

RobustImpact

Robust impact design of steel and composite building structures

Grant Agreement Number: RFSR-CT-2012-00029

Drawings for producing the test specimens

Deliverable D.4.2

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1 INTRODUCTION

Within the RFCS project RobustImpact an experimental campaign, aimed at investigating the behaviour of framed steel and concrete composite structures under accidental loading conditions, was planned.

The residual strength of the damaged member after an impact as well as the dynamic interaction of the member with the surrounding structure will be experimentally investigated. The activation of alternate load paths will be also studied through tests on 3D slab and 2D-beam systems. Additional tests on composite joints, beam-to-column joints, column bases and T-stub tests under high speed loading and impact loading will contribute at investigating the influences of strain rate effect on the deformation capacity of the connections.

The partners involved in the experimental activities are:

- University of Stuttgart (USTUTT);
- University of Liege (ULg);
- Università degli Studi di Trento (UTRE);
- University of Aachen (RWTH).

This document collects the drawings prepared by the above mentioned partners for producing the test specimens.

The document is divided in four parts (parts A-D) each of them related to the contribution of one of the partners. In particular:

- Part A is related to the contribution from the University of Stuttgart;
- Part B refers to the contribution from the University of Liege;
- Part C collects the contribution from the Università degli Studi di Trento;
- Part D is related the contribution from the University of Aachen.

The four parts are in the following collected.



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PART A



University of Stuttgart Institute of Structural Design



Date: 17.03.2014

2 INTRODUCTION

Within the experimental tests planned to investigate at University of Stuttgart 6 test will be performed.

4 tests will be performed on composite joint tests with the objective to investigate the behaviour of the joint under high speed loading and the influence of the strain rate effect on the joints. The composite joint tests will be divided as follows:

- Two tests on positive moment joints
- Two tests on negative moment joints

Within these four tests two different speed loads will be investigated each for positive and negative moment joint respectively.

Additionally, two tests on 2D frames will be investigated at University of Stuttgart.

The detailed drawings for production of the test specimens are prepared by University of Stuttgart and sent to Arcelor Mittal.

In the following the pieces to be produced by Arcelor Mittal are reported, while the drawings are given in the Annex.

3 COMPOSITE JOINT TEST SPECIMENS

3.1 Joint tests series 1

Two test specimens for the positive composite joint tests (series 1) have to be produced. Therefore the following material is needed:

Test series 1	<u>Material</u>	<u>Dimensions</u>	Number of pieces
IPE 240	S355	2.675 m	4
HEB 220	S355	0.700 m	2
Endplate	S355	260 * 150 * 10 mm	4
Steel plate P1	S355	700 * 450 * 30 mm	4
Steel plate P2	S355	350 * 350 * 20 mm	8
Steel plate P8	S355	250 * 200 * 5 mm	16

3.2 Joint tests series 2

Two test specimens for the positive composite joint tests (series 2) have to be produced. Therefore the following material is needed:

Test series 2	<u>Material</u>	<u>Dimensions</u>	Number of pieces
IPE 240	S355	2.675 m	4
HEB 220	S355	0.700 m	2
Endplate	S355	260 * 150 * 10 mm	4
Steel plate P1	S355	700 * 450 * 30 mm	4
Steel plate P2	S355	350 * 350 * 20 mm	8
Steel plate P8	S355	250 * 200 * 5 mm	16

3.3 Test setup for joint tests

The test setup for the joint tests can be used for all the tests of both joint test series. The following material is needed:

Test setup 1	<u>Material</u>	<u>Dimensions</u>	Number of pieces
HEB 220	S355	0.754 m	2
Steel plate P3	S355	320 * 220 * 20 mm	4
Steel plate P4	S355	220 * 220 * 20 mm	4
Steel plate P5	S355	270 * 220 * 20 mm	2

Test setup 2	Material	<u>Dimensions</u>	Number of pieces
Steel plate P6	S355	350 * 300 * 20 mm	4
Steel plate P7	S355	350 * 350 * 30 mm	2

.

4 FRAME TEST SPECIMENS

Two test specimens for the 2D frame tests have to be produced.

Therefore the following material is needed:

Frame tests	Material	<u>Dimensions</u>	Number of pieces
IPE 240	S355	4.760 m	4
IPE 240	S355	1.675 m	4
HEB 220	S355	0.700 m	2
HEB 220	S355	1.300 m	4
Endplate	S355	260 * 150 * 10 mm	12
Steel plate P1	S355	700 * 450 * 30 mm	4
Steel plate P2	S355	350 * 350 * 20 mm	8
Steel plate P8	S355	250 * 200 * 5 mm	16

Test setup 1	<u>Material</u>	<u>Dimensions</u>	Number of pieces
HEB 220	S355	0.754 m	2
Steel plate P3	S355	320 * 220 * 20 mm	4
Steel plate P4	S355	220 * 220 * 20 mm	4
Steel plate P5	S355	270 * 220 * 20 mm	2

Test setup 2	Material	<u>Dimensions</u>	Number of pieces
Steel plate P6	S355	350 * 300 * 20 mm	8
Steel plate P7	S355	350 * 350 * 30 mm	4

5 ANNEX: DRAWINGS FOR PRODUCING THE TEST SPECIMENS





































Column C4
























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Drawings for producing the test specimens Contribution from the University of Liege

Deliverable D.4.2

Authors:

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PART B



Date: 07.01.2014

6 INTRODUCTION

In the test campaign planned at the University of Liege, 44 impact tests will be conducted:

- 22 on beam-to-column joints

- 22 on column bases

These assemblies will be impacted by a mass dropped from a certain height.

