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**DETECTION AND DIAGNOSIS
OF CHANGES
IN THE EIGENSTRUCTURE
OF NONSTATIONARY
MULTIVARIABLE SYSTEMS**

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DETECTION AND DIAGNOSIS OF CHANGES IN THE EIGENSTRUCTURE OF NONSTATIONARY MULTIVARIABLE SYSTEMS

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Georges Moustakides (IRISA/INRIA), Anne Rougée (IRISA/Univ.)

Résumé.— On étudie les deux problèmes de détection et diagnostic de changements dans la matrice de transition d'état d'un système multivariable excité par un bruit d'état inconnu et non-stationnaire. On propose des tests nouveaux, dits "instrumentaux", dont on montre numériquement l'efficacité, même pour des changements petits. On décrit enfin l'application de ces méthodes au problème de surveillance des vibrations pour les plateformes offshore.

Abstract.— The two problems of detection and diagnosis of changes in the state transition matrix of a multivariable system with nonstationary unknown state noise, are addressed. New instrumental tests are derived and shown to be numerically powerful, even for small changes. The application to vibration monitoring of offshore platforms is described.

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Abstract. - The two problems of detection and diagnosis of changes in the state transition matrix of a multivariable system with nonstationary unknown state noise, are addressed. New instrumental tests are derived and shown to be numerically powerful, even for small changes. The application to vibration monitoring of offshore platforms is described.

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I. INTRODUCTION

The problem of detecting changes in dynamical systems has received a large attention in Willsky (1976), Mironovskii (1980), Pau (1981), Isermann (1984), M. Basseville, A. Benveniste (Ed.) (1986), in many fields of application, for the purposes of failure detection in controlled systems or signal segmentation for recognition. Most of the time domain model-based methods use the entire set of the known or estimated model parameters for solving the two basic steps of change detection, namely residual generation and choice of the (statistical) decision function (Willsky, 1976). For example, both filter innovations and parity checks involve all the model parameters, with possible inclusion of parameters uncertainties, and on the other hand classical likelihood ratio or bayesian tests proceed similarly.

However, for some applications, it may be necessary to detect changes in one subset of the model parameters without knowing or using any estimates of the parameters belonging to the complementary subset. These last parameters have therefore to be considered as nuisance parameters. In section IV, we will describe such an application concerned with vibration monitoring of a structure subject to nonstationary and unmeasured natural excitation. The related change detection problem may be formulated in the two following equivalent ways :

i) consider a multivariable observable system described by the discrete time state space model :

$$\begin{cases} X_{t+1} = F X_t + V_{t+1} \\ Y_t = H X_t \end{cases}, \quad \text{cov}(V_{t+1}) = Q_t \quad (1)$$

where the dimensions of the state X and the observation Y are n and r respectively, with r (much) smaller than n in practice, and where the additive state noise V_t is an **unmeasured** gaussian process with **time-varying** covariance matrix Q_t . Then the problem is to detect changes in the state transition matrix F , up to a change of basis, without using the nuisance parameters Q_t which may be highly time-varying (see section IV). Here the observation matrix H is assumed to be fixed ; the related problem of optimal sensor location for change detection will be reported elsewhere.

ii) consider a multidimensional ARMA process :

$$Y_t = \sum_{i=1}^p A_i Y_{t-i} + \sum_{j=0}^{p-1} B_j(t) E_{t-j} \quad (2)$$

with **constant** autoregressive $r \times r$ parameters $(A_i)_{1 \leq i \leq p}$ and **time-varying** moving average $r \times r$ parameters $(B_j)_{0 \leq j \leq p-1}$; and where $(E_t)_t$ is a gaussian white noise with identity covariance matrix. The model in (2) may be obtained from (1) in a classical way (Akaike, 1974) by solving the following linear system of equations :

$$HF^p = \sum_{i=1}^p A_i HF^{p-i} \quad (3)$$

Let $\theta^T = (A_p, \dots, A_1)$ (4) be the set of AR parameters.

In this situation, the problem is to detect changes in the AR parameters θ without using the nuisance moving-average parameters $B_j(t)$ ($0 \leq j \leq p-1$).

In each of these two model formulations, the second problem we address here is the problem of diagnosis : when a change has been detected, decide which pole(s) and eigenvector(s) have been changed. In (1), we are interested in monitoring the eigenstructure of the system, i.e. the pairs $(\lambda, H\phi_\lambda)$ where λ and ϕ_λ are the eigenvalues and corresponding eigenvectors of the state transition matrix F , which is assumed to be asymptotically stable. In (2), we consider equivalently the generalized eigenvalues and eigenvectors of the matrix polynomial :

$$I - \sum_{i=1}^P A_i z^{-i}$$

In section IV, these will be the vibrating characteristics of the monitored structure.

Because of the timing characteristics of our application, namely very slowly occurring changes (months or years) and high sampling frequency, in all what follows, we will solve rather a model validation problem than a change detection problem : namely, given a reference model, F^0 or θ^0 and a new record of observations $(Y_t)_{1 \leq t \leq s}$, decide whether this model still adequately describe it (global detection) and solve the diagnosis problem. However, the tests which we propose here may be used for on-line change detection purposes, as generally explained in M. Basseville, A. Benveniste (Ed.) (1986) : this has been done for speech signal segmentation in R. André-Obrecht (1985).

The paper is organized as follows. In section II, we present our original approach for solving the global detection problem, using both an

instrumental statistics and a statistical local approach for detection. In section III, we study the diagnosis problem, following the same local approach and using a linearization to relate changes in eigencharacteristics to changes in AR parameters. Numerical experiments on both simulated and real data are reported in section V, following the description of the underlying mechanical engineering problem of vibration monitoring, which is given in section IV. Finally some conclusions are drawn in section VI.

II. GLOBAL CHANGE DETECTION

In this section, we investigate the two equivalent problems of detecting changes in the F matrix of model (1) or in the AR parameters (A_i) of model (2), with as much robustness as possible with respect to the non-stationary unknown excitation V_t or moving average parameters $(B_j(t))$.

As discussed in Basseville et al. (1985b) in the scalar case, it is not possible to use standard likelihood techniques for solving this problem. The two key reasons are that, first, the Fisher information matrix of an ARMA process is not block-diagonal, and thus there is a coupling effect between poles and zeroes ; and, second, the highly time varying behavior of the unknown parameters prevent from using nuisance parameters elimination methods, usually based upon estimation or integration. We recall that in Bohlin (1977) for example, convenient estimated values of the MA coefficients were used.

