

Geology of Belton Reservoir Area Leon River, Bell County, Texas

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REGIONAL GEOLOGY

Location of project. Belton Reservoir will be formed by construction of an earthen dam on the Leon River at about river mile 16.7, approximately 3 miles north of Belton. The reservoir will occupy a northern portion of Bell County and a southeastern portion of Coryell County.

Sites investigated. An extensive subsurface investigation was made of the project document site at river mile 15.5. Subsurface explorations disclosed adverse leakage and unfavorable geologic conditions, and as a result the site was abandoned. A dam site at about river mile 16.7 was investigated and found to be geologically satisfactory for development of the definite project dam.

PHYSIOGRAPHY

The Belton Reservoir will occupy a portion of the Central Texas section of the Great Plains physiographic province. The original surface of the Central Texas section was a broad plateau supported by resistant Cretaceous limestones. The plateau has been eroded by major streams and their numerous tributaries to such an extent that at the present time it appears as a maturely dissected plateau. Resistant limestones of the Edwards formation form the cap rock on the divides between the principal drainage ways and on the numerous buttes and outliers in the area.

There are numerous flat-surfaced areas between the divides, buttes, and outliers, but much of the area is of rolling or hilly character becoming moderately rugged in the southeastern portion of the reservoir. The valley slopes, in general, are moderately steep and rough, and the valleys are about one-half mile wide. The drainage of the area is to the southeast, generally consequent upon the dip-plain of the underlying Cretaceous formations.

The topography of the section within and adjacent to the lower portion of the reservoir, where the Edwards limestones are more prevalent, is moderately rough. The Edwards usually forms steep bluffs along the principal streams and the smaller creeks and drains. The numerous laterals of the main streams are deeply incised into the resistant formation. The relief of the section near the upstream portion of the reservoir is comparatively flat and featureless except for the remnants of the old plateau surface which occur as steep sloped

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buttes and outliers or as narrow ridges along the stream divides. The area is locally designated as the Walnut Prairie, so termed because it is within the outcrop of the shale and argillaceous limestones of the Walnut formation.

STRATIGRAPHY

The reservoir area and dam sites are located within the outcrop of the formations of Lower Cretaceous age. These formations are of the Trinity, Fredericksburg, and Washita groups. The aerial geology of Belton Reservoir is shown on plate 1. Data for the plate were compiled from various maps, texts, and bulletins on the geology of the region and a study of surface outcrops in the reservoir and dam site areas.

The oldest or Trinity group outcrops in the western portion of the reservoir along Cowhouse Creek, and dip eastward beneath younger beds. The group is represented by outcrops of Glen Rose limestones and Paluxy sands.

The Fredericksburg group consists of the Walnut shale, Comanche Peak limestone, and Edwards limestone. These formations outcrop throughout the dam site area and in a major portion of the reservoir area.

Strata of the Washita group outcrop over a limited portion of the reservoir area east of the Leon River. The section is composed chiefly of limestone and shale strata.

STRUCTURE

The Trinity, Fredericksburg, and Washita strata dip eastward at a rate of 30 to 40 feet per mile and the strike of these beds in Central Texas is almost north-south. The oldest formations outcrop in the upper reservoir area and dip under younger formations at a greater angle than the regional slope of the land surface. Consequently, a progressively younger series of formations is encountered southeastward from the upstream reservoir areas.

Two faults of the Balcones zone of faulting cross the Leon River Valley in the vicinity of the dam sites. One fault crosses the valley at about river mile 15.3, the other at mile 16.1. The faults have a prevailing trend to the northeast, are of normal character, and have small displacement with downthrow to the east. The displacement in each fault is about 30 feet. Investigations revealed that the faults do not extend under the dam sites nor into the reservoir area. There are no records or evidences of recent damaging earthquakes occurring in the area under consideration.

DEFINITE PROJECT SITE

SUBSURFACE INVESTIGATIONS

Previous investigations. Geological explorations at the definite project site (mile 16.7) were made in 1938 and 1946 in connection with the investigation of the project document site. The explorations

consisted of drilling 13 auger borings and 5 core borings, and conducting a general geological reconnaissance of the dam site and reservoir area.

Definite project investigations. Geological investigations performed for the definite project report consisted of an extensive core drilling program to afford visual examination of the foundation materials and to determine the structural and stratigraphic conditions at the dam site. A program of water-pressure testing the core holes was conducted to determine the porosity of the foundation materials.

Fifty-seven borings were made, consisting of 35 two-inch and 5 six-inch core borings and 17 six-inch undisturbed borings. Nine of the undisturbed borings were extended by means of 8-inch auger and/or 2-inch core borings, as required. Boring locations and a geologic profile of the foundation materials adjacent to the axis of the dam are shown on plate 2. Geologic profiles of various sections of the dam site foundation are shown on plates 3 and 4 and the graphic logs of the 57 borings are shown on plates 5 to 16, inclusive.

GEOLOGY OF THE DAM SITE

SOILS

The upland areas of the dam site are covered with a thin mantle of residual soils consisting of silts, clays, sands, gravels, and limestone materials in combination. The residual materials generally range from a few inches, near rock outcrops, to 2 or 3 feet in thickness and in upland areas occasionally attain a maximum thickness of 5 to 10 feet. On the abutment faces, which have slopes ranging from about 1 on 2 to 1 on 3, the mantle is very thin, consisting of residual clay and limestone materials. Large boulders and limestone fragments occur on the slopes between weathered outcrops. The soil mantle in the flood plain area of the dam site consists chiefly of alluvial sands, silts, and clays in various combinations. The overburden varies from 20 to 40 feet in thickness with the maximum section occurring in the vicinity of the right bank of the Leon River. A stratum of sand and/or gravel, 2 to 15 feet in thickness, underlies the overburden and rests upon a 2- to 7-foot stratum of weathered Walnut formation. Talus-like deposits composed of weathered rock fragments, boulders, and limestone boulders in a clay, sand, and gravel matrix occur along the toe of each abutment.

STRATIGRAPHY

Rock strata outcropping at and penetrated by borings at the dam site, from oldest to youngest, are those of the Walnut, Comanche Peak, and Edwards formations of the Fredericksburg group of Lower Cretaceous age.

Walnut formation. The Walnut formation underlies the alluvial deposits in the flood plain area, and underlies the Comanche Peak formation in the right and left abutments. The formation is medium

hard and compact except where exposed to weathering or reached by groundwater which softens the shale strata to a depth of 2 to 7 feet below contact with alluvial materials. The formation is made up of alternating beds of argillaceous limestone and calcareous shale. The limestones are moderately hard to hard, depending on the content of argillaceous material, and range in color from light to dark gray. Numerous thin irregular partings and seams of calcareous shale occur in almost all the limestone strata. The limestones are fossiliferous and occasionally contain flakes, clusters, and crystals of pyrite. Many of the shale strata of the Walnut formation are very calcareous, and it is difficult to distinguish them from the argillaceous limestone phases of the formation. They range from light gray, where high in lime content, to dark gray or black in color and are thinly laminated where calcareous materials are absent. The shales are moderately hard, fossiliferous, and pyritic and contain numerous thin seams and nodular pieces of gray limestone. Typical Walnut shell beds occur in the lower portions of the formation. These beds are a shell agglomerate in a dark gray, calcareous, shale matrix. They are moderately hard and for the most part the shells are firmly cemented together. These beds range from a few inches to 10 feet in thickness and often are interbedded with seams of limestone or shale. The maximum thickness of Walnut penetrated by core borings was 116 feet. However, the formation attains a maximum thickness of from 165 to 185 feet in Bell County.

Comanche Peak formation. The Comanche Peak formation forms the steep slopes of both abutments, between elevations of 540 feet and 605 feet on the right abutment and elevations of 535 feet and 600 feet on the left abutment. The formation is uniformly 65 feet in thickness at the dam site and consists chiefly of chalky, nodular pieces of limestone in a matrix of limy marl or shale. The limestones are white to light gray in color and are moderately hard and occasionally fossiliferous. The formation is not distinctly bedded and the contact with the underlying Walnut is transitional. On the outcrop, the formation has a dull chalky texture. Weathering of the formation, on the outcrop or where overlain by alluvial deposits, rarely extends into the limestones more than 1 or 2 feet.

Edwards formation. The Edwards formation conformably overlies the Comanche Peak and is readily distinguishable from it by the absence of argillaceous materials, the presence of flint and chert, and the honeycombed or vuggy appearance of the limestone. Facies of the Edwards are varied, some of the strata appearing as nearly pure, fragmental, coquina-like limestone, miliolid limestone, arenaceous limestone, marly limestone, or rudistid and caprinid shell agglomerates. The formation is predominantly tan to light gray in color, is generally medium hard to hard, and is massively bedded. The Edwards outcrops above an elevation of 605 feet on the right abutment and just below an elevation of 600 feet on the left abutment. Borings at the dam site indicated the formation to be 52 feet thick. Many of the limestone strata appear porous and vuggy; however, evidence of

large continuous openings, solution channels, or caves was not revealed in any of the borings. A study of outcrops in the dam site area did not reveal any evidence of cavernous condition in the limestones, although a honeycombed condition does exist on surface exposures.

STRUCTURE

Analysis of the geological profiles shown on plates 3 and 4 reveal that the dip of the primary formations is towards the east at a rate of about 35 feet per mile. The geological profiles on plates 2 and 3 show that the contact between the Comanche Peak and Edwards formation outcrops on the right abutment about 5 feet higher than on the left abutment. The profiles indicate that the main portion of the dam axis nearly parallels the strike of the primary formations.

A normal fault occurs about 2,000 feet downstream of the dam axis crossing the Leon River Valley at about river mile 16.1. It has a displacement of about 30 feet with downthrow to the east and can be traced in a northeast-southwest direction for a distance of about 1½ miles on each side of the Leon River. The line of faulting does not extend beneath the dam site nor into the reservoir area. The fault was not involved in the construction of the dam.

WEATHERING OF BEDROCK

The zone of weathering in the Edwards limestone formation is difficult to determine due to the color and composition of the beds. Solution action has in the past discolored and altered various strata in the formation giving the limestone a weathered appearance. Several borings in the spillway section indicate weathered rock to a depth of 3 to 5 feet. Weathering has penetrated the Edwards limestones from 3 to 7 feet on the right abutment. Inspection of Comanche Peak outcrops on the abutment slopes indicates that weathering has not penetrated the formation more than 2 feet. A considerable depth of weathering has occurred in the exposed Walnut formation outcrops on the abutment faces below an elevation of 540 feet and it appears that the formation cannot be classed as being resistant to weathering or erosion. Weathering of the Walnut strata underlying alluvial deposits in the floodplain area varies from 2 to 7 feet into the formation. The base of the weathered zone ranges from an elevation of 455 feet to 460 feet in the flood plain area, is at about an elevation of 470 feet near the toe of the right abutment, and at an elevation of 480 feet at the toe of the left abutment.

FOUNDATION CONDITIONS AT STRUCTURE LOCATIONS

Earth embankment. The formations beneath the earth embankment area on the right and left abutments consisting of limestone and shale strata were found to be structurally sound and satisfactory to serve as the foundation for the earth embankment. The rock formations below an elevation of 600 feet were found to be highly impervious. Borings in the Edwards formation reveal the presence of numerous small to large solution cavities and channels partly to

tightly filled with marl and in some strata with porous lime deposits. The Walnut formation under the flood plain area of the embankment is structurally sound and impervious below the weathered zone. Tests performed on samples of the Walnut formation obtained at the project document site indicate that the unconfined compression strength of the shales ranged from 14.8 to 19.8 tons per square foot.

Outlet works. Two sites were investigated for the location of the outlet works. One site considered the construction of a tunnel through the right abutment. The other site considered a concrete conduit beneath the earth embankment located near the toe of the right abutment. The location of these sites is shown by sections C-C and D-D, respectively, on plate 2.

A 22-foot diameter flood-control tunnel having an intake invert elevation of 483 feet and an outlet invert elevation of 477 feet is considered adequate to pass the flood flows through the dam. Section C-C, plate 2, and boring logs on plates 14, 15, and 16 reveal that the tunnel would be cut through the Walnut formation consisting of argillaceous limestone, calcareous shale, and shell agglomerate beds. It was determined that these strata are moderately hard, impervious, and structurally sound. The results of water-pressure testing the tunnel core borings are shown in table 1.

Analysis of the conduit alignment borings reveals that the base of the weathered shale and limestone strata of the Walnut formation occurs at about an elevation of 470 feet, and that a satisfactory foundation for the conduit outlet works can be had at and below elevation 470. Approximately 40 feet of overburden including a stratum of weathered shale and limestone would have to be removed to secure satisfactory foundation rock. Additional borings at the selected outlet works site will be required for final design purposes.

Spillway. The spillway is located on the left abutment as indicated on plate 2. A 1300-foot wide, broad crested spillway having an approach channel and crest at an elevation of 631.0 feet was found to be most economical. The crest of the spillway coincides with the axis of the earth embankment extending across the spillway. From the crest, the spillway discharge channel extends downward on a 2 per cent grade to intersect natural ground at a low elevation of about 610. Thus, the spillway is within and underlain by the Edwards formation which is considered to be very resistant to erosion and weathering.

Core borings within the spillway area reveal that the Edwards limestone is porous and honeycombed, and will permit some leakage through the strata if water should be impounded above elevation 600 for a long period of time. At the downstream end of the spillway discharge channel, the Comanche Peak formation occurs beneath the overburden between approximate elevations of 600 feet and 530 feet and immediately overlies the Walnut formation. The Comanche Peak is impervious and fairly resistant to erosion but slakes down rapidly when exposed to repeated wetting and drying. The Walnut is im-

pervious but cannot be classed as resistant to erosion, although a few hard limestone layers occur within the formation. Investigation reveals that the spillway will be in operation only during floods considerably greater than the maximum flood of record and accordingly will be used very infrequently. In view of the foregoing, it appears that the spillway foundation materials are structurally sound and that weathering and erosion of the spillway, if any, will not be detrimental to its stability.

LEAKAGE

Leakage through alluvial and residual materials. Alluvial deposits in the flood plain at the dam site are generally of silt and clay content but contain layers of sand and gravel of varying thicknesses. Pervious strata of sand and gravel occur at the base of the alluvium and overlie a zone of weathered Walnut formation. A maximum thickness of 16 feet of sand and gravel was found in boring 6D8A-101. The pervious strata are not well sealed by overlying alluvium, and outcrops of sand and gravel occurring in the upstream flood plain may be interconnected with those at the dam site whereby water would have easy entrance to the basal pervious strata. Borings in the area also revealed that weathering has penetrated into the primary formation from 2 to 7 feet with a maximum thickness of 19 feet of partly softened and slightly weathered formation occurring in boring 6D8A6C-96. An impervious cut-off wall extending through the pervious strata and zone of weathered formation for the full width of the flood plain is necessary to prevent detrimental leakage of water beneath the earth embankment. The residual soil mantle on the face and top of the abutments is of porous character, and its removal or construction of an impervious cut-off extending into the primary formation was deemed necessary to prevent leakage.

Leakage through bedrock. Water-pressure tests were conducted in 35 two-inch core holes in the dam site area. These tests indicated that all weathered materials regardless of the characteristics of the formation with which they are associated will readily transmit water and should be removed, grouted, or cut off by a core trench to insure a water-tight foundation. The results of the water-pressure tests made on strata of the three formations at the dam site are given in table 1.

On the left abutment, water-pressure tests were conducted in 18 core holes. Considerable leakage was developed in core hole 2C-77 and of lesser amount in six other holes of which five are located in the spillway discharge area and the other beyond the spillway. Testing of the six holes revealed that the zone of leakage exists in the Edwards formation at or above an elevation of 607 feet. The maximum leakage in these holes was 19.5 gallons per minute under 20-pound pressure in boring 2C-93. Core hole 2C-77 is located on the crest of the abutment about 350 feet downstream of the axis of the dam. Top of hole is at an elevation of 626.6 feet and bottom at about an elevation of 450 feet. Testing at a pressure of 30 pounds per square

inch between the elevations of 593 feet and 450 feet disclosed leakage of 23.4 gallons per minute. At 5- and 10-pound pressures between the elevations of 598 feet and 505 feet the leakage was negligible, being 0.75 gallons per minute. Between the elevations of 503 feet and 450 feet at 5-pound pressure, the leakage was 7.3 gallons per minute; thereby indicating that leakage occurs in the Walnut formation. In view of the imperviousness of the formation as revealed by tests in adjacent holes, it appears that leakage in 2C-77 was probably towards the abutment face.

Eleven core holes on the right abutment were water-pressure tested. Excessive leakage was developed in hole 2C-123 through Edwards strata and of lesser amount in five other holes. The leakage zone in these five holes was found to exist in the Edwards formation at and above an elevation of 620 feet. The maximum leakage occurred in hole 2C-118 which amounted to 34 gallons per minute under 5-pound pressure. Boring 2C-123 is located near the crest of the abutment in close proximity to the axis of the dam. Ground surface at the boring is at about an elevation of 637 feet and bottom of hole at an elevation of 474 feet. Leakage between the elevations of 615 feet and 604 feet amounted to 19 gallons per minute with no pressure applied. Between the elevations of 605 feet and 594 feet no leakage was developed under 50-pound pressure, between the elevations of 595 feet and 474 feet the leakage was about 15 gallons per minute at 50-pound pressure, and between the elevations of 555 feet to 474 feet about 0.4 gallons per minute at 10-pound pressure. The tests indicate that leakage occurred in both the Edwards and Comanche Peak formations.

In general, the tests reveal that the primary Comanche Peak and Walnut formations at the dam site are relatively impervious below an elevation of 600 feet and that some leakage will occur through the Edwards formation above this elevation. Grouting of the pervious Edwards formation is deemed inadvisable at this time. However, excavation of the spillway area will reveal the character of the porous strata and may dictate the desirability of providing a grout cut-off through the strata. Therefore, it is proposed that an item of foundation grouting be included in the estimated cost of the project to provide for such action. With respect to the leakage in 2C-77 and 2C-123, it is proposed that additional water-pressure tests be performed to determine more definitely the leakage characteristics of foundation rocks near both abutment faces and to provide adequate data for the design of any necessary cut-off walls.

SUMMARY AND CONCLUSIONS

Summary. Subsurface investigations at the definite project site, together with examination of rock outcrops, made a 225-foot section of Lower Cretaceous strata available for visual examination and revealed the composition of the formations, formational contacts, the dip and strike of the beds, and structural characteristics. Water-

pressure tests performed in the core borings provided accurate data for determination of leakage characteristics of the formations. The investigations also revealed the nature and extent of alluvial and residual materials, and weathering characteristics of the rock formations.

Conclusions. Based on results of the foregoing investigations, it is concluded that:

1. The dam and reservoir sites are physiographically and geologically suitable for impounding the waters of the Leon River.
2. The dam site is free of adverse structural features.
3. Unweathered portions of the Walnut, Comanche Peak, and Edwards formations are structurally sound and will form a satisfactory foundation for the proposed structures.
4. Pervious strata occur in the alluvial deposits in the flood plain at the dam site and removal of these strata or a cut-off through them will be required to prevent leakage under an earth embankment section.
5. The residual materials on the steep slopes and tops of both abutments are pervious and should be removed to provide satisfactory foundation conditions.
6. Removal of the weathered portion of foundation rocks or a cut-off through them will be required to insure satisfactory foundation conditions.
7. Edwards strata above an elevation of 600 feet on both abutments are pervious and leakage will take place through them.
8. Comanche Peak strata at the dam site are impervious, except in localized zones on the crests of the left and right abutments. These leaky zones can be effectively grouted to prevent leakage through the foundation rocks.
9. The Edwards and Comanche Peak strata in the spillway discharge channel are erosionally resistant and will provide a protective cap over the more easily eroded Walnut strata.
10. A large amount of weathered Walnut shale and argillaceous limestone will have to be removed from a narrow bench or terrace in the area of the outlet works at the toe of the right abutment to insure satisfactory foundation conditions.
11. Shale and limestone strata of the Walnut and Comanche Peak formations should be protected against weathering when exposed during construction operations.

PROJECT DOCUMENT SITE

SUBSURFACE INVESTIGATIONS

Investigation of project document site. Geological investigations of the dam site at mile 15.5 on the Leon River were made in 1938 for the purpose of determining the most feasible site for the project document dam. Field investigations included the drilling of four 2-inch core borings along proposed dam axis, and conducting a general reconnaissance of the dam site and reservoir area.

Upon authorization to prepare a definite project report on Belton Reservoir, an extensive program of core drilling and pressure testing of the project document dam site was initiated in February, 1948, to determine more definitely the suitability of the site and to obtain adequate data for design purposes. The program was cancelled only after considerable investigational work definitely established the unsuitability of the site. A total of 39 core borings were made, of which

25 were 2-inch, 10 were 6-inch, and 4 were 30-inch. Water-pressure tests were performed in 13 core borings. A grout test panel covering 40 feet of spillway length was made on the left abutment. Unconfined compression tests were made on samples of the primary formation underlying the flood plain. The location of the completed borings is shown on plate 18. Graphic logs of 12 borings which reveal the detrimental character of the foundation materials are shown on plate 19.

GENERAL GEOLOGY OF DAM SITE

Geologically, the project document site at river mile 15.5 and the definite project site at mile 16.7 are similar in character with the exception that the primary formations at the project document site occur at lower elevations. Thus, the information given in Geology of the Dam Site of this report is applicable, in general, to the project document site.

Two normal faults are present near the site. One is located about 1,000 feet downstream of and somewhat parallel to the proposed dam axis. The fault has a displacement of about 30 feet with downthrow to the east, and rock strata in the fault zone are badly broken and contain solution channels. The fault would not be involved in the construction of the dam. The other fault crosses the Leon River Valley at about river mile 16.1 and crosses the south divide of the reservoir at about 1/2-mile upstream of the site. This fault also has a displacement of about 30 feet with downthrow to the east. Preliminary investigations indicated the possibility that the Edwards limestone may be considerably more leaky along the fault zone than elsewhere, requiring a grout curtain to prevent excessive losses of reservoir water through the south divide into the Nolan Creek Basin. An investigation of this fault zone was not completed prior to abandonment of the project document site.

FOUNDATION CONDITIONS

Foundation stability. Unconfined compression tests were performed on three samples of shale strata of the Walnut formation underlying alluvial materials in the flood plain and on one sample of the Edwards formation from the spillway area. The tests indicate compressive strengths of 14.8 to 19.8 tons per square foot for the Walnut shale and 166 tons per square foot for the Edwards limestone. Tests were not performed on samples of the Comanche Peak formation which is structurally sound and of harder quality than the Walnut formation. Data regarding the compression tests are given in table 3.

Foundation leakage. It is planned that the Belton Reservoir will ultimately be operated for the generation of hydroelectric power in addition to its present planned use for flood-control and water-conservation purposes. The proposed top of power pool of the project

document plan would be at an elevation of 592 feet, which would necessitate an impervious foundation and reservoir below such elevation. Almost all strata in the Edwards formation above an elevation of 540 feet on both abutments were found to be potential leakage zones. Strata both immediately above and below the chert seam on both abutments are vuggy to cavernous. Core recovery was very poor in these strata and many cavities both open and clay filled were encountered. Water-pressure tests performed in the 2-inch core holes on both abutments indicated a large amount of leakage could be expected to take place through Edwards strata; however, the tests also showed that the areas of leakage were somewhat irregular, indicating that channeling has developed through the Edwards strata. Results of water-pressure tests at the project document site are presented in table 2. The Edwards limestone rises above the proposed power-pool elevation about a mile upstream of the dam site and is the only formation through which important leakage might take place.

Grout tests. Because of the porous condition of the rock formations at the project document site, grout tests were performed on the left abutment to determine the amount of grouting required to insure satisfactory foundation conditions. Two areas were selected for testing; one in a low saddle on the left abutment near boring 6C-31, where average leakage conditions were expected, and the other near boring 2C-61 where the worst leakage conditions were indicated by water-pressure tests.

In the saddle section, 3-inch diameter holes were drilled and grouted on 20-foot centers, covering a 40-foot panel. Intermediate holes were then drilled, pressure tested, and grouted on 10-foot centers and finally holes were drilled, tested, and grouted on 5-foot centers. These tests disclosed that an effective grout curtain could be placed at a reasonable cost in strata indicated by pressure tests to have a small or average amount of leakage through them. Location of the grout test panel and borings is shown on plate 18. A summary of the grout tests is presented in table 4.

Water-pressure tests in boring 2C-61, on the left abutment, indicated a potential leakage of more than 55 gallons per minute in 10-foot sections of the foundation rocks between an elevation of 555 feet and the ground surface (elevation 597 feet). These tests also revealed that strata from elevations of 555 feet to 523 feet are water tight. A grout test was performed in this hole by setting a packer at an elevation of 585 feet and pumping grout into the hole at water-cement mixes varying from 4 to 1 to $\frac{5}{8}$ to 1. The hole took 452 cubic feet of grout in eight and three-quarters hours with no pressure developed in the grout lines or hole during the test. The grout test clearly indicated abnormal foundation conditions occur in this area on the order of a large fracture or cavernous condition in the limestone strata. Investigation of this condition was continued by drilling large diameter inspection holes.

Inspection holes. Four 30-inch holes were drilled along the axis of the proposed dam. Three of these holes were drilled on the left abutment in the vicinity of boring 2C-61, and the other on the right abutment as boring 30C-4, (see plate 17). The holes were drilled to make possible visual examination of the foundation rocks in place and to examine foundation conditions in the grout test area on the left abutment near 2C-61.

