

Final Report

**FHWA/IN/JTRP-2000/28**

**DEVELOPMENT OF A PROCEDURE TO IDENTIFY AGGREGATES FOR  
BITUMINOUS SURFACES IN INDIANA**

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Joint Transportation Research Program  
Project No. C-36-6RR  
File No. 2-4-44  
SPR-2206

Prepared in cooperation with the  
Indiana Department of Transportation and  
the U.S. Department of Transportation  
Federal Highway Administration

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Purdue University  
West Lafayette, Indiana  
September 2001

TECHNICAL REPORT STANDARD TITLE PAGE

1. Report No. FHWA/IN/JTRP-2000/28		2. Government Accession No.		3. Recipient's Catalog No.	
4. Title and Subtitle Development of a Procedure to Identify Aggregate for Bituminous Surfaces in Indiana				5. Report Date September 2001	
				6. Performing Organization Code	
7. Author(s) Terry R. West and Kyu H. Cho				8. Performing Organization Report No. FHWA/IN/JTRP-2000/28	
9. Performing Organization Name and Address Joint Transportation Research Program 1284 Civil Engineering Building Purdue University West Lafayette, Indiana 47907-1284				10. Work Unit No.	
				11. Contract or Grant No. SPR-2206	
12. Sponsoring Agency Name and Address Indiana Department of Transportation State Office Building 100 North Senate Avenue Indianapolis, IN 46204				13. Type of Report and Period Covered Final Report	
				14. Sponsoring Agency Code	
15. Supplementary Notes Prepared in cooperation with the Indiana Department of Transportation and Federal Highway Administration.					
16. Abstract  Adequate friction resistance is needed to prevent pavement slipperiness allowing vehicles to stop in a reasonable distance. For stone mastic asphalt surfaces, friction resistance is mainly a function of the interaction between the aggregates exposed at the road surface and vehicle tires. Aggregate performance is reduced with time by wear and polishing as a consequence of vehicular traffic. In this research a method to investigate performance based on physical, chemical and petrographic factors has been evaluated. The objective was to develop a laboratory method to test Indiana dolomite, limestone, sandstone, and gravel aggregates to predict friction resistance in the field and determine causes for the range of values among these aggregates. Assessment of gravel sources was based on individual rock types and their proportions comprising the gravel. Initial friction Values (IFV) and Polished Values (PV) were determined in the laboratory with the British Wheel and Pendulum test and field values obtained from the towed friction trailer. For two laboratories involved a significant difference in IFV and PV was obtained so that further verification is required. Correlations between parameters were established which provide predictions of friction resistance based on laboratory specimens. A database of physical and chemical properties should be collected on aggregates used or considered for bituminous wearing courses. This includes the testing required for Class A aggregates plus elemental Mg and elemental Ca content.					
17. Key Words bituminous surfaces, friction resistance, dolomite, limestone.			18. Distribution Statement No restrictions. This document is available to the public through the National Technical Information Service, Springfield, VA 22161		
19. Security Classif. (of this report) Unclassified		20. Security Classif. (of this page) Unclassified		21. No. of Pages 115	22. Price



INDOT Research

# TECHNICAL *Summary*

Technology Transfer and Project Implementation Information

TRB Subject Code: 35-1 Mineral Aggregate Characteristics and Tests  
Publication No.: FHWA/IN/JTRP-2000/28, SPR-2206

September 2001  
Final Report

## ***Development of a Procedure to Identify Aggregates for Bituminous Surfaces in Indiana***

### **Introduction**

Adequate friction resistance is needed to prevent pavement slipperiness allowing vehicles to stop in a reasonable distance. For stone mastic asphalt surfaces, friction resistance is mainly a function of the interaction between the aggregates exposed at the road surface and vehicle tires. Aggregate performance is reduced with time by wear and polishing as a consequence of vehicular traffic. In this research a method to investigate performance based on physical, chemical and petrographic factors has been evaluated. The objective was to develop a laboratory method to test Indiana dolomite, limestone, sandstone, and gravel aggregates to predict friction resistance in the field and determine causes for the range of values among these aggregates. Assessment of gravel sources

was based on individual rock types and their proportions comprising the gravel. Initial friction Values (IFV) and Polished Values (PV) were determined in the laboratory with the British Wheel and Pendulum test and field values obtained from the towed friction trailer. For two laboratories involved a significant difference in IFV and PV was obtained so that further verification is required. Correlations between parameters were established which provide predictions of friction resistance based on laboratory specimens. A data base of physical and chemical properties should be collected on aggregates used or considered for bituminous wearing courses. This includes the testing required for Class A aggregates plus elemental Mg and elemental Ca content.

### **Findings**

This study was a continuation of previous work by Bruner, Choi and West, 1995, FHWA/IN/JHRP 95/11, which focused primarily on dolomite aggregates (19 sources). In the current study, 21 limestone sources, three sandstones and six gravel sources were added. Data from both studies were evaluated to provide an overall conclusion. Frictional performance is determined by polishing aggregate coupons with the British Wheel machine and measuring friction values with the British Pendulum tester. This provides the IFV (initial friction value), PV (polished value or BPN at 10 hours) and the difference between IFV and PV, WI, wear index.

Following aggregate collection, megascopic and microscopic evaluation of the aggregate samples was accomplished. Twenty-four additional rock thin sections were examined to extend those from the first study. Laboratory tests included acid insoluble residue (ASTM, D3042), size distribution of acid insoluble residue, elemental Mg content (ASTM, C602), Los Angeles abrasion (ASTM, C131), sulfate soundness (ASTM, C88), freeze-thaw loss in water and in brine solution (AASHTO, T103, Procedure A), absorption and specific gravity (ASTM, C127). These were conducted at INDOT, Division of Materials and Tests, and at Purdue University.

Aggregate coupons of limestones, sandstones and gravels were made for the British Wheel test and British Pendulum test (ASTM, D3319, E303). The coarsest portion of the No. 11 INDOT gradation, 12.5 mm to 9.5 mm in size, was used. To construct the coupons, aggregates were embedded in epoxy resin and a hardener component added. The epoxy was different from that used in the 1995 study (Bruner, Choi and West). Coupons were polished using the British Wheel machine and measured with the British Pendulum tester after zero hours (IFV), 1 hour, 3 hours, 6 hours and 10 hours (Polished value, PV or BPN<sub>10</sub>). These data were added to the existing information from the previous dolomite study (Bruner, Choi and West, 1995, FHWA/IN/JHRP 95/11).

Finally, analysis of all data from the current and the 1995 study were combined and a statistical evaluation performed. Results were compared to those from other states. Based on this, PV of dolomites and limestones required for bituminous surface courses can be recommended. Results can be used to develop specifications to evaluate additional sources for medium and high vehicular traffic pavements having bituminous overlays in Indiana.

Gravels consisted primarily of limestones and dolomites. The weighted PV ranged from 22.6 to 26.3 and PV correlates best with freeze-thaw loss. Other important factors are absorption, percent of crushed gravel pieces and percent of metamorphic rocks. Crushed gravels showed PN values greater than some crushed dolomites and crushed limestones. Crushed gravel sources should be considered for bituminous surface courses if they meet the standard requirements for Class A stone.

For carbonate aggregates, the difference in mineral hardness within an aggregate piece has a significant effect on friction resistance. Uneven texture after polishing yields a high PV. Materials that yield a contrast in hardness are: quartz vs. calcite, calcite vs. insoluble materials, dolomite vs. calcite, impure dolomite and impure limestone.

Certain geologic formations yield impure aggregate sources. The Kokomo Member and

Mississinewa Member are impure carbonates with higher friction values. The Huntington Dolomite and Brassfield Limestone possessing high carbonate purity, showed lower friction values.

Factors greatly affecting PV for dolomite aggregates are IFV, absorption, specific gravity, sodium sulfate loss, elemental Mg content, and percentage of insoluble residue, minus #200 sieve size. The most influential independent variables for dolomite are absorption and elemental Mg content.

The important factors affecting PV for limestone aggregates are total insoluble residue and percent insoluble residue, minus #200 sieve size. In all, the most influential independent variable is the total insoluble residue content.

Considering dolomite and limestone aggregates collectively, the most important variables are absorption, elemental Mg content and total insoluble residue.

Multiple linear regression equations were developed which can be used to predict PV. These are provided in the report.

As a starting point for further research using the British Polishing Wheel and Pendulum test the following is proposed:

<u>Minimum Polish Value</u>	<u>Frictional Resistance of Bituminous Surface</u>
24 or less	Poor
25 to 30	Marginal
31 or more	Good

For the calcareous sandstones studied, as total insoluble residue content increases (quartz, clay), PV also increases. With an increase in the ratio of plus #200 to minus #200 size insoluble residue, PV also increases.

Sandstones in the study showed a higher average PV (33.61) than did carbonate aggregate (PV=28.50 for dolomite and PV=24.77 for limestone) because of the heterogeneity of the calcareous sandstones. Quartz, calcite and clay provide this varied composition.

## Implementation

Higher elemental Mg values for dolomite aggregates indicate a greater presence of dolomite mineral. Such aggregates experience a lower PV. For the INDOT specifications, a

minimum 10.3 % elemental Mg is required for carbonate aggregates when used in surface courses with intermediate traffic requirements. Based on results of this study, dolomites with



less than 10.3 % elemental Mg should be considered for aggregate use if other requirements such as absorption and soundness are met. Impure limestones, containing clay, quartz or dolomite should also be considered if they meet those same requirements.

Carbonates with higher insoluble residue contents show a higher friction resistance than do purer carbonates with their low insoluble residue values. Data on insoluble residue content including grain size evaluation (+#200 size fraction vs - #200 size fraction) should also be determined. A data base for aggregates used in bituminous wearing courses should be compiled by INDOT which includes the insoluble residue content and absorption. Elemental Mg and Elemental Cu should also be determined.

A discrepancy in PV was observed between two pieces of test equipment for the British Pendulum test (Purdue University

equipment vs Alabama DOT equipment). Before BPN is selected as a standard criterion for evaluating aggregate quality in Indiana, the standard method must be developed using new equipment and a detailed evaluation of inherent variations. PVs need to be evaluated on a continuing basis because PV can vary even within the same aggregate source.

Some impure limestones with a higher frictional value should be considered as a source for bituminous overlays, but field performance of these limestones must be verified through field and laboratory evaluation prior to their use as overlays.

Additional testing using new Polishing Wheel and Pendulum test equipment should be conducted on Indiana aggregates to verify these results. INDOT studies assisted by research performed at Purdue University should address this objective.

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## List of Abbreviation

AASHTO -- American Association of State Highway and Transportation Officials  
AB -- Absorption  
AFN -- Average Friction Number  
ASTM -- American Society for Testing and Materials  
BPN -- British Pendulum Number  
DOL -- Dolomite  
FNR -- Friction Number Ribbed  
FNS -- Friction Number Smooth  
F-T -- Freeze-Thaw Loss  
IFV -- Initial Friction Value  
IG -- Igneous Rock  
INDOT -- Indiana Department of Transportation  
ITM -- Indiana Test Method  
ISM -- Indiana Standard Method  
LA -- Los Angeles Abrasion Loss  
LS -- Limestone  
META -- Metamorphic Rock  
MG -- Elemental Mg Content  
MgO -- Magnesium Oxide  
M\_200 -- Insoluble Residue < # 200 sieve  
OS -- Other Sedimentary Rock  
P\_200 -- Insoluble Residue > # 200 sieve  
PV -- Polished Value  
SPG -- Specific Gravity  
SUL -- Sulphate Soundness Loss  
T\_ACID -- Total Acid Insoluble Residue  
WI -- Wear Index



## 1. INTRODUCTION

The goal of this project is to extend the knowledge obtained from the 1995 JTRP study directed by T.R. West (Bruner, Choi and West, 1995) which focused mostly on dolomite aggregate sources to additional aggregate types, such as limestones, gravels and selected sandstones. Dolomite was the focus of the 1995 study because of its prevalent use in bituminous surface pavements for medium and high vehicular traffic roads.

Bituminous courses are placed over concrete pavements after the concrete has experienced years of wear from highway traffic. The coarse aggregate in the bituminous overlay must supply the primary roughness to yield needed resistance for braking. Although most coarse aggregate types in new bituminous pavements initially provide high friction values, polishing of the coarse aggregate to an equilibrium level eventually occurs. The extent of polishing an aggregate will experience is a function of rock type and gradation, as well as its physical and chemical properties.

For high vehicular traffic roads, equal amounts of dolomite and blast furnace slag are used as the coarse aggregate in bituminous surfaces. A minimum value of 10.3 % elemental Mg (78.1 % dolomite) is required for aggregates to qualify as an acceptable dolomite material. As high purity dolomites occur in only certain geologic formations in Indiana and therefore, in only certain locations, high purity dolomite aggregate must be shipped long distances to produce the bituminous surfaces for paving projects on Interstate highways. These greater transportation distances greatly increase the cost of construction.

The results of the study by Bruner, Choi and West (1995) suggest that some limestones may provide equal or better frictional resistance than do some dolomites that qualify because of their elemental Mg content. Difficulties can arise when attempting to isolate the major factors influencing polish and friction properties, and prioritizing those parameters because of their indeterminate and interrelated nature. The frictional resistance of the limestone and dolomite aggregates is controlled by their physical and chemical properties.

A detailed petrographic examination is necessary to determine the texture and composition of aggregates. The two most important, direct textural parameters affecting frictional properties are grain size and shape. Angular grains, at least initially, have a tendency to develop harshly textured surfaces and provide a greater degree of resistance as compared to rocks containing smooth grains. Dierstein and LaCroix (1984) reported that resistance to polishing effects of aggregates is related to grain size, hardness and durability. Larger and harder grains generally provide greater friction values. As an example to illustrate this relationship, Dierstien and LaCroix compared dolomite and limestone sources. Dolomites with generally larger and harder grains, proved superior to limestones. Shupe (1960) noted that particle-by-particle type of wear in rocks such as sandstone consisting of hard quartz and weak calcite matrix was related with high friction values. In a study by Shakoor and West (1979), grain size and particle shape were found to affect polish and thus friction properties; however, they contributed to a lesser degree than did composition. According to the 1995 JTRP study (Bruner, Choi and West, 1995), texture of dolomite aggregates such as grain shape and grain size did not relate well with frictional resistance. Only median grain size showed a high correlation to Polished Values.

Therefore, in this study, the major consideration is given to the compositional properties of dolomite and limestone aggregates to find the critical factors that provide some limestones with a better performance in skid resistance.

As mentioned previously, composition has been found to be a principal factor influencing aggregate performance. Review of the study by Russell (1972), provides the reason why magnesium content was selected as the criterion for acceptance of dolomite aggregates in Indiana. A high percentage of MgO corresponds directly with a high dolomite concentration. Russell's study of Illinois aggregates showed that percent MgO had a positive correlation with friction value, that is, greater friction values occurred with increases in MgO content. In this study Russell considered dolomite aggregate to consist of >50% dolomite mineral or 10.9% MgO. However, in a study on dolomite aggregates by Bruner, Choi and West (1995), the higher elemental Mg content of dolomite correlates

with smaller PV (Polished Value) and higher WI (Wear Index). This means that dolomites with a high elemental Mg content polish easier under traffic conditions.

The amount of dolomite can be calculated from the percent of elemental Mg or from the percent of MgO in the following way.

$$a) \text{ Mg in Dolomite} = \frac{\text{Molecular Weight of Mg}}{\text{Molecular Weight of } (CaMg)(CO_3)_2} = \frac{24.31g}{184.4g} = 0.1318$$

$$\% \text{ Dolomite} = \frac{\% \text{ Elemental Mg}}{0.1318}$$

Therefore 10.3% elemental magnesium corresponds to 78.1% dolomite, whereas 13.2% corresponds to 100% dolomite and 50% dolomite corresponds to 6.6% Mg.

$$b) \text{ MgO in Dolomite} = \frac{\text{Molecular Weight of MgO}}{\text{Molecular Weight of } (CaMg)(CO_3)_2} = \frac{40.32g}{184.4g} = 0.2186$$

$$\% \text{ Dolomite} = \frac{\% \text{ MgO}}{0.2186}$$

Therefore 10.93% MgO corresponds to 50% dolomite whereas 21.86% MgO corresponds to 100% dolomite and 78.1% dolomite corresponds to 17.07% MgO.

A comparison of the representation of Mg and MgO in dolomite is illustrated in Figure 1-1. As shown in this figure, acceptable dolomite amount for Illinois DOT use ranges from 50 to 100% dolomite (10.93 to 21.86% MgO) whereas for INDOT, acceptable dolomite amounts ranges from 78.1 to 100% dolomite (10.3 to 13.19% Mg).

In a study performed by Shupe (1958), an increase in magnesium content for Indiana limestone sources showed an increase in friction properties. In other words, the less pure limestones having greater magnesium content and thus greater mineral diversity were better aggregates with regard to frictional resistance.

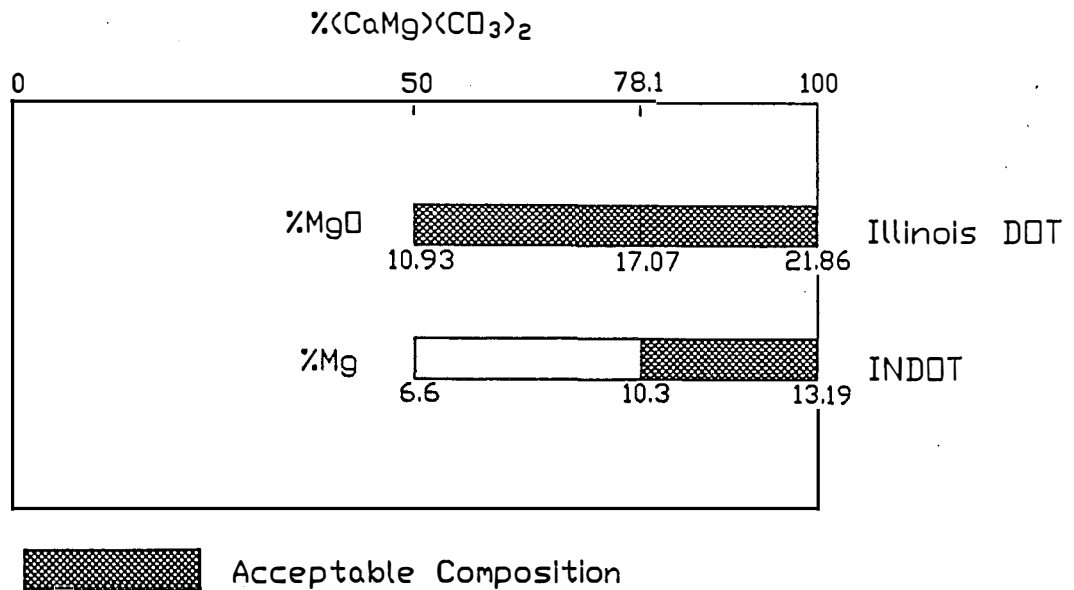


Figure 1-1. Percent dolomite in carbonate aggregates related to percent MgO and Mg content for bituminous surface courses

Russell also observed a negative correlation between friction value and quartz content for these dolomite sources. A general decrease in friction value was also observed with an increase in the calcite concentration.

Russell's study of carbonate aggregates, and that by Cummings (1976) focused on acid-insoluble residue content. Russell's study included carbonate aggregates containing less than 12 percent insoluble residue. A positive correlation was found between acid-insoluble residues >75 microns in size and friction values for the dolomite aggregates; however, little correlation was found between these factors for carbonate aggregates in general. Cummings analyzed aggregates containing up to 25 percent acid-insoluble residue. He found a positive correlation with friction value for dolomite aggregates containing insoluble-residue retained on the No. 200 sieve, as well as with the ratio between the No. 30 and No. 200 sieves. In essence, regarding those aggregates with greater than 70 percent insoluble material retained on the No. 30 sieve, they show poor

results in terms of friction values as compared to samples whose majority of insoluble material occurred on the No. 200 sieve.

Dierstein and LaCroix (1984) also indicated a relationship between acid-insoluble residue and friction values. They concluded that grain size was the primary mechanism affecting variation in friction values for carbonates containing less than 10 percent insoluble content. At greater than 25 percent, the sand size acid-insoluble residue generally accounts for higher friction values.

Dahir and Mullen (1971) observed that skid resistance increases with an increase in the amount of insoluble residue, sand size residue probably is more important than total residue and the presence of clay generally contributes to lower frictional resistance. They also mentioned that skid resistance was higher for aggregate having mixed composition of hard and soft minerals than for aggregate consisting predominantly of minerals of the same type with the same hardness.

Sherwood and Mahone (1970) mentioned the positive relationship between the amount of acid insoluble residue and frictional values of carbonate rocks. Also they noticed that size distribution of the insoluble fraction affects pavement friction values. The better relationship may exist between total insoluble residue and friction coefficient.

In the study by Bruner, Choi and West (1995), involving dolomite aggregates, insoluble residue does not correlate with WI (Wear Index). However, dolomite aggregates containing greater amounts of insoluble residue less than #200 size show a higher PV (Polished Value).

The common physical tests considered to have major influence on aggregate performance are absorption, soundness, abrasion and specific gravity. Senior and Rogers (1991) provided a thorough overview of the first three of these parameters and their significance in evaluating highway aggregates. They indicated that the abrasion loss was increased nine units with additional moisture content of 5 percent. Aggregate type also influences the test results. Generally, weaker argillaceous rocks have a tendency to absorb the impact of the steel balls, whereas coarse-grained crystalline materials do not and they experience higher losses.

The sodium sulfate soundness test is a measure of an aggregate's durability and thus, its resistance to weathering. For INDOT, sodium sulfate testing was previously used to determine aggregate soundness but currently it is performed only occasionally. Because of problems obtaining reproducible results for sodium sulfate testing, unconfined freeze-thaw testing is now used as a replacement.

A relationship between absorption and freeze-thaw resistance is known to exist. Although there are exceptions, aggregates with high absorption generally have lower freeze-thaw resistance. Senior and Rogers (1991) concluded that the following combinations of tests; Los Angeles abrasion loss, sodium sulfate loss, and absorption; can be used to differentiate between good and poor aggregates. However, these tests are not particularly accurate in predicting the performance of marginal aggregates. Therefore, as indicated above, these physical tests have some shortcomings regarding the prediction of field performance for frictional resistance.

Specific gravity is related to composition and absorption. Variations in composition within an aggregate source increase with differential polishing. The presence of compositional variation has been shown to reduce the effects of polishing by vehicular traffic.

In a study by Bruner, Choi and West (1995), for dolomite aggregates, Los Angeles abrasion did not show a correlation with either WI or PV. However, dolomites with higher absorption values show lower WI and higher PV. Also higher specific gravities correlated with lower PV and higher WI. The dolomites showing higher sulfate soundness also show a higher PV and lower WI. However, the correlation between the Los Angeles abrasion loss and frictional resistance was not significant.

## 2. PROBLEM STATEMENT

The highway pavement system requires aggregates of multifunctional characteristics to meet various demands. For bituminous wearing courses these characteristics not only include strength and durability but also adequate frictional resistance. Aggregate having all these properties frequently are not locally available and have to be imported which thereby increases transportation costs.

Natural aggregates used in highway construction in Indiana are crushed carbonate rocks and glacial river gravels. This includes both base courses and aggregates for the pavement itself. Historically, dolomites have been specified for use in surface courses for medium volume roads and a combination of dolomite and blast furnace slag used for surface courses of high volume roads.

Following pavement construction, the frictional resistance of pavement is monitored after specific time intervals using the ASTM procedure, "Towed Friction Trailer" (ASTM Standard E-274). When the FNS (Friction Number Smooth) or FNR (Friction Number Ribbed) fall below a critical value, replacement of the overlay is considered.

In 1995 a 2-1/2 year JHRP study on the contribution of aggregates to frictional resistance of bituminous surfaces was completed under the direction of T.R. West (Bruner, Choi and West, 1995). In this study coupon samples of aggregates were evaluated using the British Polishing Wheel and the British Pendulum Tester. Also a comparison was made to the frictional resistance of pavements as measured by the skid trailer.

Detailed petrographic evaluation and compilation of laboratory data allowed for the correlation of frictional resistance measurements. Because of its prevalent use in medium and high vehicular traffic roads, dolomite was the primary aggregate type studied, along with selected examples of crushed stream gravels. Low volume vehicular traffic roads are those with less than 1 million ESALs, or Equivalent Single Axle Loads, whereas medium volume roads have from 1 to 3 million ESALs. Limestones, because of their limited use in medium and high vehicular traffic roads, were represented by only

two examples in this study. This did not adequately depict the range of characteristics shown by limestone aggregates. Despite this limited number of samples, these limestones performed better in the British Pendulum Tester than did some of the dolomite samples from approved sources.

For high vehicular traffic roads, equal amounts of dolomite and blast furnace slag are used as the coarse aggregate in bituminous surfaces. This is to provide a greater frictional resistance for the pavement surface. Based on past research on carbonate rocks it had been concluded that high purity dolomites provide a better frictional resistance in pavements than do limestones. However, the study by Bruner, Choi and West (1995) noted that impure dolomites having low elemental Mg content provided better frictional resistance. Elemental magnesium is used as an indication of dolomite content as dolomite has the composition  $\text{CaMg}(\text{CO}_3)_2$ . A minimum value of 10.3 % Mg is required for aggregates to qualify as an acceptable dolomite material, which corresponds to 78.1% dolomite (INDOT Specifications, 1999). As high quality dolomites occur in only certain geologic formations in Indiana and therefore, in only certain locations, high quality dolomite aggregates are commonly shipped long distances to produce the bituminous surfaces for paving projects on Interstate highways. These greater transportation distances greatly increase the cost of construction.

Results of the study by Bruner, Choi and West (1995) suggest that some limestones may provide equal or better frictional resistance than do some dolomites that qualify because of their elemental Mg content. In the current study, in order to determine factors other than elemental Mg content that affect high quality limestone performance, various limestone aggregates were evaluated to determine their frictional resistance based on the British Polishing Wheel and Pendulum Tester. Acid insoluble residue and Mg content of carbonates showed considerable promise in the previous study, and therefore would be conducted on the limestone samples as well.



### 3. OBJECTIVES

The objectives of this study were to 1) Extend the knowledge obtained from FHWA/IN/JHRP-95/11 (Bruner, Choi and West, 1995) by evaluating gravels, limestones and sandstones, and expanding the base of dolomites. 2) Find factors that gave some limestones a higher frictional resistance on bituminous pavement, and 3) Develop specifications, if possible, to allow certain limestones and crushed gravels to be used in medium and high vehicular traffic roads.. To accomplish this, additional aggregate types, such as limestones, gravels and selected sandstones would be evaluated. Dolomite had been the focus of the 1995 study because of its prevalent use in surface pavement for medium and high vehicular traffic roads.

For high vehicular traffic roads equal amounts of dolomite and blast furnace slag are used as the coarse aggregate in the bituminous surface. A minimum value of 10.3% elemental Mg is required for aggregates to qualify as an acceptable dolomite material, which corresponds to 78.1% dolomite in the sample. As high quality dolomites occur in only certain geologic formations in Indiana and therefore, in only certain locations, high quality dolomite aggregates are commonly shipped long distances to produce the bituminous surfaces for paving projects on interstate highways. These greater transportation distances significantly increase the cost of construction.

Therefore, this study focuses on high quality limestones. These aggregates were sampled and analyzed in order to find the factors that give limestone higher frictional resistance for bituminous pavement. Results of this study will be used to evaluate frictional resistance provided by various aggregate types. Development of specifications that allow certain limestones and crushed gravels to be used in high vehicular traffic roads is a stated objective. The outcome would be a reduction in cost of materials for road construction or resurfacing of medium and high vehicular traffic projects including interstate highway projects.

#### 4. WORK PLAN

The work plan builds on the results obtained in the previous JTRP study directed by T.R. West (Bruner, Choi and West, 1995). Predominantly, dolomite sources were included in that study. In the current study, additional rock types including limestones, crushed gravels and selected sandstones were chosen from those specifically addressed under the INDOT Specification.

First, through a meeting with SAC members, the aggregate sources including limestones, crushed gravels and sandstones were selected for study. Input from INDOT (Division of Materials and Tests), FHWA and industry (Indiana Mineral Aggregates Association and representatives of aggregate suppliers) was obtained.

A total of 21 sources of limestone aggregates, 3 sources of sandstones and 6 sources of crushed gravels were collected based on bedrock formation, rock types, texture, composition and geographic location (ASTM Standard C702, D75).

Following aggregate collection, megascopic and microscopic observations on the aggregate sources were accomplished. A total of 24 rock thin sections were made for the limestone and sandstone sources.

Also, laboratory tests including acid insoluble residue (ASTM Standard D3042), size distribution of the acid insoluble residue, elemental Mg content (ASTM Standard C602 and Atomic Absorption Method), Los Angeles abrasion (ASTM Standard C131), sulfate soundness (ASTM Standard C88), freeze-thaw loss in water and brine solution (AASHTO Standard Specification, T103 Procedure A), absorption and specific gravity (ASTM Standard C127) were conducted at the INDOT Division of Materials and Tests and at Purdue University. In particular, the acid insoluble residue and Mg content were used for the identifications of quality limestones.

Aggregate coupons of limestones, sandstones and crushed gravels were made for the British Wheel test and British Pendulum test (ASTM Standard D3319, E303). In order to make aggregate coupons, the largest pieces of crushed stone comprising the No.11 INDOT gradation, 12.5mm to 9.5mm in size, were selected. To construct the

coupons the aggregates are embedded in epoxy that has a resin component and a hardener component mixed in a one to one ratio by volume. The aggregate coupons were polished with British Wheel machine and frictional resistance measured with British Pendulum Tester after 0 (Initial Friction Value, IFV), 1 hour, 3 hour, 6 hour and 10 hour (Polish Value, PV) during the polishing process. These data were added to the list of existing information of a similar nature on dolomites from the 1995 study directed by T.R. West (Bruner, Choi and West, 1995).

Finally, analysis of all data to provide results and conclusions relating information from the various tasks was accomplished. Results of this study were compared to those from other states. From this the PV of dolomite and limestone for bituminous surface pavement required for pavement surfaces can be recommended. Also, results can be used to develop specifications yielding the approval of additional aggregate sources for medium and high vehicular traffic bituminous surface pavements in Indiana.

## 5. ANALYSIS OF DATA

### 5.1 Gravel Study

Six gravel sources were selected for this study (Figure 5-1). For these samples a megascopic, petrographic examination was performed and the Initial Friction Value (IFV) and Polish Value (PV) were determined on coupons made from these gravels.

The objective of the petrographic examination for the gravel samples was to describe and classify constituents and determine the relative amounts of these different materials. This method is based on ASTM Standard C295.

The major gravel resources in Indiana are found in landforms deposited directly by glacial melt-water runoff during continental glaciation. At first glance, the composition of the Indiana gravels seems to be a bewildering array of rock and mineral types, and in many samples 10 to 20 varieties of rocks can be found (Carr and Webb, 1970). Included are carbonates, sandstone, siltstone, chert, shale, iron concretion, gneiss, schist, quartzite, granite, granodiorite, diorite, gabbro, andesite, basalt, gabbro, syenite, dacite, rhyolite, amphibolite, and quartz (Shakoor and West, 1979, McGregor, 1960).

#### 5.1.1. Selection of Samples for Examination

Approximately 2.5 kg of a gravel sample were prepared for sieve analysis by reducing the sample material to the required quantity according to ASTM Standard C 702. From this, by sample splitting, 300 representative particles were selected and subsequently examined.

The size ranging from 9.5mm to No. 4 (4.75mm) was chosen and fractions smaller than the No. 4 were not included in the analysis. This is because it is generally accepted that the coarse aggregate portion largely determines the skid resistance for bituminous pavements, and the finer portion represents only ten to thirty percent of the total No. 11 size sample used for bituminous surface aggregate. All particles present in the size fraction were examined if they numbered fewer than 300 particles.

# Aggregate Sources

- ▲ Dolomite (Part 1)
- ★ Limestone (Part 2)
- Sandstone (Part 2)
- Gravel (Part 2)

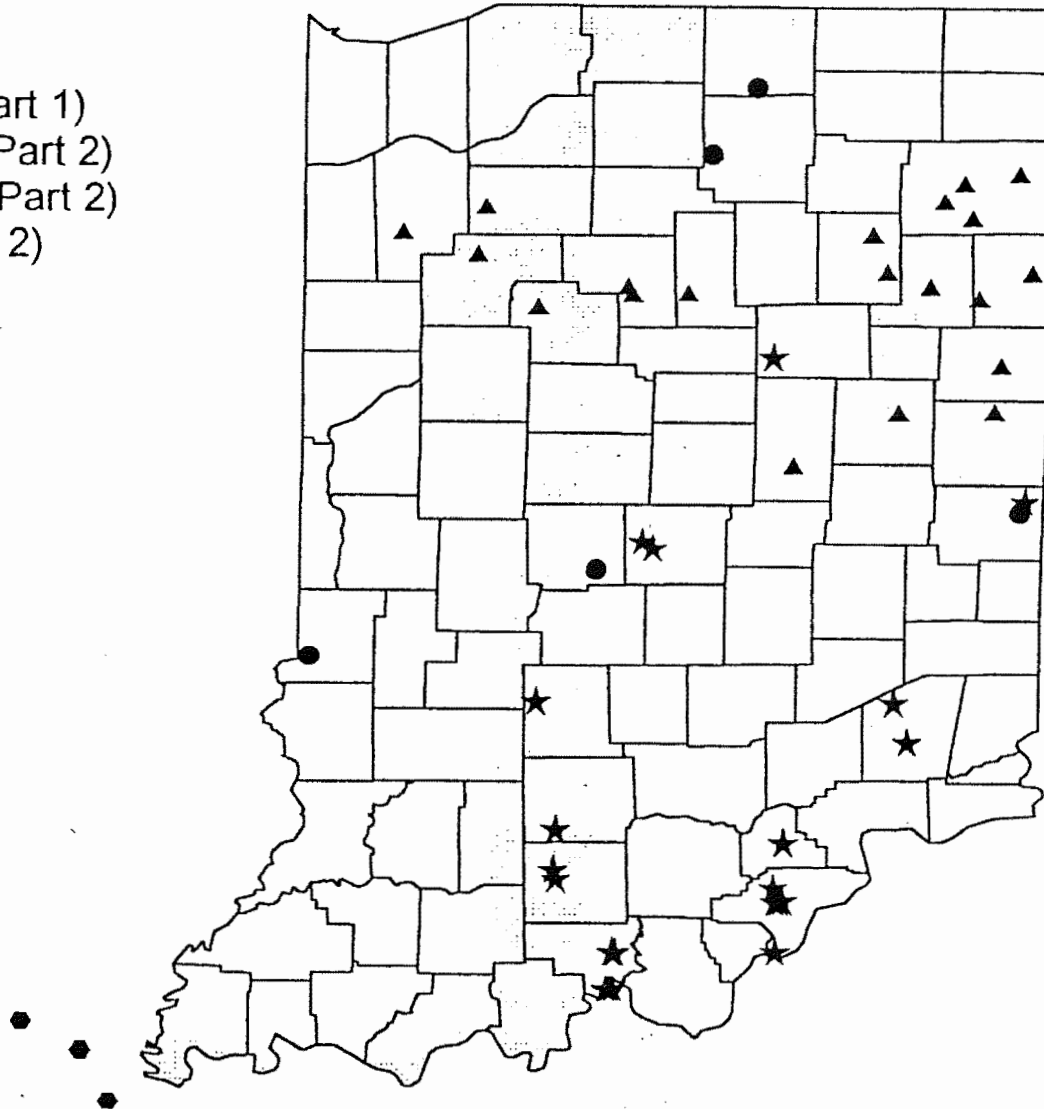


Figure 5-1. Location map of aggregate sources

### 5.1.2 Examination of Gravels

Rock particles were identified in a wet surface condition that enhances the color and structure of the particles. Hand lens and binocular microscope were used to identify individual rock constituents. Following this, 0.1 N HCl was applied on soft rocks to differentiate carbonates from other constituents. Limestones produced a brisk effervescence whereas dolomite showed slow effervescence or produced effervescence only when scratched. Ten rock types were chosen according to their frequency in the sample. If particles of indeterminate type were encountered in the samples, they were included with known types having similar texture and hardness. The ten rock categories selected are limestone, sandstone, siltstone, shale, chert, granite, diorite, felsite, gneiss, and quartzite.

The percentage of crushed particles was determined according to the definition of a fractured face in the Indiana Test Method (ITM) 204. It reads as follows: Fractured surface: A broken surface constituting an area of at least 25% of the largest cross sectional area of the particle. A fractured particle is defined as one being fractured either by mechanical means or by nature. Natural fractures must be similar to those fractures produced by a crusher.

The ten rock types were collected into five major rock types, that is, igneous, metamorphic, limestone, dolomite and other sedimentary rocks. The petrographic analyses for the gravels are summarized in Table 5-1.

Results show that gravel samples examined consist mainly of carbonates (limestone and dolomite) ranging from 6.6 percent to 56.3 percent. Igneous rock is the second most abundant constituent of the gravel, ranging from 9.0 percent to 49.2 percent, followed next by other sedimentary rocks ranging from 16.0 percent to 46.9 percent. Metamorphic rocks consist of a portion ranging between 3.6 percent and 18.7 percent of gravel samples (Figure 5-2).