The aim of these tests is to observe the ability of the joints to absorb energy by plastic deformation.

The 22 tests on beam-to-column joints are divided as follows:

- 5 for which the failure will develop in the end-plate in bending (1 static test and 4 impact tests with 4 different level of energy) ("EPB")

- 5 for which the failure will develop in the column flange in bending (1 static test and 4 impact tests with 4 different level of energy) ("CFB")

- 4 for which the failure will develop in the beam flange in compression (1 static test and 3 impact tests with 3 different level of energy) ("BFC")

- 4 for which the failure will develop in the column web compression (1 static test and 3 impact tests with 3 different level of energy) ("CWC")

- 4 for which the failure will develop as in a real joint, activating 2 different components (1 static test and 3 impact tests with 3 different level of energy) ("Real joint")

The 22 tests on column bases are divided as follows:

- 6 tests for which the failure happens in the concrete, in a ductile mode ("duct_conc_1")

- 6 tests for which the failure happens in the concrete, in a ductile mode ("duct_conc_2")

- 5 tests for which the failure happens in the concrete, in a fragile mode ("frag_conc_1")

- 5 tests for which the failure happens in the steel end-plate ("duct_steel")

The detailed drawings for the production of the specimen have been produced by ULg and sent to ArcelorMittal.

In the following sections, a list of the pieces to be produced is given. All the drawings are reported in the annexes.

7 BEAM-TO-COLUMN JOINTS

7.0 Supports

Drawing A.0.1

Description	Number of pieces
Steel plates 300x300x30	4

7.1 EPB (5 specimens)

Drawings A.1.1 --> A.1.3

Description	Number of pieces
Steel beam profile	10
IPE180, length: 730 – Steel S355	10
End-plate: 210x120x8 – Steel S355	10
Stiffeners : 164x34x20 – Steel S355	2*2*5=20
Plates: 110x200x30 – Steel S355	2*5=10
Steel column profile	5
HEB140, length: 460 – Steel S355	5
Repartition plate: 130x140x50 – Steel S355	5
Joint	
Bolts M16, 10.9	4*2*5 = 40

7.2 CFB (5 specimens)

Description	Number of pieces
Steel beam profile	10
IPE180, length: 730 – Steel S355	10
End-plate: 220x120x18 – Steel S355	10
Reinforcement plate (beam flange): 150x79x6 – Steel S355	10
Stiffeners : 164x34x20 – Steel S355	2*2*5=20
Plates: 110x200x30 – Steel S355	2*5=10
Steel column profile	5
HEB140, length: 460 – Steel S355	5
Repartition plate: 130x140x50 – Steel S355	5
Joint	
Bolts M16, 10.9	4*2*5 = 40

7.3 BFC (4 specimens)

Drawings A.3.1 --> A.3.3

Description	Number of pieces
Steel beam profile	8
IPE180, length: 730 – Steel S355	8
Stiffeners : 164x34x20 – Steel S355	2*2*4=16
Plates: 110x200x30 – Steel S355	2*4=8
Steel column profile	4
HEB140, length: 460 – Steel S355	4
Repartition plate: 130x140x50 – Steel S355	4
Stiffeners: 116x54.5x12 – Steel S355	2*2*4=16

7.4 CWC (4 specimens)

Drawings A.4.1 --> A.4.3

Description	Number of pieces
Steel beam profile	8
IPE180, length: 710 – Steel S355	8
Stiffeners : 164x34x20 – Steel S355	2*2*4 = 16
Plates: 110x200x30 – Steel S355	2*4=8
Steel column profile	4
IPE180, length: 540 – Steel S355	4
Repartition plate: 170x91x50– Steel S355	4
Stiffeners: 164x34x10 – Steel S355	2*4=8

7.5 Real joint (4 specimens)

Drawings A.5.1 --> A.5.3

Description	Number of pieces
Steel beam profile	8
IPE180, length: 730 – Steel S355	8
End-plate: 210x120x15 – Steel S355	8
Stiffeners : 164x34x20 – Steel S355	2*2*4 = 16
Plates: 110x200x30 – Steel S355	2*4=8
Steel column profile	4
HEB140, length: 760 – Steel S355	4
Repartition plate: 130x140x50 – Steel S355	4
Joint	
Bolts M16, 10.9	2*4*4 = 32

8 COLUMN BASES

8.0 Mass

One masse needed, drawing B.0.1, used for all the tests (minimum weight: 75 kg)

8.1 duct_conc_1 – duct_conc_2 – frag_conc_1

The difference between these 3 configurations is in the scheme of reinforcement, not given here.