The idea of our approach is to transform the problems i) or ii) into the simpler problem of detecting a change in the mean of a Gaussian process

with known covariance matrix, which is then solved by the classical χ^2 (generalized) likelihood ratio test.

1. An instrumental statistics

For the above mentioned purpose, we consider the following so-called instrumental statistics :

$$U_N(s) = \sum_{t=1}^s Z_t W_t^T \quad (5)$$

where :

$$Z_t^T = (Y_{t-p}^T, \dots, Y_{t-p-N+1}^T)$$

is a vector of $N \times p$ instrumental variables, i.e. here simply delayed observations, and where W_t is

$$W_t = Y_t - \sum_{i=1}^p A_i^0 Y_{t-i} = Y_t - \theta^{0T} \phi_t \quad (6)$$

with

$$\phi_t^T = (Y_{t-p}^T, \dots, Y_{t-1}^T).$$

θ^0 is the reference AR model, the adequacy of which has to be tested for the new record $(Y_t)_{1 \leq t \leq s}$. The process $U_N(s)$ may be alternatively generated in a practically more attractive way, using the following formula :

$$U_N(s) = \mathcal{H}_{p+1, N}^T(s) \begin{pmatrix} \theta^0 \\ -I_r \end{pmatrix} \quad (7)$$

where $\mathcal{H}_{p+1, N}^T(s)$ is the empirical Hankel matrix of the observed process (Y_t) :

$$\mathcal{R}_{p,q}(s) = \begin{pmatrix} R_0(s) & \dots & R_{q-1}(s) \\ \vdots & & \vdots \\ R_{p-1}(s) & \dots & R_{p+q-2}(s) \end{pmatrix}; R_m(s) = \sum_{t=1}^{s-m} Y_{t+m} Y_t^T.$$

It is well known (Söderström and Stoica, 1980) that equating to zero the right-hand side of (7) and solving, in the least squares sense if $N > p$, the resulting system of delayed Yule-Walker equations is nothing but the **instrumental variable I.V. identification method**, the consistency of which is established under stationary conditions. Benveniste and Fuchs (1985) show that the I.V. method also leads to a **consistent estimate** of the AR parameters θ , in the present situation of **nonstationary unknown MA coefficients**. This last robustness property is numerically proved in Prevosto et al. (1982), and in Prevosto et al. (1983) where favorable comparisons with frequency domain methods are made. Let us now investigate the detection problem for the process U_N .

Under the hypothesis H_0 of no change, ie $\theta = \theta^0$, W_t defined by (5) is actually a MA process, which is uncorrelated with Z_t , and thus $U_N(s)$ is zero-mean.

Under the local alternative hypothesis H_1 of small change, ie $\theta = \theta^0 + \frac{\delta\theta}{\sqrt{s}}$, the mean of $U_N(s)$ can be easily checked to be equal to the mean of :

$$\frac{1}{\sqrt{s}} \mathcal{R}_{p,N}^T(s) \delta\theta \tag{8}$$

In order to be able to compute covariances, it is necessary to consider a convenient vector u_N instead of the matrix U_N , and from now on we will use notations and basic results concerning kronecker products which have been summarized for example in Yuan and Ljung (1984).

$$\begin{aligned} \text{Let :} \quad u_{N(s)} &\triangleq \text{col} (U_N^T(s)) \\ &= \sum_{t=1}^s Z_t \otimes W_t \end{aligned} \quad (9)$$

be the vector of size Nr^2 obtained by stacking the Nr columns of $U_N^T(s)$ on top of each other, and $(H) = \text{col} (\theta^T)$ obtained in the same way from θ (4). Furthermore, let $\Sigma_N(s)$ be the covariance matrix of $u_N(s)$ under the no change hypothesis H_0 . Then, because of the independence of (Z_t, W_t) and (Z_s, W_s) for $|t-s| \geq p$, we have :

$$\Sigma_N(s) = \sum_{t=1}^s \sum_{i=-p+1}^{p-1} \mathbb{E}_0(Z_t Z_{t-i}^T \otimes W_t W_{t-i}^T) \quad (10a)$$

Let us consider the following estimate :

$$\hat{\Sigma}_N(s) = \sum_{t=1}^s \sum_{i=-p+1}^{p-1} (Z_t Z_{t-i}^T \otimes W_t W_{t-i}^T) \quad (10b)$$

It is shown in Moustakides and Benveniste (1985), that $\frac{1}{s} \hat{\Sigma}_N(s)$ is a consistent estimate of $\frac{1}{s} \Sigma_N(s)$ under both hypotheses H_0 and H_1 (because first order approximations do not affect covariances), and that $\frac{1}{\sqrt{s}} u_N(s)$ is asymptotically gaussian distributed under both hypotheses, ie :

$$\begin{aligned} \Sigma_N^{-1/2}(s) u_N(s) &\xrightarrow[s \rightarrow \infty]{\mathcal{L}} \mathcal{N}(0, I) \quad \text{under } H_0 \\ \Sigma_N^{-1/2}(s) (u_N(s) - \mu(s)) &\xrightarrow[s \rightarrow \infty]{\mathcal{L}} \mathcal{N}(0, I) \quad \text{under } H_1 \end{aligned}$$

where

$$\mu(s) = \frac{1}{\sqrt{s}} (\mathcal{H}_{p,N}^T(s) \otimes I_r) \delta(H) \quad (11)$$

Let us emphasize that these law of large numbers and central limit theorems are obtained in the **nonstationary** framework we have assumed (time-varying moving average parameters). Therefore, as stated at the beginning of this section, we have transformed the initial problems i) and ii) into the classical problem of change in the mean of a gaussian process with known covariance matrix. Precise assumptions and theorem statements are given in Appendix A.

2. The changes which can be detected

We now emphasize which changes can be detected with the aid of the instrumental statistics $U_N(s)$ (7). Because of its mean value (8) under H_1 , $U_N(s)$ will allow the detection of any change $\delta\theta$ belonging to the range of $\frac{1}{s} \mathcal{H}_{p,N}^T(s)$.