### 5.1.3 Physical Properties of Gravels

The physical test results for gravel samples were obtained from INDOT, These test results are listed in Table 5-2.

According to results of the physical properties of the gravels, absorption ranges from 1.21 % to 3.21 %, specific gravity from 2.509 to 2.676, Los Angeles abrasion loss from 22.64 % to 28.00 % and the freeze and thaw loss in brine solution from 4.49 to 9.18. For sample GR-6 the freeze and thaw loss in water was performed instead, resulting in 2.06 % loss.

#### 5.1.4 Estimation of IFV and PV for Gravel Samples

The aggregate coupons for gravel samples were made using single individual rock types. The Initial Friction Value (IFV) and Polished Value (PV) were measured using the British Pendulum tester. The IFV and PV for gravel aggregates are listed in Table 5-3: According to the results of the British Pendulum test, igneous rocks in gravels have an IFV ranging from 32.2 to 45.6 and a PV ranging from 21.0 to 29.9. Aggregate coupons of metamorphic rocks were made only from GR-1 and GR-6 gravels because not enough metamorphic rock pieces in other gravel sources were present. The metamorphic rocks in gravels show IFV ranging from 39.6 to 43.3 and PV ranging from 27.5 to 28.3. The limestone in gravels show IFV ranging from 33.2 to 46.6 and PV ranging from 18.6 to 31.4. The dolomite in gravels show IFV ranging from 40.2 to 46.7 and PV ranging from 24.5 to 27.3. The other sedimentary rocks such as sandstone, shale, siltstone and chert show IFV ranging from 35.2 to 43.7 and PV ranging from 21.6 to 25.3.

As indicated in Table 5-3, it is noted that on average dolomites show higher IFV and PV values than do limestones. Weighted averages of IFV and PV for each gravel sample based on their percentage of rock compositions are summarized in Table 5-4.

As indicated in Figures 5-3 and 5-4, as the percentage of crushed faces of gravel increases, both IFV and PV also increase. In Table 5-5, the percent of crushed gravel faces is more highly correlated with IFV ( $R=0.890$ ) than with PV ( $R=0.804$ ). IFV depends more on the percent of the crushed gravel than it does on PV. This shows that the influence of the amount of crushed gravel faces is diminished as the gravels are polished.

Table 5-1. Rock type distribution in gravel samples

Id. No.	Igneous Rock	Metamorphic Rock	Limestone	Dolomite	Other Sedimentary Rock	Remarks
GR-1	27.5	11.1	19.5	20.8	21.1	99% crushed
GR-2	35.8	3.6	20.8	21.5	18.3	30% crushed
GR-3	29.7	6.8	14.1	14.7	34.7	10% crushed
GR-4	49.2	8.7	17.5	8.6	16.0	78% crushed
GR-5	9.0	3.7	15.9	40.4	31.0	57% crushed
GR-6	27.8	18.7	6.2	0.4	46.9	93% crushed
Average	29.8	8.8	15.7	17.7	28.0	61% crushed

Table 5-2. Physical properties of gravel samples

Id. No.	Absorption	Specific Gravity	L.A. Abrasion Loss	Freeze/Thaw Loss in Brine
GR-1	3.21%	2.590	27.99%	9.18%
GR-2	1.42%	2.593	26.26%	4.49%
GR-3	1.21%	2.676	22.64%	5.67%
GR-4	1.33%	2.638	24.22%	4.64%
GR-5	1.57%	2.645	23.19%	5.77%
GR-6	2.03%	2.509	28.00%	*2.06%
Average	1.80%	2.609	25.38%	5.95%

- \*: Freeze - thaw loss in water



Table 5-3. IFV and PV for rock constituents in gravel samples

Id. No.	Igneous Rock		Metamorphic Rock		Limestone		Dolomite		Other Sedimentary Rock	
	IFV	PV	IFV	PV	IFV	PV	IFV	PV	IFV	PV
GR-1	43.3	26.3	43.3	28.3	43.3	25.3	46.7	27.3	43.7	25.3
GR-2	32.2	21.0	-	-	33.2	23.1	41.7	24.6	36.6	23.1
GR-3	34.2	21.0	-	-	34.2	24.1	40.2	26.9	35.2	24.1
GR-4	40.3	23.7	-	-	36.3	25.3	42.3	25.3	35.3	23.6
GR-5	40.3	24.3	-	-	35.7	18.6	41.6	24.5	37.2	22.6
GR-6	45.6	29.9	39.6	27.5	46.6	31.4	-	-	35.7	21.6
Average	39.3	24.4	41.5	27.9	38.2	24.6	42.5	25.7	37.3	23.4

Table 5-4. Weighted average of IFV and PV for gravel samples

Identification No.	IFV	PV	Remark
GR-1	44.1	26.3	99% crushed
GR-2	35.3	22.6	30% crushed
GR-3	35.6	23.5	10% crushed
GR-4	39.0	24.1	78% crushed
GR-5	39.1	22.9	57% crushed
GR-6	39.9	25.6	93% crushed
Average	38.8	24.2	61% crushed

\* Example (GR-1):

$$\text{IFV} = 43.3(0.275)+43.3(0.111)+43.3(0.195)+46.7(0.208)+43.7(0.211)=44.1$$

$$\text{PV} = 26.3(0.275)+28.3(0.111)+25.3(0.195)+27.3(0.208)+25.3(0.211)=26.3$$

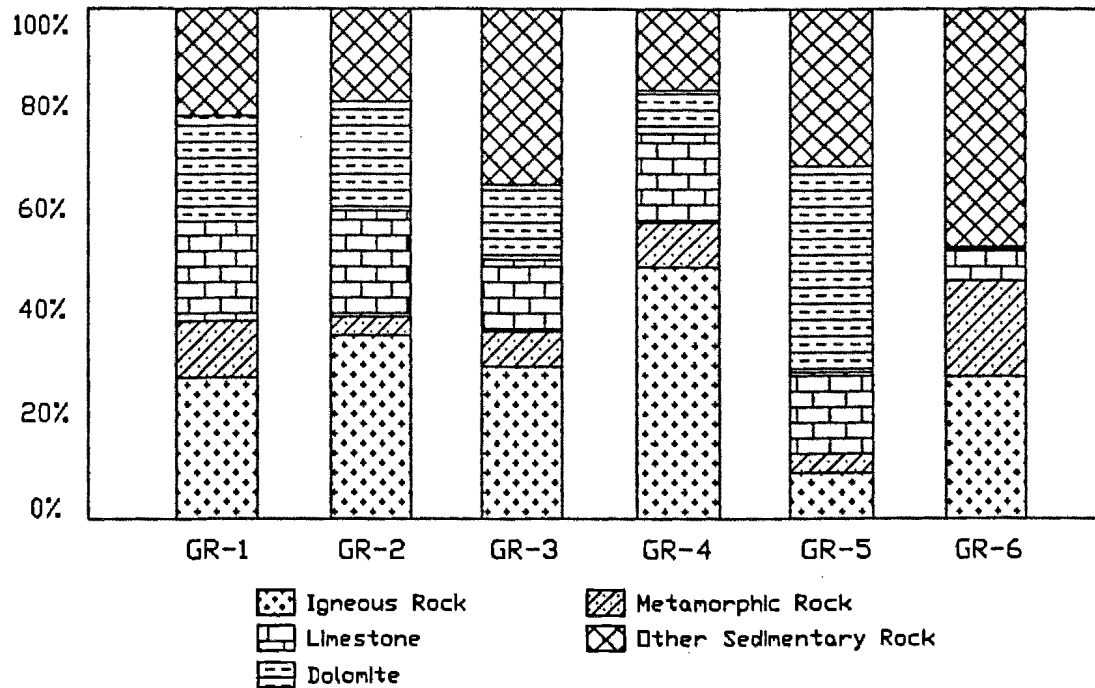


Figure 5-2. Rock type distribution of gravel samples

### 5.1.5 Correlation of Frictional Properties for Gravel Samples

In order to find the most critical factor for frictional properties of gravels, data analyses for correlations between British Pendulum Number (BPN) and other aggregate properties were performed. Using a correlation analysis (SAS program), the statistics and correlation matrix for all parameters of the gravels were developed as shown in Table 5-5. The top number (R-value) in each cell is the coefficient of correlation between two variables defining the cell. R-value measures the degree of linear relationship among variables. The middle number (P-value) in each cell is developed from hypothesis testing and indicates the significance of the correlation; lower P-values imply greater significance. The bottom number in each cell is the number of data used for correlation analysis. For the purpose of this study, the correlation coefficient (R-value) is considered to be low if it falls below 0.50, fair if between 0.50 and 0.80, and good if above 0.80 (Kandhal et al., 1993).

## **5.2. Carbonate Rock Study**

Results of the current research on limestone aggregates along with those on dolomite aggregates by Bruner, Choi and West (1995) were analyzed collectively. The total number of samples was 21 limestone sources and 20 dolomite sources. The bedrock distribution of principal carbonate rocks and their columnar sections are shown in Figures 5-5 and 5-6, respectively. The lithology of the geologic formations of the carbonate aggregates is described below based on the Indiana Geological Survey Bulletin 59, 1986 and Bulletin 42H, 1971, extending from oldest to youngest rocks.

### [Silurian Period]

- Brassfield Limestone
  - Reddish brown and gray coarsely crystalline limestone.
  - Total, 1 source: L006
- Salamonie Dolomite
  - White to tan porous, fossil-fragmental dolomite. In some places this dolomite grades in two units: an upper cherty limestone and dolomite unit (Laurel Member) and a lower argillaceous dolomitic limestone unit (Osgood Member).
  - Total, 8 sources: D002, D010, D011, D012, L003, L009, L010, L011.
- Louisville Limestone
  - Bluish-gray silty dolomitic limestones that contain varying amounts of shale. In some areas, this rock is a blue and gray mottled dolomite.
  - Total, 3 sources: D008, L008, L012
- Pleasant Mills Formation
  - Tan to brown micritic to fine-grained and sugary dolomite that on outcrop appears thin to rather massively bedded but that is also color banded and faintly laminated. Oolites are abundant.
  - Total, 3 sources: D007, D009, D020.

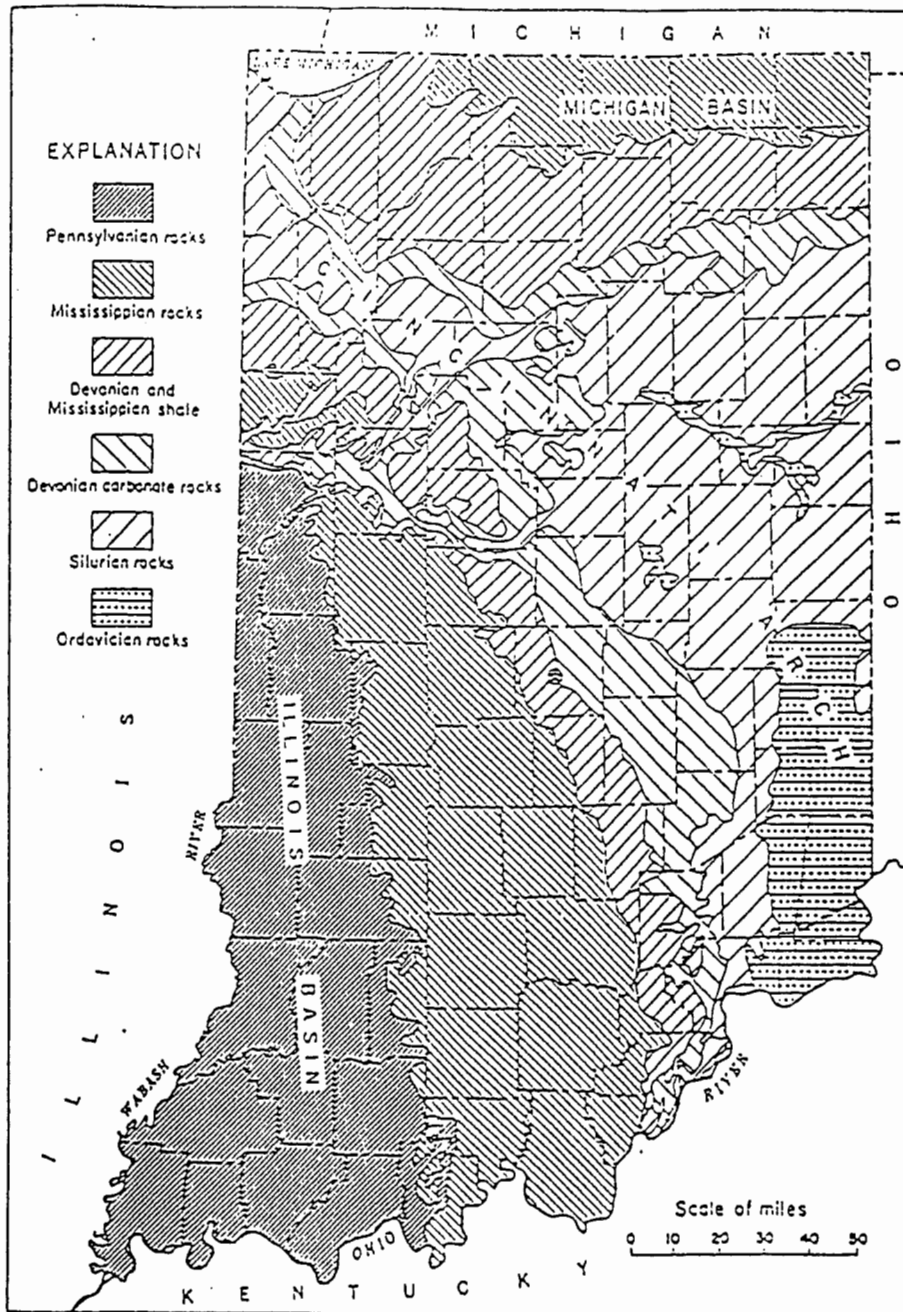


Figure 5-5. Generalized bedrock map of Indiana (Dawson and Carpenter, 1963)

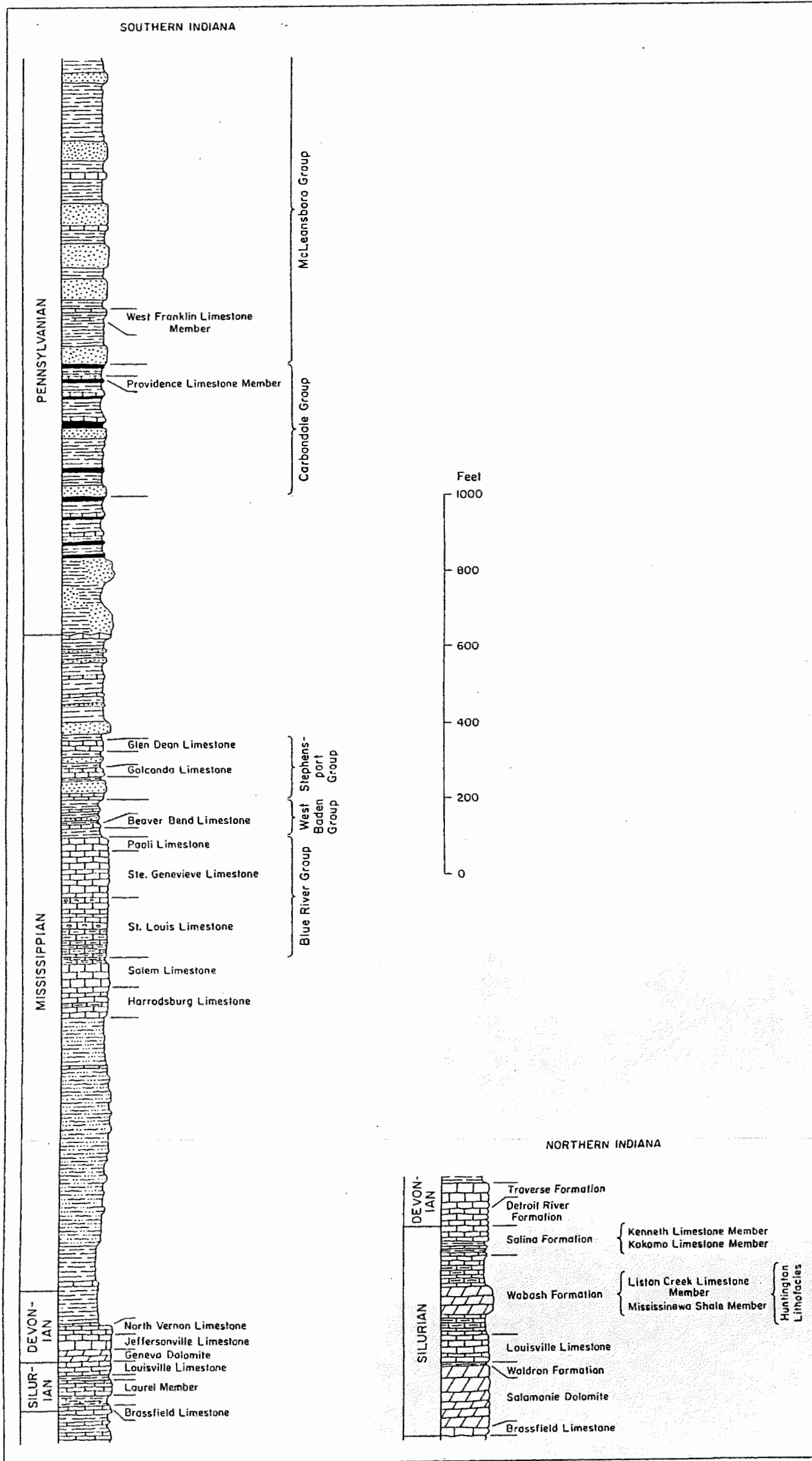


Figure 5-6. Generalized columnar sections showing rock units, except Ordovician, quarried for crushed stone in Indiana (D. D. Carr et al, 1971)

- Wabash Formation
  - Kenneth Member: light-colored dense to fine-grained bedded to massive cherty limestone.
  - Kokomo Member: strikingly banded tan and gray micritic to very fine grained, thinly laminated and dolomitic limestone.
  - Mississinewa member: argillaceous dolomitic siltstone and silty dolomite, fairly calcareous in places, that is in various shades of gray and is dense to fine grained and massive appearing in un-weathered exposures.
  - Huntington dolomite: reef, bank, and biohermal deposits of coarse-grained porous dolomite.
  - Total, 10 sources: D001, D004, D005, D013, D014, D015, D016, D017, L001, L004.

[Devonian Period]

- Jeffersonville Limestone
  - Grayish-brown carbonaceous limestone or dolomitic limestones. Three fauna zones are present in the formation: the middle and upper zones contain very thin argillaceous and pyretic laminae in places. The lower zone generally does not contain the argillaceous and pyretic laminae.
  - Total, 2 sources: L007, L013.
- North Vernon Limestone
  - Gray dense massive argillaceous limestone (Silver Creek Member)
  - Gray granular to shaly thin-bedded very fossiliferous limestone (Speed Member)
  - Gray and dark gray medium-grained and very coarse-grained crinoidal limestone (Beechwood Member)
  - Total, 1 source: L002 (mixed with Jeffersonville Limestone).
- Detroit River Formation
  - Brownish-gray fine- to coarse-grained limestones and dolomites.
  - Total, 3 sources: D003, D018, D019 (mixed with Traverse Formation, Wabash Formation or Louisville Limestone)

- Traverse Formation
  - Brownish-gray calcareous dolomites and dolomitic limestones that contain a few scattered chert bands and shaly layers near the base.
  - Total, 3 sources: D003, D018, D019 (mixed with Detroit River Formation, Wabash Formation or Louisville Limestone)

[Mississippian period]

- St. Louis Limestone
  - Gray to light brown thin-bedded micritic limestone. Contains small amounts of shale and chert (Upper unit)
  - Gray to brown thin-bedded dolomitic limestone interbedded with calcareous shale (Lower unit)
  - Total, 1 source: L005 (mixed with Ste. Genevieve limestone)
- Ste. Genevieve Limestone
  - Fredonia member: dominantly an oolitic limestone that is from light gray to gray, medium grained.
  - Rosiclare member: gray, fine- to medium-grained, crystalline, oolitic limestone, which is locally argillaceous, arenaceous, or conglomeratic. May contain beds of calcareous shale and sandstone.
  - Levias Member: light-gray, thin-bedded to massive, crystalline, oolitic limestone
  - Total, 2 sources: L015, L016.
- Beaver Bend Limestone
  - White to light gray, highly oolitic, thick-bedded to massive limestone.
  - Total, 5 sources: L017, L018, L019, L020, L021.
- Haney Limestone
  - Dominantly biomicritic and includes skeletal limestone and micritic dolomite with minor shale.
  - Total, 1 source: L014

Table 5-6 BPN and Physical Properties, Mg contents and Insoluble residues

A. Limestone

Id. No.	IFV	PV	WI	Absorption	Sp. Gravity	LA Abrasion	F-T	Mg (%)	Insol (Total)	>#200	<#200
L-001	38.60	28.14	10.46	2.170	2.591	28.500		5.40	12.72	2.40	10.32
L-002	40.20	24.72	15.48	1.780	2.586	27.000	22.250	0.70	5.76	3.64	2.12
L-003	44.38	22.69	21.69	2.000	2.604	21.370	7.980	9.50	4.09	2.04	2.05
L-004	43.14	23.49	19.65	1.030	2.633	32.090		3.60	3.37	0.24	3.13
L-005	39.48	21.67	17.81	0.930	2.652	28.600		2.20	4.96	1.53	3.43
L-006	43.00	20.87	22.13	1.350	2.631	37.230	10.920	0.60	1.97	0.15	1.82
L-007	41.13	25.50	15.63	1.260	2.624		16.620	0.90	4.54	2.36	2.18
L-008	41.76	24.97	16.79	1.150	2.653	26.490	27.460	2.50	6.00	0.70	5.30
L-009	42.26	27.40	14.86	2.065	2.642	25.935	15.430	9.05	6.38	0.30	6.08
L-010	45.30	25.39	19.91	2.330	2.621	23.030	19.470	10.00	6.17	1.80	4.37
L-011	43.68	25.39	18.29	0.780	2.698	32.240	7.460	4.60	5.14	0.86	4.28
L-012	46.69	27.51	19.18	1.005	2.670	25.730		2.30	6.04	0.60	5.44
L-013	48.87	29.24	19.63	1.050	2.639	27.720		1.90	11.21	4.83	6.38
L-014	44.10	25.00	19.10	1.530	2.626	28.490	8.060	1.60	4.13	1.43	2.70
L-015	43.21	27.12	16.09	0.880	2.638	22.400		1.10	4.41	1.86	2.55
L-016	41.02	24.72	16.30					1.10	4.59	1.16	3.43
L-017	47.85	25.73	22.12	2.470	2.517	38.170	3.010	0.50	1.41	0.14	1.27
L-018	44.69	22.55	22.14	1.970	2.592	34.760		0.70	2.28	0.12	2.16
L-019	42.37	23.95	18.42	1.320	2.634	24.790	11.300	0.70	4.01	0.64	3.37
L-020	40.10	22.05	18.05	1.150	2.689	24.390		1.20	3.61	0.47	3.14
L-021	42.50	22.01	20.49	1.690	2.639	26.705	5.380	1.00	3.60	0.16	3.44
Average	43.06	24.77	18.30	1.496	2.629	28.192	12.945	2.91	5.07	1.31	3.76



**B. Dolomite**

Id. No.	IFV	PV	WI	Absorption	Sp. Gravity	LA Abrasion	Sulfate	Mg (%)	Insol (Total)	>#200	<#200
D-001	40.00	24.90	15.10	0.79	2.732	25.53	0.36	12.40	1.70	1.23	0.48
D-002	43.30	25.00	18.30	2.39	2.599	28.59		12.90	0.71	0.02	0.69
D-003	41.80	27.30	14.50	2.64	2.605	27.05	9.57	11.30	3.13	2.07	1.06
D-004	46.90	35.30	11.60	6.25	2.390	29.36	9.86	11.30	2.11	0.11	2.00
D-005	44.00	27.70	16.30	2.02	2.616	29.19	7.30	11.60	9.82	3.55	6.28
D-006	42.70	26.80	15.90	1.56	2.626	26.62	2.24	12.40	5.29	1.79	3.50
D-007	44.00	28.80	15.20	3.05	2.583	32.43	4.79	12.90	1.34	0.21	1.13
D-008	44.60	28.70	15.90	2.38	2.594	31.73	0.76	13.10	0.22	0.03	0.20
D-009	44.80	31.00	13.80	1.21	2.671	22.47	5.59	10.70	7.42	4.40	3.02
D-010	45.90	31.30	14.60	3.76	2.489	36.98	3.29	12.60	4.16	1.81	2.35
D-011	46.20	30.00	16.20	2.02	2.632	24.75	5.99	12.20	6.31	2.71	3.59
D-012	47.60	31.30	16.30	2.74	2.588	29.50	2.24	11.90	7.65	3.39	4.26
D-013	43.50	32.70	10.80					10.10	5.56	0.78	4.78
D-014	46.70	32.00	14.70	4.00	2.480	30.28	13.18	11.20	5.27	0.71	4.55
D-015	40.50	24.10	16.40	1.03	2.669	29.76	2.56	12.80	0.50	0.27	0.22
D-016	40.10	23.60	16.50	1.86	2.614	32.62	0.55	12.90	0.08	0.02	0.06
D-017	41.30	24.90	16.40	1.00	2.718	25.69	0.31	12.90	0.35	0.03	0.32
D-018	43.60	28.40	15.20	2.48	2.619	27.25	3.62		2.15	0.94	1.21
D-019	41.60	28.00	13.60	1.59	2.648	30.75	6.45		6.66	4.45	2.21
D-020	41.90	28.10	13.80	2.95	2.590	28.54	5.33		5.90	3.14	2.76
Average	43.55	28.50	15.06	2.41	2.60	28.90	4.67	12.07	3.82	1.58	2.23

### 5.2.1 Aggregate Lithology and Friction Resistance Properties

Friction resistance and other aggregate properties for limestone and dolomite samples are listed in Table 5-6. In order to determine how a specific geological formation relates to fictional properties, aggregate sources sampled from only one geological formation were used in the correlation analysis. The IFV and PV for each geological formation are shown in Table 5-7.

The Wabash Formation is divided into four members these being the Kenneth, Kokomo, Mississinewa and Huntington. Of the samples obtained from the Wabash Formation, L004 and D016 were excluded from consideration because these two samples were so difficult to assign to specific rock members.

As shown in Table 5-7, for the dolomite aggregates, the Kokomo Member (Upper Silurian) shows the highest PV (average 33.3) and the Huntington Dolomite (Upper Silurian) shows the lowest PV (average 25.6). For the limestone aggregates, Mississinewa Member (Upper Silurian) shows the highest PV (28.1) and Brassfield Limestone (Lower Silurian) shows the lowest PV (20.9).

In order to determine the factors affecting the frictional properties of dolomite and limestone aggregates, elemental magnesium content and acid insoluble residue value for the geological formations were determined. This was because INDOT currently uses the elemental magnesium content analysis as the acceptance criteria for dolomite sources with the minimum level set at 10.3%. Also impure dolomite and impure limestone have been known to be quality aggregate sources.

Elemental magnesium contents and acid insoluble residues for the various geological formations are provided in Table 5-8.

As presented in Table 5-8, considering elemental Mg content, for dolomite aggregates, Louisville Limestone (Middle Silurian) shows the highest elemental Mg content (13.1%) and the Kokomo Member, which has the highest PV, shows the lowest elemental Mg content (10.9%).

Table 5-7. Friction resistance properties of carbonate aggregate samples.

Formation		Dolomite			Limestone			Remark
		IFV	PV	WI	IFV	PV	WI	
W a b a s h Fm	Kokomo Mbr.	43.5- 46.9 (45.7)	32.0- 35.3 (33.3)	10.8- 14.7 (12.4)	-	-	-	D004, D013 D014
	Mississinewa Mbr.	-	-	-	38.6	28.1	10.5	L001
	Huntington Dol.	40.0- 44.0 (41.5)	24.1- 27.7 (25.4)	15.1- 16.4 (16.1)	-	-	-	D001, D005 D015, D017
Salamonie Dolomite		43.0- 47.6 (45.7)	25.0- 31.3 (29.4)	14.6- 18.3 (16.4)	42.3- 45.3 (43.9)	22.7- 27.4 (25.2)	14.9- 21.7 (18.7)	L003, L009 L010, L011 D002, D010 D011, D012
Brassfield Limestone		-	-	-	43.0	20.9	22.1	L006
Jeffersonville Limestone		-	-	-	41.1- 48.9 (45.0)	25.5- 29.2 (27.4)	15.6- 19.6 (17.6)	L007, L013
Louisville Limestone		44.6	28.7	15.9	41.8- 46.7 (44.2)	25.0- 27.5 (26.2)	16.8- 19.2 (18.0)	L008, L012 D008
Haney Limestone		-	-	-	44.1	25.0	19.1	L014
Ste. Genevieve Limestone		-	-	-	41.0- 43.2 (42.1)	24.7- 27.1 (25.9)	16.1- 16.3 (16.2)	L015, L016
Beaver Bend Limestone		-	-	-	40.1- 47.9 (43.5)	22.0- 25.7 (23.3)	18.1- 22.1 (20.2)	L017, L018 L019, L020 L021
Pleasant Mills Formation		41.9- 44.8 (43.6)	28.1- 31.0 (29.3)	13.8- 15.2 (14.3)	-	-	-	D007, D009 D020

For limestone aggregates, Salamonie Dolomite (Lower Silurian) shows the highest elemental Mg content (8.29%) and Brassfield Limestone with the lowest PV shows the lowest elemental Mg content (0.6%).

It is important to consider the following:

For dolomite aggregates:

- The lower the elemental Mg content, the more impure is the dolomite
- The lower the elemental Mg content, the higher is the PV (Figure 5-7).

higher the elemental Mg content, the more impure is the limestone

- The higher the elemental Mg content, the higher is the PV (Figure 5-8).

Considering acid insoluble residue, for dolomite aggregates, the Pleasant Mills Formation (Middle Silurian) has the highest acid insoluble residue (4.89%) and Louisville Limestone (Middle Silurian) has the lowest insoluble residue (0.22%).

For limestone aggregates, the Mississinewa Member (Upper Silurian) has the highest PV but also indicates the highest insoluble residue (12.72%). Brassfield Limestone (Lower Silurian) has the lowest PV and also has the lowest insoluble residue (1.97%).

It is also important to consider that:

For both dolomite and limestone aggregates:

- The higher the acid insoluble residue (fine quartz and clay), the more impure are the dolomites and limestones
- The higher the acid insoluble residue, the higher the PV (Figures 5-9 and 5-10).

As shown in Figures 5-9 and 5-10, for the dolomite case, the PV is correlated more highly with elemental Mg content ( $R=0.707$ ,  $R^2=0.50$ ) than is the acid insoluble residue ( $R=0.318$ ). Unlike dolomite, in the case of limestone, PV is more highly correlated with acid insoluble residue ( $R=0.853$ ,  $R^2=0.73$ ) than with elemental Mg content ( $R=0.376$ ,  $R^2=0.14$ ).

Table 5-8. Elemental Mg content of carbonate aggregate samples.

Formation		Dolomite		Limestone		Remark
		Elemental Mg Content (%)	Acid Insoluble Residue (%)	Elemental Mg Content (%)	Acid Insoluble Residue (%)	
W a b a s h Fm	Kokomo Mbr	10.1-11.3 (10.9)	2.11-5.56 (4.31)	-	-	D004, D013 D014
	Mississinewa Mbr.	-	-	5.40	12.72	L001
	Huntington Dol.	11.6-12.9 (12.4)	0.35-9.82 (3.09)	-	-	D001, D005 D015, D017
Salamonie Dolomite		11.9-12.9 (12.4)	0.71-7.65 (4.71)	4.60-10.00 (8.29)	4.09-6.38 (5.45)	L003, L009 L010, L011 D002, D010 D011, D012
Brassfield Limestone		-	-	0.6	1.97	L006
Jeffersonville Limestone		-	-	0.9-1.9 (1.4)	4.54-11.21 (7.88)	L007, L013
Louisville Limestone		13.1	0.22	2.3-2.5 (2.4)	6.00-6.04 (6.02)	L008, L012 D008
Haney Limestone		-	-	1.6	4.13	L014
Ste. Genevieve Limestone		-	-	1.1 (1.1)	4.41-4.59 (4.50)	L015, L016
Beaver Bend Limestone		-	-	0.5-1.2 (0.82)	1.41-4.01 (2.98)	L017, L018 L019, L020 L021
Pleasant Mills Formation		10.7-12.9 (11.8)	1.34-7.42 (4.89)	-	-	D007, D009 D020

