

Transfer Matrix Analysis

A Project Submitted
In Partial Fulfillment of the Requirements
For the Degree of

**Bachelor of Technology
In Civil Engineering**

By
Jasmeet Singh
10401015
&
Suman Samal
10401020



**DEPARTMENT OF CIVIL ENGINEERING
NATIONAL INSTITUTE OF TECHNOLOGY, ROURKELA**

2008

Transfer Matrix Analysis

A Project Submitted
In Partial Fulfillment of the Requirements
For the Degree of

**Bachelor of Technology
In Civil Engineering**

By
Jasmeet Singh
10401015
&
Suman Samal
10401020

Under the Guidance of
Dr. M. R. Barik



**DEPARTMENT OF CIVIL ENGINEERING
NATIONAL INSTITUTE OF TECHNOLOGY, ROURKELA**

2008



**NATIONAL INSTITUTE OF TECHNOLOGY
ROURKELA**

CERTIFICATE

This is to certify that the thesis entitled, “**TRANSFER MATRIX ANALYSIS**” submitted by Jasmeet Singh (10401015) and Suman Samal (10401020) in partial fulfillment of the requirements for the award of Bachelor of Technology Degree in Civil Engineering at the National Institute of Technology, Rourkela (Deemed University) is an authentic work carried out by them under my supervision and guidance.

Date:-

Dr.M.R.Barik

**Dept. of Civil Engineering
National Institute of Technology
Rourkela - 769008**

ACKNOWLEDGEMENT

We would like to articulate our deep gratitude to our project guide **Dr. M.R.Barik** who has always been our motivation for carrying out the project.

We wish to convey our sincere gratitude to all the faculties of Civil Engineering Department who have enlightened us during our studies. The facilities and co-operation received from the technical staff of Civil Engineering Department is thankfully acknowledged.

We would like to thank the computer lab in charge M.tech. students for their timely inputs and fulsome support. Last, but certainly not the least, we wish to thank our friends who stood beside through all our endeavors.

Jasmeet Singh

Roll no.:-10401015

Dept. of Civil Engineering

National Institute of Technology

Rourkela – 769008

Suman Samal

Roll no.:-10401020

Dept. of Civil Engineering

National Institute of Technology

Rourkela - 769008

CONTENTS

CHAPTER NO.	TITLE	PAGE NO.
	CERTIFICATE	iii
	ACKNOWLEDGEMENT	iv
	CONTENTS	v
	LIST OF FIGURES	viii
	ABSTRACT	x
1	INTRODUCTION	1
2	BASIC PRINCIPLE OF MATRIX METHOD	3
	2.1 INTRODUCTION	4
	2.2 MATRIX ADDITION AND SUBTRACTION	5
	2.3 MATRIX MULTIPLICATION	5
	2.4 TRANSPOSE OF A MATRIX	5
	2.5 DETERMINANT OF A MATRIX	6
	2.6 INVERSE OF A MATRIX	6
	2.7 SOLVING A SYSTEM OF EQUATIONS USING MATRIX	7
	2.7.1 Inverse Matrix Method	8
	2.7.2 Cramer's Rule	9
3	TRANSFER MATRIX METHOD	10
	3.1 ALGORITHM OF TRANSFER MATRIX METHOD	11
	3.2 STATIC ANALYSIS	11
4	VIBRATIONS AND NATURAL FREQUENCY	16
	4.1 INTRODUCTION	17

	4.2 DYNAMIC LOAD AND D’ALEMBERT PRINCIPLE	18
	4.2.1 Modes of Vibration	19
	4.3 DETERMINATION OF THE FUNDAMENTAL FREQUENCY OF A MULTISTOREYED BUILDING	20
5	TORSIONAL VIBRATION OF SHAFTS	22
	5.1 INTRODUCTION	23
	5.2 SOLVING USING TRANSFER MATRIX METHOD	23
6	COMPUTER PROGRAMMING AND RESULTS	25
	6.1 FUNDAMENTAL FREQUENCY OF SINGLE BAY MULTI-STOREY BUILDING	
	6.1.1 Program	26
	6.1.2 Flowchart	28
	6.1.3 Result	29
	6.2 FUNDAMENTAL FREQUENCY OF MULTIPLE BAY MULTIPLE-STOREY BUILDING	
	6.2.1 Program	30
	6.2.2 Flowchart	33
	6.2.3 Result	34
	6.3 TORSIONAL VIBRATION OF SHAFT	
	6.3.1 Program	35
	6.3.2 Flowchart	38
	6.3.3 Result	39
	6.4. ANALYSIS OF A BEAM SECTION	
	6.4.1 Program	40
	6.4.2 Flowchart	51
	6.4.3 Result	55
	6.5. ANALYSIS OF A BEAM SECTION WITH UNEQUAL SECTIONS AND WITH STIFFNERS	
	6.5.1 Program	56

	6.5.2 Flowchart	71
	6.5.3 Result	75
7	CONCLUSION	76

LIST OF FIGURES

SL No.	Title	Page No.
Fig.4.1	Modes of Vibration in Building	19
Fig.4.2	Modes of Vibration in String	20
Fig.4.3	Wind Load on Multi-Storeyed Building	20
Fig.5.1	Torsional Vibration of Shaft	24
Fig.6.1	Flowchart for calculating the Fundamental Frequency of a Multi-Storeyed Building	28
Fig.6.2	Problem Statement	29
Fig.6.3	Solution obtained from Program	29
Fig.6.4	Flowchart for calculating the Fundamental Frequency of a Multi-Storeyed Building with Multiple Bays	33
Fig.6.5	Problem Statement	34
Fig.6.6	Solution obtained from Program	34
Fig 6.7	Flowchart for calculating the Fundamental Frequency of a shaft with torsional forces acting on it	38
Fig.6.8	Problem Statement	39
Fig.6.9	Solution obtained from Program	39
Fig 6.10	Flowchart for calculating the slope, displacement, moment and reaction of a beam section with a single cross-sectional area	51-54
Fig.6.11	Problem Statement	55
Fig.6.12	Solution obtained from Program	55
Fig 6.13	Flowchart for calculating the slope,	

	displacement, moment and reaction of a beam section with a various cross-sectional area and having vertical and angular stiffeners	71-74
Fig.6.14.	Problem Statement	75
Fig.6.15.	Solution obtained from Program	75



NATIONAL INSTITUTE OF TECHNOLOGY
ROURKELA

ABSTRACT

Vibration analysis of arbitrary shaped structures has been of interest to structural designers for several decades. Dynamic behavior of these structures is strongly dependent on boundary conditions, geometrical shapes, material properties, different theories, and various complicating effects. Closed-form solutions are possible only for a limited set of simple boundary conditions and geometries. For analysis of arbitrary shaped structures, several numerical methods, such as finite element method, finite difference method, boundary element method, and so on, are usually applied. In this paper, we have tried to put some light on the methods of calculation of methods to find out the vibration in structural members by taking advantages Transfer Matrix Methods in calculating the vibration in Multi-Storied Buildings, the various beam actions in a given beam with some given loading criteria and also used to compute the natural frequency of a shaft under torsional vibration. Computer-Aided Analysis has also been provided to calculate the vibration in a multi-storied building.

1

INTRODUCTION

Transfer matrix method is an approach to matrix structural analysis that uses a mixed form of the element force-displacement relationship and transfers the structural behavior parameters the joint forces and displacement from one end of the structures of line element to other). An advantage of transfer matrix method is that it produces a system of equation to be solved that are quite small in comparison with those produced by the stiffness method. A disadvantage is the extensive sequence of operations that are required on a small matrix.

The transfer-matrix method is used when the total system can be broken into a *sequence* of subsystems that interact only with adjacent subsystems. This method is suitable for line structures such as arches and cables.

To implement the transfer matrix method, we need a relationship that gives the state of forces and displacements at one end of the element in terms of force and displacement at the other end.

2

BASIC PRINCIPLES OF MATRIX METHODS

3

2.1 INTRODUCTION

A matrix is defined as an ordered rectangular array of numbers. They can be used to represent systems of linear equations, as will be explained below

Here are a couple of examples of different types of matrices:

- Symmetric matrix

$$\begin{bmatrix} 1 & 2 & 3 \\ 2 & 0 & -5 \\ 3 & -5 & 6 \end{bmatrix}$$

- Diagonal matrix

$$\begin{bmatrix} 1 & 0 & 0 \\ 0 & 4 & 0 \\ 0 & 0 & 6 \end{bmatrix}$$

- Upper Triangular matrix

$$\begin{bmatrix} 1 & 2 & 3 \\ 0 & 7 & -5 \\ 0 & 0 & -4 \end{bmatrix}$$

- Lower Triangular matrix

$$\begin{bmatrix} 1 & 0 & 0 \\ -4 & 7 & 0 \\ 12 & 5 & 3 \end{bmatrix}$$

- Zero matrix

$$\begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

- Identity matrix

$$\begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

And a fully expanded mxn matrix A, would look like this:

$$A = \begin{pmatrix} a_{11} & a_{12} & \cdots & a_{1n} \\ a_{21} & a_{22} & \cdots & a_{2n} \\ \vdots & \vdots & \ddots & \vdots \\ a_{m1} & a_{m1} & \cdots & a_{mn} \end{pmatrix}$$

or in a more compact form:

$$A = (a_{ij})$$

2.2 MATRIX ADDITION AND SUBTRACTION

Two matrices A and B can be added or subtracted if and only if their dimensions are the same (i.e. both matrices have the identical amount of rows and columns).

2.3 MATRIX MULTIPLICATION

When the number of columns of the first matrix is the same as the number of rows in the second matrix then matrix multiplication can be performed.

Where A has dimensions mxn, B has dimensions nxp. Then the product of A and B is the matrix C, which has dimensions mxp. The ijth element of matrix C is found by multiplying the entries of the ith row of A with the corresponding entries in the jth column of B and summing the n terms

Note: That AxB is not the same as BxA

$$\begin{pmatrix} a & b & c \\ d & e & f \\ g & h & i \end{pmatrix} \begin{pmatrix} j & k & l \\ m & n & o \\ p & q & r \end{pmatrix} = \begin{pmatrix} (aj+bm+cp) & (ak+bn+cq) & (al+bo+cr) \\ (dj+em+fp) & (dk+en+fq) & (dl+eo+fr) \\ (gj+hm+ip) & (gk+hn+iq) & (gl+ho+ir) \end{pmatrix}$$

2.4 TRANSPOSE OF A MATRIX

The transpose of a matrix is found by exchanging rows for columns i.e. Matrix A = (aij) and the transpose of A is:

AT=(aji) where j is the column number and i is the row number of matrix A.

In the case of a square matrix (m=n), the transpose can be used to check if a matrix is symmetric.

For a symmetric matrix A = AT

2.5 DETERMINANT OF A MATRIX

Determinants play an important role in finding the inverse of a matrix and also in solving systems of linear equations. We assume we have a square matrix ($m=n$). The determinant of a matrix A will be denoted by $\det(A)$ or $|A|$. Firstly the determinant of a 3×3 matrix will be introduced then the $n \times n$ case will be shown.

Determinant of a 3×3 matrix

$$A = \begin{pmatrix} a_{11} & a_{12} & a_{13} \\ a_{21} & a_{22} & a_{23} \\ a_{31} & a_{32} & a_{33} \end{pmatrix}$$

$$\det(A) = |A| = \begin{vmatrix} a_{11} & a_{12} & a_{13} \\ a_{21} & a_{22} & a_{23} \\ a_{31} & a_{32} & a_{33} \end{vmatrix} = a_{11} \begin{vmatrix} a_{22} & a_{23} \\ a_{32} & a_{33} \end{vmatrix} - a_{12} \begin{vmatrix} a_{21} & a_{23} \\ a_{31} & a_{33} \end{vmatrix} + a_{13} \begin{vmatrix} a_{21} & a_{22} \\ a_{31} & a_{32} \end{vmatrix}$$

Determinant of a $n \times n$ matrix

For the general case, where A is an $n \times n$ matrix the determinant is given by:

$$\det(A) = |A| = a_{11}a_{11} + a_{12}a_{12} + \dots + a_{1n}a_{1n}$$

Where the coefficients a_{ij} are given by the relation

$$a_{ij} = (-1)^{i+j} \beta_{ij}$$

where β_{ij} is the determinant of the $(n-1) \times (n-1)$ matrix that is obtained by deleting row i and column j . This coefficient a_{ij} is also called the cofactor of a_{ij} .

2.6 INVERSE OF A MATRIX

Assuming we have a square matrix A , which is non-singular (i.e. $\det(A)$ does not equal zero), then there exists an $n \times n$ matrix A^{-1} which is called the inverse of A , such that this property holds:

$$AA^{-1} = A^{-1}A = I \text{ where } I \text{ is the identity matrix.}$$

The inverse of a $n \times n$ matrix

The inverse of a general $n \times n$ matrix A can be found by using the following equation:

$$A^{-1} = \frac{adj(A)}{\det(A)}$$

Where the $adj(A)$ denotes the adjoint (or adjugate) of a matrix. It can be calculated by the following method

- Given the $n \times n$ matrix A , define

$$B = (b_{ij})$$

to be the matrix whose coefficients are found by taking the determinant of the $(n-1) \times (n-1)$ matrix obtained by deleting the i th row and j th column of A . The terms of B (i.e. $B = b_{ij}$) are known as the cofactors of A .

- And define the matrix C , where

$$c_{ij} = (-1)^{i+j} b_{ij}$$

- The transpose of C (i.e. C^T) is called the adjoint of matrix A .

Lastly to find the inverse of A divide the matrix C^T by the determinant of A to give its inverse.