Description	Number of pieces
Steel profile	6+6+5 = 17
HEB140, length: 1000 – Steel S355	17
Stiffener: 116x54.5x30 – Steel S355	17*2 = 34
<u>Joint</u>	
End-plate: 300x200x30 – Steel S355	17
Bolts M24, 10.9	4*17 = 68
Embedded plate: 300x200x30 – Steel S355 with studs (drawing B.1.4) and treaded bars welded on it (drawing B.1.3)	17

8.2 duct_steel

Description	Number of pieces
Steel profile	5
HEB140, length: 1000 – Steel S355	5
Stiffener: 116x54.5x30– Steel S355	5*2 = 10
Joint	
End-plate: 260x150x10 – Steel S355	5
Bolts M12, 10.9	4*5 = 20
Embedded plate: 300x200x30 – Steel S355 with studs (drawing B.1.4) and treaded bars welded on it (drawing B.2.3)	5

9 ANNEX A: BEAM-TO-COLUMN JOINTS

9.0 Supports

Only piece A has to be provided (x4)



A.0.1

9.1 EPB (x5)







A.1.2: Column 1 (x5)





9.2 CFB (x5)



A.2.1: General configuration (x5)





A.2.3: Beam 2 (x10)

9.3 BFC (x4)



A.3.1: General configuration (x4)



A.3.2: Column 2 (x4)



A.3.3: Beam 3 (x8)

9.4 CWC (x4)







A.4.2: Column 3 (x4)



A.4.3: Beam 4 (x8)

9.5 Real joint (x4)



A.5.1: General configuration (x4)

A.5.2: Column 1 (confer drawing A.1.2) (x4)



A.5.3: Beam 5 (x8)

10 ANNEX B: COLUMN BASES

10.0 Mass



B.0.1

(around 75 kg)

10.1 duct_conc_1 - duct_conc_2 - frag_conc_1



B.1.1 General view of the specimen



B.1.2 Beam 1



B.1.3 Embedded plate



B.1.4 Studs

Studs KÖCO SD, DIN EN ISO 13918 (n° of the article: 017-0044-001)

 $d_1 = 22 \text{ mm}$ $d_5 = 35 \text{ mm}$ $d_3 = 29 \text{ mm}$ $h_3 = 10 \text{ mm}$ $h_4 = 6 \text{ mm}$ $l_2 = 100 \text{ mm}$

10.2 duct_steel



B.2.1 General view of the specimen







B.2.3 Embedded plate



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PART C



UNIVERSITÀ DEGLI STUDI DI TRENTO

Date: 18.03.2014

11 INTRODUCTION

The tests' campaign planned by the Università degli Studi di Trento (UTRE) comprises of:

- 2 full-scale tests on specimens representative of steel and concrete composite structures. At this aim two steel and concrete composite structures, characterised by the same overall dimensions, have been designed. One of the two structures is characterised by a symmetric configuration of the columns, i.e., the position of the columns defines a square grid (symmetric structure). An asymmetric configurations of the columns in one direction characterises the second structure (asymmetric configuration).The same steel profiles, connections and slab thickness have been considered for both the structures. The main difference between the two configurations lies in the rebars (diameter and layout) of the slab.

From the reference structures two representative substructure characterised by the same overall dimensions but by a symmetric and an asymmetric configuration, respectively, have been identified. The substructures will be built in laboratory (Figs.1.1 and 1.2) and tested reproducing the collapse of a central column. The aim of the tests is to investigate the influence of the biaxial membrane effect associated with the concrete slab in a framed structure when the collapse of a column happens.

- 20 tensile tests on T-stubs related to the beam-to-column joint adopted in the reference structures and in the full-scale specimens. T-stubs associated to both the column (10 specimens) and the end-plate (10 specimens) will be tested. The tests will be performed at two different speed loading aiming at investigating the influence of strain rate effects on the joint deformation capacity. As a reference also quasi-static tests will be carried out.



Figures 1.1: Symmetric full-scale specimen.



Figures 1.2: Asymmetric full-scale specimen.

The drawings for the production of the specimens have been produced by UTRE and sent to ArcelorMittal, to local fabricators and to local companies, depending on the material to be provided.

In the following sections, a list of the pieces to be produced is given. All the drawings are reported in the annexes A-D.

12 FULL-SCALE TESTS

12.1 Steel components

Table 2.1 summarises the steel components needed for the construction of steel skeleton of the full-scale specimens.

Table 2.1: Summary of the steel components for the steel skeleton of the full-scale specimens.

DESCRIPTION	DRAWING NAME	NUMBER OF PIECES	SPEC. CON Symmetric	FIGURATION Asymmetric
Summary of the steel components	FST 01	-	X	X
	COLUMNS TYP	E 1		
Steel profile HEB220 length 5500mm	FST 02-09	18	Х	Х
Steel plate type B 400x400x30mm	FST 03-09	18	Х	Х
Steel plate type C 220x220x20mm	FST 03-09	18	Х	Х
Column assembly	FST 09	-	Х	Х
	BEAMS			
Beam type 1 Steel profile IPE240 length 5460mm	FST 04	12	х	х
Beam type 2 Steel profile IPE240 length 5670mm	FST 05	6	х	
Beam type 3 Steel profile IPE240 length 7095mm	FST 06	3		х
Beam type 4 Steel profile IPE240 length 4246mm	FST 07	3		х
Beam type 5 Steel profile IPE240 length 1500mm	FST 08	6	х	х
Steel plate type A	FST 03-04-05-06- 07-08	54	Х	Х

Figures 2.1 and 2.2 provide a plan view of the two full-scale specimens (steel skeleton) which allows identifying the position of the various steel components.