Boring No. 30C-1 was drilled where unusually poor foundation conditions were indicated by water-pressure and grout tests. A cavern was reached in this boring at an elevation of 580 feet, which extended downward to an elevation of 576.8 feet. The opening was partially filled with clay and grout as well as typical calcareous cavern deposits. The boring was continued from an elevation of 576.8 feet to 552 feet with a 6-inch bit and the core showed grout filled cavities from elevations of 563.2 feet to 558.3 feet, indicating the zone of grout penetration in the grout test on boring No. 2C-61. Borings 30C-2 and 30C-3 were drilled 100 feet south and west, respectively, of boring 30C-1 and reached the cavernous zone at approximately the same elevation as in boring 30C-1. The cavernous zone was explored by cleaning out cavern deposits for a distance of 100 feet southwest from 30C-1. At this point, the cavern branched to the south and west for a distance of at least 50 feet in each direction. Examination of the cavern upon completion of clearing operation revealed that numerous small solution channels and openings lead off from the main opening. These laterals were observed to extend a considerable distance from the main cavern. All observations in the areas indicate that the openings are irregular, extensive, and inter-connected, and that a large amount of leakage would take place through the strata in which solution has taken place. Smoke generated at the bottom of boring No. 30C-1 vented from both the other 30-inch borings. Views of the cavernous condition of the foundation encountered in boring 30C-1 are shown on plates 20 and 21.

A 30-inch boring (30C-4) was made on the right abutment in an area indicated to be porous or cavernous by borings 2C-62 and 2C-11 and an opening was encountered between the elevations of 572.5 feet and 570 feet. This opening was observed to be the result of fracturing or faulting of the foundation rocks. Large scale solution action was not in evidence in the fracture zone; however, a definite opening is established on an east-west line and extends at least 15 to 20 feet in each direction from the 30-inch boring. Two-inch core borings drilled in this abutment strongly indicate that porous to cavernous openings and solution channels exist in the Edwards limestone.

CONCLUSIONS

The project document site is the most favorable site topographically, but cavernous Edwards limestones occur in the abutments below the top of the proposed power pool (elevation 592 feet). Exten-

sive explorations disclosed that serious leakage conditions exist within the Edwards limestones which could not be economically corrected by means of grouting. The existence of a fault which crosses the south reservoir divide is also considered an important source of leakage, but this condition was not fully explored. Representatives of the offices of the Chief of Engineers, Division, and District Engineers inspected the cavernous condition of the site and reviewed the results of the subsurface investigations. The representatives concluded that the site should be abandoned because of its unfavorable geologic conditions, and suggested that investigation be made of an upstream dam site at about river mile 16.7 where geological conditions appeared to be more favorable.

TABLE 1
SUMMARY OF WATER-PRESSURE TESTS
DEFINITE PROJECT SITE

Core Boring No.	Section tested		Length of core hole tested	Leakage — Gallons per minute						
	El. top	El. bottom		Pressure per sq. inch						
				5	10	15	20	30	50	
77	597.9	450.1	147.8	0.2					18.4	
77	592.9	450.1	142.8	2.2	8.9	7.5			23.4	
77	512.9	450.1	62.8	6.6						
77	502.9	450.1	52.8	7.3						
77	492.9	450.1	42.8	2.9						
77	482.9	450.1	32.8	1.8	3.0					
77	472.9	450.1	22.8	1.8	3.0					
77	597.9	505.1	92.8	0.6	0.8					
78	595.3	450.5	144.8						0.6	
79	594.4	445.4	149.0						0.0	
80	612.8	571.9	43.9		3.0					
80	607.0	571.9	38.1							0.0
81	594.6	444.8	149.8						0.0	
82	595.8	575.9	19.9						0.0	
83	599.6	562.2	37.4	2.9	3.7					
85	612.0	549.1	62.9						2.4	
85	607.0	549.1	57.9						1.9	
85	602.0	549.1	52.9							0.0
86	611.5	441.9	169.6				5.4			
86	601.5	441.9	159.6							0.0
87	618.4	549.1	69.3						0.6	
88	626.4	450.6	175.8							0.7
89	622.8	565.0	57.8							0.0
90	599.4	561.6	37.8							0.0
92	625.8	585.0	40.8						0.9	2.3
92	620.8	585.0	35.8							1.7
93	623.0	575.6	47.4				19.5			
93	618.0	575.6	42.4							0.1
94	615.2	527.3	87.9							0.1
115	635.0	565.4	69.6				0.3			
115	630.0	565.4	64.6							0.7
130	634.4	572.0	62.4							0.4
Flood Plain										
102	456.6	438.1	18.5						0.8	5.0
102	453.6	438.1	15.5							0.0
70**	475.1	447.1	28.0						1.0	
73**	465.0	445.2	19.8						14.9*	
105	462.0	448.1	14.5	5.5	13.8*					
105	459.8	448.1	12.3	10.5*						
105	455.8	448.1	8.3							0.0
131	484.4	436.6	47.8					0.2		
Tunnel Outlet Works										
122	596.5	473.7	122.8							0.0
123	595.1	473.7	121.4						6.7	14.9
123	579.0	474.7	105.3				4.0		7.5	
123	575.1	473.7	101.4	4.0						
123	570.1	473.7	96.4	4.0						
123	565.1	473.7	91.4	4.0						
123	555.1	473.7	81.4			0.42				
123	615.1	603.8	11.3	19.2***						
123	605.1	593.8	11.3							0.0
123	595.1	583.8	11.3	0.8						
123	585.1	573.8	11.3	0.7						
123	575.1	563.8	11.3							0.7
123	565.1	553.8	11.3	3.0						
124	594.2	467.5	126.3						0.0	
125	600.7	474.5	125.3	0.4						

THE WOODBINE AND ADJACENT STRATA

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TABLE 1 (Continued)

Core Boring No.	Section tested		Length of core hole tested	Leakage — Gallons per minute					
	El. top	El. bottom		Pressure per sq. inch					
				5	10	15	20	30	50
112	633.5	472.1	161.4					1.0	
112	628.5	472.1	156.4						0.0
113	629.8	471.0	158.8	16.5					
113	619.8	471.0	148.8						0.1
114	629.7	473.5	156.2			0.7			
114	624.7	473.5	151.2					0.2	
117	621.2	473.3	147.9						0.0
118	631.4	473.0	158.4	34.1					
118	622.2	473.0	149.2						0.0
119	631.2	471.8	159.4					0.6	1.1
120	630.3	472.0	158.3					2.5	
120	620.3	472.0	148.3					0.2	
122	596.5	473.7	122.8						0.0

* Water leak around packer.

** Located on center line of proposed concrete conduit outlet works.

***Hole took 19.2 Gal. per min. under no pressure.

TABLE 2

SUMMARY OF WATER-PRESSURE TESTS
PROJECT DOCUMENT SITE

Core Boring No.	Elevation of test section		Length tested	Leakage — Gallons per minute					
	Top	Bottom		Pressure per square inch					
				0	10	15	20	25	30
2C-11	568.8	518.6	50.2		32.3*				
2C-11	563.8	518.6	45.2						10.1
2C-11	543.8	518.6	25.2						0.0
2C-12	551.7	510.9	40.8	16.8*					
2C-12	546.2	510.9	35.3						1.6
2C-13	560.2	507.7	52.8						7.9
2C-13	536.2	507.7	28.5						7.7
2C-13	526.2	507.7	18.5						0.0
2C-14	549.5	442.0	107.2						0.8
2C-62	598.6	489.2	109.4	24.2*					
2C-62	593.6	489.2	104.4				5.7		
2C-62	558.6	489.2	69.4				2.17		
2C-62	548.6	489.2	59.4				0.15		
Left Abutment									
2C-28	559.0	517.1	41.9						0.5
2C-28	535.0	517.1	17.9		0.0				
2C-34	594.0	531.9	62.1		18.7*				
2C-34	589.0	531.9	57.1					3.56	
2C-34	579.0	531.9	47.1					2.9	
2C-35	564.1	526.6	37.5						9.7
2C-35	559.1	526.6	32.5						0.6
2C-42	575.8	535.8	40.0		44.4*				
2C-42	570.8	535.8	35.0						0.8
2C-43	590.6	531.5	59.1			29.1*			
2C-43	585.6	531.5	54.1					9.8	
2C-43	540.6	531.5	9.1						0.0
2C-61	597.0	523.3	73.7	55.0*					
2C-61	585.3	574.3	11.0	28.2*					
2C-61	575.3	523.3	52.0	27.4*					
2C-61	565.3	523.3	42.0	27.8*					
2C-61	555.3	523.3	32.0				0.51		
2C-63	596.5	517.8	78.7			45.8			
2C-63	591.5	517.8	73.7						4.2
2C-63	561.5	517.8	43.7						0.7
G-1	589.2	534.4	54.8					23.8	
G-1	587.4	534.4	53.0					1.1	

*Pump capacity on this test.

TABLE 3

SUMMARY OF UNCONFINED COMPRESSION TESTS
PROJECT DOCUMENT SITE

Core Boring No.	Elevation of sample	Depth feet *	Unconfined compression Tons/sq. foot	Type of formation
6DC-18	464.1	39.6	16.5	Walnut shale
6DC-21	461.9	30.0	14.8	Walnut shale
6DC-24	456.5	35.3	19.8	Walnut shale
6DC-31	589.1	11.4	166.1	Edwards limestone

*Depth below ground surface at boring location.

TABLE 4

SUMMARY OF GROUT TESTS
PROJECT DOCUMENT SITE

Core Boring No.	Location dam axis stationing	Elevation of test		Grout mixes water-cement	Maximum pressure lbs./sq. in.	Sacks of cement used
		From	To			
G1	70+57	589.4	534.4	2:1 to 1:1	30	8
G6	70+62	589.4	573.6	6:1	20	3
G4	70+67	588.4	571.4	6:1 to ¾:1	25	40
G7	70+72	594.8	572.5	6:1 to 1:1	20	14
G3	70+77	588.4	530.6	6:1 to 1:1	30	34
G8	70+82	594.2	572.6	6:1 to 4:1	20	2
G5	70+87	589.2	572.7	6:1	25	2
G9	70+92	593.8	577.8	6:1 to 2:1	20	21
G2	70+97	588.8	527.0	6:1 to 4:1	25	16

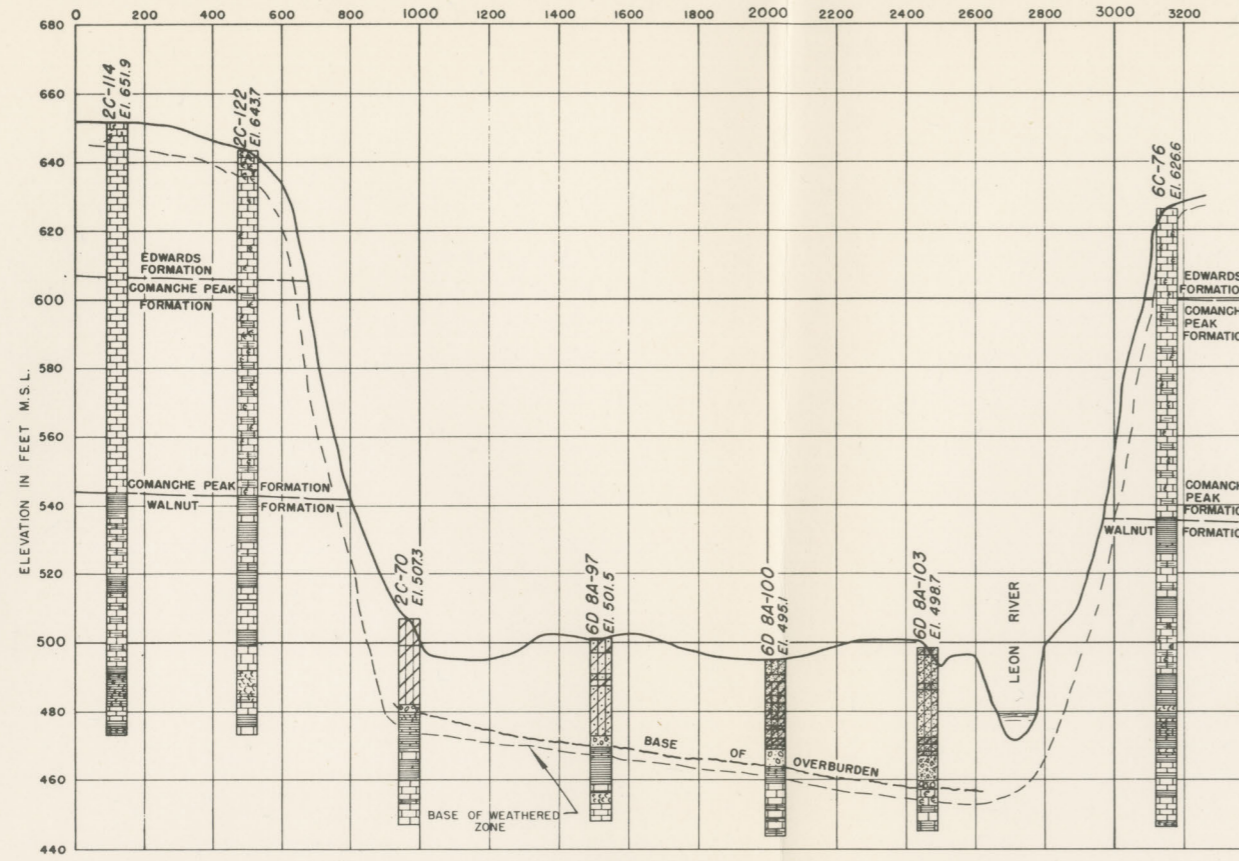


- LEGEND**
- LOWER CRETACEOUS AGE
- Kw WASHITA GROUP (UNDIFFERENTIATED)
 - FREDERICKSBURG GROUP
 - Kcep EDWARDS-COMANCHE PEAK
 - Kwo WALNUT
 - K1 TRINITY GROUP (UNDIFFERENTIATED)

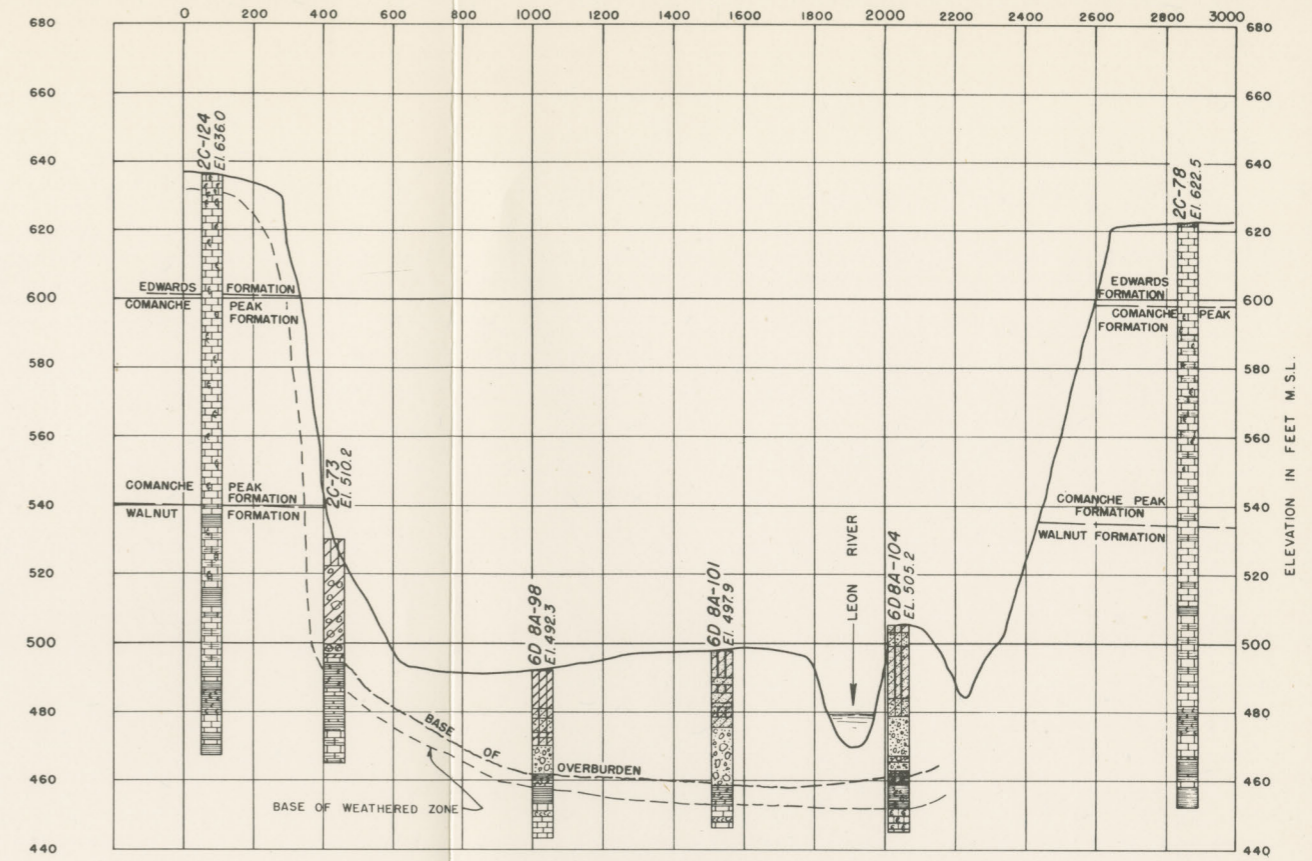
BRAZOS RIVER BASIN, TEXAS
 BELTON RESERVOIR
 LEON RIVER, TEXAS
AREAL GEOLOGY

SCALE OF FEET
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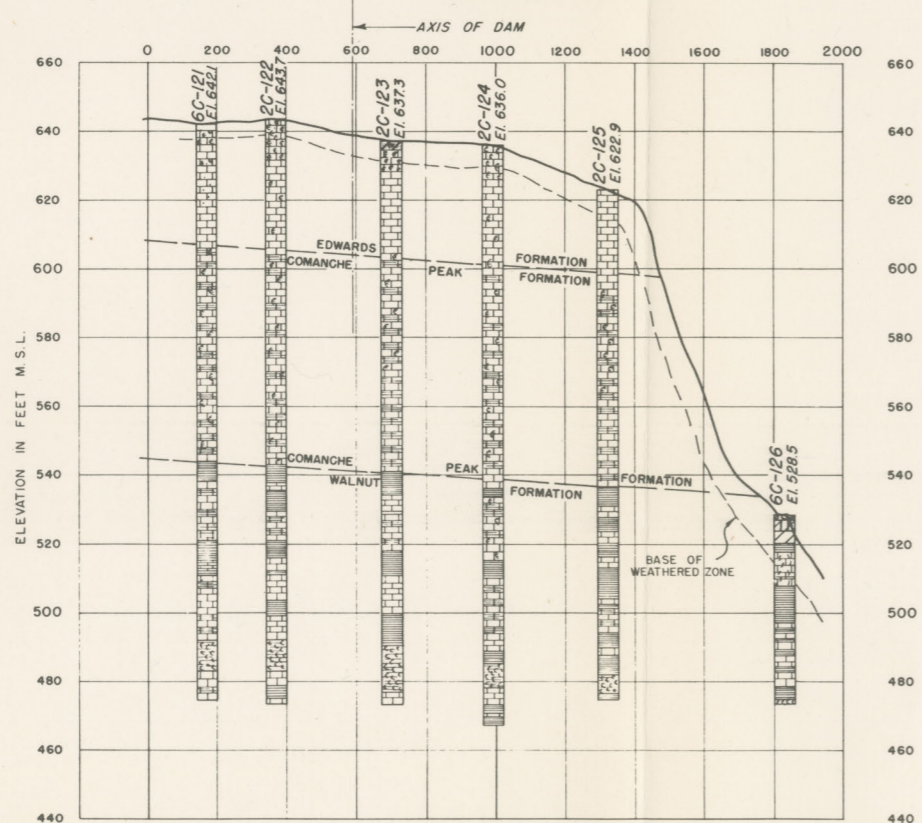
GALVESTON DISTRICT, GALVESTON, TEXAS MAY 1949
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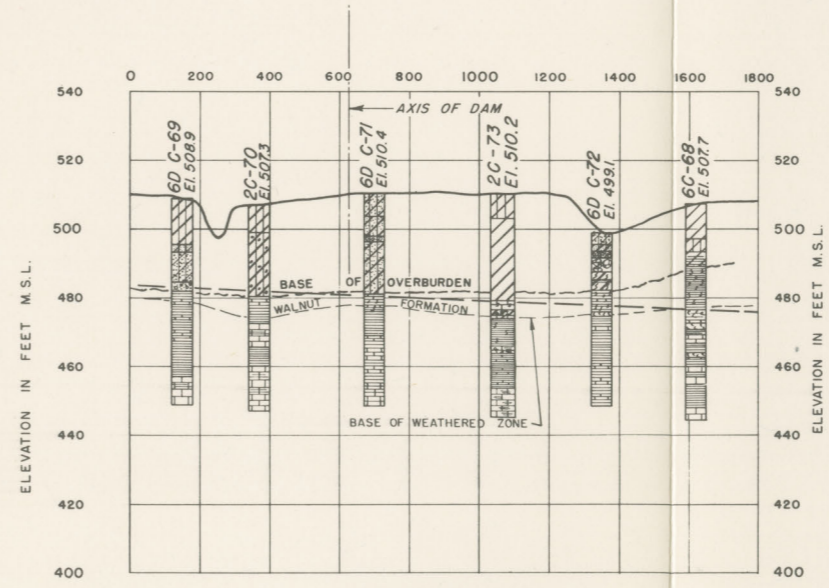
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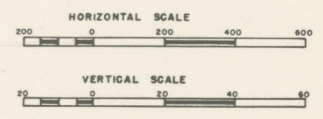
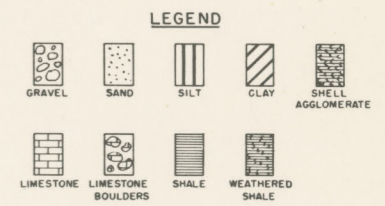
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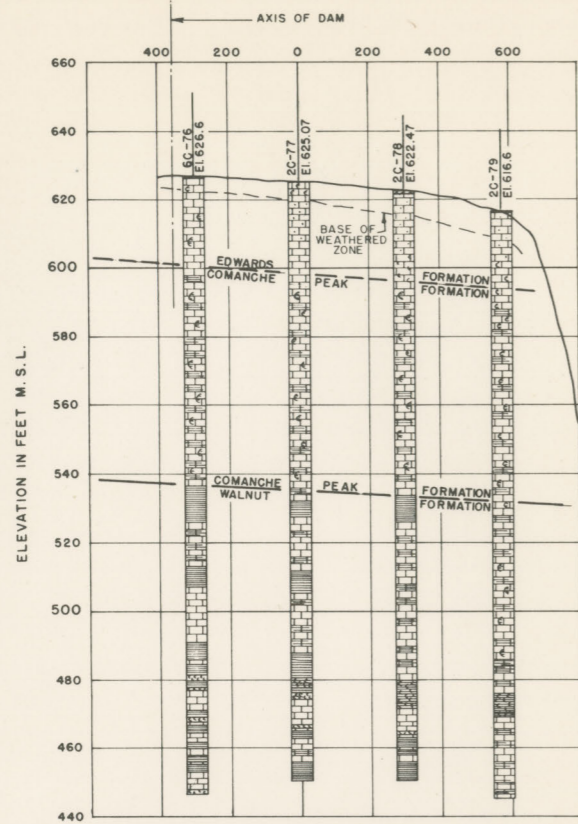
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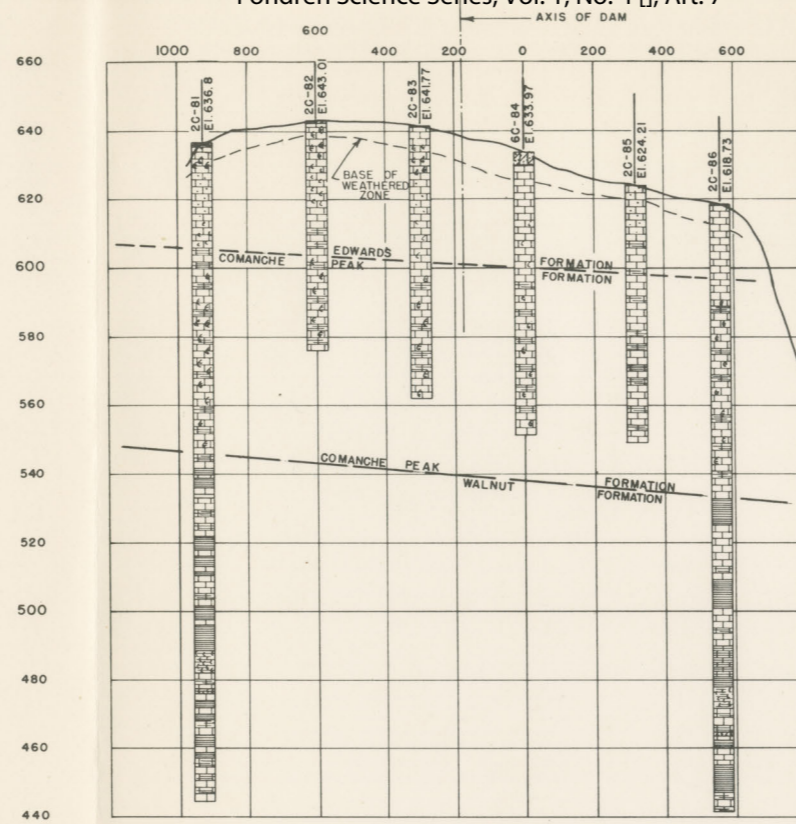
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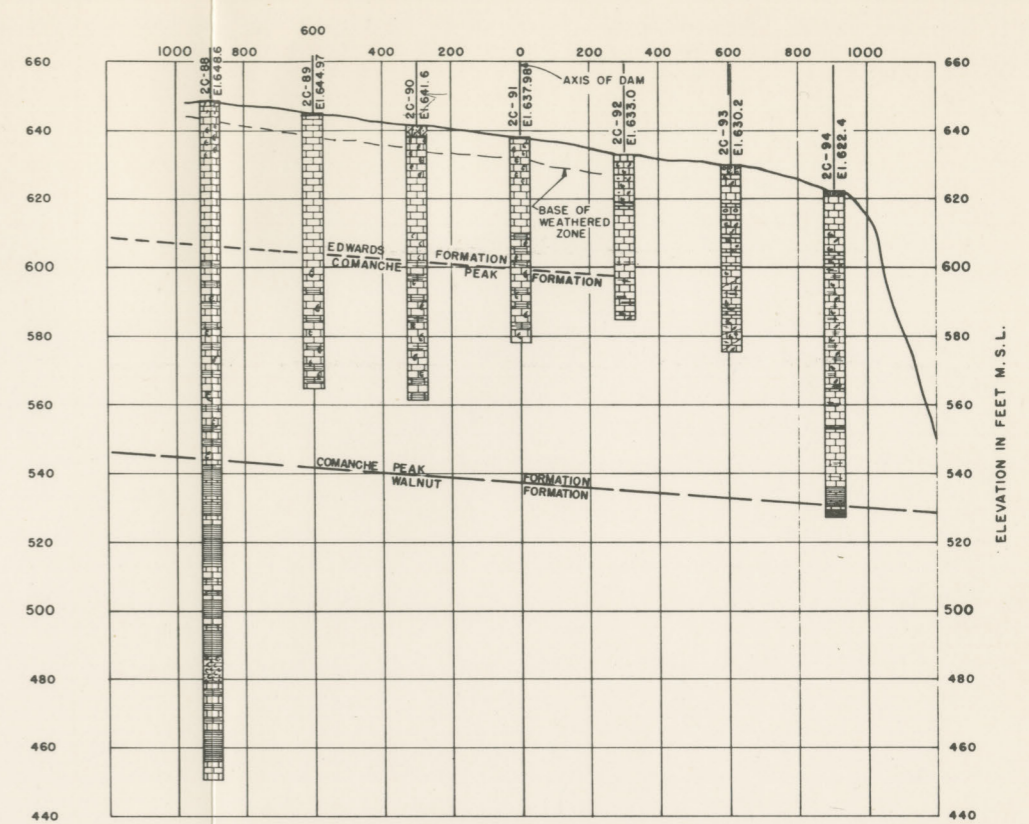
BRAZOS RIVER BASIN, TEXAS
 BELTON RESERVOIR
 LEON RIVER, TEXAS
CORE BORINGS
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 SCALE AS SHOWN
 GALVESTON DISTRICT, GALVESTON, TEXAS
 TO ACCOMPANY DEFINITE PROGRESS REPORT
 FILE BRAZ 503-203 APPENDIX III PLATE 3



