

# Compressive strength and durability properties of ceramic wastes based concrete

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**Abstract** This paper presents an experimental study on the properties and on the durability of concrete containing ceramic wastes. Several concrete mixes possessing a target mean compressive strength of 30 MPa were prepared with 20% cement replacement by ceramic powder ( $W/B = 0.6$ ). A concrete mix with ceramic sand and granite aggregates were also prepared as well as a concrete mix with natural sand and coarse ceramic aggregates ( $W/B = 0.5$ ). The mechanical and durability performance of ceramic waste based concrete are assessed by means of mechanical tests, water performance, permeability, chloride diffusion and also accelerated aging tests. Results show that concrete with partial cement replacement by ceramic powder although it has minor strength loss possess increase durability performance. Results also shows that concrete mixtures with ceramic aggregates perform better than the control concrete mixtures concerning compressive strength, capillarity water absorption, oxygen permeability and chloride diffusion. The replacement of

cement and aggregates in concrete by ceramic wastes will have major environmental benefits.

**Keywords** Concrete · Ceramic powder · Ceramic aggregates · Compressive strength · Durability performance

## 1 Introduction

In Europe the amount of wastes in the different production stages of the ceramic industry reaches some 3–7% of its global production meaning millions of tons of calcined-clays per year that are just land filled [1]. With increasing restrictions on landfills in European Union area, the cost of deposition will increase and the industries will have to find ways for reusing their wastes. Although the reutilization of ceramic wastes and has been practiced, the amount of wastes reused in that way is still negligible. Hence, the need for its application in other industries is becoming absolutely vital. Construction industry can be the end user of all ceramic wastes and in this way can contribute to solve this environmental problem. The nature of construction industry, especially the concrete industry, is such that ceramic wastes can be used safely with no need for dramatic change in production and application process. On one hand, the cost of deposition of ceramic waste in landfill will be saved and, on the other, raw materials and natural

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resources will be replaced, thus saving energy and protecting the environment. According to some authors the best way for the construction industry to become a more sustainable one is by using wastes from other industries as building materials [2, 3]. The production of cement requires high energy input (850 kcal per kg of clinker) and implies the extraction of large quantities of raw materials from the earth (1.7 tonnes of rock to produce 1 tonne of clinker). On the other hand the production of one tonne of cement generates 0.55 tonnes of chemical CO<sub>2</sub> and requires an additional 0.39 tonnes of CO<sub>2</sub> in fuel emissions, accounting for a total of 0.94 tonnes of CO<sub>2</sub> [4]. Therefore, the replacement of cement in concrete by ceramic wastes represents a tremendous saving of energy and has important environmental benefits. Besides, it will also have a major effect on decreasing concrete costs, since the cost of cement represents more than 45% of the concrete cost. Several authors already confirmed the pozzolanic reactivity of ceramic wastes [5–7], nevertheless, research carried out so far by reusing ceramic wastes in concrete are scarce and do not evaluate concrete durability performance which is a key issue [8, 9]. Consequently the aim of this research project is the assessment of the mechanical and durability properties of ceramic waste based concrete.

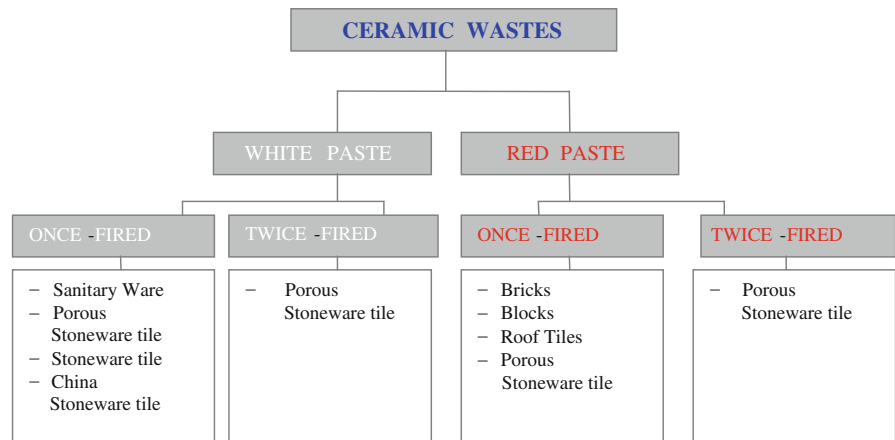
## 2 Experimental work

### 2.1 Materials and concrete mix design

Ceramic wastes can be separated in two categories in accordance with the source of raw materials. The first one are all fired wastes generated by the structural ceramic factories that use only red pastes to manufacture their products, such as brick, blocks and roof tiles. The second one is all fired waste produced in stoneware ceramic such as wall, floor tiles and sanitary ware. These producers use red and white pastes, nevertheless, the usage of white paste is more frequent and much higher in volume. In each category the fired ceramic waste was classified according to the production process. This classification is reported in the following diagram (Fig. 1). The chemical compositions of ceramic pastes were analysed and results obtained are reported in Table 1. It is well established that the chemical composition of