2.7 SOLVING A SYSTEM OF EQUATIONS USING MATRIX

A system of linear equations is a set of equations with n equations and n unknowns, is of the form of

$$\begin{aligned} a_{11}x_1 + a_{12}x_2 + \dots + a_{1n}x_n &= b_1 \\ a_{21}x_1 + a_{22}x_2 + \dots + a_{2n}x_n &= b_2 \\ &\vdots \\ a_{n1}x_1 + a_{n2}x_2 + \dots + a_{nn}x_n &= b_n \end{aligned}$$

The unknowns are denoted by x_1, x_2, \dots, x_n and the coefficients (a 's and b 's above) are assumed to be given. In matrix form the system of equations above can be written as:

$$\begin{pmatrix} a_{11} & a_{12} & \dots & a_{1n} \\ a_{21} & a_{22} & \dots & a_{2n} \\ \vdots & \vdots & \ddots & \vdots \\ a_{n1} & a_{n2} & \dots & a_{nn} \end{pmatrix} \begin{pmatrix} x_1 \\ x_2 \\ \vdots \\ x_n \end{pmatrix} = \begin{pmatrix} b_1 \\ b_2 \\ \vdots \\ b_n \end{pmatrix}$$

A simplified way of writing above is like this ; $Ax = b$

After looking at this there are two methods used to solve matrices these are

- Inverse Matrix Method
- Cramer's Rule

2.7.1 Inverse Matrix Method

The inverse matrix method uses the inverse of a matrix to help solve a system of equations, such like the above $Ax = b$. By pre-multiplying both sides of this equation by A^{-1} gives:

$$A^{-1}(Ax) = A^{-1}b$$

$$(A^{-1}A)x = A^{-1}b$$

or alternatively this gives

$$x = A^{-1}b$$

So by calculating the inverse of the matrix and multiplying this by the vector b we can find the solution to the system of equations directly. And from earlier we found that the inverse is given by

$$A^{-1} = \frac{adj(A)}{\det(A)}$$

From the above it is clear that the existence of a solution depends on the value of the determinant of A . There are three cases:

If the $\det(A)$ does not equal zero then solutions exist using

If the $\det(A)$ is zero and $b=0$ then the solution will be not be unique or does not exist.

If the $\det(A)$ is zero and $b \neq 0$ then the solution can be $x = 0$ but as in 2. is not unique or does not exist.

Looking at two equations we might have that

$$ax + by = c$$

$$dx + ey = f$$

Written in matrix form would look like

$$\begin{pmatrix} a & b \\ d & e \end{pmatrix} \begin{pmatrix} x \\ y \end{pmatrix} = \begin{pmatrix} c \\ f \end{pmatrix}$$

and by rearranging we would get that the solution would look like

$$\begin{pmatrix} x \\ y \end{pmatrix} = \begin{pmatrix} a & b \\ d & e \end{pmatrix}^{-1} \begin{pmatrix} c \\ f \end{pmatrix}$$

2.7.2 Cramer's Rule

Cramer's rules uses a method of determinants to solve systems of equations. Starting with equation below,

$$\begin{aligned} a_{11}x_1 + a_{12}x_2 + \dots + a_{1n}x_n &= b_1 \\ a_{21}x_1 + a_{22}x_2 + \dots + a_{2n}x_n &= b_2 \\ &\vdots \\ a_{n1}x_1 + a_{n2}x_2 + \dots + a_{nn}x_n &= b_n \end{aligned}$$

The first term x_1 above can be found by replacing the first column of A by $(b_1 \ b_2 \ \dots \ b_n)^T$.

Doing this we obtain:

$$x_1 = \frac{1}{|A|} \begin{vmatrix} b_1 & a_{12} & a_{13} & \dots & a_{1n} \\ b_2 & a_{22} & a_{23} & \dots & a_{2n} \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ b_n & a_{n2} & a_{n3} & \dots & a_{nn} \end{vmatrix}$$

Similarly for the general case for solving x_r we replace the r th column of A by b and expand the determinant.

This method of using determinants can be applied to solve systems of linear equations. We will illustrate this for solving two simultaneous equations in x and y and three equations with 3 unknowns x , y and z .

Three simultaneous equations in x , y and z

$$ax+by+cz = p$$

$$dx+ey+fz = q$$

$$gx+hy+iz = r$$

To solve use the following:

$$x = \frac{\text{Det} \begin{bmatrix} p & b & c \\ q & e & f \\ r & h & i \end{bmatrix}}{\text{Det} \begin{bmatrix} a & b & c \\ d & e & f \\ g & h & i \end{bmatrix}}, \quad y = \frac{\text{Det} \begin{bmatrix} a & p & c \\ d & q & f \\ g & r & i \end{bmatrix}}{\text{Det} \begin{bmatrix} a & b & c \\ d & e & f \\ g & h & i \end{bmatrix}}, \quad \text{and} \quad z = \frac{\text{Det} \begin{bmatrix} a & b & p \\ d & e & q \\ g & h & r \end{bmatrix}}{\text{Det} \begin{bmatrix} a & b & c \\ d & e & f \\ g & h & i \end{bmatrix}}$$

3

TRANSFER MATRIX METHOD

3.1 ALGORITHM OF TRANSFER MATRIX METHOD

The algorithm of the transfer matrix method is defined in two principal steps:-

1. The unknown initial parameter is defined by the matrix of initial parameters are transferred using the matrix multiplication of transfer and nodal matrices into the end point of the applied simulation. The boundary condition in the end point, implemented in the matrix of boundary conditions, define the set of algebraic equations for determination of the unknown initial parameters.
2. The calculated initial parameters are put into the state vector in initial point of simulation and is repeated multiplications with transfer and nodal matrices determine the set of resulting state vectors, stress and strain components in nodal points of the used discrete model.

3.2 STATIC ANALYSIS

The state vector of bending components in the initial point of the arbitrary one-dimensional element of the discrete simulation is given by:-

$$\mathbf{k}_k(0) = (v_k(0), \varphi_k(0), M_k(0), Q_k(0), 1)^T$$

where v_k , φ_k , M_k , Q_k are the displacements, slope, moment and the lateral force in node K, respectively. The last term of the vector \mathbf{k} is the preliminary unit and has the importance for incorporation of the external loading or deformation components into calculation process.

When deriving the corresponding transfer matrix \mathbf{BK} , the known transformation equations for the couplings of the components of the state vector \mathbf{k}_k between initial and end points of the studied element can be written as

$$v_k(l_k) = v_k(0) + l_k \varphi_k(0) - \frac{l_k^2}{2EJ_k} M_k(0) - \frac{l_k^3}{6EJ_k} Q_k(0),$$

$$\varphi_k(l_k) = \varphi_k(0) - \frac{l_k}{EJ_k} M_k(0) - \frac{l_k^2}{2EJ_k} Q_k(0),$$

$$M_k(l_k) = M_k(0) + l_k Q_k(0),$$

$$Q_k(l_k) = Q_k(0),$$

$$1 = 1,$$

where l_k is the length of the element and EJ_k is its stiffness.

These equations may be written as the vector-matrix relation

$$k_k(l_k) = BK * k_k(0),$$

where **BK** is the transfer matrix of the beam

$$BK = \begin{pmatrix} 1 & l_k & -\frac{l_k^2}{2EJ_k} & -\frac{l_k^3}{6EJ_k} \\ 0 & 1 & -\frac{l_k}{EJ_k} & -\frac{l_k^2}{2EJ_k} \\ 0 & 0 & 1 & l_k \\ 0 & 0 & 0 & 1 \end{pmatrix}$$

which produced the desired coupling between state vectors.

In nodes of adjacent elements the loading or inertial parameters may be concentrated as well as various elastic or rigid intermediate supports. The transfer of the state vector over the nodal points of adjacent elements is performed using nodal matrices. The coupling equations in a single node of two elements of the simulation under the investigation are

$$\begin{aligned} v_k(l_{k+1}) &= v_k(l_k), \\ \varphi_k(l_{k+1}) &= \varphi_k(l_k), \\ M_k(l_{k+1}) &= M_k(l_k), \\ Q_k(l_{k+1}) &= Q_k(l_k), \\ 1 &= 1, \end{aligned}$$

which, written in the vector-matrix form as

$$\begin{bmatrix} v_k(l_{k+1}) \\ \varphi_k(l_{k+1}) \\ M_k(l_{k+1}) \\ Q_k(l_{k+1}) \\ 1 \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} v_k(l_k) \\ \varphi_k(l_k) \\ M_k(l_k) \\ Q_k(l_k) \\ 1 \end{bmatrix}$$

determines the nodal matrix **CK** as

$$CK = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

When the node under investigation, a external force $\pm P_k$ is present, the equation is modified via

$$Q_{k+1} = Q_k(l_k) \pm P_k$$

and the nodal matrix assumes form

$$CK = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & \pm P_k \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

with the nonzero force in the last column. Other external state components (initial deformation, slope or moment) concentrated in this node may be incorporated into the last column of the nodal matrix. If in addition the elastic support with stiffness characteristics H_k , h_k is located in this node

$$M_{k+1} = \pm H_k \varphi_k(l_k) + M_k(l_k),$$

$$Q_{k+1} = \pm h_k v_k(l_k) + Q_k(l_k) \pm P_k,$$

and the nodal matrix is given by

$$CK = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & \pm H_k & 1 & 0 & 0 \\ \pm h_k & 0 & 0 & 1 & \pm P_k \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

For the case of rigid support the stiffness characteristics H_k and h_k tend to infinity (10^{20} and greater, depending on the type of computer). Hence, the degree of static redundancy of the structure doesn't influence the line algorithm of the solution. Generally, the significance of the nodal matrix **CK** is the defined as

$$k_{k+1}(0) = CK^* k_k(l_k).$$

The nodal matrix, like the transfer matrix, allows the general variability of all transported physical parameters in individual nodal points.

The boundary conditions defined in the initial point of the studied discrete simulation are given by

$$v_0(0) = 0,$$

$$\varphi_0(0) = ?,$$

$$M_0(0) = 0,$$

$$Q_0(0) = ?,$$

where $\varphi_0(0)$ and $Q_0(0)$ are the unknown initial parameters.

The state vector k_0 and the vector of the initial parameters u_0 in the initial point of the applied model are then given by

$$k_0 = \begin{bmatrix} v_0(0) \\ \varphi_0(0) \\ M_0(0) \\ Q_0(0) \\ 1 \end{bmatrix}, \quad u_0 = \begin{bmatrix} \varphi_0(0) \\ Q_0(0) \\ 1 \end{bmatrix}$$

The relation between these two vectors is given by

$$k_0 = \mathbf{HL} u_0,$$

where \mathbf{HL} is the matrix of initial parameters.

The matrix of initial parameters for the boundary conditions is then written as

$$HL = \begin{bmatrix} 0 & 0 & 0 \\ 1 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

The boundary conditions in the end point of the studied simulation

$$\begin{aligned}
v_n(L) &= 0, \\
\varphi_n(L) &= ?, \\
M_n(L) &= 0, \\
Q_n(L) &= ?
\end{aligned}$$

are implemented into the algorithm of method over the matrix of the boundary conditions **RP**, derived similarly as **HL**, and expressed as

$$RP = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \end{bmatrix}$$

The algorithm of the TMM in a typical two-step concept is defined by two following operations:

STEP 1- determination of the unknown initial parameters

$$\left(RP \left(\left(\prod_{i=1}^n (CK * BK) \right) HL \right) \right) u_0 \Rightarrow \begin{cases} \varphi_0(0) \\ Q_0(0) \end{cases}$$

STEP 2- input of calculated initial parameters into the initial state vector k_0 and determination of the state vectors in each node as

$$k_i = \prod_{i=1}^n (CK * BK) k_0$$

i.e.,

$$k_n = (CK BK) k_{n-1}$$

The present line concept may be adopted for the analysis of one-dimensional or multi-dimensional structural simulations.

4

VIBRATIONS AND NATURAL FREQUENCY

4.1 INTRODUCTION

The vibrating object is the source of the disturbance which moves through the medium. The vibrating object which creates the disturbance could be the vocal chords of a person, the vibrating string and sound board of a guitar or violin, the vibrating tines of a tuning fork, or the vibrating diaphragm of a radio speaker. Any object which vibrates will create a sound. The sound could be musical or it could be noisy; but regardless of its quality, the sound was created by a vibrating object.

The fundamental tone, often referred to simply as the fundamental and abbreviated f_0 , is the lowest frequency in a harmonic series.

The fundamental frequency (also called a natural frequency) of a periodic signal is the inverse of the pitch period length. The pitch period is, in turn, the smallest repeating unit of a signal. One pitch period thus describes the periodic signal completely. The significance of defining the pitch period as the smallest repeating unit can be appreciated by noting that two or more concatenated pitch periods form a repeating pattern in the signal. However, the concatenated signal unit obviously contains redundant information.

A ‘fundamental bass’ is the root note, or lowest note or pitch in a chord or sonority when that chord is in root position or normal form.

In terms of a superposition of sinusoids (for example, Fourier series), the fundamental frequency is the lowest frequency sinusoidal in the sum.

To find the fundamental frequency of a sound wave in a tube that has a closed end you will use the equation:

$$F = \frac{V}{4L}$$

To find L you will use:

$$L = \frac{\lambda}{4}$$

A key aspect of structural dynamic analysis concerns the behavior of a structure at “resonance.” The natural frequency of vibration of a structure --- whether a wood-frame house or a radio tower --- corresponds to that structure’s resonant frequency. If a structure is subjected to

vibration at its natural frequency, the displacements of that structure will reach a maximum (“resonance”). The greater the displacements, greater is the stresses that are developed in the framing members and connections of the structure.