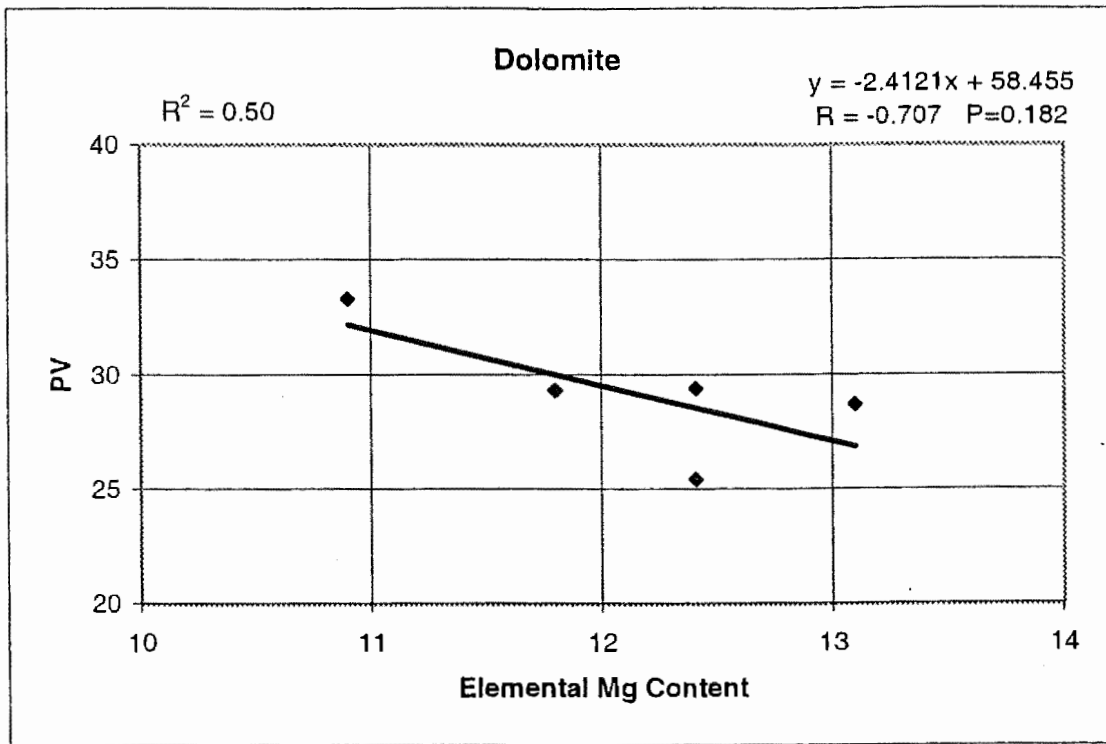


Figure 5-7. Elemental Mg content of dolomite samples

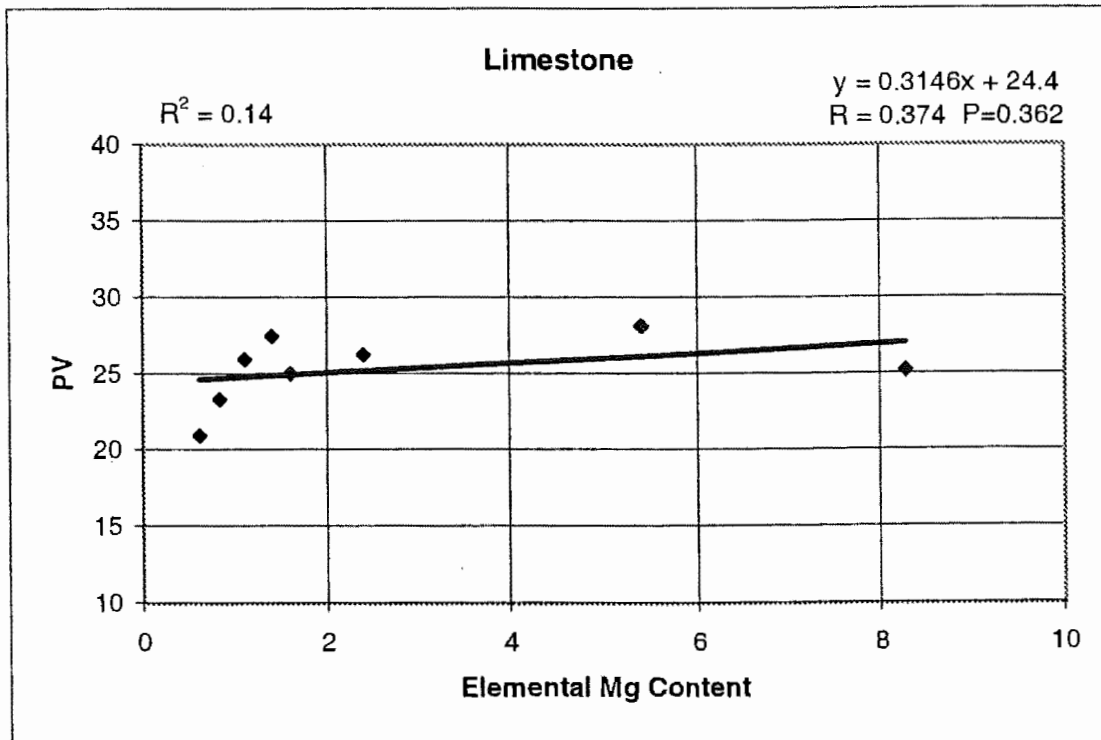


Figure 5-8. Elemental Mg content of limestone samples

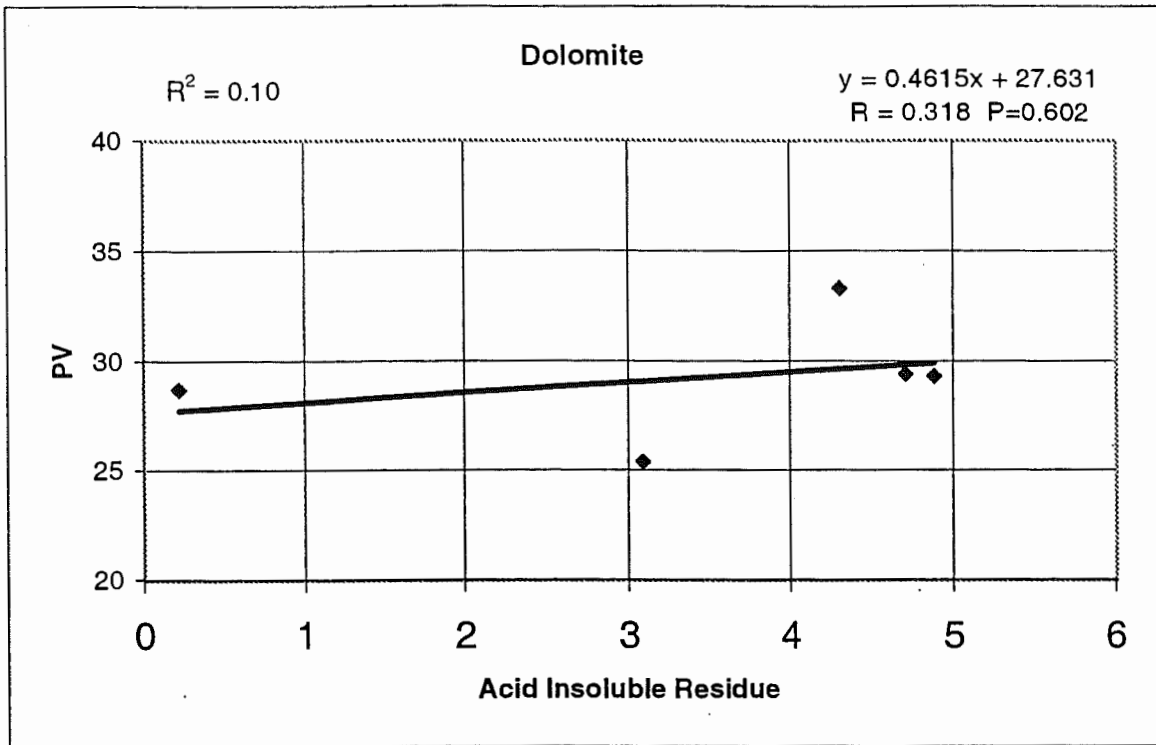


Figure 5-9. Acid insoluble residue of dolomite samples

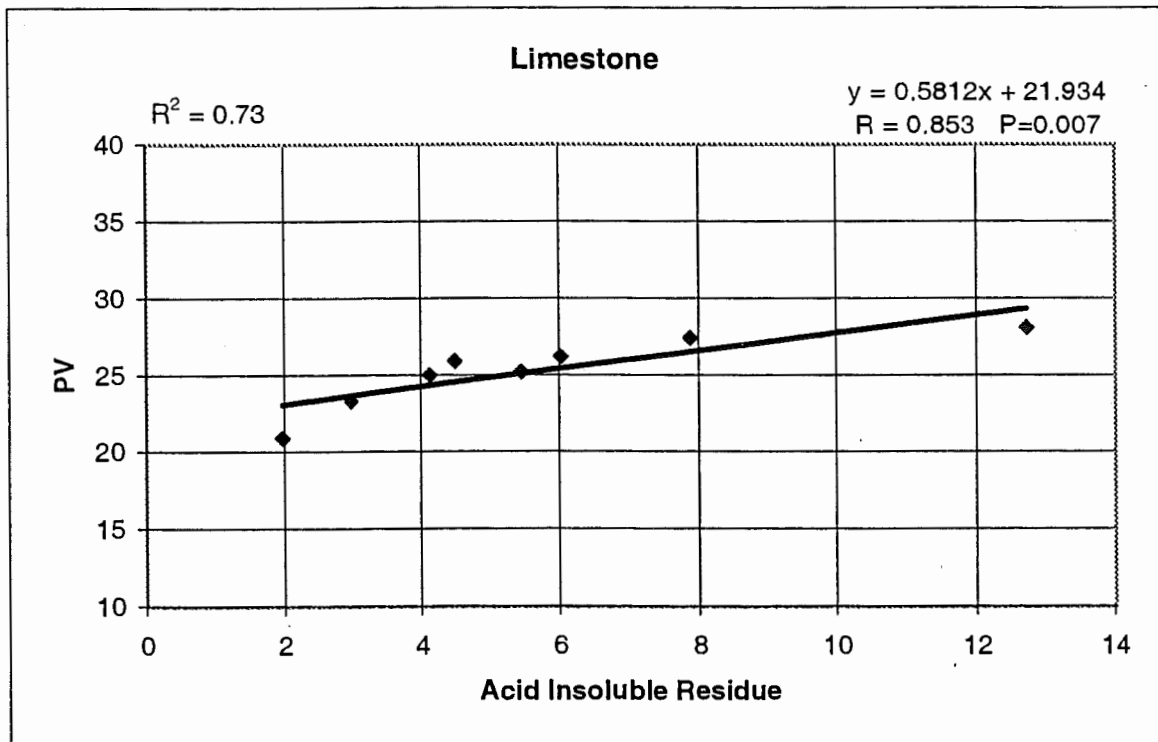


Figure 5-10. Acid insoluble residue of limestone samples

## 5.2.2 Physical and Frictional Resistance Properties of Carbonate Aggregates

Data on the physical properties for representative samples of dolomite and limestone were obtained from INDOT.

For the dolomite study (Bruner, Choi and West, 1995), sulfate soundness results were used for correlation with frictional properties but in the subsequent limestone study, freeze-thaw data were used as a replacement for sulfate soundness data. In the time between the two studies, INDOT changed its standard testing from the sodium sulfate to the freeze and thaw test.

Among the physical tests, specific gravity, absorption and sodium sulfate soundness, correlated best with the PV and WI for dolomite. However, for limestone only freeze-thaw correlated with WI. Los Angeles abrasion did not correlate with IFV or PV at a significant probability level for either dolomite or limestone. The physical tests are summarized in Table 5-6.

### 5.2.2.1 Specific Gravity

Specific gravity can be considered as a measure of mineral purity of a sample. As a consequence of large amounts of impurities, the specific gravity will be reduced from that of a more pure sample. As mentioned in a previous section, the more impure samples provided greater polish resistance. The specific gravity for dolomites generally ranges from 2.86 to 3.10. Dolomites in the previous study ranged from 2.390 to 2.732, with an average of 2.60. Specific gravity for limestones generally ranges from 2.71 to 2.83. Limestone in this study ranged from 2.519 to 2.698 with an average of 2.63.

As can be observed in Figures 5-11 and 5.12, for dolomite, PV decreases with increasing specific gravity ( $R=-0.745$ ,  $R^2=0.56$  and  $P=0.000$ ) and WI increases with increasing specific gravity ( $R=0.462$ ,  $R^2=0.21$  and  $P=0.046$ ). Therefore, PV is increased for samples with a lower specific gravity, thus indicating greater polish resistance with increasing impurity and decreasing specific gravity.

As shown in Figures 5-13 and 5-14, for limestone samples, PV and WI do not correlate with the specific gravity, having  $R=-0.073$  and  $P=0.761$  for PV and  $R=-0.127$  and  $P=0.593$  for WI.



#### 5.2.2.2 Absorption

Materials with a high absorptive capacity are generally weaker aggregates. However, regarding frictional resistance, a weaker fraction yields a rougher surface during polishing. For dolomites, as absorptive capacity increases, PV increases ( $R=0.765$ ,  $R^2=0.59$  and  $P=0.000$ ) and WI decreases ( $R=-0.538$ ,  $R^2=0.29$  and  $P=0.018$ ) as shown in Figures 5-15 and 5-16.

For limestones, PV and WI do not correlate with absorption as indicated in Figures 5-17 and 5-18. For dolomite, the presence of weaker material aids in the reduction of the polishing effect. This is because the weaker materials easily absorb water and increase micro-texture on aggregate surface causing an increase in PV. However, this phenomenon is not significant for the limestone samples.

#### 5.2.2.3 Sodium Sulfate Soundness

This test is a measure of aggregate durability and is related to absorption. As shown in Figures 5-19 and Figure 5-20, for dolomites, as sulfate soundness loss increases, PV increases ( $R=0.603$ ,  $R^2=0.36$  and  $P=0.008$ ) and WI decreases ( $R=-0.567$ ,  $R^2=0.32$  and  $P=0.014$ ).

For limestones, the sulfate soundness loss data were too few in number to provide a meaningful analysis. Therefore, freeze-thaw data were used for the comparison with PV and WI instead of with sulfate soundness data.

#### 5.2.2.4 Freeze and Thaw Loss

In the Indiana specification, 50 cycles of freezing and thawing can be used for aggregate verification instead of the sodium sulfate soundness test. For the limestone aggregate study freeze and thaw loss results were used instead of those for the sulfate soundness test.

As shown in Figures 5-21 and 5-22, PV of the limestone samples does not correlate well with freeze and thaw loss ( $R=0.286$ ,  $R^2=0.08$  and  $P=0.367$ ). However, WI

decreases as the freeze and thaw loss increases ( $R=-0.653$ ,  $R^2=0.34$  and  $P=0.021$ ) indicating a lower polishing susceptibility with higher freeze and thaw loss.

#### 5.2.2.5 Los Angeles Abrasion Loss

As shown in Figures 5-23 and 5-24, the PV and WI of the dolomite samples do not correlate well with freeze and thaw loss, having  $R=0.107$  and  $P=0.662$  for PV and  $R=-0.008$  and  $P=0.973$ . The PV and WI of the limestone samples in Figures 5-25 and 5-26 do not correlate well with the freeze and thaw loss, having  $R=-0.210$ ,  $R^2=0.04$  and  $P=0.388$  for PV and  $R=0.359$ ,  $R^2=0.13$  and  $P=0.131$  for WI.

#### 5.2.3 Magnesium Content and Frictional Properties of Carbonate Aggregates

Data acquired from the magnesium analysis, based on 17 samples for dolomites and 21 samples for limestone, were analyzed to find the relationship between elemental Mg content and frictional resistance. The elemental magnesium contents range from 10.1% to 13.1% for the dolomite samples and 0.5% to 10% for the limestone samples.

As shown in Figures 5-27 and 5-28, for dolomites, elemental Mg contents correlate with PV ( $R=-0.653$ ,  $R^2=0.43$  and  $P=0.005$ ) and WI ( $R=0.756$ ,  $R^2=0.57$  and  $P=0.000$ ). This indicates that purer dolomites with higher elemental Mg values show lower PV and higher WI. Therefore, impure dolomite (lower elemental Mg content) is a potentially acceptable aggregate for bituminous surface pavements.

As shown in Figures 5-29 and 5-30, for limestone samples, elemental Mg contents do not correlate with either PV ( $R=0.205$ ,  $R^2=0.04$  and  $P=0.373$ ) or WI ( $R=-0.109$ ,  $R^2=0.01$  and  $P=0.639$ ).

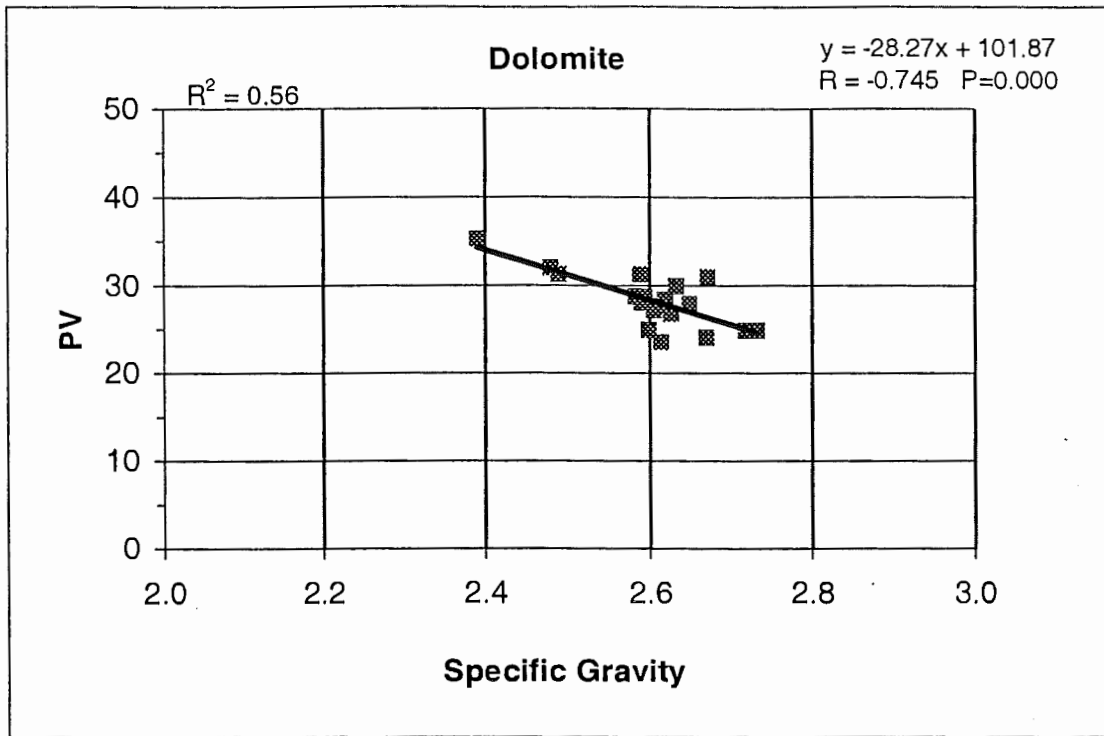


Figure 5-11. Correlation of Polished Value and Specific Gravity in dolomite samples

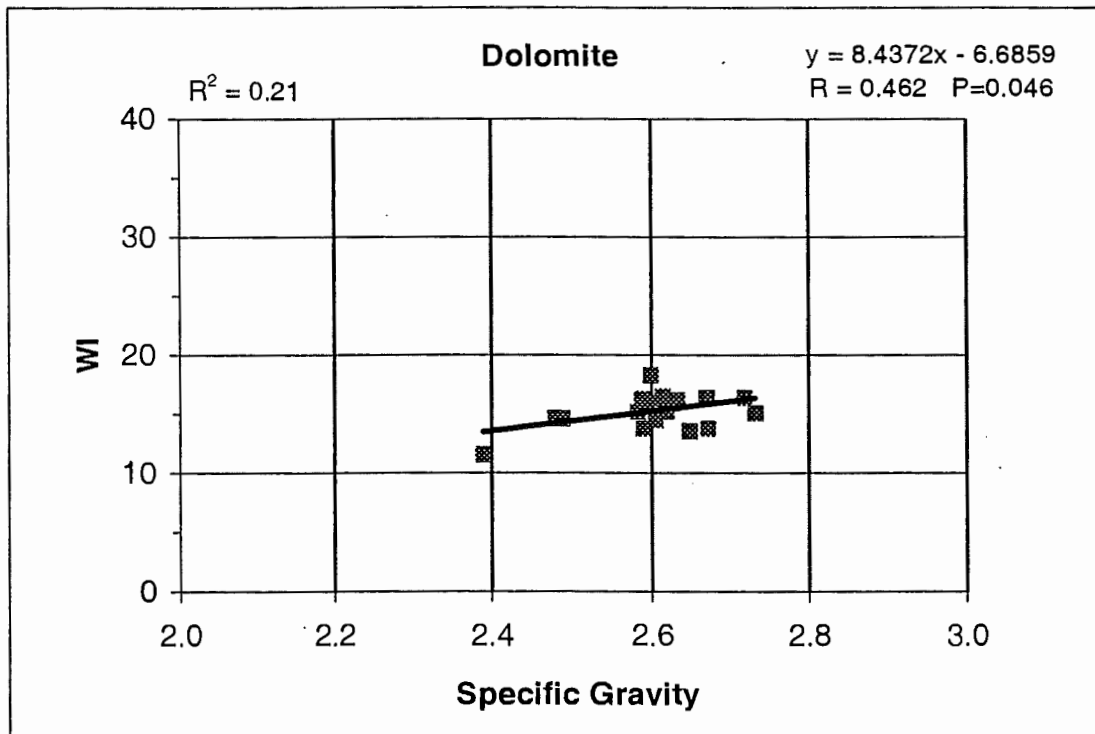


Figure 5-12. Correlation of Wear Index and Specific Gravity in dolomite samples

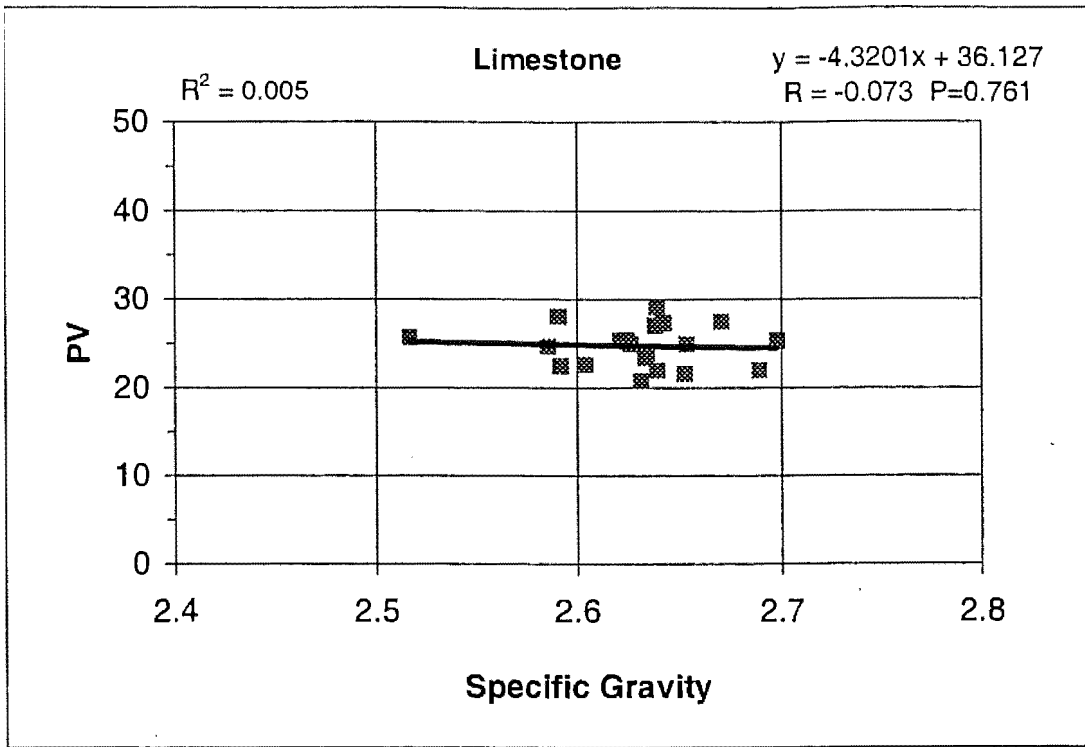


Figure 5-13. Correlation of Polished Value and Specific Gravity in limestone samples

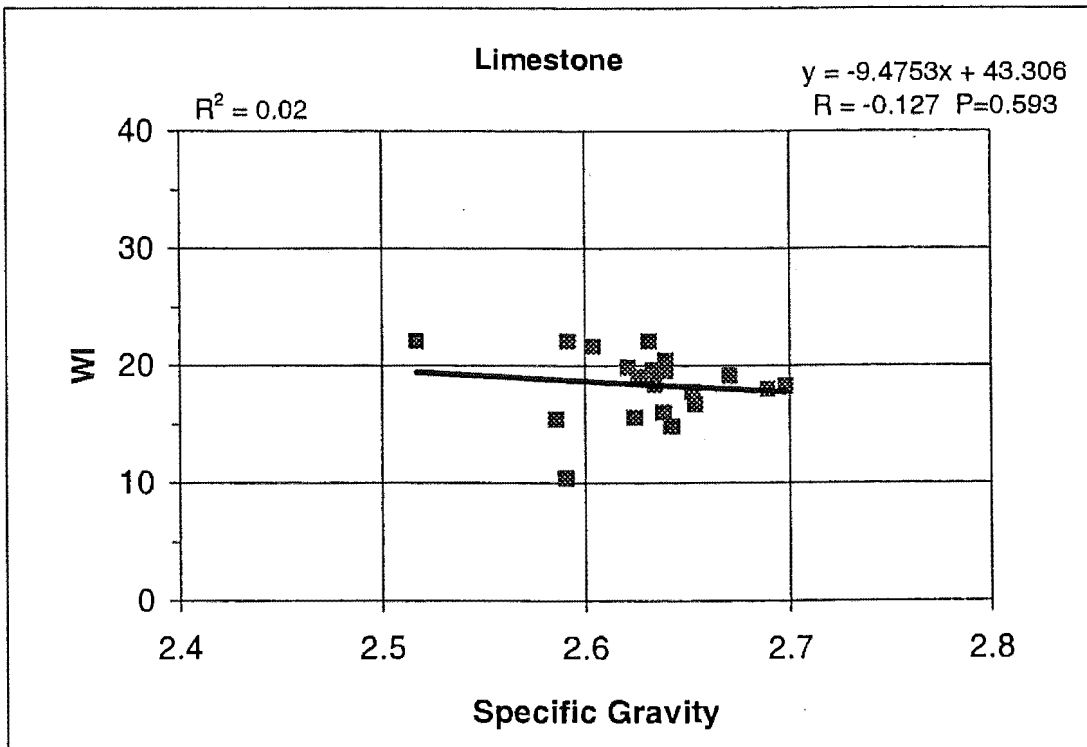


Figure 5-14. Correlation of Wear Index and Specific Gravity in limestone samples

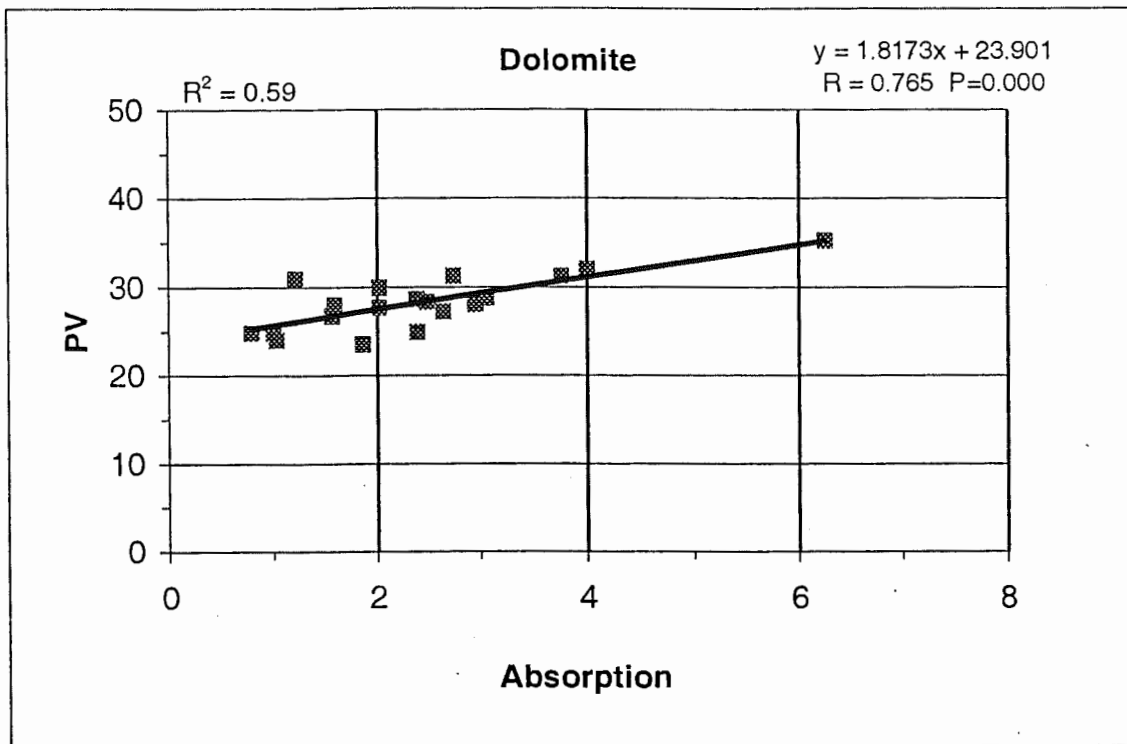


Figure 5-15. Correlation of Polished Value and Absorption in dolomite samples

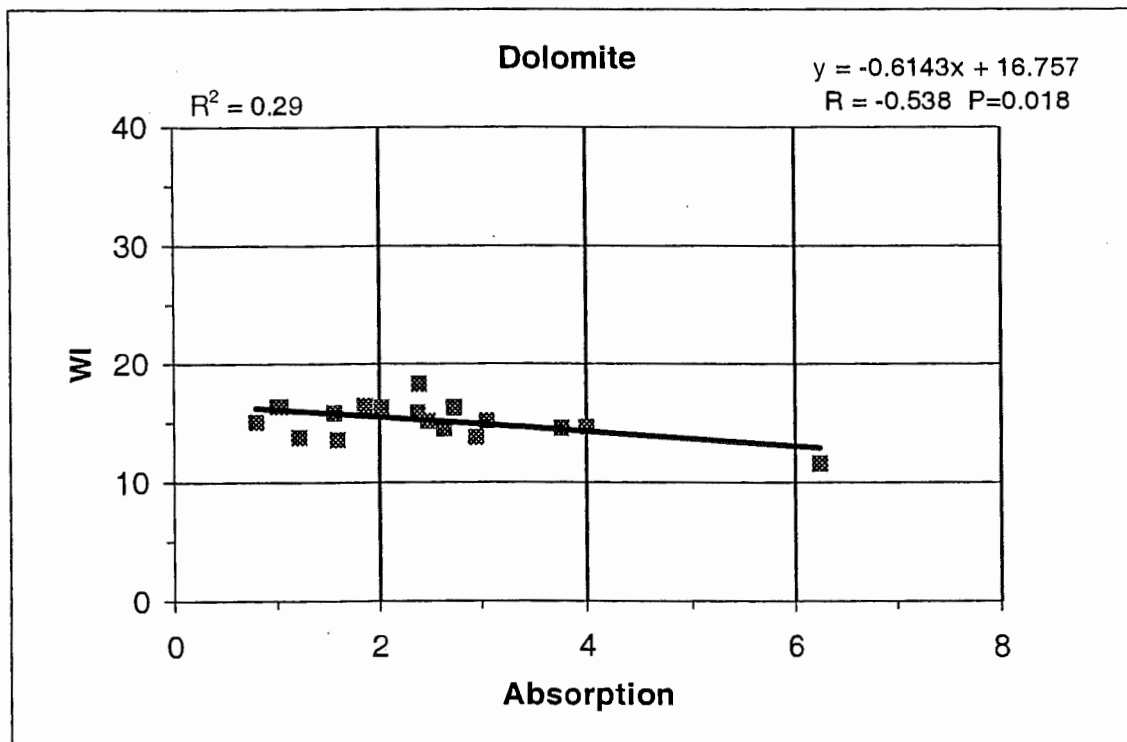


Figure 5-16. Correlation of Wear Index and Absorption in dolomite samples

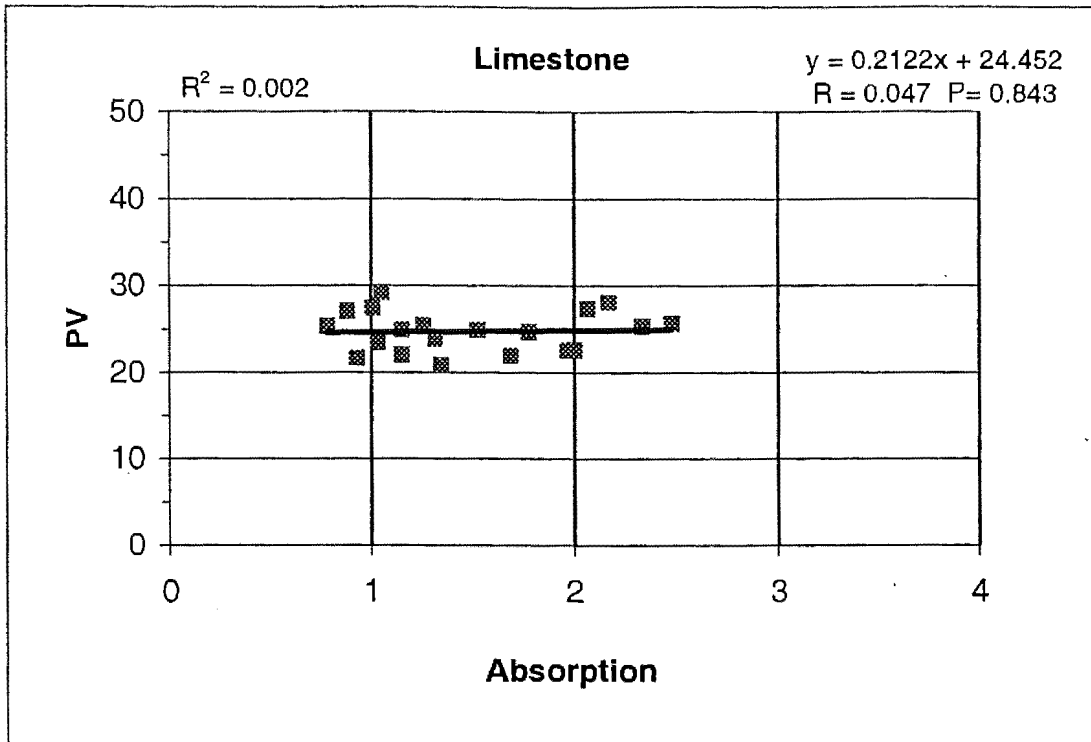


Figure 5-17. Correlation of Polished Value and Absorption in limestone samples

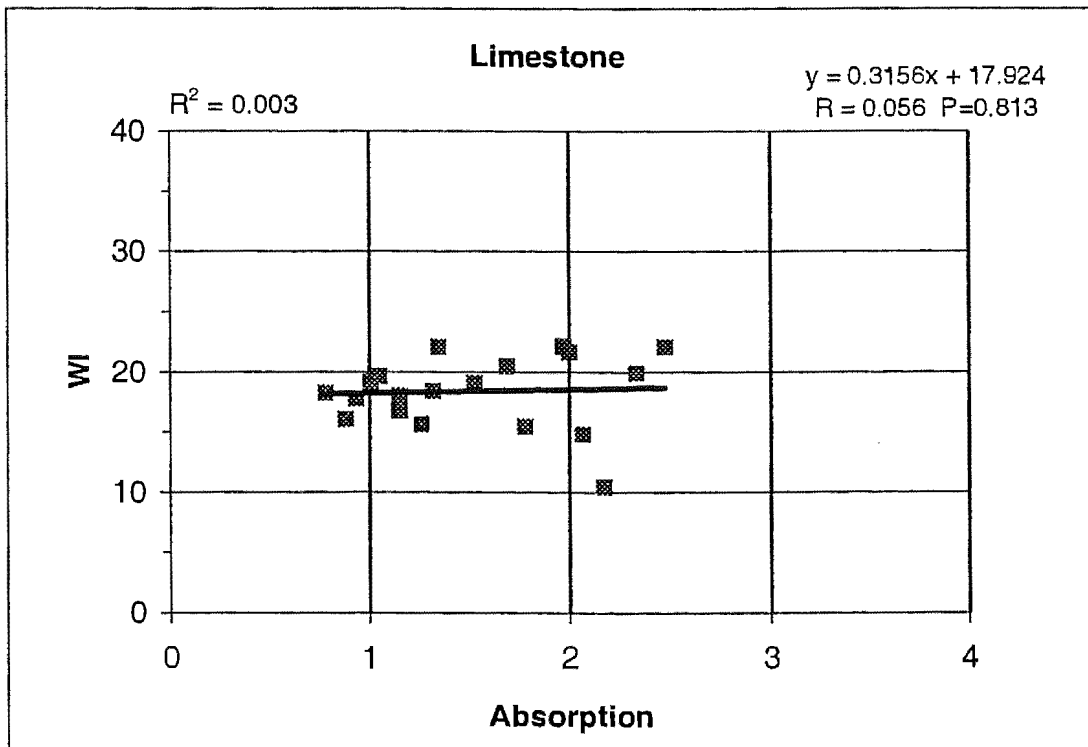


Figure 5-18. Correlation of Wear Index and Absorption in limestone samples

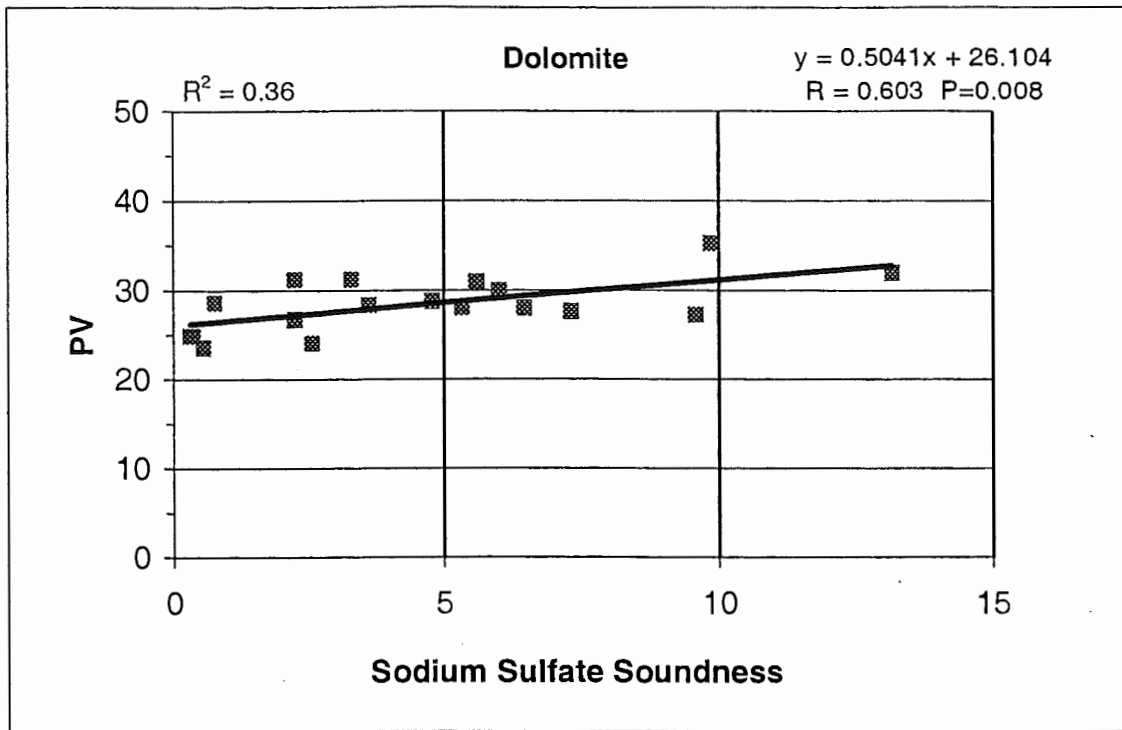


Figure 5-19. Correlation of Polished Value and Sodium sulfate soundness in dolomite samples