fired ceramic products is not significantly different of the one of raw materials used to make these products. Only the mineralogical composition is modified when these materials are heated. The silica and alumina are the most significant oxides present in the ceramic pastes. The variation of proportion of the silica and alumina is due to the clay used. It should be noted that the red paste shows high proportion of iron oxide responsible for the red colour of the products. In order to determine the mineralogical composition the representative samples were analysed by XRD in a Philips diffractometer PW3710 with CuK $\alpha$  radiation, using secondary monochromator, automatic divergence slit and a receiving slit of 0.2°. Routine conditions of 40 kV and 20 mA in range of 5°–75°, with steps of 0.025° and 1 s, were used. Table 2 shows the results the mineralogical composition of ceramic wastes. As expected quartz and feldspars essentially compose ceramic wastes. Approximately 500 kg of wastes from a Portuguese ceramic company was crushed with a jaw crusher to make the ceramic aggregate. Thus, by using this system to crush ceramic wastes is possible to obtain coarse aggregates, fine aggregates and ceramic powder that after sieving can be used without additional work and with minimal cost implications. At laboratory scale the ceramic wastes were ground with a ball mill with metal balls to obtain ceramic powder. The cost of grinding is related to the amount of material grounded. For 20% cement replacement represents 3.75 of the cost of Portland cement. This indicates saving of around 17% in the cost of Portland cement in concrete. The cost of cement represents almost 45% of the concrete cost. Therefore, over all cost of concrete will be reduced by more than 7.5%. This is a considerable gain in the direct savings of materials costs. The cost of deposition and land occupied by land fill has not been considered and will be an additional gain in efficiency and savings. The ceramic powders with a particle size <75  $\mu$ m were chosen for the partial substitution of cement. The coarse aggregate and the sand used in the concrete mixes with cement replacement by ceramic powder were crushed granite aggregates. The water absorption, particle size distribution, density and the fineness modulus of the aggregates were determined following tests methods described in the NP EN 12620 [10]. Table 3 shows gradation and physical characteristics of these aggregates. For the production of concrete samples in

**Fig. 1** Classification of ceramic wastes by type and production process



**Table 1** Chemical composition of ceramic pastes (%)

Type	SiO <sub>2</sub>	Al <sub>2</sub> O <sub>3</sub>	Fe <sub>2</sub> O <sub>3</sub>	CaO	MgO	Na <sub>2</sub> O	K <sub>2</sub> O	TiO <sub>2</sub>	Other
Red paste twice fired ceramic	51.7	18.2	6.1	6.1	2.4	0.2	4.6	0.8	9.9
White paste once-fired ceramic	58.0	18.0	1.0	8.3	0.6	0.2	1.2	0.8	11.9
White paste for twice-fired ceramic	59.8	18.6	1.7	5.5	3.5	1.6	2.5	0.4	6.4
Red paste for stoneware tile	49.1	20.3	7.7	1.2	1.1	0.4	4.2	0.9	15.1
White paste for stoneware tile	65.0	21.3	1.3	0.2	0.3	2.5	3.7	0.2	5.5
White paste for sanitary ware	65.8	22.2	0.6	0.1	0.1	1.0	3.5	0.3	6.4

**Table 2** Mineralogical composition of ceramic wastes

Sample	Majors	Minors	Traces
Ceramic brick	Q	He, C, Ah, Mv, R	Fd
Overheated brick	Q, Fd4	Mv, Cr, Ah, R, He	Lm, G
White roof tile	Q, Mv, Fd4	Ah, He, Cr	G, Hr, Cs, R
Red roof tile	Q, I(Mv)	C, Fd	Ah, He
Ceramic table for cover	Q, Fd	An, He, R	M, E
White porous stoneware tile in double baking	Q	Fd2, Px	Cr, He, R-Cu, G
Red porous stoneware tile in single baking	Q	Zr, He, Fd2	SF, Cr, Mu, Hr, R, Mg
White stoneware tile	Q, Fd4	Cr, ICN, G, Ha	Zr, Mu, He, Ti
Red stoneware tile	Q, Fd1	Cr, Mu, Px	Ah, Zr, He, Mg, Lm
China stoneware tile	Q	Fd1, Cr, Px	Cu, He

Ah anhydrite, C calcite, Co cordierite, Cs celsian, Cr cristobalite, Cu corundum, E esseneite, Fd feldspars, Fd1 albite Ca, ord, Fd2 anorthite Na, ord, Fd3 orthoclase, Fd4 anorthoclase, G gehlenite, Ha hauyne, He hematites, Hr hercynite, I illite, Lm lime, M mullite, Mg magnetite, Mv muscovite, Px piroxene, Q quartz, SF franklinite, Ti titanite, Zr zircon

phase A, Portland cement, CEM I-32.5, complying with the NP EN 197-1 [11] was used along with both natural sand and granite aggregates. A mix design, with characteristic strength  $f_c = 25 \text{ N/mm}^2$  and a target mean strength  $f_m = 30 \text{ N/mm}^2$  was studied. Four mixes with 20% replacement of cement by

ceramic powder were also prepared. Each one is named after the source of the ceramic waste: ceramic bricks (CB); White stoneware twice-fired (WSTF); sanitary ware (SW); White stoneware once-fired (WSOF). The compositions of the concrete mix used in phase A are described in Table 4. The equivalent ceramic

aggregate concrete for each mix was obtained by replacing the natural aggregates with an equal volume of crushed ceramic, everything else remained the same. To have a direct comparison of concrete made with crushed-ceramic aggregate with conventional concrete, two corresponding mixes were prepared one replacing natural sand by ceramic sand (MCS) and other where coarse granite aggregate were replaced by ceramic coarse aggregate (MCCA). Because all other parameters were constant for the three mixes, it was expected that the effect of the type of coarse aggregate would be reflected in any differences in concrete properties. Table 5 shows gradation and physical characteristics of aggregates used in phase B. The ceramic aggregates were soaked in water for 24 h and then air-dried to a saturated surface dry condition before mixing with other ingredients. For the production of concrete samples in phase B, Portland cement, CEM I-42.5, complying with the NP EN 197-1 [11] was used. The compositions of the concrete mix used in phase B are described in Table 6. Mixing was done in a drum mixer, and workability of the fresh concrete was measured with a standard slump cone [12] immediately after mixing and a slump of 80 mm was obtained. This low workability is due to the high water absorption of recycled aggregates. The test specimens were cast in steel moulds and compacted on a vibrating table. Concrete specimens were demoulded 1 day after cast.

