

1 **Forensic engineering analysis applied to flood control: The Great Karun basin flood of**

2 **2019**

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6 **Abstract**

7 Flooding can have various impacts, including loss of life and damage to property. Flood-  
8 management reservoirs can help mitigate floods, but their operation can also worsen flood impacts.

9 This paper presents a novel forensic engineering approach to assess the role of reservoir operation  
10 on flood control. Fourteen criteria are employed for assessing forecast-based prereleases of water  
11 from reservoir storage to reduce the impact of flooding. The proposed approach is applied to assess  
12 the performance of a system of reservoirs during the large flood of 2019 in southwestern Iran (the  
13 Great Karun Basin). The two main study areas are in the sub-basins of Karun and Dez. The Karun  
14 sub-basin includes five reservoirs, which are Karun 4, Karun 3, Karun 1, Masjed-Soleiman, and  
15 Gotvand (from upstream to downstream). The Dez sub-basin includes two reservoirs, Rudbar-

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16 Lorestan and Dez. Results concerning two key performance criteria (the Peak Discharge Reduction  
17 (PDR) and Flood Volume Reduction (FVR)) show that the PDR criterion in the Karun sub-basin  
18 multi-reservoir system reached about 79% (where 100% is the theoretically best performance)  
19 under the historical scenario (actual operating conditions in 2019) and improved from 8 to 19%  
20 for various prerelease operations. The FVR achieved about 33% in the historical situation and  
21 improved from 20 to 59% for prerelease operations scenarios, respectively. The PDR criterion  
22 achieved 26% under the historical scenario, but with better operation could exceed 55% in the Dez  
23 sub-basin multi-reservoir system, whereas FVR was as low as 11% but can be raised to between  
24 15 and 25% under prerelease operations. This work's calculated scenarios' criteria values establish  
25 that improved reservoir operation could be achieved by applying specialized operation approaches.  
26 **Keywords:** Forensic engineering, Flood events, Reservoirs. Flood Criteria, Flood management.

## 27 **1. Introduction**

28 Floods inflict recurring damages the world over with far-reaching consequences in terms of  
29 losses of property and life (Pham, 2011). Various flood control models and methods have been  
30 proposed including optimization, prediction, and uncertainty analysis (e.g. Qiu et al., 2010; Wang  
31 et al., 2012; Woodward et al., 2014; Shao et al., 2017; Volpi et al. 2018; Kundzewicz et al., 2019;  
32 Leandro et al., 2020). There are also, structural flood-control methods, such as the construction  
33 and operation of reservoirs (Gomez-Ullate et al., 2010; 2011; Zhao et al., 2014; Chen et al., 2015  
34 and 2020). Reservoirs play an important role in the planning and management of water resources,  
35 especially in arid and semi-arid regions. Real-time operation of multi-reservoir is central to flood  
36 control and management (e.g. Kuo et al., 1990; Mesbah et al., 2009; Liu et al., 2011 and 2017; Wu  
37 and Chen, 2013; Ming et al., 2017; Huang et al., 2018).

38 Flood control in reservoir operation is affected by many factors, so the judicious operation of  
39 reservoirs is difficult and necessary during a flood event. The operation of a flood control reservoir  
40 is normally accomplished using specific operating rules and policies, which involves guidelines  
41 for water-release decision making under various conditions (Liu et al., 2015a, b; Zhou et al., 2015a,  
42 b). Flood control has two main simultaneous objectives: to prevent flood damage downstream of  
43 reservoirs, and to ensure dam safety. Accordingly, releases are limited by the maximum allowable  
44 safe discharge to downstream channels and rivers. Moreover, flood forecasts provide information  
45 about future streamflow and are vital in operating a flood control reservoir (e.g. Windsor, 1973;  
46 Reddy and Kumar, 2006; Wei and Hsu, 2008; Zhu et al., 2017a, b; Wallington et al., 2020).

47 Qi et al. (2017) developed a preference-based multi-objective optimization model for reservoir  
48 flood control operation. Their model took water demand into consideration while optimizing two  
49 conflicting flood control objectives, namely, minimizing the highest upstream water level (to  
50 guarantee the safety of the upstream side) and minimizing the largest water release volume (to  
51 protect the downstream side). The schedules obtained by their model could significantly reduce  
52 the flood peak and guarantee reservoir safety. Liu et al. (2017) developed a multi-objective flood  
53 control and hydropower generation operation model for Three Gorges Reservoirs in China. Results  
54 showed that the use of spillways would have a significant impact on reservoir operation in flood  
55 conditions. As a result, it is necessary to consider the number and order of spillways which should  
56 be operated. The latter authors concluded that the application of the Smooth Support Vector  
57 Machine (SSVM) model could have twofold benefits by reducing flood risk and increasing  
58 hydropower generation during the flood seasons. Huang et al. (2018) proposed a stochastic copula-  
59 based simulation method accounting for flood forecasting uncertainty at the Three Gorges

60 Reservoirs (TGR) in China. Results demonstrated that the entropy method was effective for  
61 evaluating flood risk due to different uncertainties.

62 Zhang et al. (2019) developed a two-stage flood risk analysis model in multi-reservoir systems  
63 to evaluate uncertainty in flood forecast by dividing the operation horizon into beyond-forecast  
64 time period and forecast lead-time. They concluded that hydropower generation could increase  
65 during the summer flood season without increasing the flood risk in the multi-reservoir system.

66 Despite advances in flood management there are systematic errors ( e.g., faults in the functions  
67 of gates and spillways, incorrect streamflow predictions) and human errors (i.e., no water  
68 prerelease because of socio-political and other issues) in the operation of reservoirs. Also, it is  
69 important to assess the reservoir operators' ability to make optimal decisions under emergency  
70 flood conditions. Forensic engineering has made substantial contributions in recent decades to the  
71 identification and study of failure causes, their mechanisms and progression in buildings, complex  
72 facilities, etc (e.g., Carper, 2000 and Noon, 2001). Forensic hydrology has emerged in recent years  
73 to discern the causes and processes of hydrologic events causing economic and life losses  
74 (Loáiciga, 2001; Hurst, 2007; Lischeid et al., 2017). Generally, forensic hydrology studies  
75 extremes such as floods and droughts and their impacts, water-quality degradation, and the causes  
76 of adverse groundwater phenomena. Forensic hydrology is a part of Forensic Disaster Analyses  
77 (FDA) (Keating et al., 2016).

78 For example, Loáiciga (2001) demonstrated that flood damages caused in San Luis Obispo  
79 County near Avila Beach, California, in 1995 were not due to extreme rainfall, but, rather, to  
80 progressive changes made to streams and flood plains over many years. Such changes required  
81 higher water levels to pass the design floods than those predicted before the changes, thus leading  
82 to the submergence and collapse of buildings. Bronstert et al. (2018) provided forensic hydrologic

83 analysis of the hydrological consequences of the Braunsbach flash flood in 2016. The results  
84 showed that the flood event was due to a very rare rainfall intensity, which, in combination with  
85 catchment properties, led to extreme runoff coupled with severe geomorphological hazard.  
86 Bronstert et al. (2018) determined that due to the complex and interacting processes no single flood  
87 could be identified for the severe damage that occurred, while the interaction and cascading  
88 characteristics led to such an event.

89 Many published studies have dealt with several aspects of flood control by reservoirs (e.g.  
90 Marien, 1984; Tung et al. 2006; Zhou, 2010; Li et al., 2010; Yan et al., 2014; Chen et al., 2019;  
91 Jing et al., 2020). However, studies considering how forensic engineering can be used to improve  
92 the operation of flood reservoirs and how best to conduct these forensic investigations are rare.  
93 This study's contributions are (1) developing applicable criteria to guide forensic engineering  
94 assessments of reservoirs' flood control performance during flood events under diverse managing  
95 scenarios, (2) developing pre-release prediction-based scenarios for the severe 2019 flood event in  
96 southwestern Iran, which is this work's case study. The 2019 flood event raised the question of  
97 whether the reservoirs in the flood region were operated properly. This work evaluates the  
98 reservoir operators' performance by means of forensic engineering.

## 99 **2. Methods**

100 The operation of multi-reservoir systems is a complex task, especially during flood events. In  
101 the case of reservoirs in series the downstream reservoirs are directly affected by water releases  
102 from upstream reservoirs. The releases of water from reservoirs in parallel may converge  
103 downstream in which case they may cause serious damages. The complexities of multi-reservoir  
104 configurations require that forensic engineering analyses be performed for the reservoirs  
105 individually and as a system to evaluate the sub-basin and basin storage-release performance. Both

106 quantitative and qualitative criteria are required to evaluate single- and multi-reservoir systems  
107 operation performance under flood conditions. A criterion must be defined for each managerial  
108 aspect or reservoir function to evaluate the reservoir or multi-reservoir system performance  
109 concerning the defined functions.