We assume that the nominal representation (H_0, F_0) (1) is observable and that the following factorization of the empirical Hankel matrix holds :

$$\frac{1}{s} \mathcal{H}_{p,N}(s) = \sigma_p(H_0, F_0) \cdot \mathcal{C}_N(F_0, G_s) + \varepsilon(s) \quad (12)$$

where

$$\sigma_p(H_0, F_0) = \begin{pmatrix} H_0 \\ H_0 F_0 \\ \vdots \\ H_0 F_0^{p-1} \end{pmatrix}$$

is the observability matrix, and

$$\mathcal{C}_N(F_0, G_s) = (G_s, F_0 G_s, \dots, F_0^{N-1} G_s)$$

is the controllability matrix, uniformly of full row rank n , G_s being the empirical cross-correlation between the state X and the observation Y . In (12), $\varepsilon(s)$ converges to zero in distribution as $s \rightarrow \infty$. We refer to Appendix A for the assumptions under which these properties are true.

In such a case, because of the factorization (12), the only changes on θ we will not be able to detect with this approach are those that satisfy :

$$\mathcal{O}_p^T (H_0, F_0) \delta\theta = 0 \tag{13}$$

But these last changes do **not** correspond to any change in the minimal representation (1) of the system, as the following argument shows. In fact, formula (3) shows that the representations (1) and (2) of the system are related to each other through the relationship :

$$\mathcal{O}_{p+1}^T (H_0, F_0) \theta_0 = 0$$

and any θ_0 satisfying this relation gives rise to a valid ARMA representation of the system. But two different parameters θ_0 and $\theta_0 + \delta\theta$ satisfying the above relation, are precisely related through (13). Further discussions may be found in (Benveniste et al.) (1985). This property (13) is of particular importance in practice, because it means that, for detecting changes in a minimal representation (H_0, F_0) (1) of (Y_t) with

the aid of the instrumental statistics $U_N(s)$ (7), it is not necessary to use a minimal ARMA representation (2) of (Y_t) .

3. The global test

We now give the test for detecting changes in the mean of $u_N(s)$ (9). Recall that, if a vector U is gaussian with mean μ and covariance Σ , for testing $\mu = 0$ against $\mu \in \text{Range}(M)$ where M is a full column rank matrix, the (generalized) likelihood ratio test is :

$$U^T \Sigma^{-1} M (M^T \Sigma^{-1} M)^{-1} M^T \Sigma^{-1} U \quad (14)$$

Let us consider the application of this result to the instrumental statistics $u_N(s)$.

In order to apply (14), it is necessary to reduce

$$M = \mathcal{H}_{p,N}^T(s) \otimes I_r \quad (15)$$

to a full (nr) column rank matrix. This is possible because the system (1) is assumed to be observable. A solution involving selection of rows in $\mathcal{O}_p(H_0, F_0)$ and of columns in $\mathcal{H}_{p,N}(s)$ is described in (Basseville et al., 1985a) and (Rougée, 1985).

Therefore, according to the previous discussion, the global test for detecting changes in \textcircled{H} is :

$$t_0 = u_N^T(s) \hat{\Sigma}_N^{-1}(s) M \left(M^T \hat{\Sigma}_N^{-1}(s) M \right)^{-1} M^T \hat{\Sigma}_N^{-1}(s) u_N(s) \quad (16)$$

with M given by (15). Under H_0 , t_0 is distributed as a χ^2 variable with nr degrees of freedom. Under H_1 , t_0 is distributed as a χ'^2 with the same number of degrees of freedom, and with non centrality parameter :

$$\delta \oplus M^T \hat{\Sigma}_N^{-1}(s) M \delta \oplus \quad (17)$$

which is non zero in all the cases of interest as previously discussed.

Let us briefly mention an interesting special case which will be useful in practice. Assume that the numbers of sensors r divides the state dimension n , that the AR order of the ARMA model (2) is $p = n/r$ (this means that the first n rows of $\Theta_p(H_0, F_0)$ are independent), and that the number of instruments N is equal to the AR order p . In this situation, M (15) is invertible and the global test reduces to :

$$t_0 = \mathbf{u}_p^T(s) \hat{\Sigma}_p^{-1}(s) \mathbf{u}_p(s) \quad (18)$$

The efficiency of the test (16) is numerically investigated in section IV, and theoretically analyzed by Rougée (1985) and Rougée et al. (1985) under stationarity assumptions. It turns out that, in the case where $n = pr$, ie where the dimension of the observation is a divisor of the state dimension, the asymptotic power of the test t_0 is related to the asymptotic precision of the instrumental variable identification method, as derived by Stoica et al. (1985). Furthermore, it may be shown that, in the scalar case and for a special choice of instruments, this test is equivalent to the min-max optimal local likelihood test, which is **robust** with respect to uncertainties on the moving average part. Finally, preliminary numerical results for nonstationary scalar signals are described by Basseville et al. (1985b) and show that small changes (1%) in eigenfrequencies may

be detected, using sufficiently long records.

III. THE DIAGNOSIS PROBLEM

We now discuss the diagnosis problem, as stated in the introduction. When a change on the AR part has been detected by the global test t_0 , the problem is to get insights onto which poles and modes have been changed, again without knowing the nonstationary MA part. As far as we know, the multiple model approach is the only solution which has ever been given to this problem, even in the stationary autoregressive case, ie when in (2) only B_0 is non zero and is moreover constant.

Our suggestion is to use the same approach as for the global test, together with a relationship between changes in eigencharacteristics and changes in AR parameters, to be used in the expression (11) of the mean value of the instrumental statistics u_N under H_1 . As we develop our detection tools under the assumption of small changes, we use first order Taylor expansions for \textcircled{H} , in the following manner.

Let Φ a minimal parameterization of the AR part of the process. In section IV, Φ will be the set of the eigen (or modal) characteristics of the structure. We assume that the application $\Phi \rightarrow \textcircled{H} = f(\Phi)$ is continuously differentiable in the neighborhood of the nominal model Φ_0 . If we want to monitor a particular subset of the coordinates of Φ , we consider the matrix J obtained by selection of the corresponding columns of the jacobian matrix $f'(\Phi_0)$, and we apply formula (16) with :

$$M = \left(\mathcal{H}_{p,N}^T(s) \otimes I_r \right) J \tag{19}$$

in order to get what we call a sensitivity test.

Several computations of Jacobians of interest for our vibration monitoring application are described in section IV.

The main advantage of this method is to allow separate monitoring of subsets of parameters of interest (for example one pole together with the corresponding eigenvector), without knowing in advance which subsets will actually change. The main drawback is that no theoretical argument may assess some decoupling property, concerning for example separate monitoring of modes : the corresponding tests (16) computed with the relevant Jacobians (19) are not statistically independent of each other. However, simulations results presented by Basseville et al. (1985b) show that, in the scalar case ($r=1$), this decoupling property concerning poles hold, provided that the poles are close to unit circle. In the vector case, this seems to be still true, provided that furthermore the number of sensors r is equal to the actual number of modes ($n/2$ in case of real F). See (Basseville, 1985).