SECTION E-E







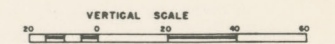
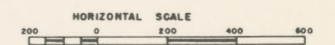
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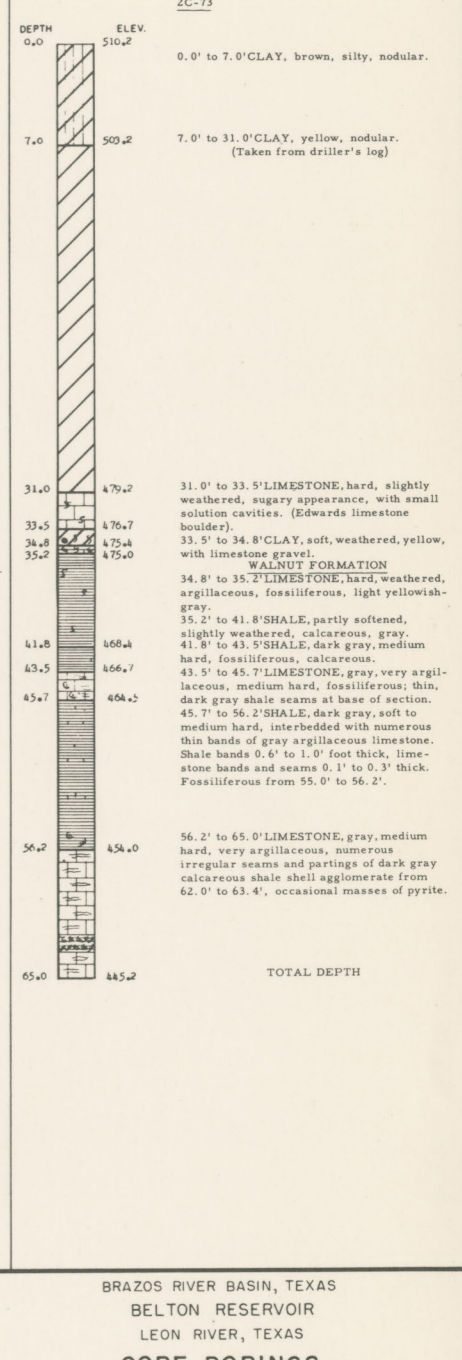
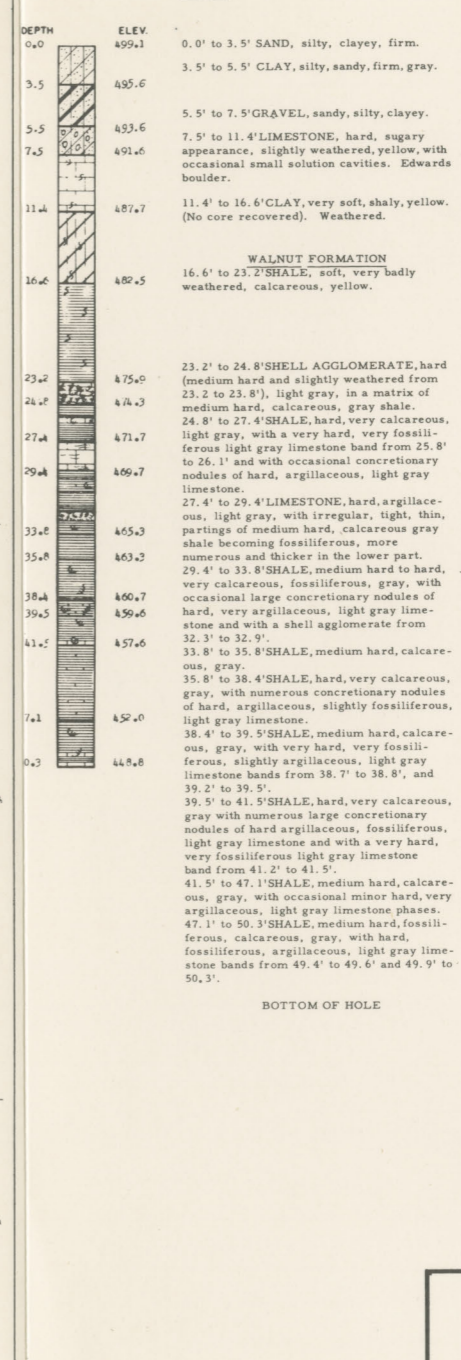
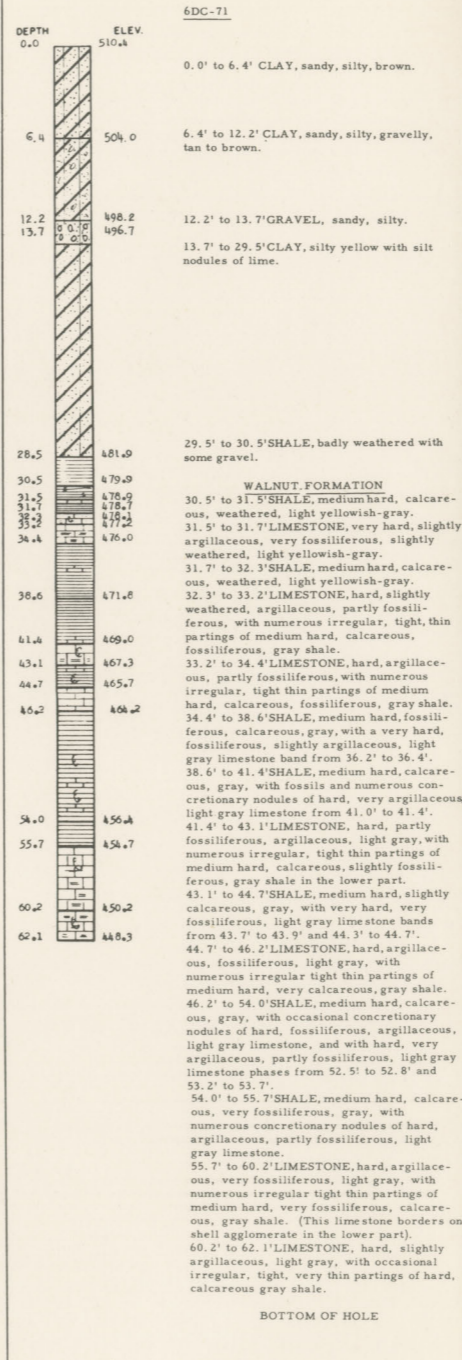
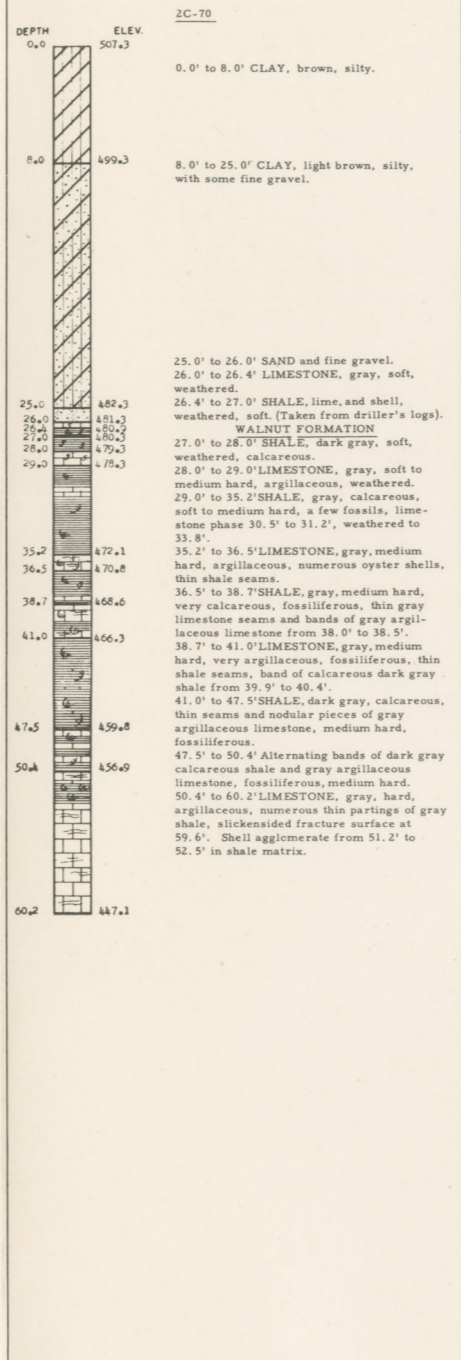
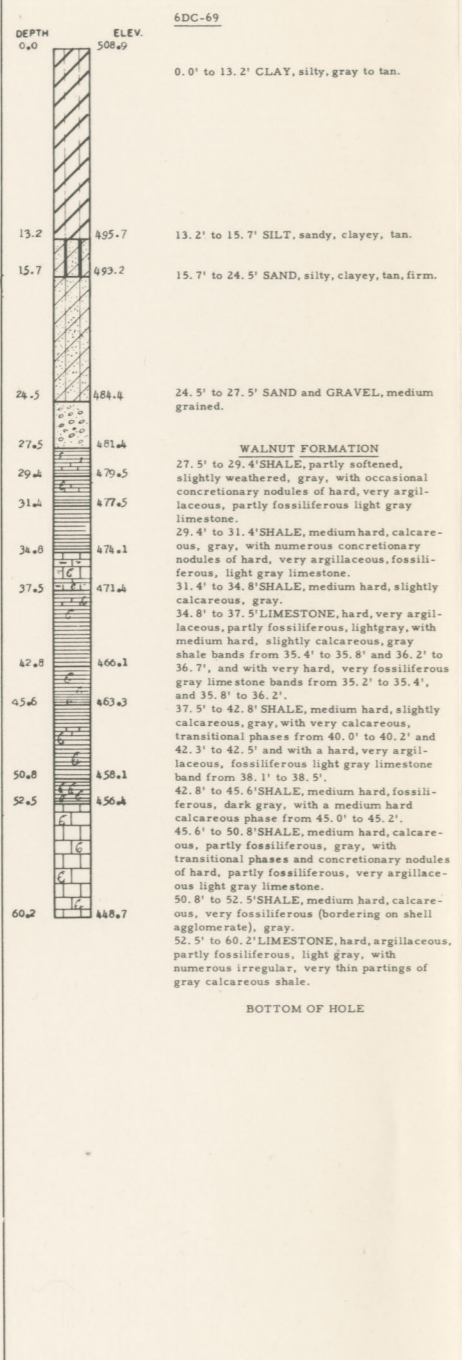
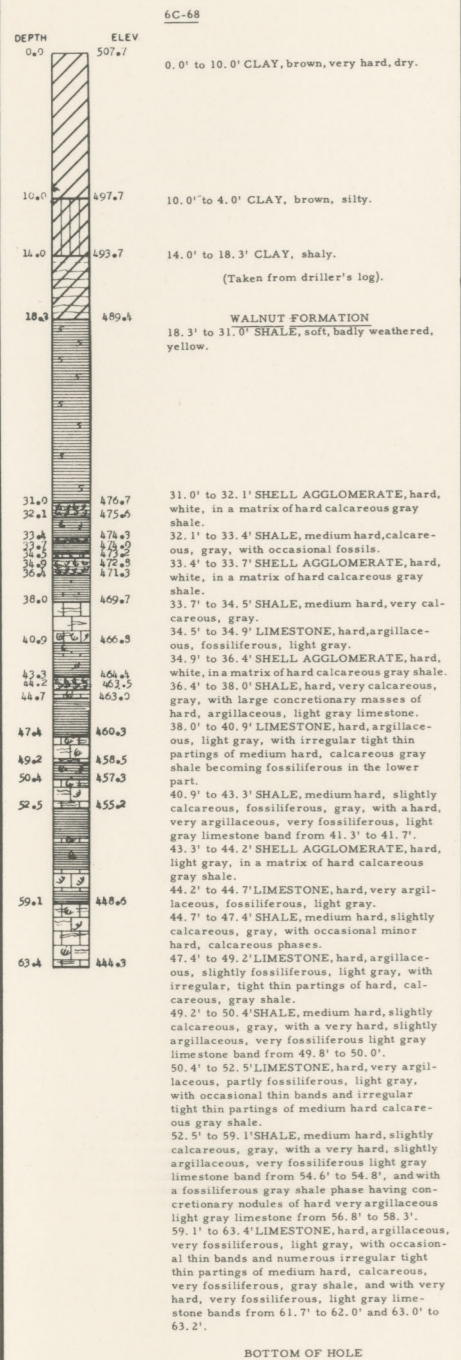
SECTION G-G

LEGEND

-  Limestone
-  Shaly Limestone
-  Shale
-  Shell Agglomerate



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 FILE BRAZ 503-203 APPENDIX III PLATE 4

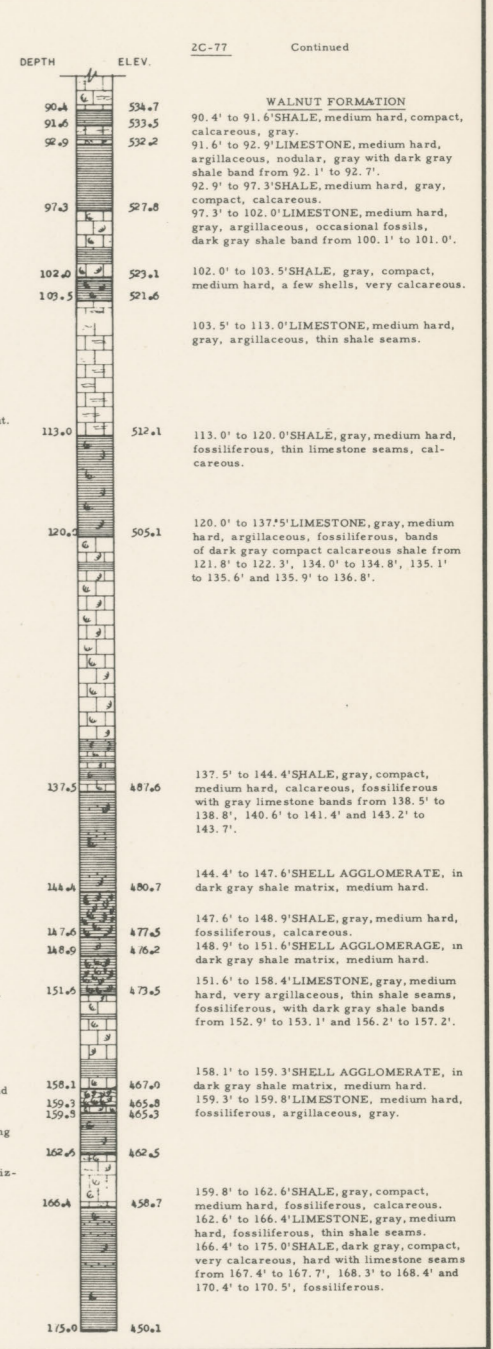
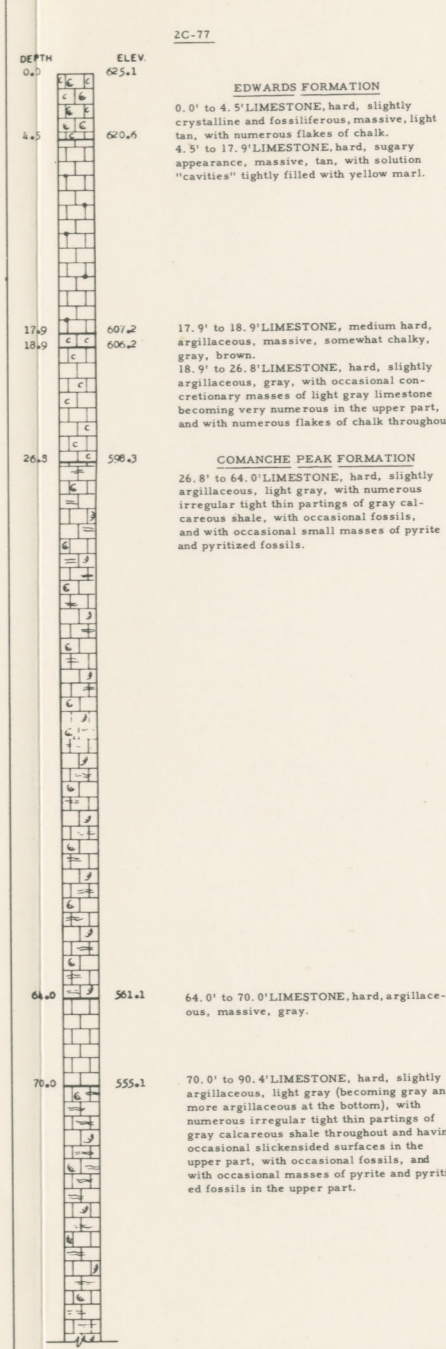
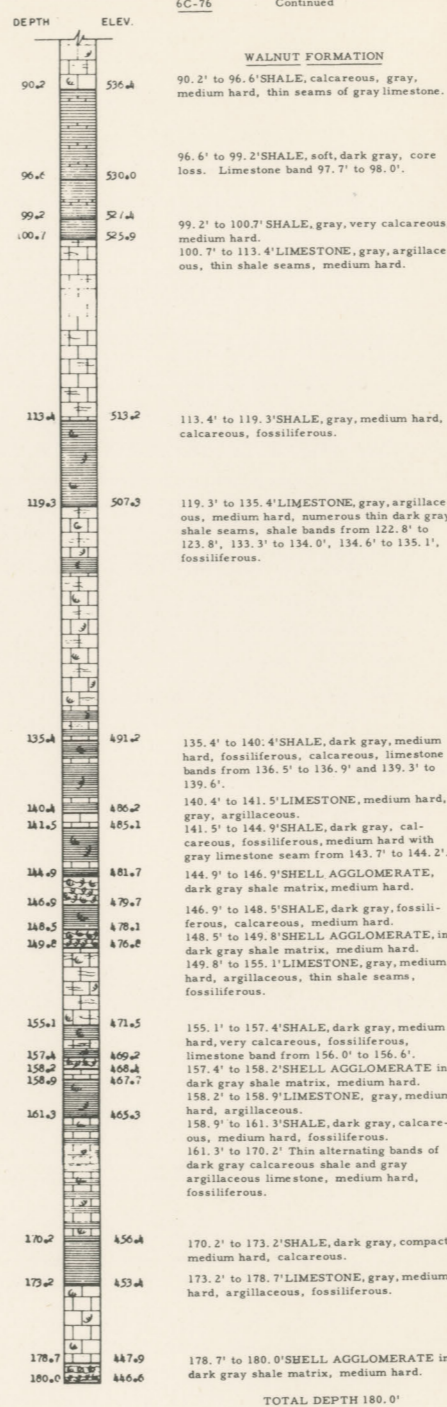
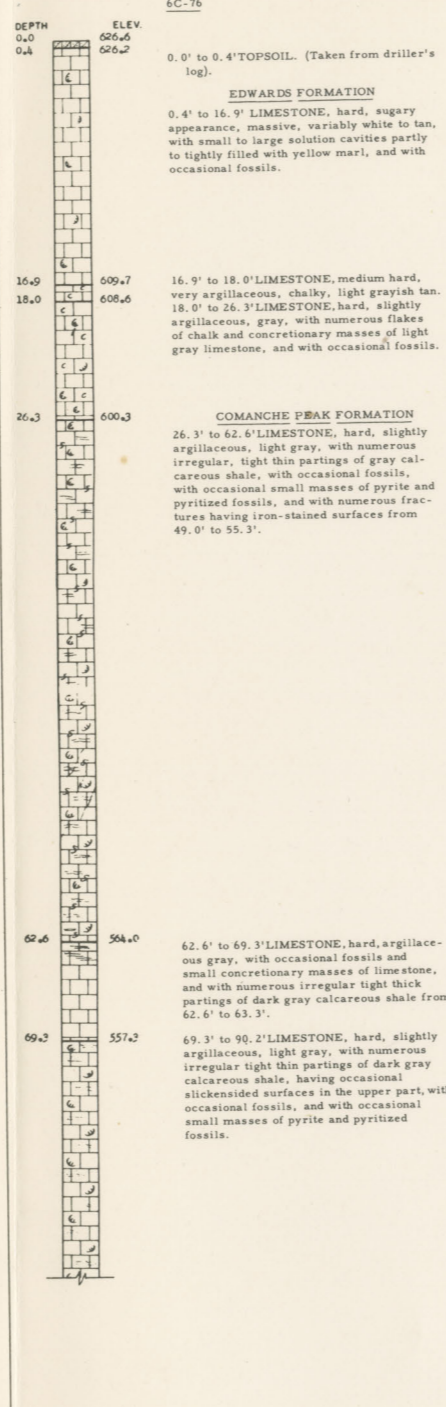
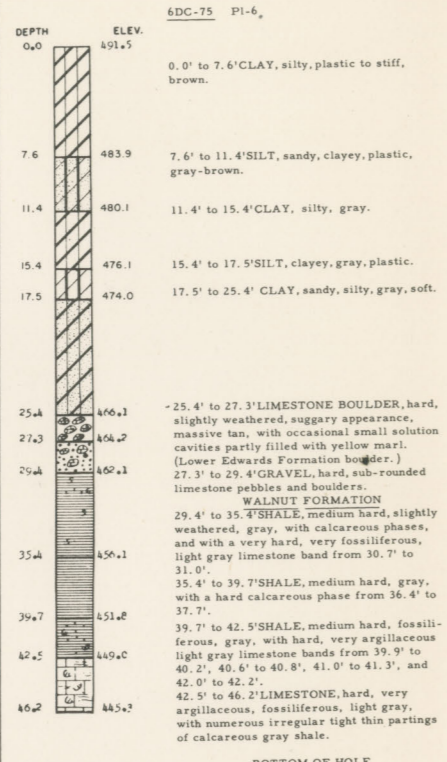
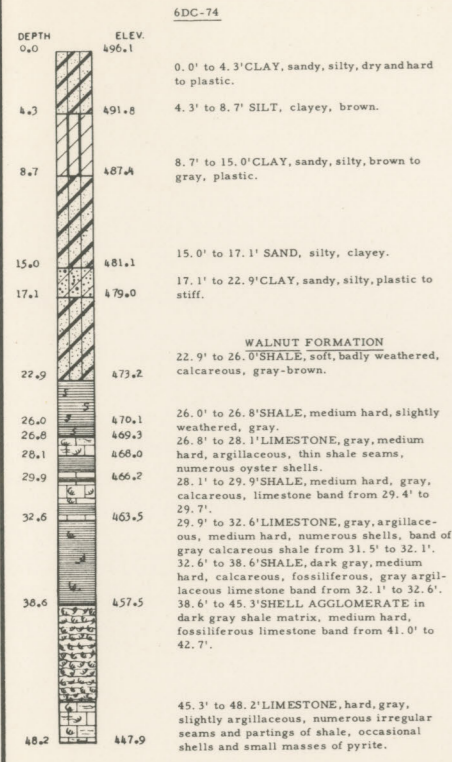