The drawings for the production of the profiles needed for the steel skeleton of both the specimens (Annex A) have been produced by UTRE and sent to ArcelorMittal.

	Be	am type 5		00	Bear	m type 5	0 0	Be	am	type 5	0 0 0 0
امرما	Г	В	eam ty	vpe 2		Beam type 2				Column	ا م م
		Column				Column				type 1	<u> </u>
000	oe 1				oe 1				be 1		
	am typ				am typ				am typ		00
	Be				Be				Be		0 0 0
0,0		B	eam ty	/pe 2		Bea	m type	e 2			
0 0										Column	0,01
		Column				Column				type 1	
	e 1	Column type 1			e 1-	Column type 1			e 1	type 1	
	m type 1	Column type 1			m type 1	Column type 1			im type 1	type 1	
	Beam type 1	Column type 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			Beam type 1	Column type 1			Beam type 1	Column type 1	
	Beam type 1	Column type 1	C C C C C C C C C C C C C C C C C C C	(pe 2	Beam type 1	Column type 1 1 00 1 00 Bea	i i i i i i i i i i i i i i i i i i i	© © © © © © © © © © © © © © © © © © ©	Beam type 1	Column type 1	
	C Beam type 1	Column type 1	eam ty	(pe 2	Beam type 1	Column type 1	i i i i i i i i i i i i i i i i i i i	© © © © © © © © © © © © © © © © © © ©	n Beam type 1	Column type 1	

Figures 2.1: Steel profiles for the symmetric full-scale specimen.



Figures 2.2: Steel profiles for the asymmetric full-scale specimen.

12.2 Restraints

In order to reproduce the effect provided by the remaining part of the structure and the base connections, the specimens will be restrained to both the floor slab and the vertical walls of the laboratory. Ai this aim suitable restraints were identified trough FE numerical investigations and accordingly designed. Furthermore, the columns of the specimens, at their upper ends, will be connected together in order to reproduce the effect provided by the levels of the structure above the one considered in the tests.

Figures 2.3-2.5 allow identifying the position of the restraints.

The main drawings related to the restraints are summarised in Table 2.2 and collected in Annex B. The drawings for the production of the restraints (Annex B) have been produced by UTRE and sent to local fabricators.



Figures 2.3: Restraints configuration for the full-scale specimens.

Table 2.2: Summary of the restraints									
DESCRIPTION	DRAWING NAME	NUMBER OF PIECES	SPEC CONFIG Symmetric	CIMEN'S SURATION Asymmetric					
RESTRAINTS AT THE COLUMN'S BASE									
Column Base Restraint type 1	CBR1	8	Х	Х					
Column Base Restraint type 2	CBR2	1	Х	Х					
LATERAL RESTRAINTS									
Lateral Restraint type 1	LR1	1	Х	Х					
Lateral Restraint type 2	LR2 2		Х	Х					
Lateral Restraint type 3	LR3	1	Х	Х					
TOP COLUMN RESTRAINTS									
Top connection type 1	TC1	1	Х	Х					
Top connection type 2	TC2	4	Х	Х					
Top connection type 3	TC3	4	Х	Х					

Table 2.2: Summary of the restraint





Figures 2.5: Restraints for the asymmetric full-scale specimen.

12.3

Rebars

The reinforcement of the slabs of the two full-scale specimens is realized with an electrowelded wire mesh located on both the upper and the lower sides of the slab and additional rebars added where required by the design calculations.

A summary of the reinforcement needed for the slabs of the two full-scale specimens is reported in Tables 2.3-2.7. In detail:

- Tables 2.3 summarises the electrowelded wire mesh required for both the specimens;
- Tables 2.4 and 2.5 list the additional rebars for the symmetric specimen, for the lower and the upper side, respectively;
- Tables 2.6 and 2.7 collect the additional rebars for the asymmetric specimen, related to the lower and the upper side, respectively.

A detailed description of the layout of the rebars for both the specimens is presented in Annex C.

Tables 2.3-2.7 and the drawings of Annex C have been sent to a local Company for their production.

Т	able 2.4: Summary of the reb	ars for the sy	mmetric co	onfiguration	- Lower side
S	SLAB REBARS - LOWER S	IDE - SYM	METRIC C	ONFIGUR	ATION
Position	Shape	Diameter mm	Length mm	Quantity n°	Nominal weight _{kgf}
Pos. C	2400mm	10	2400	22	33
Pos. D	2400mm **	10	2400	8	12
Pos. E	2400mmX	10	2400	30	44

10

¥

Total nominal weight (kgf)

