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To be or not to be quadratic (with X-Fem)

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Abstract — A novel implementation of the eXtended Finite Element Method that makes use of quadratic elements is discussed. The present formulation is shown to behave fairly better compared to usual quadratic elements enriched with Heaviside functions in that both the number of unknowns and the condition number of element matrices considerably decrease with respect to a classical second order scheme. A representative example demonstrates the capabilities of the proposed approach.

Keywords — Fracture, X-Fem, quadratic elements.

1 Introduction

There is no question that the quality of eXtended Finite Element (X-Fem) solutions to problems with discontinuities has dramatically improved in the last decade, nor can it be doubted that there is still a long way to go.

For instance, most X-Fem implementations rely upon use of either linear triangles/quadrilaterals in 2D or linear tetrahedra/hexahedra in 3D. Aiming to get better performances and higher order convergence properties the X-Fem approximation could be modified *to be quadratic*. However, despite all expected beneficial effects of quadratic elements, in published X-Fem literature almost all but a few authors, see e.g [1, 2, 3, 4], choose *not to be quadratic*. The main motivation beyond this choice is that in the present context a successful implementation of higher order elements is conditional to the possibility that element matrices created by X-Fem are not ill-conditioned, that in a sense is intrinsic to the method itself.

2 X-Fem for discontinuities

Generally speaking, the X-Fem concept can be regarded as a special case of the partition of unity paradigm [5], the basic underlying idea being the augmentation of the approximation space generated by standard finite element shape functions. This is obtained in turn via suitable enrichment functions that incorporate some a priori knowledge about the solution of the problem under consideration. For instance, in fracture mechanics displacement discontinuities can be introduced via the generalized Heaviside function :

$$H(x) = h(\varphi(x)) = \begin{cases} +1 & \text{if } \varphi(x) \geq 0 \\ -1 & \text{if } \varphi(x) < 0 \end{cases} \quad (1)$$

where $\varphi(x)$ is a function that defines the interface position, e.g. typically a signed distance function. At any point x of the finite element mesh the displacement approximation is then obtained as :

$$\mathbf{u}^h(x) = \sum_{i=1}^{numnp} \phi_i(x) \mathbf{a}_i + \sum_{j \in N_H} \psi_j(x) H(x) \mathbf{q}_j \quad (2)$$

where $\phi_i(x)$ and $\psi_j(x)$ are standard finite element shape functions and $numnp$ is the number of nodal points. The degrees of freedom of the regular part of the interpolated displacement field are denoted by \mathbf{a}_i , whereas the additional degrees of freedom associated with the Heaviside enrichment are denoted as \mathbf{q}_j . The latter are active on the set of nodes N_H whose support is bisected by the interface ; therefore, at the element level the matrices do not have in general equal dimensions.

In usual X-Fem implementations the functions ψ_j that are used to describe the jumps are taken identical to the shape functions ϕ_i representing the regular part of the displacement field. This is not

strictly required by the method since for the Heaviside function to be exactly represented it is sufficient that functions ψ_j do comply with the partition of unity property. Likewise, the degrees of freedom \mathbf{q}_j of the jumps are usually attached to the same nodal points carrying the unknowns \mathbf{a}_i of the regular part of the displacement. Once again, this is not a compulsory condition but only a customary choice, probably originating from the fact that, at least for fracture mechanics problems, most X-Fem implementations make use of linear elements.

Unlike continuous elements, increasing the order of the standard shape functions in the X-Fem approximation (2) is not sufficient to guarantee an improvement in performances and convergence properties. Actually, when considering the Heaviside enrichment, severe ill-conditioning is likely to occur for elements crossed by an interface whenever the area or volume ratio between the two parts on the opposite sides of the interface is very small, see e.g. Figure 3. In such cases one of the Heaviside-enriched functions describing the displacement jumps tend to coincide with one standard continuous shape function, whereby the stiffness matrix becomes rank-deficient.

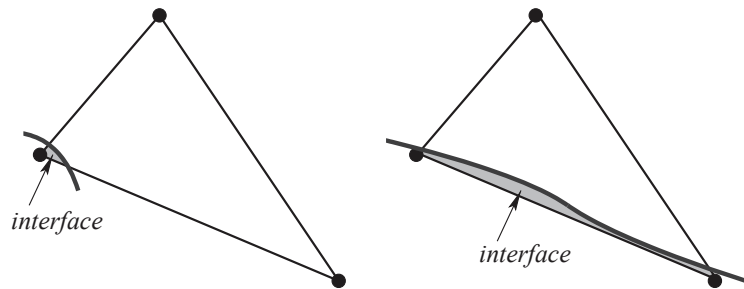


Figure 1 – interface positions that cause ill-conditioning.