## 110 **2.1. Flood Control Policy (FCP)**

111 Each basin may be divided into several sub-basins. The sub-basins may or may not have  
112 reservoir(s) in them. The operation of each reservoir affects the operation of downstream  
113 reservoirs, and may also affect the performance of reservoirs in other sub-basin(s). These  
114 interrelated impacts may cause both positive and negative effects on the downstream flood  
115 situation. For example, in a flood situation, each reservoir can prevent damages by means of  
116 prereleases of water, whereas it can also cause otherwise preventable damages via its operation.  
117 This highlights the importance of forensic engineering investigations in assessing reservoir  
118 operation during historical flood situations.

119 Reservoir inflows and outflows generally change over time and space. Inflows, which either  
120 originate from the associate watershed or are combined with the releases from upstream reservoirs,  
121 are regulated by reservoirs to reduce downstream flood damages. Water is often released from  
122 reservoirs before a flood event to create additional storage capacity for flood control. This is called  
123 a prerelease. During flood periods reservoir releases are managed so that excess water is stored to  
124 help meet water demands during subsequent low-inflow periods and to prevent downstream  
125 flooding. The flood volume may become so large that reservoir releases may reach their maximum  
126 magnitudes thus endangering the spillway and dam integrity. .

127 Reservoir flood simulation may be expressed in terms of a series of water balance equations.  
128 Equation (1) represents the change of storage in reservoir  $i$  during period  $t$  ( $S_{i,t}$ ) :

$$\Delta S_{i,t} = S_{i,t+1} - S_{i,t} \quad i=1,2,\dots,N \text{ and } t=1,2,\dots,T \quad (1)$$

129 where  $N$  denotes the total number of reservoirs in a multi-reservoir system;  $t$  = operation day index;  
 130  $T$  = total days in the operation period;  $S$  = reservoir storage. When reservoir releases are controlled  
 131 through several gates the water balance equation takes the following form:

$$S_{i,t+1} = S_{i,t} + Q_{i,t} + Q'_{i,t} - E_{i,t} - L_{i,t} + \sum_{j=1}^m R_{i,j,t} + SP_{i,t} \quad (2)$$

$$E_{i,t} = \left[ \frac{A_{i,t} + A_{i,t+1}}{2} \right] Ev_{i,t} \quad (3)$$

$$A_{i,t} = f_i(S_{i,t}) \quad (4)$$

$$A_{i,t+1} = f_i(S_{i,t+1}) \quad (5)$$

$$0 \leq S_i^{Min} \leq S_{i,t} \leq S_i^{Max} \quad (6)$$

$$0 \leq R_{i,j}^{Min} \leq R_{i,j,t} \leq R_{i,j}^{Max} \quad (7)$$

$$0 \leq Sp_i^{Min} \leq Sp_{i,t} \leq Sp_i^{Max} \quad (8)$$

132 where  $j = 1, 2, \dots, m$  denotes the number of gates;  $E$  = the volume of water loss or gain due to the  
 133 difference between reservoir evaporation and precipitation;  $A$  = the reservoir water surface area;  
 134  $R$  = the released volume of water from the reservoir except the spill;  $Q$  and  $Q'$  denote  
 135 respectively natural reservoir inflow and releases from upstream reservoir and return flows which  
 136 indicates upstream non-regulated flows (such as middle basin runoff);  $Sp$  = the volume of spilled  
 137 water from the reservoir;  $S^{Min}$  = the minimum operating volume;  $S^{Max}$  = the maximum operating  
 138 volume;  $R^{Min}$  = is the minimum allowable release volume;  $R^{max}$  = is the maximum allowable

139 release volume;  $S_p^{Min}$  = is the minimum allowable spill volume;  $S_p^{Max}$  = is the maximum allowable  
140 spill volume;  $E_v$  = the difference between the evaporation and precipitation rates.

141 The integrated operation of a multi-reservoir system is essential for successful flood control  
142 during floods. Reservoirs built along a river's main reach constitute a system of cascade lakes, or  
143 reservoirs in series. In this case, their operation must be carried out jointly because of the effect of  
144 upstream reservoirs' releases on downstream reservoirs. The total inflow into the downstream  
145 reservoir is a combination of releases and spills from an upstream reservoir and the natural inflows  
146 generated downstream of reservoirs. The downstream reservoirs must be operated based on the  
147 total inflow. Reservoirs built on different branches of a river are said to be in parallel. The  
148 operation of parallel-reservoir subsystems may or may not have to be carried out jointly with  
149 respect to flood control depending on the locations of vulnerable areas. Figure 1 shows a schematic  
150 of reservoirs. Reservoir 1 and 2 are in series above the point of confluence, and so are reservoirs  
151 3, 4. The subsystems (1,2) and (3,4) are in parallel. Reservoir 5 is affected by the operation of all  
152 reservoirs. Reservoir 5 is in series with respect to subsystems (1,2) and (3,4). Area A is impacted  
153 by the operation of reservoir 3 and 4. Area B is influenced by the operation of all reservoirs.

## 154 **2.2. Criteria Development**

155 This paper's purpose is to perform a forensic analysis of the performance of reservoir operations  
156 under severe flood conditions. It is, therefore, necessary to develop quantitative criteria to evaluate  
157 performance at the local or single-reservoir level and the global or multi-reservoir-system level.  
158 The performance evaluation of a single reservoir is conducted assuming that downstream  
159 reservoirs receive inflows that are not regulated. In other words, the effects of the upstream  
160 reservoirs are not considered in the single-reservoir performance evaluation.



161 The criteria development accounts for the main characteristics of floods, such as inflow and  
 162 outflow flood volumes, the inflow and outflow peak discharges, and the safe downstream  
 163 discharge, which defines the Maximum Allowable Discharge (MAD) from a reservoir. The  
 164 following criteria were developed to simplify the forensic-engineering assessments in evaluating  
 165 the performance of reservoirs' operators under flood conditions:

166 1- Peak Discharge Reduction (PDR) of a single reservoir:

$$I1_i = \left[ 1 - \frac{\frac{\text{Max}(Q_{i,t}^{Out})}{T}}{\frac{\text{Max}(Q_{i,t}^{In})}{T}} \right] \times 100 \quad i = 1, 2, \dots, N \quad (9)$$

$$Q_{i,t}^{Out} = R_{i,j,t} + SP_{i,t} \quad (10)$$

167 in which  $I1_i$  =PDR criterion for reservoir  $i$ ;  $Q_{i,t}^{In}$  and  $Q_{i,t}^{Out}$  denote the reservoir inflow and outflow  
 168 in day  $t$ , respectively.

169 2- Peak Discharge Reduction (PDR) of multi-reservoir systems:

$$I2 = \left[ 1 - \frac{\frac{\text{Max}(Q_i^{Out})}{T}}{\frac{\text{Max}(QB_i^{In})}{T}} \right] \times 100 \quad (11)$$

$$QB = \begin{cases} \text{For } i = 1 & QB_1 = Q_1 \\ \text{For } i = 2 & QB_2 = Q_2 + QB_1 \\ \text{For } i = 3 & QB_3 = Q_3 + QB_2 \\ \vdots & \vdots \\ \text{For } i = N & QB_N = Q_N + QB_{N-1} \end{cases} \quad (12)$$

170 in which  $I2$  = PDR of multi-reservoirs system criterion;  $QB_i^{In}$  is the non-regulated inflow of the  
 171 flooded basin (the downstream reservoir of the basin) in day  $t$ , respectively.

172 3- Flood Volume Reduction (FVR) of a single reservoir:

$$I3_i = \left[ 1 - \frac{\sum_{t=1}^T Q_{it}^{Out}}{\sum_{t=1}^T Q_{it}^{In}} \right] \cdot 100 \quad i = 1, 2, \dots, N \quad (13)$$

173 in which  $I3_i =$  FVR of reservoir  $i$ .

174 4- Flood Volume Reduction (FVR) of multi-reservoir systems:

$$I4 = \left[ 1 - \frac{\sum_{t=1}^T Q_t^{Out}}{\sum_{t=1}^T QB_t^{In}} \right] \cdot 100 \quad (14)$$

175 in which  $I4 =$  FVR of multi-reservoir system.