2400mm

Pos. F

Table 2.5: Summary of the rebars for the symmetric configuration - Upper side

2400

8

12

153

	SLAB REBARS - UPPER S	SIDE - SYMN	METRIC C	ONFIGUR	ATION
Position	Shape	Diameter	Length	Quantity	Nominal
		mm	mm	n°	kgf
Pos. A	튵柔	10	3000	28	52
Pos. G1	E S S S S S S S S S S S S S S S S S S S	16	3500	3	17
Pos. G2	₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩ ₩	16	3200	3	15
Pos. H	 	16	3000	24	114
Pos. I	⊀⊀⊀	16	3000	48	227
Pos. L	2400mm //////////////////////////////////	10	2400	92	136
Pos. M		16	3000	12	57
Pos. N	 ★	16	3000	44	206
Pos. O	 ∠400mm⊀	10	2400	35	52
Total non	ninal weight (kgf)				878

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Ta	able 2.6: Summary of the reba	ars for the as	ymmetric c	onfiguration	- Lower side
SI	_AB REBARS - LOWER SI	DE - ASYM	METRIC	CONFIGU	RATION
Position	Shape	Diameter mm	Length mm	Quantity n°	Nominal weight kgf
Pos. C	 ⊀⊀	10	4700	41	119
Pos. D	2400mm **	10	2400	8	12
Pos. E	2400mm **	10	2400	21	31
Pos. F	 	10	2400	8	12
Pos. G	 ∦¥	10	8500	8	42
Pos. H	6000mm	10	6000	76	281
Total nomin	al weight (kgf)				527

Table 2.7: Summary of the rebars for the asymmetric configuration - Upper side	
--	--

S	SLAB REBARS - UPPER S	SIDE - ASYM	METRIC (CONFIGUE	RATION
Position	Shape	Diameter mm	Length mm	Quantity n°	Nominal _{kgf}
Pos. A	₩ 4 5 5 1450mm 1450mm 1450mm 1450mm	10	3000	28	52
Pos. I	 ⊀⊀	16	3000	40	189
Pos. L	 	10	2400	98	145
Pos. M	 ⊀ 4000mm ⊀	16	4000	12	76
Pos. N	 ⊀	16	4000	45	284
Pos. O	 ★	10	3000	35	65
Pos. P1	E S S S S S S S S S S S S S S S S S S S	16	3500	3	17
Pos. G2	E + S + 1502m + ★ 1502m + ★	16	3200	3	15
Pos. Q		16	3000	24	114
Total nor	ninal weight (kgf)				957

13 T-STUB TESTS

The tensile tests on T-stubs will be related to the beam-to-column joints adopted in the reference structures and in the full-scale specimens. A total of twenty tests will be performed: ten of them will investigate the behaviour of the column and the remaining, the end-plate response (Tab. 2.8).

A detailed description of the specimens is presented in Annex D. The drawings of Annex D have been sent to a ArcelorMittal for the production.

DESCRIPTION	DRAWING NAME	NUMBER OF PIECES
Summary of the material for T-stub tests	TST 01	-
COLUM	N'S T-STUB	
Column T-stubs (column HEB220)	TST03A-TST03B	10
END-PLA	ATE T-STUB	
End-Plate T-stubs	TST02	10

Table 2.8: Summary of the material for the T-stub tests.