## 2.2 Experimental procedures

### 2.2.1 Compressive strength

The compressive strength was determined following the NP EN 12390-3 [13]. The specimens were cured under water at a temperature equal to  $18 \pm 1^\circ\text{C}$  until they have reached the testing ages. Tests were performed on  $100 \times 100 \times 100 \text{ mm}^3$  specimens. Compressive strength for each mixture was obtained from an average of three cubic specimens determined at the age of 7, 14, 28, 56 and 90 days of curing.

### 2.2.2 Vacuum water absorption

Specification to evaluate vacuum water absorption is based on water saturation of the sample under vacuum (Fig. 2). The test was performed by using cylindrical specimens 4 cm high and with 5 cm

diameter. Vacuum water absorption test is carried out in two main steps: step one in which the specimens are submitted to vacuum conditions (0.7 mbar) during 3 h and step two, the specimens are saturated with distilled water and again submitted to vacuum conditions during 3 h.

Absorption is determined as follows:

$$A (\%) = \frac{W_{\text{sat}} - W_{\text{dry}}}{W_{\text{sat}} - W_{\text{wat}}} \times 100 \quad (1)$$

where A is the absorption, in percentage,  $W_{\text{sat}}$  weight of the saturated specimen,  $W_{\text{dry}}$  weight of the dry specimen and  $W_{\text{wat}}$  is the weight of the saturated specimen immersed in water.

### 2.2.3 Capillary water absorption

The test consists in placing the cylindrical specimens in a container with a thin water lamina in order to maintain immersed just one of the sides of the specimen. Preparation of test specimens is done as follows: after drying in an oven at  $105^\circ\text{C}$  for 48 h, they are waterproof along the lateral surface with a fine layer of silicon in order to reduce water evaporation and guarantee capillarity water absorption. The test specimens are then placed on desiccators, for some hours, to allow the hardening of the silicon. This test is carried out according to the Portuguese Standard LNEC E393 [14]. Water absorption has been measured after (10, 30, 60, 90, 120, 180, 360, 480, 1440, 2880 and 4320) minutes. Capillarity water absorption coefficient was obtained from an average of three specimens.

### 2.2.4 Oxygen permeability

To evaluate the oxygen permeability a permeability cell was used (Fig. 3). This permeability cell allows to submitting specimens, with 4 cm high and 5 cm diameter, to a certain pressure, guaranteeing that the flow of oxygen through the specimen is uniaxial. The oxygen permeability (being  $\eta$ —oxygen dynamic viscosity =  $2.02 \times 10^{-5} \text{ N s/m}^2$ ) can be determined as follow:

$$K = \frac{4.04 \times R \times L \times 10^{-16}}{A \times (P_2^2 - 1)} \quad (2)$$

where R is the oxygen flow through the specimen ( $\text{cm}^3/\text{s}$ ), L thickness of the test specimen (m), A area

**Table 3** Gradation and physical characteristics of aggregates used in phase A

Sieve size	Cumulative percentage passing		
	Fine sand	Sand	Coarse aggregate
25.4	100	100	100
19.0	100	100	100
12.7	100	100	98.5
9.5 mm	100	100	70.1
4.76 mm	100	99.8	13.6
2.38 mm	98.3	79.4	2.5
1.19 mm	89.4	55.7	2.2
590 $\mu\text{m}$	66.9	37.2	2.0
297 $\mu\text{m}$	37.3	20.5	1.8
149 $\mu\text{m}$	13.6	8.3	1.4
74 $\mu\text{m}$	4.5	3.9	0.8
Fineness modulus	1.946	2.991	6.063
Density ( $\text{kg/m}^3$ )	2658	2631	2619
Water absorption (%)	1.3	1.6	2.0

of the section crossed by oxygen ( $\text{m}^2$ ),  $P_2$  oxygen pressure at the forefront of specimen (bar), being the outlet pressure of 1 bar,  $K$  is the intrinsic oxygen permeability ( $\text{m}^2$ ).

### 2.2.5 Water permeability

The water permeability uses the permeability cell, in a similar way and with in the same specimen. The water permeability coefficient can be determined as follow:

$$k_w = \frac{d_p^2 \cdot \delta}{2 \cdot h \cdot t} \quad (3)$$

where  $k_w$  is the water permeability coefficient ( $\text{m s}^{-1}$ ),  $d_p$  water penetration depth (m),  $\delta$  absorption of the test specimen (open porosity),  $t$  time that took

to penetrate to the depth  $d_p$ , and  $h$  is the height of water column (m), 1 bar = 10.207 m of water column.

### 2.2.6 Chloride diffusion test

This test method consists of determining the depth of penetration of chloride ions through 50 mm thick slices of 110 mm nominal diameter cylinders (Fig. 4). A potential difference of  $30 \pm 0.2$  V is maintained across the specimen. One face is immersed in a sodium chloride and sodium hydroxide solution, the other in a sodium hydroxide solution. The chloride diffusion coefficient can be calculated using the following equation:

$$D = (RTL/zFU) \cdot [X_d - (\alpha\sqrt{X_d})/t]$$

where

$$\alpha = 2\sqrt{(RTL/zFU)} \cdot \text{erf}^{-1}(1 - 2C_d/C_o)$$

$D$  is the diffusion coefficient ( $\text{m}^2/\text{s}$ ),  $z$  absolute valence of the ion involved, for chloride ion,  $z = 1$ ,  $F$  Faraday constant,  $F = 9.648 \times 10^4$  J/(V mol),  $U$  absolute potential difference (V),  $R$  constant of ideal gases,  $R = 8.314$  J/(K mol),  $T$  solution temperature (K),  $L$  thickness of specimen (m),  $X_d$  depth of penetration (m),  $t$  duration of the test (s),  $\text{erf}^{-1}$  inverse of error function,  $c_d$  chloride ion concentration with which the colour changes, and  $c_o$  is the concentration of chloride ion in the sodium chloride solution.