The natural frequencies of vibration of a building depend on its mass and its stiffness (or how flexible it is).

The natural frequency for each mode of vibration follows this rule:

$$f = \frac{1}{2\pi} \sqrt{\frac{K}{M}}$$

f = natural frequency in Hertz.

K = the stiffness of the building associated with this mode

M = the mass of the building associated with this mode

Buildings tend to have lower natural frequencies when they are:

- Either heavier (more mass)
- Or more flexible (that is less stiff).

One of the main things that affect the stiffness of a building is its height. Taller buildings tend to be more flexible, so they tend to have lower natural frequencies compared to shorter buildings.

4.2 DYNAMIC LOAD AND D’ALEMBERT PRINCIPLE

When we come to study vibrations, what we have is not static load but dynamic load. The very nature of the load means that the response of the structure i.e the displacement, stress, reactions etc. also varies with time.

The fundamental difference between static and dynamic loads is that acceleration of the body must be taken into account. From Newton's Law, we know that any force produces acceleration, however in statics we assumed that the load had been applied slowly and whatever effects were

to take place are over and the body is now in equilibrium. This cannot be assumed under dynamic load.

The solution to this problem was proposed by D'Alembert who said that the acceleration can be treated as another force acting opposite to the acceleration and of magnitude the mass times the acceleration. This force is called the inertial force. The forces that act on the body can now be treated as a set of forces in equilibrium.

4.2.1 Modes of Vibration:-

Structures can Have More than one Mode of Vibration namely, 1st , 2nd , 3rd , 4th , etc. Lowest Frequency is 1st Mode. Frequency increases with each subsequent mode of vibration

Modes of Vibration

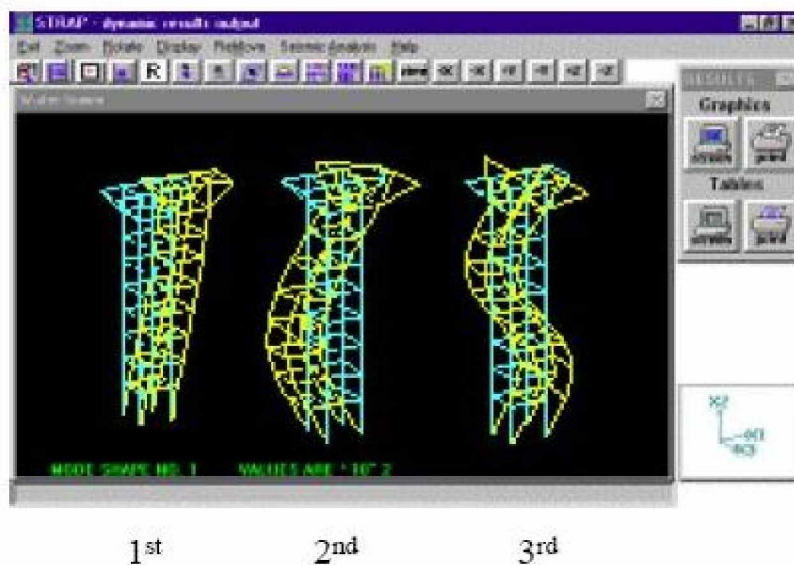


Fig.4.1. Modes of Vibration in Building

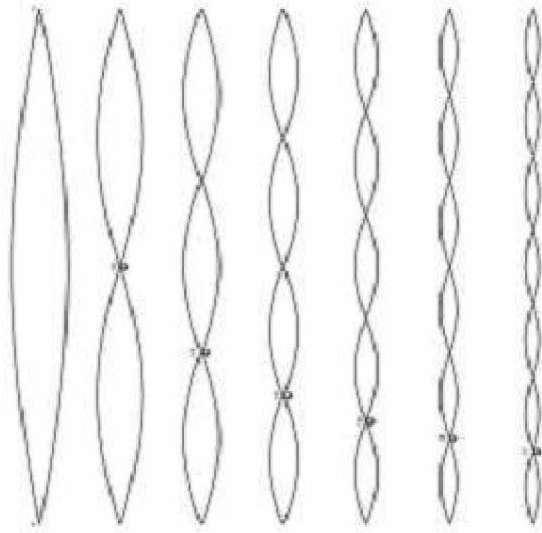


Fig.4.2. Modes of Vibration in String

4.3 DETERMINATION OF THE FUNDAMENTAL FREQUENCY OF A MULTISTOREYED BUILDING

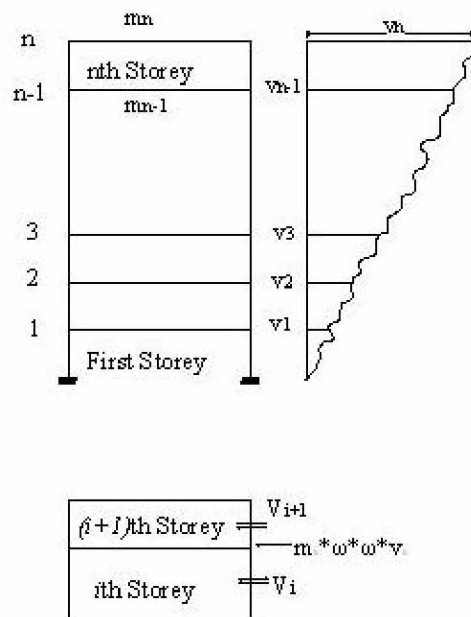


Fig.4.3. Wind Load on Multi-Storeyed Building

Consider a shear frame as shown consisting of n storey. Consider the free body diagram of the i th storey. Here, m is the mass, K is the stiffness, V the shear, ω the frequency and v gives the load displacements.

Initially, the displacement at the top storey level and the natural frequency are both assumed. By using the transfer matrix method it is possible to find the displacement at the base as well as the shear in the base storey. If the support displacement is not zero, a new value for the natural frequency is assumed and the procedure is repeated till the value for the natural frequency is assumed and the procedure is repeated till we get the value for the base displacement at the base equal to zero.

$$V_i = V_{i+1} + m_i \omega^2 v_i$$

$$v_i - v_{i-1} = V_i / k_i = (V_{i+1} + m_i \omega^2 v_i) / k_i$$

5

TORSIONAL VIBRATION OF A SHAFT

5.1 INTRODUCTION

Torsional vibration is angular vibration of an object--commonly a shaft along its axis of rotation. A ROTATING shaft can exhibit torsional vibration when, because the dynamic coefficient of friction is less than the static value, the frictional force at the bearings supporting the shaft decreases as the shaft begins to rotate. The torque in the shaft when it begins to turn is greater than is required to overcome the running friction in the bearings, and the shaft accelerates and may overrun the drive until the speed of the shaft is reduced to zero. Then superposed on the steady rotation of the shaft there is a vibration of an amplitude such that the instantaneous angular velocity of the shaft varies between zero and rather more than twice the mean value.

5.2 SOLVING USING TRANSFER MATRIX METHOD

Torsional vibration of the shaft can be solved by the transfer matrix method by applying the following equation

$$\begin{Bmatrix} \theta \\ T \end{Bmatrix}_n^R = \begin{pmatrix} 1 & \frac{1}{k_n} \\ 0 & 1 \end{pmatrix} \begin{Bmatrix} \theta \\ T \end{Bmatrix}_{n-1}^R \qquad \begin{Bmatrix} \theta \\ T \end{Bmatrix}_n^R = \begin{pmatrix} 1 & 0 \\ -\omega^2 J_n & 1 \end{pmatrix} \begin{Bmatrix} \theta \\ T \end{Bmatrix}_{n-1}^R$$

where J_n =Rotary inertia of the rotor n

k_n =stiffness of the shaft connecting (n-1) to n rotors.

θ_n =Twist at rotor n

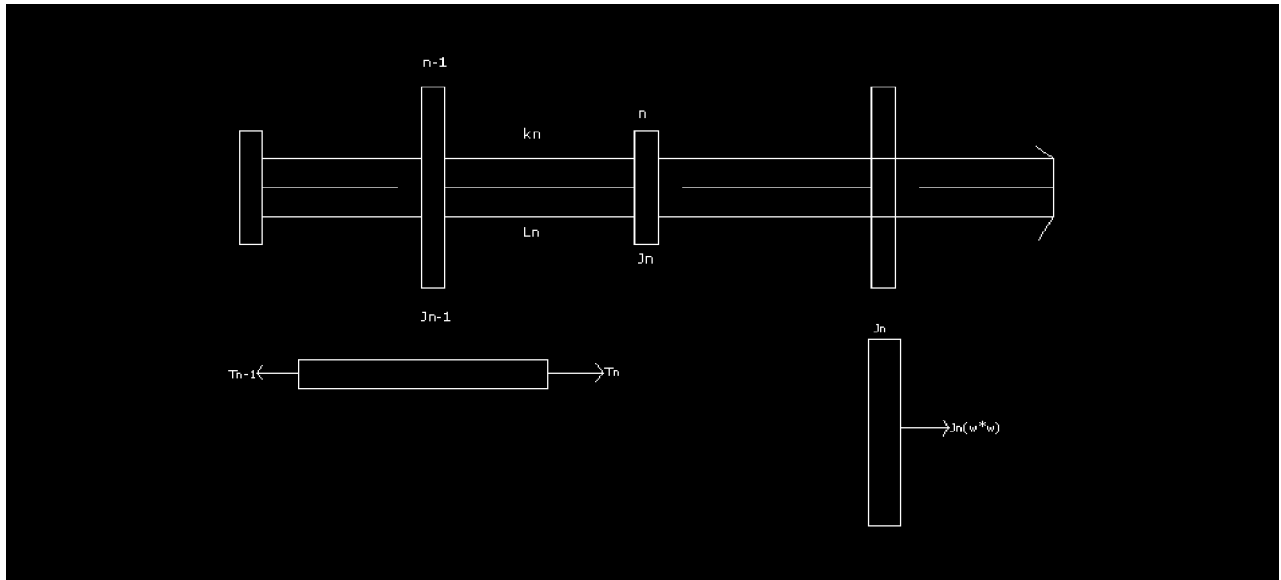


Fig.5.1. Torsional Vibration of Shaft

Combining we get

$$\begin{Bmatrix} \theta \\ T \end{Bmatrix}_n^R = \begin{pmatrix} 1 & \frac{1}{k_n} \\ -\omega^2 J_n & \left(1 - \frac{\omega^2 J_n}{k_n}\right) \end{pmatrix} \begin{Bmatrix} \theta \\ T \end{Bmatrix}_{n-1}^R$$

6

COMPUTER PROGRAMMING AND RESULTS

6.1 FUNDAMENTAL FREQUENCY OF SINGLE BAY MULTI-STOREY BUILDING

6.1.1 Program

```
#include<iostream.h>
#include<conio.h>
void main()
{
    double mass[10], stiffness[10], V[10], v[10], m=1.0, n=1.0;
    int storey=0, i, j=0, k, l=0;
    for(i=0;i<10;i++)
    {
        mass[i]=0.0;
        stiffness[i]=0.0;
        v[i]=0.0;
        V[i]=0.0;
    }
    cout<<"ENTER THE NUMBER OF STOREYS: ";
    cin>>storey;
    for(i=1;i<=storey;i++)
    {
        cout<<"ENTER THE MASS OF BEAM IN STOREY "<<i<<" = ";
        cin>>mass[i];
        cout<<"ENTER THE STIFFNESS OF BEAM IN STOREY "<<i<<" = ";
        cin>>stiffness[i];
    }
    V[storey+1]=0;
    v[storey]=1;
    for(i=0; i<500; i++)
    {
        j=0;
```

```

while (j<storey)
    {
    k=storey-j;
    V[k]=(V[k+1])+(mass[k]*i*v[k]);
    v[k-1]=((-1)*V[k+1]/stiffness[k])+v[k]-(mass[k]*i*v[k]/stiffness[k]);
    j=j+1;
    m=v[0];
    }
if((n/m)<0)
    cout<<i-1;
n=m;
}
}

```

6.1.2 Flowchart

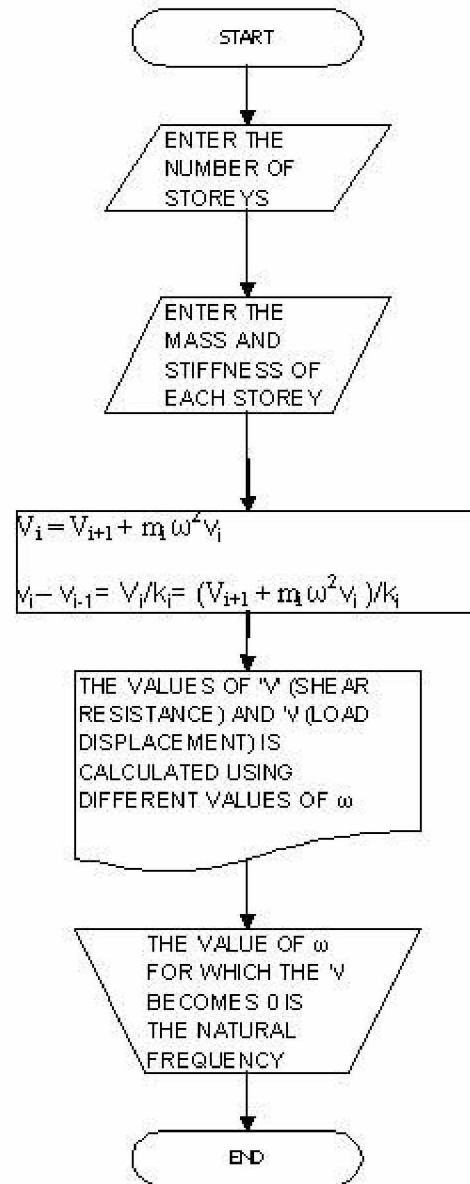


Fig.6.1. Flowchart for calculating the Fundamental Frequency of a Multi-Storeyed Building