14 ANNEX A: FULL-SCALE TESTS - STEEL COMPONENTS

ΤΓ			IPE240					
			5460					
			0.00		-			
Beam Type 2	Number	of pieces - 6						
ТГ			IPE240					
			5670					
			5070		-			
Beam Type 3	Number	of pieces - 3						
T	Humber	or preces - o						
				IPE240				
-				7095				
Beam Type 4	4 Number	of pieces - 3						
		or proces of the						
I		IPE240	D					
-		4	246	-				
Beam Type 5	Number	of pieces - 6						
IC	IPE240							
	1500							
Column type 1 r	Number of p	ieces - 18						
IL			HEB 220					
-			5500					
Vlate type A Numb	per of pieces	- 54	Dista					
	ion on process	- 34	Plate	type B Number of pie	ces-18	Plate ty	pe C Number of pie	ces - 18
0	F		Plate	type B Number of pier	ces - 18	Plate ty	pe C Numberofpie	ces-18
260			Plate	type B Number of pier	ces - 18	Plate ty	pe C Number of pie	ces - 18
260			Plate	type B Number of pier	ces - 18	Plate ty	pe C Number of pie	ces - 18
260	150		10	B Number of pier	ces - 18	Plate ty	pe C Number of pier	20
ar Connector SD 3	150		10	type B Number of pier	ces - 18	Plate ty	pe C Number of pie	20
ar Connector SD 3.	150 /4" × 4"; Milo		Plate	type B Number of pier	ces - 18	Plate ty	pe C Number of pier	20
ar Connector SD 3.	150 //4" × 4"; Milc	d Steel; 1182 He	10	type B Number of pier	ces - 18	Plate ty	pe C Number of pie-	ces - 18
ar Connector SD 3.	150 14" x 4"; Mile D=19 ;H=3	d Steel; 1182 He	rate	type B Number of pier	ces - 18	Plate ty	pe C Number of pie-	ces - 18
ar Connector SD 3.	150 //4" x 4"; Mila D=19 ;H=3	d Steel; 1182 He	Plate	type B Number of pier	ces - 18	Plate ty	pe C Number of pie-	ces - 18
ear Connector SD 3.	150 //4" x 4"; Milc D=19 ;H=3	d Steel; 1182 He 31.75 ;A=9.52 ;L-	eaded studs =100 Steel Grade	Dimensions [mm]	ces - 18	Plate ty	pe C Number of pier	Drawing No.
ear Connector SD 3	D=19 ;H=3	d Steel; 1182 He 31.75 ;A=9.52 ;L=	eaded studs =100 Steel Grade S 355 JR S 355 JR	Dimensions [mm]	Weight [t] 0.168	Number of pieces	pe C Number of pie-	Drawing No. FST 04 FST 04
ar Connector SD 3	150 150 150 D=19 ;H=3 1 2 3	d Steel; 1182 He 31.75 ;A=9.52 ;L= IPE 240 IPE 240 IPE 240	aded studs =100 Steel Grade S 355 JR S 355 JR	bimensions [mm] 240x120x5660 240x120x5670 240x120x7095	Weight [t] 0.168 0.174 0.218	Number of pieces 12 6 3	pe C Number of pie- 220 70tal Weight [t] 2.011 1.044 0.653	Drawing No. FST 04 FST 06
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=3 1 2 3 4	d Steel; 1182 He 31.75 ;A=9.52 ;L= IPE 240 IPE 240 IPE 240 IPE 240	10	Dimensions [mm] 240x120x5450 240x120x5970 240x120x7095 240x120x4246	Weight [t] 0.168 0.174 0.218 0.130	Number of pieces 12 6 3 3 3	Total Weight [t] 2.011 1.044 0.653 0.391	Drawing No. FST 04 FST 06 FST 07
ar Connector SD 3	150 144* x 4"; Mile D=19 ;H=: 11 2 3 4 5	IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240	10	Dimensions [mm] 240x120x5460 240x120x5670 240x120x5670 240x120x295 240x120x295 240x120x295	Weight [t] 0.168 0.174 0.218 0.046	Number of pieces 12 6 3 3 6	Total Weight [t] 2.011 1.044 0.653 0.391 0.276	Drawing No. FST 04 FST 06 FST 07 FST 08
Name Beam type Beam type Beam type Beam type Beam type Beam type Beam type Beam type Beam type	150 144" x 4"; Milc D=19 ;H=; 1 2 3 4 5 6	d Steel; 1182 He 31.75 ;A=9.52 ;L= IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240	10	bimensions [mm] 2400 240x120x5460 240x120x5460 240x120x5460 240x120x24246 240x120x500 220x220x5500	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393	Number of pieces 12 6 3 6 6 18	Total Weight [t] 220 220 220 220 201 2.011 1.044 0.653 0.391 0.276 7.079	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 09 FST 02-09
ar Connector SD 3	D=19 ;H=: 150 D=19 ;H=: 1 2 3 4 5 6 1 A	IPE 240 IPE 240	10	type B Number of pier 400 400 240x120x5460 240x120x5670 240x120x466 240x120x4246 240x120x500 220x220x5500 260x150x10 260x150x10	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003	Number of pieces 12 6 3 6 18 54	pe C Number of pie- 220 220 Total Weight [t] 2.011 1.044 0.653 0.331 0.276 7.079 0.165	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 02-09 FST 02-09 FST 02-09-05-06-07-08
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=: 11 2 3 4 5 e 1 A B	IPE 240 IPE 240	10	Dimensions [mm] 240x120x5640 240x120x5640 240x120x5670 240x120x5670 240x120x5670 240x120x500 220x220x5500 220x220x5501 400x400x30	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.038	Number of pieces 12 6 3 6 18	Total Weight [t] 220 220 220 220 220 220 220 201 201	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 07 FST 08 FST 09 FST 03-09 FST 03-09
Name Beam type Beam type	150 144 × 4"; Mile D=19 ;H=: 11 2 3 4 5 6 1 A B C	IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240	10	Dimensions [mm] 2400 400 240x120x5460 240x120x5670 240x120x5670 240x120x5670 240x120x5670 240x120x5670 240x120x5670 240x120x5500 260x160x100 220x220x5500 260x160x10 400x400x30 220x220x20	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.003 0.003	Number of pieces 12 6 3 6 18 18 18	Total Weight [t] 220 220 220 220 220 220 220 201 201	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 07 FST 08 FST 07 FST 08 FST 07 FST 08 FST 07 FST 08 FST 03-09 FST 03-09
Arr Connector SD 3	150 144" x 4"; Mile D=19 ;H=: 11 2 3 4 5 e 1 A B C C S	IPE 240 IPE 24	10	Dimensions [mm] 2400 2401/20x5460 240x120x5670 240x120x5670 240x120x5670 240x120x500 220x220x5500 250x150x10 400x400x30 220x220x220 D=19; H=31.75; A=9.52; L=100	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.003 0.038 0.007	Number of pieces 12 6 13 6 18 54 18 18 18 18 18 18 18	Total Weight [t] 220 220 220 220 220 220 220 22	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 07 FST 08 FST 09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 04-05-06-07-08
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=3 11 2 3 4 5 6 1 A B C C C S	IPE 240 IPE 24	10	Dimensions [mm] 240x120x5460 240x120x5670 240x120x7095 240x120x7095 240x120x5600 220x220x5500 260x150x10 400x400x30 220x220x2500 20x150x10 400x400x30 220x220x20 D=15; H=31.75; A=9.52; L=100	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.003 0.003 0.003	Number of pieces 12 6 33 6 18 54 18 18 18 71 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18	Total Weight [1] 220 220 220 220 220 220 220 22	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 07 FST 08 FST 09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09