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 BELTON RESERVOIR
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CORE BORINGS
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 DEFINITE PROJECT SITE

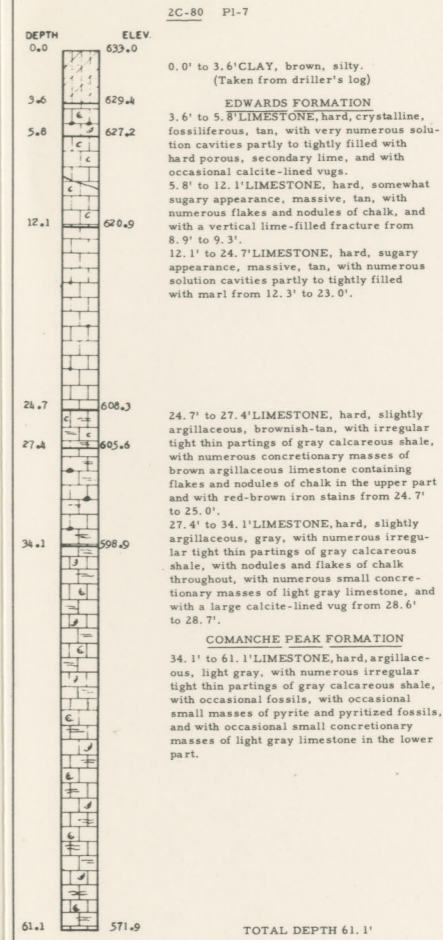
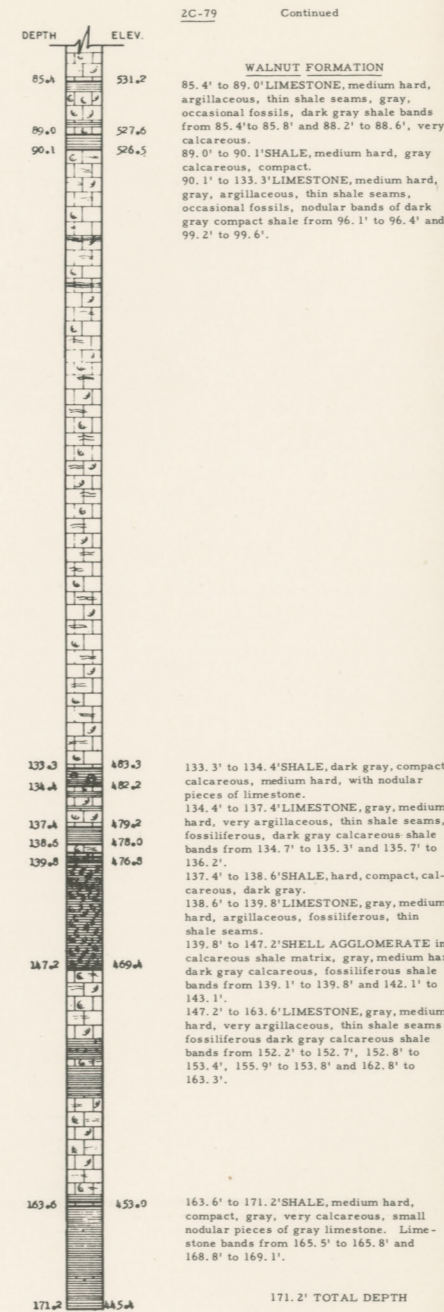
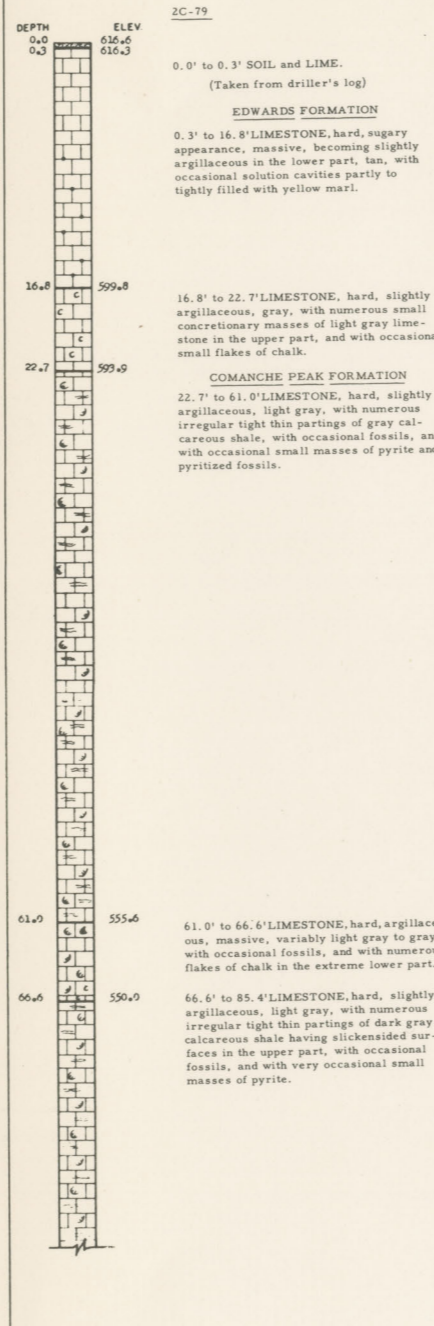
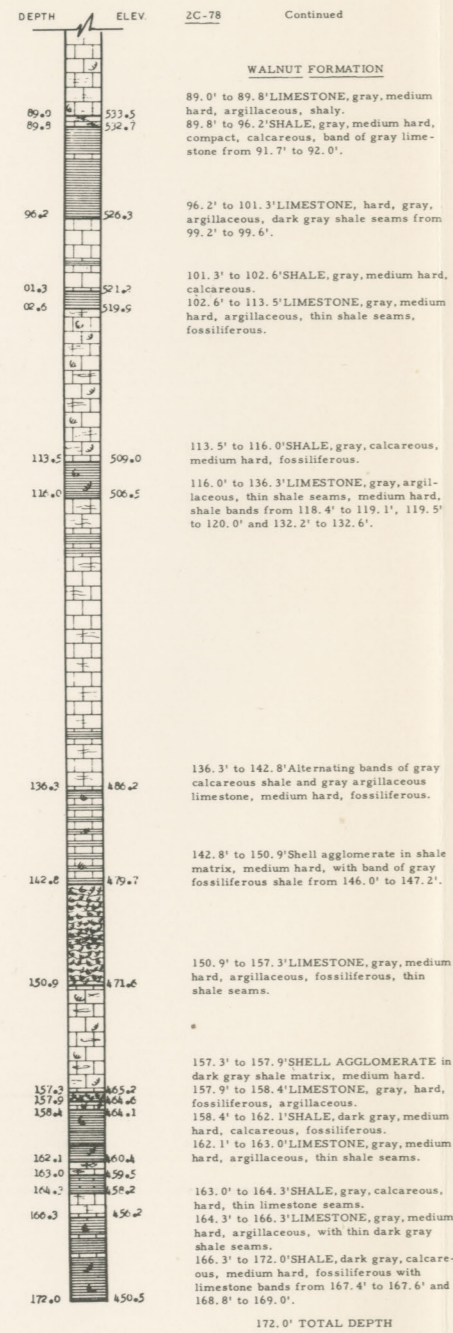
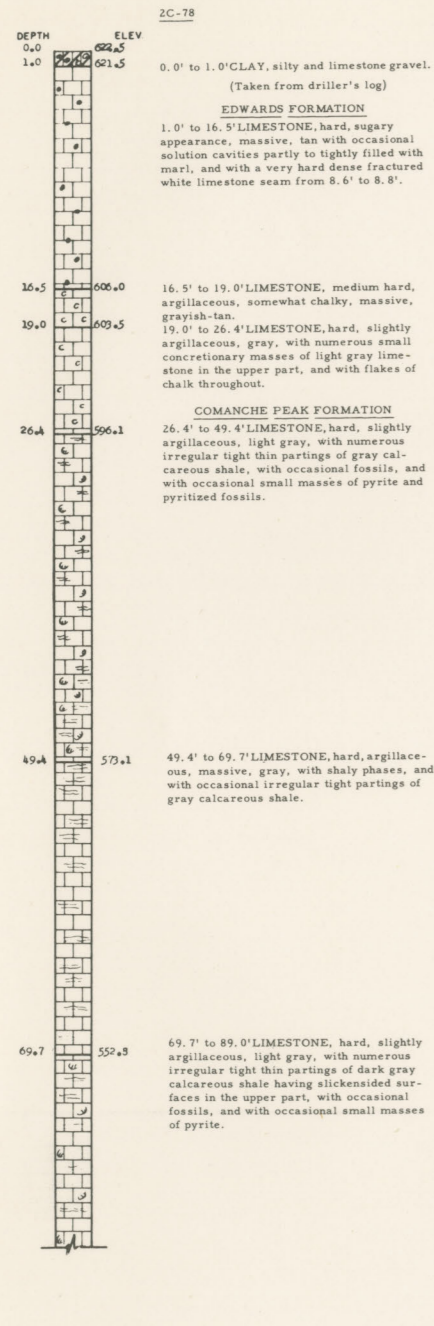
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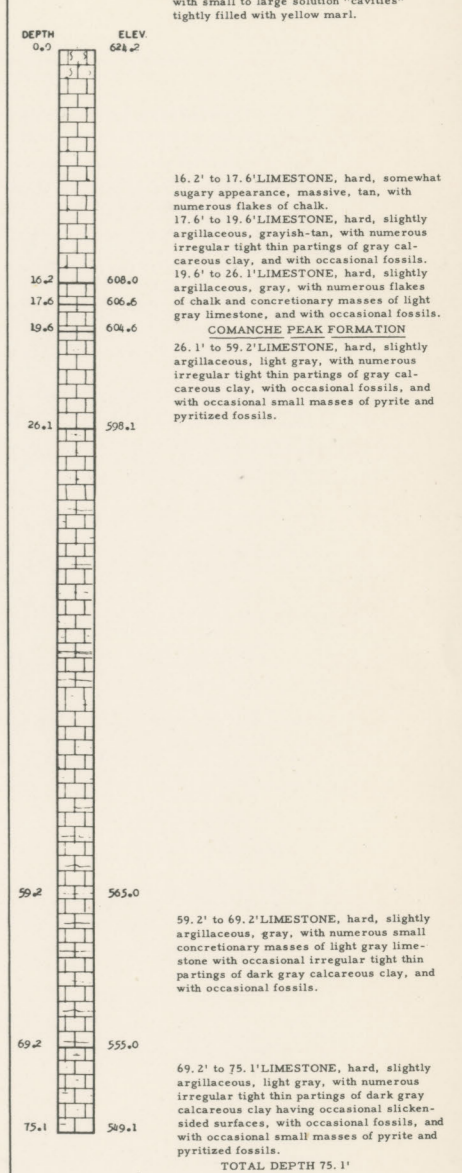
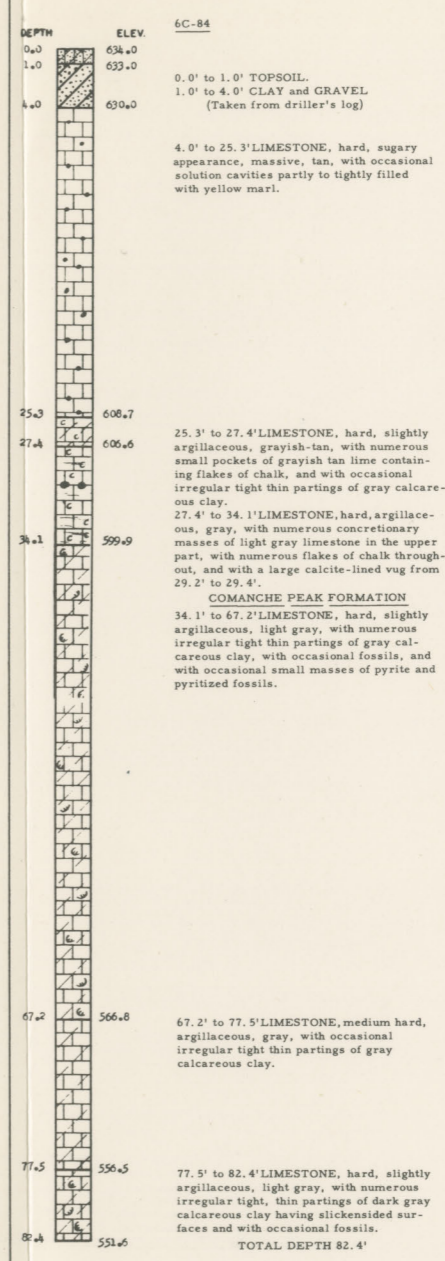
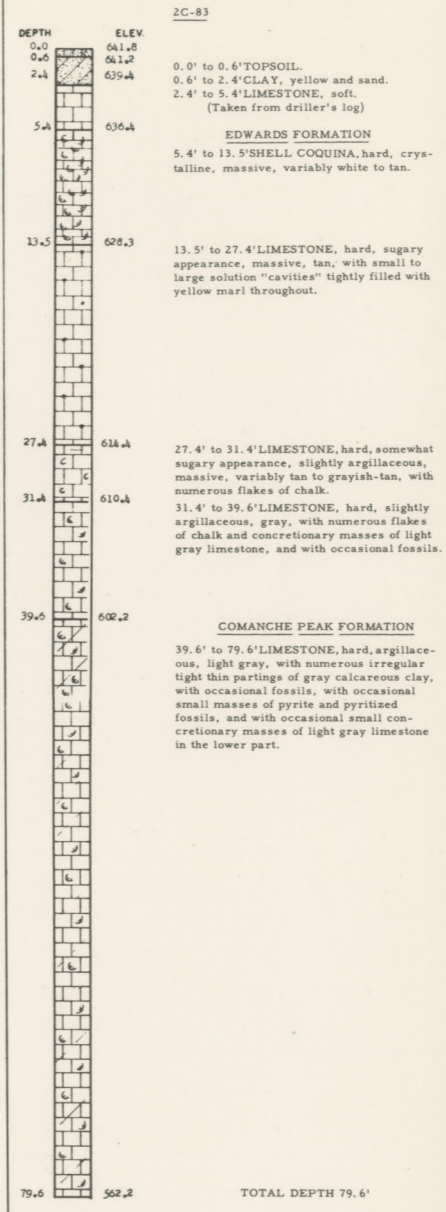
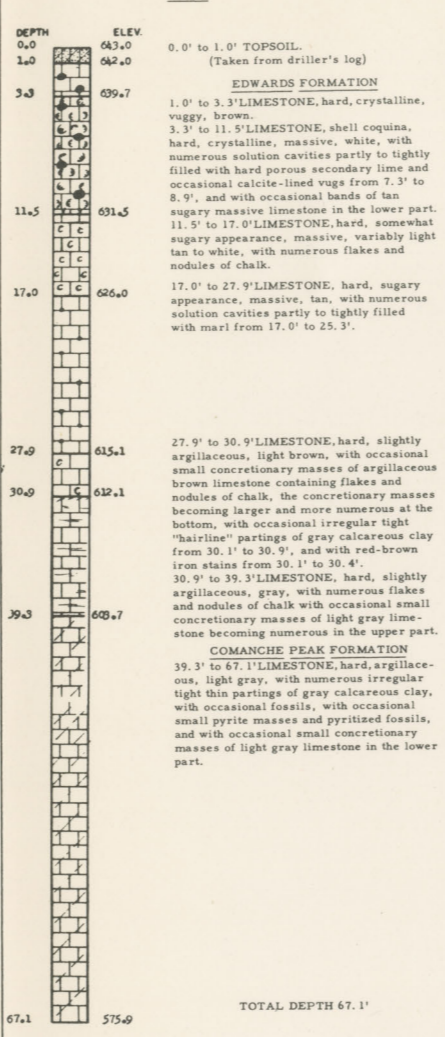
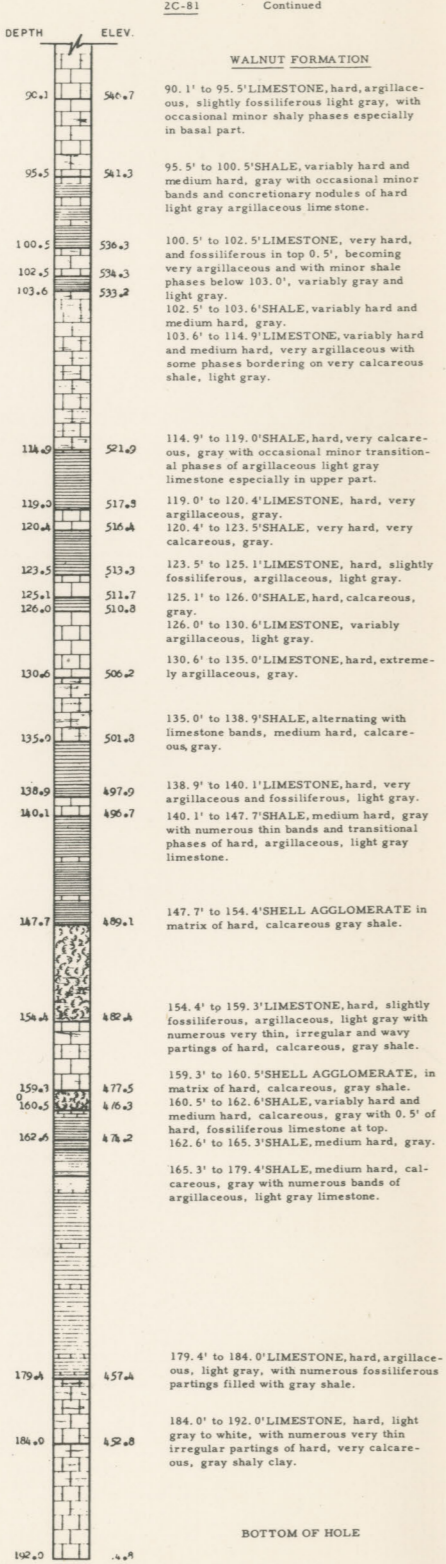
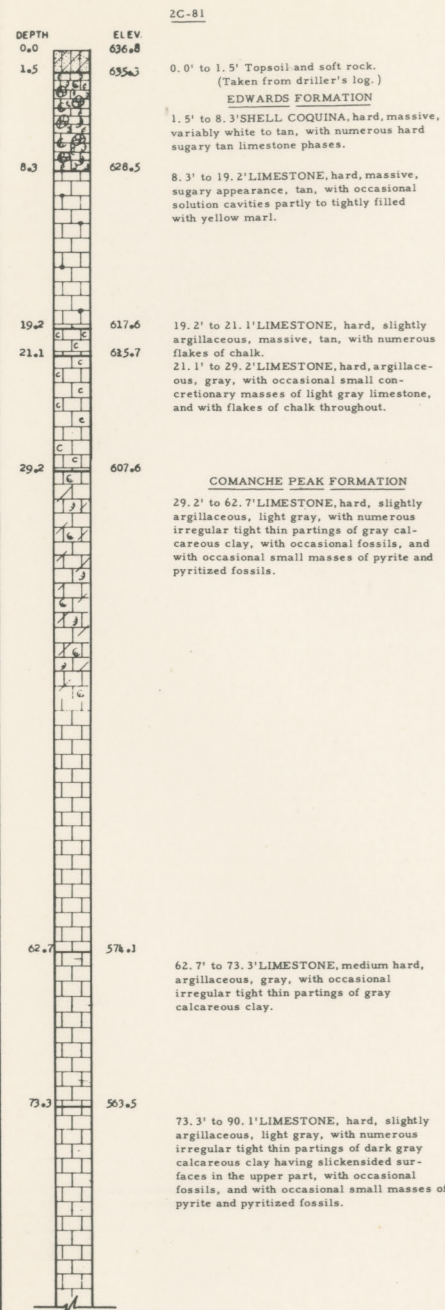
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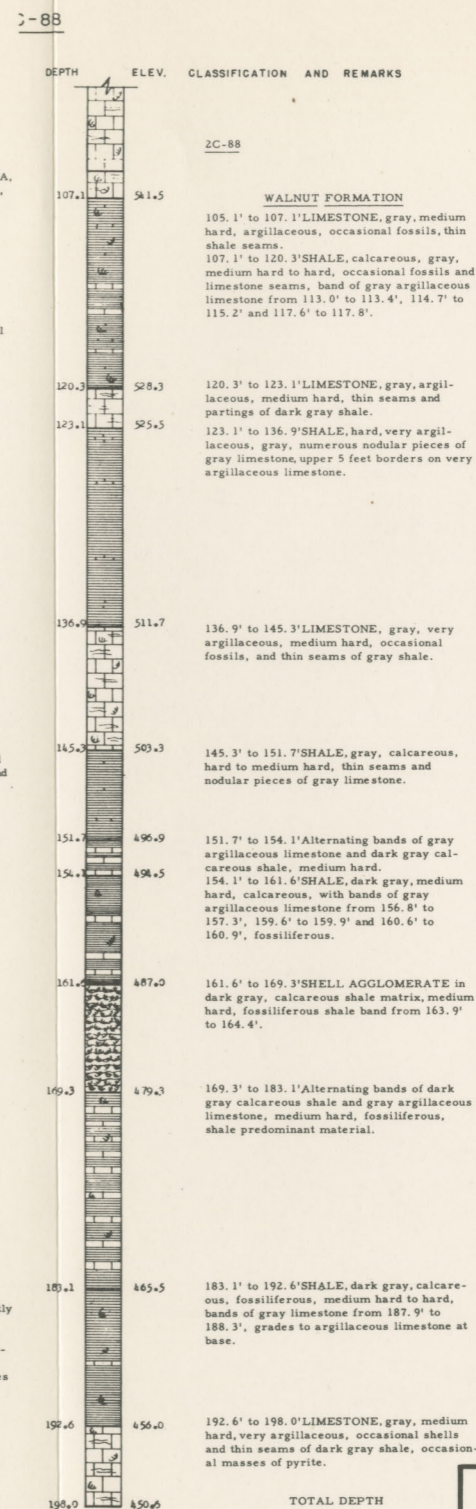
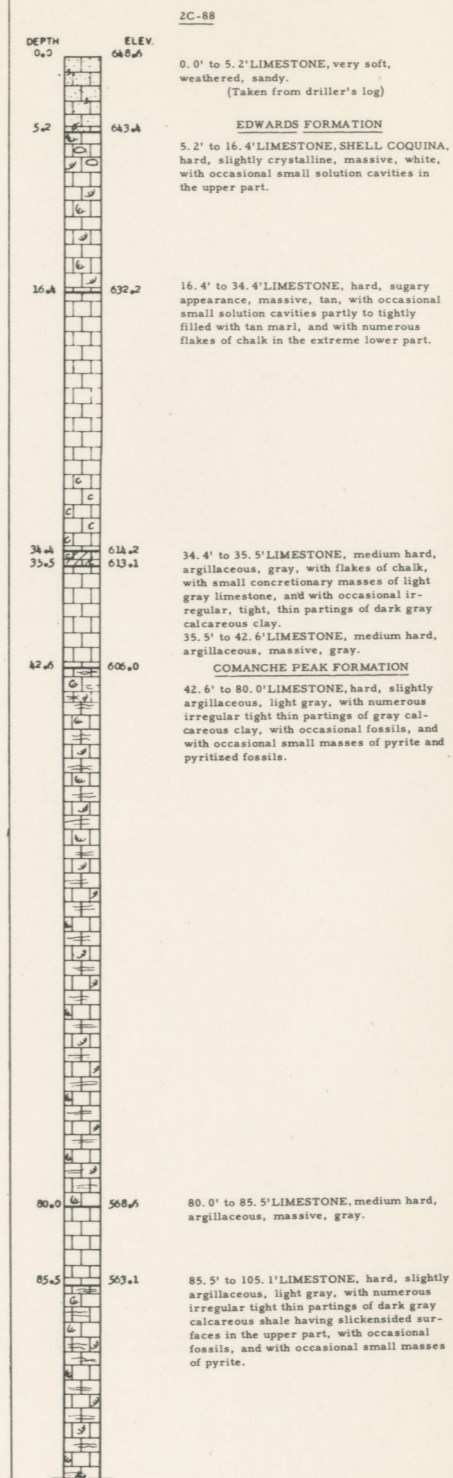
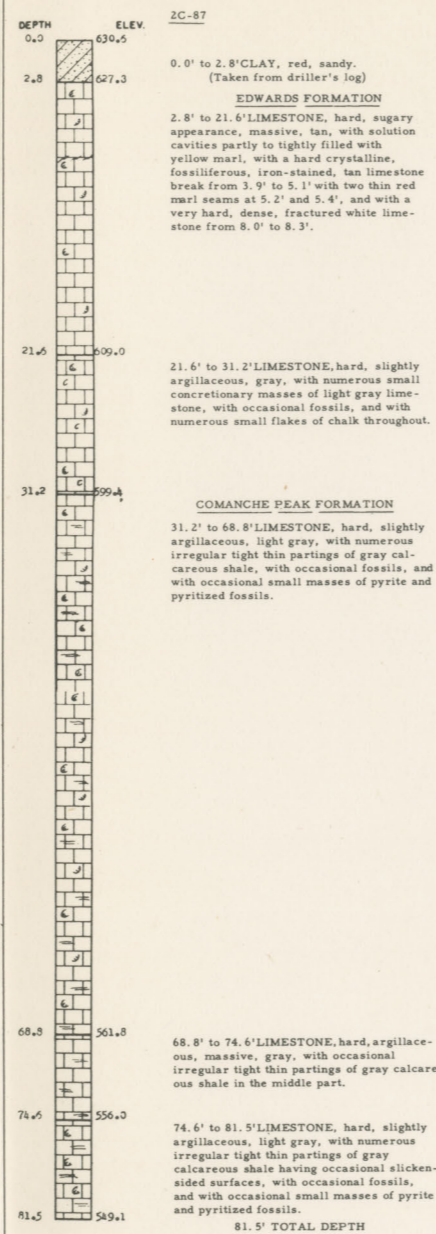
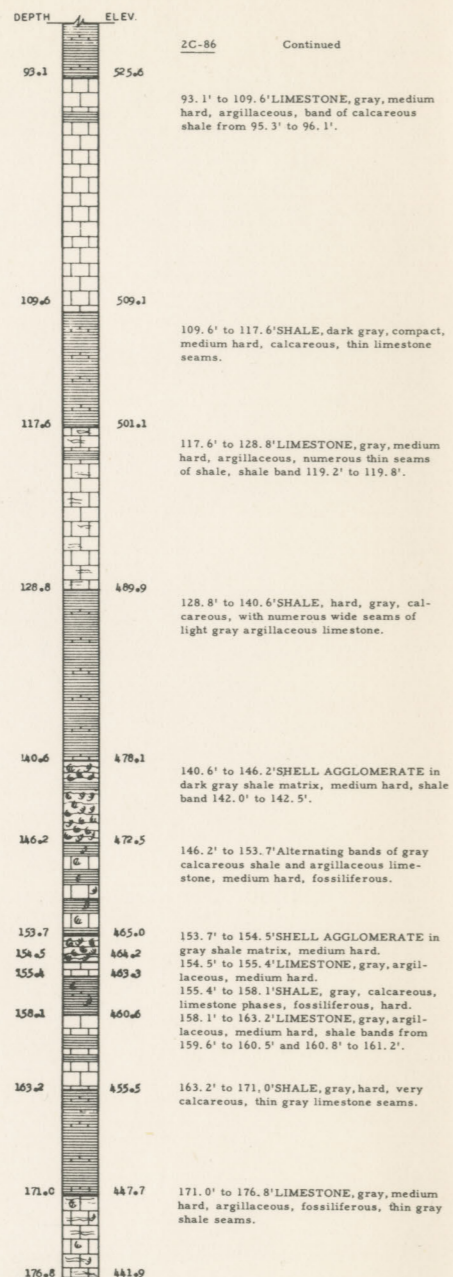
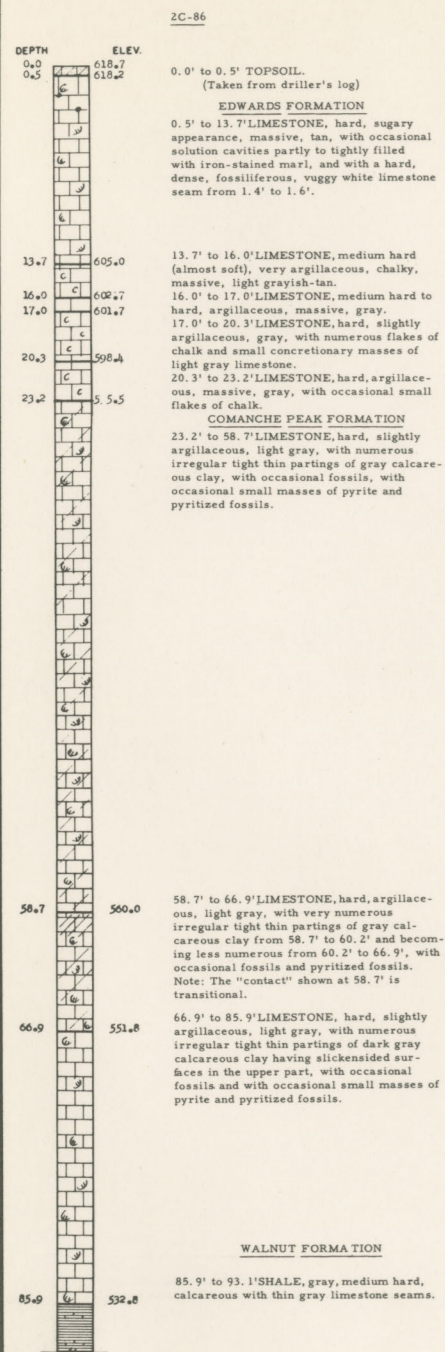
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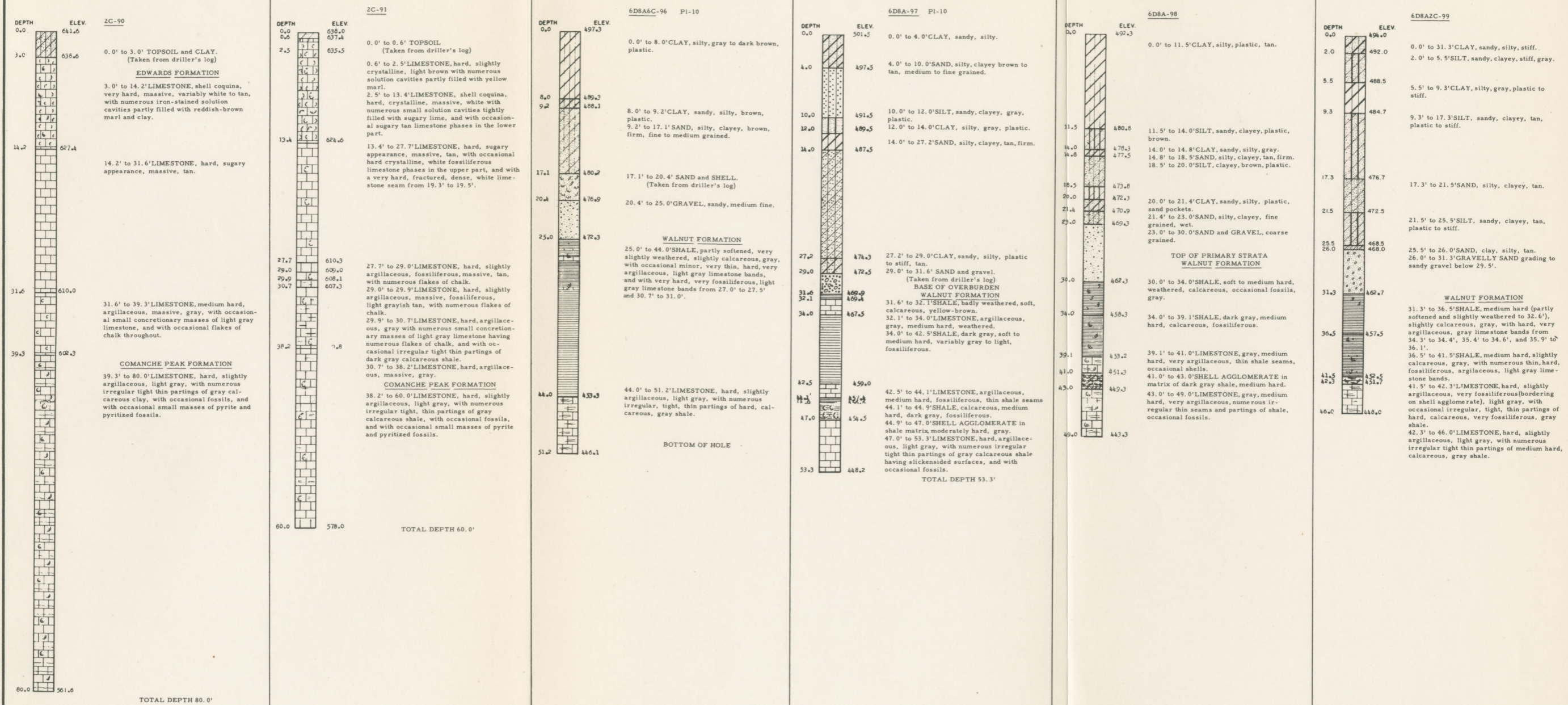