### 2.2.7 Durability aging test

The aging test performed consists of twelve cycles; each cycle has four steps and one conditioning period (Fig. 5; Table 7). The different phases have been selected to simulate dry heat, cold weather and wet heat

**Table 4** Concrete mix proportions per cubic meter of concrete used in phase A

Control mix		Concrete mixtures with ceramic wastes	
Portland cement	350.0 kg	Portland cement	280.0 kg
Coarse aggregate	1084.0 kg	Ceramic waste powder	70.0 kg
Fine sand	135.4 kg	Coarse aggregate	1084.0 kg
Sand	564.0 kg	Fine sand	135.4 kg
Water	218.5 l	Sand	564.0 kg
W/C	0.6	Water	218.5 l

**Table 5** Gradation and physical characteristics of aggregates used in phase B

Sieve size	Cumulative percentage passing			
	Sand	Ceramic sand	Coarse aggregate	Ceramic coarse aggregate
25.4	100	100	100	100
19.0	100	100	100	100
12.7	100	100	97.1	99.2
9.5 mm	100	100	76.2	72.2
4.76 mm	98.6	99.0	13.5	2.8
2.38 mm	78.2	71.9	4.9	0.7
1.19 mm	58	48.8	4.0	0.6
590 $\mu\text{m}$	38.6	31.5	3.4	0.5
297 $\mu\text{m}$	21.6	19.0	2.7	0.5
149 $\mu\text{m}$	9.6	9.6	2.0	0.4
74 $\mu\text{m}$	4.4	4.1	1.3	0.3
Density ( $\text{kg/m}^3$ )	2610	2210	2643	2263
Water absorption (%)	1.5	6.1	1.4	6.0

**Table 6** Concrete mix proportions per cubic meter of concrete in phase B

Materials	Control	MCS—mix with ceramic sand	MCCA—mix with ceramic coarse aggregate
CEM-I 42.5	350	350	350
Sand	861	–	861
Ceramic sand	–	729	–
Coarse aggregate	958	958	–
Ceramic coarse aggregate	–	–	820
Water	175	175	175
W/C	0.5	0.5	0.5

and cold. Further the rapid change of temperature from 75 to  $-10^\circ\text{C}$  and then from  $-10$  to  $55^\circ\text{C}$  and dropping quickly to  $-10^\circ\text{C}$  in each cycle to simulate the thermal shock that concrete can suffer. Figure 5 represents the scheduling of daily sequence. Following the curing period, all the specimens were weighed and half of the specimens were submitted to twelve cycles of aging 24 h long each, the remaining specimens were kept in the curing chamber at constant  $21^\circ\text{C}$ . All specimens were subjected to compressive strength test.

### 3 Results and discussion

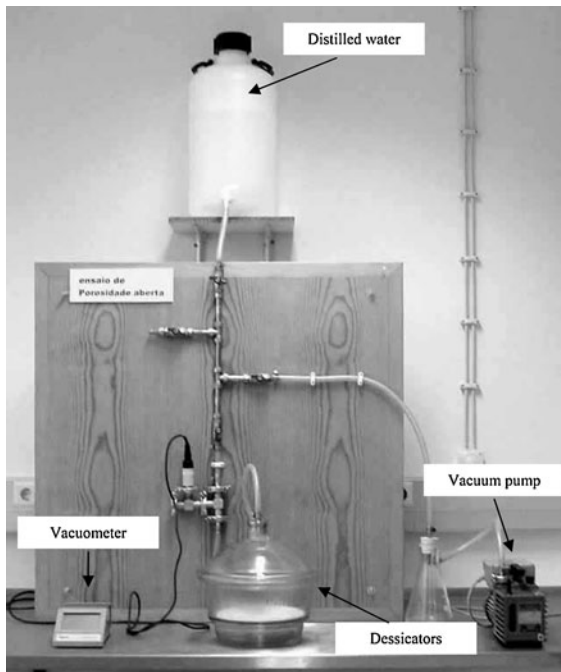
#### 3.1 Ceramic powder as partial replacement of cement

##### 3.1.1 Compressive strength

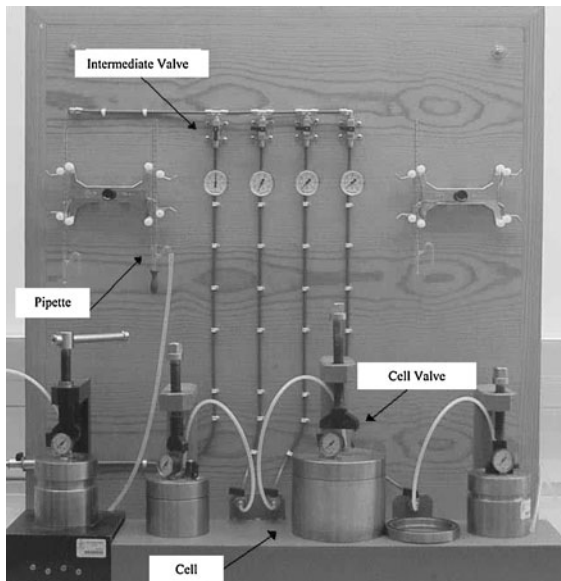
The results obtained indicate, as expected, large differences in early curing ages and smaller differences at long curing ages (Fig. 6). The concrete mixture with 20% of CB waste has the highest compressive strength performance for all ceramic wastes which means it has higher pozzolanic reactivity. The concrete mixture with WSOF wastes has the worst mechanical performance at early ages, representing 74.8% of control strength for 7 days curing. For 28 days curing that concrete mixture has almost 80% of the control strength index. However, for 90 days curing the compressive strength activity index of the WSOF mixture reaches 90.4%. For long curing ages concrete mixtures with 20% cement replacement has minor the strength loss. Since higher curing temperatures increase the rate of pozzolanic activity pre-fabrication industry may be an ideal way to their employment. This is because higher curing temperatures are used in concrete pre-fabrication for accelerated demoulding.