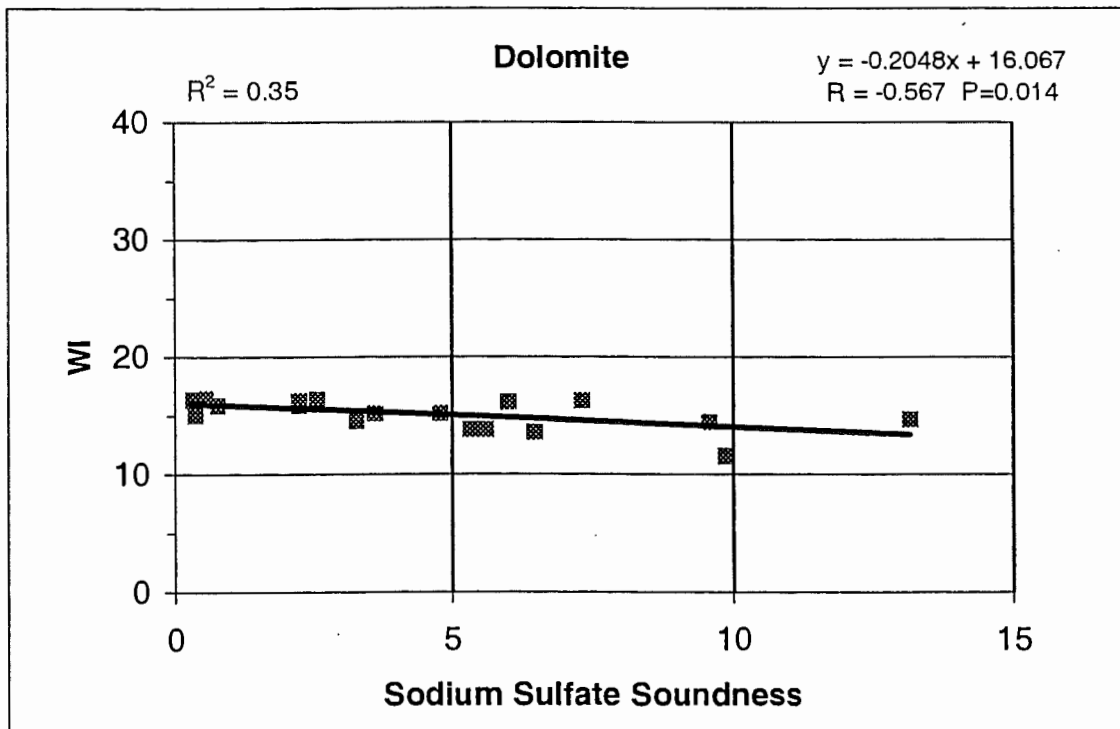


Figure 5-20. Correlation of Wear Index and Sodium sulfate soundness in dolomite samples

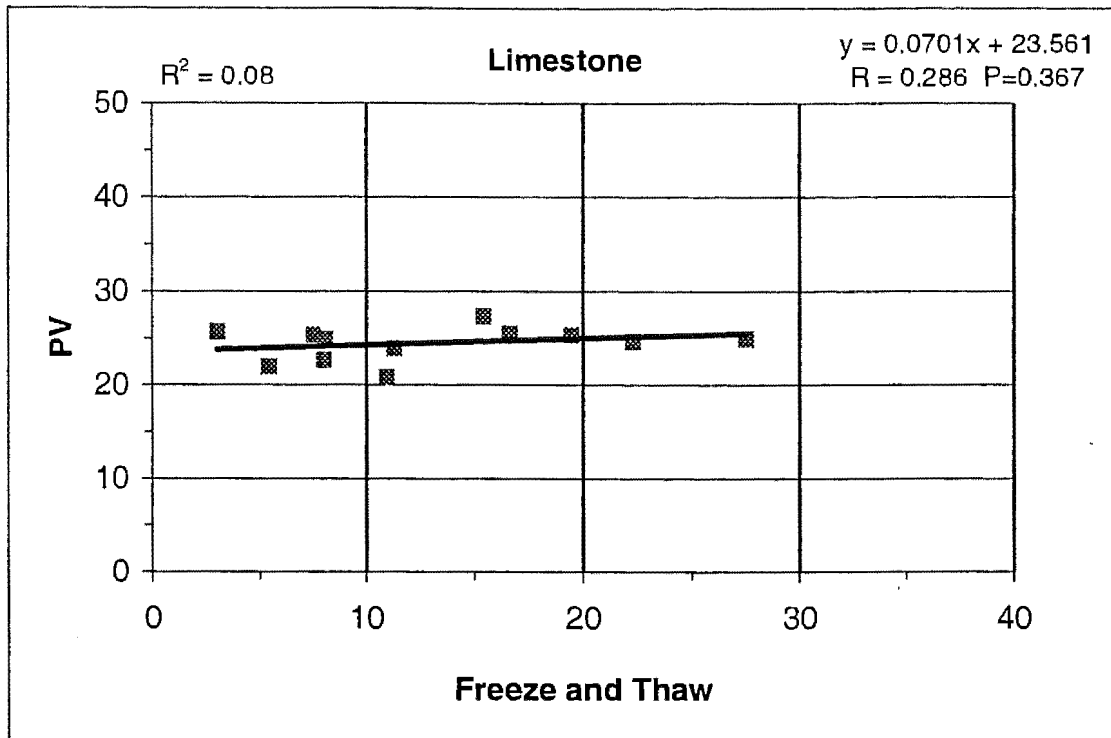


Figure 5-21. Correlation of Polished Value and Sodium sulfate soundness in limestone samples

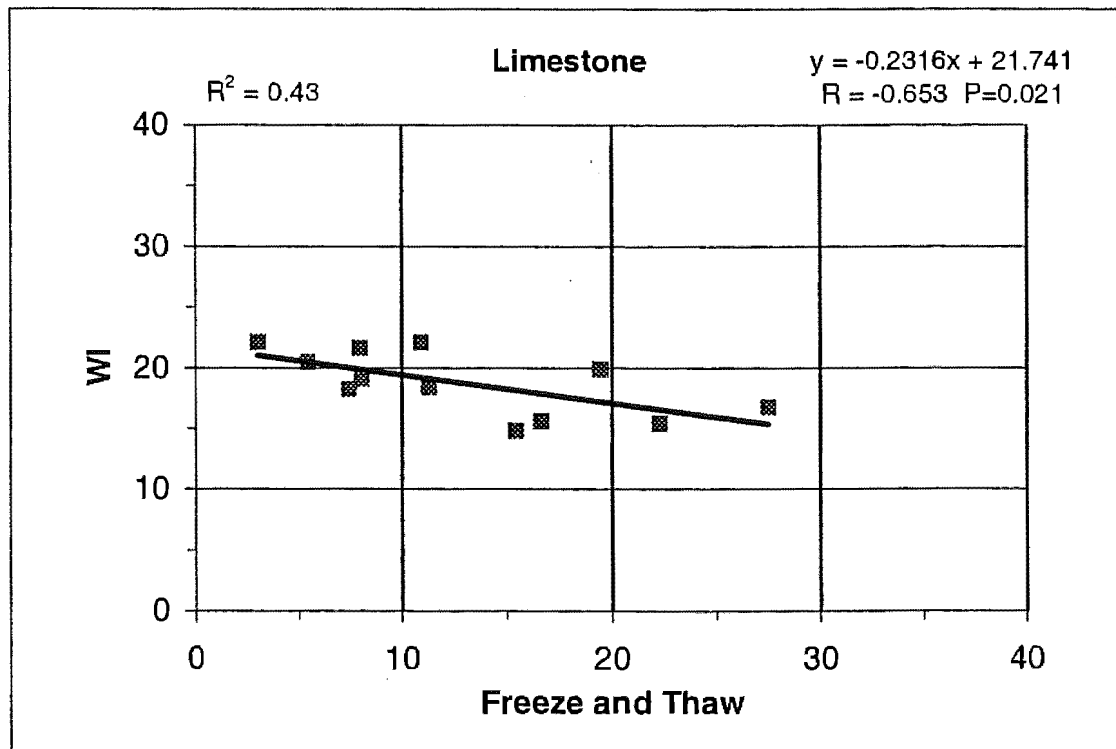


Figure 5-22. Correlation of Wear Index and Sodium sulfate soundness in limestone samples



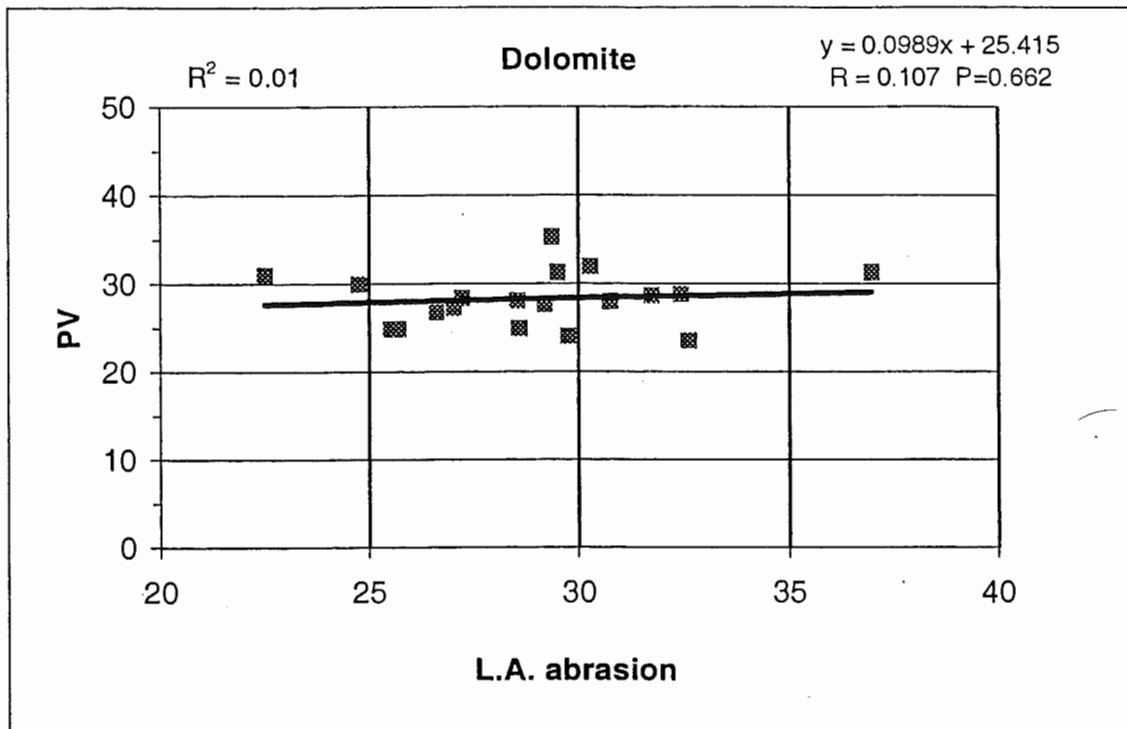


Figure 5-23. Correlation of Polished Value and L.A. Abrasion in dolomite samples

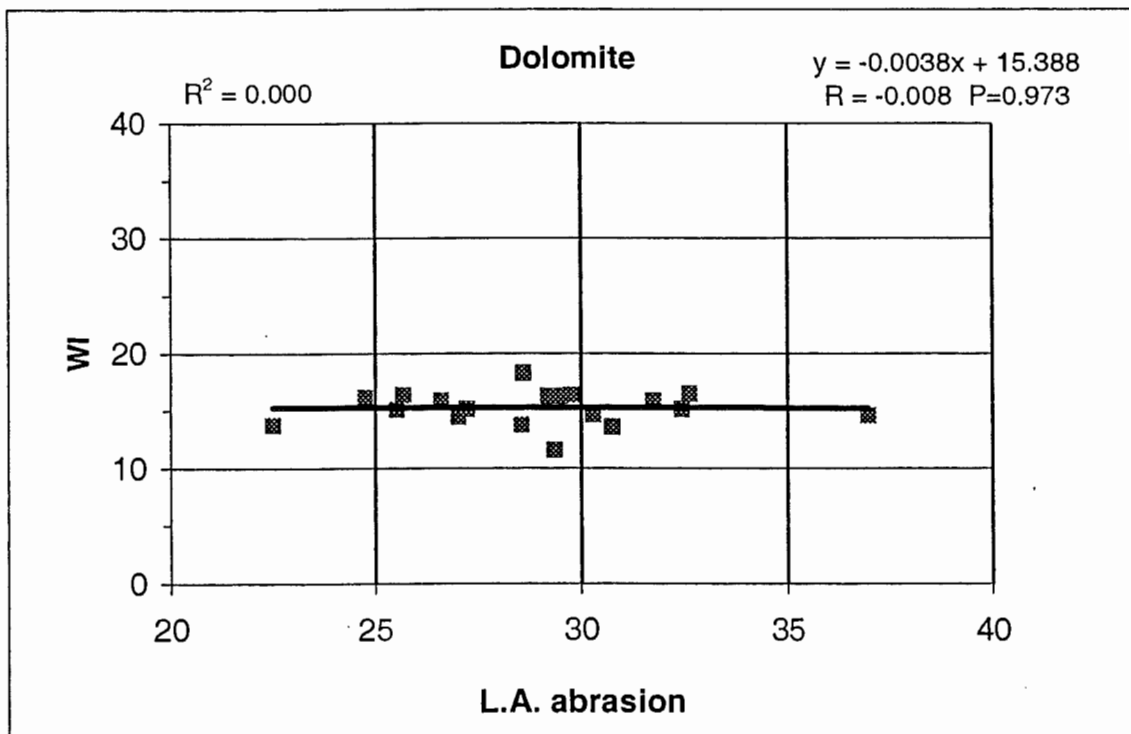


Figure 5-24. Correlation of Wear Index and L.A. Abrasion in dolomite samples

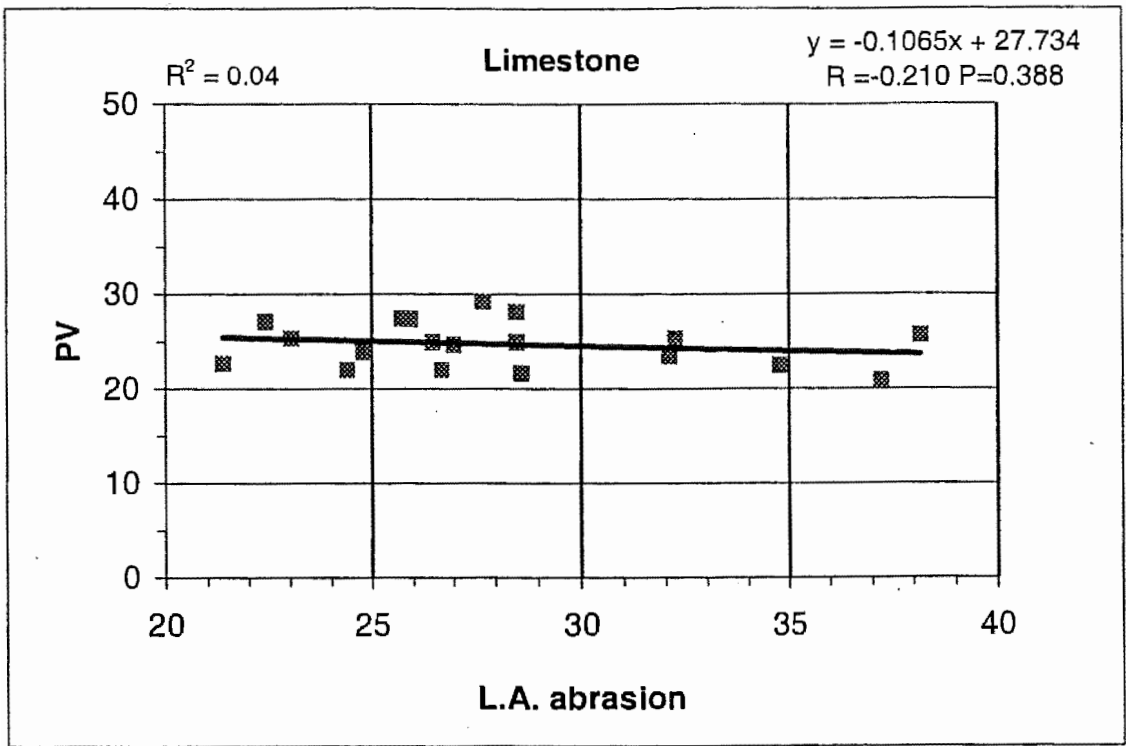


Figure 5-25. Correlation of Polished Value and L.A. Abrasion in limestone samples

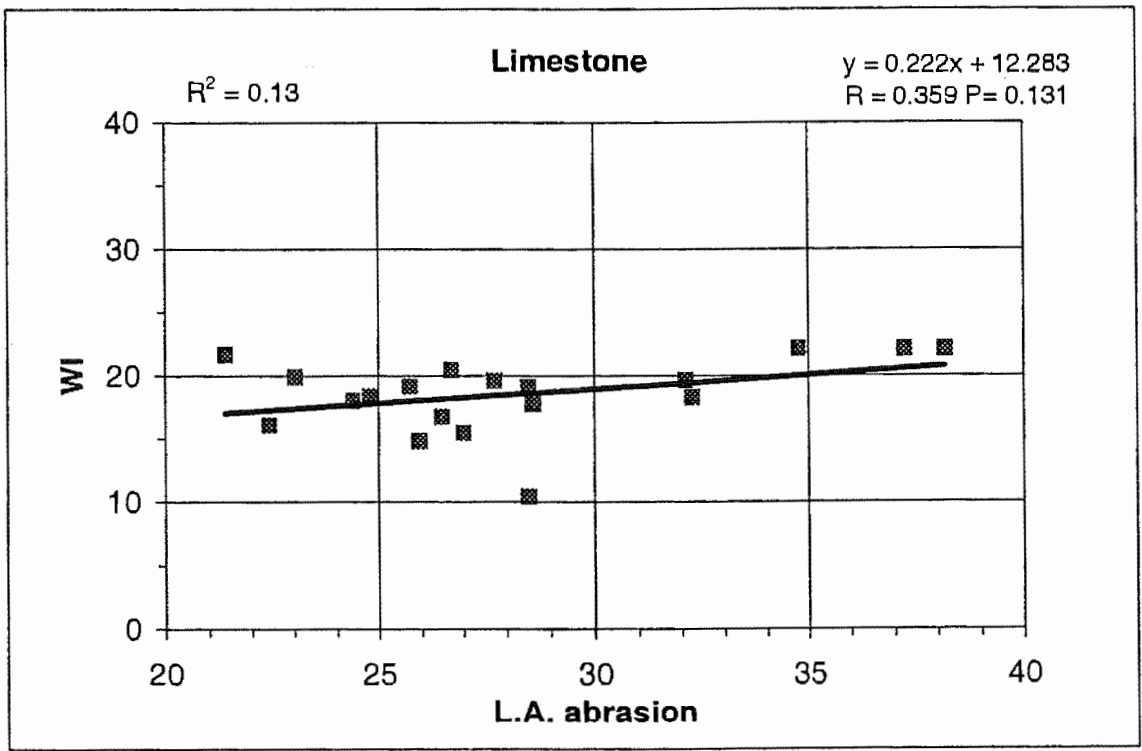


Figure 5-26. Correlation of Wear Index and L.A. Abrasion in limestone samples

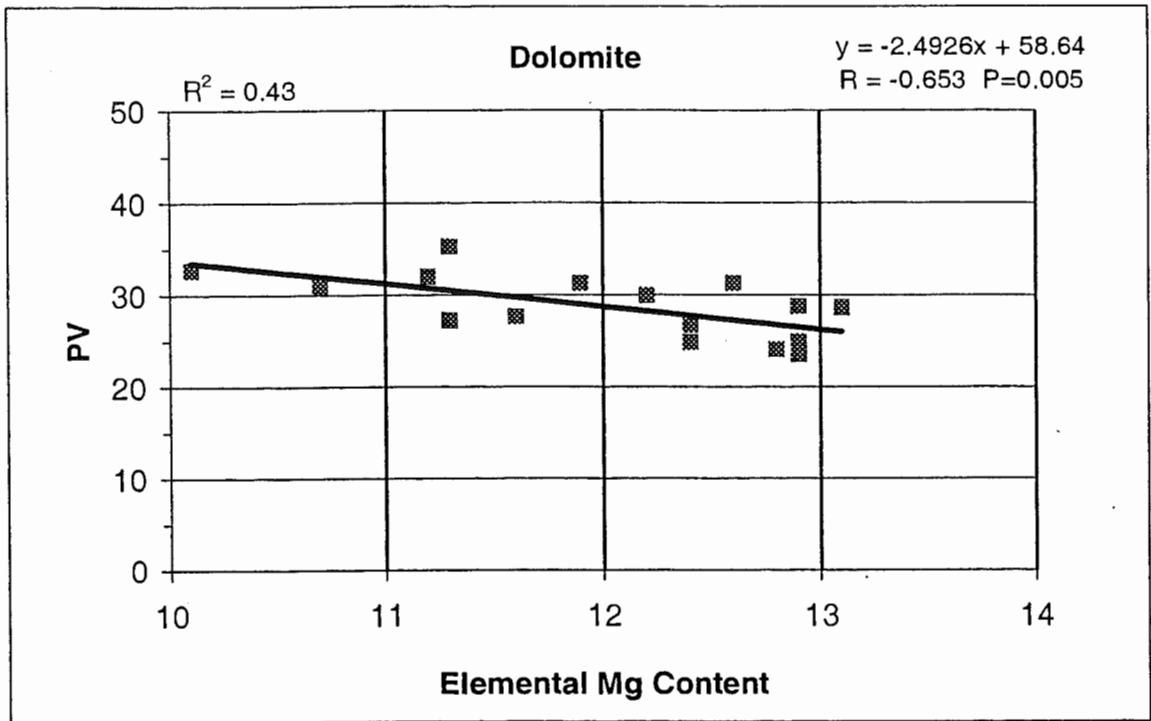


Figure 5-27. Correlation of Polished Value and Mg content in dolomite samples

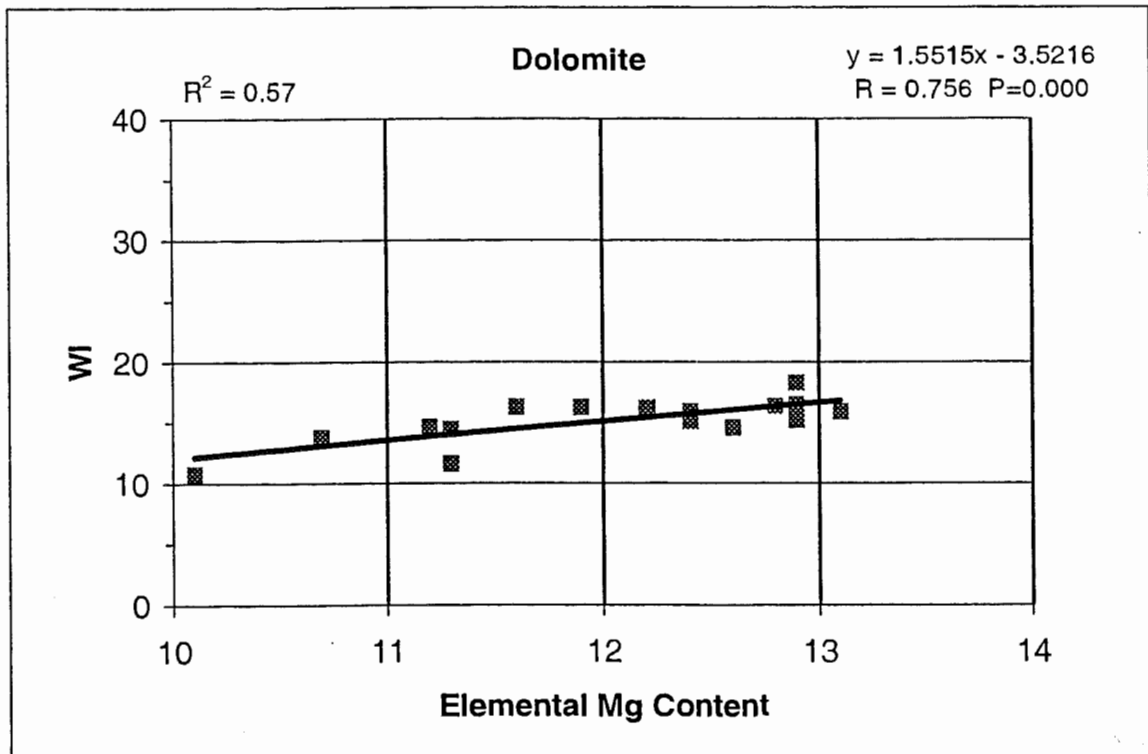


Figure 5-28. Correlation of Wear Index and Mg content in dolomite samples

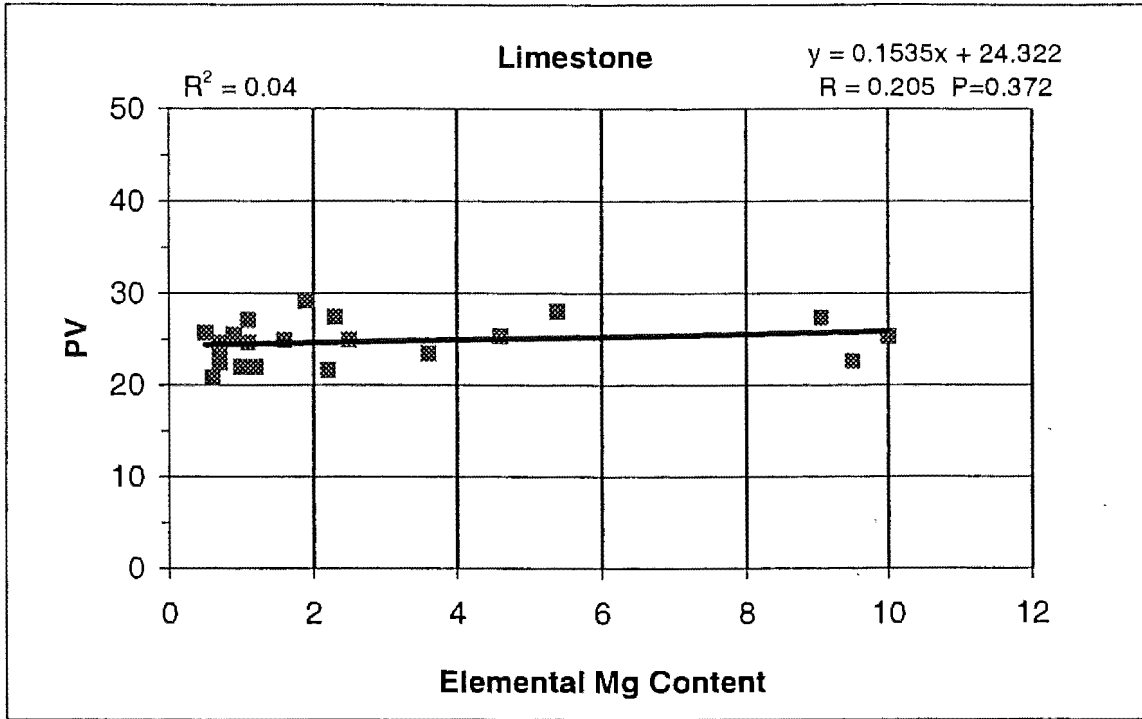


Figure 5-29. Correlation of Polished Value and Mg content in limestone samples

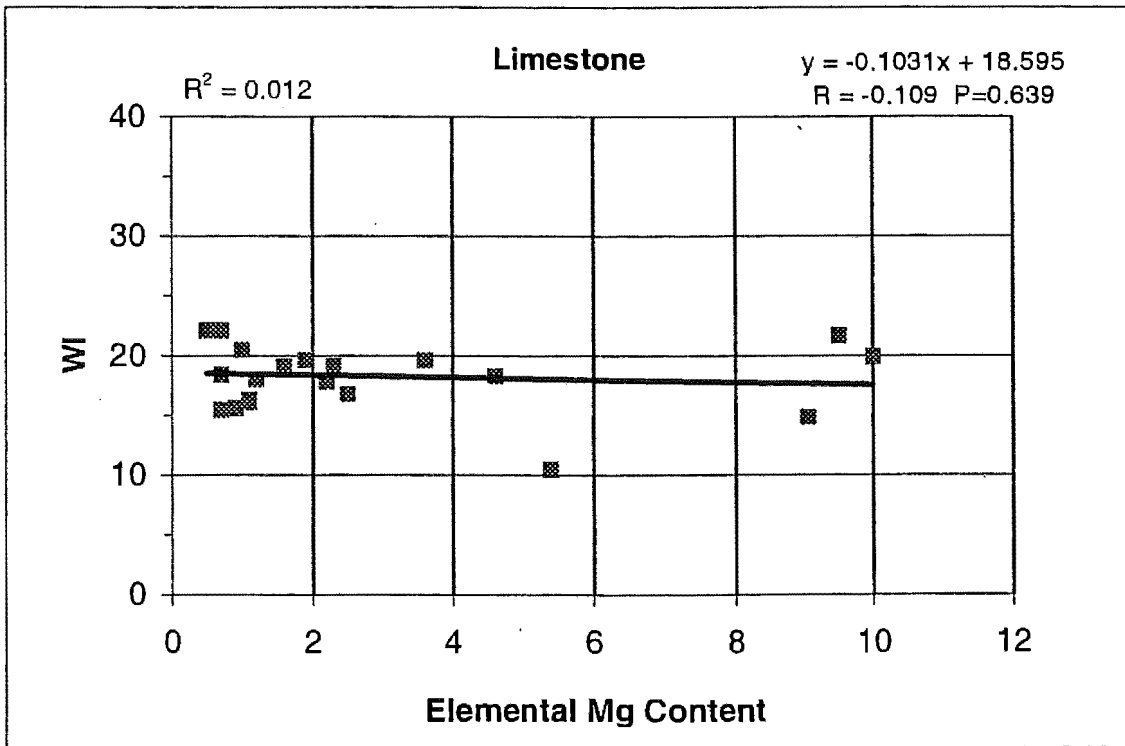


Figure 5-30. Correlation of Wear Index and Mg content in limestone samples

#### 5.2.4 Acid Insoluble Residue and Frictional Properties of Carbonate Aggregates

The data evaluated on the acid-insoluble residue test were divided into three groups. These consisted of 1) total acid-insoluble residue, 2) percent residue greater than #200 sieve (75 microns) and 3) percent residue smaller than #200 sieve (75 microns).

Based on the results of the comparison between PV and WI and acid-insoluble residues, both PV and WI correlate well with total acid-insoluble residue. The PV and WI are least correlated with the percent material greater than #200. This indicates the percent amounts of smaller than #200 has a positive effect on frictional resistance.

As shown in Figures 5-31 and 5-32, total acid-insoluble residue in the dolomite samples correlate poorly with PV ( $R=0.448$ ,  $R^2=0.20$  and  $P=0.048$ ) and WI ( $R=-0.255$ ,  $R^2=0.07$  and  $P=0.277$ ). However, as shown in Figures 5-33 and 5-34, total acid-insoluble residue for limestone samples correlates strongly with PV ( $0.703$ ,  $R^2=0.49$  and  $P=0.000$ ) and WI ( $R=-0.639$ ,  $R^2=0.41$  and  $P=0.002$ ). For both dolomite and limestone, as the acid-insoluble residue increases, the polished value (PV) increases and polishing susceptibility (WI) decreases.

The total acid-insoluble residue may be the most critical factor for the evaluation of limestone as a source for bituminous pavement surfaces than it is in the evaluation of dolomite because of the higher correlation of limestone PV with total acid-insoluble residue.

As shown in Figures 5-35 through 5-38, regarding percent insoluble materials greater than #200 in dolomite samples, acid-insoluble residues do not correlate well with PV or WI. But the limestone shows a slight correlation between PV and WI and percent insoluble materials greater than #200.

As shown in Figures 5-39 through 5-42, in the case of percent insoluble materials smaller than #200 in dolomite, the acid-insoluble residue shows a moderate correlation with PV ( $R=0.560$ ,  $R^2=0.26$  and  $P=0.010$ ) and WI ( $R=-0.286$ ,  $R^2=0.08$  and  $P=0.222$ ). The acid-insoluble residue of limestones is well correlated with PV ( $R=0.616$ ,  $R^2=0.38$  and  $P=0.003$ ) and WI ( $R=-0.623$ ,  $R^2=0.39$  and  $P=0.003$ ).

As shown in Figures 5-43 through 5-46, the ratio of percent insoluble residue greater than #200 to percent insoluble residue smaller than #200 does not correlate well with PV or WI.

From the above results for both dolomite and limestone, it is important to note that as the acid-insoluble residues increase, the polished values increase and polishing susceptibility decreases. Particularly for the total acid-insoluble residue, there is a strong correlation for limestone aggregates.

### 5.2.5 Field and Laboratory Testing

The frictional resistance of aggregates can be evaluated using field tests. In this method the level of friction is measured on traveled sections of selected highways using the towed friction trailer (ASTM Standard E274). Recorded friction values for a given road surface are known as Average Friction Numbers (AFN). Typically both the Average Friction Numbers from the smooth and from ribbed tires are reported (Friction Number Smooth (FNS) and Friction Number Ribbed (FNR)).

An AFN of 20 for the FNS and of 30 for FNR are values of concern used by INDOT to indicate the need for follow-up evaluations to determine if corrective action on the pavement may be required.

After the field test was performed with towed trailers on the highway, 8-in cores were obtained and brought to INDOT lab. Then the aggregate was extracted from the cores. Fresh pieces of +9.5mm size were used to construct coupons. Coupons were measured using the British Pendulum before and after polishing by the British Wheel test

Following extraction of aggregates from core samples, coupons were prepared and tested in the same way as that used for quarried aggregates. This allowed for a direct comparison between laboratory friction values and those obtained in the field.

Correlation analysis between BPN and AFN was performed on the carbonate aggregates. As shown in Figures 5-47 and 5-48, the FNR and FNS are more highly correlated with IFV than with PV. Using these correlations, the FNR=30 corresponds to PV=24.7 and IFV=36.9. The FNS=20 corresponds to PV=19.2 and IFV=30.5. In all, a PV of 25 may be considered as a minimum, acceptable friction value. The PV=25

corresponds to FNS=27.7 and FNR=30.7. However, these correlations were performed using only 6 data points, but showed a high significance level ( $P=0.067-0.106$ ) for PV.