176 5- Peak Flow Delay (PFD) of a single reservoir:

$$I5_i = D \left( \text{Max} \left( Q_{i,t}^{Out} \right)_{t=1}^T \right) - D \left( \text{Max} \left( Q_{i,t}^{In} \right)_{t=1}^T \right) \quad i = 1, 2, \dots, N \quad (15)$$

177 in which  $I5_i =$  PFD criterion in reservoir  $i$ ;  $D \left( \text{Max} \left( Q_{i,t}^{Out} \right)_{t=1}^T \right)$  and  $D \left( \text{Max} \left( Q_{i,t}^{In} \right)_{t=1}^T \right)$  are the peak

178 discharge occurrence time of the inflow and outflow of reservoir  $i$ , respectively.

179 6- Peak Flow Delay (PFD) of multi-reservoir systems:

$$I6 = D \left( \text{Max} \left( Q_{i,t}^{Out} \right)_{t=1}^T \right) - D \left( \text{Max} \left( QB_{i,t}^{In} \right)_{t=1}^T \right) \quad (16)$$

180 in which  $I6 =$  PFD criterion of the multi-reservoirs system;  $D \left( \text{Max} \left( Q_{i,t}^{Out} \right)_{t=1}^T \right)$  and  $D \left( \text{Max} \left( QB_{i,t}^{In} \right)_{t=1}^T \right)$

181 are the peak discharge occurrence time of the inflow and outflow of the flooded basin (the

182 downstream reservoir of the basin), respectively.

183 7- Flood Control Readiness (FCR) of a single reservoir:

$$I7_i = \left[ \frac{S_{i,0}^{Empty}}{\Delta T \sum_{t=1}^T Q_{i,t}^{In}} \right] \times 100 \quad i = 1, 2, \dots, N \quad (17)$$

184 in which  $I7_i$  = FCR criterion of the reservoir  $i$ ;  $S_{i,0}^{Empty}$  = the empty volume of reservoir  $i$ , in day 0  
 185 (the day preceding the flood occurrence).

186 8- Flood Control Readiness (FCR) of multi-reservoir systems:

$$I8 = \left[ \frac{SB_0^{Empty}}{\Delta T \sum_{t=1}^T QB_t^{In}} \right] \times 100 \quad (18)$$

187 in which  $I8$  = FCR criterion of the multi-reservoirs system;  $SB_0^{Empty}$  = the total empty volume of  
 188 the multi-reservoir system in the day preceding the flood occurrence.

189 Reservoir operation must be planned in such a way that reservoir safety is assured and water-  
 190 supply targets (such as meeting water demands and non-violation of the MAD) are met. Just as  
 191 reservoir safety is important for operators, so is the outflow volume, flow, and timing for  
 192 stakeholders and downstream residents. This study selects the MAD as the main target because  
 193 the violation of this parameter could result in reservoir and downstream destruction and damages.  
 194 MAD-based criteria are also herein developed to analyze the performance of reservoir operators  
 195 in terms of the number of MAD violations, their severity, and the time to return to desirable  
 196 operation following violations.

197 The reliability of the system indicates the level of the system's ability to meet acceptable targets  
 198 and is calculated for any time period including the flood duration and also longer periods extend  
 199 to the entire operation period of a reservoir system. The reliability criterion does not provide any  
 200 information about the rate of return to a satisfactory state in the event of a failure. Also, reliability

201 does not measure the severity of a failure. Criteria such as vulnerability and resiliency are used to  
 202 quantify the severity of failures and the system's ability to return to a satisfactory state following  
 203 a system failure to perform adequately, respectively (Bozorg-haddad, 2018). Any operational  
 204 period in which reservoir releases exceed the MAD is considered as a failure period in this work.  
 205 Otherwise, it is considered as a normal period. Therefore, reservoir operation as envisioned in this  
 206 work aims to ensure that all outflows do not exceed the MAD to prevent flood damages.

207 9- Reliability of avoiding downstream damage of single reservoirs:

$$I9_i = 1 - f_i / T \quad i = 1, 2, \dots, N \quad (19)$$

208 in which  $I9_i$  = reliability of no downstream damage criterion of reservoir  $i$   $f_i$  = the number of  
 209 failure days which is calculated as follows:

$$f_i = \sum_{t=1}^T a_{i,t}, \quad a_{i,t} = \begin{cases} 1 & \sum_{j=1}^m R_{i,j,t} + Sp_{i,t} > MAD_i \\ 0 & \sum_{j=1}^m R_{i,j,t} + Sp_{i,t} \leq MAD_i \end{cases} \quad i = 1, 2, \dots, N \quad (20)$$

210 in which  $MAD_i$  = maximum allowable discharge to river downstream of reservoir  $i$ .

211 10- Reliability of no downstream damage in multi-reservoir systems:

212 This is calculated as follows:

$$I10 = 1 - f_B / T, \quad f_B = \begin{cases} 1 & \sum_{i=1}^N f_i \geq 1 \\ 0 & \sum_{i=1}^N f_i < 1 \end{cases} \quad (21)$$

213 in which means that the multi-reservoir system would incur a failure whenever one or more of its  
 214 components incur failure. Failure occurs whenever the system does not have sufficient capacity to  
 215 meet the desired goals.

216 11- Resiliency to downstream damage of a single-reservoir system

$$I11_i = \frac{1}{\left(\frac{f_i}{fs_i}\right)} \quad i = 1, 2, \dots, N \quad (22)$$

217 in which  $I11_i$  = resiliency to downstream damage criterion of reservoir  $i$  and  $fs_i$  = number of  
218 continuous failure days.

219 The system resiliency criterion is the probability that a reservoir system returns to a normal state  
220 after a failure state. The higher the resiliency of a system, the greater it is the capacity to cope with  
221 changes in the factors affecting that system.

222 12- Resiliency to downstream damage of multi-reservoir systems:

$$I12 = \frac{1}{\left(\frac{f_B}{fs_B}\right)} \quad (23)$$

223 in which  $I12$  = resiliency to downstream damage criterion of multi-reservoirs systems and  $fs_B$  =  
224 number of continuous failure days of the multi-reservoir system. The definition of failure in the  
225 context of the resiliency of a multi-reservoir system is such that failure by one or more reservoirs  
226 means system failure, also. Is

227 13- Vulnerability to downstream damage of single reservoirs:

$$I13_i = \frac{\text{Max}\left(\sum_{t=1}^T \left(\sum_{j=1}^m R_{i,j,t} + Sp_{i,t} - MAD_i\right), 0\right)}{MAD_i} \quad (24)$$

228 in which  $I13_i$  = vulnerability to downstream damage criterion of reservoir  $i$ . Vulnerability  
229 measures the difference between the normal and the failure states of reservoirs; it is, therefore, a

230 measure of the severity of the failure, and it is a probabilistic criterion. The lower the vulnerability,  
231 the greater is the capacity to maintain satisfactory operating conditions.

232 14- Vulnerability to downstream damage of a multi-reservoir system:

$$I14 = \frac{\text{Max}_{t=1}^T \left( \sum_{i=1}^N \text{Max}_{j=1}^m (R_{i,j,t} + Sp_{i,t} - MAD_i, 0) \right)}{\sum_{i=1}^N MAD_i} \quad (25)$$

233 in which  $I14$ =vulnerability to downstream damage criterion of multi-reservoir systems.

## 234 2.2. Prerelease scenarios

235 Operation of a single-reservoir or multi-reservoir systems during floods is beset by multiple  
236 complexities. Evaluating the operation of multi-reservoir systems requires simulating system  
237 operation with observed data and under new scenarios (i.e., “unseen data”). These scenarios are  
238 intended to demonstrate if a system’s operation could have been improved by prerelease of water  
239 in a timely manner. Therefore, this work analyses various prerelease scenarios to assess the  
240 performance of reservoir systems’ operation.

### 241 • Using short-term forecasting models in reservoir operation

242 In recent years technology and models have been developed to forecast runoff during flood  
243 events. This relies on scenarios developed based on one-week and two-week flood predictions  
244 (these time periods will give enough time for operators to make decisions about timing and  
245 magnitude of releases from reservoirs), which is one of the forensic engineering methods to assess  
246 the possibility of improved operation relying on this type of predictions.