6.1.3 Result

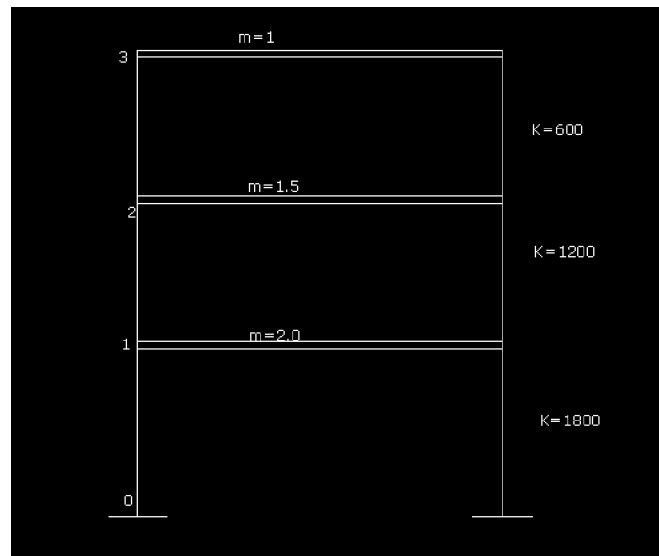


Fig.6.2. Problem Statement

SOLUTION-

Natural Frequency=209 Hz

From Program

```
(Inactive G:\PROJECTWIBRAT-1\WIBRATIO.EXE)
ENTER THE NUMBER OF STOREYS: 3
ENTER THE MASS OF BEAM IN STOREY 1 = 2
ENTER THE STIFFNESS OF BEAM IN STOREY 1 = 1800
ENTER THE MASS OF BEAM IN STOREY 2 = 1.5
ENTER THE STIFFNESS OF BEAM IN STOREY 2 = 1200
ENTER THE MASS OF BEAM IN STOREY 3 = 1
ENTER THE STIFFNESS OF BEAM IN STOREY 3 = 600
THE NATURAL FREQUENCY 210
```

Fig.6.3. Solution obtained from Program

6.2 FUNDAMENTAL FREQUENCY OF MULTIPLE BAY MULTIPLE-STOREY BUILDING

6.2.1 Program

```
#include<iostream.h>
#include<conio.h>
void main()
{
    double m[10][10], ks[10][10], mass[10], stiffness[10], V[10], v[10], p=1.0;
    double n=1.0, tempm, tempk, tempmk, t;
    int storey=0, bay=0, i, j=0, k, q, r;
    for(i=1;i<=10;i++)
        for(q=1;q<=10;q++)
            {
                m[i][q]=0.0;
                ks[i][q]=0.0;
                mass[i]=0.0;
                stiffness[i]=0.0;
                v[i]=0.0;
                V[i]=0.0;
            }
    cout<<"ENTER THE NUMBER OF STOREYS: ";
    cin>>storey;
    cout<<"ENTER THE NUMBER OF BAYS: ";
    cin>>bay;
    for(i=1;i<=storey;i++)
        for(q=1;q<=bay;q++)
            {
                cout<<"ENTER THE MASS OF BEAM IN STOREY "<<i<<" BAY
"<<q<<" = ";
                cin>>m[i][q];
```



```

        cout<<"ENTER THE STIFFNESS OF BEAM IN STOREY "<<i<<"
BAY "<<q<<" = ";
        cin>>ks[i][q];
    }
    for(i=1;i<=storey;i++)
    {
        for(q=1;q<=bay;q++)
        {
            mass[i]=mass[i]+(m[i][q]/bay);
        }
    }
    for(i=1;i<=storey;i++)
    {
        tempm=0.0;
        tempk=1.0;
        tempmk=0.0;
        for(q=1;q<=bay;q++)
        {
            tempm=tempm+m[i][q];
            /*cout<<"\n tempm = "<<tempm;*/
            tempk=tempk*ks[i][q];
            /*cout<<"\n tempk = "<<tempk;*/
            t=1.0;
            for(r=1;r<=bay;r++)
            {
                if(q!=r)
                t=t*ks[i][r];
                /*cout<<"\n ks="<<ks[i][r];
                cout<<"\n t = "<<t;*/
            }
            tempmk=tempmk+(m[i][q]*t);

```

```

        }
        stiffness[i]=(tempk*tempm)/tempmk;
        /*cout<<"stiffness"<<stiffness[i];*/
    }
    V[storey+1]=0;
    v[storey]=1;
    for(i=0; i<500; i++)
    {
        j=0;
        while (j<storey)
        {
            k=storey-j;
            V[k]=(V[k+1])+(mass[k]*i*v[k]);
            v[k-1]=((-1)*V[k+1]/stiffness[k])+v[k]-(mass[k]*i*v[k]/stiffness[k]);
            j=j+1;
            p=v[0];
        }
        if((n/p)<0)
            cout<<"THE NATURAL FREQUENCY " <<(i-1);
        n=p;
    }
}

```

6.2.2 Flowchart

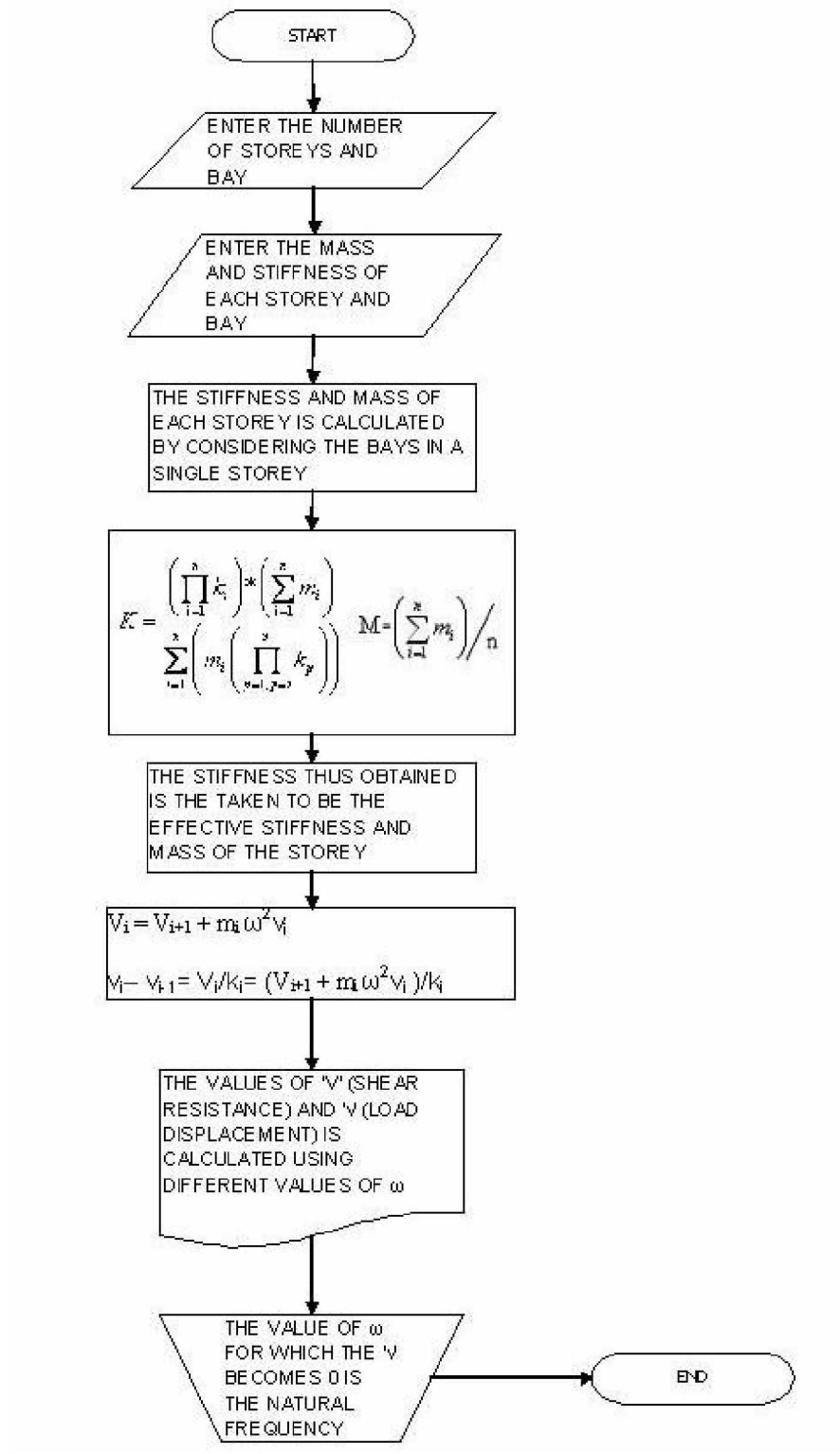


Fig.6.4. Flowchart for calculating the Fundamental Frequency of a Multi-Storeyed Building with Multiple Bays

6.2.3 Result

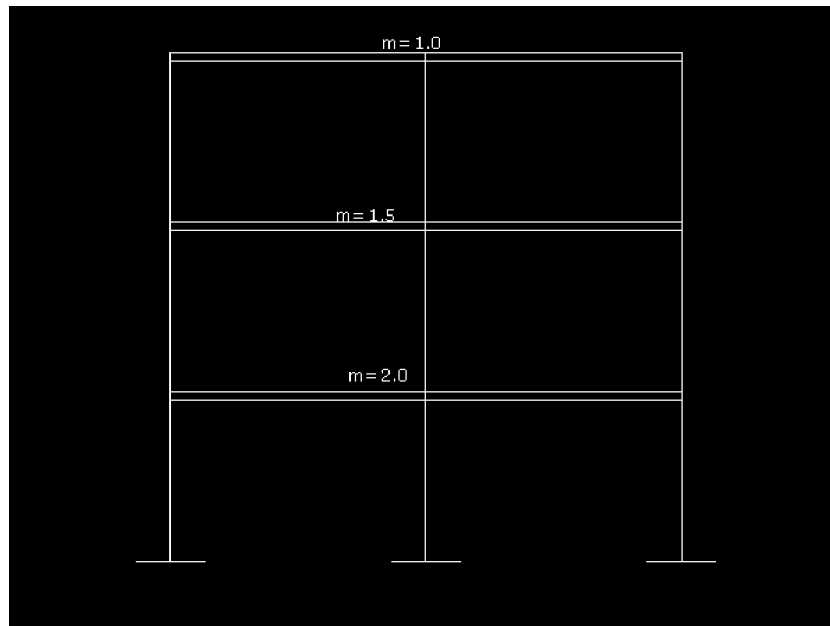


Fig.6.5. Problem Statement

SOLUTION-

Natural Frequency=209 Hz

From Program

```
G:\PROJECT\VIBRAT-1\BAYVIB-1.EXE
ENTER THE NUMBER OF STOREYS: 3
ENTER THE NUMBER OF BAYS: 2
ENTER THE MASS OF BEAM IN STOREY 1 BAY 1 = 2
ENTER THE STIFFNESS OF BEAM IN STOREY 1 BAY 1 = 1800
ENTER THE MASS OF BEAM IN STOREY 1 BAY 2 = 2
ENTER THE STIFFNESS OF BEAM IN STOREY 1 BAY 2 = 1800
ENTER THE MASS OF BEAM IN STOREY 2 BAY 1 = 1.5
ENTER THE STIFFNESS OF BEAM IN STOREY 2 BAY 1 = 1200
ENTER THE MASS OF BEAM IN STOREY 2 BAY 2 = 1.5
ENTER THE STIFFNESS OF BEAM IN STOREY 2 BAY 2 = 1200
ENTER THE MASS OF BEAM IN STOREY 3 BAY 1 = 1
ENTER THE STIFFNESS OF BEAM IN STOREY 3 BAY 1 = 600
ENTER THE MASS OF BEAM IN STOREY 3 BAY 2 = 1
ENTER THE STIFFNESS OF BEAM IN STOREY 3 BAY 2 = 600
THE NATURAL FREQUENCY 210
```

Fig.6.6. Solution obtained from Program

6.3 TORSIONAL VIBRATION OF SHAFT

6.3.1 Program

```
#include<iostream.h>
#include<conio.h>
#include<math.h>
#include<stdio.h>
#include "nrutil.h"
#include "nrutil.c"

void matrix_mult( float **q1, float **q2, float **ans, int r1, int c1, int r2, int c2)
{
    int i,j,k;
    if(c1!=r2)
        cout<<"\nTHE MATRIX MULTIPLICATION IS NOT POSSIBLE\n";
    else
        for(i=1;i<=r1;i++)
            for(j=1;j<=c2;j++)
                {
                    ans[i][j]=0;
                    for(k=1;k<=c1;k++)
                        ans[i][j]=ans[i][j]+(q1[i][k]*q2[k][j]);
                }
}

void main()
{
    double J[10], ks[10], Th[10], T[10], m=1.0, n=1.0, fin;
    int rotor=0, i, j, w, f, k;
    float **a, **b, **c;
    a = matrix(1,2,1,2);
    b = matrix(1,2,1,2);
    c = matrix(1,2,1,2);
```

```

for(i=1;i<=10;i++)
    {
        J[i]=0.0;
        ks[i]=0.0;
        Th[i]=0.0;
        T[i]=0.0;
    }

cout<<"ENTER THE NUMBER OF ROTOR: ";
cin>>rotor;
for(i=1;i<=rotor;i++)
    {
        cout<<"THE ROTARY INERTIA OF ROTOR NO."<<i<<" = ";
        cin>>J[i];
    }

for(i=2;i<=rotor;i++)
    {
        cout<<"THE STIFFNESS OF SHAFT NO."<<i<<" = ";
        cin>>ks[i];
    }

for(w=10;w<=1000;w++)
    {
        a[1][1]=1.0;
        a[1][2]=0.0;
        a[2][1]=0.0;
        a[2][2]=1.0;
        for(i=rotor;i>=2;i--)
            {
                b[1][1]=1;
                b[1][2]=1/ks[i];
                b[2][1]=-1*w*w*J[i];
            }
    }