BRAZOS RIVER BASIN, TEXAS
 BELTON RESERVOIR
 LEON RIVER, TEXAS
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GALVESTON DISTRICT, GALVESTON, TEXAS DEC. 1948

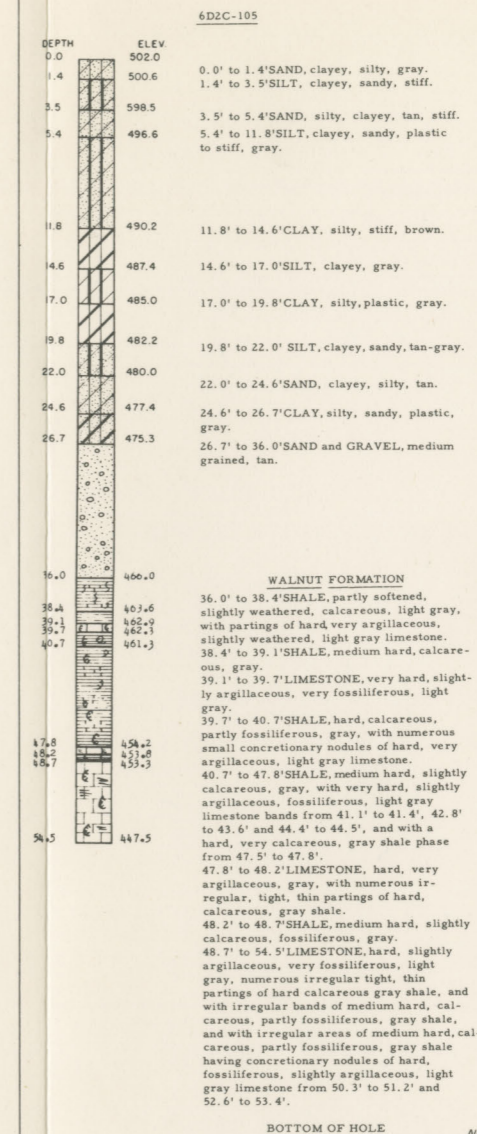
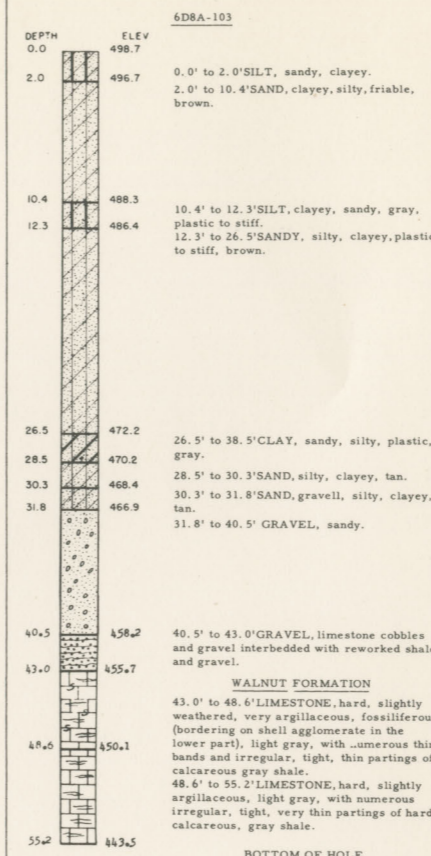
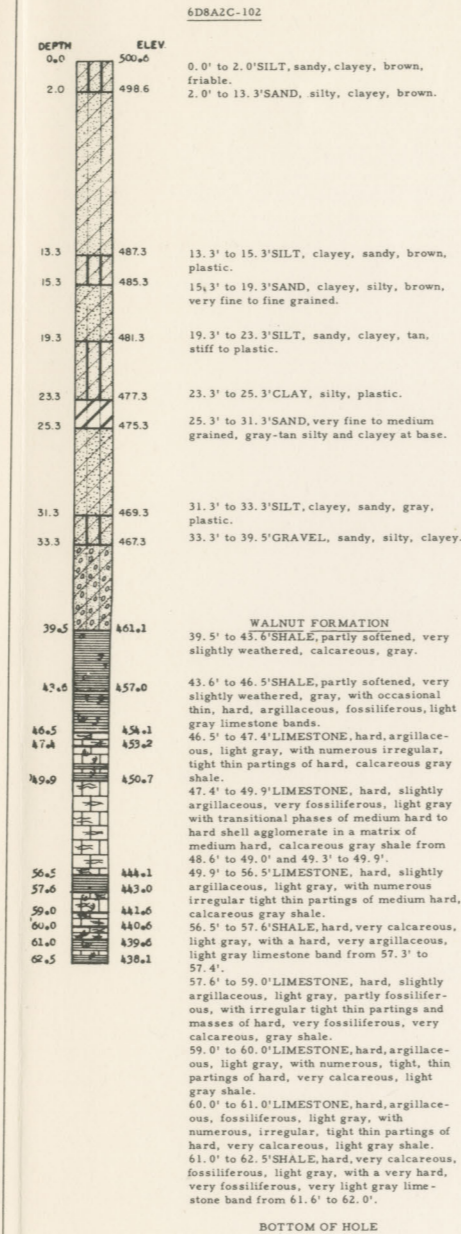
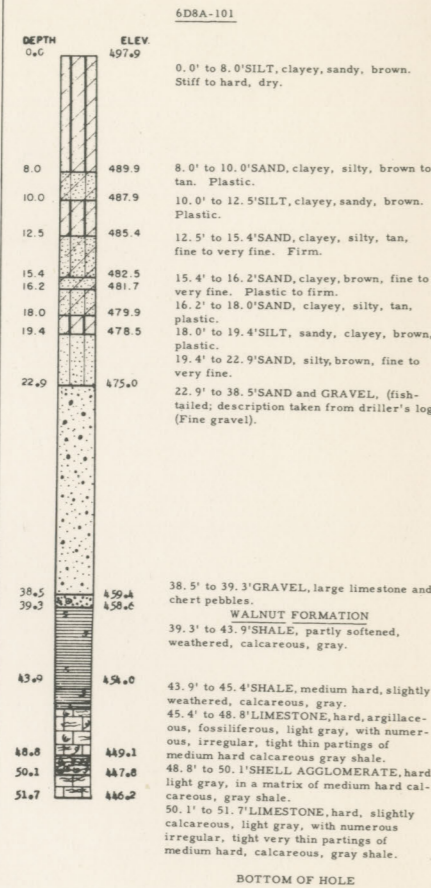
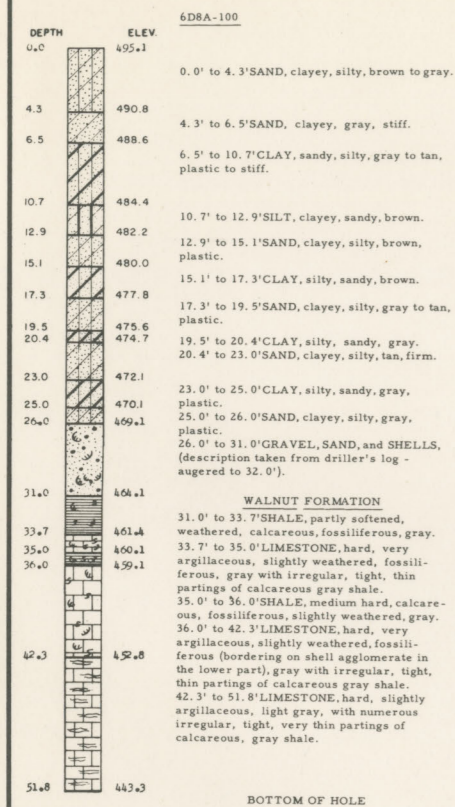
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 TO ACCOMPANY DEFINITE PROJECT REPORT
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Colligan: Geology of Belton Reservoir Area, Leon River, Bell County, Texas



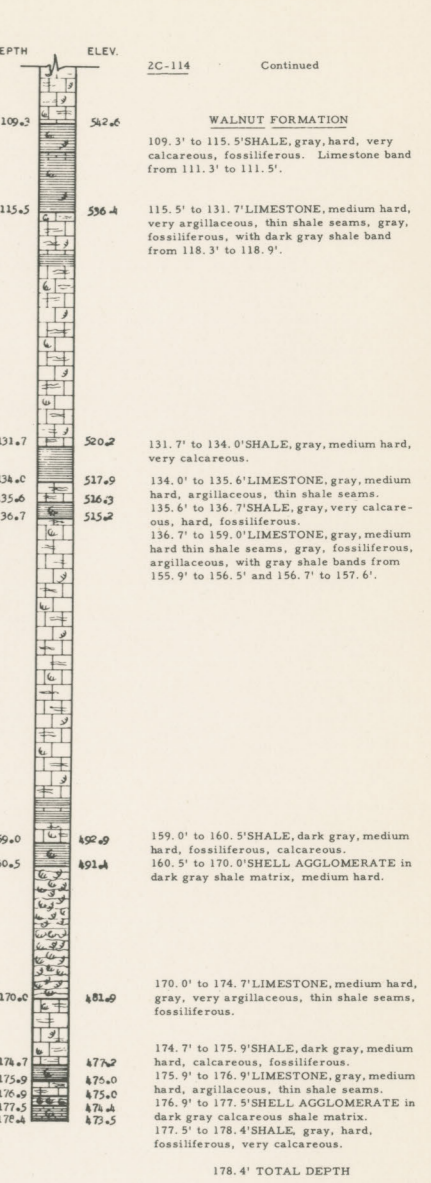
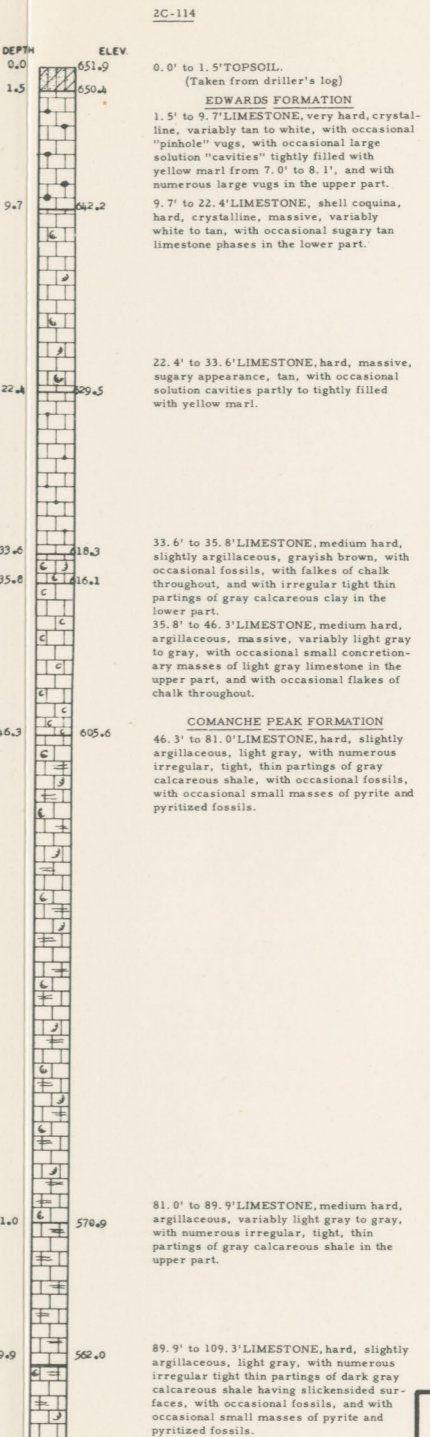
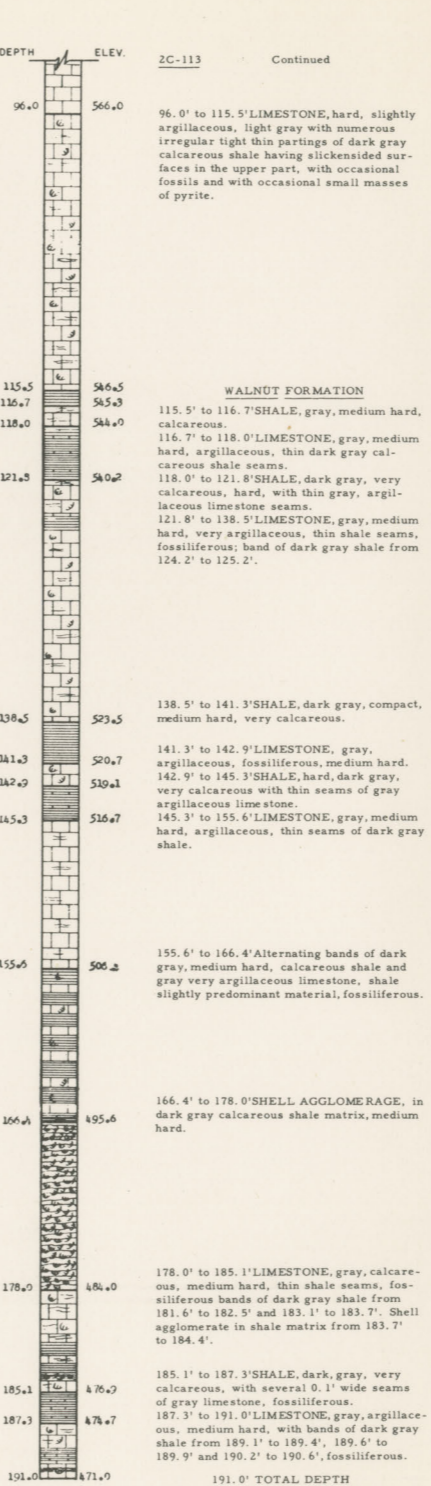
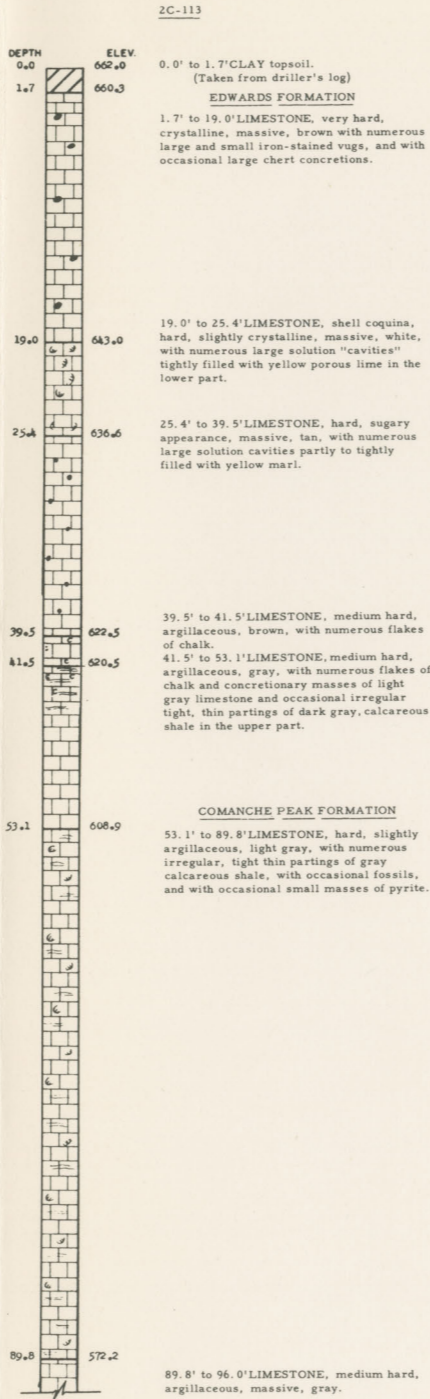
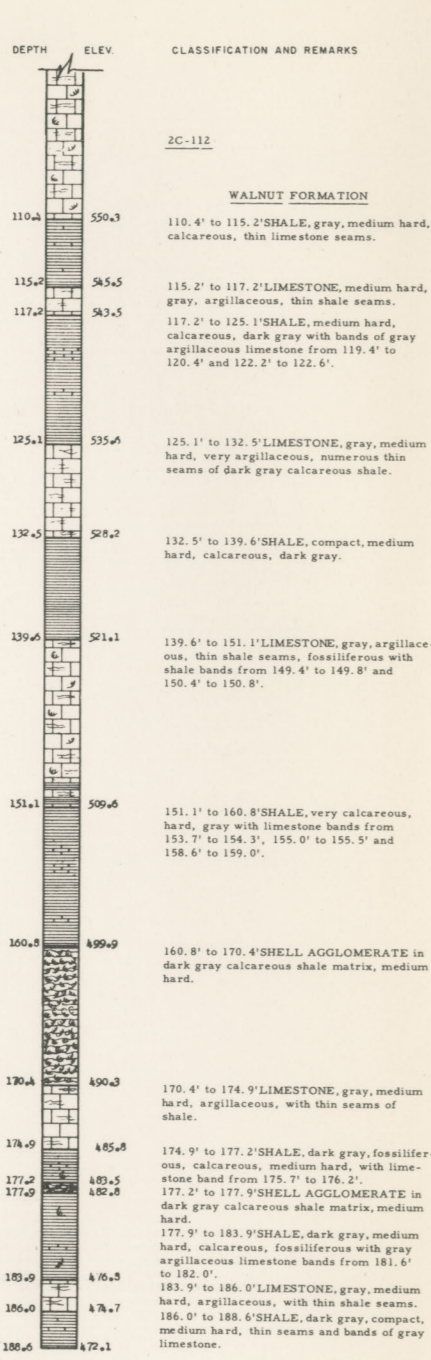
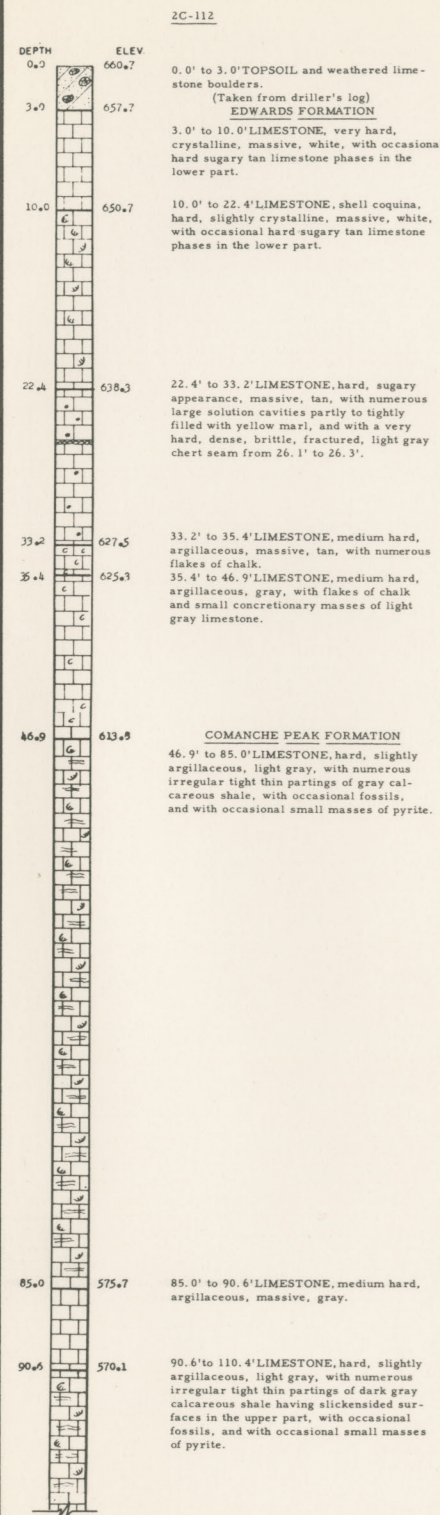
NOTE: Borings Nos. 6D-106 to 6D-111 are shown in Appendix IV. 6D-104 is shown on Plate 17 Appendix III.