##### 3.1.2 Durability performance

With the exception of the CB mixture which has a 5% higher vacuum water absorption than the control mixture, all the others have lower vacuum water absorption than the control mixture (Fig. 7). As for the oxygen permeability (Fig. 8) it can seem that two mixes (CB and SW) shows values slightly higher than the control while two others mixes indicate values smaller (WSTF and WSOF). The differences, however, are between 6 and 12% in comparison with control mix. The results related to water permeability are shown in Fig. 9. All the concrete mixtures reusing ceramic wastes as cement replacement perform better than the control mixture. Different ceramic wastes have different water permeability. WSOF wastes have the lower water permeability and since this wastes have the lower strength index meaning a less dense microstructure the reduction in water permeability may be due to the hydration of unreacted particles during the water permeability test. Chloride



**Fig. 2** Water absorption test



**Fig. 3** Oxygen and water permeability cells

ion diffusion is shown in Fig. 10. All mixtures with ceramic wastes performed better than the control, although SW mixture has almost the same diffusion coefficient of the control. As for the durability aging test (Fig. 11), results indicate that all mixtures

without exception have higher durability performance which confirms the positive impact of the ceramic additions.

### 3.2 Ceramic aggregates replacing traditional aggregates

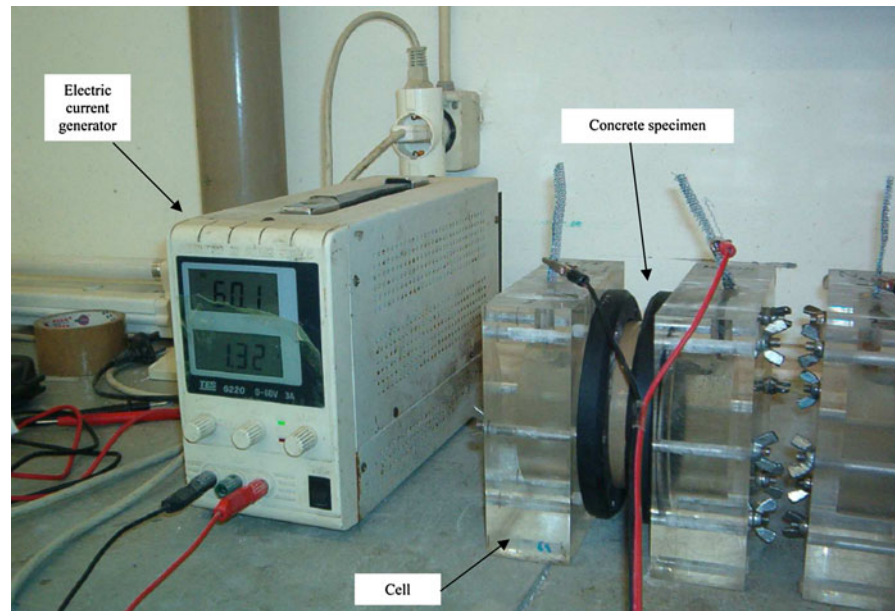
#### 3.2.1 Compressive strength

The results obtained (Fig. 12) indicate that the strength is higher for concrete with both replacements coarse ceramic aggregate (MCCA) and ceramic sand (MCS) than control concrete with traditional aggregates. This behaviour although confirmed by some authors [15, 16], is opposite to the one obtained by others using low percentage crushed brick aggregates [17, 18]. One explanation for that may be due to the fact that they used brick aggregates with a water absorption level between 16 and 19%, which is much higher than the water absorption of the ceramic aggregates used in the present investigation (6%). Another explanation relates to the fact that ceramic aggregates used in this investigation are used in a pre-saturated state so they can provide water for cement hydration. Nevertheless, the strength performance is higher for concrete mixtures with ceramic sand beyond 14 days curing. As for concrete mixtures with coarse ceramic aggregate a maximum strength level is attained at 28 days curing.

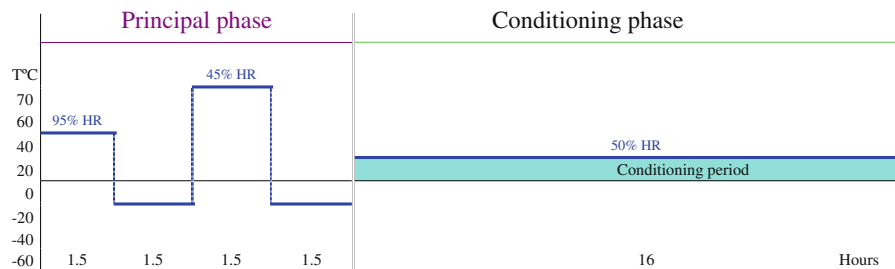
#### 3.2.2 Durability performance

Coarse ceramic aggregates based concrete has higher water absorption under vacuum than traditional aggregate concrete (Fig. 13). This difference is 11% higher for concrete with ceramic sand and it is almost 20% higher for concrete with coarse ceramic aggregates. Concrete with ceramic sand replacing natural aggregates (MCCA) has higher water absorption than the control concrete but also higher compressive strength. This can be explained by a higher hydration degree of the cement matrix related to the higher water absorption of ceramic sand. As for concrete with coarse ceramic aggregates on can assume that the extra water content lead to a better internal curing than the control concrete. However, this gain is lost by the probable existence of porous interface transition zones near the ceramic aggregates. As for capillary water absorption (Table 8) the differences

**Fig. 4** Chloride diffusion test



**Fig. 5** Schedule of the sequence for the durability aging test



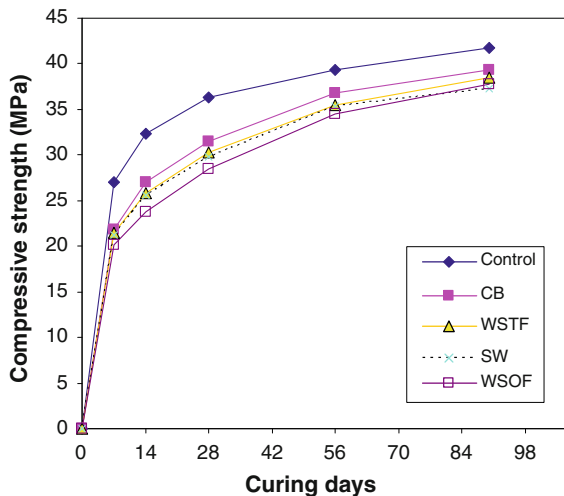
**Table 7** Durability aging test phases