In practical applications there is no way to avoid the occurrence of discontinuities arbitrarily close to nodes or element sides within the mesh, since by its very definition an X-Fem interface can be arbitrarily positioned within a mesh. Special treatments of this pathological situation have been presented in [2, 3, 6], that make use either of a pre-conditioner to eliminate linear dependencies between standard shape functions and enrichment functions or rely upon geometric or stiffness weighting criteria to decide whether to enrich a given node. Unfortunately, the above treatments seems to be not robust enough to let quadratic lagrangian elements perform well [4].

In view of general industrial applications, where simplicity of model preparation and mesh generation are essential requirements as much as good accuracy and convergence properties, *to be quadratic* with X-Fem we design elements including corner rotations as degrees of freedom. Such rotations are usually termed drilling degrees of freedom in reference to their tendency to twist nodes about the normal to the element surface.

3 Quadratic elements with drilling rotations

Finite element methods designed to include corner rotations have been discussed in many papers since the 1980s. Earlier works did not succeed in the derivation of elements with an independent rotation field in the sense of Reissner [7] either due to the presence of zero-energy modes or to the instability of finite dimensional formulations despite the well-posedness of the continuum problem.

A consistent variational framework for problems including rotational degrees of freedom has been presented by Hughes and Brezzi in [8], where a methodology is provided to obtain a robust finite element method that permits the representation of drilling rotations using a $C^{(0)}$ interpolation.

In the following it is summarized the derivation of the quadratic quadrilateral sketched in Figure 2 ; key points in setting the functional from which the element matrices are obtained are as follows :

1. the stress tensor is not a priori assumed to be symmetric ;
2. the drilling rotation is identified with the component of the infinitesimal continuum rotation normal to the plane of the element ;

These conditions are both enforced in weak form using the following functional, see [8] pag 115 :

$$\Pi_\gamma(\mathbf{u}, \boldsymbol{\theta}, \mathbf{T}^u) = \frac{1}{2} \int_\Omega \mathbb{E} (\nabla \mathbf{u})^s \cdot (\nabla \mathbf{u})^s d\Omega + \int_\Omega ((\nabla \mathbf{u})^u - \boldsymbol{\theta}) \cdot \mathbf{T}^u d\Omega - \frac{\gamma^{-1}}{2} \int_\Omega \mathbf{T}^u \cdot \mathbf{T}^u d\Omega - \int_\Omega \mathbf{f} \cdot \mathbf{u} d\Omega \quad (3)$$

where \mathbb{E} is the 2D plane elasticity matrix, \mathbf{f} are the body forces, \mathbf{u} is the displacement field, $\boldsymbol{\theta}$ and \mathbf{T} are the infinitesimal rotation and stress, and the symbols $(\cdot)^s$ and $(\cdot)^u$ stand for symmetric and skew-symmetric part of the argument, respectively. Moreover, the constant γ in (3) is a positive penalty parameter that in linear elastostatics ensures that the discrete variational problem inherits the ellipticity property from its continuum counterpart.

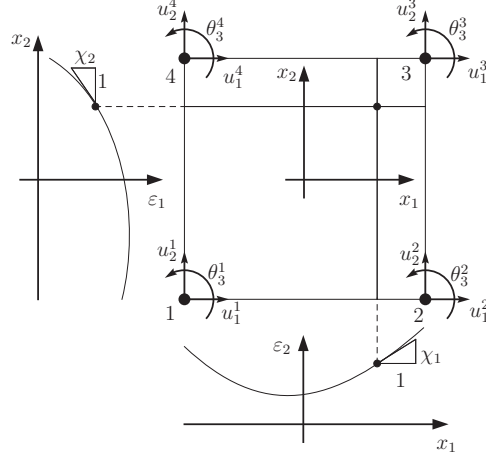


Figure 2 – quadrilateral element with drilling degrees of freedom.