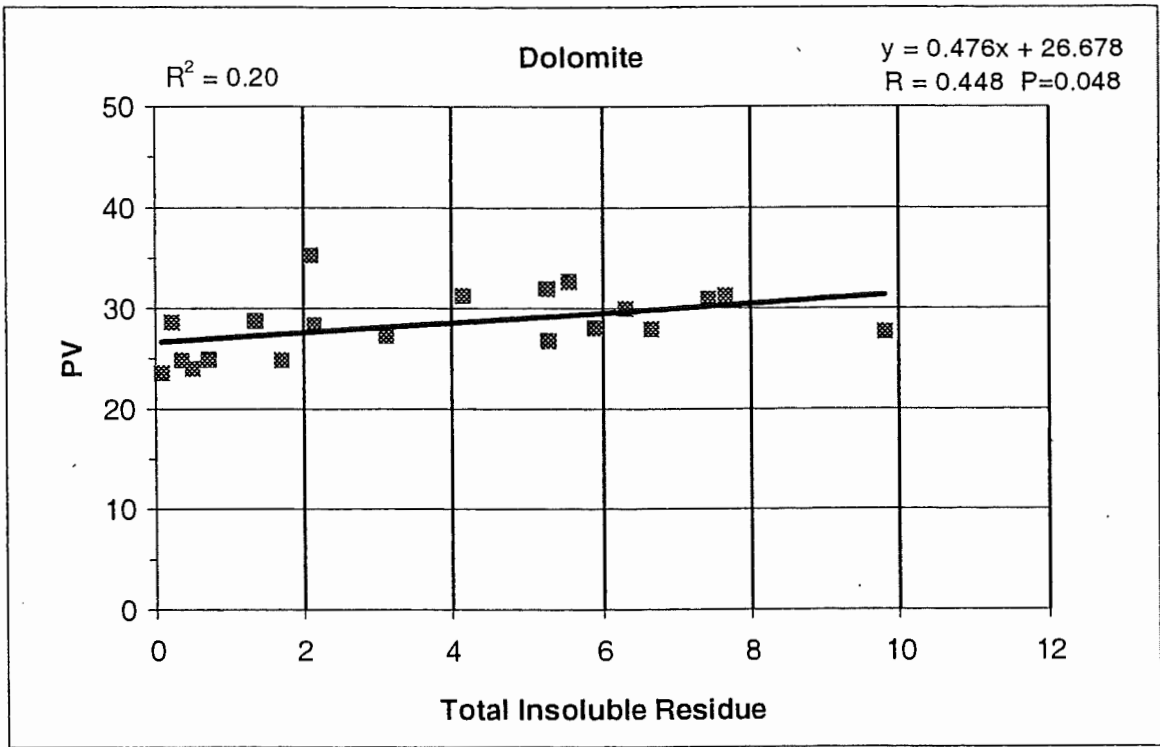


Figure 5-31. Correlation of Polished Value and Total insoluble residue in dolomite samples

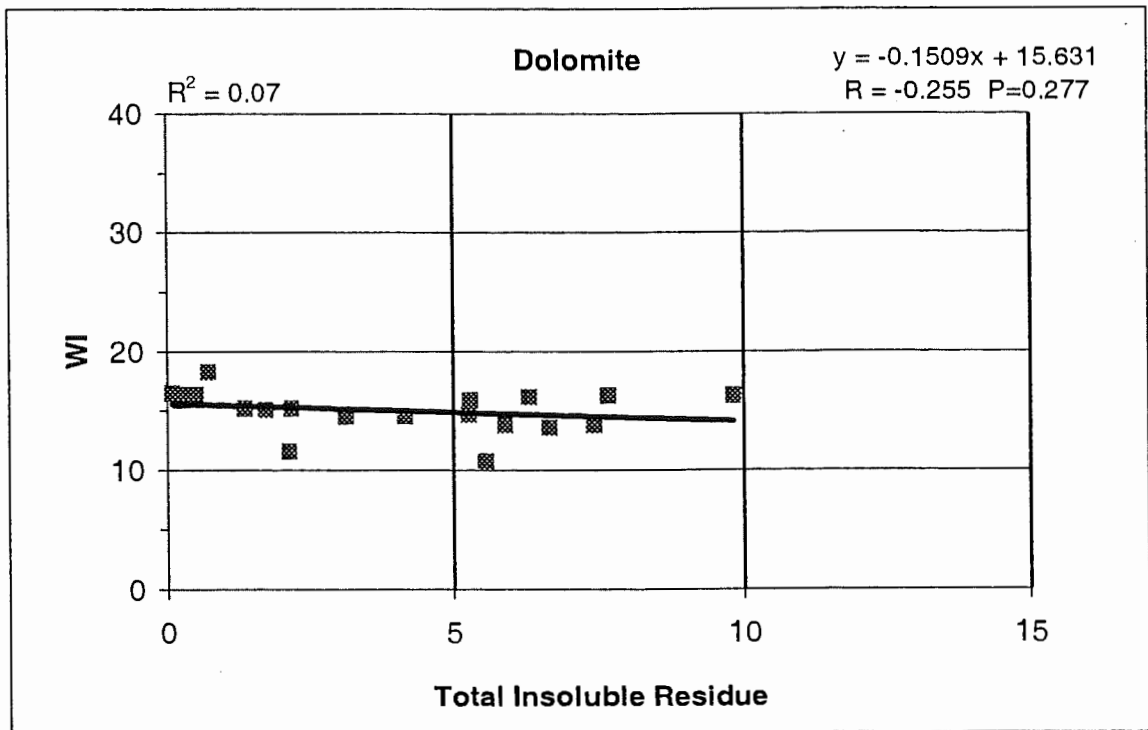


Figure 5-32. Correlation of Wear Index and Total insoluble residue in dolomite samples



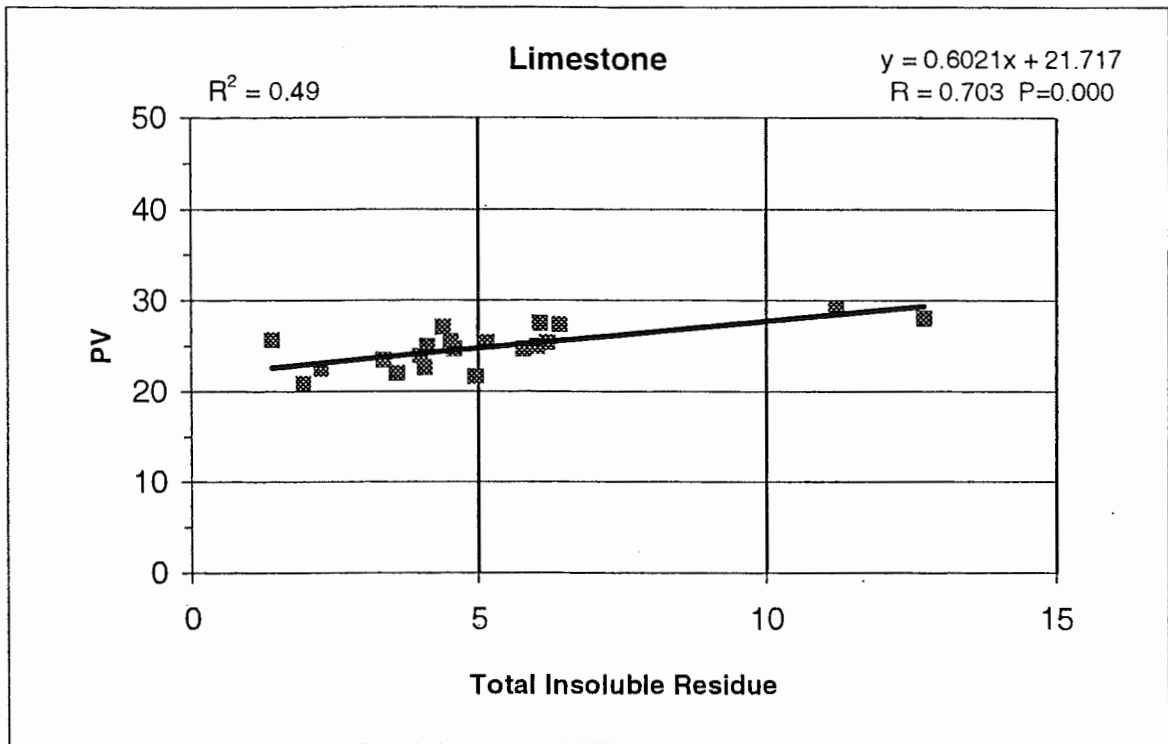


Figure 5-33. Correlation of Polished Value and Total insoluble residue in limestone samples

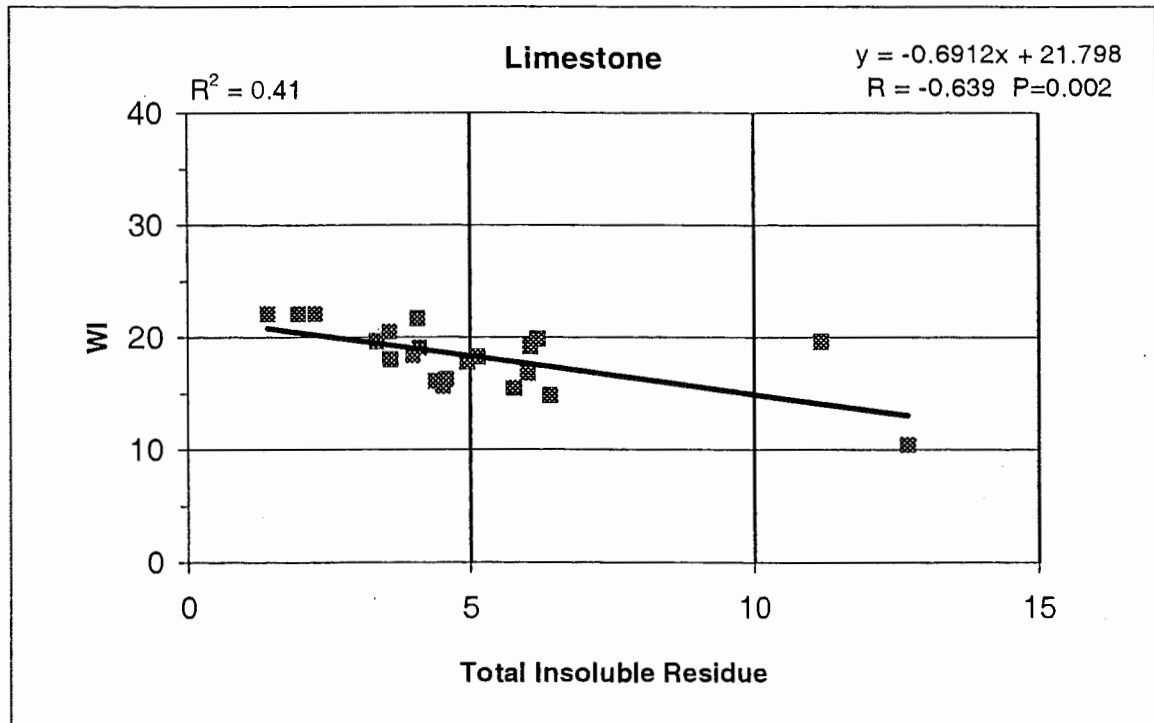


Figure 5-34. Correlation of Wear Index and Total insoluble residue in limestone samples

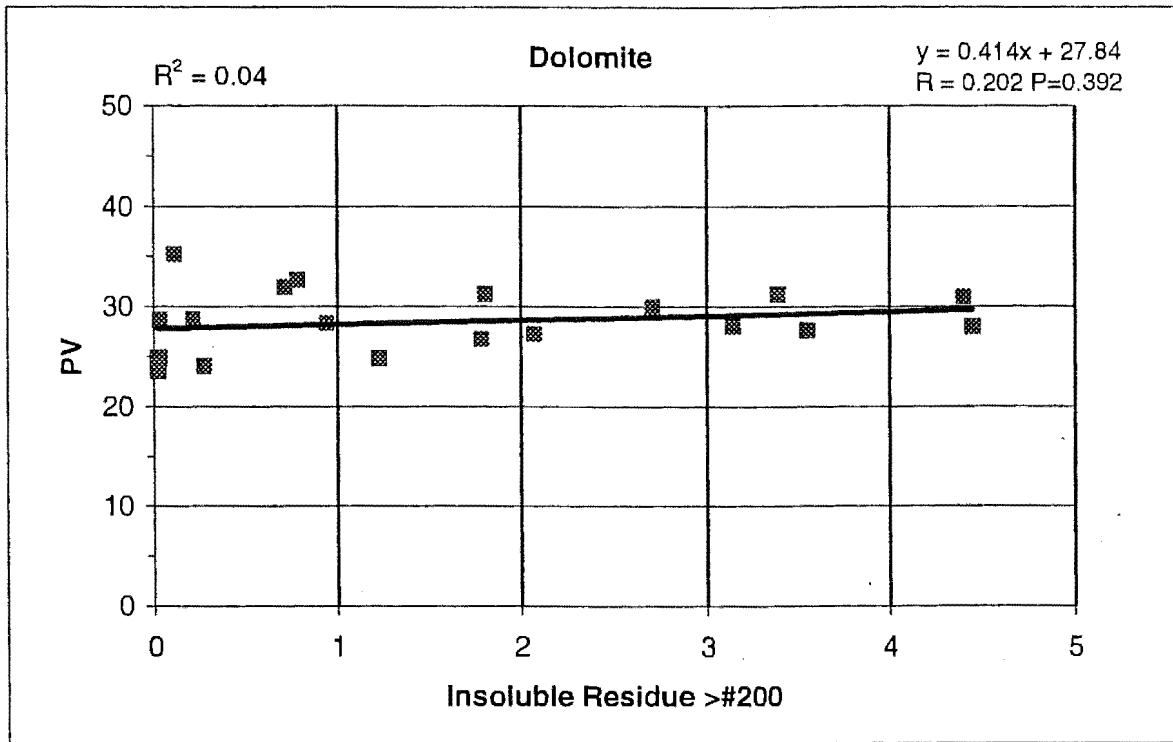


Figure 5-35. Correlation of Polished Value and insoluble residue >#200 in dolomite samples

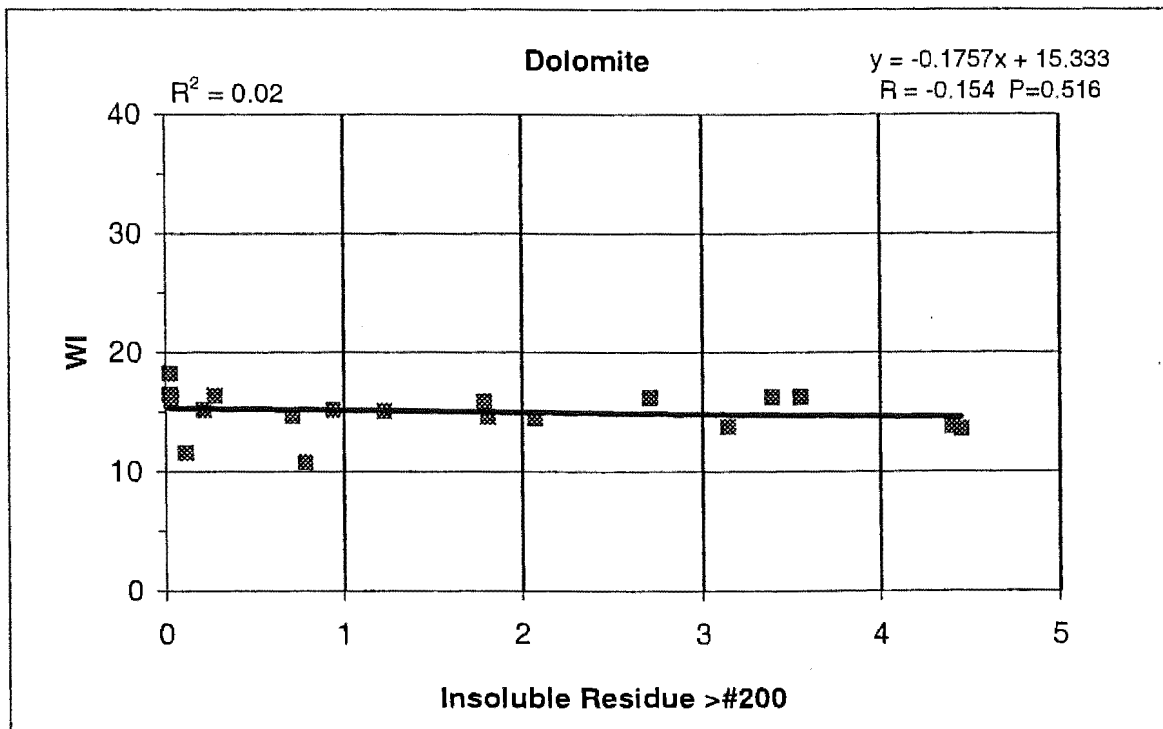
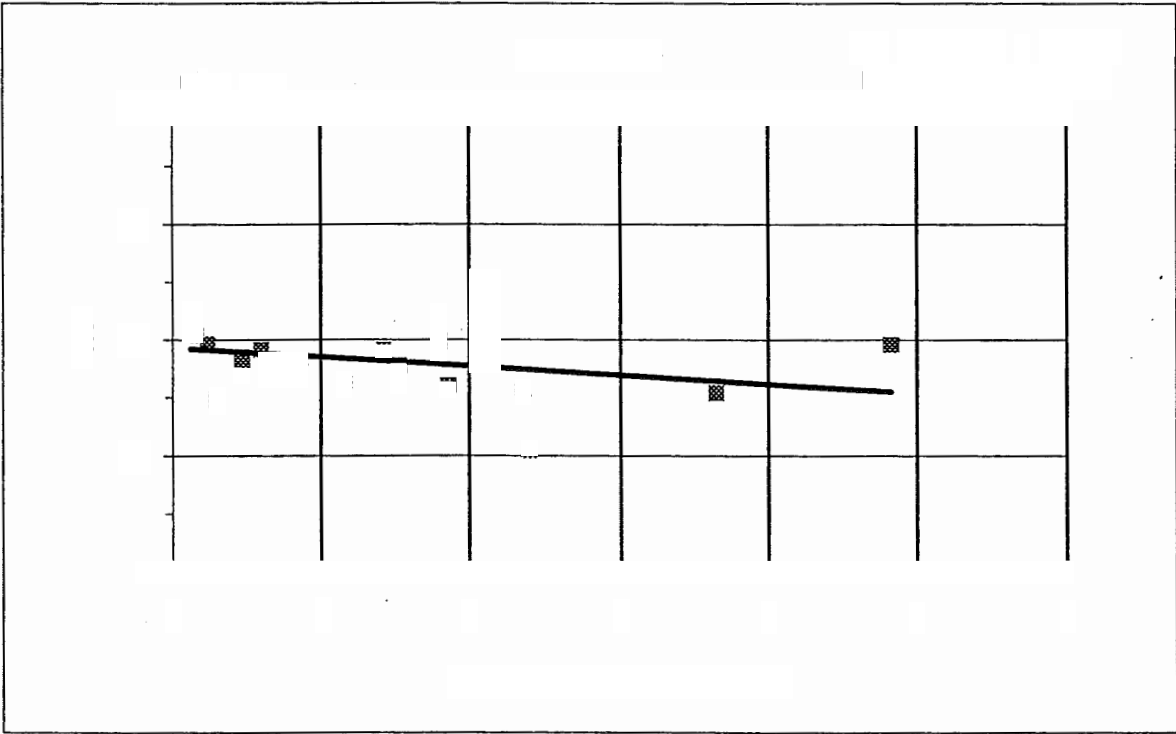
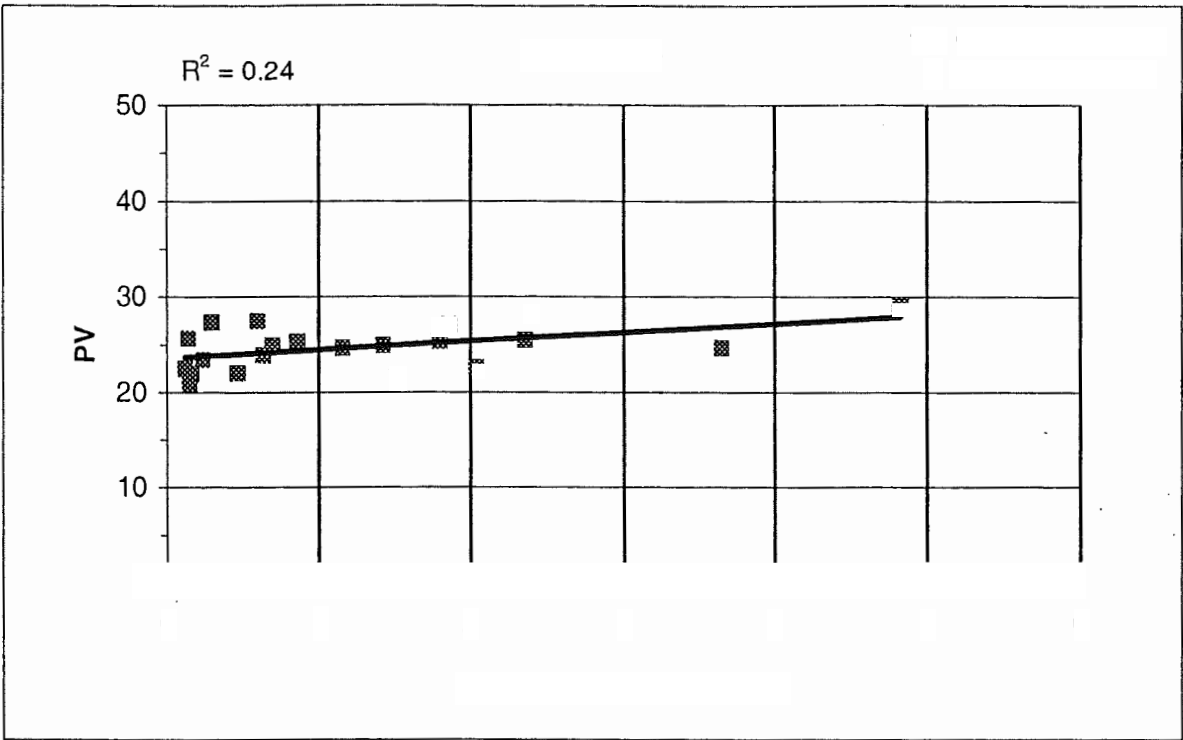


Figure 5-36. Correlation of Wear Index and insoluble residue >#200 in dolomite samples



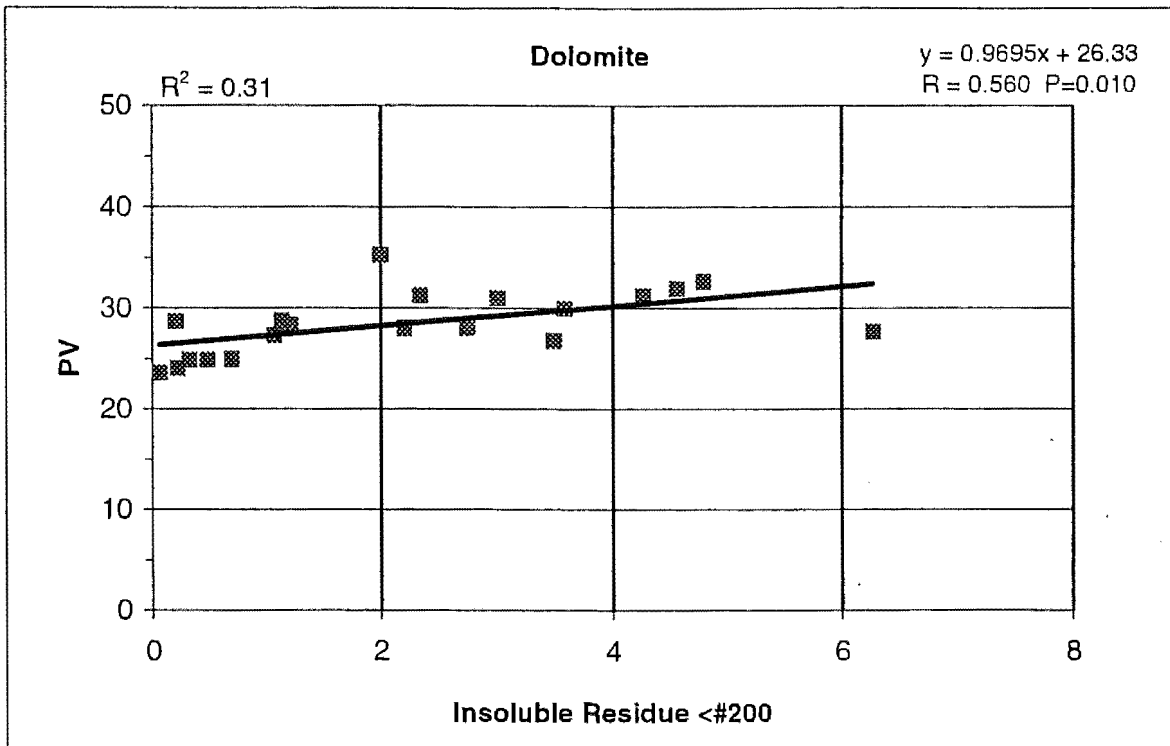


Figure 5-39. Correlation of Polished Value and insoluble residue <#200 in dolomite samples

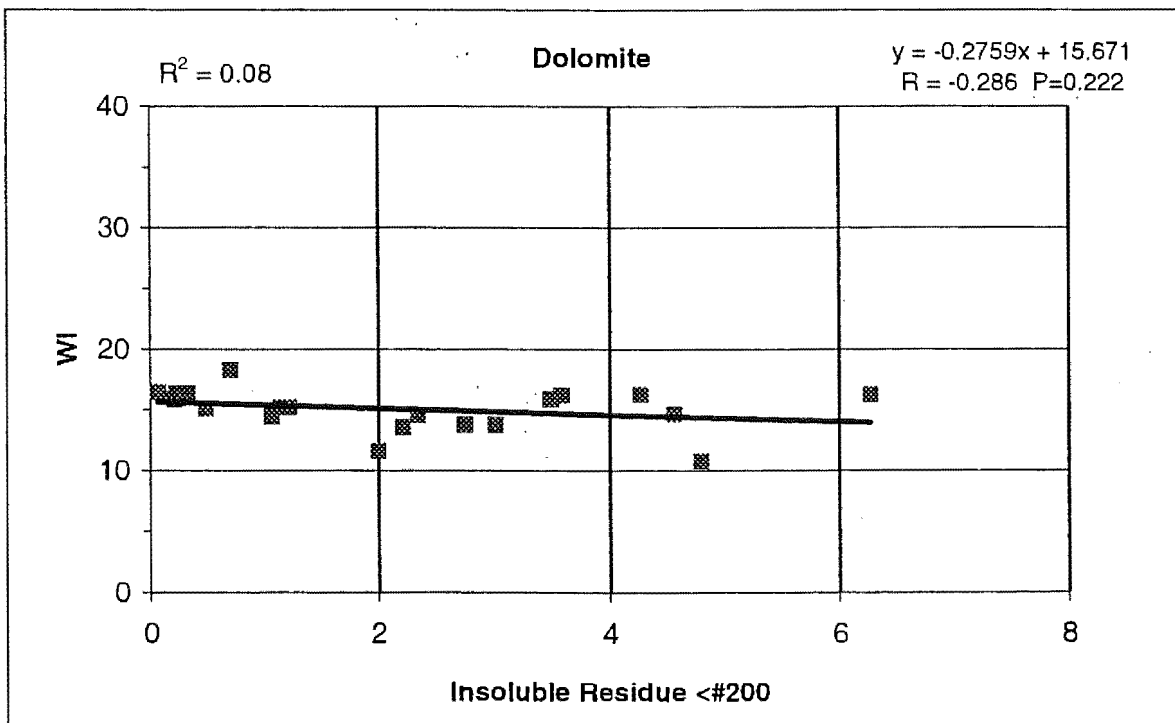


Figure 5-40. Correlation of Wear Index and Total insoluble residue in dolomite samples

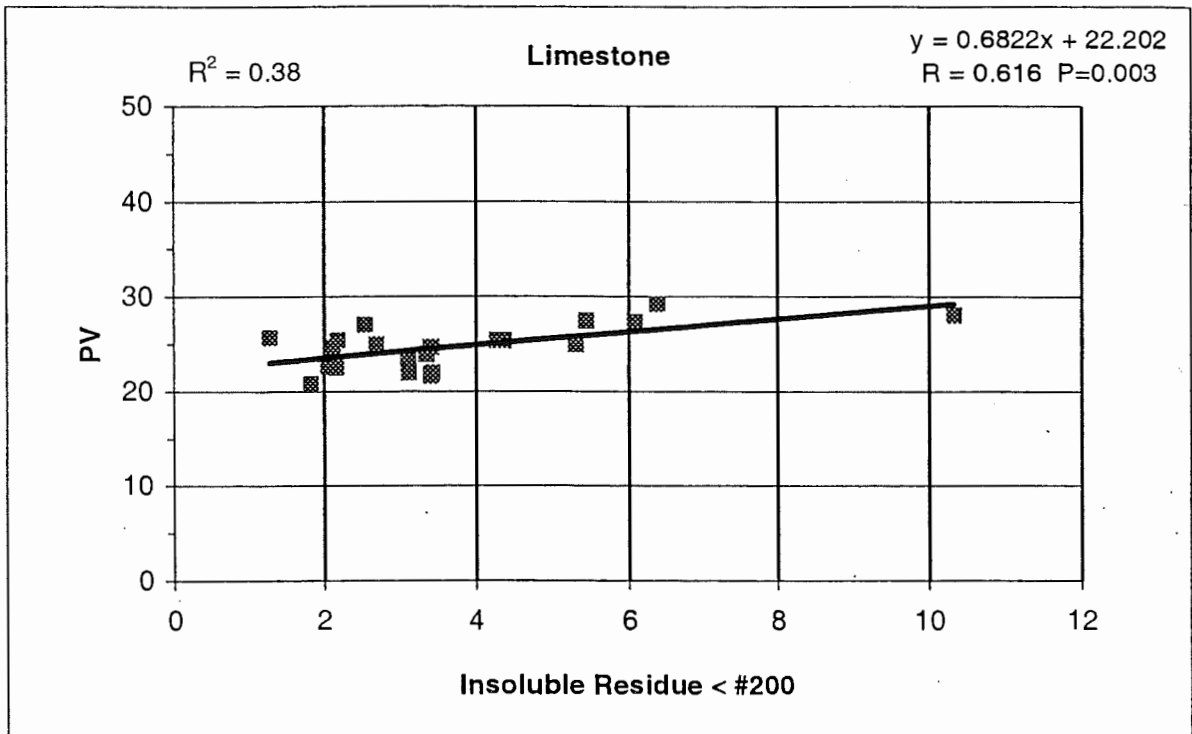


Figure 5-41. Correlation of Polished Value and insoluble residue <#200 in limestone samples

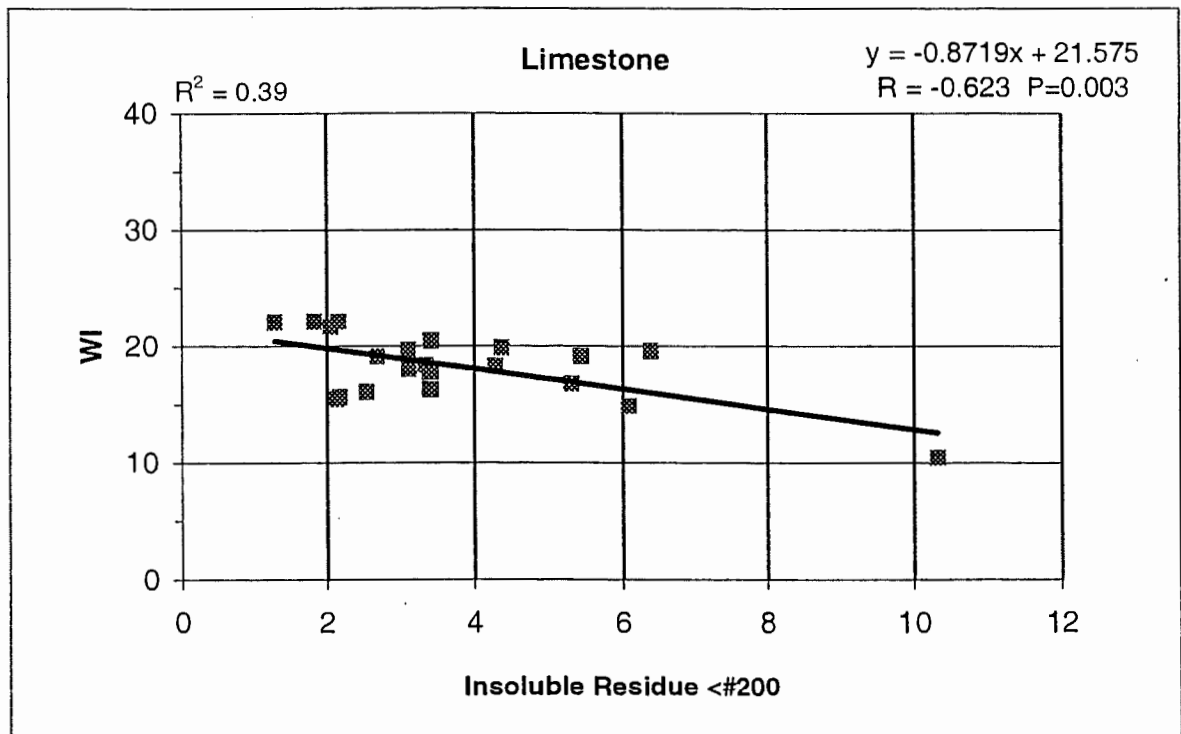


Figure 5-42. Correlation of Wear Index and insoluble residue <#200 in limestone samples

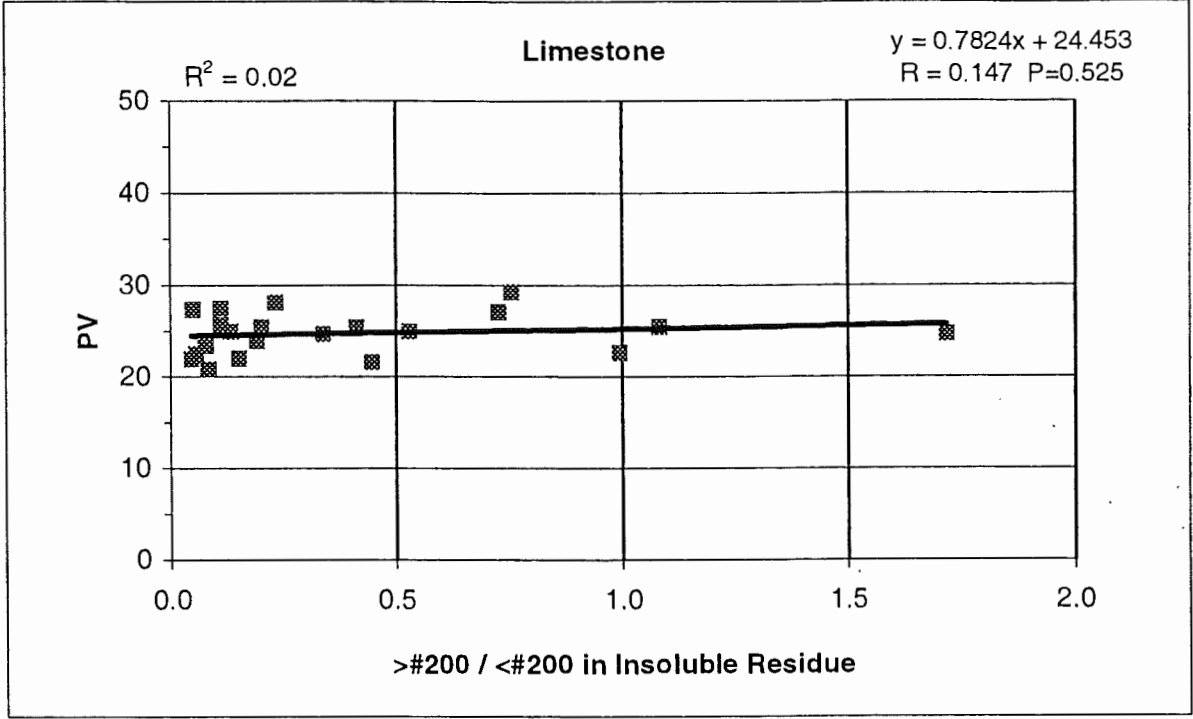


Figure 5-43. Correlation of Polished Value and the ratio of >#200 / <#200 in insoluble residue in limestone samples

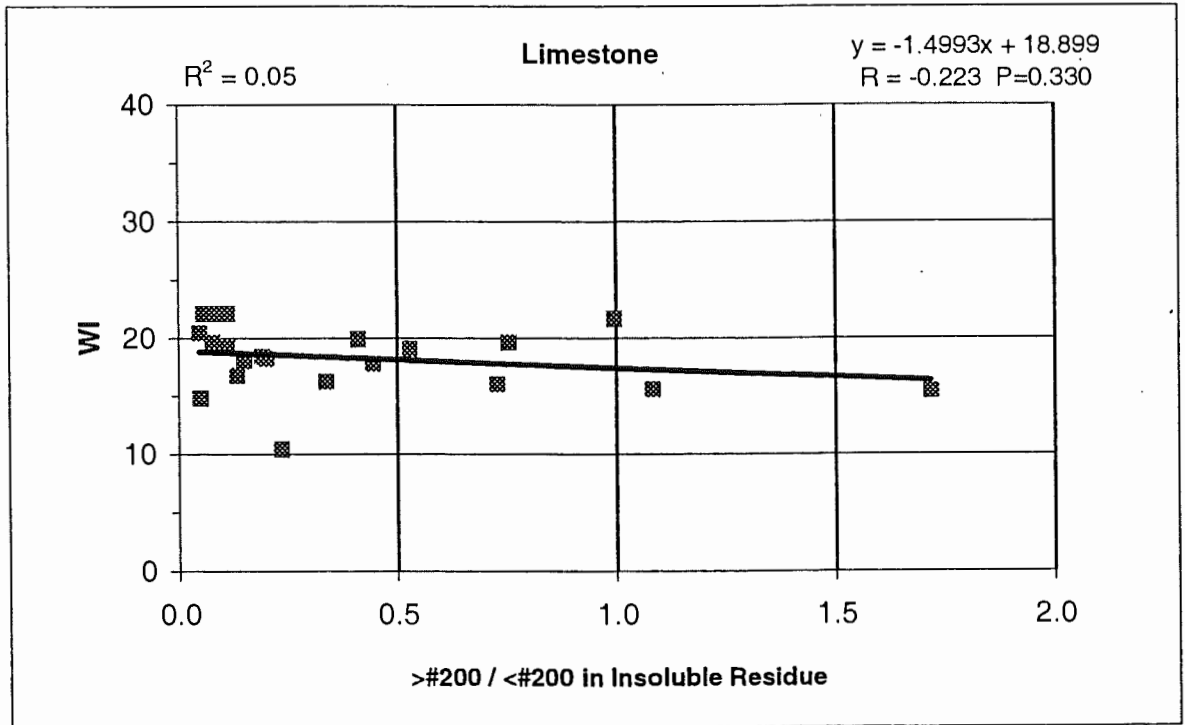


Figure 5-44. Correlation of Wear Index and the ratio of >#200 / <#200 in insoluble residue in limestone samples

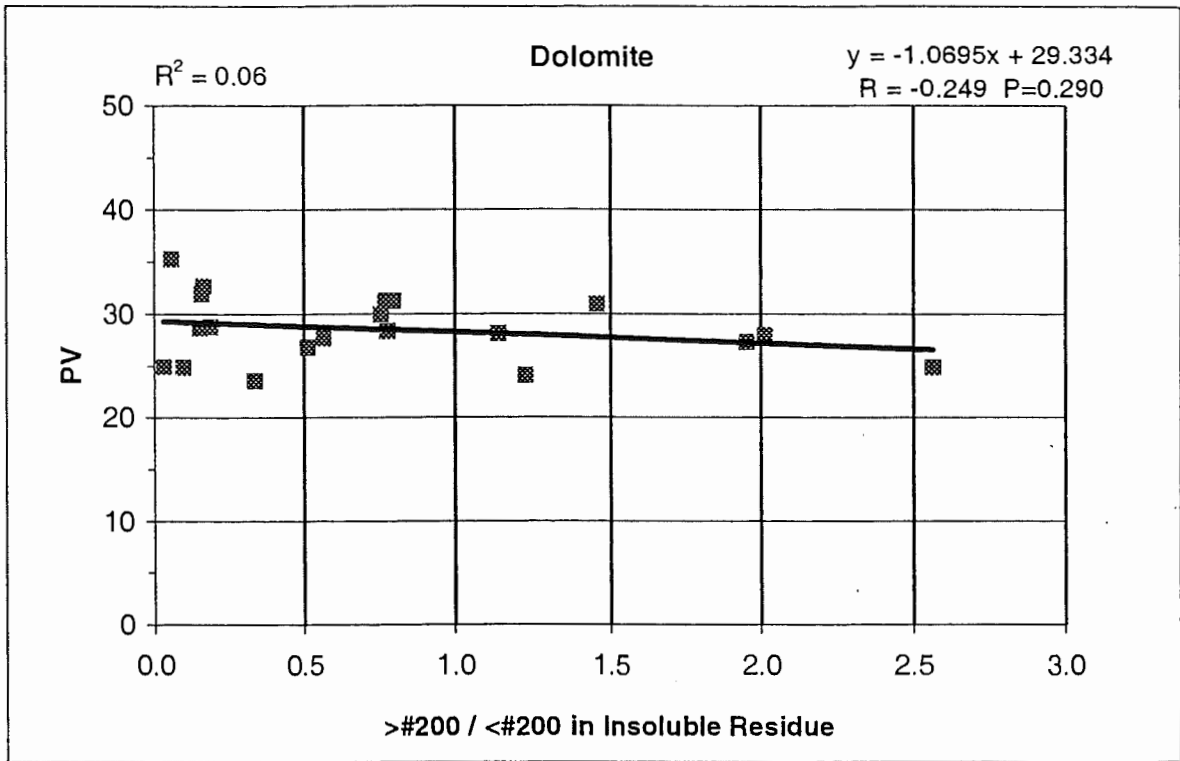


Figure 5-45. Correlation of Polished Value and the ratio of >#200 / <#200 in insoluble residue in dolomite samples