```

```

b[2][2]=1-(w*w*J[i]/ks[i]);
matrix_mult( a, b, c, 2, 2, 2, 2);
for(k=1;k<=2;k++)
    for(j=1;j<=2;j++)
        {
            a[k][j]=c[k][j];
        }
    }
fin=(a[2][1]-a[2][2]*w*w*J[1])/10000;
/*cout<<"\n fin " <<w<<"=" <<abs(fin);*/
if((abs(fin))<1)
    cout<<"\n THE NATURAL FREQUENCY IS " <<w;
}
}

```

6.3.2 Flowchart

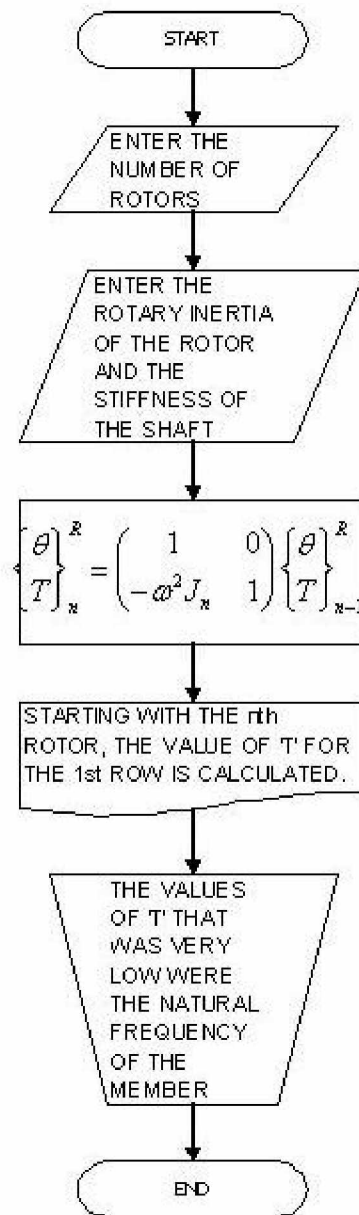


Fig 6.7. Flowchart for calculating the Fundamental Frequency of a shaft with torsional forces acting on it

6.3.3 Result

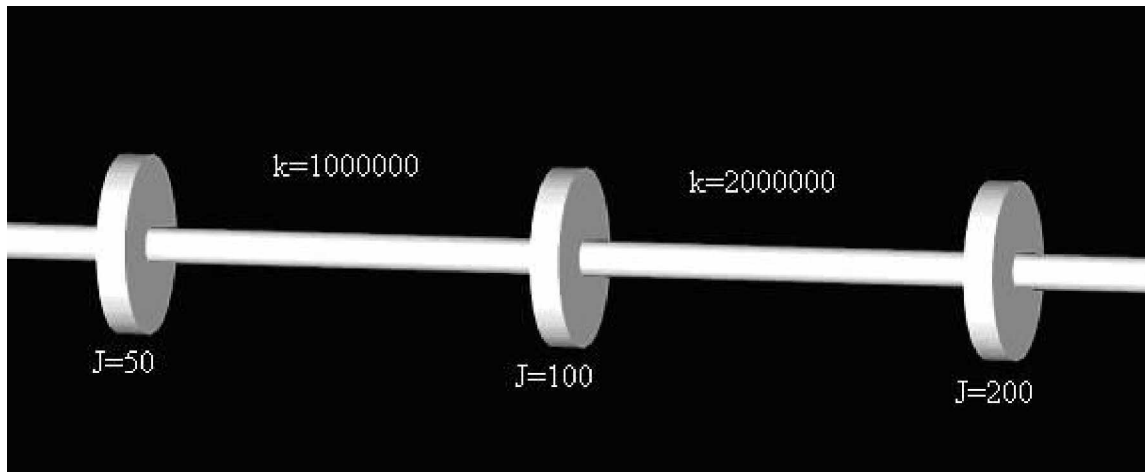


Fig.6.8. Problem Statement

SOLUTION-

Natural Frequency-126 Hz

From Program

```
(Inactive G:\PROJECT\TORSION\TORSION.EXE)
ENTER THE NUMBER OF ROTOR: 3
THE ROTARY INERTIA OF ROTOR NO.1 = 50
THE ROTARY INERTIA OF ROTOR NO.2 = 100
THE ROTARY INERTIA OF ROTOR NO.3 = 200
THE STIFFNESS OF SHAFT NO.2 = 1000000
THE STIFFNESS OF SHAFT NO.3 = 2000000

THE NATURAL FREQUENCY IS 126
```

Fig.6.9. Solution obtained from Program

6.4 ANALYSIS OF A BEAM SECTION

6.4.1 Program

```
#include<iostream.h>
#include<conio.h>
#include<math.h>
#include "nrutil.h"
#include "nrutil.c"
struct uniform_load
    {
        float w, epoa, spoa;
    };
uniform_load ul[10];
struct concentrated_load
    {
        float W, poa;
    };
concentrated_load cl[10];
void matrix_multiplication( float **q1, float **q2, float *ans, int r1, int c1, int r2, int c2=1)
    {
        int i,j,k;
        if(c1!=r2)
            cout<<"\nTHE MATRIX MULTIPLICATION IS NOT POSSIBLE\n";
        else
            for(i=1;i<=r1;i++)
                for(j=1;j<=c2;j++)
                    {
                        ans[i]=0;
                        for(k=1;k<=c1;k++)
                            ans[i]=ans[i]+(q1[i][k]*q2[k][j]);
                    }
    }
```

```
}
```

```
void matrix_addition( float *q1, float *q2, float *ans, int r1, int c1, int r2, int c2=1)
```

```
{
```

```
int i,j;
```

```
if(r1!=r2 && c1!=c2)
```

```
    cout<<"\nTHE MATRIX ADDITION IS NOT POSSIBLE\n";
```

```
else
```

```
for(i=1;i<=r1;i++)
```

```
    ans[i]=q1[i]+q2[i];
```

```
}
```

```
float swappingrowsfn(float **mat, int row, int order)
```

```
{
```

```
    float delement,tempvar;
```

```
    int var=0;
```

```
for(int i=row;i<order;i++)
```

```
{
```

```
    if(mat[i][row]!=0)
```

```
    {
```

```
        var=1;
```

```
        for(int j=0;j<order;j++)
```

```
        {
```

```
            tempvar=mat[row][j];
```

```
            mat[row][j]=mat[i][j];
```

```
            mat[i][j]=tempvar;
```

```
        }
```

```
    }
```

```
if(var==1)
```

```
break;
```

```
}
```

```

delement=mat[row][row];cout<<"\n"<<delement;
return(delement);
}
void g_valfunc( float **g_val, float l, float ei)
{
int i,j;
for(i=1;i<=4;i++)
    for(j=1;j<=4;j++)
        g_val[i][j]=0.0;
g_val[1][1]=1.0;
g_val[1][2]=1;
g_val[1][3]=-1*l/(2*ei);
g_val[1][4]=-1*l*l/(6*ei);
g_val[2][2]=1.0;
g_val[2][3]=-1*l/(ei);
g_val[2][4]=-1*l/(2*ei);
g_val[3][3]=1.0;
g_val[3][4]=1;
g_val[4][4]=1.0;
}
void z_valfunc(float *z, float d, float s, float m, float r)
{
z[1]=d;
z[2]=s;
z[3]=m;
z[4]=r;
}
void z_valfuncforc(float **z, float w)
{
int i;
for(i=1;i<4;i++)

```

```

        z[i][1]=0;
        z[4][1]=-1.0*w;
    }
void z_valfuncforu(float *z, float w, float l, float ei)
    {
    z[1]=w*1*1*1/(24*ei);
    z[2]=w*1*1*1/(6*ei);
    z[3]=-1.0*w*1*1/2;
    z[4]=-1.0*w*1;
    }
void main()
    {
    int flag1, i=0, j=0, k=1, bar=0, n, u=1, c=1, x, y, p, q, con=1, z, l, m;
    char flag2='y';
    float len, leng, length[10], EI[10], tl=0, ch, **Z, **G, *cons;
    float **equa, **inv, **Zfor, answ, temp, *temp1, *constant;
    for(i=1;i<=10;i++)
        {
        length[i]=0;
        EI[i]=0;
        }
    while (flag2=='y' || flag2=='Y')
        {
        bar=bar+1;
        cout<<"ENTER THE LENGTH: ";
        cin>>length[bar];
        length[bar]=length[bar]*1.0;
        cout<<"ENTER THE VALUE OF EI: ";
        cin>>EI[bar];
        EI[bar]=EI[bar]*1.0;
        cout<<"DO YOU WANT TO ENTER MORE (Y/N): ";

```

```

        cin>>flag2;
    }
    for(i=1;i<=10;i++)
        t1=t1+length[i];
    n=bar+1;
    flag2='y';
    cout<<" :ENTER THE LOAD DETAILS:";
    while (flag2=='y' || flag2=='Y')
    {
        cout<<"\n ENTER YOUR PREFERENCE: \n";
        cout<<"1 FOR UDL \n2 FOR POINT LOAD: ";
        cin>>flag1;
        switch(flag1)
        {
            case(1): cout<<"ENTER THE UNIFORM LOAD: ";
                    cin>>ul[u].w;
                    cout<<"ENTER THE STARTING POINT OF
APPLICATION: ";
                    cin>>ul[u].spoa;
                    cout<<"ENTER THE END POINT OF
APPLICATION: ";
                    cin>>ul[u].epoa;
                    u=u+1;
                    break;
            case(2): cout<<"ENTER THE LOAD: ";
                    cin>>cl[c].W;
                    cout<<"ENTER THE POINT OF APPLICATION:
";
                    cin>>cl[c].poa;
                    c=c+1;
                    break;

```

```

        default:      cout<<"WRONG OPTION";
                    break;
    }
    cout<<"DO YOU WANT TO ENTER MORE (Y/N): ";
    cin>>flag2;
}
Z = matrix(0,bar,1,4);
equa = matrix(1,n,1,n);
inv = matrix(1,n,1,n);
G = matrix(1,4,1,4);
cons = vector(1,n);
temp1 = vector(1,4);
Zfor = matrix(1,4,1,1);
constant = vector(1,4);
for(i=1; i<=n; i++)
    for(j=0; j<=n; j++)
        equa[i][j]=0.0;
cout<<"ENTER 1 IF THE STARTING POINT IS SIMPLY SUPPORTED\n";
cout<<"ENTER 2 IF THE STARTING POINT IS FIXED\n";
cin>>flag1;
switch(flag1)
{
    case(1): z_valfunc(Z[0], 0, 1, 0, 1);
            break;
    case(2): z_valfunc(Z[0], 0, 0, 1, 1);
            break;
    default: cout<<"WRONG OPTION";
}
cout<<"ENTER 1 IF THE END POINT IS SIMPLY SUPPORTED\n";
cout<<"ENTER 2 IF THE END POINT IS FIXED\n";
cin>>flag1;

```

```

switch(flag1)
{
case(1): z_valfunc(Z[bar], 0, 1, 0, 1);
        break;
case(2): z_valfunc(Z[bar], 0, 0, 1, 1);
        break;
default: cout<<"WRONG OPTION";
}
p=0;
for(i=1;i<bar;i++)
    z_valfunc(Z[i], 0, 0, 0, 1);
for(i=1;i<=bar;i++)
{
    leng=0.0;
    j=1;
k=1;

    for(m=1;m<=4;m++)
        constant[m]=0.0;
    for(m=1;m<=i;m++)
        leng=leng+length[m];
    while (leng>ul[j].spoa)
    {
        if(leng>ul[j].epoa)
            len=ul[j].epoa-ul[j].spoa;
        else
            len=leng-ul[j].spoa;
        z_valfuncforu(temp1, ul[j].w, len, EI[i]);
        matrix_addition(temp1, constant, constant, 4, 1, 4, 1);
        j++;
        /*for(m=1;m<=4;m++)
            cout<<"\n CONS["<<m<<"]="<<constant[m];*/

```



```

    }
while (leng>cl[k].poa)
    {
    len=leng-cl[k].poa;
    g_valfunc(G, len, EI[i]);
    z_valfuncforc(Zfor, cl[k].W);
    matrix_multiplication(G, Zfor, temp1, 4, 4, 4, 1);
    matrix_addition(temp1, constant, constant, 4, 1, 4, 1);
    k++;
    /*for(m=1;m<=4;m++)
        cout<<"\n CONS["<<m<<"]="<<constant[m];*/
    }
x=1;
do
    {
    if(Z[i][x]==0.0)
        {
        cons[con]=-1*constant[x];
        con++;
        }
    x++;
    }
while (i==bar && x<=4);
/*for(m=1;m<=n;m++)
    cout<<"\n CONS["<<m<<"]="<<cons[m];*/
for(l=i; l>=1; l--)
    {
    len=0;
    for(m=1; m<=l; m++)
        len=len+length[m];