Exposure	Temperature (°C)	Relative humidity (%)	Duration (min)
<b>Principal phase</b>			
Dry heat	75 ± 2	45	90
Cold	-10 ± 2	-	90
Wet heat	55 ± 2	95	90
Cold	-10 ± 2	-	90
<b>Conditioning phase</b>			
Conditioning	23 ± 2	50	960

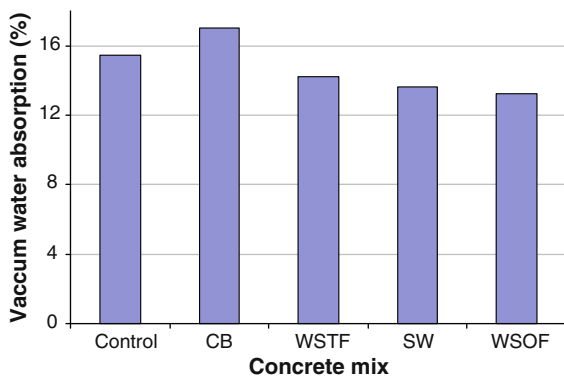
are rather different since control concrete absorption almost doubles the capillary water absorption of ceramic based concrete which means that ceramic aggregate based concrete has superior durability performance. With the exception of the water permeability (Fig. 14), the results of oxygen permeability and chloride diffusion (Fig. 15) confirms the good performance of ceramic based concrete. As for the durability aging test (Fig. 16) compressive

strength reduction is higher for ceramic based concrete. Nevertheless, compressive strength after the aging test is still higher in ceramic based concrete. Therefore, the replacement of traditional sand by ceramic sand in concrete production represents important environmental benefits. It avoids the extraction of large quantities of raw materials from the earth, also avoids energy costs and landfill problems. Since the penetration of oxygen, water,



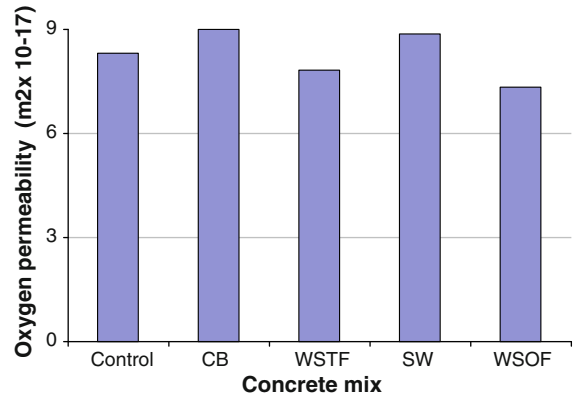


**Fig. 6** Compressive strength of concretes with ceramic powder at different curing ages (*Control* control concrete, *CB* concrete with ceramic brick powder, *WSTF* concrete with ceramic powder from stoneware twice-fired, *SW* concrete with ceramic powder from sanitary ware, *WSOF* concrete with ceramic powder from stoneware once-fired)

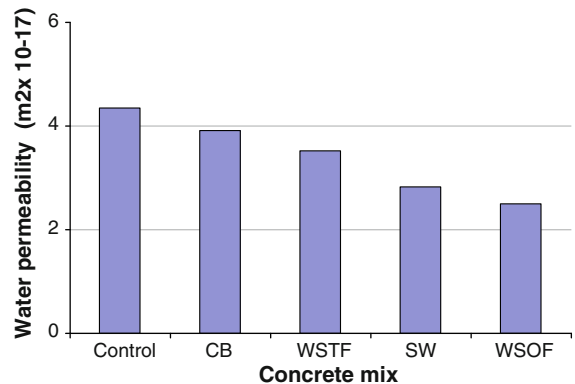


**Fig. 7** Water absorption of concretes with ceramic powder (*Control* control concrete, *CB* concrete with ceramic brick powder, *WSTF* concrete with ceramic powder from stoneware twice-fired, *SW* concrete with ceramic powder from sanitary ware, *WSOF* concrete with ceramic powder from stoneware once-fired)

chloride and other aggressive ions into concrete is the most important factor in the physical and chemical process of concrete deterioration [19], ceramic sand based concrete does not compromise the durability requirements. The higher ceramic sand concrete durability means less environmental impacts, for instance if we increase concrete durability from 50 to 500 years, we would reduce the environmental impacts by a factor of 10 [20].



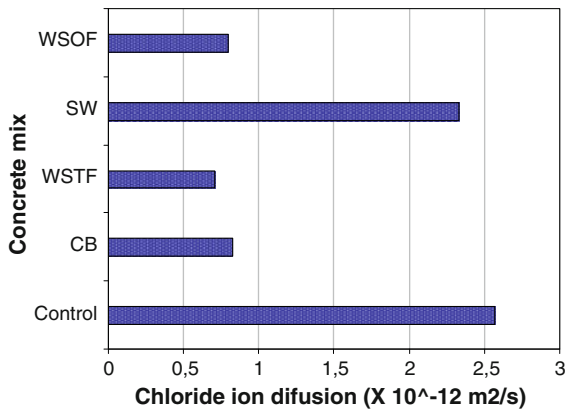
**Fig. 8** Oxygen permeability of concretes with ceramic powder (*Control* control concrete, *CB* concrete with ceramic brick powder, *WSTF* concrete with ceramic powder from stoneware twice-fired, *SW* concrete with ceramic powder from sanitary ware, *WSOF* concrete with ceramic powder from stoneware once-fired)