### 247 • Ideal Reservoir Operation

248 Forensic engineering approach involves the evaluation of the historical operation of reservoirs  
249 by comparing it with a defined ideal practical operation. The ideal operation is simulated based on  
250 having perfect foresight. Reservoir inflows (one and two months before the flood) can be

251 forecasted using regression methods or other data mining methods (such as neural networks) based  
252 on monthly long-time discharge series. The model's accuracy generally increases with the length  
253 and quality of the time series. It should be noted that, depending on the any reservoir's capacity  
254 and also its downstream MAD, the predictions lead time could be changed and so in this study,  
255 one- and two-months periods, is herein considered as an ideal foresight lead time. Ideal reservoir  
256 operation must be such that reservoir storage does not exceed the maximum allowable storage (this  
257 ensures dam safety) and the reservoir outflow (release plus spill) does not cause downstream  
258 damage during flood events.

### 259 **3- Case Study**

260 The Great Karun basin was chosen as a case study to illustrate this paper's methodology. The  
261 basin is located in southwestern Iran and covers about 4.2% of the total area of the country. Great  
262 Karun consists of two sub-basins, which are (1) the Karun sub-basin, and (2) the Dez sub-basin.  
263 The Karun River (Iran's largest) drains the basin and it is a key element of Iran' water resources.  
264 Many regions of southwestern Iran meet their agricultural, industrial, domestic, and environmental  
265 demands from reservoirs built on the Karun River. Droughts and floods have a significant impact  
266 on the Great Karun basin water use. Floods constitute a hazard to life and property in the basin.  
267 Figure 2 shows five reservoirs in the Karun sub-basin which from upstream to downstream are:  
268 (1) Karun 4, (2) Karun 3, (3) Karun 1, (4) Masjed-Soleiman, and (5) Gotvand. The Dez sub-basin  
269 features two reservoirs, which are: (1) Rudbar-Lorestan (upstream), and (2) Dez (downstream).  
270 Outflows from the Gotvand and Dez reservoirs converge at Bande-Ghir and flow to Ahwaz City.  
271 The operation of the two downstream reservoirs must be coordinated to provide flood protection  
272 to Ahwaz City. The reservoirs' characteristics are listed in Table 1.

273 This work assesses the 2019 flood event in southwestern Iran (Great Karun Basin) using the  
274 forensic engineering approach herein developed. The 2019 flood is one of three major floods in  
275 the past 70 years in the Great Karun basin. The flood began on March 23<sup>rd</sup> and ended on April 3<sup>rd</sup>.  
276 It caused severe economic and human losses. The forensic assessment of the 2019 flood evaluates  
277 the performance of reservoir operation in the study area and analyses the periods immediately  
278 before, during, and immediately after the flood. The "before flood" period starts on September 23,  
279 2018, and ends on March 22, 2019 (180 days); The "during flood" period starts on March 23, 2019,  
280 and ends on April 3, 2019 (13 days), and the "post-flood" period starts on April 4, 2019, and ends  
281 on April 19, 2019 (16 days).

#### 282 **4. Results and discussion**

283 The 2019 flood caused losses of life and properties in the Great Karun basin, for this reason this  
284 paper's forensic analysis of reservoirs operations takes heightened relevance to avoid future losses.  
285 This paper evaluates 14 quantitative criteria (Eqs 9-25) to assess operation performance of an  
286 individual reservoir and a multi-reservoir system under several prerelease scenarios. The pre-  
287 release scenarios cover one-week and two-week prereleases. The ideal scenario was developed  
288 based on runoff prediction with a lead time of one and two months.

##### 289 **4.1. Scenario 1**

290 This scenario was developed using short-term prediction models for reservoir operation. This  
291 means that the forensic analysis assumes that reservoir operators can utilize the inflow predictions  
292 up to two weeks in advance of the flood event. Thus, all reservoirs were allowed to pre-empty and  
293 release the maximum allowable water without endangering the reservoir dam structure or  
294 downstream areas. Based on Scenario 1 the prerelease of all reservoirs in the two sub-basins started



295 two weeks before the flood (March 11, 2019). The specification of the prerelease flows and other  
296 details are listed in Table 2 and Table 3 for the Karun and Dez sub-basins, respectively.

#### 297 **4.1.1. The Karun Sub-basin**

298 This sub-basin includes five reservoirs (upstream to downstream): (1) Karun 4, (2) Karun 3, (3)  
299 Karun 1, (4) Masjed-Soleiman, and (5) Gotvand. For Karun 4, the maximum inflow during the  
300 flood was 2,546 m<sup>3</sup>/s, while the peak outflow discharge was 595 m<sup>3</sup>/s (Table 4). Under this  
301 scenario the Karun 4 reservoir attenuates the flood peak by about 77 %. However, this performance  
302 criterion achieved 66% under the historical scenario, i.e., the actual performance during the flood  
303 event. This means that under Scenario 1 Karun 4 Reservoir stored 47% of the flood volume, which  
304 is about 20% higher under the historical scenario (see Figure 3).

305 Based on scenario 1 Karun 3 had more than 870 x 10<sup>6</sup> m<sup>3</sup> of free storage space for flood control  
306 at the beginning of the flood event, which is equivalent to 36% of its capacity. Therefore, this  
307 reservoir managed to store 30% of the 2,445 x 10<sup>6</sup> m<sup>3</sup> of reservoir inflow and released about 1,718  
308 x 10<sup>6</sup> m<sup>3</sup>, which is far better than the 13% achieved under the historical scenario. The Karun 3  
309 reservoir reduces the flood peak discharge by 71%, which resulted in the inflow peak of 2,393  
310 m<sup>3</sup>/s being reduced to 686 m<sup>3</sup>/s. However, under the historical scenario, this reduction was only  
311 about 3% (see Figure 3).

312 Concerning the Karun 1 reservoir the calculated PDR criterion was about 30%, while under the  
313 historical scenario it achieved only 1% (Table 4). This means that the peak discharge decreases  
314 from 1,412 to 995 m<sup>3</sup>/s under scenario 1. As expected the PDR value for Karun 1 is lower  
315 compared to its upstream reservoirs, and the reason for this is that this reservoir stores the release  
316 discharge of upstream reservoirs (see Figure 3). Also, this reservoir stores about 13% of the flood  
317 volume, which is about 3% higher than the historical volume.

318 The main purpose of the Masjed-Soleiman reservoir is hydropower generation. Scenario 1  
319 assumes that its outflow equals its inflow, and, therefore, did not play any considerable role in  
320 reducing the flood volume or the discharge (see Table 4).

321 The Gotvand reservoir is the largest in the Karun basin. This reservoir attenuates the peak  
322 inflow by 73% (the inflow discharge decreases from 3,119 to 843 m<sup>3</sup>/s), compared with 47% under  
323 the historical scenario. Also, the achieved FVR criterion value is 22%, which is about 12% higher  
324 than its historical counterpart (Table 4). This means a reduction from 2,699 x 10<sup>6</sup> m<sup>3</sup> of inflow to  
325 2,112 x 10<sup>6</sup> m<sup>3</sup>. According to Scenario 1 the Gotvand reservoir had about 600 x 10<sup>6</sup> m<sup>3</sup> of free  
326 capacity for flood control just before the flood event (see Figure 3). Despite the presence of  
327 upstream reservoirs it released an outflow larger than the safe discharge under the historical  
328 scenario. This is a clearly undesirable situation that did not occur under the developed scenarios  
329 herein considered.

330 Concerning the evaluation of the multi-reservoir system (the basin-wide criteria) it was  
331 determined that the peak inflow discharge to the Karun sub-basin is 7,706 m<sup>3</sup>/s, which is reduced  
332 to 843 m<sup>3</sup>/s by the upstream reservoirs. This means an 89% attenuation of the peak discharge in  
333 the Karun basin, which is 10% more than the corresponding historical value. During the flood  
334 4,579 x 10<sup>6</sup> m<sup>3</sup> of water enters the Karun basin. Under Scenario 1 2,112 x 10<sup>6</sup> m<sup>3</sup> is released, and  
335 the rest is stored in the reservoir system. Therefore, 54% of the volume that enters the Karun Basin  
336 is stored in the reservoir system, which compares with 33% in the historical scenario. It is worth  
337 noting that under Scenario 1 the reservoir system attenuates the flood peak discharge during a  
338 single day (Figure 4).