```

```

g_valfunc(G, len, EI[l]);
for(m=1; m<=4; m++)
    temp1[m]=Z[i-1][m];
x=1;
do
    {
    if (Z[i][x]==0)
        {
        q=0;
        p++;
        for(y=1; y<=4; y++)
            {
            if (temp1[y]!=0)
                {
                q++;
                if (p<=n)
                    equa[p][q]=G[x][y];
                else
                    if (p>n)
                        equa[p+q-n][n]=G[x][y];
                }
            }
        }
        x++;
    }
while (i==bar && x<=4);
}
}
/*for(i=1;i<=n;i++)
    for(j=1;j<=n;j++)
        cout<<"\n eq["<<i<<"]["<<j<<"]="<<equa[i][j];*/

```

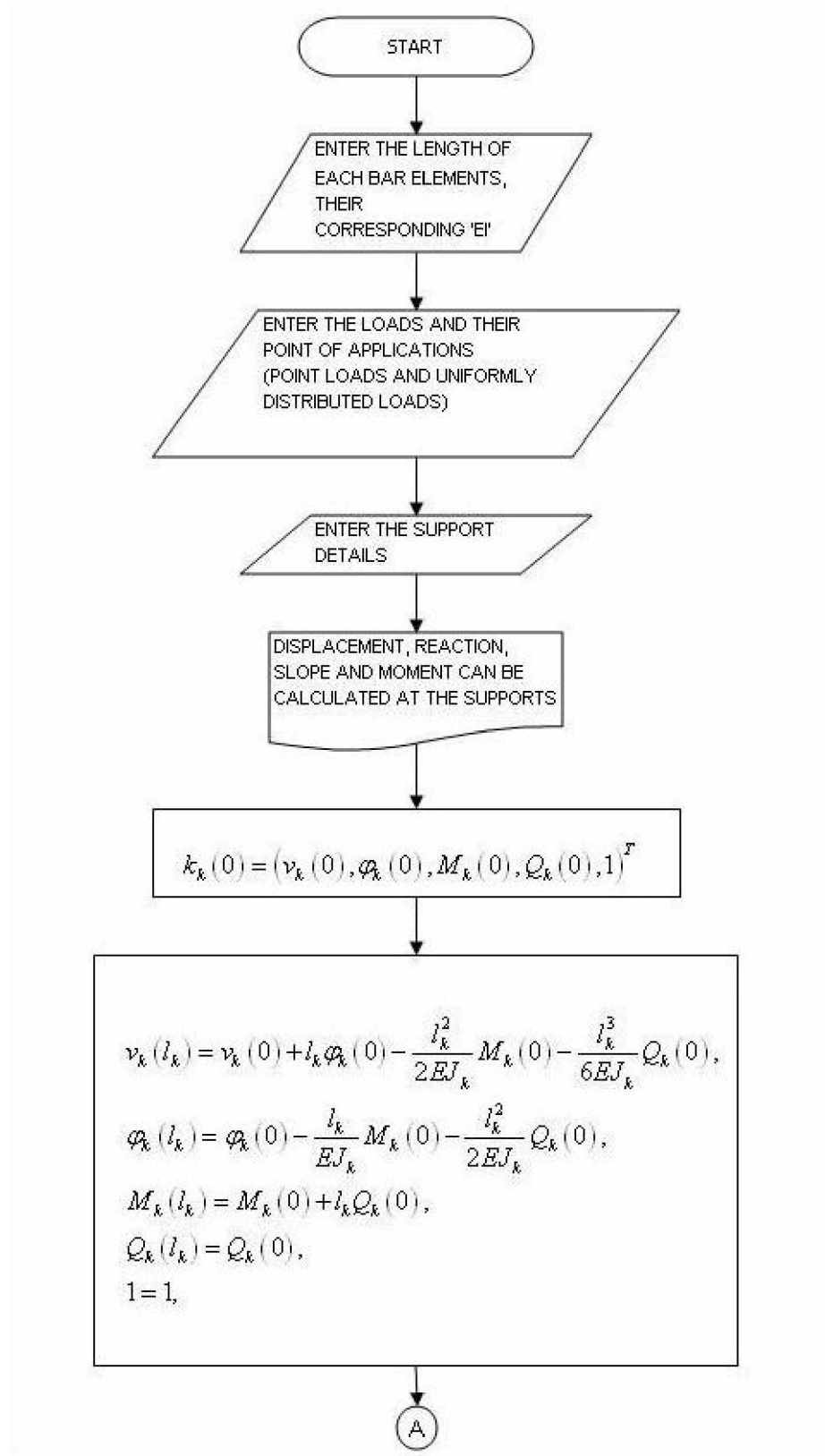
```

for(x=1; x<=n; x++)
    for(y=1; y<=n; y++)
        inv[x][y]=0.0;
for(x=1; x<=n; x++)
    inv[x][x]=1.0;
for(x=1; x<=n; x++)
    {
    temp=equa[x][x];
    if(equa[x][x]==0)
        temp=swappingrowsfn(equa,x,n);
    if(temp==0)
        break;
    for(y=1; y<=n; y++)
        {
        equa[x][y]= (equa[x][y])/temp;
        inv[x][y]= (inv[x][y])/temp;
        }
    for(y=1; y<=n; y++)
        {
        if (y!=x)
            {
            temp=equa[y][x];
            for(z=1; z<=n; z++)
                {
                equa[y][z]=equa[y][z]-temp*equa[x][z];
                inv[y][z]=inv[y][z]-temp*inv[x][z];
                }
            }
        }
    }
if(temp==1)

```

```
        cout<<"\nThe solution does not exist.";
for(i=1; i<=n; i++)
    {
    answ=0.0;
    for(j=1; j<=n; j++)
        answ= answ + ((inv[i][j])*(cons[j]));
    cout<<"\nTHE ANSWER IS: "<<answ;
    }
}
```

6.4.2 Flowchart



(A)

$$BK = \begin{pmatrix} 1 & l_k & -\frac{l_k^2}{2EJ_k} & -\frac{l_k^3}{6EJ_k} \\ 0 & 1 & -\frac{l_k}{EJ_k} & -\frac{l_k^2}{2EJ_k} \\ 0 & 0 & 1 & l_k \\ 0 & 0 & 0 & 1 \end{pmatrix}$$

$$k_k(l_k) = BK * k_k(0),$$

$$\begin{bmatrix} v_k(l_{k+1}) \\ \varphi_k(l_{k+1}) \\ M_k(l_{k+1}) \\ Q_k(l_{k+1}) \\ 1 \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} v_k(l_k) \\ \varphi_k(l_k) \\ M_k(l_k) \\ Q_k(l_k) \\ 1 \end{bmatrix}$$

$$CK = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

(B)

ⓑ

$$\begin{aligned}v_0(0) &= 0, \\ \varphi_0(0) &= ?, \\ M_0(0) &= 0, \\ Q_0(0) &= ?,\end{aligned}$$

$$u_0 = \begin{bmatrix} \varphi_0(0) \\ Q_0(0) \\ 1 \end{bmatrix}$$

$$k_0 = HL * u_0,$$

$$HL = \begin{bmatrix} 0 & 0 & 0 \\ 1 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$\begin{aligned}v_x(L) &= 0, \\ \varphi_x(L) &= ?, \\ M_x(L) &= 0, \\ Q_x(L) &= ?,\end{aligned}$$

ⓒ

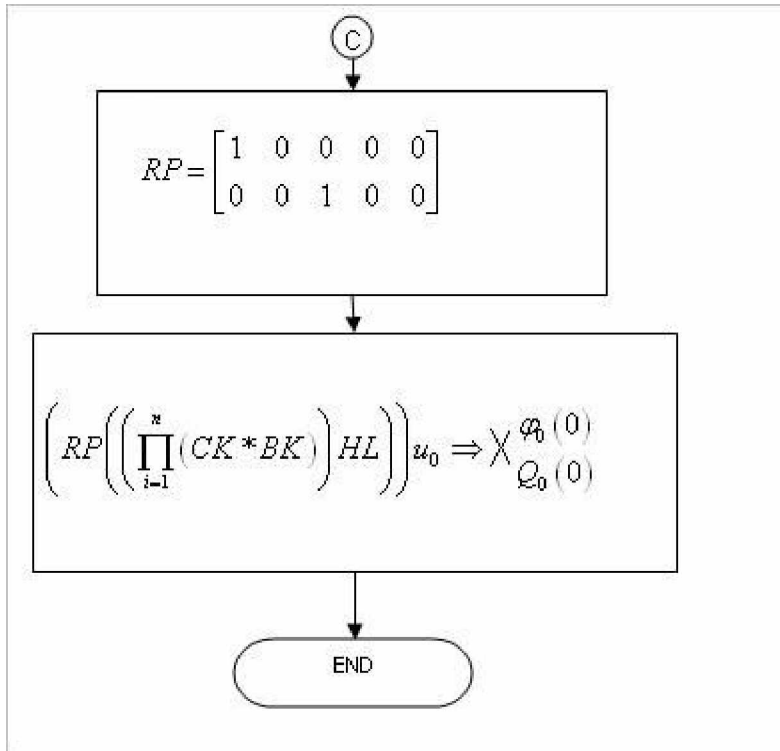


Fig 6.10. Flowchart for calculating the slope, displacement, moment and reaction of a beam section with a single cross-sectional area

6.4.3 Result

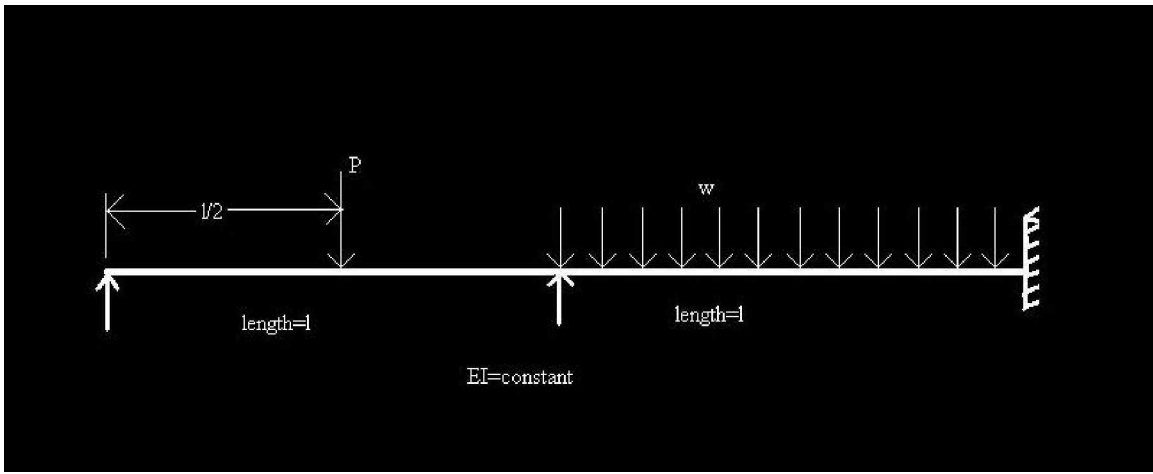


Fig.6.11. Problem Statement

SOLUTION-

$$\theta_0 = 43.7085$$

$$Q_0 = 3.04285 \text{ kN}$$

$$R_B = 18.8213 \text{ kN}$$

From Program

```
G:\PROJECT\TRANSF-1\MODIFIED.EXE
ENTER THE LENGTH: 12
ENTER THE VALUE OF EI: 1
DO YOU WANT TO ENTER MORE (Y/N): y
ENTER THE LENGTH: 12
ENTER THE VALUE OF EI: 1
DO YOU WANT TO ENTER MORE (Y/N): n
:ENTER THE LOAD DETAILS:
ENTER YOUR PREFERENCE:
1 FOR UDL
2 FOR POINT LOAD: 1
ENTER THE UNIFORM LOAD: 2
ENTER THE STARTING POINT OF APPLICATION: 12
ENTER THE END POINT OF APPLICATION: 24
DO YOU WANT TO ENTER MORE (Y/N): y

ENTER YOUR PREFERENCE:
1 FOR UDL
2 FOR POINT LOAD: 2
ENTER THE LOAD: 10
ENTER THE POINT OF APPLICATION: 6
DO YOU WANT TO ENTER MORE (Y/N): n
ENTER 1 IF THE STARTING POINT IS SIMPLY SUPPORTED
ENTER 2 IF THE STARTING POINT IS FIXED
1
ENTER 1 IF THE END POINT IS SIMPLY SUPPORTED
ENTER 2 IF THE END POINT IS FIXED
2

THE ANSWER IS: 43.7143
THE ANSWER IS: 3.07143
THE ANSWER IS: 18.8214
```

Fig.6.12. Solution obtained from Program

6.5 ANALYSIS OF A BEAM SECTION WITH UNEQUAL SECTIONS AND WITH STIFFNERS

6.5.1 Program

```
#include<iostream.h>
#include<conio.h>
#include<math.h>
#include "nrutil.h"
#include "nrutil.c"
int n;
float length[10], EI[10], tl=0;
struct uniform_load
    {
        float w, epoa, spoa;
    };
uniform_load ul[10];
struct concentrated_load
    {
        float W, poa;
    };
concentrated_load cl[10];
struct support_constraints
    {
        float Hk, hk;
    };
support_constraints sc[10];
void matrix_mult( float **q1, float **q2, float **ans, int r1, int c1, int r2, int c2)
    {
        int i,j,k,q,m;
        if(c1!=r2)
            cout<<"\nTHE MATRIX MULTIPLICATION IS NOT POSSIBLE\n";
```

```

else
for(i=1;i<=r1;i++)
    for(j=1;j<=c2;j++)
        {
            ans[i][j]=0;
            for(k=1;k<=c1;k++)
                ans[i][j]=ans[i][j]+(q1[i][k]*q2[k][j]);
        }
/*for(q=1;q<=4;q++)
    {
        cout<<"\n";
        for(m=1;m<=4;m++)
            cout<<"  q1["<<q<<"]["<<m<<"]="<<q1[q][m];
    }
for(q=1;q<=4;q++)
    {
        cout<<"\n";
        for(m=1;m<=4;m++)
            cout<<"  q2["<<q<<"]["<<m<<"]="<<q2[q][m];
    }
for(q=1;q<=4;q++)
    {
        cout<<"\n";
        for(m=1;m<=4;m++)
            cout<<"  ans["<<q<<"]["<<m<<"]="<<ans[q][m];
    }*/
}