Finally, let us mention another approach for the diagnosis problem which is investigated by Basseville et al. (1985b) in the scalar case. This approach is based upon a theoretical decoupling property, but is difficult to be implemented because of combinatorial problems in (real) case of no a priori knowledge of which parameter subsets are to change.

IV. APPLICATION TO THE VIBRATION MONITORING PROBLEM

1. Motivations

As mentioned in the introduction, the reason for which we studied the problems addressed in this paper is a mechanical engineering problem: how to supervise the vibrating characteristics of a structure subject to a nonstationary and unmeasured natural excitation. Examples of such vibrating structures are offshore platforms subject to the swell (the purpose of our study), buildings or bridges subject to wind or earthquakes, mechanical objects subject to fluid interactions,... One of the goals of vibration monitoring is the detection of cracks and fatigues. Our approach for solving such a problem involves the following steps :

i. On site identification of the vibrating characteristics of the structure. This step is necessary because the finite elements models provided by the designer have to be significantly readjusted when the structure is settled down. Furthermore, the possible model deviations due to fatigue appearance are often less important than the deviations between the designer's model and the behavior actually observed when the structure is settled down. Therefore the designer's model cannot be used as a reference model for fatigue detection by modal analysis.

ii. Detection of changes in the modal characteristics. As explained in the Introduction, the purpose of this task is to decide whether the model which was identified on the safe structure still adequately represent the new accelerometer signals obtained during a new inspection.

iii. **Diagnosis of the change.** The problem is then to discriminate between changes in the masses distribution, which are of no interest for fatigue detection, and changes in the stiffness coefficients ; and furthermore to estimate the localization of the fatigues in the structure.

From the implementation point of view, tasks i and iii should be done in a remote computing center because they are time consuming. Task ii, previously called global detection in section II, should be done on board.

Up to our knowledge, it seems that no other parametric model approach has been followed for solving these types of problems. Furthermore the way by which we manage with the excitation and the fluid/structure interactions also seems to be non-standard. Because the vibrating modes of an offshore platform lie beyond one hertz (and thus beyond the fundamental frequency of the swell), the structure is mainly excited by shock effects and turbulences which induce non-stationarities in the signals and which are **unmeasurable** by the existing swell sensors. In such a situation, our approach is to model the excitation by a nonstationary gaussian white noise, to neglect the fluid/structure interactions, and to develop algorithms as much robust as possible with respect to these assumptions, for the three above mentionned tasks.

2. The model to be used

Assuming that the structure may be decomposed in finite elements and has a linear behavior (Prevosto, 1982), we model a vibrating structure by a damped system of masses connected by springs, which obeys the following equation :

$$M \ddot{D}_t + C \dot{D}_t + K D_t = \epsilon_t \quad (20)$$

where M , C , K are the masses, damping, and stiffness matrices respectively ; D_t is the vector of displacement of the m degrees of freedom ; ϵ_t is the excitation vector, assumed to be a non-stationary gaussian white noise with covariance matrix R_t . The modal characteristics (λ, ϕ_λ) of this structure are the solutions of :

$$\begin{cases} \det (M \lambda^2 + C \lambda + K) = 0 \\ (M \lambda^2 + C \lambda + K) \phi_\lambda = 0 \end{cases} \quad (21)$$

As M , C , K are real, the $2m$ solutions $\lambda = c + i\omega$ are pairwise conjugate. They are called eigenfrequencies ; the ω are the eigen-pulsations ; and $c/\sqrt{c^2 + \omega^2}$ is the corresponding (negative) damping coefficient. In case of proportional damping, ie when $C = \alpha M + \beta K$, the modes ϕ_λ are real.

Let Y_t be the observation of $r \ll m$ degrees of freedom of the system. Y_t is described by :

$$Y_t = L D_t \quad (22)$$

where L_{ij} is equal to one if sensor i observes the degree j , and zero otherwise. Then the modal identification is the obtention of $(\Delta, L\Phi)$ where Δ is the diagonal matrix filled by the λ , and $L\Phi$ is the observed part of the modes ϕ_λ .

The system described by the equations (20) and (22) is equivalent to the following continuous time state space model :

$$\begin{cases} \dot{X}_t = A X_t + B_t \\ Y_t = H X_t \end{cases} \quad (23)$$

where $X_t = \begin{pmatrix} D_t \\ \cdot \\ D_t \end{pmatrix}$, $\dim X_t = 2m = n$, $A = \begin{pmatrix} 0 & I \\ -M^{-1}K & -M^{-1}C \end{pmatrix}$, $B_t = \begin{pmatrix} 0 \\ -M^{-1}\epsilon_t \end{pmatrix}$

$H = (L \ 0)$. The discrete time equivalent model is :

$$\begin{cases} X_{t+\Delta t} = F X_t + V_t \\ Y_t = H X_t \end{cases} \quad (24)$$

where $F = e^{A \cdot \Delta t}$ and $V_t = \int_t^{t+\Delta t} e^{A(t+\Delta t-\tau)} B_\tau d\tau$.

Because of the above assumption on ϵ_t (20), V_t is a white noise with covariance matrix :

$$Q_t = \int_t^{t+\Delta t} e^{A\tau} \cdot \tilde{R}_\tau \cdot e^{A^T\tau} d\tau$$

where $\tilde{R}_\tau = \begin{pmatrix} 0 & 0 \\ 0 & M^{-1}R_\tau M^{-1} \end{pmatrix}$.

The eigenvalues μ and eigenvectors ψ_μ of $F(24)$ are related to the modal characteristics (21) by :

$$\mu = e^{\lambda \cdot \Delta t} \quad \text{and} \quad H \psi_\mu = L \phi_\lambda \quad (25)$$

and are solutions of :

$$(\mu^p I_r - \sum_{j=1}^p \mu^{p-j} A_j) H \psi_\mu = 0 \quad (26)$$

where the autoregressive parameters A_1, \dots, A_p are given by (3). Actually, assuming that $\Delta t = 1$, F may be diagonalized in the following manner :

$$F = \Psi \cdot e^D \cdot \Psi^{-1} \quad (27)$$

where

$$D = \begin{pmatrix} \Delta & 0 \\ 0 & \bar{\Delta} \end{pmatrix} \quad \text{and} \quad :$$

$$\Psi = \begin{pmatrix} \Phi & \bar{\Phi} \\ \Phi\Delta & \bar{\Phi}\Delta \end{pmatrix} .$$

Consequently, identifying and monitoring the set of $(\mu, H\psi_\mu)$ given by (26) is equivalent to the same tasks for the set of $(\lambda, L\phi_\lambda)$ given by (21). The relevance of the problems and methods investigated in the previous sections is thus stated for the vibration monitoring problem.