The normal rotation field over the quadrilateral is interpolated by the standard bilinear shape functions whereas the in-plane displacement components are approximated using an Allman-like interpolation. In matrix notation one has :

$$\mathbf{u} = \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = \sum_{I=1}^4 \mathbf{N}_I(x_1, x_2) \mathbf{u}^I = \mathbf{N} \mathbf{a} \quad (4)$$

$$(\nabla \mathbf{u})^s = \begin{bmatrix} \varepsilon_1 \\ \varepsilon_2 \\ \gamma_{12} \end{bmatrix} = \sum_{I=1}^4 \mathbf{B}_I(x_1, x_2) \mathbf{u}^I = \mathbf{B} \mathbf{a} \quad (5)$$

$$(\nabla \mathbf{u})^u - \boldsymbol{\theta} = \sum_{I=1}^4 \mathbf{b}_I(x_1, x_2) \mathbf{u}^I = \mathbf{b} \mathbf{a} \quad (6)$$

$\mathbf{u}^I = [u_1^I, u_2^I, \theta_3^I]^T$ being the displacement vector at node I and \mathbf{a} the vector collecting all the element displacement components. The discretized counterpart of the functional (3) that is arrived at reads :

$$\Pi_\gamma^h(\mathbf{a}, \boldsymbol{\tau}) = \frac{1}{2} \int_\Omega \mathbb{E} \mathbf{B} \mathbf{a} \cdot \mathbf{B} \mathbf{a} d\Omega + \int_\Omega \mathbf{b} \mathbf{a} \boldsymbol{\tau} d\Omega - \frac{1}{2} \gamma^{-1} \int_\Omega \boldsymbol{\tau}^2 d\Omega - \int_\Omega \mathbf{f} \cdot \mathbf{N} \mathbf{a} d\Omega \quad (7)$$

where the parameter $\boldsymbol{\tau}$ represents the interpolation for the skew-symmetric stress ; since no continuity is needed for it, an element-wise constant interpolation is chosen and the unknown $\boldsymbol{\tau}$ is condensed out at the element level. Therefore, the element stiffness matrix is obtained by adding to the usual stiffness matrix the rank-one correction :

$$\frac{\gamma}{meas(\Omega)} \int_\Omega (\mathbf{b} \otimes \mathbf{b}) d\Omega \quad (8)$$

where $meas(\Omega)$ is the element area.

4 Numerical example

To demonstrate the current element capabilities a simple problem is considered that consists of a single element, either a standard serendipity element (Q8) or a quadrilateral with drilling rotations (QD4), occupying the domain $[-1, 1] \times [-1, 1]$. The element is cut by an X-Fem interface parallel to one of its sides and the discontinuity is described via the Heaviside function (1).

The position of the interface on the element is parametrized through the abscissa ε ($\varepsilon = 0$ corresponds to the left side of the element). No boundary condition is imposed on the element, that it is completely free; therefore it possesses six rigid-body motions, i.e. the three rigid body motions of the underlying continuum element plus three extra rigid-body motions due to the presence of the X-Fem interface.

In figure 3 is depicted the condition number of the Q8 and QD4 element stiffness matrices at varying interface position. The superior performance of the QD4 element, that remains workable for up to $\varepsilon = 1 \cdot 10^{-7}$, is immediately recognized whereas the stiffness matrix of the Q8 element is already almost singular for $\varepsilon = 1 \cdot 10^{-3}$.

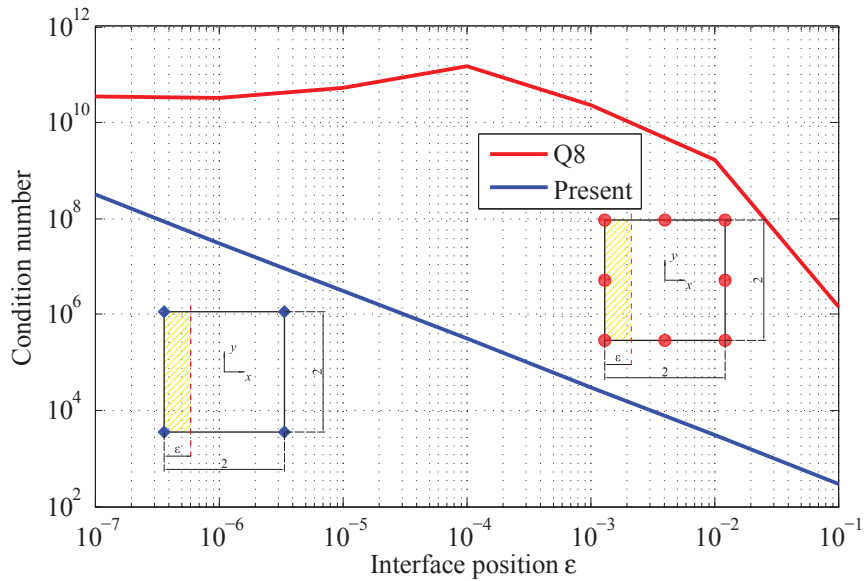


Figure 3 – conditioning of element matrices.

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