```

```

void matrix_multiplication( float **q1, float **q2, float *ans, int r1, int c1, int r2, int c2)
{
    int i,j,k;
    if(c1!=r2)

```

```

        cout<<"\nTHE MATRIX MULTIPLICATION IS NOT POSSIBLE\n";
else
for(i=1;i<=r1;i++)
    for(j=1;j<=c2;j++)
        {
            ans[i]=0;
            for(k=1;k<=c1;k++)
                ans[i]=ans[i]+(q1[i][k]*q2[k][j]);
        }
}

void matrix_addition( float *q1, float *q2, float *ans, int r1, int c1, int r2, int c2=1)
{
int i;
if(r1!=r2 && c1!=c2)
    cout<<"\nTHE MATRIX ADDITION IS NOT POSSIBLE\n";
else
for(i=1;i<=r1;i++)
    ans[i]=q1[i]+q2[i];
}

void g_valfunction(float **g_val, float l, float ei)
{
int i,j;
for(i=1;i<=4;i++)
    for(j=1;j<=4;j++)
        g_val[i][j]=0.0;

g_val[1][1]=1.0;
g_val[1][2]=l;
g_val[1][3]=-1*l/(2*ei);
g_val[1][4]=-1*l/(6*ei);
g_val[2][2]=1.0;

```

```

    g_val[2][3]=-1*1/(ei);
    g_val[2][4]=-1*1/(2*ei);
    g_val[3][3]=1.0;
    g_val[3][4]=1;
    g_val[4][4]=1.0;
    }
void g_valfunc(float **gvalf, float ep, float sp)
    {
    int i, j, k, m, l, flag=0;
    float len=0, leng=0, temp, le;
float **gv,**gv1,**gv2;
    gv = matrix(1,4,1,4);
    gv1 = matrix(1,4,1,4);
    gv2 = matrix(1,4,1,4);
    for(i=1;i<=4;i++)
        for(j=1;j<=4;j++)
            {
                gvalf[i][j]=0.0;
                gv[i][j]=0.0;
                gv1[i][j]=0.0;
                gv2[i][j]=0.0;
            }
    for(i=1;i<=4;i++)
        {
            gvalf[i][i]=1.0;
            gv[i][i]=1.0;
            gv1[i][i]=1.0;
            gv2[i][i]=1.0;
        }
    for(i=1; i<n; i++)
        {

```

```

len=len+length[i];
if(sp==len)
{
/*cout<<"\n i"<<i;*/
for(l=i; l>0; l--)
{
if(ep<(len-length[l]))
{
gv2[3][2]=sc[l].Hk;
gv2[4][1]=sc[l].hk;
le=length[l];

/*cout<<"l"<<l<<"ep"<<ep<<"sp"<<sp<<"len"<<len<<"le"<<le<<"length"<<length[l]-
1];*/

g_valfunction(gv, le, EI[l]);
for(k=1;k<=4;k++)
for(m=1;m<=4;m++)
cout<<"\n
gv["<<k<<"]["<<m<<"]="<<gv[k][m];

matrix_mult(gv2, gv, gv1, 4, 4, 4, 4);
for(k=1;k<=4;k++)
for(m=1;m<=4;m++)
{
gv[k][m]=gv1[k][m];
/*cout<<"\n
gv["<<k<<"]["<<m<<"]="<<gv[k][m];*/

}

matrix_mult(gv, gvalf, gv1, 4, 4, 4, 4);
for(k=1;k<=4;k++)
for(m=1;m<=4;m++)
gvalf[k][m]=gv1[k][m];

```

```

        }
else
if(flag==0)
{
    for(k=1; k<=4; k++)
        leng=leng+length[k];
    temp=ep-leng;
    gv2[3][2]=sc[l].Hk;
    gv2[4][1]=sc[l].hk;
    g_valfunction(gv, temp, EI[l]);
    matrix_mult(gv2, gv, gv1, 4, 4, 4, 4);
    for(k=1;k<=4;k++)
        for(m=1;m<=4;m++)
            gv[k][m]=gv1[k][m];
    matrix_mult(gv, gvalf, gv1, 4, 4, 4, 4);
    for(k=1;k<=4;k++)
        for(m=1;m<=4;m++)
            gvalf[k][m]=gv1[k][m];
    flag=1;
}
}
}
}

void z_valfunc(float *z, float d, float s, float m, float r)
{
    z[1]=d;
    z[2]=s;
    z[3]=m;
    z[4]=r;
}

```

```

void z_valfuncforc(float **z, float w)
{
    int i;
    for(i=1;i<4;i++)
        z[i][1]=0;
        z[4][1]=-1.0*w;
}

void z_valfuncforu(float *z, float w, float l, float ei)
{
    z[1]=w*1*1*1/(24*ei);
    z[2]=w*1*1*1/(6*ei);
    z[3]=-1.0*w*1*1/2;
    z[4]=-1.0*w*1;
}

void main()
{
    int flag1, i=0, j=0, k=1, bar=0, u=1, c=1, x, y, p, q, con=1, z, l, m, a, b;
    char flag2='y';
    float len, leng, ch, **Z, **G, *cons, *constant, check;
    float **equa, **inv, **Zfor, answ, temp, *temp1;
    for(i=0;i<=10;i++)
        {
            length[i]=0;
            EI[i]=0;
            ul[i].w=0;
            ul[i].epoa=0;
            ul[i].spoa=0;
            cl[i].W=0;
            cl[i].poa=0;
            sc[i].Hk=0;
            sc[i].hk=0;
        }
}

```



```

    }
while (flag2=='y' || flag2=='Y')
    {
    bar=bar+1;
    cout<<"ENTER THE LENGTH: ";
    cin>>length[bar];
    length[bar]=length[bar]*1.0;
    cout<<"ENTER THE VALUE OF EI: ";
    cin>>EI[bar];
    EI[bar]=EI[bar]*1.0;
    cout<<"DO YOU WANT TO ENTER MORE (Y/N): ";
    cin>>flag2;
    }
for(i=1;i<=10;i++)
    tl=tl+length[i];
n=bar+1;
flag2='y';
cout<<" :ENTER THE STIFFENERS DETAILS:";
while (flag2=='y' || flag2=='Y')
    {
    cout<<"\n ENTER YOUR PREFERENCE: \n";
    cout<<"1 FOR VERTICAL STIFFENER \n2 FOR ANGULAR STIFFENERS: ";
    cin>>flag1;
    switch(flag1)
        {
        case(1): cout<<"ENTER THE NODE: ";
                    cin>>i;
                    cout<<"ENTER THE VALUE OF VERTICAL
STIFFENER: ";
                    cin>>sc[i].hk;
                    break;

```

```

        case(2): cout<<"ENTER THE NODE: ";
                cin>>i;
                cout<<"ENTER THE VALUE OF ANGULAR
STIFFENER: ";

                cin>>sc[i].Hk;
                break;
        default: cout<<"WRONG OPTION";
                break;
    }
    cout<<"\nDO YOU WANT TO ENTER MORE (Y/N): ";
    cin>>flag2;
}
flag2='y';
cout<<" :ENTER THE LOAD DETAILS:";
while (flag2=='y' || flag2=='Y')
{
    cout<<"\n ENTER YOUR PREFERENCE: \n";
    cout<<"1 FOR UDL \n2 FOR POINT LOAD: ";
    cin>>flag1;
    switch(flag1)
    {
        case(1): cout<<"ENTER THE UNIFORM LOAD: ";
                cin>>ul[u].w;
                cout<<"ENTER THE STARTING POINT OF
APPLICATION: ";

                cin>>ul[u].spoa;
                cout<<"ENTER THE END POINT OF
APPLICATION: ";

                cin>>ul[u].epoa;
                u=u+1;
                break;

```

```

        case(2): cout<<"ENTER THE LOAD: ";
                cin>>cl[c].W;
                cout<<"ENTER THE POINT OF APPLICATION:
";

                cin>>cl[c].poa;
                c=c+1;
                break;

        default: cout<<"WRONG OPTION";
                break;

    }

    cout<<"DO YOU WANT TO ENTER MORE (Y/N): ";
    cin>>flag2;
}

Z = matrix(0,bar,1,4);
equa = matrix(1,n,1,n);
inv = matrix(1,n,1,n);
G = matrix(1,4,1,4);
cons = vector(1,n);
temp1 = vector(1,4);
Zfor = matrix(1,4,1,1);
constant = vector(1,4);
for(i=1; i<=n; i++)
    for(j=0; j<=n; j++)
        equa[i][j]=0.0;

cout<<"ENTER 1 IF THE STARTING POINT IS SIMPLY SUPPORTED\n";
cout<<"ENTER 2 IF THE STARTING POINT IS FIXED\n";
cin>>flag1;
switch(flag1)
{
    case(1): z_valfunc(Z[0], 0, 1, 0, 1);
            break;

```

```

        case(2): z_valfunc(Z[0], 0, 0, 1, 1);
                    break;
        default: cout<<"WRONG OPTION";
    }
cout<<"ENTER 1 IF THE END POINT IS SIMPLY SUPPORTED\n";
cout<<"ENTER 2 IF THE END POINT IS FIXED\n";
cin>>flag1;
switch(flag1)
    {
        case(1): z_valfunc(Z[bar], 0, 1, 0, 1);
                    break;
        case(2): z_valfunc(Z[bar], 0, 0, 1, 1);
                    break;
        default: cout<<"WRONG OPTION";
    }
p=0;
for(i=1;i<bar;i++)
    z_valfunc(Z[i], 0, 0, 0, 1);
for(i=1;i<=bar;i++)
    {
        leng=0.0;
        j=1;
        k=1;
        for(m=1;m<=4;m++)
            constant[m]=0.0;
        for(m=1;m<=i;m++)
            leng=leng+length[m];
        while ((leng>ul[j].spoa) && (check!=ul[j].spoa))
            {
                check=ul[j].spoa;
                if(leng>ul[j].epoa)

```

```

        len=ul[j].epoa-ul[j].spoa;
else
        len=leng-ul[j].spoa;
z_valfuncforu(temp1, ul[j].w, len, EI[i]);
matrix_addition(temp1, constant, constant, 4, 1, 4, 1);
j++;
/*for(m=1;m<=4;m++)
        cout<<"\n CONS["<<m<<"]="<<constant[m];*/
}
while ((leng>cl[k].poa) && (check!=cl[k].poa))
{
check=cl[k].poa;
/*cout<<" poa="<<cl[k].poa<<" length= "<<leng;*/
g_valfunc(G, cl[k].poa, leng);
/*for(a=1;a<=4;a++)
        for(b=1;b<=4;b++)
                cout<<"\n G["<<a<<"]["<<b<<"]="<<G[a][b];*/
z_valfuncforc(Zfor, cl[k].W);
matrix_multiplication(G, Zfor, temp1, 4, 4, 4, 1);
/*for(b=1;b<=4;b++)
        cout<<"\n temp1["<<b<<"]="<<temp1[b];*/
matrix_addition(temp1, constant, constant, 4, 1, 4, 1);
k++;
/*for(m=1;m<=4;m++)
        cout<<"\n CONS["<<m<<"]="<<constant[m];*/
}
x=1;
do
{
if(Z[i][x]==0.0)
{

```

```

        cons[con]=-1*constant[x];
        con++;
    }
    x++;
}
while (i==bar && x<=4);
/*for(m=1;m<=n;m++)
    cout<<"\n CONS["<<m<<"]="<<cons[m];*/
for(l=i; l>=1; l--)
{
    len=0;
    for(m=1; m<=i; m++)
        len=len+length[m];
    g_valfunc(G, length[i-1], len);
    for(m=1; m<=4; m++)
        temp1[m]=Z[i-1][m];
    x=1;
    do
    {
        if (Z[i][x]==0)
        {
            q=0;
            p++;
            for(y=1; y<=4; y++)
            {
                if (temp1[y]!=0)
                {
                    q++;
                    if (p<=n)
                        equa[p][q]=G[x][y];
                }
            }
        }
    }
}

```

```

                if (p>n)
                    equa[p+q-n][n]=G[x][y];
                }
            }
        }
        x++;
    }
    while (i==bar && x<=4);
}
}
/*for(i=1;i<=n;i++)
    for(j=1;j<=n;j++)
        cout<<"\n eq["<<i<<"]["<<j<<"]="<<equa[i][j];*/
for(x=1; x<=n; x++)
    for(y=1; y<=n; y++)
        inv[x][y]=0.0;
for(x=1; x<=n; x++)
    inv[x][x]=1.0;
for(x=1; x<=n; x++)
    {
        temp=equa[x][x];
        for(y=1; y<=n; y++)
            {
                equa[x][y]= (equa[x][y])/temp;
                inv[x][y]= (inv[x][y])/temp;
            }
        for(y=1; y<=n; y++)
            {
                if (y!=x)
                    {
                        temp=equa[y][x];