Before describing the experiments which have been done for validating our approach for the detection and diagnosis problems, let us recall that, for the identification problem using (3) and (26), strong results have been obtained from both theoretical and experimental points of view. Several known identification methods (instrumental variable, balanced realization, Ho-Kalman) have been shown to lead to consistent estimates \hat{A}_i in (3), even with a nonstationary excitation (fast changes in the geometry of the excitation are allowed). See (Benveniste and Fuchs, 1985). From the experimental point of view, Prevosto et al. (1983) have shown that these methods are more convenient than classical Fourier methods for the identification of high

order modes, and they investigated the stability of the identified modes with respect to the waves height and the swell direction. Other numerical results for the identification problem may be found in (Prevosto, 1982) and (Prevosto et al., 1982).

3. The diagnoses to be investigated

With respect to the diagnosis problem statement iii which was given in the paragraph IV.1., our purpose is here more restrictive. The diagnoses we will consider are only in terms of the vibrating characteristics $(\lambda, L\phi_\lambda)$, rather than in terms of the physical parameters M, C, K. This last point is currently under study, and has not been addressed before because it is a (highly) ill-conditioned problem: in practice, the number of sensors r is between 2 and 5, and the size of the M, C, K matrices is several hundreds. Notice that Isermann (1985) also assumes for diagnosis that the process coefficients of interest may be uniquely determined from the model parameters at hand. Therefore, we consider only the problems described in section III, and present results concerning some Jacobians (19) of interest for monitoring the vibrating or modal characteristics $(\Delta, L\Phi)$ (21). The key point is the computation of convenient differentiations of equation (3), which may be written equivalently :

$$\theta^T \sigma_p(H, F) = \sum_{j=0}^p A_j H F^{p-j} = 0 \quad (28)$$

where $A_0 = -I_r$. After the change of basis defined by (27), we get :

$$\theta^T \sigma_p(L\Phi, e^\Delta) = \sum_{j=0}^p A_j L \Phi e^{(p-j)\Delta} = 0 \quad (29)$$

where
$$\theta_p(L\Phi, e^\Delta) = \begin{pmatrix} L\Phi \\ L\Phi e^\Delta \\ \vdots \\ L\Phi e^{(p-1)\Delta} \end{pmatrix} .$$

Then, differentiating (29) results in :

$$\begin{aligned} d\theta^T \cdot \theta_p(L\Phi, e^\Delta) &= - \sum_{j=0}^p A_j L d(\Phi e^{(p-j)\Delta}) \\ &= - \sum_{j=0}^p A_j L d\Phi e^{(p-j)\Delta} \\ &= - \sum_{j=0}^{p-1} (p-j) A_j \cdot L\Phi \cdot e^{(p-j)\Delta} \cdot d\Delta . \end{aligned}$$

The corresponding equation for $d \mathbb{H}$, where $\mathbb{H} = \text{col}(\theta^T)$ was introduced in section II.1, is then :

$$\begin{aligned} (\theta_p(L\Phi, e^\Delta)^T \otimes I_r) d \mathbb{H} &= - \sum_{j=0}^p (e^{(p-j)\Delta} \otimes A_j L) \text{col}(d\Phi) \\ &= - \sum_{j=0}^{p-1} (I_m \otimes (p-j) A_j L \Phi e^{(p-j)\Delta}) \text{col}(d\Delta) . \end{aligned} \tag{30}$$

After some computations which may be found in Appendix B, we get :

$$(\theta_p^{*T} \otimes I_r) d \mathbb{H} = \begin{pmatrix} J_\psi & 0 \\ 0 & \overline{J_\psi} \end{pmatrix} \begin{pmatrix} d\psi \\ \overline{d\psi} \end{pmatrix} + \begin{pmatrix} J_\lambda & 0 \\ 0 & \overline{J_\lambda} \end{pmatrix} \begin{pmatrix} d\lambda \\ \overline{d\lambda} \end{pmatrix} \tag{31}$$

where θ_p^* is the observability matrix in the modal basis and easily shown to be :

$$\theta_p^* = (\theta_p(L\Phi, e^\Delta), \overline{\theta_p(L\Phi, e^\Delta)}) ;$$

where

$$d\lambda = \begin{pmatrix} d\lambda_1 \\ \vdots \\ d\lambda_m \end{pmatrix}, \quad d\psi = \begin{pmatrix} L & d\phi_1 \\ \vdots & \\ L & d\phi_m \end{pmatrix}$$

contains the variations of the observed part of the modes,

$$J_\psi = \begin{pmatrix} A(\mu_1) & & 0 \\ & \ddots & \\ 0 & & A(\mu_m) \end{pmatrix},$$

$$J_\lambda = \begin{pmatrix} \mu_1 A'(\mu_1) L \phi_1 & & 0 \\ & \ddots & \\ 0 & & \mu_m A'(\mu_m) L \phi_m \end{pmatrix}$$

with

$$\mu_j = e^{\lambda_j} \quad (1 \leq j \leq m),$$

and

$$A(X) = - \sum_{j=0}^P A_j X^{P-j}$$

If we use :

$$d\psi = d\psi^{\text{re}} + i d\psi^{\text{im}}$$

and

$$d\lambda_j = dc_j + i d\omega_j \quad (\text{see (21)})$$

we get :

$$\left(\mathcal{D}_p^* \otimes I_r \right) d \mathcal{H} = \begin{pmatrix} J_\psi & iJ_\psi \\ - & - \\ J_\psi & -iJ_\psi \end{pmatrix} \begin{pmatrix} d\psi^{re} \\ \\ d\psi^{im} \end{pmatrix} + \begin{pmatrix} J_\lambda \\ \\ J_\lambda \end{pmatrix} dc + \begin{pmatrix} iJ_\lambda \\ \\ -iJ_\lambda \end{pmatrix} d\omega \quad (32)$$

We may then monitor various vectors β containing any free modal parameters among the ψ_j, c_j, ω_j ($1 \leq j \leq m$), by selecting the corresponding columns in the right-hand side of (32), in order to get a full column rank matrix J_β such that :

$$\left(\mathcal{D}_p^* \otimes I_r \right) d \mathcal{H} = J_\beta d\beta \quad (33)$$

Using the factorization (13) written in the modal basis, it turns out that, because of (19) and (33), the sensitivity test for monitoring β is given by (16) with :

$$M = \left(\mathcal{C}_N^{*T} \otimes I_r \right) J_\beta \quad (34)$$

where $\mathcal{C}_N^* = \Psi^{-1} \mathcal{C}_N(F, G_s)$.