BRAZOS RIVER BASIN, TEXAS
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FILE: BRAZ. 503-203 APPENDIX III PLATE II

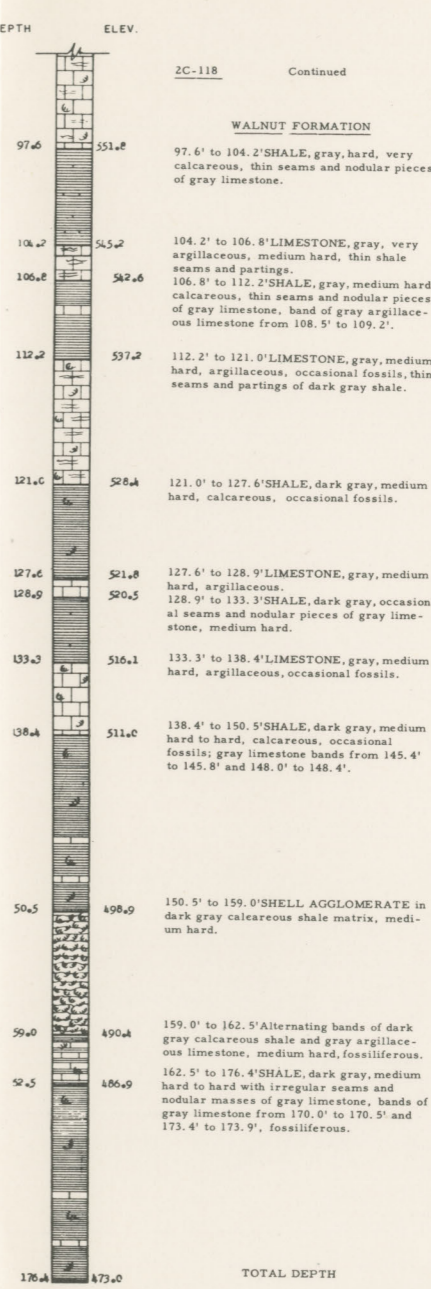
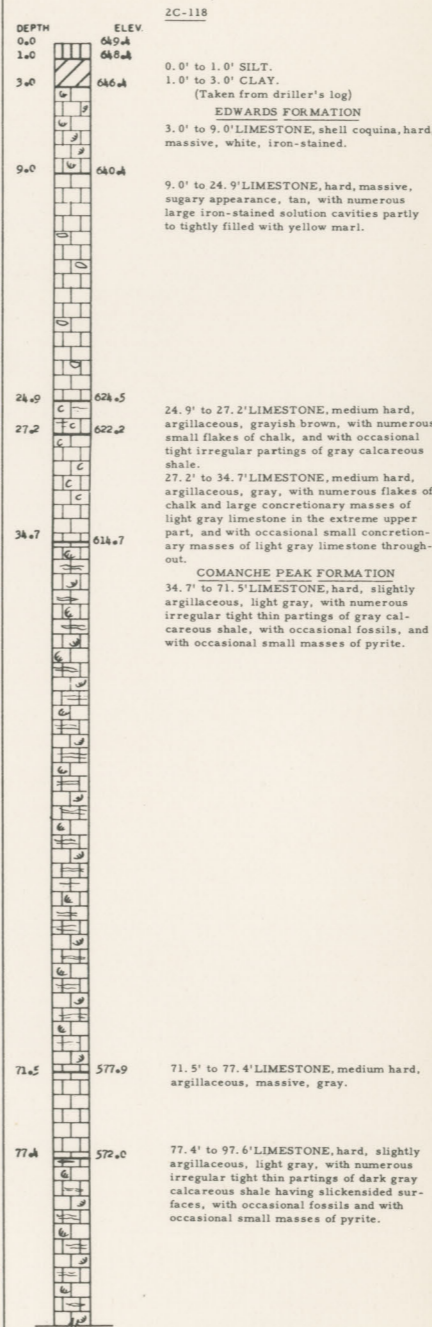
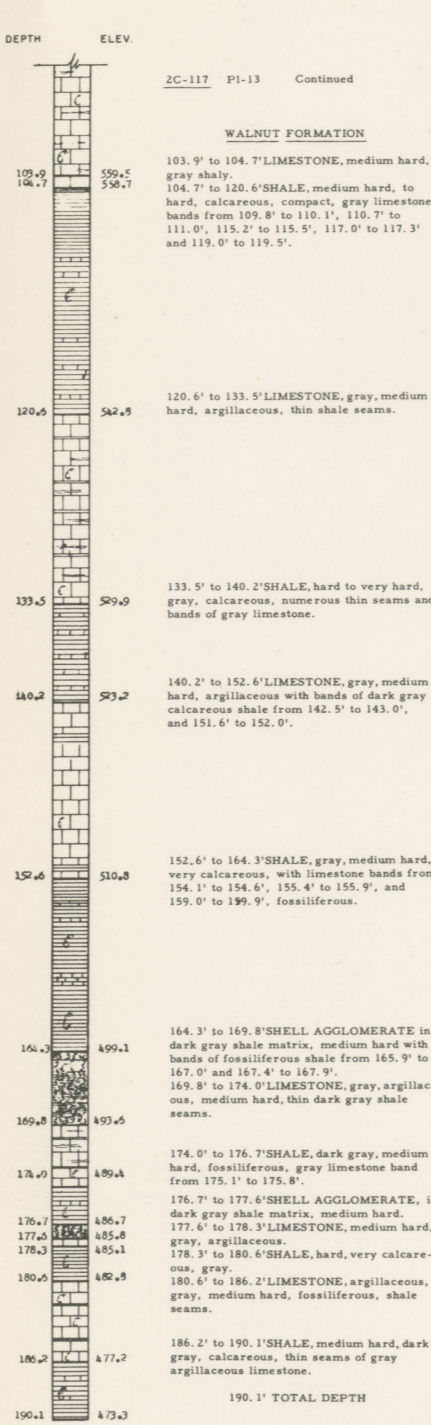
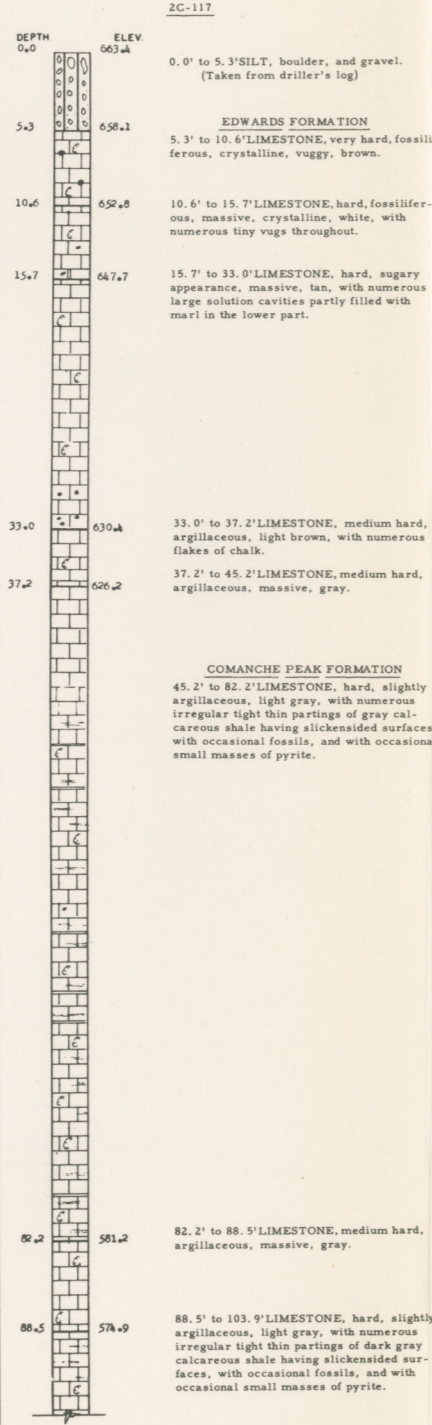
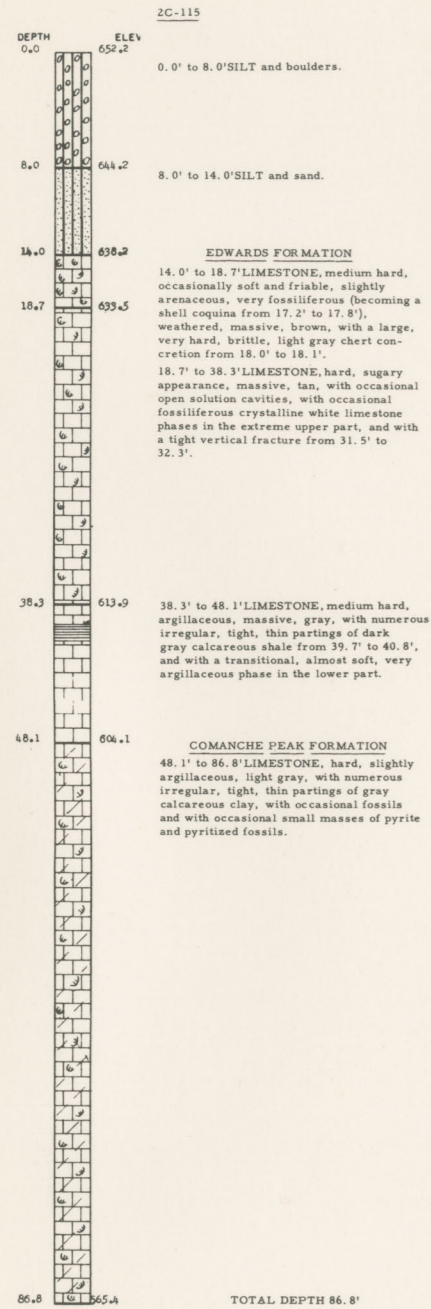


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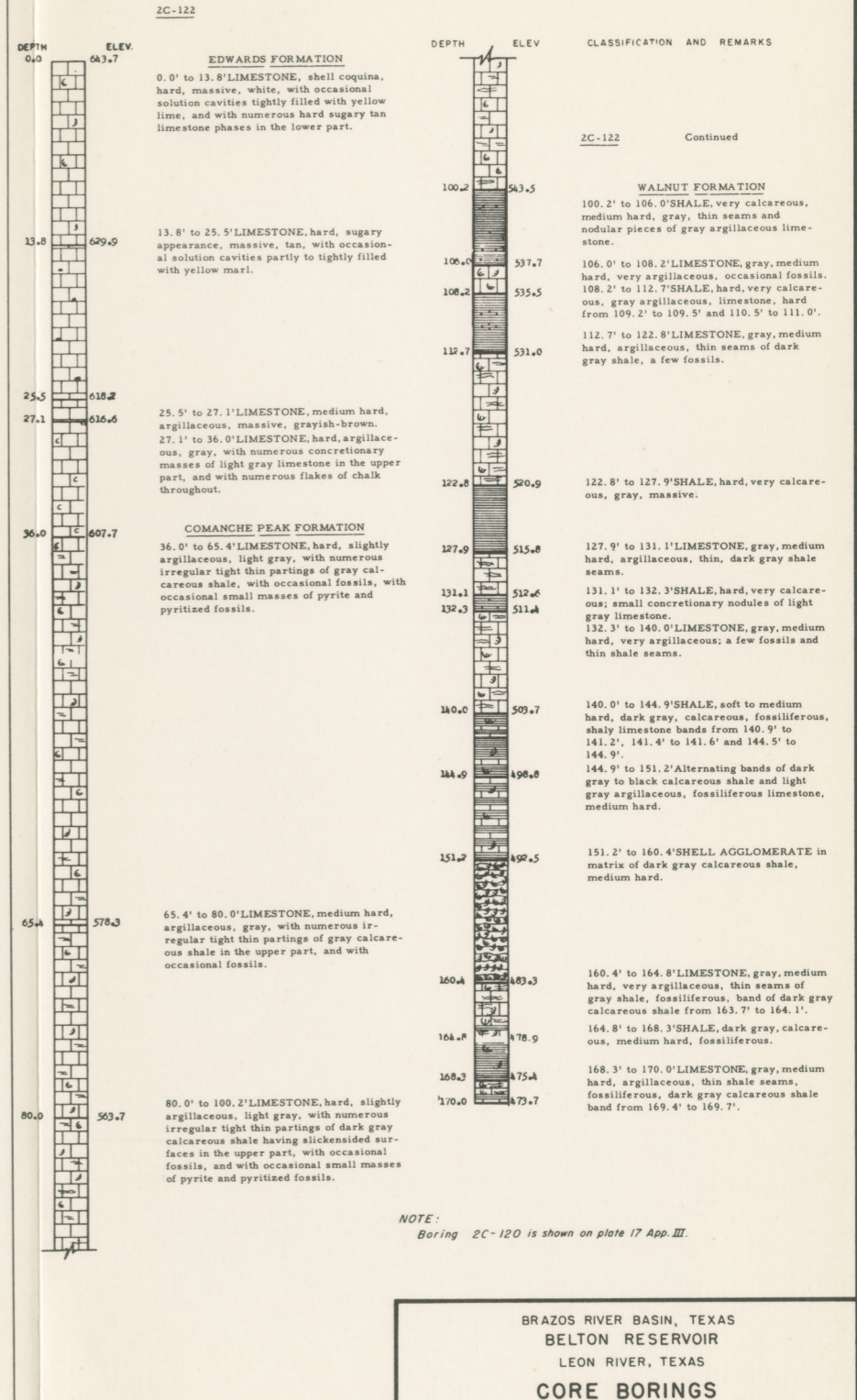
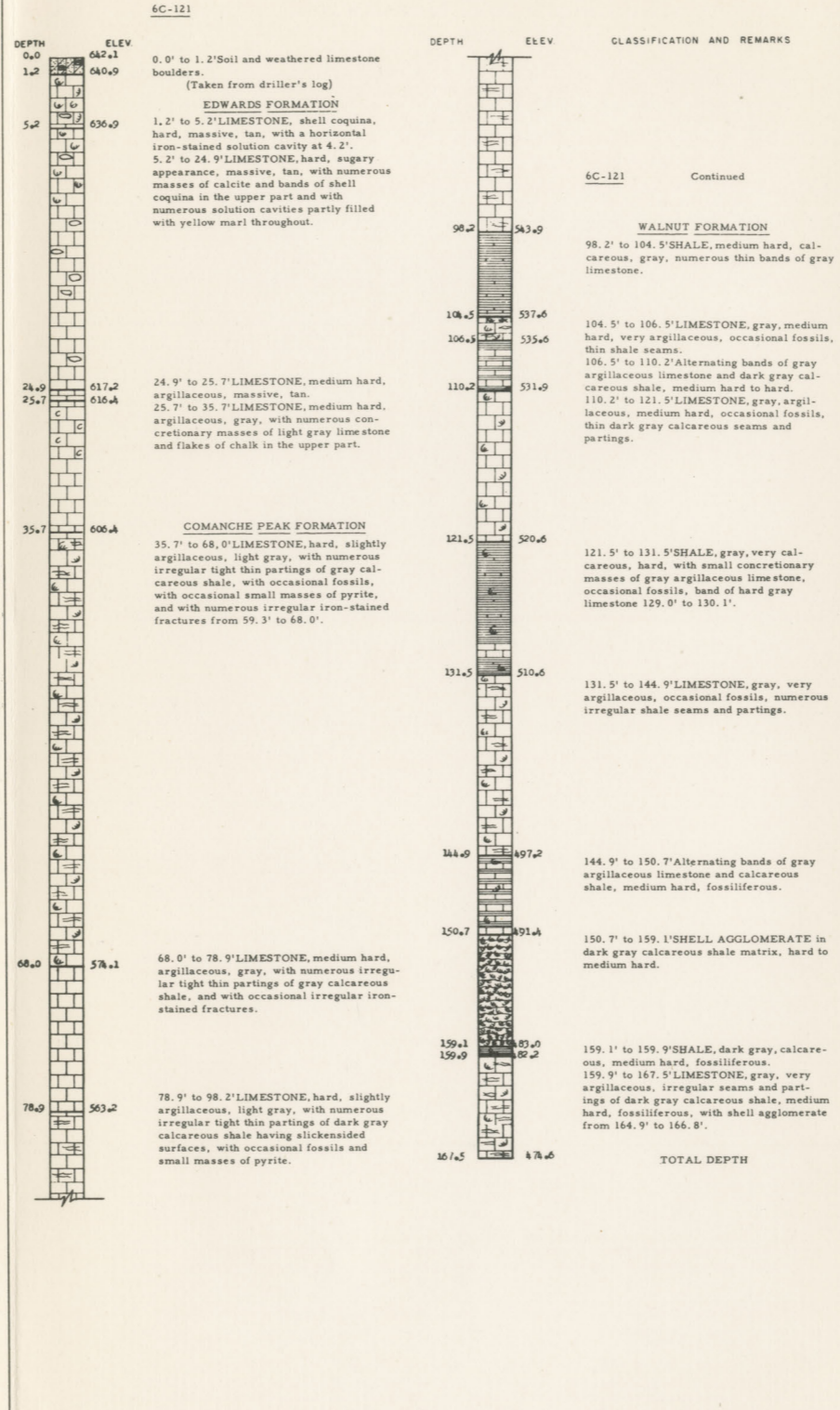
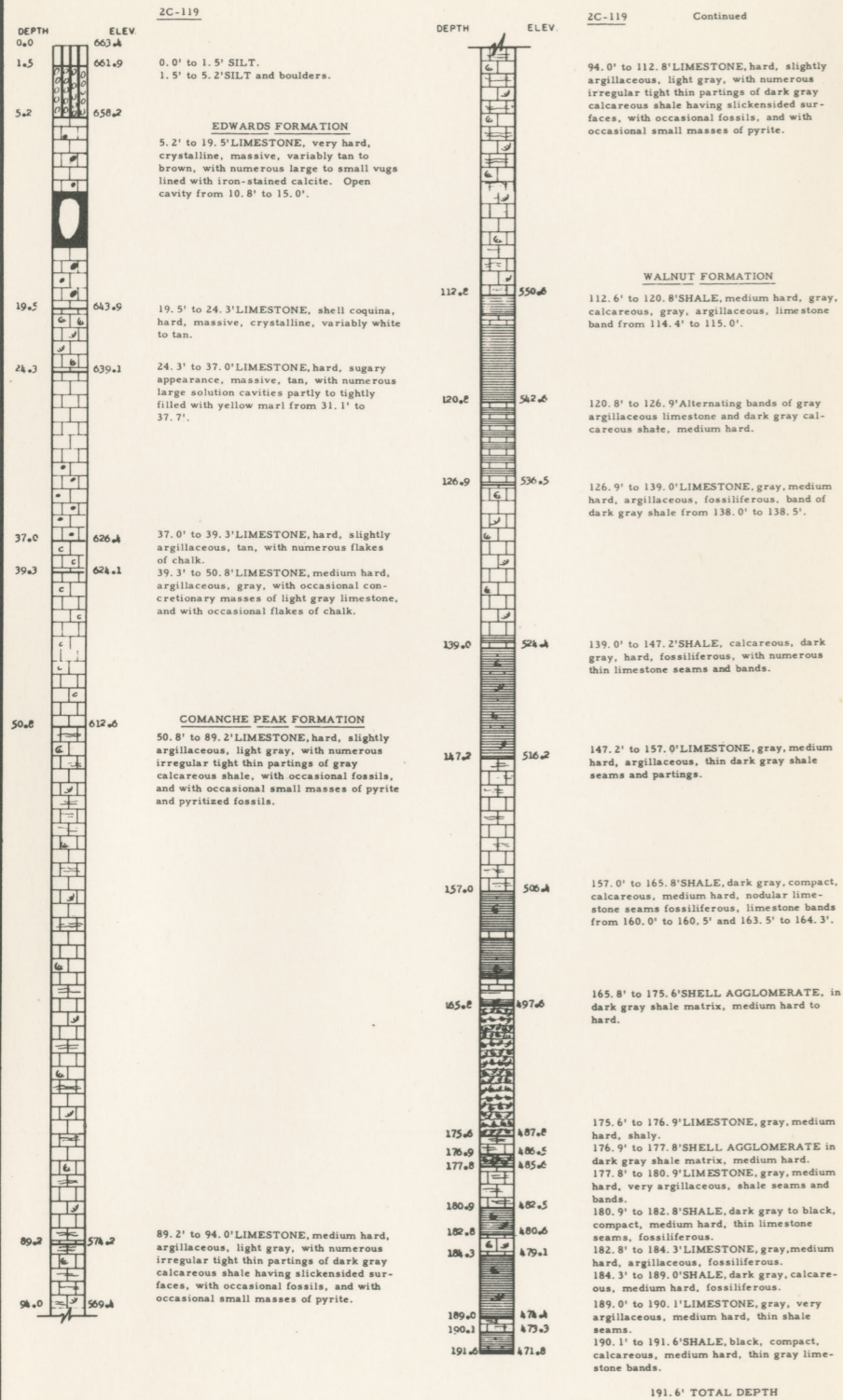
NOTE:
Boring 2C-116 is shown on Plate 17 Appendix III.