ar Connector SD 3	150 150 150 150 150 150 150 150	J Steel; 1182 He 31.75 ;A=9.52 ;L= IPE 240 IPE	10 eaded studs =100 Steel Grade S 355 JR S 355 JR	type B Number of pier 400 400 400 400 400 400 400 40	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.003 0.038 0.007	Number of pieces 12 6 3 6 18 54 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18 18 19	Total Weight [t] 220 220 220 220 220 220 220 22	20 20 20 20 20 20 20 20 20 20 20 20 20 2
ar Connector SD 3	150 150 150 150 150 150 150 150	J Steel; 1182 He 31.75 ;A=9.52 ;L= IPE 240 IPE	10	type B Number of pier 400 400 400 400 400 400 400 40	Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.003 0.038 0.007 	Number of pieces 12 6 3 6 18 54 18 18 18 17 Total	Total Weight [t] 220 220 220 220 220 220 220 22	20 20 20 20 20 20 20 20 20 20
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=3 11 2 3 4 5 6 1 A B C C ctors S	IPE 240 IPE 24	10	bimensions [mm] 240x120x5460 240x120x5460 240x120x5670 240x120x5670 240x120x500 220x220x5500 220x220x5500 220x220x5500 220x220x5500 220x220x5500 20x120x150x10 400x400x30 220x220x20x20x20x20x20 D=15; H=31.75; A=9.52; L=100	Weight [t] 3 Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.038 0.007 UNIVERS DITRENT DITRENT DITRENT DITRENT	Number of pieces 12 6 33 6 18 18 118 17 Total TTÀ DEGLI STUDI YO Materia Consta e Constructiones e Constructions	Total Weight [t] 220 220 220 220 220 220 220 22	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 02-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 04-05-06-07-08
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=: 11 2 3 4 5 e 1 A B C C ctors \$	IPE 240 IPE 24	10	type B Number of pier 400 400 240x120x5460 240x120x5670 240x120x5670 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x420x5500 260x150x10 250x220x220 D=19; H=31.76; A=9.52; L=100	Weight [t] 3 Weight [t] 168 0.174 0.218 0.130 0.046 0.393 0.003 0.038 0.007	Plate ty Pla	Total Weight [1] _	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 07 FST 09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 04-05-06-07-08
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=: 11 2 3 4 5 e 1 A B C ctors \$	IPE 240 IPE 24	10	type B Number of pier 400 400 240x120x5460 240x120x5670 240x120x5670 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x4246 240x120x420x5500 260x150x10 250x220x220 D=19; H=31.76; A=9.52; L=100	Weight [t] 3 Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.038 0.007 UNIVERS DITRENT Laboratorio Pr	Plate ty Pla	Total Weight [t] _	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 08 FST 07 FST 09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 04-05-06-07-08 FST 04-05-06-07-08
Plate type Plate type Shear Cooned	150 144" x 4"; Mile D=19 ;H=: 11 2 3 4 5 e 1 A B C C ctors 5	IPE 240 IPE 24	10	Umber of pier 400 400 240x120x5460 240x120x5670 240x120x5670 240x120x5670 240x120x5670 240x120x5500 260x150x100 220x220x5500 260x150x10 400x400x30 220x220x200 D=19; H=31.75; A=9.52; L=100	Weight [t] 3 Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.038 0.007 UNIVERS DI TRENT DI TRENT Durd tarro P Laboratorio P	Plate ty Pla	Total Weight [t] 200 220 200 201 201 201 201 201 201 201	Drawing No. FST 04 FST 04 FST 05 FST 06 FST 06 FST 07 FST 08 FST 009 FST 03-09 FST 03-09 FST 04-05-06-07-08 FST 04-05-06-07-08 COLUMN
ar Connector SD 3	150 144" x 4"; Mile D=19 ;H=: 11 2 3 4 5 e 1 A B C C C C S	IPE 240 IPE 24	10	Dimensions [mm] 2400 400 400 240x120x5460 240x120x5670 240x120x5670 240x120x500 260x150x100 220x220x5500 260x150x10 200x220x200 D=19; H=31.75; A=9.52; L=100	Weight [t] 3 Weight [t] 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.038 0.007 UNIVERS DI TRENT Laboratorio P Laboratorio P Note:	Plate ty Pla	Total Weight [t] 220 220 220 220 220 2011 1.044 0.653 0.391 0.276 7.079 0.165 0.678 0.137 12.434 t N° RFSR-CT-2012 ronym: ROBUSTIM tion a cronym: UNIT	Drawing No. FST 04 FST 05 FST 06 FST 06 FST 07 FST 08 FST 07 FST 08 FST 09 FST 03-09 FST 03-09 FST 04-05-06-07-08 FST 04-05-06-07-08 COLUE BCALE BCALE BCALE BCALE BCALE BCALE BCALE BCALE BCALE BCALE
Aar Connector SD 3	150 150 14" x 4"; Mile D=19 ;H=: :1 :1 :2 :3 :4 5 e 1 A B C ctors :1	IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 SD 3/4*x 4*	10	bimensions [mm] 2400 400 400 400 240x120x5450 240x120x5670 240x120x7095 240x120x1500 220x220x20 D=13; H=31.75; A=9.52; L=100 Full Full Full	Weight [t] 3 Weight [t] 0.168 0.168 0.174 0.218 0.046 0.393 0.003 0.038 0.007 UNIVERS DITRENT Lidoratorio Pr Lidoratorio Pr Note: Note: Nite:	Plate ty Pla	Total Weight [t] 220 220 220 220 201 2011 1.044 0.653 0.391 0.276 7.079 0.165 0.678 0.137 12.434 tN° RFSR-CT-2012 tN° RFSR 01	Drawing No. FST 04 FST 04 FST 05 FST 05 FST 07 FST 08 FST 07 FST 08 FST 07 FST 08 FST 07 FST 08 FST 09 FST 03-09 FST 03-09 FST 03-09 FST 03-09 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08
ear Connector SD 3	150 144 x 4"; Mile D=19 ;H=3 11 2 3 4 5 e 1 A B C ctors \$	d Steel; 1182 He 31.75 ;A=9.52 ;L= IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 IPE 240 SD 3/4*x 4*	10	bimensions [mm] 2400 240x120x5460 240x120x5460 240x120x5460 240x120x5460 240x120x5460 240x120x5460 240x120x5460 240x120x5500 220x220x5500 260x150x10 20x220x20 D=19; H=31.75; A=9.52; L=100	Weight [t] 3 Weight [t] 0.168 0.168 0.174 0.218 0.130 0.046 0.393 0.003 0.038 0.007 UNIVERS DITRENT DITRENT Laboratorio Pr Laboratorio Pr Ile Note: He All dim illustrated d d	Plate ty Pla	Pe C Number of pie 220 220 220 220 220 220 220 22	Drawing No. FST 04 FST 05 FST 06 FST 07 FST 03-09 FST 03-09 FST 04-05-06-07-08 FST 03-09 FST 03-09 FST 04-05-06-07-08 Color FST 03-09 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08 FST 04-05-06-07-08


