#### 339 **4.1.2. The Dez sub-basin**

340 The Dez sub-basin includes Rudbar-Lorestan and Dez as its two main reservoirs in the upstream  
341 and downstream sections of basin, respectively. The flood readiness criterion for Rudbar-Lorestan  
342 reservoir is 31%, which is slightly higher than the historical value of 28% (see Table 5). Judging  
343 by the storage in the Rudbar-Lorestan reservoir compared with the Dez reservoir the prerelease of  
344 former during the pre-flood period did not make much difference to flood control readiness in this  
345 reservoir. However, during the flood event, the power plant was operating at half of its capacity  
346 with a steady discharge being released during 10 days. In this case, the FVR criterion for Rudbar-  
347 Lorestan reservoir reached 21%, which exceeds the historical state criterion of 14%. For Scenario  
348 1 the Rudbar-Lorestan reservoir did not have any significant releases in excess of the safe  
349 discharge and did not spill during the flood period. The reason for this is the effect of the prerelease  
350 policy (see Figure 5). Also, this reservoir performed the best in terms of reliability, resiliency and  
351 vulnerability to downstream damage criteria, which equaled 100%, 100%, and 0%, respectively.

352 The FCR criterion corresponding to the developed and historical scenarios for the Dez reservoir  
353 equal 21% and 16%, respectively (Table 5). The peak outflow discharge under Scenario 1 is 1,956  
354 m<sup>3</sup>/s, which is about 39% less than under the historical scenario. According to the FVR criterion,  
355 Dez reservoir stores 15% of the flood flow in the Dez Reservoir under Scenario 1, which was 10%  
356 under the historical scenario. Also, the vulnerability to the downstream damage criterion was about  
357 78%, which is about half of the value achieved under the historical scenario (see Figure 5).

358 , The results for the multi-reservoir system show that there is a similar trend for all developed  
359 criteria, whereby the PDR criterion by the reservoirs is equal to 55%. Thus, there is a significant  
360 effect of the prereleases in reducing the peak discharge. Also, the occurrence of the peak outflow  
361 discharge from the reservoir system is delayed by three days. The FVR criterion in the multi-

362 reservoir system is 17%, with most of the relief volume stored in the Dez reservoir and the rest in  
363 the Rudbar-Lorestan reservoir (Figure 6).

## 364 **4.2. Scenario 2**

365 This scenario on the of runoff predictions made one week before the flood. Therefore, the  
366 prerelease from all reservoirs of the Karun and Dez sub-basins begins two weeks before the flood  
367 (March 18, 2019). The scenario's specifications are listed in Tables 2 and 3.

### 368 **4.2.1. The Karun sub-basin**

369 This sub-basin includes five reservoirs, which from upstream to downstream are: (1) Karun 4, (2)  
370 Karun 3, (3) Karun 1, (4) Masjed-Soleiman and (5) Gotvand.

371 Under scenario 2 the Karun 4 reservoir reduces the inflow discharge from 2,546 m<sup>3</sup>/s to 595  
372 m<sup>3</sup>/s in the outflow, which is equivalent to 77% of the PDR (Table 4). Also, this reservoir releases  
373 only 53% of the inflow flood volume. At the time beginning of the flood the reservoir has ample  
374 storage capacity as its total active capacity of 834 x 10<sup>6</sup> m<sup>3</sup> provides an FCR of 47%, and uses the  
375 available storage to store the flood (Figure 7).

376 Concerning the evaluation of Karun 3 the results of Table 4 indicate the maximum inflow  
377 discharge of the Karun 3 during the flood equals 2,393 m<sup>3</sup>/s, while the peak outflow discharge is  
378 reduced to 686 m<sup>3</sup>/s. In other words, this reservoir reduces the flood peak by about 71%. This  
379 means that of the 2,444 x 10<sup>6</sup> m<sup>3</sup> of water entering the reservoir, about 30 % are stored and 1,718  
380 x 10<sup>6</sup> m<sup>3</sup> are released. It should be noted that Karun 3 reservoir operation under Scenario 2 at the  
381 beginning of the flood the readiness criterion is about 30% of the reservoir volume (see Figure 7).

382 The calculated criteria establish that at the beginning of the flood the Karun 1 had more than  
383 330 x 10<sup>6</sup> m<sup>3</sup> of empty volume for flood control, which was equivalent to 14% of its active capacity  
384 (Table 4). Due to the empty volume in the reservoir Karun 1 releases about 2,026 x 10<sup>6</sup> m<sup>3</sup> of the

385 2,366 x 10<sup>6</sup> m<sup>3</sup> of water entering the reservoir and stores the rest. The reservoir reduces the peak  
386 inflow discharge by 30 % which means that it reduces the peak inflow discharge from 1,412 m<sup>3</sup>/s  
387 to 987 m<sup>3</sup>/s in the outflow (see Figure 7).

388 It is seen in Figure 7 that the Masjed-Soleiman reservoir exhibits similar results as those of  
389 Scenario 1, which means that it does not play any role in reducing the flood volume or discharge  
390 (Table 4).

391 The PDR in the Gotvand Reservoir is about 67% (Table 4), which means the peak of discharge  
392 decreases from 3,119 to 1,027 m<sup>3</sup>/s (see Figure 7). Accordingly, the reservoir stores about 20% of  
393 the inflow flood volume and releases the rest of the inflow downstream. Also, it should be noted  
394 that the total spill volume from Gotvand during this period was about 41 x 10<sup>6</sup> m<sup>3</sup>.

395 With respect to the evaluation of the reservoir system it was calculated that the peak outflow  
396 discharge under this scenario was 1,027 m<sup>3</sup>/s, while the inflow peak was 7,706 m<sup>3</sup>/s. Therefore,  
397 the operation of the reservoir system under Scenario 2 reduces the peak discharge by 87%. During  
398 and after the flood 4,577 x 10<sup>6</sup> m<sup>3</sup> of water entered the Karun basin and 2,135 x 10<sup>6</sup> m<sup>3</sup> is released.  
399 The rest of the water is stored in the reservoirs, which amounts to about 53% of the total flood  
400 volume. It is worth noting that under Scenario 2 the reservoir system delays the peak flood  
401 discharge by 24 days (Figure 4).

#### 402 **4.2.2. The Dez sub-basin**

403 It is seen in Figure 8 that low inflow and the adequate volume of available storage compared to  
404 the flood volume in Rudbar-Lorestan lead to similar results under Scenarios 1 and 2 in terms of  
405 pre-flood performance. However, larger outflow under Scenario 2 causes the volume of Rudbar-  
406 Lorestan to be equal to the minimum operational volume. During the flood this reservoir stores

407 more water by releasing less than historical operation. Therefore, its FVR criterion is 21%, which  
408 is higher than the historical value (see Table 5).

409 Concerning the evaluation of the Dez reservoir operation it was determined that the volume of  
410 water released under Scenario 2 is larger than the historical value, and the FCR criterion under this  
411 scenario is about 19% (Table 5). It is worthy of notice that under Scenario 2 the peak outflow  
412 discharge is about 1,956 m<sup>3</sup>/s, but the value of this variable under historical operation was about  
413 3,226 m<sup>3</sup>/s, which means a reduction of flood damages (see Figure 8). This reduction demonstrates  
414 the positive effect of prereleases.

415 Overall, the Dez sub-basin under Scenario 2 exhibits similar results to those obtained under  
416 Scenario 1. This means that reservoir operators could reduce the flood peak by changing the release  
417 pattern and by keeping sufficient storage capacity to store floods in the reservoir system (see Figure  
418 6).

### 419 **4.3. Scenario 3 (Ideal Operation)**

420 This scenario was developed based on March and April reservoir inflow prediction using long-  
421 term inflow series and the data mining method Artificial Neural Network (ANN). The specification  
422 of the ANN model and prediction results are listed in Table 6. This scenario specifies that the  
423 reservoirs' initial volume must be at its minimum level if the volume of reservoir inflow in March  
424 and April is larger than reservoir capacity; otherwise, the reservoirs must have available storage  
425 capacity equal to the predicted volume of inflow. As a result, the reservoirs would be at their  
426 maximum operational level at the end of April.

#### 427 **4.3.1. The Karun sub-basin**

428 The rate of release from the reservoir reaches the maximum capacity of the power plant's  
429 tunnels (684 m<sup>3</sup>/s). With this volume of release the Karun 4 reservoir, the most upstream reservoir

430 in the Karun basin, would be empty at the beginning of the flood, and, therefore, would store a  
431 large flood volume. According to the calculated criteria for this reservoir the peak flow and volume  
432 reduction criteria of this reservoir are 66% and 46%, respectively (see Table 4). The reservoir also  
433 delays the peak discharge by 13 days. Due to the low storage volume of the reservoir on the day  
434 before the start of the flood (816 million cubic meters) the readiness for flood control for this  
435 reservoir is 46% (see Figure 9).