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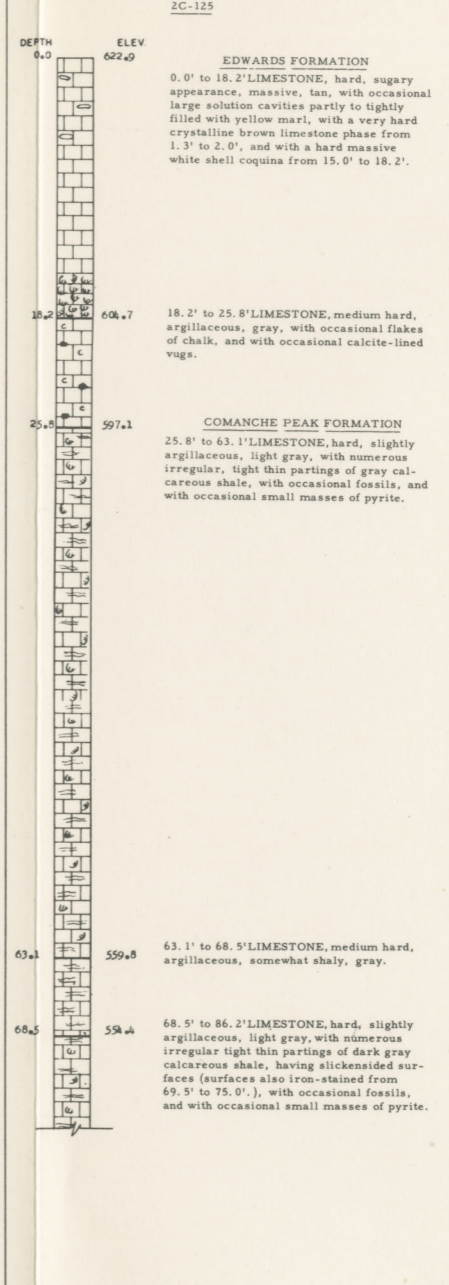
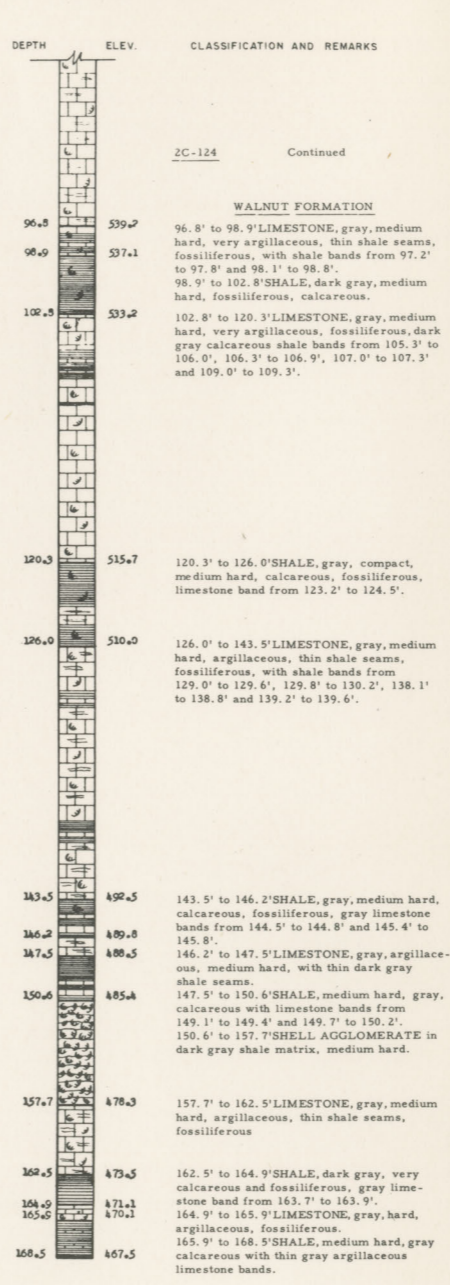
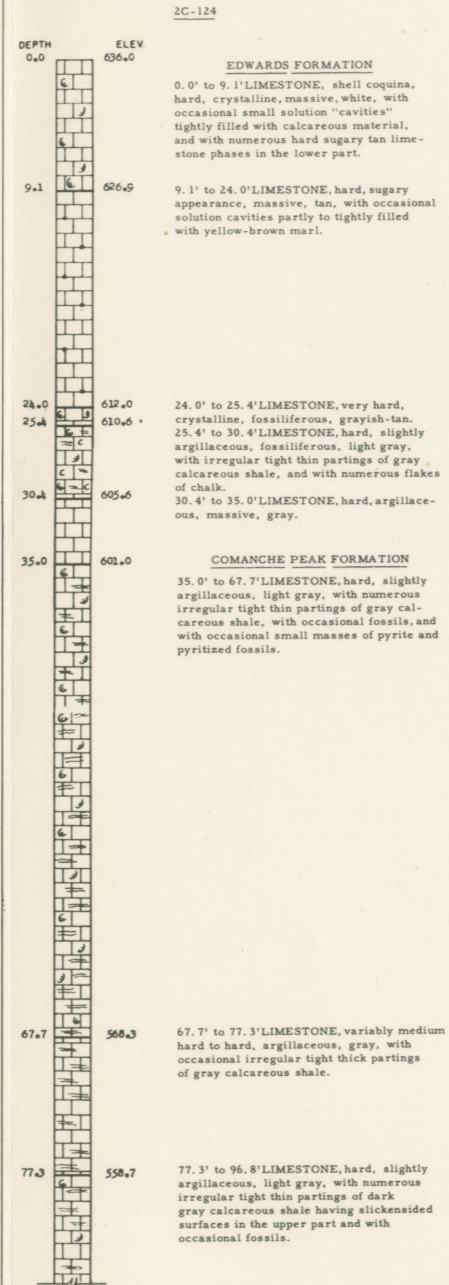
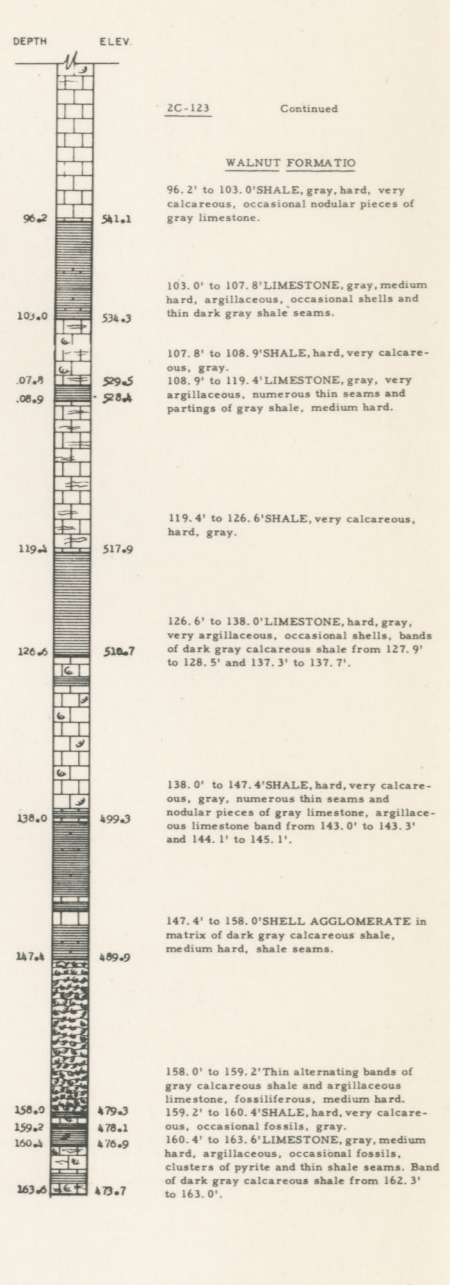
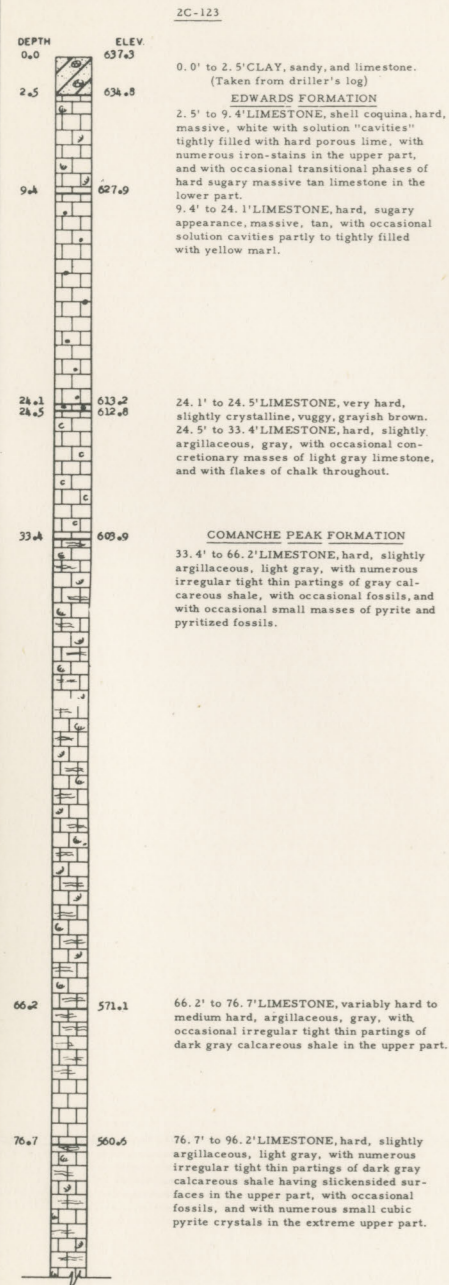
TO ACCOMPANY DEFINITE PROJECT REPORT

FILE: BRAZ. 503-203 APPENDIX III PLATE 13

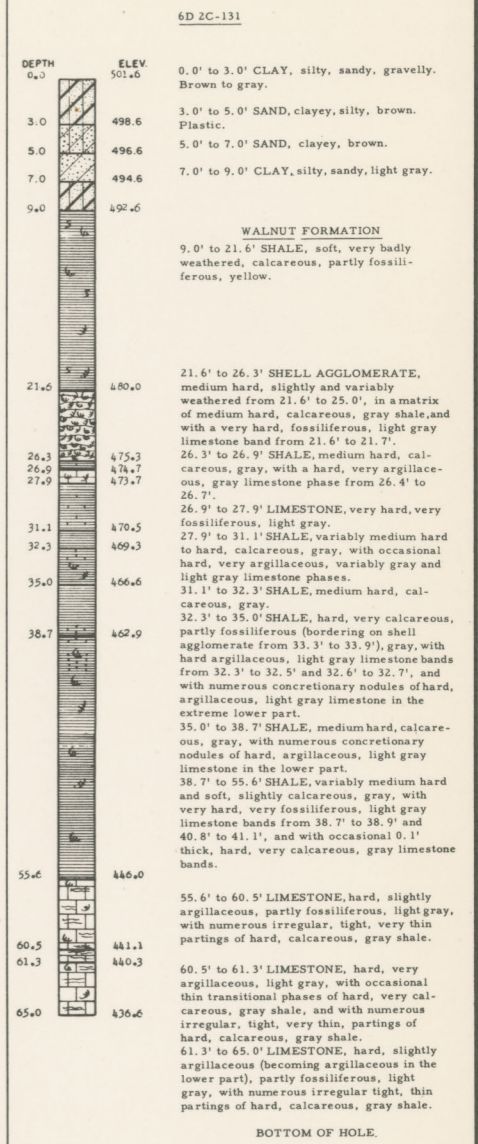
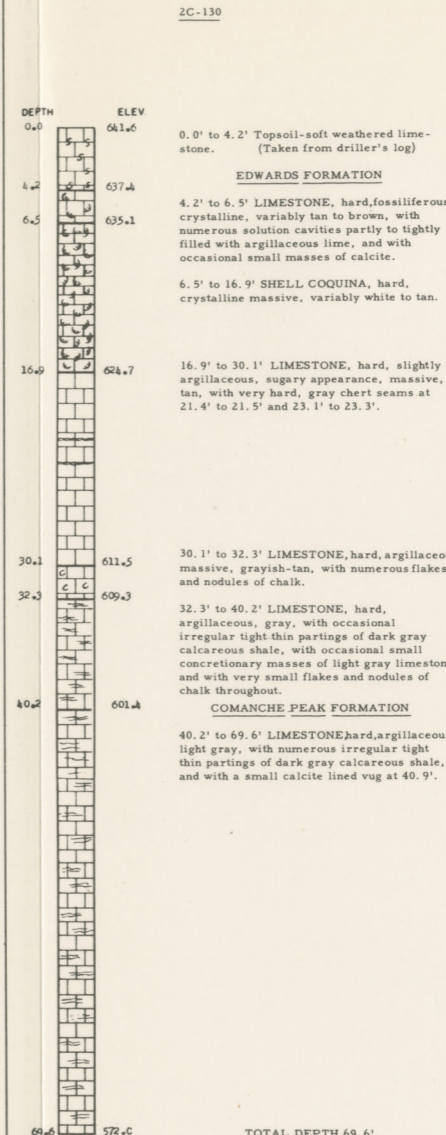
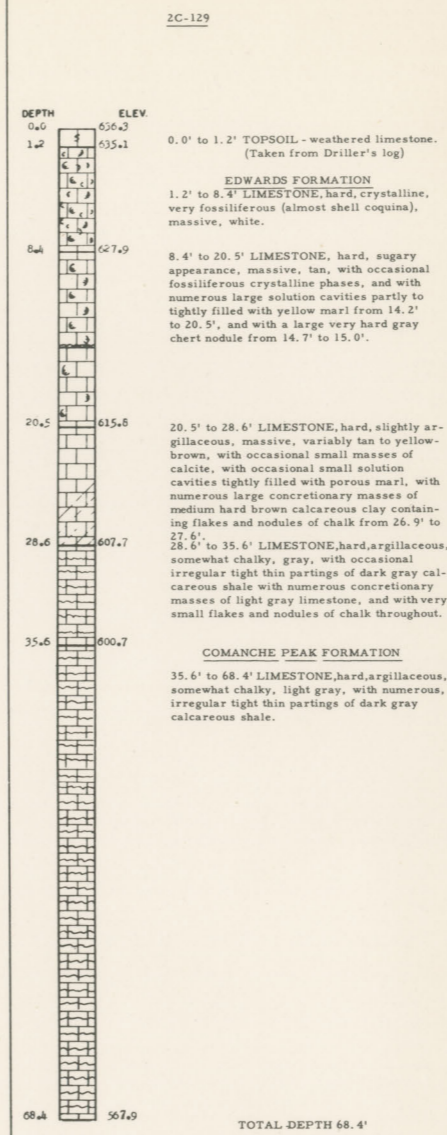
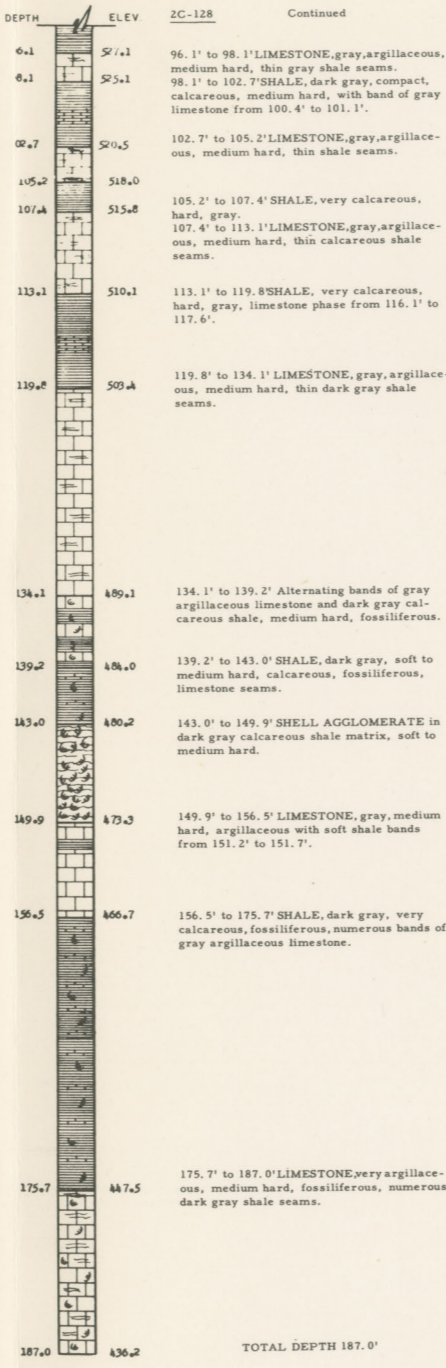
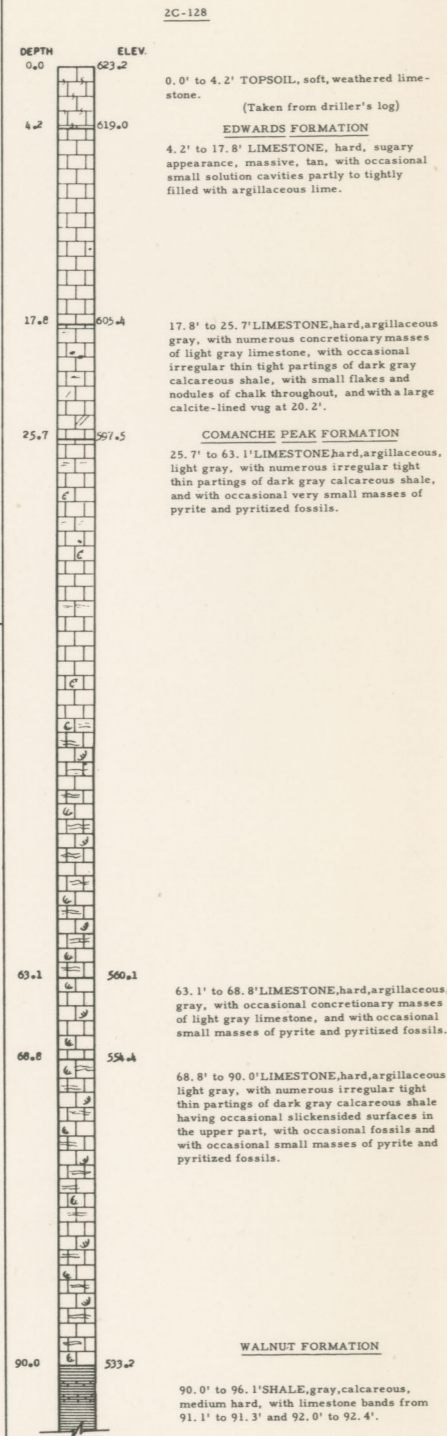
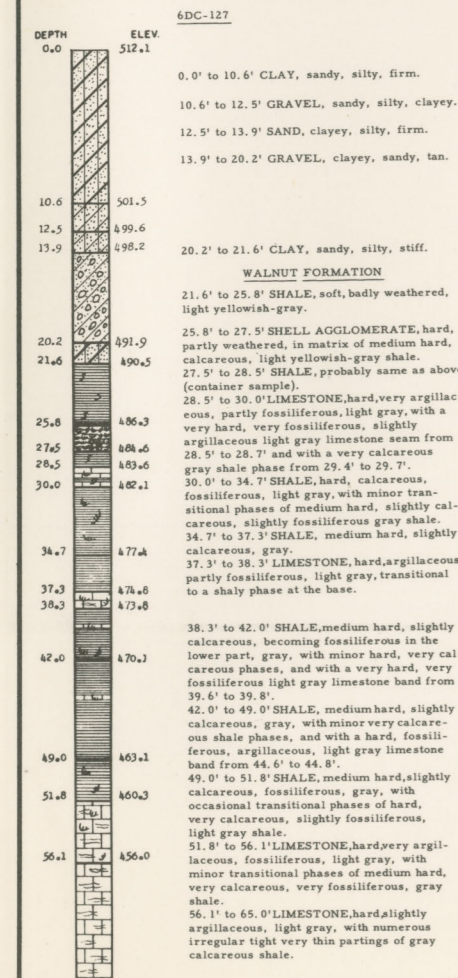
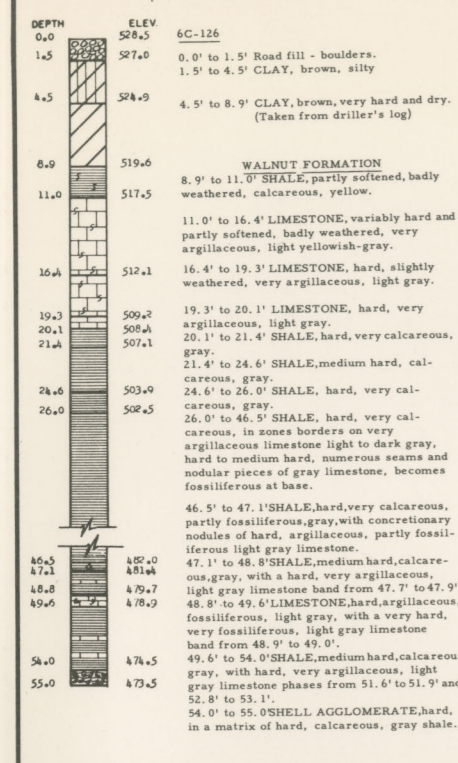


NOTE:
Boring 2C-120 is shown on plate 17 App. III.