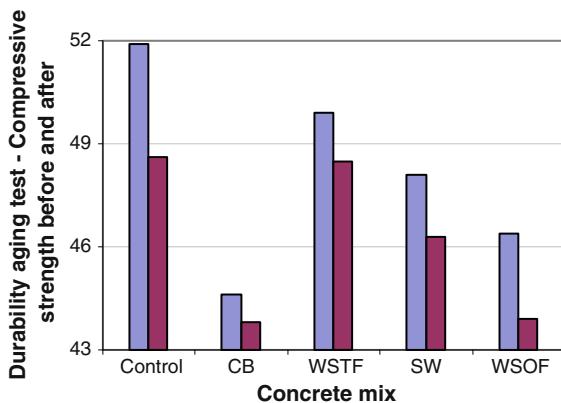
**Fig. 9** Water permeability of concretes with ceramic powder (*Control* control concrete, *CB* concrete with ceramic brick powder, *WSTF* concrete with ceramic powder from stoneware twice-fired, *SW* concrete with ceramic powder from sanitary ware, *WSOF* concrete with ceramic powder from stoneware once-fired)

### 3.3 Environmental advantages related to ceramic wastes in concrete

Ceramic wastes production in Portugal amounts to almost 400,000 ton/year [1]. Therefore, the replacement of cement by ceramic wastes has the advantage of solving several environmental problems. On one hand, it allows the reduction of carbon dioxide generated by the cement production which is a key aspect in this country. Since European Community agreed to reduce GHG by 8% (burden sharing agreement) [21], overall target differentiated emissions limitation have been

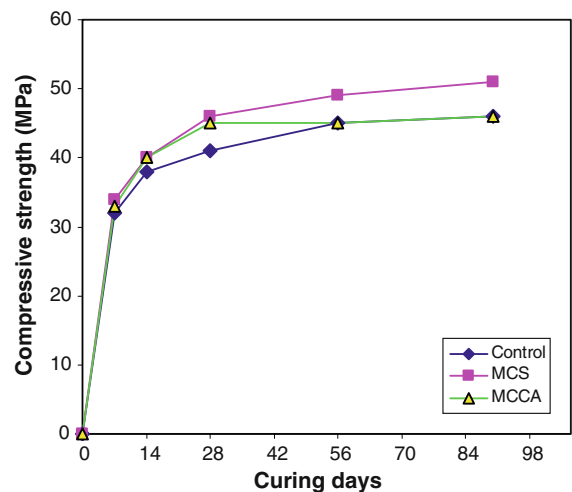


**Fig. 10** Chloride ion penetration coefficient of concretes with ceramic powder (*Control* control concrete, *CB* concrete with ceramic brick powder, *WSTF* concrete with ceramic powder from stoneware twice-fired, *SW* concrete with ceramic powder from sanitary ware, *WSOF* concrete with ceramic powder from stoneware once-fired)



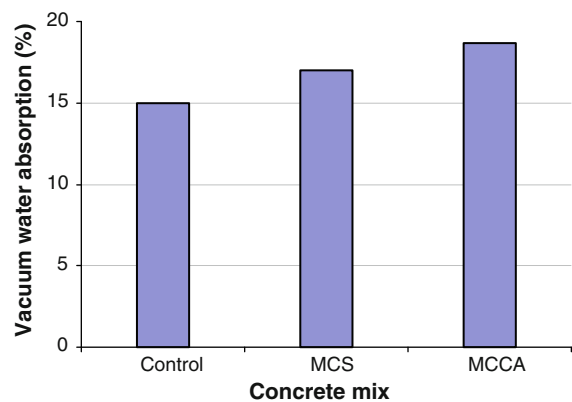
**Fig. 11** Compressive strength of concretes with ceramic powder: Before and after the durability aging test (*Control* control concrete, *CB* concrete with ceramic brick powder, *WSTF* concrete with ceramic powder from stoneware twice-fired, *SW* concrete with ceramic powder from sanitary ware, *WSOF* concrete with ceramic powder from stoneware once-fired)

agreed for each EU-15 members [22]. For Portugal Kyoto commitments implies a reduction of greenhouse gas emissions to no more of 27% above base-year level until 2012. However in 2001 emissions were 36%, above Portuguese individual greenhouse gas reduction targets and according to greenhouse gas emissions trends for EU-15 members in 2002, Portugal emissions reach 41% above 2010 target emissions. Portuguese Portland cement industry produces 10 Mt every year being responsible for almost 10% of global GHG,



**Fig. 12** Compressive strength of concretes with ceramic aggregates at different curing ages (*Control* control concrete, *MCS* concrete with ceramic sand replacing natural aggregates, *MCCA* concrete with ceramic coarse aggregate replacing coarse granite aggregates)

twice the European and world averages [4]. Portugal generates every year only about 0.25 Mt of fly ash from power stations and 0.15 Mt of blast furnace slag which represents only 4% of Portland cement production. On the other hand, Portugal has serious problems related to biodiversity. According to the World Union for Conservation Portugal is the European country with the higher number of endangered species [1]. To prevent that scenario, large areas of Portuguese land must be protected. Currently Portugal protected areas represent only 7.2% of the total area. The recent Natura