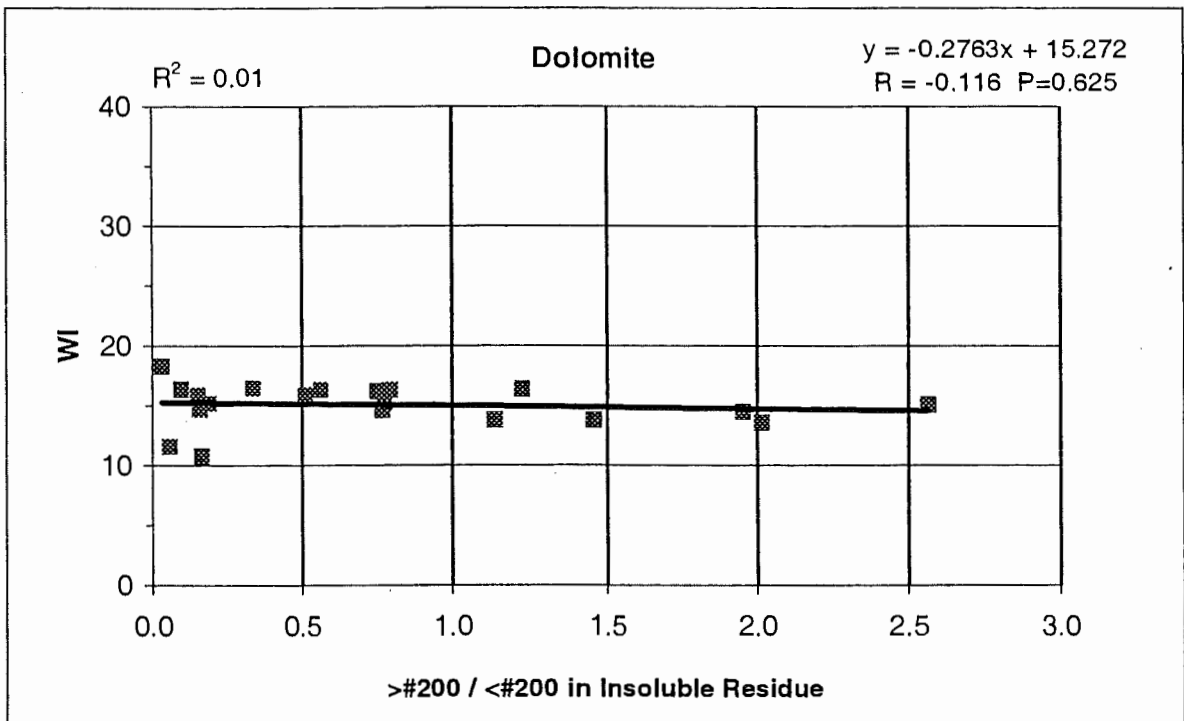


Figure 5-46. Correlation of Wear Index and the ratio of >#200 / <#200 in insoluble residue in dolomite samples

Table 5-9. Correlation matrix for carbonate aggregates

A. Limestone

	IFV	PV	WI	AB	SPG	LA	F-T	MG	T_ACID	P_200	M_200	RATIO
IFV	1	0.32288	0.6628	0.10318	-0.20263	0.20225	-0.58439	0.05871	-0.08972	0.05221	-0.14758	-0.11676
	0	0.1534	0.0011	0.6651	0.3916	0.4063	0.046	0.8004	0.699	0.8222	0.5232	0.6142
	21	21	21	20	20	19	12	21	21	21	21	21
PV	0.32288	1	-0.49469	0.0474	-0.0725	-0.21025	0.28632	0.20545	0.70343	0.48816	0.61593	0.1469
	0.1534	0	0.0226	0.8427	0.7613	0.3876	0.3669	0.3716	0.0004	0.0248	0.003	0.5252
	21	21	21	20	20	19	12	21	21	21	21	21
WI	0.6628	-0.49469	1	0.05648	-0.1274	0.35913	-0.6528	-0.10864	-0.63892	-0.33827	-0.62283	-0.22344
	0.0011	0.0226	0	0.813	0.5925	0.131	0.0214	0.6392	0.0018	0.1336	0.0026	0.3302
	21	21	21	20	20	19	12	21	21	21	21	21
AB	0.10318	0.0474	0.05648	1	-0.75713	0.10063	-0.13827	0.40689	0.00952	-0.05069	0.04289	0.00236
	0.6651	0.8427	0.813	0	0.0001	0.6819	0.6683	0.075	0.9682	0.8319	0.8575	0.9921
	20	20	20	20	20	19	12	20	20	20	20	20
SPG	-0.20263	-0.0725	-0.1274	-0.75713	1	-0.36788	0.19899	0.05525	0.11193	-0.11354	0.21331	-0.22235
	0.3916	0.7613	0.5925	0.0001	0	0.1212	0.5353	0.817	0.6385	0.6336	0.3665	0.3461
	20	20	20	20	20	19	12	20	20	20	20	20
LA	0.20225	-0.21025	0.35913	0.10063	-0.36788	1	-0.39756	-0.41644	-0.33871	-0.34775	-0.23565	-0.37292
	0.4063	0.3876	0.131	0.6819	0.1212	0	0.226	0.0761	0.156	0.1446	0.3314	0.1158
	19	19	19	19	19	19	11	19	19	19	19	19
F-T	-0.58439	0.28632	-0.6528	-0.13827	0.19899	-0.39756	1	0.12455	0.71913	0.44226	0.43973	0.32463
	0.046	0.3669	0.0214	0.6683	0.5353	0.226	0	0.6997	0.0084	0.15	0.1526	0.3032
	12	12	12	12	12	11	12	12	12	12	12	12
MG	0.05871	0.20545	-0.10864	0.40689	0.05525	-0.41644	0.12455	1	0.32261	0.0703	0.37509	-0.01538
	0.8004	0.3716	0.6392	0.075	0.817	0.0761	0.6997	0	0.1538	0.762	0.0938	0.9472
	21	21	21	20	20	19	12	21	21	21	21	21
T_ACID	-0.08972	0.70343	-0.63892	0.00952	0.11193	-0.33871	0.71913	0.32261	1	0.66525	0.89292	0.1802
	0.699	0.0004	0.0018	0.9682	0.6385	0.156	0.0084	0.1538	0	0.001	0.0001	0.4344
	21	21	21	20	20	19	12	21	21	21	21	21
P_200	0.05221	0.48816	-0.33827	-0.05069	-0.11354	-0.34775	0.44226	0.0703	0.66525	1	0.25789	0.78597
	0.8222	0.0248	0.1336	0.8319	0.6336	0.1446	0.15	0.762	0.001	0	0.2591	0.0001
	21	21	21	20	20	19	12	21	21	21	21	21
M_200	-0.14758	0.61593	-0.62283	0.04289	0.21331	-0.23565	0.43973	0.37509	0.89292	0.25789	1	-0.24075
	0.5232	0.003	0.0026	0.8575	0.3665	0.3314	0.1526	0.0938	0.0001	0.2591	0	0.2931
	21	21	21	20	20	19	12	21	21	21	21	21
RATIO	-0.11676	0.1469	-0.22344	0.00236	-0.22235	-0.37292	0.32463	-0.01538	0.1802	0.78597	-0.24075	1
	0.6142	0.5252	0.3302	0.9921	0.3461	0.1158	0.3032	0.9472	0.4344	0.0001	0.2931	0
	21	21	21	20	20	19	12	21	21	21	21	21



B. Dolomite

	IFV	PV	WI	AB	SPG	LA	SUL	MG	T_ACID	P_200	M_200	RATIO
IFV	1	0.83636	-0.17435	0.65025	-0.67087	0.13232	0.44441	-0.3454	0.41405	0.15757	0.54152	-0.42429
	0	0.0001	0.4622	0.0026	0.0017	0.5892	0.0646	0.1745	0.0695	0.507	0.0137	0.0623
	20	20	20	19	19	19	18	17	20	20	20	20
PV	0.83636	1	-0.6856	0.76523	-0.74493	0.10716	0.6034	-0.6527	0.44825	0.20234	0.55958	-0.24888
	0.0001	0	0.0008	0.0001	0.0003	0.6624	0.008	0.0045	0.0475	0.3923	0.0103	0.29
	20	20	20	19	19	19	18	17	20	20	20	20
WI	-0.17435	-0.6856	1	-0.53805	0.4624	-0.00849	-0.56742	0.75604	-0.25533	-0.1542	-0.28601	-0.11639
	0.4622	0.0008	0	0.0175	0.0462	0.9725	0.014	0.0004	0.2773	0.5163	0.2215	0.6251
	20	20	20	19	19	19	18	17	20	20	20	20
AB	0.65025	0.76523	-0.53805	1	-0.96817	0.42739	0.59233	-0.31927	0.0092	-0.21238	0.20439	-0.43279
	0.0026	0.0001	0.0175	0	0.0001	0.068	0.0096	0.2281	0.9702	0.3827	0.4013	0.0642
	19	19	19	19	19	19	18	16	19	19	19	19
SPG	-0.67087	-0.74493	0.4624	-0.96817	1	-0.5287	-0.58818	0.29564	-0.05644	0.19434	-0.26904	0.48014
	0.0017	0.0003	0.0462	0.0001	0	0.02	0.0102	0.2663	0.8185	0.4253	0.2654	0.0375
	19	19	19	19	19	19	18	16	19	19	19	19
LA	0.13232	0.10716	-0.00849	0.42739	-0.5287	1	-0.03362	0.41488	-0.2046	-0.28123	-0.09973	-0.30987
	0.5892	0.6624	0.9725	0.068	0.02	0	0.8946	0.1101	0.4008	0.2435	0.6846	0.1967
	19	19	19	19	19	19	18	16	19	19	19	19
SUL	0.44441	0.6034	-0.56742	0.59233	-0.58818	-0.03362	1	-0.76433	0.41806	0.22345	0.51016	-0.03427
	0.0646	0.008	0.014	0.0096	0.0102	0.8946	0	0.0009	0.0843	0.3728	0.0305	0.8926
	18	18	18	18	18	18	18	15	18	18	18	18
MG	-0.3454	-0.6527	0.75604	-0.31927	0.29564	0.41488	-0.76433	1	-0.63132	-0.45573	-0.65587	-0.12587
	0.1745	0.0045	0.0004	0.2281	0.2663	0.1101	0.0009	0	0.0066	0.066	0.0043	0.6303
	17	17	17	16	16	16	15	17	17	17	17	17
T_ACID	0.41405	0.44825	-0.25533	0.0092	-0.05644	-0.2046	0.41806	-0.63132	1	0.86083	0.90262	0.20251
	0.0695	0.0475	0.2773	0.9702	0.8185	0.4008	0.0843	0.0066	0	0.0001	0.0001	0.3918
	20	20	20	19	19	19	18	17	20	20	20	20
P_200	0.15757	0.20234	-0.1542	-0.21238	0.19434	-0.28123	0.22345	-0.45573	0.86083	1	0.55796	0.54759
	0.507	0.3923	0.5163	0.3827	0.4253	0.2435	0.3728	0.066	0.0001	0	0.0106	0.0124
	20	20	20	19	19	19	18	17	20	20	20	20
M_200	0.54152	0.55958	-0.28601	0.20439	-0.26904	-0.09973	0.51016	-0.65587	0.90262	0.55796	1	-0.1328
	0.0137	0.0103	0.2215	0.4013	0.2654	0.6846	0.0305	0.0043	0.0001	0.0106	0	0.5767
	20	20	20	19	19	19	18	17	20	20	20	20
RATIO	-0.42429	-0.24888	-0.11639	-0.43279	0.48014	-0.30987	-0.03427	-0.12587	0.20251	0.54759	-0.1328	1
	0.0623	0.29	0.6251	0.0642	0.0375	0.1967	0.8926	0.6303	0.3918	0.0124	0.5767	0
	20	20	20	19	19	19	18	17	20	20	20	20

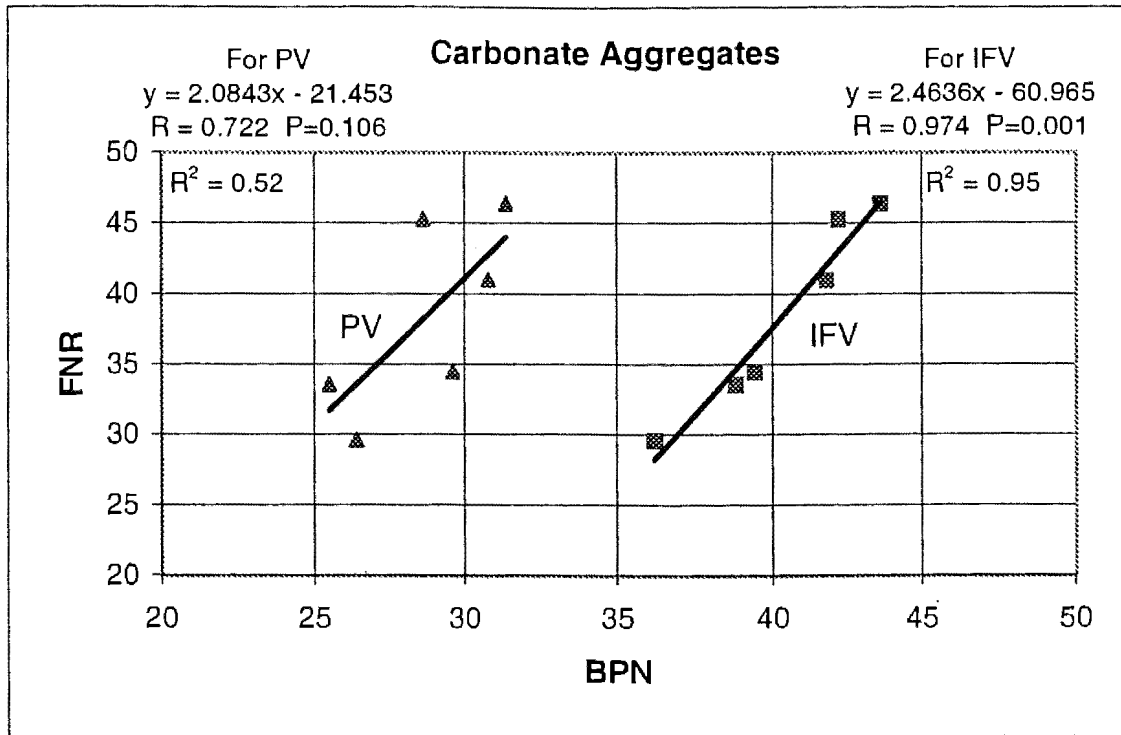


Figure 5-47. Correlation of Average Friction Number from ribbed trailer tire to IFV and PV determined from coupons composed of extracted core aggregates.

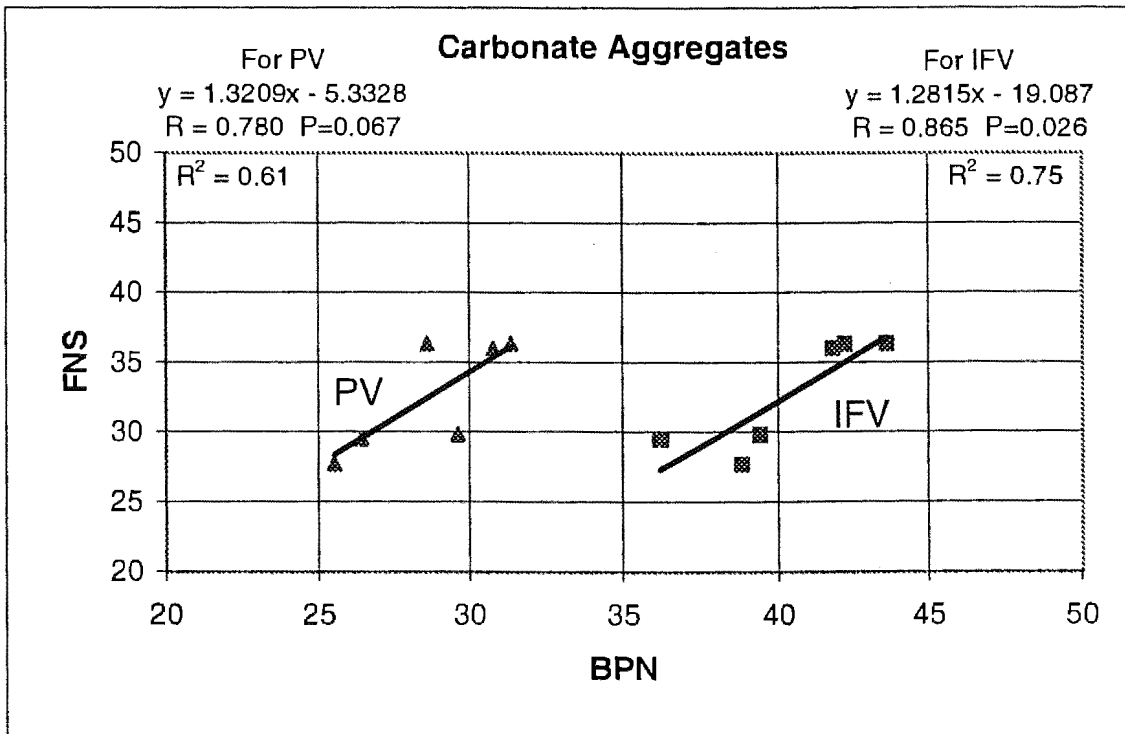


Figure 5-48. Correlation of Average Friction Number from smooth trailer tire to IFV and PV determined from coupons composed of extracted core aggregates.

## 5.3 Categorization of Carbonate Aggregates

### 5.3.1 Selection of critical parameters

For bituminous wearing courses, aggregates are required to meet various demands such as strength, durability and skid resistance. For INDOT Specifications (1999), the strength and durability requirements are designated for aggregate samples used as pavement materials. In the current study, the frictional resistance capability of aggregate sources is indicated based on the correlation analysis between PV and other parameters.

For dolomite aggregates, from results of the correlation analysis (Tables 5-9A and 5-9B) and confidence interval (Appendix 1) between polished value (PV) and other aggregate properties, the parameters having  $R > 0.50$  are initial friction value (IFV) ( $R=0.836$ ,  $0.625 \leq \rho \leq 0.933$ ,  $R^2 = 0.70$ ), absorption ( $R=0.765$ ,  $0.476 \leq \rho \leq 0.905$ ,  $R^2 = 0.59$ ), specific gravity ( $R=-0.745$ ,  $0.896 \leq \rho \leq 0.440$ ,  $R^2 = 0.56$ ), sulfate soundness loss ( $R=0.603$ ,  $0.189 \leq \rho \leq 0.835$ ,  $R^2 = 0.36$ ), elemental Mg content ( $R=-0.653$ ,  $0.863 \leq \rho \leq 0.251$ ,  $R^2 = 0.43$ ) and percent of acid insoluble residue smaller than #200 ( $R=0.560$ ,  $0.156 \leq \rho \leq 0.803$ ,  $R^2 = 0.31$ ) for dolomite aggregates.

For limestone aggregates, the parameters having  $R > 0.50$  are total acid-insoluble residue ( $R=0.703$ ,  $0.390 \leq \rho \leq 0.871$ ,  $R^2 = 0.49$ ) and percent residue less than #200 ( $R=0.616$ ,  $0.251 \leq \rho \leq 0.828$ ,  $R^2 = 0.38$ ).

Some physical properties of aggregates are interrelated. For dolomite aggregates, absorption is highly correlated with specific gravity ( $R=-0.968$ ) and sulfate soundness loss ( $R=0.592$ ). Elemental Mg content is well correlated with sulfate soundness loss ( $R=-0.764$ ) and percent of acid insoluble residue smaller than #200 ( $R=-0.656$ ).

For limestone aggregates, only absorption among physical properties is well correlated with specific gravity ( $R=-0.757$ ). In the regression analysis, the dependant variables are IFV and PV, and independent variables are physical properties, elemental Mg content and acid insoluble residue. The independent variables should be independent of each other.

### 5.3.2 Regression Analysis of Laboratory Data

In this research, a large quantity of data was generated based on laboratory analysis, INDOT records and the 1995 JTRP study. The goal of the statistical analysis was to determine aggregate properties that predict the performance of aggregates during in-service conditions with regard to skid resistance. Relevance of data was determined through correlation analysis, stepwise regression and multiple linear regression analyses.

PV was selected as the dependant variable and other laboratory data values were set as independent variables. Regression analysis was accomplished in three steps: First, the variables showing higher correlation were forced into regression and the relationship between PV and these laboratory variables were formulated separately for dolomite and limestone aggregates. Second, regression was conducted by stepwise method of the SAS program. PV and strongly related laboratory variables were chosen. Equations among these variables were generated separately for dolomite and limestone aggregates. Third, the dolomite and limestone data were combined into carbonate aggregates and analyzed collectively.

#### 5.3.2.1 Dolomite Aggregates

Among the laboratory variables for dolomite aggregates, absorption, elemental Mg content and acid insoluble residue <#200 were selected as independent variables. Using multiple linear-regression, the relationship between PV and these variables was developed:

$$\begin{aligned} \text{PV (1)} &= 38.0232 + 1.5568 (\text{Absorption}) - 1.1750 (\text{Mg content}) \\ &+ 0.3950 (\text{Insoluble residue } <\#200) \\ &(\text{R}^2 = 0.779, \text{ P } <21.19\%) \end{aligned}$$

Next, elemental Mg content was correlated with percent of acid insoluble residue less than #200. The above equation after removal of acid insoluble residue is as follows:

$$PV (2) = 46.0331 + 1.5645 (\text{Absorption}) - 1.7651 (\text{Mg content})$$

$$(R^2 = 0.747, P < 1.75\%)$$

Based on 5% significance level, PV (2) was selected as the appropriate equation for dolomite aggregates. Considering the average 2.406% for absorption values in dolomite samples and the 25 of PV for the quality aggregates, the elemental Mg content corresponds to 14.1%. This result indicates all ranges of dolomite (6.6% to 13.2% elemental Mg) have frictional resistance greater than PV=25, with minimum 1.45% absorption.

For dolomite aggregates, the higher elemental Mg values correspond with the purer dolomite aggregates. The higher purity dolomites are poorer performer with regard to skid resistance. In the Indiana specifications (INDOT, 1999), a minimum 10.3% elemental Mg content is required for carbonate aggregate sources based on a study performed in Illinois (Dierstein and LaCroix, 1984, 1990). It states: "The higher percentage of MgO corresponds with the higher frictional values". However, in the current study, the lower elemental Mg content corresponded with higher frictional values. Therefore, dolomites with less than 10.3% Mg should be considered as an acceptable aggregate source if they pass the other specifications such as absorption and sulfate soundness loss. This is also consistent with elemental Mg content being inversely correlated with sulfate soundness loss ( $R = -0.764$ ,  $R^2 = 0.58$ ). From Figure 5-49, PV can be estimated using measured absorption and elemental Mg content data.

### 5.3.2.2 Limestone Aggregates

From the laboratory test variables of limestone aggregates, absorption, elemental Mg content and total insoluble residue were selected as independent variables. Using multiple linear-regression, the relationship between PV and these variables is as follows:

$$PV (1) = 21.3374 + 0.2733 (\text{Absorption}) - 0.0376 (\text{Mg content})$$

$$+ 0.6161 (\text{Total insoluble residue})$$

$$(R^2 = 0.499, P < 81.35\%)$$

Next, only total insoluble residue was considered as an independent variable because absorption and Mg content do not correlate well with PV. The relationship between PV and total insoluble residue is as follows:

$$\text{PV (2)} = 21.7006 + 0.6029 (\text{Total insoluble residue})$$

$$(R^2 = 0.495, P < 0.05\%)$$

Using 5% significance level, PV (2) was selected as the appropriate equation for limestone aggregates. Considering the value of 25 for PV, the minimum required value yields a total insoluble residue of limestone aggregates equal to 5.47%.

### 5.3.2.3 Integration of dolomite and limestone data

IFV and PV values and laboratory data for dolomite and limestone were analyzed collectively to predict the performance of carbonate aggregates in frictional resistance. Absorption, elemental Mg content and total insoluble residue were selected as independent variables by stepwise regression. The relationship between PV and these variables is as follows:

$$\text{PV (3)} = 19.4659 + 1.7212(\text{Absorption}) + 0.2105 (\text{Mg content})$$

$$+ 0.47863(\text{Total insoluble residue})$$

$$(R^2 = 0.675, P < 0.61\%)$$

However, based on the distribution of elemental Mg content in carbonate aggregates, PV is more highly correlated with elemental Mg content by the 2<sup>nd</sup> degree polynomial equation (Figure 5-50) than by the linear equation. The equation is as follows:

$$\text{PV (4)} = 19.5464 + 1.7129 (\text{Absorption}) + 0.0164 (\text{Mg content})^2$$

$$+ 0.5189 (\text{Total insoluble residue})$$

$$(R^2 = 0.683, P < 0.39\%)$$

The measured PV is compared with predicted equations PV (1), PV (2), PV (3) and PV (4) as shown in Tables 5-10A and 5-10B. According to the comparison of the squares of residuals, the squared values of residuals are similar for PV (1) through PV (4).

### 5.3.3 Categorization of Polished Values

According to historical perspectives, the tentative acceptance criteria for the minimum polished value permitted for surface course mixes is as follows. These limits are considered as a starting point for future refinement (Dierstein and LaCroix, 1990).

Minimum Polish Value	Quality
24 or less	Poor
25 to 30	Marginal
31 or more	Good

According to the study on Alabama limestone and gravel aggregates as an asphalt wearing course, BPN9 values were divided into 3 categories as following (Kandhal et al., 1993):

BPN9 Values	Categories
Below 28	Low
28 to 32	Medium
32 or more	High

Also those authors noted that a general trend shows that as the insoluble residue percent (IR) increases the value of BPN9 also increases ( $R=0.414$ ). The relationship between two parameters was given by:

$$\text{BPN9} = 29.7 + 0.02 * (\% \text{ IR}).$$

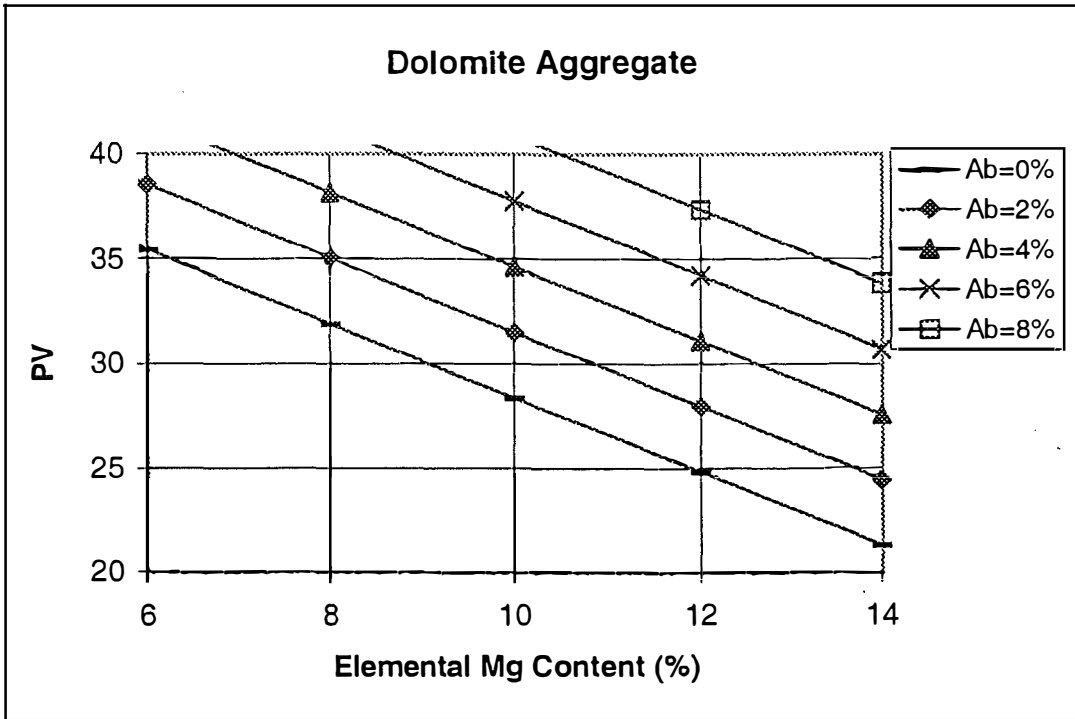


Figure 5-49. Prediction of polished value by absorption and elemental Mg content in dolomite aggregates

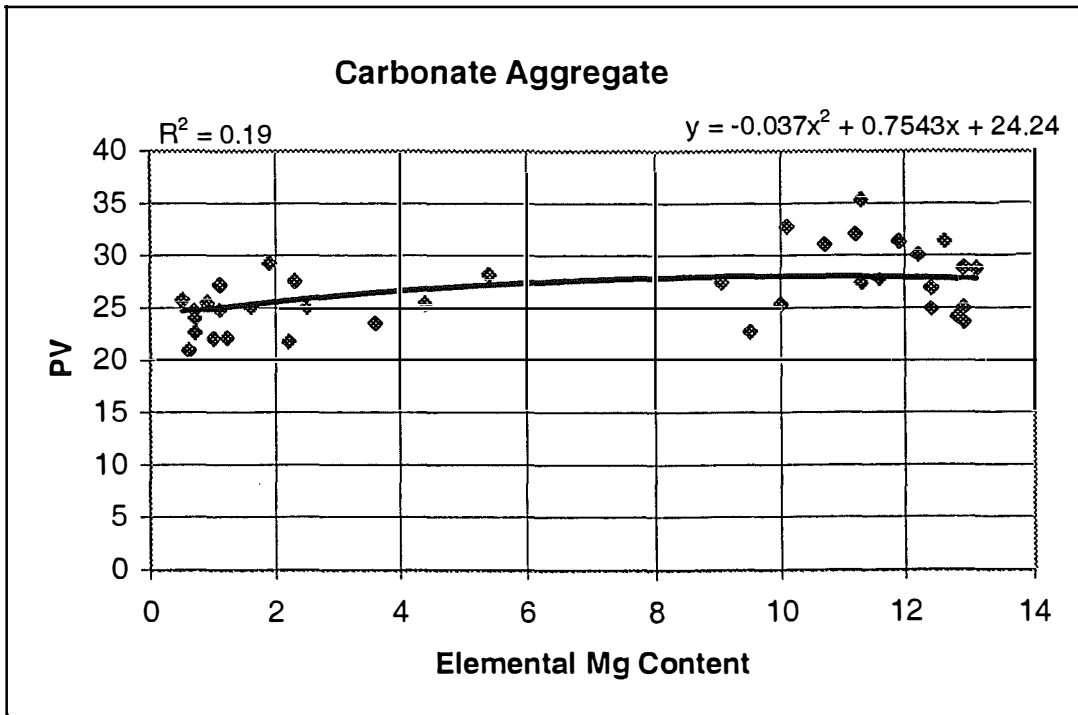


Figure 5-50. Non-linear relationship between polished value and elemental Mg content in carbonate Aggregates.





**B. Dolomite**

Id. No.	Measured PV	Pre. PV(1)	Res.(1)	SQ	Pre. PV (2)	Res.(2)	SQ	Pre. PV (3)	Res.(3)	SQ	Pre. PV (4)	Res.(4)	SQ
D-001	24.90	24.87	0.03	0.00	25.38	-0.48	0.23	24.25	0.65	0.42	24.30	0.60	0.36
D-002	25.00	26.86	-1.86	3.46	27.00	-2.00	4.01	26.63	-1.63	2.67	26.73	-1.73	2.99
D-003	27.30	29.27	-1.97	3.90	30.22	-2.92	8.51	27.89	-0.59	0.34	27.78	-0.48	0.23
D-004	35.30	35.27	0.03	0.00	35.87	-0.57	0.32	33.61	1.69	2.85	33.44	1.86	3.48
D-005	27.70	30.02	-2.32	5.38	28.72	-1.02	1.04	30.08	-2.38	5.69	30.30	-2.60	6.77
D-006	26.80	27.26	-0.46	0.22	26.59	0.21	0.05	27.29	-0.49	0.24	27.48	-0.68	0.46
D-007	28.80	28.06	0.74	0.55	28.03	0.77	0.59	28.07	0.73	0.53	28.19	0.61	0.37
D-008	28.70	26.42	2.28	5.22	26.63	2.07	4.27	26.43	2.27	5.17	26.54	2.16	4.65
D-009	31.00	28.53	2.47	6.11	29.04	1.96	3.84	27.35	3.65	13.31	27.34	3.66	13.38
D-010	31.30	30.00	1.30	1.69	29.68	1.62	2.64	30.58	0.72	0.52	30.74	0.56	0.31
D-011	30.00	28.25	1.75	3.06	27.66	2.34	5.48	28.53	1.47	2.16	28.72	1.28	1.65
D-012	31.30	29.99	1.31	1.72	29.31	1.99	3.94	30.35	0.95	0.91	30.53	0.77	0.60
D-013	32.70												
D-014	32.00	32.89	-0.89	0.79	32.52	-0.52	0.27	31.23	0.77	0.59	31.18	0.82	0.67
D-015	24.10	24.67	-0.57	0.33	25.05	-0.95	0.90	24.17	-0.07	0.01	24.25	-0.15	0.02
D-016	23.60	25.79	-2.19	4.78	26.17	-2.57	6.62	25.42	-1.82	3.32	25.50	-1.90	3.59
D-017	24.90	24.55	0.35	0.12	24.83	0.07	0.01	24.07	0.83	0.69	24.16	0.74	0.54
D-018	28.40												
D-019	28.00												
D-020	28.10												
Average	28.50	28.29	0.00	2.33	28.29	0.00	2.67	27.87	0.42	2.46	27.95	0.35	2.51

In the current study on Indiana limestones and the previous study on Indiana dolomites (Bruner, Choi and West, 1995), the BPN10 was used as the polished value. By the comparison between laboratory BPN and in-field AFN (Figures 5-47 and 5-48), PV=25 may be considered as a minimum acceptable friction value for bituminous pavement aggregates. Carbonate aggregates (dolomite and limestone) are categorized into three groups as following:

Polished Values	Categories
24 or less	Low
25 to 30	Medium
31 or more	High

## **5.4 Sandstone Study**

### 5.4.1 Selection of Sandstone Samples

Samples were obtained from three sandstone sources in Illinois. They were designated as "S-001" and "S-002" obtained from the Aux Vases Sandstone that is predominantly a well sorted, calcareous, and very fine-grained quartz sandstone. The sample designated "S-003" is from the Rosiclare Sandstone. Rosiclare Sandstone is also calcareous sandstone and is considered as a member of the Aux Vases Sandstone.

The petrographic description for the calcareous sandstone is as follows (Hockett, 1987):

"A calcareous sandstone in which the clastic quartz grains are bound in a matrix of calcite. The angular to subangular quartz grains are from 0.1 to 0.5mm in length and constitute about 60 percent of the material. The remainder is primarily a calcite cement, with grains 0.1 to 0.3mm in length, with a trace of microcline, plagioclase, quartzite or chert, and dark carbonaceous matter."

### 5.4.2 Physical Properties of Sandstone Samples

The physical test data for sandstone samples were obtained from INDOT and the data are listed in Table 5-11.

According to the results of the physical properties of the sandstone, absorption ranges from 1.12 % to 1.70 %, the specific gravity ranges from 2.584 to 2.624 and Los Angeles abrasion losses range from 26.24 % to 28.66 %. Freeze and thaw loss in brine is 30.28% for S-001 and the freeze and thaw loss in water ranges from 0.72 to 8.12% for S-002 and S-003.

### 5.4.3 Magnesium Content and Acid-insoluble Residue of Sandstone Samples

The elemental Mg content and the acid-insoluble residue of sandstone samples are listed in Table 5-12.