In the experiments which will be described in the next section, the selected vectors β are separate vibrating modes together with the corresponding vibrating pulsation, namely :

$$\psi_j^{re}, \psi_j^{im}, \omega_j$$

The damping coefficients c_j are not monitored because usually not precisely identified.

Finally, let us mention that, as for the global test t_0 (16), theoretical investigations have been made by Rougée (1985) for computing the power of any sensitivity test of the form (16) with M given by (34). These results are also discussed in (Rougée et al., 1985) and used in (Basseville et al., 1985c).

V. EXPERIMENTAL RESULTS

In this section, we present results obtained from the computer implementation of the various tests presented in sections II and III, using both simulated and real vector data related to the mechanical engineering problem described in section IV. Let us simply recall that experiments done for scalar signals are described by Basseville et al. (1985b) and lead to the conclusion that it is possible to detect and diagnose small changes (1%) in eigenfrequencies provided that the damping coefficients are low, and that the record sizes are high enough (several thousands).

According to section IV.2, the simulation model which we used is a tied down system of 18 masses of one degree of freedom connected by springs, as shown on figure 1, with known weights, stiffness, and damping coefficients. Six dimensional signals (displacements of the masses 1, 7, 13, 6, 12, 18) have been generated under different hypotheses, including no change, change in mass $n^{\circ}1$, change in the stiffness of the connection to the ground, and change in the stiffness of the connection 8-11. These physically different changes are indicated by arrows on figure 1.

We have computed the global test t_0 (16) and nine sensitivity tests, using (34), corresponding to the nine modes of lowest order, considered together with the pulsation ω_j as mentioned at the end of section IV. These tests computations have been done for all the subsets of components corresponding to $r = 2, 3, 4, 6$ and to at least one observation on each of the two opposite "legs" of the structure.

For simulated as well as real data, the reference AR model θ_0 , to be used for the tests, has been computed by solving (29) where Φ and Δ contain the available eigencharacteristics (a few number for real data). The AR order p is chosen to be the integer part of $2m/r$, and not such that : $2m/r < p < 2m/r + 1$ as done for the identification task 1 of section IV. The reason for doing so is that it is intuitively more sensible to monitor the structure without introducing other modes than those which are available. The chosen number of instruments is $N = p$, for reasons which were discussed in (Basseville et al., 1985b), and therefore t_0 is computed with the aid of (18).

Finally, the Jacobians J_β of section IV.3 are computed with the aid of (31), where the eigenvalues and eigenvectors which are used are the **available** eigencharacteristics, and not the modes associated to the computed A_i .

The results are presented on the three-dimensional figures 2 to 5, one horizontal axis corresponding to the different sensors location and numbers, from 2 to six from left to right, and the other horizontal axis

corresponds to various tests, with the global test in the back and the nine sensitivity tests from the back to the front according to increasing frequencies. As pictures are drawn with automatic scaling, the maximum value is indicated. Figure 2 shows the behavior of the various tests under no change hypothesis ; Figure 3 corresponds to a change (14%) in mass 1, figure 4 to a change (12%) in stiffness of ground connection, figure 5 to a cancellation of connection 8-11 (which was small). These results show that some sensor locations may be very poor, in the sense that no detection is possible (nearly same value of the tests under H_0 and H_1), and therefore moving sensors (along legs) may be of interest. On the other hand, the visually significantly different profiles obtained on figure 3 to 5 lead to the conclusion that these types of tests contain informations which allow discrimination among physically different changes. An attempt to get direct physical diagnosis is currently under development.

VI. CONCLUSION

We have presented an original approach for solving the two problems of detection and diagnosis of changes in the eigenstructure of nonstationary multivariable systems. This approach is based upon the use of an instrumental statistics which reduces the detection problem to a problem of change in the mean of a gaussian process with known covariance matrix. The diagnosis problem is solved in the same manner, with the aid of first order approximations relating the changes in the system parameters to be monitored to the changes in the model parameters which are used for monitoring.

The application of this methodology to the mechanical engineering problem of vibration monitoring has been described for complex vibrating structures subject to natural nonstationary and uncontrolled excitation. Experimental results have been presented, which show the efficiency of the proposed tests, for detecting small changes (typically a few per cent in eigenfrequencies) and for discriminating different physical changes. Our opinion is that such an approach for diagnosis ("sensitivity" method) is general enough for a possible direct use on physical parameters. This approach is currently under study.

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APPENDIX A

ASSUMPTIONS AND THEOREMS STATEMENT

We here state the precise assumptions and theorems involved in our detection approach.

Let (Y_t) be a nonstationary multivariable process having as state space representation :

$$\begin{cases} X_{t+1} = F X_t + V_{t+1} \\ Y_t = H X_t \end{cases} \quad (A1)$$

where $X_t \in \mathbb{R}^n$, $Y_t \in \mathbb{R}^r$, $\text{cov}(V_{t+1}) = Q_t$, (V_t) is a white noise with time varying covariance matrix.

*The assumptions concerning the model (A1) are as follows :

(C1) F is full rank and asymptotically stable

(C2) there exists $k > 0$ such that, for any vector u and any integer t ,

$$E(u^T V_t)^4 < k \|u\|^4$$

(C3) (H, F) is observable, ie there exists an integer p such that :

$$\mathcal{O}_p(H, F) \triangleq \begin{pmatrix} H \\ HF \\ \vdots \\ HF^{p-1} \end{pmatrix}$$

is of rank n .

(C4) there exists a matrix G of rank r such that, for any integer t :

$$Q_t \succ G G^T$$

This last assumption, which is **not** a controllability condition, allows changes in the direction of the excitation.