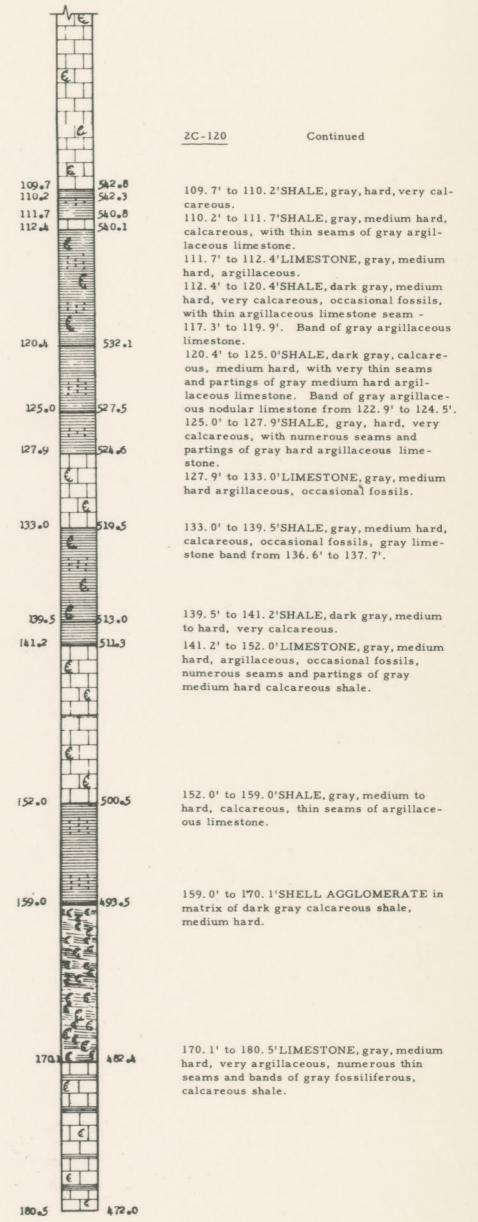
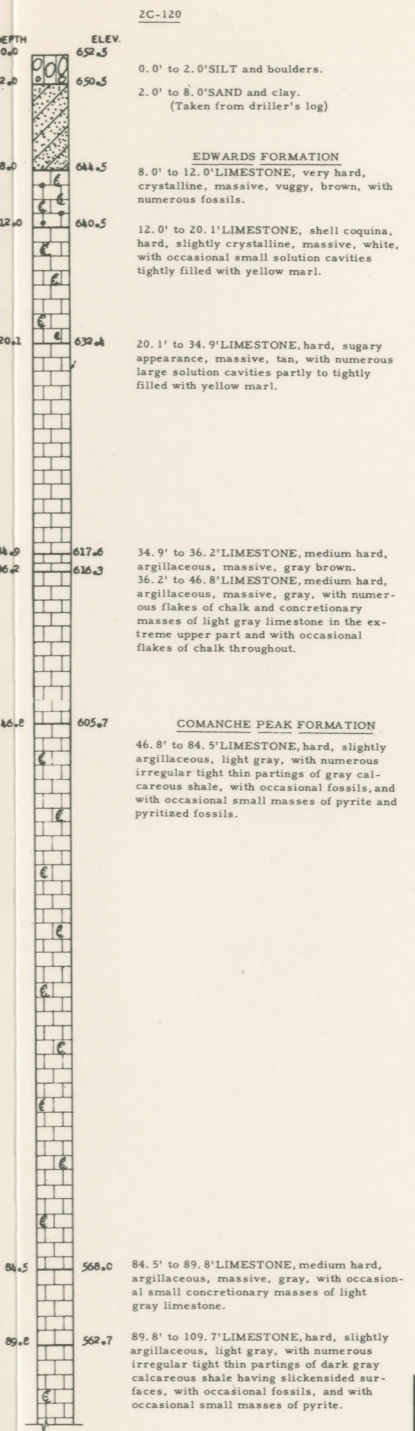
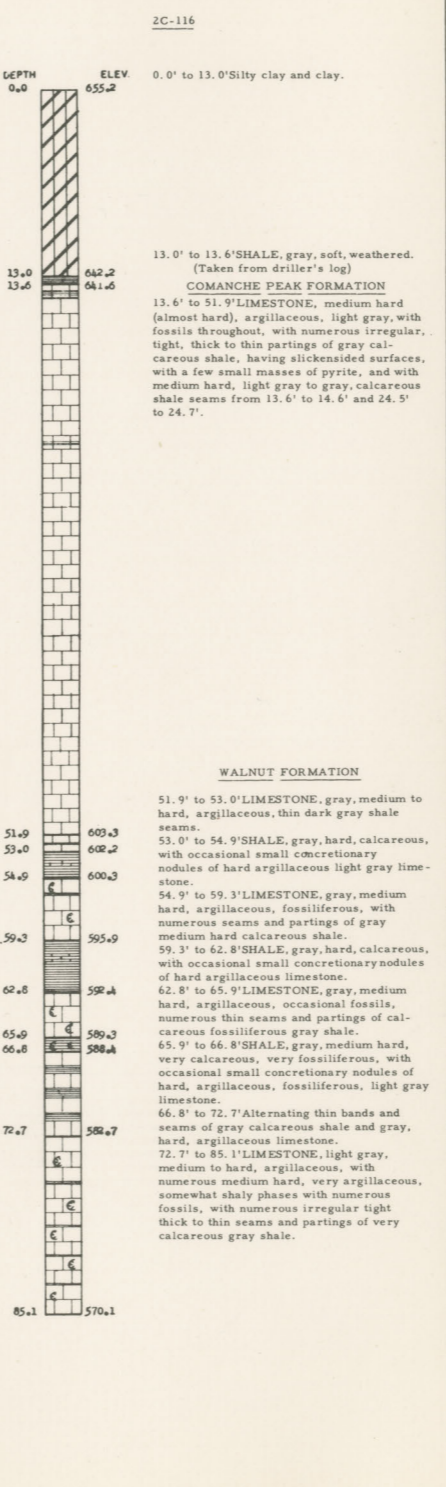
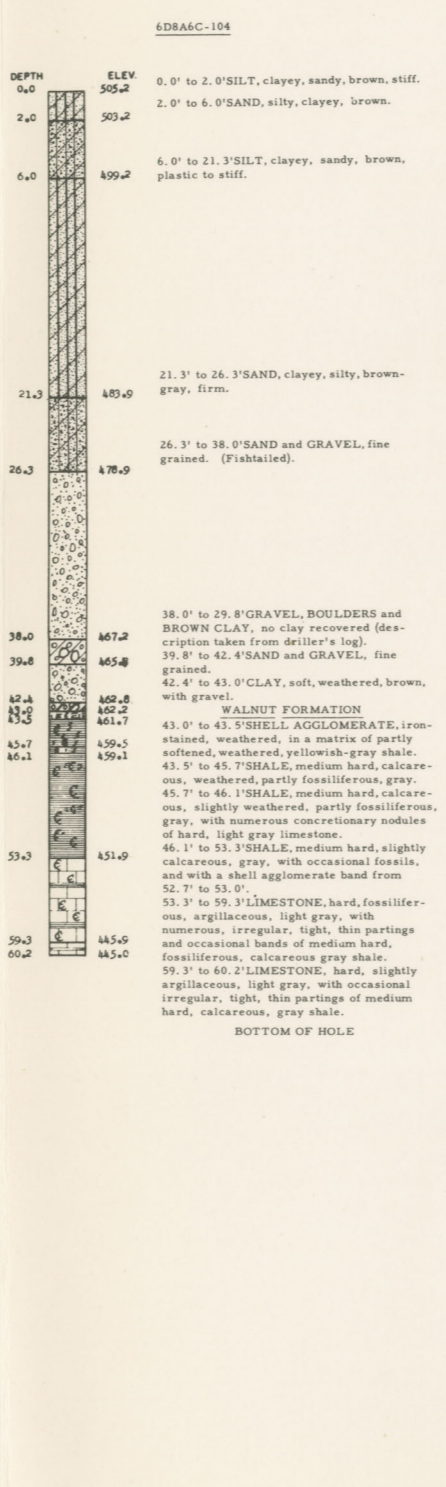
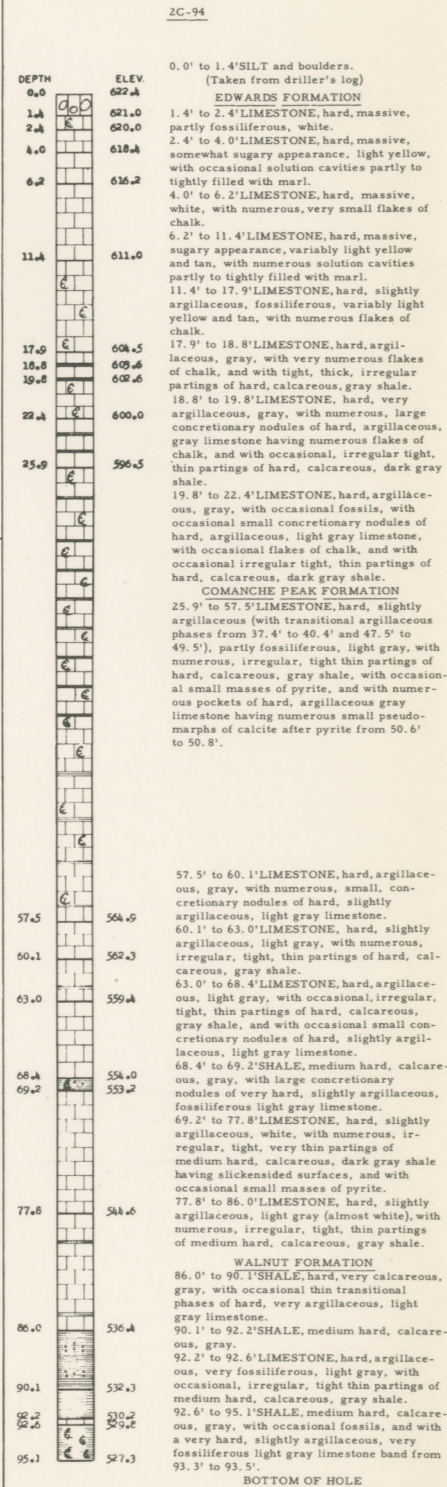
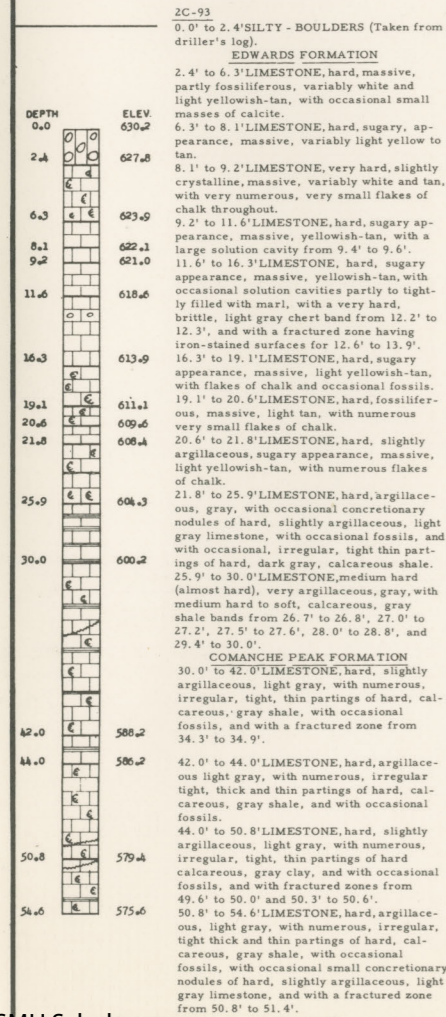
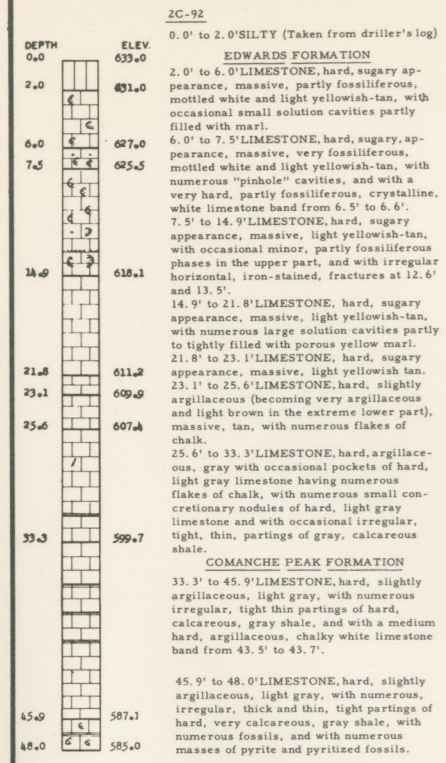
BRAZOS RIVER BASIN, TEXAS
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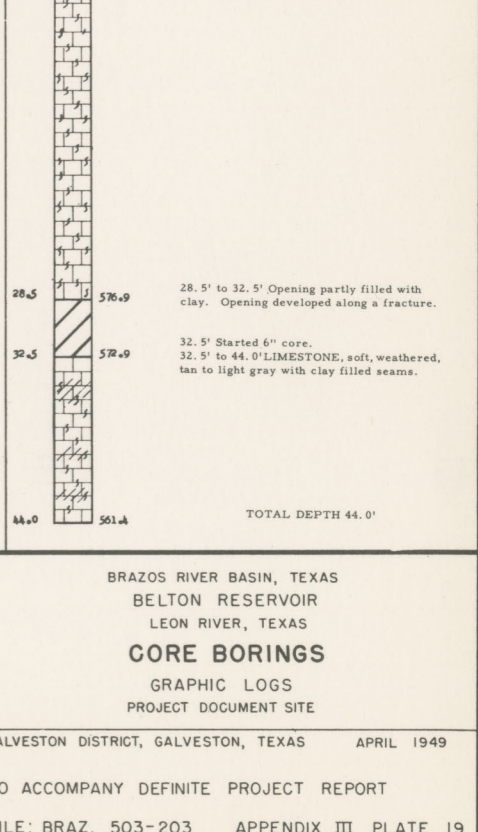
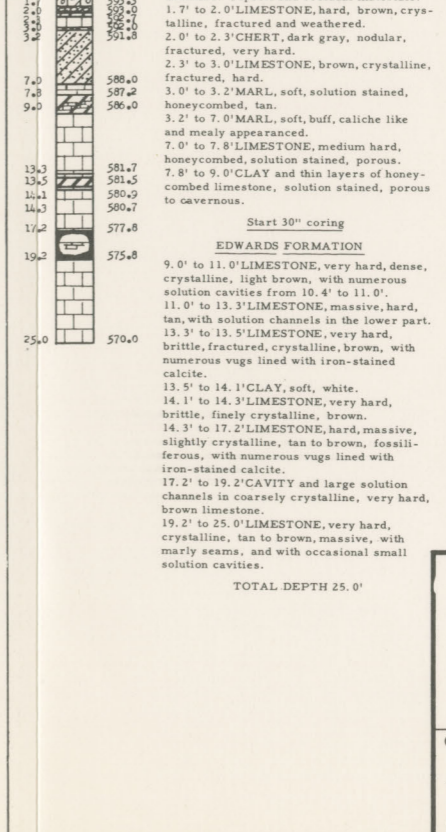
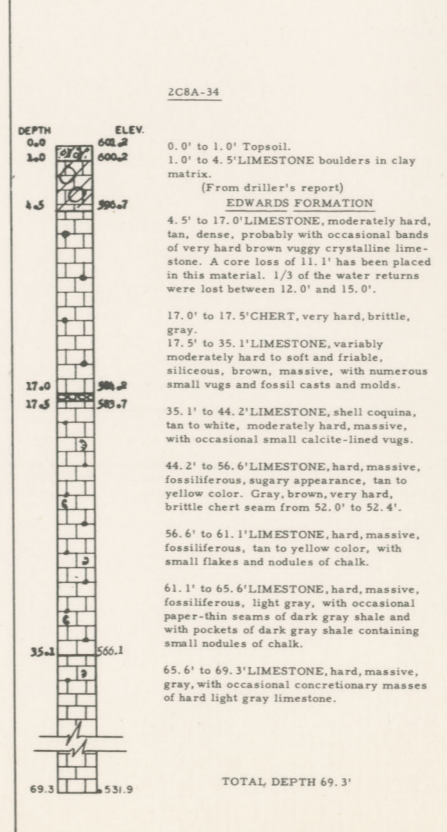
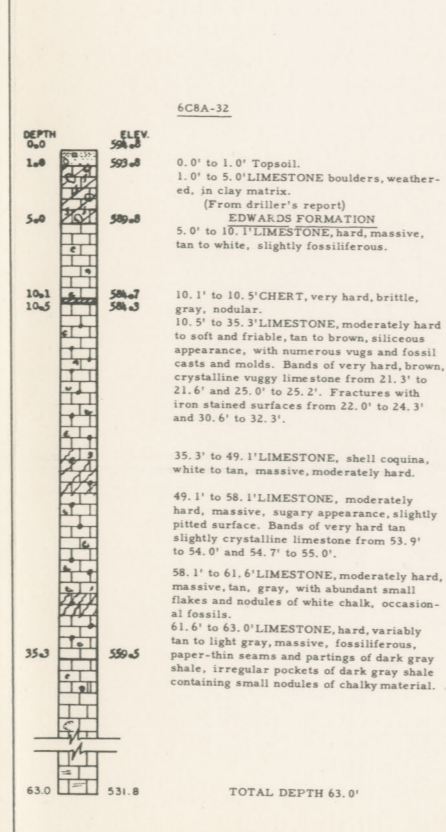
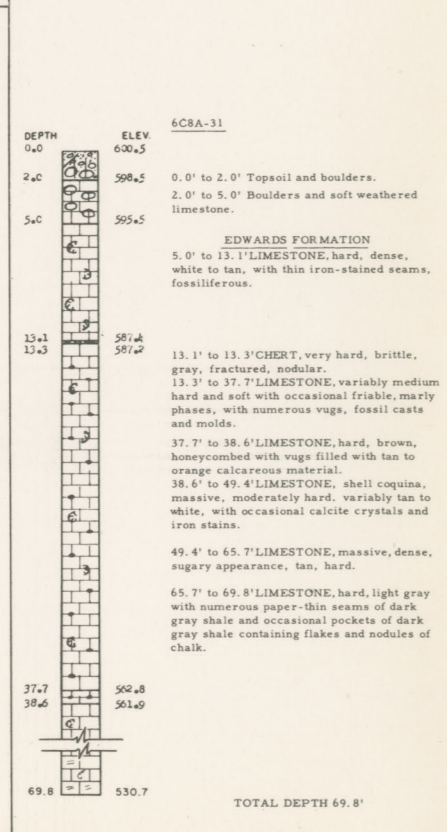
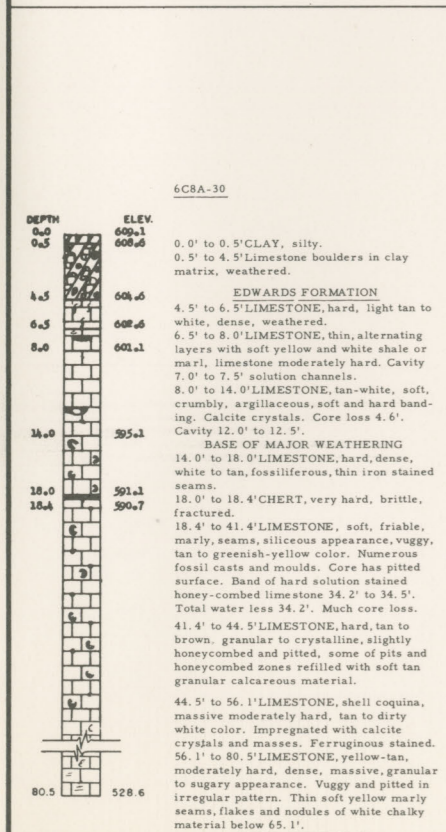
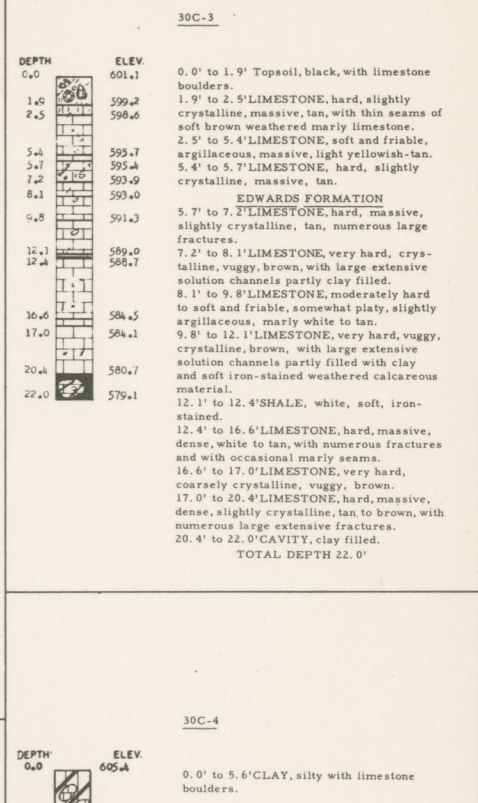
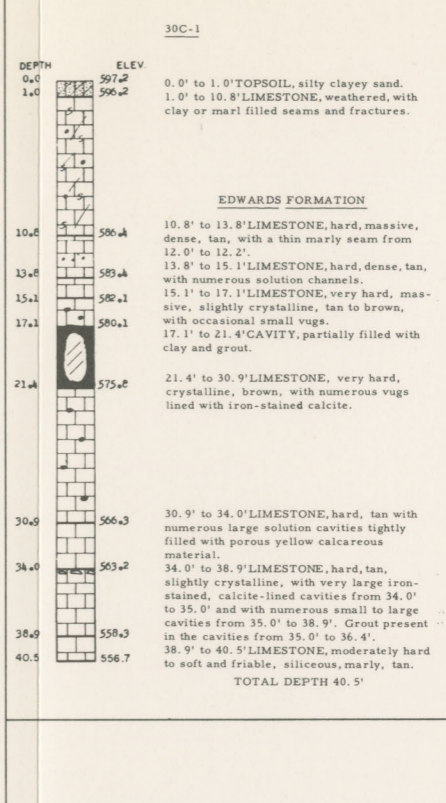
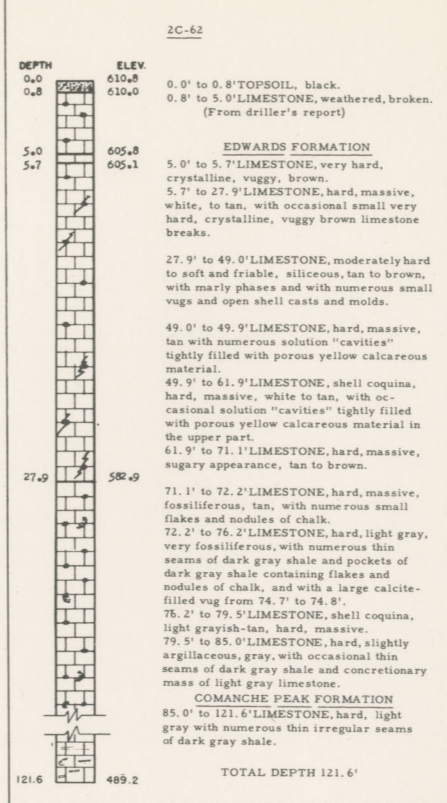
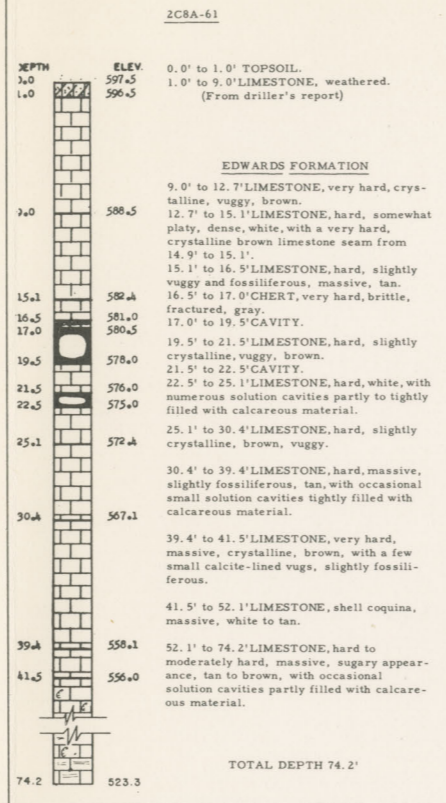
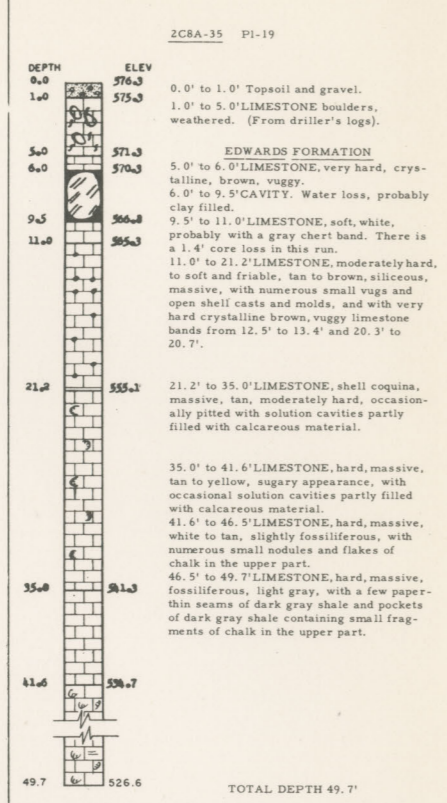
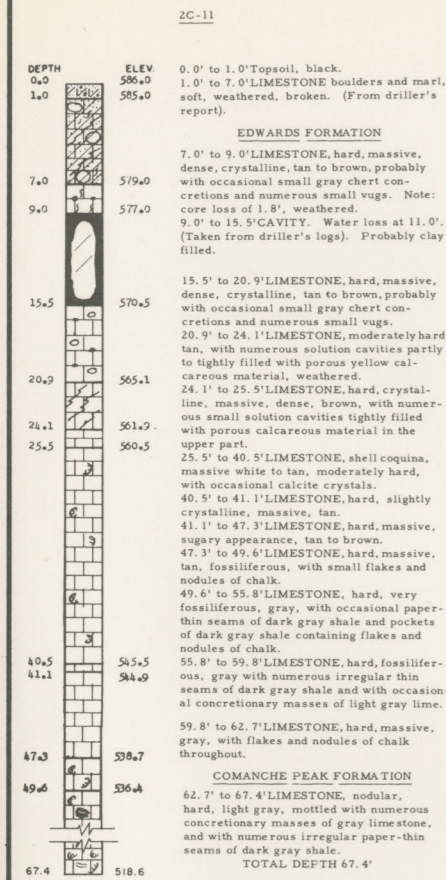


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BRAZOS RIVER BASIN, TEXAS
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SCALE AS SHOWN
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FILE: BRAZ. 503-203 APPENDIX III PLATE 17

Colligan: Geology of Belton Reservoir Area, Leon River, Bell County, Texas



BRAZOS RIVER BASIN, TEXAS
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LEON RIVER, TEXAS
CORE BORINGS
GRAPHIC LOGS
PROJECT DOCUMENT SITE

GALVESTON DISTRICT, GALVESTON, TEXAS APRIL 1949

TO ACCOMPANY DEFINITE PROJECT REPORT
FILE: BRAZ. 503-203 APPENDIX III PLATE 19

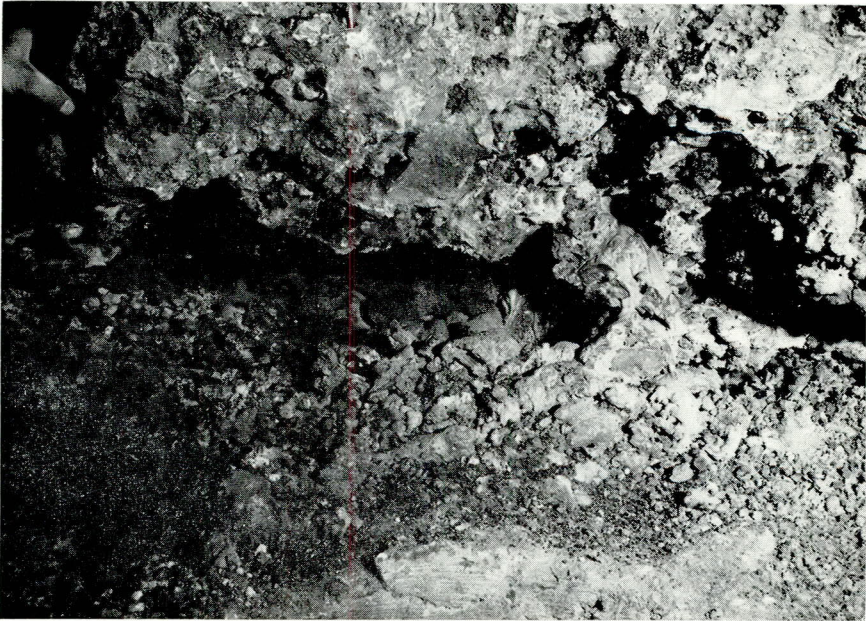


Fig. 1. Exposed cavern in wall of core boring 30C-1 at elevation 577.0 in left abutment.

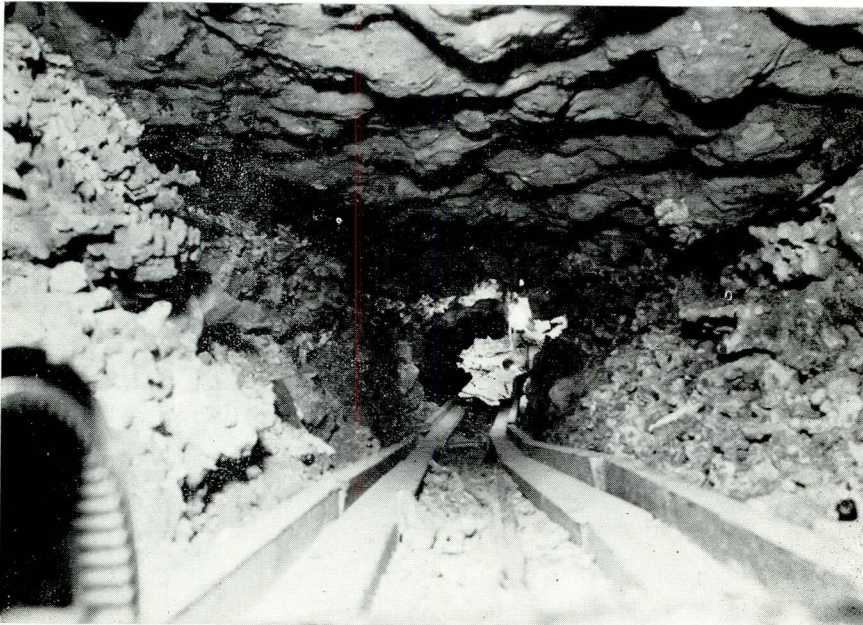


Fig. 2. Mucked-out cavern (total length about 100 feet) extending southwesterly from boring 30C-1.

BELTON RESERVOIR



Fig. 1. End of mucked-out cavern beyond which two solution channels extend approximately 50 feet.



Fig. 2. Partly mucked-out solution channel at end of mucked-out cavern.

BELTON RESERVOIR