436 The Karun 3 reservoir reduced the inflow peak discharge of 2,165 m<sup>3</sup>/s to 947 m<sup>3</sup>/s in the  
437 outflow, which is 56% of the PDR criterion (see Figure 9). Also, in terms of reducing the volume  
438 of incoming floods into the reservoir Karun 3 releases only 42% of the total flood volume (see  
439 Table 4). At the time of the start of the flood the reservoir has 1,433 x 10<sup>6</sup> m<sup>3</sup> of available storage  
440 to control the flood which is used to store the flood waters.

441 The maximum inflow discharge into the Karun 1 reservoir during the flood is 1,220 m<sup>3</sup>/s, while  
442 the peak outflow discharge is reduced to 512 m<sup>3</sup>/s. In other words, the Karun 1 reservoir reduces  
443 the flood peak by about 58% (see Table 4). This means that of the 1,674 x 10<sup>6</sup> m<sup>3</sup> of water entering  
444 the reservoir about 62 % is stored, and 637 x 10<sup>6</sup> m<sup>3</sup> is released. As expected, this reservoir's  
445 performance is far better than the upstream reservoirs for flood control (see Figure 9).

446 It is seen in Figure 9 that the Masjed-Soleiman reservoir does not have any significant role in  
447 flood control under this scenario (see Table 4).

448 The calculated criteria calculated for evaluating the Gotvand reservoir (Table 4) establish that  
449 at the beginning of the flood the reservoir has an empty volume of about 900 x 10<sup>6</sup> m<sup>3</sup> to control  
450 the flood, which is about 30% of its active volume. Therefore, this reservoir releases about 358 x  
451 10<sup>6</sup> m<sup>3</sup>) of the inflow volume of 1286 x 10<sup>6</sup> m<sup>3</sup> and stores the rest. The reservoir also reduces the

452 peak food discharge by 91%, which means that it reduces the inflow peak discharge from 2,628  
453  $\text{m}^3/\text{s}$  to 224  $\text{m}^3/\text{s}$  in the outflow (see Figure 9).

454 The outflow peak discharge of the reservoir system under Scenario 3 is 224  $\text{m}^3/\text{s}$ , while the  
455 inflow peak discharge of the system is 7,706  $\text{m}^3/\text{s}$ . Therefore, reservoir system operation under  
456 Scenario 3 reduces the peak inflow discharge by 97% (Figure 4). During and after the flood 4,579  
457  $\times 10^6 \text{ m}^3$  of water entered the Karun sub-basin, about 359  $\times 10^6 \text{ m}^3$  is under ideal operation, and  
458 the rest, or 92%, is stored in the reservoir system (Figure 4).

459 Figure 4 compares the operation of the Karun sub-basin in each scenario. It is seen in Figure 4  
460 that the PDR criterion in this sub-basin in the historical scenario was about 80%, but ideally it  
461 could have improved up to 98%. Based on the FCR and FVR criteria the difference between the  
462 ideal and historical values increases, which means that it is possible to improve these criteria by  
463 about 50 and 60%, respectively. Therefore, it can be concluded that in this sub-basin it is possible  
464 to improve the criteria to a large extent with specialized operation.

#### 465 **4.3.2. The Dez sub-basin**

466 The Rudbar-Lorestan reservoir's peak inflow during the flood is 418  $\text{m}^3/\text{s}$ , while the peak  
467 outflow discharge is reduced to 237  $\text{m}^3/\text{s}$ . In other words, this reservoir reduces the flood peak by  
468 about 64% in discharge (see Table 5). This means that about 79 % of 382  $\times 10^6 \text{ m}^3$  of the water  
469 entering the reservoir is stored (see Figure 10).

470 According to Figure 10, the Dez reservoir cannot release as much as it does under the other  
471 scenarios due to its high inflows before the flood because its release is near the safe discharge.  
472 Under this scenario and starting prerelease on February 28, 2019, the PDR increases to 52%, and  
473 the FVR is about 23%, which yields better criteria in comparison to other scenarios (see Table 5).



474 Figure 6 compares the operation of the Dez sub-basin in each scenario, where it is seen that the  
475 PDR criterion in this sub-basin in the historical scenario was about 28%, but ideally it could have  
476 been improved by up to 56%. Based on the FCR and FVR criteria the difference between the ideal  
477 and historical values increased, which means that it is possible to improve these criteria by about  
478 10 and 12%, respectively. Therefore, it can be concluded that in this sub-basin it is possible to  
479 reduce the floods effects with specialized operation.

## 480 **5. Concluding Remarks**

481 Floods affect many parts of the world inflicting loss of property and life. Many approaches have  
482 been devised for flood control, and reservoirs represent one of the key structural measures. Historic  
483 reservoir operation for flood control can be assessed by forensic engineering and studied for  
484 making improvements to flood control operation planning.

485 This work developed 14 criteria and three prerelease scenarios to perform forensic engineering  
486 assessment of the 2019 flood. The main flood characteristics were considered in developing these  
487 criteria, including inflow and outflow flood volumes, inflow and outflow peak discharges, MAD,  
488 etc. These criteria quantify reservoir operation performance before, during, and after the flood  
489 event. Also, prerelease scenarios were based on realistic runoff predictions with a lead time of one  
490 and two weeks. Furthermore, an ideal scenario was considered with the lead times equal to one  
491 and two months depending on the both flood and reservoir capacity volumes. These scenarios  
492 assist forensic engineers in assessing reservoir operation and in comparing their performance with  
493 a defined ideal operation.

494 The results show that reservoirs in the Karun Sub-Basin reduce inflow peak discharge by 79 %.  
495 Also, the outflow flood volume is reduced by about 33% compared to the inflow flood volume in  
496 the reservoir system during the floods of April 2019. An evaluation of historical data concerning

497 the operation of the Karun Sub-Basin reservoirs shows that the reservoirs played a vital role in  
498 attenuating the flood hydrographs. Without the reservoirs system, the maximum daily inflow to  
499 the Gotvand would have been 7,706 m<sup>3</sup>/s, but with the reservoirs, the discharge peak was reduced  
500 to about 1,650 m<sup>3</sup>/s. The FCR criterion ranges between 53 and 57%, and under the historical  
501 scenario it equals 51%. The reliability of no downstream damage criterion under the prerelease  
502 scenarios equals 100%, and under the historical scenario is 79%. The vulnerability to downstream  
503 damage criterion under the prerelease scenarios is 0%, and under the historical scenario equals  
504 1%. The resiliency to downstream damage criterion is calculated as 100% under the prerelease  
505 scenarios and 33% under the historical scenario. Overall reservoir operators in the Karun Sub-  
506 Basin performed well in 2019. This work demonstrates it would have been possible to perform  
507 better with a more specialized approach.

508 The Dez sub-basin reservoirs feature a FCR criterion ranges between 22 and 24%, and it  
509 equaled 18% under the historical scenario. The reliability of no downstream damage criterion  
510 under prerelease scenarios is 14% and 21% under the historical scenario. The vulnerability to  
511 downstream damage criterion under prerelease scenarios was 55% and 136% under the historical  
512 scenario. The Resiliency to downstream damage criterion is 4% under the prerelease scenarios.  
513 The Dez Sub-Basin has high reservoir inflows in the pre-flood period, which made violation of the  
514 safe discharge inevitable. However, ideal reservoir operation reduces the size of this violation.  
515 Therefore, as it is obvious, with more specific operation, better performance is possible which  
516 could reduce the downstream damages and destructions and with developed criteria, managers  
517 could assess more easily and specifically the operators' performances in order to preventing future  
518 faults happenings.