*If the nominal model (H_0, F_0) fulfills the conditions C1 to C4, then the two following theorems hold :

1. Nonstationary law of large numbers

$\sum_N(s)$ given by (10a) is asymptotically uniformly positive definite and bounded, and the estimate $\hat{\sum}_N(s)$ given by (10b) is consistent :

$$\sum_N(s)^{-1} \hat{\sum}_N(s) \xrightarrow{s \rightarrow \infty} I \quad \text{a.s.}$$

under both no change hypothesis $H_0 : \theta = \theta_0$, and small change hypothesis

$$H_1 : \theta = \theta_0 + \frac{\delta\theta}{\sqrt{s}}$$

2. Nonstationary central limit theorem

Under H_0 , $\sum_N^{-1/2}(s) u_N(s) \xrightarrow{s \rightarrow \infty} \mathcal{N}(0, I)$

where

$u_N(s)$ is given by (9) ;

Under H_1 , $\sum_N^{-1/2}(s) \left(u_N(s) - \left(\frac{1}{s} \mathcal{H}_{p,N}^T(s) \otimes I_r \right) \delta \oplus \mathcal{H} \right) \xrightarrow{s \rightarrow \infty} \mathcal{N}_{(0,I)}$

*If we further assume that the following condition is fulfilled :

$$(C5) \quad \liminf_{s \rightarrow \infty} \sigma_n \left(\frac{1}{s} \mathcal{H}_{p,N}(s) \right) > \sigma > 0 \quad \text{a.s.}$$

where $\sigma_n(M)$ is the n -th singular value of M , then with (C3) we ensure the uniform minimality of the state space model (A1) to represent (Y_t) .

The assumptions (C1) to (C5) imply the assumptions needed by Benveniste et al. (1985) for the consistency of the identification, and also imply that the factorization (13) of the Hankel matrix hold.

APPENDIX B

DIFFERENTIATION OF A MATRIX POLYMIAL

We here give the details of the computations needed from equation (30) to equation (31), related to the Jacobians to be used for diagnosis.

Rewriting (30) :

$$\begin{aligned} \left(\frac{\partial}{\partial \Delta} (L\phi, e^{\Delta})^T \otimes I_r \right) d \mathbb{H} &= - \sum_{j=0}^p (e^{(p-j)\Delta} \otimes A_j L) \text{col} (d\phi) \\ &\quad - \sum_{j=0}^{p-1} (I_m \otimes (p-j)A_j L \phi e^{(p-j)\Delta}) \text{col} (d\Delta) \end{aligned}$$

$$\underline{\Delta} = (J_1 + J_2) \tag{B1}$$

we first consider the term J_1 .

$$\begin{aligned} - \sum_{j=0}^p (e^{(p-j)\Delta} \otimes A_j L) &= - \sum_{j=0}^p \begin{pmatrix} e^{(p-j)\lambda_1} A_j L & & 0 \\ & \ddots & \\ 0 & & e^{(p-j)\lambda_m} A_j L \end{pmatrix} \\ &= \begin{pmatrix} A(\mu_1) L & & 0 \\ & \ddots & \\ 0 & & A(\mu_m) L \end{pmatrix} \end{aligned}$$

where $A(X) = X^p I_r - A_1 X^{p-1} - \dots - A_{p-1} X - A_p$

and $\mu_j = e^{\lambda_j}$.

Thus :

$$J_1 = \begin{pmatrix} A(\mu_1) & 0 \\ \vdots & \vdots \\ 0 & A(\mu_m) \end{pmatrix} \begin{pmatrix} L d\phi_1 \\ \vdots \\ L d\phi_m \end{pmatrix} \quad (B2)$$

On the other hand, for computing J_2 , we need :

$$\begin{aligned} \text{Col}(d\Delta) &= \text{Col} \begin{pmatrix} d\lambda_1 & 0 \\ \vdots & \vdots \\ 0 & d\lambda_m \end{pmatrix} \\ &= \begin{pmatrix} d\lambda_1 & e_1 \\ \vdots & \vdots \\ d\lambda_m & e_m \end{pmatrix} \end{aligned}$$

where e_j is the j -th vector of the canonical basis of \mathbb{R}^m . Therefore :

$$J_2 = - \sum_{j=0}^{p-1} \begin{pmatrix} (p-j)A_j L\phi e^{(p-j)\Delta} & 0 \\ \vdots & \vdots \\ 0 & (p-j)A_j L\phi e^{(p-j)\Delta} \end{pmatrix} \begin{pmatrix} e_1 & 0 \\ \vdots & \vdots \\ 0 & e_m \end{pmatrix} \begin{pmatrix} d\lambda_1 \\ \vdots \\ d\lambda_m \end{pmatrix}$$

$$\begin{aligned}
 &= - \sum_{j=0}^{p-1} \begin{pmatrix} (p-j)A_j L\phi_1 e^{(p-j)\lambda_1} & 0 \\ & \ddots \\ 0 & (p-j)A_j L\phi_m e^{(p-j)\lambda_m} \end{pmatrix} \begin{pmatrix} d\lambda_1 \\ \vdots \\ d\lambda_m \end{pmatrix} \\
 &= \begin{pmatrix} \mu_1 A'(\mu_1) L\phi_1 & 0 \\ & \ddots \\ 0 & \mu_m A'(\mu_m) L\phi_m \end{pmatrix} \begin{pmatrix} d\lambda_1 \\ \vdots \\ d\lambda_m \end{pmatrix} \tag{B3}
 \end{aligned}$$

where $A'(X) = p X^{p-1} I_r - (p-1) A_1 X^{p-2} - \dots - A_{p-1}$.

Including (B2) and (B3) into (B1), we get :

$$\left(\theta_p(L\phi, e^\Delta)^T \otimes I_r \right) d \textcircled{H} = J_\psi d\psi + J_\lambda d\lambda$$

where $J_\psi, J_\lambda, d\psi, d\lambda$ are defined below formula (31).

Finally, using the observability matrix in the modal basis :

$$\theta_p^* = \theta_p(H, F) \Psi$$

where Ψ is defined in (27), we get (31).

FIGURE CAPTION

Figure 1 : Simulation system

Figure 2 : The tests under H_0

Figure 3 : A change in mass 1

Figure 4 : A change in the stiffness of the connection to the ground

Figure 5 : A change in the connection 8-11.