```

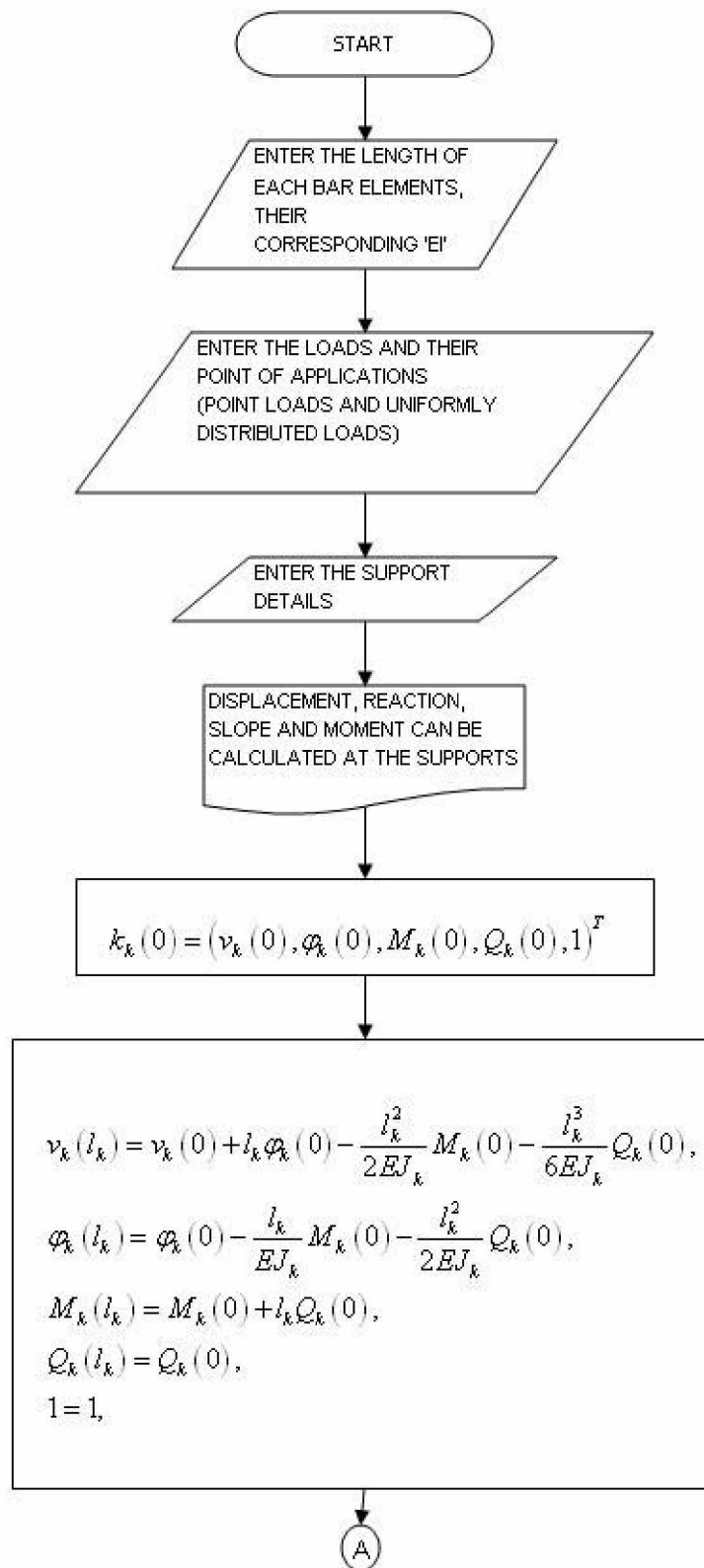
```

        for(z=1; z<=n; z++)
            {
                equa[y][z]=equa[y][z]-temp*equa[x][z];
                inv[y][z]=inv[y][z]-temp*inv[x][z];
            }
    }
}

for(i=1; i<=n; i++)
    {
        answ=0.0;
        for(j=1; j<=n; j++)
            answ= answ + ((inv[i][j])*(cons[j]));
        cout<<"\nTHE ANSWER IS: "<<answ;
    }
}

```


6.5.2 Flowchart



(A)

$$BK = \begin{pmatrix} 1 & l_k & -\frac{l_k^2}{2EJ_k} & -\frac{l_k^3}{6EJ_k} \\ 0 & 1 & -\frac{l_k}{EJ_k} & -\frac{l_k^2}{2EJ_k} \\ 0 & 0 & 1 & l_k \\ 0 & 0 & 0 & 1 \end{pmatrix}$$

$$k_k(l_k) = BK * k_k(0),$$

$$\begin{bmatrix} v_k(l_{k+1}) \\ \varphi_k(l_{k+1}) \\ M_k(l_{k+1}) \\ Q_k(l_{k+1}) \\ 1 \end{bmatrix} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} v_k(l_k) \\ \varphi_k(l_k) \\ M_k(l_k) \\ Q_k(l_k) \\ 1 \end{bmatrix}$$

$$CK = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

(B)

ⓑ

$$\begin{aligned} M_{k+1} &= \pm H_k \varphi_k(l_k) + M_k(l_k), \\ Q_{k+1} &= \pm h_k v_k(l_k) + Q_k(l_k) \pm P_k. \end{aligned}$$

$$CK = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & \pm H_k & 1 & 0 & 0 \\ \pm h_k & 0 & 0 & 1 & \pm P_k \\ 0 & 0 & 0 & 0 & 1 \end{bmatrix}$$

$$k_{k+1}(0) = CK * k_k(l_k).$$

$$\begin{aligned} v_0(0) &= 0, \\ \varphi_0(0) &= ?, \\ M_0(0) &= 0, \\ Q_0(0) &= ?, \end{aligned}$$

$$u_0 = \begin{bmatrix} \varphi_0(0) \\ Q_0(0) \\ 1 \end{bmatrix}$$

$$k_0 = HL * u_0.$$

$$HL = \begin{bmatrix} 0 & 0 & 0 \\ 1 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

ⓒ

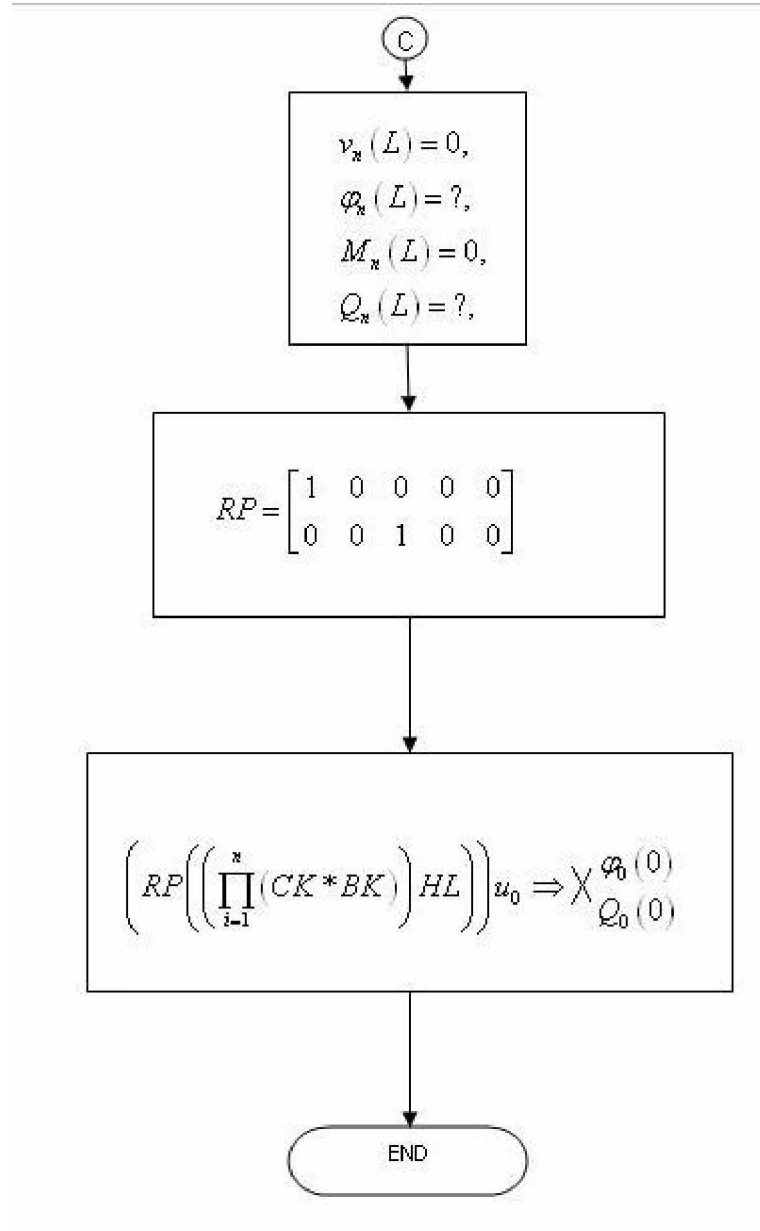


Fig 6.13. Flowchart for calculating the slope, displacement, moment and reaction of a beam section with a various cross-sectional area and having vertical and angular stiffeners

6.5.3 Result

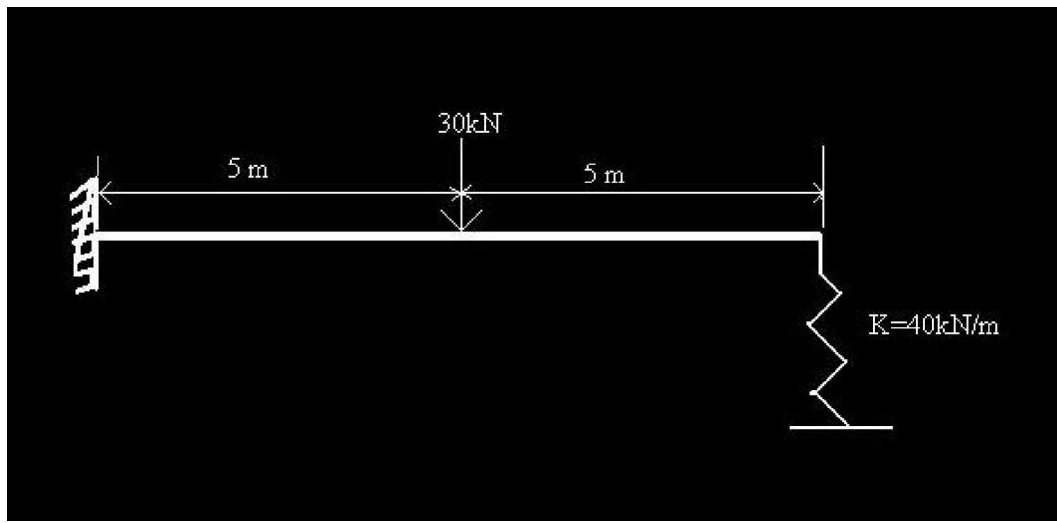


Fig.6.14. Problem Statement

SOLUTION-

$$M=56.2516 \text{ kN-m}$$

$$P=20.615 \text{ kN}$$

From Program

```
G:\PROJECT\TRANSF-1\ADDENDUM.EXE
ENTER THE LENGTH: 10
ENTER THE VALUE OF EI: 12000
DO YOU WANT TO ENTER MORE (Y/N): n
:ENTER THE STIFFENERS DETAILS:
ENTER YOUR PREFERENCE:
1 FOR VERTICAL STIFFENER
2 FOR ANGULAR STIFFENERS: 1
ENTER THE NODE: 2
ENTER THE VALUE OF VERTICAL STIFFENER: 40

DO YOU WANT TO ENTER MORE (Y/N): n
:ENTER THE LOAD DETAILS:
ENTER YOUR PREFERENCE:
1 FOR UDL
2 FOR POINT LOAD: 2
ENTER THE LOAD: 30
ENTER THE POINT OF APPLICATION: 5
DO YOU WANT TO ENTER MORE (Y/N): n
ENTER 1 IF THE STARTING POINT IS SIMPLY SUPPORTED
ENTER 2 IF THE STARTING POINT IS FIXED
2
ENTER 1 IF THE END POINT IS SIMPLY SUPPORTED
ENTER 2 IF THE END POINT IS FIXED
1

THE ANSWER IS: 56.25
THE ANSWER IS: 20.625
```

Fig.6.15. Solution obtained from Program

7

CONCLUSION

Transfer matrix is a powerful tool to solve for solving large type of problem using computer programming. We have can taken advantage of this method of analysis to make problem more simpler. The transfer-matrix method is used when the total system can be broken into a *sequence* of subsystems that interact only with adjacent subsystems. To implement the transfer matrix method, we need a relationship that gives the state of forces and displacements at one end of the element in terms of force and displacement at the other end. Transfer matrix method is an approach to matrix structural analysis that uses a mixed form of the element force-displacement relationship and transfers the structural behavior parameters the joint forces and displacement from one end of the structures of line element to other). An advantage of transfer matrix method is that it produces a system of equations that are to be solved that are quite small in comparison with those produced by the stiffness method. A disadvantage is the extensive sequence of operations that are required on a small matrix.

REFERENCES

- By Rajasekaran S. and Sankarasubramanian G., Computational Structural Mechanics, New Delhi, Prentice-Hall of India Private Limited, 2001.
- By Tesar Alexander and Fillo Ludovit, Transfer Matrix Method, Czechoslovakia, Kluwer Academic Publisher, 1988.
- By Kanetkar Yashavant, Let us C, New Delhi, BPB Publication, 2005.
- By Pandit G.S. and Gupta S.P., Structural Analysis A Matrix Approach, New Delhi, Tata McGraw-Hill Publishing Company Limited, 1981.
- By Kanetkar Yashavant, Pointers in C, New Delhi, BPB Publication, 2005.
- By Kanetkar Yashavant, Data Structure in C, New Delhi, BPB Publication, 2005.
- www.wikipedia.com
- www.howstuffworks.com