519 **Conflict of Interests:**

520 None.

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524 **References:**

- 525 Bozorg-Haddad, O., (2018). "Water Resources Systems Optimization." Tehran university  
526 Publication, No.3, Tehran, Iran.
- 527 Bronstert, A., Agrawal, A., Boessenkool, B., Crisologo, I., Fischer, M., Heistermann, M., Köhn-  
528 Reich, L., López-Tarazón, J.A., Moran, T., Ozturk, U., Reinhardt-Imjela, C., and Wendi, D.  
529 (2018). "Forensic hydro-meteorological analysis of an extreme flash flood: The 2016-05-29  
530 event in Braunsbach, SW Germany." *Science of the Total Environment*, 630, 977-991.
- 531 Carper, K.L. (2000). "Forensic Engineering." 2nd Edition. Taylor & Francis (401pp).
- 532 Chen, J., Zhong, P, Xu, B., and Zhao, Y. (2015). "Risk Analysis for Real-Time Flood Control  
533 Operation of a Reservoir." *Journal of Water Resources Planning and Management*, 141 (8).
- 534 Chen, L., Qiu, H., Zhang, J., Singh, V.P., Zhou, J., and Huang, K. (2019). "Copula-based method  
535 for stochastic daily streamflow simulation considering lag-2 autocorrelation." *Journal of*  
536 *Hydrology*, 578, 123938.
- 537 Chen, J., Zhong, P., Zhang, W., and Zhu, F. (2020). "Improved Risk-Assessment Model for Real-  
538 Time Reservoir Flood-Control Operation." *Journal of Water Resources Planning and*  
539 *Management*, 146 (3).
- 540 Gomez-Ullate, E., Bayon, J.R., Coupe, S., and Castro-Fresno, D. (2010). "Performance of pervious  
541 pavement parking bays storing rainwater in the north of Spain." *Water Science and Technology*,  
542 62 (3), 615–621.
- 543 Gomez-Ullate, E., Castillo-Lopez, E., Castro-Fresno, D., and Bayon, J.R. (2011). "Analysis and  
544 contrast of different pervious pavements for management of storm-water in a parking area in  
545 Northern Spain." *Water Resource Management*, 25, 1525–1535.
- 546 Hurst, R. (2007). "An overview of forensic hydrology." *Southwest Hydrology*. 6(4), p. 16-17.

547 Huang, K., Ye, L., Chen, L., Wang, Q., Dai, L., Zhou, J., Singh, V.P., Huang, M., and Zhang, J.  
548 (2018). "Risk analysis of flood control reservoir operation considering multiple uncertainties."  
549 *Journal of Hydrology*, 545, 672-684.

550 Jing, Z., An, W., Zhang, S., and Xia, Z. (2020). "Flood control ability of river-type reservoirs using  
551 stochastic flood simulation and dynamic capacity flood regulation." *Journal of Cleaner*  
552 *Production*, 257.

553 Keating, A., Venkateswaran, K., Szoenyi, M., MacClune, K., and Mechler, R. (2016). "From event  
554 analysis to global lessons: disaster forensics for building resilience." *Natural Hazards and Earth*  
555 *System Sciences*, 16, 1603–1616.

556 Wallington, K. and Cai, X. (2020). "Feedback Between Reservoir Operation and Floodplain  
557 Development: Implications for Reservoir Benefits and Beneficiaries." *Water Resources*  
558 *Research*, 56 (4).

559 Kundzewicz, Z. W., Su, B., Wang, Y., Xia, J., Huang, J., and Jiang, T. (2019). "Flood risk and its  
560 reduction in China." *Advances in Water Resources*, 130, 37-45.

561 Kuo, J.T., Hsu, N.S., Chu, W.S., Wan, S., and Lin, Y.J. (1990). "Real-time operation of tanshui  
562 river reservoirs." *Journal of Water Resources Planning and Management*. 116 (3), 349–361.

563 Leandro, J., Chen, K. F., Wood, R. R., and Ludwig, R. (2020). "A scalable flood-resilience-index  
564 for measuring climate change adaptation: Munich city." *Water Research*, 173(15).

565 Li, X., Guo, S., Liu, P., and Chen, G. (2010). Dynamic control of flood limited water level for  
566 reservoir operation by considering inflow uncertainty. *Journal of Hydrology*. 391, 124-132.

567 Lischeid, G., Balla, D., Dannowski, R., Dietrich, O., Kalettka, T., Merz, C., Schindler, U., and  
568 Steidl, J. (2017). "Forensic hydrology: what function tells about structure in complex settings."  
569 *Environmental Earth Sciences*. 76, 40.

570 Liu, P., Cai, X. and Guo, S. (2011). “Deriving multiple near-optimal solutions to deterministic  
571 reservoir operation problems.” *Water Resource Research*. 47 (8).

572 Liu, P., Li, L., Guo, S., Xiong, L., Zhang, W., and Zhang, J. (2015a). “Optimal design of seasonal  
573 flood limited water levels and its application for the Three Gorges Reservoir.” *Journal of*  
574 *Hydrology*, 527, 1045–1053.

575 Liu, P., Lin, K., and Wei, X. (2015b). "A two-stage method of quantitative flood risk analysis for  
576 real-time reservoir operation using ensemble-based hydrologic forecasts." *Stochastic*  
577 *Environmental Research and Risk Assessment*, 29 (3), 803–813.

578 Liu, X., Lu, C., Zhu, Y., Singh, V. P., Qu, G., and Guo, X. (2017). “Multi-objective reservoir  
579 operation during flood season considering spillway optimization.” *Journal of Hydrology*, 552,  
580 554-563.

581 Loáiciga, H. A. (2001). “Flood damages under changing flood-plain conditions: a forensic-  
582 hydrology case study”. *Journal of the American Water Resources Association* 37(2), 467-478.

583 Mariën, J. L. (1984). “Controllability conditions for reservoir flood control systems with  
584 applications.” *Water Resources Research*, 20 (11).

585 Mesbah, S.M., Kerachian, R., and Nikoo, M.R. (2009). “Developing real time operating rules for  
586 trading discharge permits in rivers: application of Bayesian networks.” *Environmental Modelling*  
587 *and Software*, 24 (2), 238–246.

588 Ming, B., Liu, P., Guo, S., Zhang, X., Feng, M., and Wang, X. (2017). “Optimizing utility-scale  
589 photovoltaic power generation for integration into a hydropower reservoir by incorporating long-  
590 and short-term operational decisions.” *Applied Energy*, 204, 432–445.

591 Noon, R. (2001). “Forensic Engineering Investigation.” CRC-Press, Boca Raton (445 pp.).

592 Pham, T.V. (2011). "Tracking the uncertainty in streamflow prediction through a hydrological  
593 forecasting system."

594 Qi, Y., Yu, J., Li, X., Wei, Y., and Miao, Q. (2017). "Reservoir flood control operation using  
595 multi-objective evolutionary algorithm with decomposition and preferences" *Applied Soft*  
596 *Computing*, 50, 21-33.

597 Qiu, Y., Jia, Y., Zhao, J., and Wang, X. (2010). "Valuation of Flood Reductions in the Yellow  
598 River Basin under Land Use Change." *Journal of Water Resources Planning and Management*,  
599 136(1).

600 Reddy, M.J. and Kumar, D.N. (2006). "Optimal reservoir operation using multi-objective  
601 evolutionary algorithm." *Water Resource Management*. 20 (6), 861–878.

602 Shao, W., Xian, S., Lin, N., Kunreuther, H., Jackson, N., and Goidel, K. (2017). "Understanding  
603 the effects of past flood events and perceived and estimated flood risks on individuals' voluntary  
604 flood insurance purchase behavior." *Water Research*, 108, 391-400.

605 Tung, Y.K., Yen, B.C., and Melching, C.S. (2006). "Hydrosystems engineering reliability  
606 assessment and risk analysis." McGraw-Hill.

607 Volpi, E., Lazzaro, M. D., Bertola, M., Viglione, A., and Fiori, A. (2018). "Reservoir Effects on  
608 Flood Peak Discharge at the Catchment Scale." *Water Resources Research*, 54(11).

609 Wang, F., Wang, L., Zhou, H., Valeriano, O. C. S., Koike, T., and Li, W. (2012). "Ensemble  
610 hydrological prediction-based real-time optimization of a multiobjective reservoir during flood  
611 season in a semiarid basin with global numerical weather predictions." *Water Resources*  
612 *Research*, 48(7).

613 Wei, C.C. and Hsu, N.S. (2008). "Multi-reservoir flood-control optimization with neural-based  
614 linear channel level routing under tidal effects." *Water Resource Management*. 22 (11), 1625–  
615 1647.

616 Windsor, J.S. (1973). "Optimization model for the operation of flood control systems." *Water*  
617 *Resource Research*, 9 (5), 1219–1226.

618 Woodward, M., Gouldby, B., Kapelan, Z., and Hames, D. (2014). "Multiobjective Optimization  
619 for Improved Management of Flood Risk." *Journal of Water Resources Planning and*  
620 *Management*, 140(2).

621 Wu, S.-J., Yang, J.-C., and Tung, Y.-K. (2011). "Risk analysis for flood-control structure under  
622 consideration of uncertainties in design flood." *Natural hazard*, 58 (1), 117–140.

623 Wu, Y., and Chen, J. (2013). "Estimating irrigation water demand using an improved method and  
624 optimizing reservoir operation for water supply and hydropower generation: A case study of the  
625 Xinfengjiang reservoir in southern China." *Agricultural Water Management*, 116, 110–121.