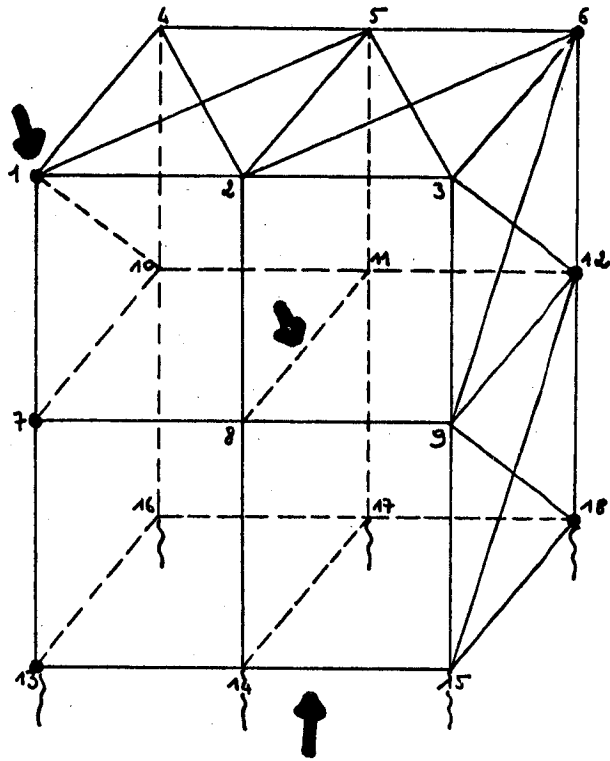


Figure 1 : Simulation system

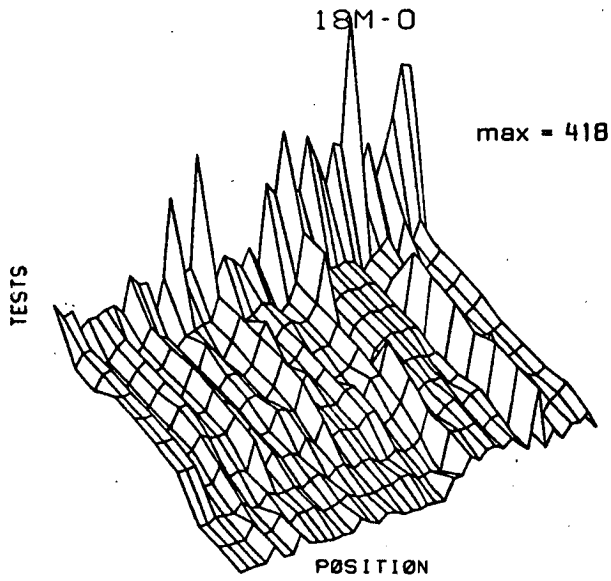


Figure 2 : Tests under H₀

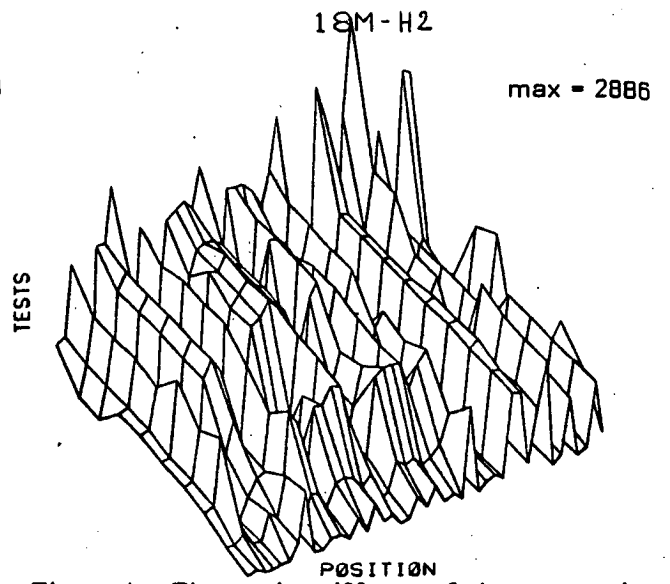


Figure 4 : Change in stiffness of the connection to the ground

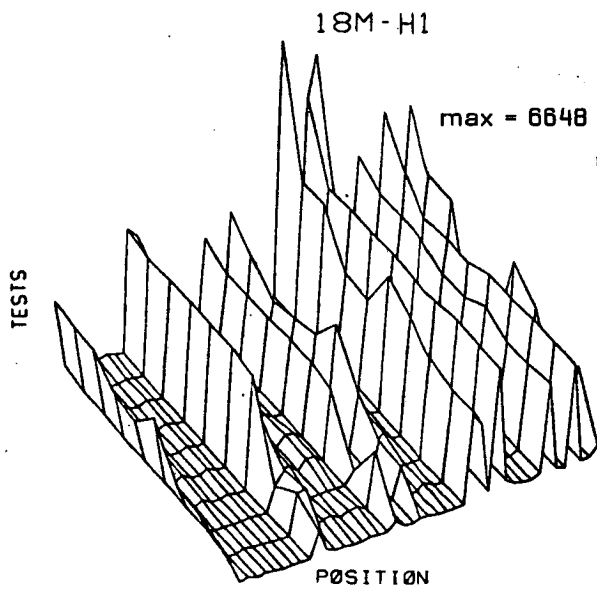


Figure 3 : Change in mass 1

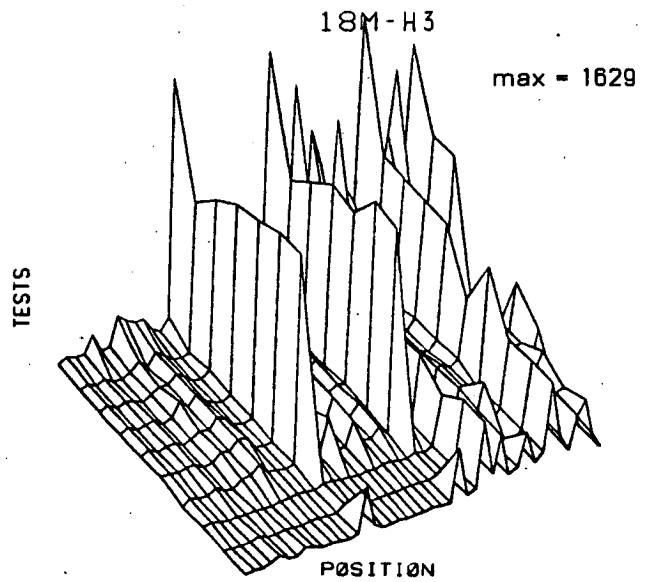


Figure 5 : Change in stiffness of the connection 8-11