Table 5-11. Physical properties of sandstone samples

Id. No.	Absorption	Specific Gravity	L.A. Abrasion Loss	Freeze/Thaw Loss in water
S-001	-	-	-	*30.28%
S-002	1.70%	2.584	28.66%	8.12%
S-003	1.12%	2.624	26.24%	0.72%
Average	1.41%	2.604	27.45%	4.42%

- \*: Freeze - thaw loss in brine.

Table 5-12. Elemental Mg content and acid-insoluble residue of sandstone samples

Unit: %

Id. No.	Elemental Mg Content	Dolomite Content	Calcite Content	Acid-insoluble Residue			
				Total	> #200	< #200	>#200 / <#200
S-001	2.5	7.94	75.5	16.60	9.40	7.20	1.31
S-002	0.4	1.27	39.8	58.94	56.91	1.98	28.7
S-003	0.4	1.27	35.4	63.36	63.06	0.30	210.0
Average	1.1	3.49	50.2	46.30	43.12	3.16	80.0

Table 5-13. IFV, PV and WI for sandstone samples

Id. No.	IFV	PV	WI
S-001	45.59	26.11	19.48
S-002	50.08	36.42	13.66
S-003	51.45	38.30	13.15
Average	49.04	33.61	15.43

Total acid-insoluble residue of sandstone samples ranged from 16.00 to 63.36%. For percent of insoluble greater than #200, it ranged from 9.40 to 63.06% and for materials smaller than #200 it ranged from 0.30 to 7.20%. The ratio of percent material > #200 to percent material < #200 ranged from 1.31 to 210.0. Based on the petrographic analysis, S-001 is considered to be significantly more calcareous than the other two sandstone samples.

#### 5.4.4 Frictional Resistance Properties of Sandstone Samples

Initial Friction Value (IFV) and Polished Value (PV) were measured on sandstone coupons using the British Pendulum tester. The IFV and PV for sandstone samples are listed in Table 5-13.

As shown in Tables 5-12 and 5-13, as the total insoluble residue increases, IFV and PV also increase but WI decreases. Also, as the ratio of percent material > #200 to percent material < #200 increases, IFV and PV also increase but WI decreases. The reason that S-001 shows a lower PV than the other sandstones is that S-001 is more properly described as a quartzose limestone. Also, as shown in Table 5-11, S-001 has higher elemental Mg content and a lower acid insoluble residue than do S-002 and S-003.

### 5.5 Variation of Frictional Resistance Values

To evaluate the variation in frictional values between different test equipments in the current study, Polished Values were obtained from two different British Pendulum Testers: BPT1 (Purdue University equipment) and BPT2 (Alabama DOT equipment). The results are tabulated in Table 5-14 to Table 5-17 based on the various types of aggregate.

Table 5-14. Comparison of Polished Values of gravel aggregates obtained from BPT1 and BPT2.

Id. No.	BPT1	BPT2	Difference (BPT1-BPT2)
GR-1	26.29	25.94	0.35
GR-2	22.58	24.63	-2.05
GR-3	23.47	25.97	-2.50
GR-4	24.12	24.52	-0.40
GR-5	22.94	25.44	-2.50
GR-6	25.63	25.52	0.38
Average	24.17	25.29	-1.12

As shown in Table 5-14, on average, the PV from BPT2 are slightly greater than those from BPT1.

Referring to Table 5-15 for limestone aggregates, the PV from BPT 1 and BPT2 showed some discrepancies between the two PVs. On average, the PV from BPT2 is slightly higher than from BPT1.

Table 5-15. Polished Values of limestone aggregates from BPT1 and BPT2.

Id. No.	BPT1	BPT2	Difference (BPT1-BPT2)
L-001	28.14	26.03	2.11
L-002	24.72	26.83	-2.11
L-003	22.69	24.12	-1.43
L-004	23.49	25.28	-1.79
L-005	21.67	23.95	-2.28
L-006	20.87	21.63	-0.76
L-007	25.50	25.50	0.00
L-008	24.97	28.56	-3.59
L-009	27.40	26.50	0.90
L-010	25.39	22.22	3.17
L-011	25.39	26.04	-0.65
L-012	27.51	20.75	6.76
L-013	29.24	25.81	3.43
L-014	25.00	26.03	-1.03
L-015	27.12	26.98	0.14
L-016	24.72	27.14	-2.42
L-017	25.73	29.94	-4.21
L-018	22.55	27.51	-4.96
L-019	23.95	22.13	1.82
L-020	22.05	26.24	-4.19
L-021	22.01	20.87	1.14
Average	24.77	25.24	-0.47



As shown in Table 5-16, PVs for dolomite aggregates measured by BPT1 and BPT2 typically show sizable differences. The first five listed (D-001B, D-003, D-007, D-011 and D-020) are from the current study, evaluated at Purdue University (BPT1) and Alabama DOT (BPT2). The last two (D-001A and D-001C) are of a different nature.

Table 5-16. Polished Values of dolomite aggregates from BPT1 and BPT2.

Id. No.	BPT1	BPT2	Difference (BPT1-BPT2)
D-001B	20.67	21.50	-0.83
D-003	27.30	25.17	2.13
D-007	28.80	27.38	1.42
D-011	30.00	30.00	0.00
D-020	28.10	23.63	4.47
D-001A	24.90	21.83	3.07
D-001C	22.77	22.17	0.60
Average	26.87	25.34	1.53

D-001A is a dolomite coupon from the 1995 JTRP study and D-001C is from the current study with new epoxy material and the 1995 JTRP aggregate sample. There is a considerable discrepancy between these results for the same source. Factors involved are 1) a different location in the same aggregate quarry, 2) different epoxy materials used, and 3) a different technician conducting the test.

It is observed that PVs of BPT1 and BPT2 for D-001B and D-001C are more consistent than are those for D-001A.

The PVs for sandstone aggregates involving BPT1 and BPT2 are shown in Table 5-17. There is an average discrepancy between BPT1 and BPT2 of about 1.5 units.

Table 5-17. Polished Values of sandstone aggregates from BPT1 and BPT2.

Id. No.	BPT1	BPT2	Difference (BPT1-BPT2)
S-001	26.11	25.35	0.76
S-002	36.42	34.08	2.34
S-003	38.30	36.81	1.49
Average	33.61	32.08	

with  $R=0.76$  as shown in Figure 5-51.

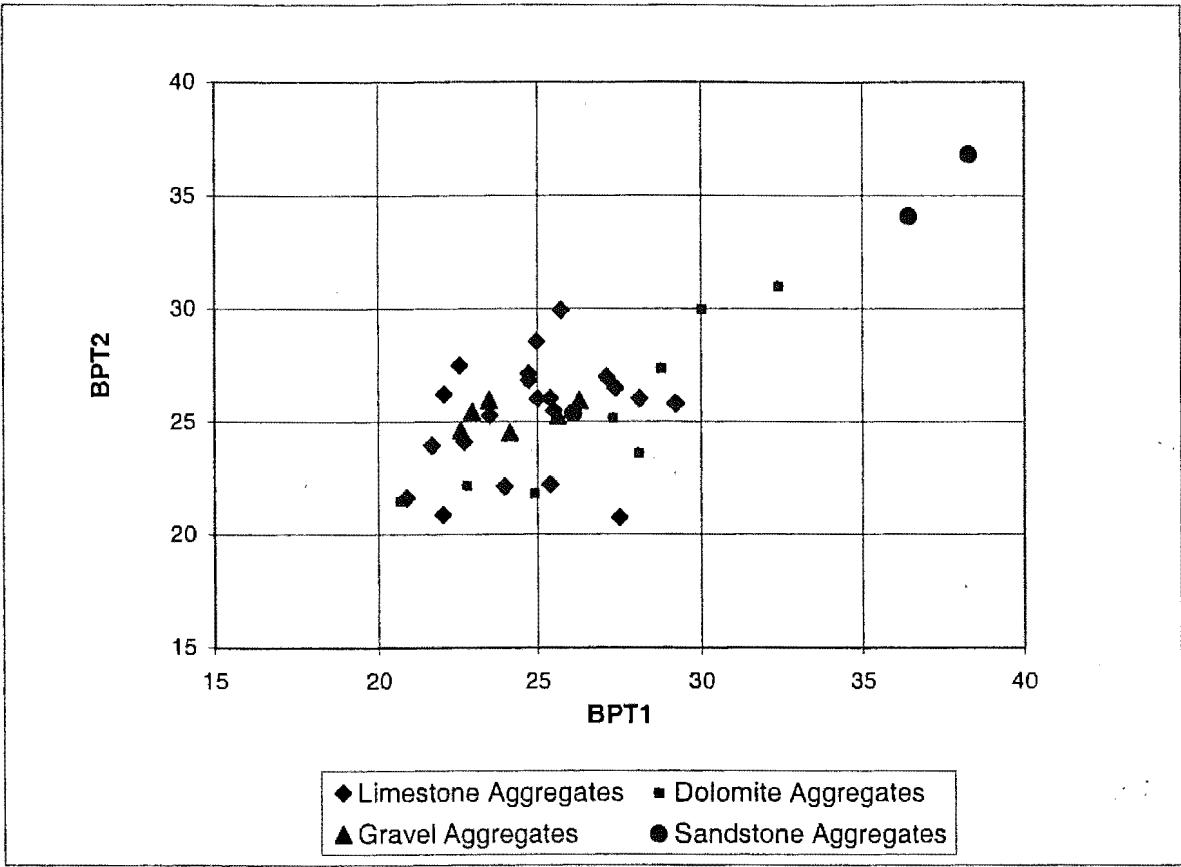


Figure 5-51. Comparison of Polished Values from BPT1 and BPT2 according to aggregate types

## 6. CONCLUSIONS

The research results for this study, Project SPR-2206, "Development of a Procedure to Identify Aggregates for Bituminous Surfaces in Indiana," proved to be quite significant. Results of this study are combined with those of 1995 JTRP on dolomite aggregates. The conclusions of this combined study are presented below:

### 1. Gravel Aggregates

- Gravels in this study consisted primarily of sedimentary rocks including limestones and dolomites. The weighted PV of gravel samples ranged from 22.6 to 26.3. PV correlates best with freeze-thaw loss ( $R=0.868$ ,  $R^2 = 0.75$ ). The other variables affecting the performance of gravel aggregates are absorption ( $R=0.832$ ,  $R^2 = 0.69$ ), percentage of crushed gravel pieces ( $R=0.804$ ,  $R^2 = 0.65$ ) and the percentage of metamorphic rocks present ( $R=0.830$ ,  $R^2 = 0.69$ ).
- Higher percentages of crushed gravel pieces show higher frictional resistance.

### 2 Carbonate Aggregates

- The acid insoluble residue results provide valuable data related to frictional resistance of carbonate aggregates. This test should be performed on aggregates used in bituminous surface courses.
- The difference in mineral hardness within a rock piece yielding an uneven texture during polishing, has a major influence on aggregate performance. For example, quartz and calcite in a calcareous sandstone, calcite and insoluble residue (fine quartz and clay) in limestone, dolomite and calcite in a dolomitic limestone or calcareous dolomite, and an impure dolomite and impure limestone.
- Regarding geologic strata, for dolomite aggregates, the Kokomo Member shows the highest PV=33.3, as the Kokomo Member is impure (10.3% Mg and 4.31% insoluble residue). By contrast, the Huntington Dolomite shows the lowest PV=25.4 because

Huntington Dolomite is very pure (12.4% Mg and a lower 3.09% insoluble residue). Among limestone aggregates, the Mississinewa Member shows the highest PV=28.1 because the Mississinewa Member is very impure (5.40% Mg and 12.72% insoluble residue). By contrast, the Brassfield Limestone shows the lowest PV=20.9 because of its purity (0.6% Mg and 1.97% insoluble residue). The higher friction values in comparable dolomite aggregates may be due to dolomite's greater hardness.

- The important factors affecting the PV of dolomite aggregates are IFV ( $R=0.836$ ,  $R^2 = 0.70$ ), absorption ( $R=0.765$ ,  $R^2 = 0.59$ ), specific gravity ( $R=-0.745$ ,  $R^2 = 0.56$ ), sulfate soundness loss ( $R=0.603$ ,  $R^2 = 0.36$ ), elemental Mg content ( $R=-0.653$ ,  $R^2 = 0.43$ ) and percentage of insoluble residue smaller than #200 ( $R=0.560$ ,  $R^2 = 0.31$ ). Among the variables, IFV and PV, absorption, specific gravity and sulfate soundness loss, and the elemental Mg content and the percentage of insoluble residue are intercorrelated with each other. Therefore the most effective independent variables for dolomite aggregate performance are absorption and elemental Mg content.
- The important factors affecting the PV of the limestone aggregates are total insoluble residue ( $R=0.703$ ,  $R^2 = 0.49$ ) and percent residue smaller than #200 ( $R=0.616$ ,  $R^2 = 0.38$ ). Both of these, insoluble residue and percent residue smaller than #200, are intercorrelated with each other. Therefore the most effective independent factor for limestone aggregates is the total insoluble residue as it has the higher R-value.
- Considering dolomite and limestone aggregates collectively as carbonate aggregates, the important factors affecting the performance of these carbonate aggregates are absorption ( $R=0.664$ ,  $R^2 = 0.44$ ), specific gravity ( $R=-0.556$ ,  $R^2 = 0.31$ ), and elemental Mg content ( $R=0.497$ ,  $R^2 = 0.25$ ). Absorption and specific gravity are highly dependant on each other. The total acid insoluble residue correlates well with both the percentage of insoluble residue <#200 and the percentage >#200. Therefore, the most effective variables for carbonate aggregates collectively are absorption, elemental Mg content and total insoluble residue.
- Multiple linear regression proved satisfactory in determining interactions between laboratory tests performed on dolomites and limestones in regard to friction values.

Empirical equations were developed providing reasonable probability for estimating the laboratory friction values. These equations are as follows:

For dolomite aggregate at the 5% significance level,

$$PV = 46.0331 + 1.5645 (\text{Absorption}) - 1.7651 (\text{Mg content})$$

For limestone aggregates at the 5% significance level,

$$PV = 21.7006 + 0.6029 (\text{Total insoluble residue})$$

For carbonate aggregates at the 5% significance level,

$$PV = 19.5464 + 1.7129 (\text{Absorption}) + 0.0164 (\text{Mg content})^2 \\ + 0.5189 (\text{Total insoluble residue})$$

- For dolomite aggregates, the higher elemental Mg values relate to higher purity dolomite materials. These dolomites have a lower friction resistance. Regarding INDOT specifications, a minimum 10.3% elemental Mg content is required for carbonate aggregates used for surface courses with intermediate traffic requirements. This is a consequence of the study of Illinois aggregates, which stated, “The higher percentage of MgO corresponds with the higher frictional values”. However, in the current study, lower elemental Mg content values correspond to higher frictional values. Therefore dolomite aggregates with less than 10.3% elemental Mg should be considered as potential aggregate sources if these aggregates pass the other specifications such as absorption and soundness loss.
- For Class A aggregate (INDOT Standard Specifications, 1999) carbonate aggregates show an increase in frictional resistance with increasing absorption percentage. However, aggregates with greater than 5% absorption do not qualify as Class A aggregates and there is no indication that aggregates with greater than 5% absorption would have good friction values. Strength and durability problems are likely to occur above the 5% level.

### 3. Sandstone Aggregates

- Based on the study of calcareous sandstones, as total insoluble residue (quartz, clay) increases, PV increases. Also, as the ratio of +#200 to -#200 size insoluble residue increases, PV also increases
- Sandstone aggregates showed a higher average frictional resistance  $PV=33.61$  than did carbonate aggregates (average  $PV=28.50$  for dolomite and average  $PV=24.77$  for limestone) because of the heterogeneity of calcareous sandstones. Quartz, calcite and clay provide this heterogeneity.

## 7. RECOMMENDATIONS

Some recommendations on a procedure to identify a quality aggregates were made based on the overall results of the 1995 JTRP, SPR-2206 (Bruner, Choi and West, 1995) study and the current study.

- Crushed gravel aggregates and heterogeneous sandstone aggregates performed better in bituminous surface courses than did some crushed carbonate aggregates. These gravel and sandstone aggregates should be considered for use in a wearing course of bituminous pavements.
- Both impure limestones and impure dolomites performed better as bituminous surface courses than did either pure limestones or pure dolomites. These impure carbonates should be considered for use in wearing courses of bituminous pavements.
- Frictional resistance of dolomite aggregates can be predicted based on absorption and elemental Mg content. Frictional resistance of limestone aggregates can be also predicted from acid insoluble residue values. It is recommended, however, that the aggregates used must satisfy other aggregate specifications such as soundness loss (or freeze thaw) and maximum allowable absorption for Class A stone.
- The higher purity dolomites showed a lower friction resistance in the current study. Regarding INDOT specifications, a minimum 10.3% elemental Mg content is required for carbonate aggregates for bituminous surface courses with intermediate traffic requirements. However, based on the current study, dolomites with less than 10.3% elemental Mg should be considered for use in surface courses if these aggregates pass the other specifications such as absorption and soundness loss. Candidates should qualify as Class A materials, as high absorption and soundness loss may give rise to a strength or durability problem. Absorption values for Class A stone should be evaluated to select potential aggregates containing less than 10.3% Mg, for possible use in bituminous overlays.
- Limited data on field and laboratory measurements (AFN and BPN) from the 1995 JTRP study suggest that a PV of 25 be considered as a minimum acceptable friction value for bituminous pavement aggregates with intermediate traffic requirements. It is



recommended that some limestones be considered for bituminous surface courses, but the field performance of these limestones should to be verified.

- A discrepancy in PV values was observed between the two pieces of equipment used for British Pendulum testing. Before BPN is selected as a standard criterion for evaluating aggregate quality in Indiana, this standard method must be verified using new equipment and a detailed evaluation of inherent variations. PVs need to be evaluated on a continuing basis as PV can vary to some extent within the same aggregate source.

## **8. IMPLEMENTATION SUGGESTIONS**

Based on the current study high purity dolomites show a lower friction resistance, or, impure dolomites (low Mg) show a higher frictional value. Based on INDOT specifications, a minimum 10.3% elemental Mg content is required for carbonate aggregates when used for surface courses with intermediate traffic requirements. Therefore dolomites with less than 10.3% elemental Mg should be considered for bituminous surface courses with intermediate traffic requirements if these aggregates pass the other specifications such as absorption and soundness loss.

Carbonates with higher insoluble residue contents show a higher friction resistance than do purer carbonates with their low insoluble residue values. Data on insoluble residue content including grain size evaluation (+#200 size fraction vs. - #200 size fraction) should also be determined. A data base for aggregates used in bituminous wearing courses should be compiled by INDOT which includes the insoluble residue content and absorption values. Elemental Mg and elemental Ca should also be determined.

A discrepancy in PV was observed between different pieces of test equipment for the British Pendulum test (Purdue University equipment vs. Alabama DOT equipment). Before BPN is selected as a standard criterion for evaluating aggregate quality in Indiana, the standard method must be developed using new equipment and a detailed evaluation of inherent variations. PVs need to be evaluated on a continuing basis because PV can vary even within the same aggregate source.

Some impure limestones with a higher frictional value should be considered for surface courses of bituminous overlays. However, field performance of these limestones must be verified through field and laboratory evaluation prior to their use as overlays.

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## APPENDICIES



Appendix 1. Confidence Interval for R-value (Fisher's Transformation)

A. Limestone

Variables	R-value	z'	n	$\sigma(z')$	$u_{z'}(\text{min.})$	$u_{z'}(\text{max.})$	$\rho(\text{min.})$	$\rho(\text{max.})$
IFV - PV	0.323	0.335	21	0.236	-0.127	0.797	-0.126	0.662
IFV - WI	0.663	0.798	21	0.236	0.336	1.260	0.324	0.851
IFV - AB	0.103	0.103	20	0.243	-0.372	0.579	-0.356	0.522
IFV - SPG	-0.203	-0.206	20	0.243	-0.681	0.270	-0.592	0.263
IFV - LA	0.202	0.205	19	0.250	-0.285	0.695	-0.278	0.601
IFV - F-T	-0.584	-0.669	12	0.333	-1.322	-0.015	-0.867	-0.015
IFV - MG	0.059	0.059	21	0.236	-0.403	0.521	-0.382	0.479
IFV - T_ACID	-0.090	-0.090	21	0.236	-0.552	0.372	-0.502	0.356
IFV - P_200	0.052	0.052	21	0.236	-0.410	0.514	-0.388	0.473
IFV - M_200	-0.148	-0.149	21	0.236	-0.611	0.313	-0.545	0.303
IFV - RATIO	-0.117	-0.118	21	0.236	-0.580	0.344	-0.522	0.331
PV - WI	-0.495	-0.543	21	0.236	-1.005	-0.081	-0.764	-0.081
PV - AB	0.047	0.047	20	0.243	-0.428	0.522	-0.404	0.480
PV - SPG	-0.073	-0.073	20	0.243	-0.548	0.402	-0.499	0.382
PV - LA	-0.210	-0.213	19	0.250	-0.703	0.277	-0.606	0.270
PV - F-T	0.286	0.294	12	0.333	-0.359	0.948	-0.344	0.739
PV - MG	0.205	0.208	21	0.236	-0.254	0.670	-0.249	0.585
PV - T_ACID	0.703	0.873	21	0.236	0.411	1.335	0.390	0.871
PV - P_200	0.488	0.533	21	0.236	0.071	0.995	0.071	0.760
PV - M_200	0.616	0.719	21	0.236	0.257	1.181	0.251	0.828
PV - RATIO	0.147	0.148	21	0.236	-0.314	0.610	-0.304	0.544
WI - AB	0.056	0.056	20	0.243	-0.419	0.531	-0.396	0.486
WI - SPG	-0.127	-0.128	20	0.243	-0.603	0.348	-0.539	0.334
WI - LA	0.359	0.376	19	0.250	-0.114	0.866	-0.114	0.699
WI - F-T	-0.653	-0.781	12	0.333	-1.434	-0.127	-0.892	-0.126
WI - MG	-0.109	-0.109	21	0.236	-0.571	0.353	-0.516	0.339
WI - T_ACID	-0.639	-0.756	21	0.236	-1.218	-0.295	-0.839	-0.286
WI - P_200	-0.338	-0.352	21	0.236	-0.814	0.110	-0.672	0.110
WI - M_200	-0.623	-0.730	21	0.236	-1.192	-0.268	-0.831	-0.262
WI - RATIO	-0.223	-0.227	21	0.236	-0.689	0.235	-0.597	0.231

$$z' = 0.5 \cdot \ln[(1+R)/(1-R)]$$

$$\sigma(z') = 1/\sqrt{n-3}$$

n = number of samples

$$u_{z'} = z' \pm z_{(0.5\alpha)} \cdot \sigma(z')$$

$z_{(0.5\alpha)} = 1.96$  for 95% confidence interval

B. Dolomite

Variables	r-value	z'	n	$\sigma(z')$	$u_z(\text{min.})$	$u_z(\text{max.})$	$\rho(\text{min.})$	$\rho(\text{max.})$
IFV - PV	0.836	1.208	20	0.243	0.732	1.683	0.625	0.933
IFV - WI	-0.174	-0.176	20	0.243	-0.651	0.300	-0.572	0.291
IFV - AB	0.650	0.775	19	0.250	0.285	1.265	0.278	0.853
IFV - SPG	-0.671	-0.813	19	0.250	-1.303	-0.323	-0.862	-0.312
IFV - LA	0.132	0.133	19	0.250	-0.357	0.623	-0.343	0.553
IFV - SUL	0.444	0.477	18	0.258	-0.029	0.983	-0.029	0.754
IFV - MG	-0.345	-0.360	17	0.267	-0.884	0.164	-0.708	0.163
IFV - T_ACID	0.414	0.440	20	0.243	-0.035	0.916	-0.035	0.724
IFV - P_200	0.158	0.159	20	0.243	-0.316	0.635	-0.306	0.561
IFV - M_200	0.542	0.607	20	0.243	0.132	1.082	0.131	0.794
IFV - RATIO	-0.424	-0.453	20	0.243	-0.928	0.023	-0.730	0.023
PV - WI	-0.686	-0.840	20	0.243	-1.316	-0.365	-0.866	-0.350
PV - AB	0.765	1.008	19	0.250	0.518	1.498	0.476	0.905
PV - SPG	-0.745	-0.962	19	0.250	-1.452	-0.472	-0.896	-0.440
PV - LA	0.107	0.107	19	0.250	-0.383	0.597	-0.365	0.535
PV - SUL	0.603	0.698	18	0.258	0.192	1.204	0.189	0.835
PV - MG	-0.653	-0.781	17	0.267	-1.304	-0.257	-0.863	-0.251
PV - T_ACID	0.448	0.482	20	0.243	0.007	0.958	0.007	0.743
PV - P_200	0.202	0.205	20	0.243	-0.271	0.680	-0.264	0.592
PV - M_200	0.560	0.633	20	0.243	0.157	1.108	0.156	0.803
PV - RATIO	-0.249	-0.254	20	0.243	-0.730	0.221	-0.623	0.217
WI - AB	-0.538	-0.601	19	0.250	-1.091	-0.111	-0.797	-0.111
WI - SPG	0.462	0.500	19	0.250	0.010	0.990	0.010	0.757
WI - LA	-0.008	-0.008	19	0.250	-0.498	0.482	-0.461	0.448
WI - SUL	-0.567	-0.643	18	0.258	-1.149	-0.137	-0.817	-0.136
WI - MG	0.756	0.987	17	0.267	0.463	1.511	0.433	0.907
WI - T_ACID	-0.255	-0.261	20	0.243	-0.736	0.215	-0.627	0.211
WI - P_200	-0.154	-0.155	20	0.243	-0.631	0.320	-0.558	0.310
WI - M_200	-0.286	-0.294	20	0.243	-0.770	0.181	-0.647	0.179
WI - RATIO	-0.116	-0.117	20	0.243	-0.592	0.359	-0.531	0.344

## C. Gravel

Variables	r-value	z'	n	$\sigma(z')$	$u_z(\text{min.})$	$u_z(\text{max.})$	$\rho(\text{min.})$	$\rho(\text{max.})$
IFV - PV	0.837	1.211	6	0.577	0.079	2.343	0.079	0.982
IFV - WI	0.924	1.616	6	0.577	0.484	2.747	0.450	0.992
IFV - AB	0.893	1.437	6	0.577	0.305	2.568	0.296	0.988
IFV - SPG	-0.372	-0.391	6	0.577	-1.522	0.741	-0.909	0.630
IFV - LA	0.563	0.637	6	0.577	-0.494	1.769	-0.458	0.943
IFV - F-T	0.846	1.242	6	0.577	0.110	2.374	0.110	0.983
IFV - IG%	-0.178	-0.180	6	0.577	-1.312	0.952	-0.865	0.741
IFV - META%	0.506	0.557	6	0.577	-0.574	1.689	-0.518	0.934
IFV - LS%	-0.031	-0.031	6	0.577	-1.163	1.101	-0.822	0.801
IFV - DOL%	-0.006	-0.006	6	0.577	-1.138	1.126	-0.814	0.810
IFV - OS%	-0.025	-0.025	6	0.577	-1.157	1.107	-0.820	0.803
IFV - CRUSH%	0.890	1.422	6	0.577	0.290	2.554	0.282	0.988
PV - WI	0.565	0.640	6	0.577	-0.491	1.772	-0.455	0.944
PV - AB	0.832	1.195	6	0.577	0.063	2.326	0.063	0.981
PV - SPG	-0.573	-0.652	6	0.577	-1.784	0.480	-0.945	0.446
PV - LA	0.714	0.895	6	0.577	-0.236	2.027	-0.232	0.966
PV - F-T	0.868	1.325	6	0.577	0.193	2.457	0.191	0.985
PV - IG%	0.082	0.082	6	0.577	-1.049	1.214	-0.782	0.838
PV - META%	0.830	1.188	6	0.577	0.057	2.320	0.056	0.981
PV - LS%	-0.329	-0.342	6	0.577	-1.473	0.790	-0.900	0.658
PV - DOL%	-0.494	-0.541	6	0.577	-1.673	0.590	-0.932	0.530
PV - OS%	0.228	0.232	6	0.577	-0.900	1.364	-0.716	0.877
PV - CRUSH%	0.804	1.110	6	0.577	-0.022	2.241	-0.022	0.978
WI - AB	0.766	1.011	6	0.577	-0.121	2.142	-0.120	0.973
WI - SPG	-0.162	-0.163	6	0.577	-1.295	0.968	-0.860	0.748
WI - LA	0.351	0.367	6	0.577	-0.765	1.498	-0.644	0.905
WI - F-T	0.727	0.922	6	0.577	-0.209	2.054	-0.206	0.968
WI - IG%	-0.325	-0.337	6	0.577	-1.469	0.794	-0.899	0.661
WI - META%	0.183	0.185	6	0.577	-0.947	1.317	-0.738	0.866
WI - LS%	0.183	0.185	6	0.577	-0.947	1.317	-0.738	0.866
WI - DOL%	0.336	0.350	6	0.577	-0.782	1.481	-0.654	0.902
WI - OS%	-0.197	-0.200	6	0.577	-1.331	0.932	-0.870	0.732
WI - CRUSH%	0.781	1.048	6	0.577	-0.084	2.180	-0.083	0.975

Appendix 2. Correlation Analysis  
 A. Limestone

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Correlation Analysis

12 'VAR' Variables: IFV PV WI AB SPG LA SUL MG  
 T\_ACID P\_200 M\_200 RATIO

Simple Statistics

Variable	N	Mean	Std Dev	Sum	Minimum	Maximum
IFV	21	43.063333	2.659395	904.330000	38.600000	48.870000
PV	21	24.767143	2.291369	520.110000	20.870000	29.240000
WI	21	18.296190	2.896157	384.220000	10.460000	22.140000
AB	20	1.495500	0.525029	29.910000	0.780000	2.470000
SPG	20	2.628950	0.039451	52.579000	2.517000	2.698000
LA	19	28.191579	4.755024	535.640000	21.370000	38.170000
SUL	12	12.945000	7.390808	155.340000	3.010000	27.460000
MG	21	2.911905	3.063246	61.150000	0.500000	10.000000
T_ACID	21	5.066190	2.677065	106.390000	1.410000	12.720000
P_200	21	1.306190	1.247431	27.430000	0.120000	4.830000
M_200	21	3.760000	2.068719	78.960000	1.270000	10.320000
RATIO	21	0.402857	0.436362	8.460000	0.050000	1.720000

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IFV	PV	WI	AB	SPG	LA
IFV	1.00000 0.0 21	0.32288 0.1534 21	0.66280 0.0011 21	0.10318 0.6651 20	-0.20263 0.3916 20	0.20225 0.4063 19
PV	0.32288 0.1534 21	1.00000 0.0 21	-0.49469 0.0226 21	0.04740 0.8427 20	-0.07250 0.7613 20	-0.21025 0.3876 19
WI	0.66280 0.0011 21	-0.49469 0.0226 21	1.00000 0.0 21	0.05648 0.8130 20	-0.12740 0.5925 20	0.35913 0.1310 19
AB	0.10318 0.6651 20	0.04740 0.8427 20	0.05648 0.8130 20	1.00000 0.0 20	-0.75713 0.0001 20	0.10063 0.6819 19

SPG	-0.20263	-0.07250	-0.12740	-0.75713	1.00000	-0.36788
	0.3916	0.7613	0.5925	0.0001	0.0	0.1212
	20	20	20	20	20	19
LA	0.20225	-0.21025	0.35913	0.10063	-0.36788	1.00000
	0.4063	0.3876	0.1310	0.6819	0.1212	0.0
	19	19	19	19	19	19
SUL	-0.58439	0.28632	-0.65280	-0.13827	0.19899	-0.39756
	0.0460	0.3669	0.0214	0.6683	0.5353	0.2260
	12	12	12	12	12	11
MG	0.05871	0.20545	-0.10864	0.40689	0.05525	-0.41644
	0.8004	0.3716	0.6392	0.0750	0.8170	0.0761
	21	21	21	20	20	19
T_ACID	-0.08972	0.70343	-0.63892	0.00952	0.11193	-0.33871
	0.6990	0.0004	0.0018	0.9682	0.6385	0.1560
	21	21	21	20	20	19
P_200	0.05221	0.48816	-0.33827	-0.05069	-0.11354	-0.34775
	0.8222	0.0248	0.1336	0.8319	0.6336	0.1446
	21	21	21	20	20	19
M_200	-0.14758	0.61593	-0.62283	0.04289	0.21331	-0.23565
	0.5232	0.0030	0.0026	0.8575	0.3665	0.3314
	21	21	21	20	20	19

The SAS System

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IFV	PV	WI	AB	SPG	LA
RATIO	-0.11676	0.14690	-0.22344	0.00236	-0.22235	-0.37292
	0.6142	0.5252	0.3302	0.9921	0.3461	0.1158
	21	21	21	20	20	19

The SAS System

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	SUL	MG	T_ACID	P_200	M_200	RATIO
IFV	-0.58439	0.05871	-0.08972	0.05221	-0.14758	-0.11676
	0.0460	0.8004	0.6990	0.8222	0.5232	0.6142
	12	21	21	21	21	21
PV	0.28632	0.20545	0.70343	0.48816	0.61593	0.14690
	0.3669	0.3716	0.0004	0.0248	0.0030	0.5252

	12	21	21	21	21	21
WI	-0.65280	-0.10864	-0.63892	-0.33827	-0.62283	-0.22344
	0.0214	0.6392	0.0018	0.1336	0.0026	0.3302
	12	21	21	21	21	21
AB	-0.13827	0.40689	0.00952	-0.05069	0.04289	0.00236
	0.6683	0.0750	0.9682	0.8319	0.8575	0.9921
	12	20	20	20	20	20
SPG	0.19899	0.05525	0.11193	-0.11354	0.21331	-0.22235
	0.5353	0.8170	0.6385	0.6336	0.3665	0.3461
	12	20	20	20	20	20
LA	-0.39756	-0.41644	-0.33871	-0.34775	-0.23565	-0.37292
	0.2260	0.0761	0.1560	0.1446	0.3314	0.1158
	11	19	19	19	19	19
SUL	1.00000	0.12455	0.71913	0.44226	0.43973	0.32463
	0.0	0.6997	0.0084	0.1500	0.1526	0.3032
	12	12	12	12	12	12
MG	0.12455	1.00000	0.32261	0.07030	0.37509	-0.01538
	0.6997	0.0	0.1538	0.7620	0.0938	0.9472
	12	21	21	21	21	21
T_ACID	0.71913	0.32261	1.00000	0.66525	0.89292	0.18020
	0.0084	0.1538	0.0	0.0010	0.0001	0.4344
	12	21	21	21	21	21
P_200	0.44226	0.07030	0.66525	1.00000	0.25789	0.78597
	0.1500	0.7620	0.0010	0.0	0.2591	0.0001
	12	21	21	21	21	21
M_200	0.43973	0.37509	0.89292	0.25789	1.00000	-0.24075
	0.1526	0.0938	0.0001	0.2591	0.0	0.2931
	12	21	21	21	21	21

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	SUL	MG	T_ACID	P_200	M_200	RATIO
RATIO	0.32463	-0.01538	0.18020	0.78597	-0.24075	1.00000
	0.3032	0.9472	0.4344	0.0001	0.2931	0.0
	12	21	21	21	21	21

B. Dolomite

The SAS System

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Correlation Analysis

12 'VAR' Variables: IFV PV WI AB SPG LA SUL MG  
 T\_ACID P\_200 M\_200 RATIO

Simple Statistics

Variable	N	Mean	Std Dev	Sum	Minimum	Maximum
IFV	20	43.550000	2.321637	871.000000	40.000000	47.600000
PV	20	28.495000	3.140311	569.900000	23.600000	35.300000
WI	20	15.055000	1.748225	301.100000	10.800000	18.300000
AB	19	2.406316	1.289350	45.720000	0.790000	6.250000
SPG	19	2.603316	0.080683	49.463000	2.390000	2.732000
LA	19	28.899474	3.317582	549.090000	22.470000	36.980000
SUL	18	4.666111	3.642939	83.990000	0.310000	13.180000
MG	17	12.070588	0.894961	205.200000	10.100000	13.100000
T_ACID	20	3.816500	2.957099	76.330000	0.080000	9.820000
P_200	20	1.583000	1.534666	31.660000	0.020000	4.450000
M_200	20	2.233500	1.812452	44.670000	0.060000	6.280000
RATIO	20	0.785000	0.733983	15.700000	0.030000	2.560000

The SAS System

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IFV	PV	WI	AB	SPG	LA
IFV	1.00000 0.0 20	0.83636 0.0001 20	-0.17435 0.4622 20	0.65025 0.0026 19	-0.67087 0.0017 19	0.13232 0.5892 19
PV	0.83636 0.0001 20	1.00000 0.0 20	-0.68560 0.0008 20	0.76523 0.0001 19	-0.74493 0.0003 19	0.10716 0.6624 19
WI	-0.17435 0.4622 20	-0.68560 0.0008 20	1.00000 0.0 20	-0.53805 0.0175 19	0.46240 0.0462 19	-0.00849 0.9725 19
AB	0.65025 0.0026 19	0.76523 0.0001 19	-0.53805 0.0175 19	1.00000 0.0 19	-0.96817 0.0001 19	0.42739 0.0680 19
SPG	-0.67087	-0.74493	0.46240	-0.96817	1.00000	-0.52870