15 ANNEX B: RESTRAINTS





















16 ANNEX C: SLAB'S REINFORCEMENT













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17 ANNEX D: T-STUB TESTS











RobustImpact

Robust impact design of steel and composite building structures

Grant Agreement Number: RFSR-CT-2012-00029

Drawings for producing the test specimens Contribution from the University of Aachen

Deliverable D.4.2

Authors:

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Date: 25.03.2014

18 INTRODUCTION

At the Institute for Steel Structures crash tests on columns are carried out to investigate the residual strength of the damaged member after the impact. The aim is to determine the dynamic response of the member during the impact as well as the dynamic interaction of the member with the surrounding structure. Furthermore, special attention on the strength and deformation demand of the joints is paid.

The test program consists of six tests in total with three different test configurations. Series 1 consist of a simple supported element as reference for pure member behaviour. In series 2, the dynamic interaction between the impacted member and an attached mass at the column head is investigated. It is expected that during the impact, the inertia of the mass provides some axial restrain, but after the impact, the downwards accelerated mass pushes on the column. In both cases, real hinges will be used. Series 3 represents a column within a real structure considering a mass at column head, the stiffness of the surrounding structure as well as realistic semi-strength beam-to-column and column foot connections. In each series, one specimen will be tested subjected to a kinetic energy, which causes moderate and strong plastic deformations respectively. Table 18-1 gives a summary of the test program and the varied parameters.

No.	Type of connection	Mass at column head [to]	Initial velocity [km/h]
1-1	real hinge	-	29
1-2	real hinge	-	36
2-1	real hinge	7 to	29
2-2	real hinge	7 to	36
3-1	semi-strength	7 to	29
3-2	semi-strength	7 to	36

Table 18-1: Overview of the test program at RWTH Aachen

The detailed drawings needed for the production of the specimens and the support structure have been prepared at the RWTH Aachen and are presented in the following.

19 SETUP FOR TEST SERIES 1 & 2



Figure 19-1: Drawings of the test setup for test series 1 & 2

20 SETUP FOR TEST SERIES 3



<u>Schnitt 1-1</u>

Schnitt 2-2



<u>Detail "C"</u> <u>Draufsicht</u>

3 H24x80 - 10. Fe= 100%

Ő

HEM 180

2 Bolzen 460 x100 Werkstoff: 42GrMo4



Ansicht Versuchskörper





<u>Detail "B"</u>

<u>Draufsicht</u>



HEB 140









<u>Detail "D"</u>

<u>Detail "E"</u>



Figure 20-1: Drawings of the test setup for test series 3

21 ASSEMBLY GROUPS

21.1 H10



Figure 21-1: Assembly group H10



Figure 21-2: Assembly group H20

21.3 H30



Figure 21-3: Assembly group H30

21.4 H40



Figure 21-4: Assembly group H40

21.5 H50



Figure 21-5: Assembly group H50

21.6 H60



Figure 21-6: Assembly group H60

21.7 H70











21.8 H120



Figure 21-8: Assembly group H120

21.9 H130



Figure 21-9: Assembly group H130