
SCRAP()	An array used for temporary storage when reading data cards
SDAYFL	The volume of runoff for one day, in inches
SEEP	The infiltration volume for the time period in inches
SERRSQ(,)	The sum of the square of the errors for the segment for the flow interval

SEVAP(,)	The sum of the evaporation for the segment for the month, in inches
SGWF(,)	The sum of the groundwater flow for the segment for the month, in inches
SIF(,)	The sum of infiltration for the month, in inches
SIMFLO()	The volume of stream flow in acre-inches for the time period
SLOPE	The slope of overland flow
SMMIN	The minimum soil moisture for all zones, in inches ³ per inch ³
SOLF(,)	The sum of overland flow for the segment for the month, in inches
SRAIN(,)	The sum of precipitation for the segment for the month, in inches
SSEVAP	A dummy variable used to accumulate the monthly sum of evaporation, and again to accumulate the annual sum of evaporation
SSF()	Monthly sum of streamflow for the segment, initially in inches, converted to second-feet days, and then to acre-feet
SSGWF	Dummy variable used to accumulate groundwater flow for month and later for year
SSIF	Dummy variable used to accumulate monthly sum and then annual sum of interflow
SSOLF	Dummy variable used to accumulate monthly sum and then annual sum of overland flow
SSRAIN	Dummy variable used to accumulate monthly sum and then annual sum of precipitation
SSSF	Dummy variable used to accumulate monthly sum and then annual sum of stream flow
SSUF	Dummy variable used to accumulate monthly sum and then annual sum of underflow
STDERR	Standard error of estimate of the simulated flow

SUF(,)	The monthly sum of underflow for the segment
SUMERR(,)	The sum of the error for the segment for the flow interval
SUMX()	The sum of the simulated flow for the segment for the year
SUMY()	The sum of the recorded flow for the segment for the year
SWEIFL()	Sum of the weighted absolute error for the segment for the year
TB	The time of beginning of evaporation, in hours
TE	The time of ending of evaporation, in hours
TM	The time of maximum evaporation, in hours
TIZS	Top Intermediate Zone Storage, in inches
TIZS1()	The Top Intermediate Zone Storage at the beginning of the month, in inches
TRANPO()	The difference between pan evaporation and consumptive use for each month
TRS()	A scratch array used at various parts of the program to hold precipitation or streamflow data temporarily
UNDFLO	Accumulator for water assigned to underflow
UZMIN	Minimum moisture in Upper Zone Storage, in inches
UZD	Upper Zone Storage, in inches
UZST	The total Upper Zone Storage available, in inches
UZS1()	The Upper Zone Storage at the beginning of the month, in inches
VINFLO	The volume of water transferred from Intermediate Zone Storage to Interflow Storage in any time period, in inches
VINSTG	Interception Storage, in inches
VINSTM	Maximum Interception Storage, in inches

VINST1()	Interception storage at the beginning of the month, in inches
VIZMIN	Minimum Moisture in either half of Intermediate Zone, in inches
VIZST	Maximum intermediate Zone Storage, in inches
VLENGH	The length of overland flow, in feet
WEIGH()	The weighting factors assigned to each flow interval in the statistical analysis. Defined as the logarithm of the midpoint of the interval plus one
WFSG	Weighting factor for the storage gauge
X	A dummy floating point variable
XK	The unsaturated permeability in the various soil zones, in inches per hour
XLAT	The latitude of the basin, in degrees
XLONG	The longitude of the basin, in degrees
XTCKS	The spacing of tick marks on the time axis of the plot (Calcomp plot routine)
XP	One of the scale values printed on the on line plot
XX()	The sum of the square of the simulated stream flow
XY()	The sum of the product of the simulated stream flow and the measured flow
X1	The volume of rain reaching the two areas producing overland flow; the accumulator for individual storm simulated flow
X11	The accumulator for the monthly simulated flow
X12	The accumulator for the monthly recorded flow
X2	The accumulator for the individual storm recorded flow
X4	A dummy floating point variable
Y	A dummy floating point variable

YESDAY	The value of the lagged portion of the streamflow generated during the last time increment of the previous day
YP	One of the scale values printed on the on line plot
YY()	The sum of the square of the measured stream flow
Y1	A factor used in converting inches per time period to c.f.s.
ZP	One of the scale values printed on the one line plot

VITA

Billy Joe Claborn was born in Brown County, Texas, on June 17, 1930, the son of Pearl Leona Chambers Claborn and Garnet Weldon Claborn. Upon completion of his work at Leverett's Chapel High School, Overton, Texas, in 1948, he studied for one year at Draughon's Business College in Dallas, Texas. He spent his freshman year at East Texas State Teachers College, and was drafted into the Army in February, 1951, after completing one semester of work at Kilgore Junior College. Discharged in February 1953, he entered Texas Technological College that fall, and received a Bachelor of Science in Civil Engineering in May, 1956. He married Nancy Ann Keele in 1954. Following a summer of employment with the City of Lubbock, he entered Stanford University in the fall of 1956 and received a Master of Science degree in June, 1957. Employment with Kern County Land Company in Bakersfield, California, was accepted following graduation from Stanford and continued until 1961. During this period two sons were born, Garnett Weldon II, and Forrest Jay. The next two years were spent in Denver, Colorado, in the employ of Tipton and Kalmbach, Inc., Consulting Engineers. During this period a third son, Mitchell Lee, was born. Appointment to the faculty of Texas Technological College, School of Engineering, Department of Civil Engineering with the rank of Assistant Professor became effective September, 1963. A leave of absence was obtained for the academic years 1966-67, 1967-68 to pursue the doctoral degree at The University of Texas at Austin. Course work was completed during this two year period. A daughter, Alison Jane was added to the family. Teaching duties at Texas

Technological College were resumed in September, 1968, with the rank of Associate Professor. Publications consists of the following:

"Testing and Maintaining Deep Wells" with Mel Jans. Presented at Northwest Well Drillers Association Meeting, 1959.

"Tolerable Water Shortages During Droughts for Various Uses" Texas Water Commission Bulletin 6512, 1965.

"The Stanford Watershed Model as a Tool for Estimating Continuous Streamflow Hydrographs" with Walter L. Moore. Presented at the Spring Meeting Texas Section, American Society of Civil Engineers, Houston, Texas, 1968.

"Application of Continuous Accounting Techniques to Evaluate the Effects of Small Structures on Mukewater Creek, Texas" with Erdal Coskun and Walter L. Moore. Presented at the Conference on The Effects of Watershed Changes on Streamflow, Austin, Texas, 1968.

"Numerical Simulation of Watersheds Using Physical Soil Parameters" with Walter L. Moore. Presented at the Thirteenth Congress of the International Association of Hydraulic Research, Kyoto, Japan, 1969.

"Numerical Model of the Ogallala as a Management Tool" with T. Al Austin and D. M. Wells. Presented at the Ogallala Aquifer Symposium at Lubbock, Texas, 1970.

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