	0.0017	0.0003	0.0462	0.0001	0.0	0.0200
	19	19	19	19	19	19
LA	0.13232	0.10716	-0.00849	0.42739	-0.52870	1.00000
	0.5892	0.6624	0.9725	0.0680	0.0200	0.0
	19	19	19	19	19	19
SUL	0.44441	0.60340	-0.56742	0.59233	-0.58818	-0.03362
	0.0646	0.0080	0.0140	0.0096	0.0102	0.8946
	18	18	18	18	18	18
MG	-0.34540	-0.65270	0.75604	-0.31927	0.29564	0.41488
	0.1745	0.0045	0.0004	0.2281	0.2663	0.1101
	17	17	17	16	16	16
T_ACID	0.41405	0.44825	-0.25533	0.00920	-0.05644	-0.20460
	0.0695	0.0475	0.2773	0.9702	0.8185	0.4008
	20	20	20	19	19	19
P_200	0.15757	0.20234	-0.15420	-0.21238	0.19434	-0.28123
	0.5070	0.3923	0.5163	0.3827	0.4253	0.2435
	20	20	20	19	19	19
M_200	0.54152	0.55958	-0.28601	0.20439	-0.26904	-0.09973
	0.0137	0.0103	0.2215	0.4013	0.2654	0.6846
	20	20	20	19	19	19

The SAS System  
Correlation Analysis

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Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IFV	PV	WI	AB	SPG	LA
RATIO	-0.42429	-0.24888	-0.11639	-0.43279	0.48014	-0.30987
	0.0623	0.2900	0.6251	0.0642	0.0375	0.1967
	20	20	20	19	19	19

The SAS System  
Correlation Analysis

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Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	SUL	MG	T_ACID	P_200	M_200	RATIO
IFV	0.44441	-0.34540	0.41405	0.15757	0.54152	-0.42429
	0.0646	0.1745	0.0695	0.5070	0.0137	0.0623
	18	17	20	20	20	20
PV	0.60340	-0.65270	0.44825	0.20234	0.55958	-0.24888
	0.0080	0.0045	0.0475	0.3923	0.0103	0.2900
	18	17	20	20	20	20
WI	-0.56742	0.75604	-0.25533	-0.15420	-0.28601	-0.11639



	0.0140	0.0004	0.2773	0.5163	0.2215	0.6251
	18	17	20	20	20	20
AB	0.59233	-0.31927	0.00920	-0.21238	0.20439	-0.43279
	0.0096	0.2281	0.9702	0.3827	0.4013	0.0642
	18	16	19	19	19	19
SPG	-0.58818	0.29564	-0.05644	0.19434	-0.26904	0.48014
	0.0102	0.2663	0.8185	0.4253	0.2654	0.0375
	18	16	19	19	19	19
LA	-0.03362	0.41488	-0.20460	-0.28123	-0.09973	-0.30987
	0.8946	0.1101	0.4008	0.2435	0.6846	0.1967
	18	16	19	19	19	19
SUL	1.00000	-0.76433	0.41806	0.22345	0.51016	-0.03427
	0.0	0.0009	0.0843	0.3728	0.0305	0.8926
	18	15	18	18	18	18
MG	-0.76433	1.00000	-0.63132	-0.45573	-0.65587	-0.12587
	0.0009	0.0	0.0066	0.0660	0.0043	0.6303
	15	17	17	17	17	17
T_ACID	0.41806	-0.63132	1.00000	0.86083	0.90262	0.20251
	0.0843	0.0066	0.0	0.0001	0.0001	0.3918
	18	17	20	20	20	20
P_200	0.22345	-0.45573	0.86083	1.00000	0.55796	0.54759
	0.3728	0.0660	0.0001	0.0	0.0106	0.0124
	18	17	20	20	20	20
M_200	0.51016	-0.65587	0.90262	0.55796	1.00000	-0.13280
	0.0305	0.0043	0.0001	0.0106	0.0	0.5767
	18	17	20	20	20	20

The SAS System  
Correlation Analysis

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Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	SUL	MG	T_ACID	P_200	M_200	RATIO
RATIO	-0.03427	-0.12587	0.20251	0.54759		
					20	20

Correlation Analysis

13 'VAR' Variables: IFV PV WI AB SPG LA FH IG  
 META LS DOL OS CRUSH

Simple Statistics

Variable	N	Mean	Std Dev	Sum	Minimum	Maximum
IFV	6	38.833333	3.222215	233.000000	35.300000	44.100000
PV	6	24.166667	1.490861	145.000000	22.600000	26.300000
WI	6	14.666667	2.136040	88.000000	12.100000	17.800000
AB	6	1.795000	0.749286	10.770000	1.210000	3.210000
SPG	6	2.608500	0.058722	15.651000	2.509000	2.676000
LA	6	25.383333	2.370660	152.300000	22.640000	28.000000
FH	5	5.950000	1.896800	29.750000	4.490000	9.180000
IG	6	29.833333	13.071292	179.000000	9.000000	49.200000
META	6	8.766667	5.663097	52.600000	3.600000	18.700000
LS	6	15.666667	5.225578	94.000000	6.200000	20.800000
DOL	6	17.733333	13.641359	106.400000	0.400000	40.400000
OS	6	28.000000	11.805084	168.000000	16.000000	46.900000
CRUSH	6	61.166667	35.594475	367.000000	10.000000	99.000000

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IFV	PV	WI	AB	SPG	LA	FH
IFV	1.00000 0.0 6	0.83710 0.0376 6	0.92424 0.0084 6	0.89316 0.0165 6	-0.37249 0.4671 6	0.56327 0.2445 6	0.84567 0.0711 5
PV	0.83710 0.0376 6	1.00000 0.0 6	0.56481 0.2429 6	0.83235 0.0398 6	-0.57341 0.2342 6	0.71429 0.1108 6	0.86769 0.0566 5
WI	0.92424 0.0084 6	0.56481 0.2429 6	1.00000 0.0 6	0.76638 0.0755 6	-0.16168 0.7596 6	0.35114 0.4949 6	0.72715 0.1639 5
AB	0.89316 0.0165 6	0.83235 0.0398 6	0.76638 0.0755 6	1.00000 0.0 6	-0.47803 0.3376 6	0.74468 0.0895 6	0.94441 0.0156 5

SPG	-0.37249 0.4671 6	-0.57341 0.2342 6	-0.16168 0.7596 6	-0.47803 0.3376 6	1.00000 0.0 6	-0.89321 0.0165 6	-0.36448 0.5464 5
LA	0.56327 0.2445 6	0.71429 0.1108 6	0.35114 0.4949 6	0.74468 0.0895 6	-0.89321 0.0165 6	1.00000 0.0 6	0.58090 0.3044 5
FH	0.84567 0.0711 5	0.86769 0.0566 5	0.72715 0.1639 5	0.94441 0.0156 5	-0.36448 0.5464 5	0.58090 0.3044 5	1.00000 0.0 5
IG	-0.17777 0.7362 6	0.08156 0.8779 6	-0.32509 0.5295 6	-0.20945 0.6904 6	-0.01175 0.9824 6	0.11203 0.8327 6	-0.34218 0.5730 5
META	0.50567 0.3061 6	0.83013 0.0408 6	0.18341 0.7280 6	0.46638 0.3511 6	-0.76446 0.0767 6	0.65743 0.1559 6	0.67684 0.2095 5
LS	-0.03104 0.9535 6	-0.32877 0.5246 6	0.18264 0.7291 6	0.06569 0.9016 6	0.48772 0.3264 6	-0.13925 0.7925 6	0.14109 0.8210 5
DOL	-0.00599 0.9910 6	-0.49449 0.3187 6	0.33610 0.5148 6	-0.00440 0.9934 6	0.49003 0.3238 6	-0.40093 0.4308 6	0.14509 0.8159 5

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IFV	PV	WI	AB	SPG	LA	FH
OS	-0.02508 0.9624 6	0.22841 0.6633 6	-0.19725 0.7080 6	-0.01580 0.9763 6	-0.40241 0.4290 6	0.08551 0.8721 6	0.08337 0.8940 5
CRUSH	0.89014 0.0174 6	0.80440 0.0536 6	0.78135 0.0665 6	0.71799 0.1081 6	-0.62133 0.1879 6	0.67435 0.1418 6	0.59219 0.2927 5

The SAS System

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Correlation Analysis

Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IG	META	LS	DOL	OS	CRUSH
IFV	-0.17777 0.7362 6	0.50567 0.3061 6	-0.03104 0.9535 6	-0.00599 0.9910 6	-0.02508 0.9624 6	0.89014 0.0174 6

PV	0.08156 0.8779 6	0.83013 0.0408 6	-0.32877 0.5246 6	-0.49449 0.3187 6	0.22841 0.6633 6	0.80440 0.0536 6
WI	-0.32509 0.5295 6	0.18341 0.7280 6	0.18264 0.7291 6	0.33610 0.5148 6	-0.19725 0.7080 6	0.78135 0.0665 6
AB	-0.20945 0.6904 6	0.46638 0.3511 6	0.06569 0.9016 6	-0.00440 0.9934 6	-0.01580 0.9763 6	0.71799 0.1081 6
SPG	-0.01175 0.9824 6	-0.76446 0.0767 6	0.48772 0.3264 6	0.49003 0.3238 6	-0.40241 0.4290 6	-0.62133 0.1879 6
LA	0.11203 0.8327 6	0.65743 0.1559 6	-0.13925 0.7925 6	-0.40093 0.4308 6	0.08551 0.8721 6	0.67435 0.1418 6
FH	-0.34218 0.5730 5	0.67684 0.2095 5	0.14109 0.8210 5	0.14509 0.8159 5	0.08337 0.8940 5	0.59219 0.2927 5
IG	1.00000 0.0 6	0.12984 0.8063 6	0.21019 0.6894 6	-0.67089 0.1446 6	-0.48734 0.3269 6	0.03468 0.9480 6
META	0.12984 0.8063 6	1.00000 0.0 6	-0.74232 0.0910 6	-0.75791 0.0808 6	0.58091 0.2266 6	0.68077 0.1366 6
LS	0.21019 0.6894 6	-0.74232 0.0910 6	1.00000 0.0 6	0.52881 0.2807 6	-0.93035 0.0071 6	-0.22179 0.6728 6
DOL	-0.67089 0.1446 6	-0.75791 0.0808 6	0.52881 0.2807 6	1.00000 0.0 6	-0.28319 0.5866 6	-0.26614 0.6102 6

The SAS System  
Correlation Analysis

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Pearson Correlation Coefficients / Prob > |R| under Ho: Rho=0 / Number of Observations

	IG	META	LS	DOL	OS	CRUSH
OS	-0.48734 0.3269 6	0.58091 0.2266 6	-0.93035 0.0071 6	-0.28319 0.5866 6	1.00000 0.0 6	0.04074 0.9389 6
CRUSH	0.03468 0.9480 6	0.68077 0.1366 6	-0.22179 0.6728 6	-0.26614 0.6102 6	0.04074 0.9389 6	1.00000 0.0 6

The CORR Procedure

11 Variables: IFV PV WI Ab SpG LA Mg T\_acid  
 P\_200 M\_200 ratio

Simple Statistics

Variable	N	Mean	Std Dev	Sum	Minimum	Maximum
IFV	41	43.30073	2.48135	1775	38.60000	48.87000
PV	41	26.58561	3.29672	1090	20.87000	35.30000
WI	41	16.71512	2.88721	685.32000	10.46000	22.14000
Ab	39	1.93923	1.06677	75.63000	0.78000	6.25000
SpG	39	2.61646	0.06348	102.04200	2.39000	2.73200
LA	38	28.54553	4.05989	1085	21.37000	38.17000
Mg	38	7.00395	5.17147	266.15000	0.50000	13.10000
T_acid	41	4.45659	2.85252	182.72000	0.08000	12.72000
P_200	41	1.44122	1.38434	59.09000	0.02000	4.83000
M_200	41	3.01537	2.07290	123.63000	0.06000	10.32000
ratio	41	0.58927	0.62330	24.16000	0.03000	2.56000

Pearson Correlation Coefficients  
 Prob > |r| under H0: Rho=0  
 Number of Observations

	IFV	PV	WI	Ab	SpG	LA
IFV	1.00000	0.53112	0.25297	0.41339	-0.46565	0.17859
		0.0004	0.1105	0.0089	0.0028	0.2834
	41	41	41	39	39	38
PV	0.53112	1.00000	-0.68538	0.66386	-0.55598	0.00014
	0.0004		<.0001	<.0001	0.0002	0.9993
	41	41	41	39	39	38
WI	0.25297	-0.68538	1.00000	-0.39155	0.22039	0.15935
	0.1105	<.0001		0.0137	0.1776	0.3393
	41	41	41	39	39	38
Ab	0.41339	0.66386	-0.39155	1.00000	-0.90864	0.26977
	0.0089	<.0001	0.0137		<.0001	0.1014
	39	39	39	39	39	38
SpG	-0.46565	-0.55598	0.22039	-0.90864	1.00000	-0.41367
	0.0028	0.0002	0.1776	<.0001		0.0098

	39	39	39	39	39	38
LA	0.17859	0.00014	0.15935	0.26977	-0.41367	1.00000
	0.2834	0.9993	0.3393	0.1014	0.0098	
	38	38	38	38	38	38

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The CORR Procedure  
Pearson Correlation Coefficients  
Prob > |r| under H0: Rho=0  
Number of Observations

	IFV	PV	WI	Ab	SpG	LA
Mg	0.11727	0.49694	-0.47838	0.41896	-0.16108	-0.05381
	0.4832	0.0015	0.0024	0.0110	0.3480	0.7588
	38	38	38	36	36	35
T_acid	0.12364	0.31281	-0.25092	-0.09470	0.04607	-0.28739
	0.4412	0.0465	0.1136	0.5663	0.7806	0.0802
	41	41	41	39	39	38
P_200	0.11289	0.31226	-0.25954	-0.09748	0.07465	-0.29027
	0.4822	0.0469	0.1013	0.5549	0.6515	0.0771
	41	41	41	39	39	38
M_200	0.09451	0.22174	-0.17196	-0.06607	0.01405	-0.20493
	0.5567	0.1635	0.2823	0.6894	0.9324	0.2171
	41	41	41	39	39	38
ratio	-0.23506	0.08068	-0.29414	-0.14606	0.22075	-0.25449
	0.1390	0.6161	0.0619	0.3749	0.1769	0.1231
	41	41	41	39	39	38

Pearson Correlation Coefficients  
Prob > |r| under H0: Rho=0  
Number of Observations

	Mg	T_acid	P_200	M_200	ratio
IFV	0.11727	0.12364	0.11289	0.09451	-0.23506
	0.4832	0.4412	0.4822	0.5567	0.1390
	38	41	41	41	41
PV	0.49694	0.31281	0.31226	0.22174	0.08068
	0.0015	0.0465	0.0469	0.1635	0.6161
	38	41	41	41	41
WI	-0.47838	-0.25092	-0.25954	-0.17196	-0.29414
	0.0024	0.1136	0.1013	0.2823	0.0619
	38	41	41	41	41

Ab	0.41896	-0.09470	-0.09748	-0.06607	-0.14606
	0.0110	0.5663	0.5549	0.6894	0.3749
	36	39	39	39	39

The SAS System  
The CORR Procedure  
Pearson Correlation Coefficients  
Prob > |r| under H0: Rho=0  
Number of Observations

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	Mg	T_acid	P_200	M_200	ratio
SpG	-0.16108	0.04607	0.07465	0.01405	0.22075
	0.3480	0.7806	0.6515	0.9324	0.1769
	36	39	39	39	39
LA	-0.05381	-0.28739	-0.29027	-0.20493	-0.25449
	0.7588	0.0802	0.0771	0.2171	0.1231
	35	38	38	38	38
Mg	1.00000	-0.17779	0.00316	-0.24490	0.20456
		0.2856	0.9850	0.1384	0.2180
	38	38	38	38	38
T_acid	-0.17779	1.00000	0.72877	0.88939	0.10784
	0.2856		<.0001	<.0001	0.5021
	38	41	41	41	41
P_200	0.00316	0.72877	1.00000	0.33514	0.61886
	0.9850	<.0001		0.0322	<.0001
	38	41	41	41	41
M_200	-0.24490	0.88939	0.33514	1.00000	-0.26468
	0.1384	<.0001	0.0322		0.0945
	38	41	41	41	41
ratio	0.20456	0.10784	0.61886	-0.26468	1.00000
	0.2180	0.5021	<.0001	0.0945	
	38	41	41	41	41

Appendix 3. Multiple Regression Analysis  
 A. Limestone

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The REG Procedure  
 Model: MODEL1  
 Dependent Variable: PV

Stepwise Selection: Step 1

Variable tacid Entered: R-Square = 0.4954 and C(p) = 0.1066

Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	1	52.01760	52.01760	17.67	0.0005
Error	18	52.98750	2.94375		
Corrected Total	19	105.00510			

Variable	Parameter Estimate	Standard Error	Type II SS	F Value	Pr > F
Intercept	21.70062	0.82472	2038.11814	692.35	<.0001
tacid	0.60292	0.14343	52.01760	17.67	0.0005

Bounds on condition number: 1, 1

-----  
 All variables left in the model are significant at the 0.1500 level.

The SAS System  
 The REG Procedure  
 Model: MODEL1  
 Dependent Variable: PV

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Stepwise Selection: Step 1

Variable tacid Entered: R-Square = 0.4954 and C(p) = 0.1066

Analysis of Variance



Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	1	52.01760	52.01760	17.67	0.0005
Error	18	52.98750	2.94375		
Corrected Total	19	105.00510			

Variable	Parameter Estimate	Standard Error	Type III SS	F Value	Pr > F
Intercept	21.70062	0.82472	2038.11814	692.35	<.0001
tacid	0.60292	0.14343	52.01760	17.67	0.0005

Bounds on condition number: 1, 1

All variables left in the model are significant at the 0.1500 level.

No other variable met the 0.1500 significance level for entry into the model.

Summary of Stepwise Selection

Step	Variable Entered	Variable Removed	Number Vars In	Partial R-Square	Model R-Square	C(p)	F Value	Pr > F
1	tacid		1	0.4954	0.4954	0.1066	17.67	0.0005

The SAS System

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Obs	IFV	PV	WI	ab	spg	la	sul	mg	tacid	p200	m200	ratio
1	38.60	28.14	10.46	2.170	2.591	28.500	.	5.40	12.72	2.40	10.32	0.23
2	40.20	24.72	15.48	1.780	2.586	27.000	22.25	0.70	5.76	3.64	2.12	1.72
3	44.38	22.69	21.69	2.000	2.604	21.370	7.98	9.50	4.09	2.04	2.05	1.00
4	43.14	23.49	19.65	1.030	2.633	32.090	.	3.60	3.37	0.24	3.13	0.08
5	39.48	21.67	17.81	0.930	2.652	28.600	.	2.20	4.96	1.53	3.43	0.45
6	43.00	20.87	22.13	1.350	2.631	37.230	10.92	0.60	1.97	0.15	1.82	0.08
7	41.13	25.50	15.63	1.260	2.624	.	16.62	0.90	4.54	2.36	2.18	1.08
8	41.76	24.97	16.79	1.150	2.653	26.490	27.46	2.50	6.00	0.70	5.30	0.13
9	42.26	27.40	14.86	2.065	2.642	25.935	15.43	9.05	6.38	0.30	6.08	0.05
10	45.30	25.39	19.91	2.330	2.621	23.030	19.47	10.00	6.17	1.80	4.37	0.41
11	43.68	25.39	18.29	0.780	2.698	32.240	7.46	4.60	5.14	0.86	4.28	0.20
12	46.69	27.51	19.18	1.005	2.670	25.730	.	2.30	6.04	0.60	5.44	0.11
13	48.87	29.24	19.63	1.050	2.639	27.720	.	1.90	11.21	4.83	6.38	0.76

14	44.10	25.00	19.10	1.530	2.626	28.490	8.06	1.60	4.13	1.43	2.70	0.53
15	43.21	27.12	16.09	0.880	2.638	22.400	.	1.10	4.41	1.86	2.55	0.73
16	41.02	24.72	16.30	.	.	.	.	1.10	4.59	1.16	3.43	0.34
17	47.85	25.73	22.12	2.470	2.517	38.170	3.01	0.50	1.41	0.14	1.27	0.11
18	44.69	22.55	22.14	1.970	2.592	34.760	.	0.70	2.28	0.12	2.16	0.06
19	42.37	23.95	18.42	1.320	2.634	24.790	11.30	0.70	4.01	0.64	3.37	0.19
20	40.10	22.05	18.05	1.150	2.689	24.390	.	1.20	3.61	0.47	3.14	0.15
21	42.50	22.01	20.49	1.690	2.639	26.705	5.38	1.00	3.60	0.16	3.44	0.05

Stepwise Procedure for Dependent Variable PV

The first 3 variables are forced into the model because of the INCLUDE= option.

Step 0 The First 3 Vars Entered R-square = 0.49883949 C(p) = 4.00000000

	DF	Sum of Squares	Mean Square	F	Prob>F
Regression	3	52.38068754	17.46022918	5.31	0.0099
Error	16	52.62440746	3.28902547		
Total	19	105.00509500			

Variable	Parameter Estimate	Standard Error	Type II Sum of Squares	F	Prob>F
INTERCEP	21.33741392	1.51508421	652.34487988	198.34	0.0001
AB	0.27327327	0.87869865	0.31811314	0.10	0.7598
MG	-0.03757113	0.15667370	0.18914010	0.06	0.8135
TACID	0.61607810	0.16170862	47.73890224	14.51	0.0015

Bounds on condition number: 1.370403, 11.21282

All variables left in the model are required or significant at the 0.1500 level.  
 No other variable met the 0.0500 significance level for entry into the model.

The REG Procedure  
 Model: MODEL1  
 Dependent Variable: PV

Stepwise Selection: Step 1

Variable ab Entered: R-Square = 0.6025 and C(p) = 9.5580

Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	1	101.59776	101.59776	21.22	0.0004
Error	14	67.03162	4.78797		
Corrected Total	15	168.62937			

Variable	Parameter Estimate	Standard Error	Type II SS	F Value	Pr > F
Intercept	23.76289	1.12548	2134.40654	445.79	<.0001
ab	1.87322	0.40665	101.59776	21.22	0.0004

Bounds on condition number: 1, 1

Stepwise Selection: Step 2

Variable mg Entered: R-Square = 0.7467 and C(p) = 3.7385

Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	2	125.91126	62.95563	19.16	0.0001
Error	13	42.71812	3.28601		
Corrected Total	15	168.62937			

The REG Procedure  
 Model: MODEL1  
 Dependent Variable: PV

Stepwise Selection: Step 2

Variable	Parameter Estimate	Standard Error	Type III SS	F Value	Pr > F
Intercept	46.03310	8.24011	102.55174	31.21	<.0001
ab	1.56450	0.35549	63.64479	19.37	0.0007
mg	-1.76512	0.64891	24.31350	7.40	0.0175

Bounds on condition number: 1.1135, 4.454

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 All variables left in the model are significant at the 0.1500 level.

No other variable met the 0.1500 significance level for entry into the model.

Summary of Stepwise Selection

Step	Variable Entered	Variable Removed	Number Vars In	Partial R-Square	Model R-Square	C(p)	F Value	Pr > F
1	ab		1	0.6025	0.6025	9.5580	21.22	0.0004
2	mg		2	0.1442	0.7467	3.7385	7.40	0.0175

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Obs	IFV	PV	WI	ab	spg	la	sul	mg	tacid	p200	m200	ratio
1	40.0	24.9	15.1	0.79	2.732	25.53	0.36	12.4	1.70	1.23	0.48	2.56
2	43.3	25.0	18.3	2.39	2.599	28.59	.	12.9	0.71	0.02	0.69	0.03
3	41.8	27.3	14.5	2.64	2.605	27.05	9.57	11.3	3.13	2.07	1.06	1.95
4	46.9	35.3	11.6	6.25	2.390	29.36	9.86	11.3	2.11	0.11	2.00	0.06
5	44.0	27.7	16.3	2.02	2.616	29.19	7.30	11.6	9.82	3.55	6.28	0.57
6	42.7	26.8	15.9	1.56	2.626	26.62	2.24	12.4	5.29	1.79	3.50	0.51
7	44.0	28.8	15.2	3.05	2.583	32.43	4.79	12.9	1.34	0.21	1.13	0.19
8	44.6	28.7	15.9	2.38	2.594	31.73	0.76	13.1	0.22	0.03	0.20	0.15
9	44.8	31.0	13.8	1.21	2.671	22.47	5.59	10.7	7.42	4.40	3.02	1.46
10	45.9	31.3	14.6	3.76	2.489	36.98	3.29	12.6	4.16	1.81	2.35	0.77
11	46.2	30.0	16.2	2.02	2.632	24.75	5.99	12.2	6.31	2.71	3.59	0.75
12	47.6	31.3	16.3	2.74	2.588	29.50	2.24	11.9	7.65	3.39	4.26	0.80

13	43.5	32.7	10.8	.	.	.	.	10.1	5.56	0.78	4.78	0.16
14	46.7	32.0	14.7	4.00	2.480	30.28	13.18	11.2	5.27	0.71	4.55	0.16
15	40.5	24.1	16.4	1.03	2.669	29.76	2.56	12.8	0.50	0.27	0.22	1.23
16	40.1	23.6	16.5	1.86	2.614	32.62	0.55	12.9	0.08	0.02	0.06	0.33
17	41.3	24.9	16.4	1.00	2.718	25.69	0.31	12.9	0.35	0.03	0.32	0.09
18	43.6	28.4	15.2	2.48	2.619	27.25	3.62	.	2.15	0.94	1.21	0.78
19	41.6	28.0	13.6	1.59	2.648	30.75	6.45	.	6.66	4.45	2.21	2.01
20	41.9	28.1	13.8	2.95	2.590	28.54	5.33	.	5.90	3.14	2.76	1.14

Stepwise Procedure for Dependent Variable PV

The first 3 variables are forced into the model because of the INCLUDE= option.

Step 0 The First 3 Vars Entered R-squarr = 0.77873139 C(p) = 4.00000000

	DF	Sum of Squares	Mean Square	F	Prob>F
Regression	3	131.31698690	43.77232897	14.08	0.0003
Error	12	37.31238810	3.10936568		
Total	15	168.62937500			

Variable	Parameter Estimate	Standard Error	Type II Sum of Squares	F	Prob>F
INTERCEP	38.02318391	10.05750671	44.44143753	14.29	0.0026
AB	1.55678486	0.34585261	63.00092053	20.26	0.0007
MG	-1.17495766	0.77381513	7.16872294	2.31	0.1548
M200	0.39500808	0.29958135	5.40572821	1.74	0.2119

Bounds on condition number: 1.673367, 13.06769

All variables left in the model are required or significant at the 0.1500 level.  
 No other the 0.0500 significance level for entry into the model.

The REG Procedure

Model: MODEL1

Dependent Variable: PV

Stepwise Selection: Step 1

Variable ab Entered: R-Square = 0.4427 and C(p) = 23.2884

Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	1	170.02372	170.02372	27.01	<.0001
Error	34	214.01375	6.29452		
Corrected Total	35	384.03747			

Variable	Parameter Estimate	Standard Error	Type II SS	F Value	Pr > F
Intercept	22.49064	0.84984	4408.51711	700.37	<.0001
ab	2.01759	0.38820	170.02372	27.01	<.0001

Bounds on condition number: 1, 1

Stepwise Selection: Step 2

Variable tacid Entered: R-Square = 0.5875 and C(p) = 10.9254

Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	2	225.62107	112.81054	23.50	<.0001
Error	33	158.41640	4.80050		
Corrected Total	35	384.03747			

The REG Procedure  
 Model: MODEL1  
 Dependent Variable: PV

## Stepwise Selection: Step 2

Variable	Parameter Estimate	Standard Error	Type II SS	F Value	Pr > F
Intercept	20.42284	0.95916	2176.36962	453.36	<.0001
ab	2.12667	0.34053	187.23176	39.00	<.0001
tacid	0.42415	0.12463	55.59735	11.58	0.0018

Bounds on condition number: 1.0089, 4.0358

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Stepwise Selection: Step 3  
 Variable mg2 Entered: R-Square = 0.6831 and C(p) = 3.4406

## Analysis of Variance

Source	DF	Sum of Squares	Mean Square	F Value	Pr > F
Model	3	262.33526	87.44509	22.99	<.0001
Error	32	121.70222	3.80319		
Corrected Total	35	384.03747			

Variable	Parameter Estimate	Standard Error	Type II SS	F Value	Pr > F
Intercept	19.54640	0.89913	1797.35912	472.59	<.0001
ab	1.71285	0.33107	101.79938	26.77	<.0001
mg2	0.01636	0.00526	36.71418	9.65	0.0039
tacid	0.51888	0.11505	77.36076	20.34	<.0001

Bounds on condition number: 1:294, 10.749

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All variables left in the model are significant at the 0.1500 level.

No other variable met the 0.1500 significance level for entry into the model.

The REG Procedure  
 Model: MODEL1  
 Dependent Variable: PV

Summary of Stepwise Selection

Step	Variable Entered	Variable Removed	Number Vars In	Partial R-Square	Model R-Square	C(p)	F Value	Pr > F
1	ab		1	0.4427	0.4427	23.2884	27.01	<.0001
2	tacid		2	0.1448	0.5875	10.9254	11.58	0.0018
3	mg2		3	0.0956	0.6831	3.4406	9.65	0.0039

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Obs	IFV	PV	WI	ab	spg	la	sul	mg	mg2	tacid	p200	m200	ratio
1	40.00	24.90	15.10	0.790	2.732	25.530	0.36	12.40	153.76	1.70	1.23	0.48	2.56
2	43.30	25.00	18.30	2.390	2.599	28.590	.	12.90	166.41	0.71	0.02	0.69	0.03
3	41.80	27.30	14.50	2.640	2.605	27.050	9.57	11.30	127.69	3.13	2.07	1.06	1.95
4	46.90	35.30	11.60	6.250	2.390	29.360	9.86	11.30	127.69	2.11	0.11	2.00	0.06
5	44.00	27.70	16.30	2.020	2.616	29.190	7.30	11.60	134.56	9.82	3.55	6.28	0.57
6	42.70	26.80	15.90	1.560	2.626	26.620	2.24	12.40	153.76	5.29	1.79	3.50	0.51
7	44.00	28.80	15.20	3.050	2.583	32.430	4.79	12.90	166.41	1.34	0.21	1.13	0.19
8	44.60	28.70	15.90	2.380	2.594	31.730	0.76	13.10	171.61	0.22	0.03	0.20	0.15
9	44.80	31.00	13.80	1.210	2.671	22.470	5.59	10.70	114.49	7.42	4.40	3.02	1.46
10	45.90	31.30	14.60	3.760	2.489	36.980	3.29	12.60	158.76	4.16	1.81	2.35	0.77
11	46.20	30.00	16.20	2.020	2.632	24.750	5.99	12.20	148.84	6.31	2.71	3.59	0.75
12	47.60	31.30	16.30	2.740	2.588	29.500	2.24	11.90	141.61	7.65	3.39	4.26	0.80
13	43.50	32.70	10.80	.	.	.	.	10.10	102.01	5.56	0.78	4.78	0.16
14	46.70	32.00	14.70	4.000	2.480	30.280	13.18	11.20	125.44	5.27	0.71	4.55	0.16
15	40.50	24.10	16.40	1.030	2.669	29.760	2.56	12.80	163.84	0.50	0.27	0.22	1.23
16	40.10	23.60	16.50	1.860	2.614	32.620	0.55	12.90	166.41	0.08	0.02	0.06	0.33
17	41.30	24.90	16.40	1.000	2.718	25.690	0.31	12.90	166.41	0.35	0.03	0.32	0.09
18	43.60	28.40	15.20	2.480	2.619	27.250	3.62	.	.	2.15	0.94	1.21	0.78
19	41.60	28.00	13.60	1.590	2.648	30.750	6.45	.	.	6.66	4.45	2.21	2.01
20	41.90	28.10	13.80	2.950	2.590	28.540	5.33	.	.	5.90	3.14	2.76	1.14
21	38.60	28.14	10.46	2.170	2.591	28.500	.	5.40	29.16	12.72	2.40	10.32	0.23
22	40.20	24.72	15.48	1.780	2.586	27.000	22.25	0.70	0.49	5.76	3.64	2.12	1.72
23	44.38	22.69	21.69	2.000	2.604	21.370	7.98	9.50	90.25	4.09	2.04	2.05	1.00
24	43.14	23.49	19.65	1.030	2.633	32.090	.	3.60	12.96	3.37	0.24	3.13	0.08
25	39.48	21.67	17.81	0.930	2.652	28.600	.	2.20	4.84	4.96	1.53	3.43	0.45
26	43.00	20.87	22.13	1.350	2.631	37.230	10.92	0.60	0.36	1.97	0.15	1.82	0.08



27	41.13	25.50	15.63	1.260	2.624	.	16.62	0.90	0.81	4.54	2.36	2.18	1.08
28	41.76	24.97	16.79	1.150	2.653	26.490	27.46	2.50	6.25	6.00	0.70	5.30	0.13
29	42.26	27.40	14.86	2.065	2.642	25.935	15.43	9.05	81.90	6.38	0.30	6.08	0.05
30	45.30	25.39	19.91	2.330	2.621	23.030	19.47	10.00	100.00	6.17	1.80	4.37	0.41
31	43.68	25.39	18.29	0.780	2.698	32.240	7.46	4.40	19.36	5.14	0.86	4.28	0.20
32	46.69	27.51	19.18	1.005	2.670	25.730	.	2.30	5.29	6.04	0.60	5.44	0.11
33	48.87	29.24	19.63	1.050	2.639	27.720	.	1.90	3.61	11.21	4.83	6.38	0.76
34	44.10	25.00	19.10	1.530	2.626	28.490	8.06	1.60	2.56	4.13	1.43	2.70	0.53
35	43.21	27.12	16.09	0.880	2.638	22.400	.	1.10	1.21	4.41	1.86	2.55	0.73
36	41.02	24.72	16.30	.	.	.	.	1.10	1.21	4.59	1.16	3.43	0.34
37	47.85	25.73	22.12	2.470	2.517	38.170	3.01	0.50	0.25	1.41	0.14	1.27	0.11
38	44.69	22.55	22.14	1.970	2.592	34.760	.	0.70	0.49	2.28	0.12	2.16	0.06
39	42.37	23.95	18.42	1.320	2.634	24.790	11.30	0.70	0.49	4.01	0.64	3.37	0.19
40	40.10	22.05	18.05	1.150	2.689	24.390	.	1.20	1.44	3.61	0.47	3.14	0.15
41	42.50	22.01	20.49	1.690	2.639	26.705	5.38	1.00	1.00	3.60	0.16	3.44	0.05

Stepwise Procedure for Dependent Variable PV

Step 1 Variable AB Entered R-square = 0.44272690 C(p) = 22.86726163

	DF	Sum of Squares	Mean Square	F	Prob>F
Regression	1	170.02372250	170.02372250	27.01	0.0001
Error	34	214.01375250	6.29452213		
Total	35	384.03747500			

Variable	Parameter Estimate	Standard Error	Type II Sum of Squares	F	Prob>F
INTERCEP	22.49064418	0.84983969	4408.51710725	700.37	0.0001
AB	2.01758941	0.38820347	170.02372250	27.01	0.0001

Bounds on condition number: 1, 1

Step 2 Variable TACID Entered R-square = 0.58749755 C(p) = 10.61362418

	DF	Sum of Squares	Mean Square	F	Prob>F
Regression	2	225.62107410	112.81053705	23.50	0.0001
Error	33	158.41640090	4.80049700		
Total	35	384.03747500			

Variable	Parameter Estimate	Standard Error	Type II Sum of Squares	F	Prob>F
INTERCEP	20.42283904	0.95916404	2176.36961585	453.36	0.0001
AB	2.12667074	0.34052860	187.23176227	39.00	0.0001
TACID	0.42415378	0.12463482	55.59735160	11.58	0.0018

Bounds on condition number: 1.008939, 4.035756

Step 3 Variable MG Entered R-square = 0.67498398 C(p) = 4.00000000

	DF	Sum of Squares	Mean Square	F	Prob>F
Regression	3	259.21914296	86.40638099	22.15	0.0001
Error	32	124.81833204	3.90057288		
Total	35	384.03747500			

Variable	Parameter Estimate	Standard Error	Type II Sum of Squares	F	Prob>F
INTERCEP	19.46590784	0.92403421	1731.01816500	443.79	0.0001
AB	1.72124710	0.33660619	101.99306429	26.15	0.0001
MG	0.21049031	0.07171981	33.59806886	8.61	0.0061
TACID	0.47863961	0.11387029	68.91675437	17.67	0.0002

Bounds on condition number: 1.246012, 10.48733

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 All variables left in the model are significant at the 0.1500 level.  
 No other variable met the 0.1500 significance level for entry into the model.

Summary of Stepwise Procedure for Dependent Variable PV

Step	Variable Entered	Variable Removed	Number In	Partial R**2	Model R**2	C(p)	F	Prob>F
1	AB		1	0.4427	0.4427	22.8673	27.0114	0.0001
2	TACID		2	0.1448	0.5875	10.6136	11.5816	0.0018
3	MG		3	0.0875	0.6750	4.0000	8.6136	0.0061