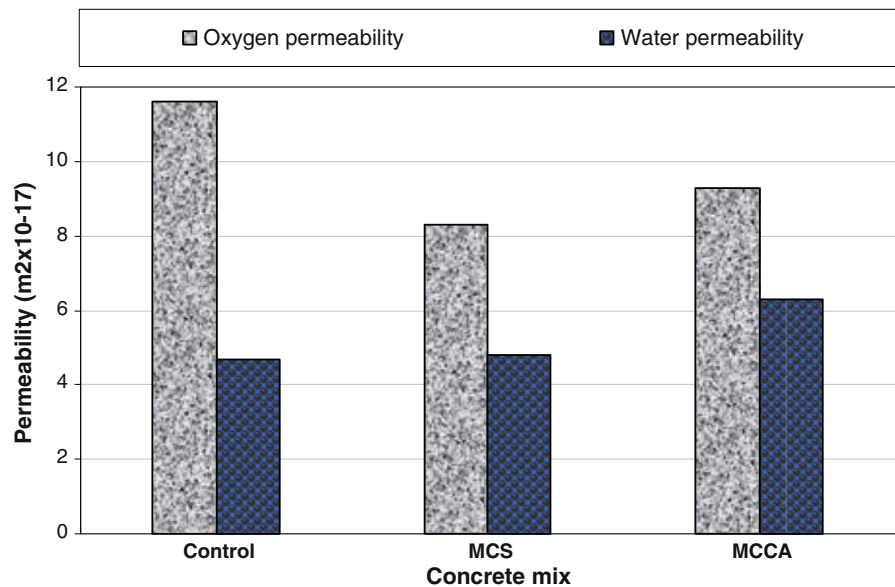


**Fig. 13** Concrete vacuum water absorption of concretes with ceramic aggregates (*Control* control concrete, *MCS* concrete with ceramic sand replacing natural aggregates, *MCCA* concrete with ceramic coarse aggregate replacing coarse granite aggregates)

**Table 8** Capillarity water absorption

	A (dm <sup>2</sup> )	W <sub>10</sub> (g)	W <sub>90</sub> (g)	(g/dm <sup>2</sup> × min <sup>0.5</sup> )	(kg/m <sup>2</sup> × h <sup>0.5</sup> )	Average
Control	0.384845					
1		1423.42	1426.42	1.233	0.01591	0.01819
2		1425.62	1428.96	1.372	0.01772	
3		1417.85	1421.80	1.623	0.02095	
MCS						
1		1338.76	1340.46	0.698	0.00902	0.00953
2		1346.58	1348.48	0.781	0.01008	
3		1367.38	1369.17	0.735	0.00949	
MCCA						
1		1335.37	1337.00	0.670	0.00865	0.00804
2		1328.10	1329.45	0.555	0.00716	
3		1330.08	1331.65	0.645	0.00833	

**Fig. 14** Concrete permeability of concretes with ceramic aggregates (*Control* control concrete, *MCS* concrete with ceramic sand replacing natural aggregates, *MCCA* concrete with ceramic coarse aggregate replacing coarse granite aggregates)

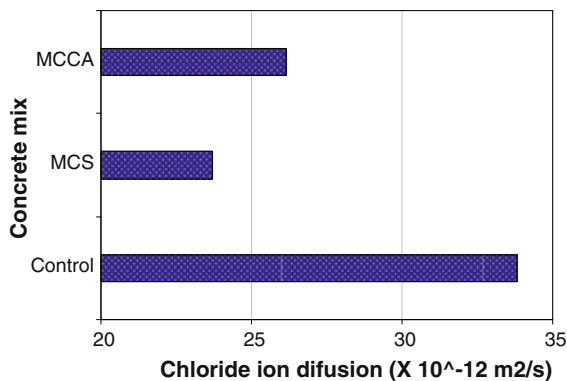


2000 Network (Directiv Birds and Habitats) proposal means a protected area to 14.1%. So that would mean a protected area of 21.3% for conservation purposes. Still the areas under the Natura 2000 Network, is no more than a balance measure between the areas needed for urban planning and for forest and agricultural production. In fact some authors believe that to prevent species extinction in an effective manner, much more protected area would be needed, about 50–70% of the total area [23]. According to Myers [24] Portugal needs to protect 77% of its land area in order to prevent species extinction. That represents a area much bigger than the

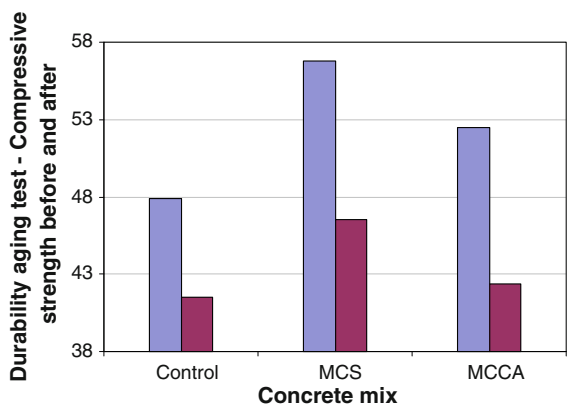
area needed for Natura 2000 Network. If we think of the areas needed for wastes disposal, we clearly understand the importance of avoiding land filled wastes by reusing ceramic wastes in concrete.

#### 4 Conclusions

Results show that concrete with ceramic waste powder has minor strength loss that is dependent on the pozzolanic reactivity of the different ceramic wastes. Results also show that despite this strength loss these



**Fig. 15** Chloride ion penetration coefficient of concretes with ceramic aggregates (*Control* control concrete, *MCS* concrete with ceramic sand replacing natural aggregates, *MCCA* concrete with ceramic coarse aggregate replacing coarse granite aggregates)



**Fig. 16** Compressive strength of concretes with ceramic aggregates: Before and after the durability aging test (*Control* control concrete, *MCS* concrete with ceramic sand replacing natural aggregates, *MCCA* concrete with ceramic coarse aggregate replacing coarse granite aggregates)

concretes possess increase durability performance. Therefore, the replacement of traditional sand by ceramic sand is a good option because does not imply strength loss and has superior durability performance. As for the replacement of traditional coarse aggregates by ceramic coarse aggregates, the results are very promising but underperformed in water absorption under vacuum test. These concretes possess a very good behaviour especially concerning capillary water absorption. Using ceramic wastes in concrete can solve several environmental problems, by one hand avoiding the extraction of large quantities of raw materials from the earth and also reducing GHG

emissions generated in the clinker production, and by another reducing the landfill areas that otherwise would be occupied by this wastes.

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