626 Yan, B., Guo, S., and Chen, L. (2014). "Estimation of reservoir flood control operation risks with  
627 considering inflow forecasting errors." *Stochastic Environmental Research and Risk Assessment*,  
628 28 (2), 359–368.

629 Zhang, X., Liu, P., Xu, C.Y., Gong, Y., Cheng, L., and He, S. (2019). "Real-time reservoir flood  
630 control operation for cascade reservoirs using a two-stage flood risk analysis method." *Journal*  
631 *of Hydrology*, 577.

632 Zhao, T., Zhao, J., Lund, J. R., and Yang, D. (2014). "Optimal Hedging Rules for Reservoir Flood  
633 Operation from Forecast Uncertainties." *Journal of Water Resources Planning and Management*.  
634 140(12).



635 Zhou, J.J. (2010). "Situation of the mid-Yangtze flood after the commencement of the Three  
636 Gorges Project and the countermeasures (I)." *Scientific and Technical Review*, 28 (22), 60-68  
637 (in Chinese).

638 Zhou, Y.L., Guo, S., Xu, C.Y., Liu, P., and Qin, H. (2015a). "Deriving joint optimal refill rules  
639 for cascade reservoirs with multi-objective evaluation." *Journal of Hydrology*, 524, 166–181.

640 Zhou, Y.L., Guo, S., Xu, J., Zhao, X., and Zhai, L. (2015b). Risk analysis for seasonal flood limited  
641 water level under uncertainties. *Journal of Hydro-environment Research*, 9 (4), 569–581.

642 Zhu, F., Zhong, P., Wu, Y., Sun, Y., Chen, J., and Jia, B. (2017a). "SMAA-based stochastic  
643 multicriteria decision making for reservoir flood control operation." *Stochastic Environmental  
644 Research and Risk Assessment*, 31, 1485–1497.

645 Zhu, F., Zhong, P., Sun, Y., and Yeh, W.W. (2017b). "Real-time optimal flood control decision  
646 making and risk propagation under multiple uncertainties." *Water Resource Research*, 53 (12),  
647 10635–10654.

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**Table 1-** Reservoirs characteristics.

Reservoir Specification	Karun 4	Karun 3	Karun 1	Masjed- Soleiman	Gotvand	Rudbar- Lorestan	Dez
Normal operating volume ( $10^6 \text{ m}^3$ )	2280	2719	2438	261.6	4671	215	2698.5
Minimum operating volume ( $10^6 \text{ m}^3$ )	1446	1094	824	201	1621	97.47	726.5
Power plant's designed discharge ( $\text{m}^3/\text{s}$ )	684	1371	1471	1605	843	116	357

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**Table 2-** Developed scenarios' specifications for Karun sub-basin reservoirs.

Name of Reservoir	Karun 4		Karun 3		Karun 1		Masjed-Soleiman		Gotvand	
Power plant's design discharge (m <sup>3</sup> /s)	684		1371		1471		1605		843	
Downstream Safe Discharge (m <sup>3</sup> /s)	3000		3000		3000		3000		1500	
Ahwaz Safe Discharge (m <sup>3</sup> /s)	3000-3200									
N. Scenario	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)
1	3/11/2019	342	3/11/2019	685.5	3/11/2019	735.5	3/11/2019	Inflow	3/11/2019	843
	3/18/2019	684	3/18/2019	1371	3/18/2019	1471	3/18/2019	Inflow	3/18/2019	843
	3/23/2019	342	3/23/2019	685.5	3/23/2019	735.5	3/23/2019	Inflow	3/23/2019	843
2	3/18/2019	684	3/18/2019	1371	3/18/2019	1471	3/18/2019	Inflow	3/18/2019	843
	3/23/2019	342	3/23/2019	685.5	3/23/2019	735.5	3/23/2019	Inflow	3/23/2019	843
3	3/20/2019	684	3/10/2019	1371	3/2/2019	1471	3/18/2019	1605	1/22/2019	843
	3/23/2019	9.84	3/23/2019	228.5	3/23/2019	245.16	3/23/2019	267.5	3/23/2019	140.5

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**Table 3-** Developed scenarios' specifications for Dez sub-basin reservoirs.

Name of Reservoir	Rudbar-Lorestan		Dez	
Power plant's design discharge (m <sup>3</sup> /s)	116		357	
Downstream Safe Discharge (m <sup>3</sup> /s)	460		1100	
N. Scenario	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)	Prerelease starting date	Releasing discharge (m <sup>3</sup> /s)
1	3/11/2019	58	3/11/2019	1100
	4/4/2019	Historical outflow	4/4/2019	Historical outflow
2	3/18/2019	58	3/18/2019	1100
	4/4/2019	Historical outflow	4/4/2019	Historical outflow
3	3/23/2019	19.33	2/28/2019	1100

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**Table 4-** Calculated criteria for Karun sub-basin reservoirs.

Criterion	Scenario	Unit	Reservoir				
			Karun 4	Karun 3	Karun 1	Masjed-Soleiman	Gotvand
PDR	Historical	%	66	2.62	1	1	47
	1		77	71	30	0	73
	2		77	71	30	0	67
	3		66	56	58	0	91
FVR	Historical	%	28	13	10	1	9
	1		47	30	13	0	22
	2		47	30	14	0	20
	3		46	58	62	0	72
PFD	Historical	Day	13	8	0	0	20
	1		24	0	13	0	0
	2		24	0	14	0	24
	3		13	24	7	0	25
FCR	Historical	%	39	16	12	1	23
	1		47	36	13	7	22
	2		47	30	14	2	19
	3		46	58	62	3	71
Reliability of no downstream damage	Historical	%	100	100	100	100	79
	1		100	100	100	100	100
	2		100	100	100	100	100
	3		100	100	100	100	100
Resiliency to downstream damage	Historical	%	100	100	100	100	33
	1		100	100	100	100	100
	2		100	100	100	100	100
	3		100	100	100	100	100
Vulnerability to downstream damage	Historical	%	0	0	0	0	10
	1		0	0	0	0	0
	2		0	0	0	0	0
	3		0	0	0	0	0

**Table 5-** Calculated developed criteria in Dez sub-basin reservoirs.

Criterion	Scenario	Unit	Reservoir	
			Rudbar-Lorestan	Dez
PDR	Historical	%	33	22
	1		33	53
	2		33	53
	3		64	52
FVR	Historical	%	14	10
	1		21	15
	2		21	14
	3		79	23
PFD	Historical	Day	7	8
	1		7	9
	2		7	10
	3		6	7
FCR	Historical	%	28	16
	1		31	21
	2		31	19
	3		79	22
Reliability of no downstream damage	Historical	%	100	21
	1		100	14
	2		100	14
	3		100	28
Resiliency to downstream damage	Historical	%	100	4
	1		100	4
	2		100	4
	3		100	10
Vulnerability to downstream damage	Historical	%	0	193
	1		0	78
	2		0	78
	3		0	77

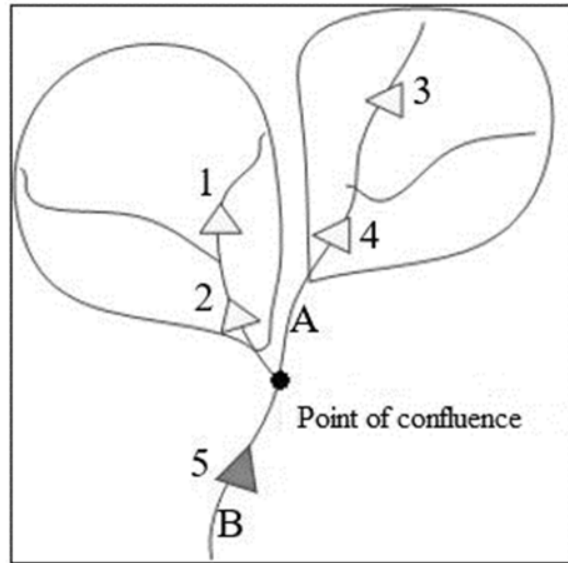
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**Table 6-** predicted values of Inflow using artificial neural network and its specifications.

Sub-Basin	Predictive months	Predicted month	Number of years in model training	Historical cumulative inflow ( $10^6 \text{ m}^3$ )	Predictive cumulative Inflow ( $10^6 \text{ m}^3$ )	Error (%)	RMSE ( $10^6 \text{ m}^3$ )	Number of layers	Number of first layer neurons	Number of second layer neurons	Epoch	Transfer function
Karun	January and February	March	58	7440.29	8585.42	15.39	1145.13	2	3	1	1000	Logsig
		April		12080.71	14981.29	24.01	2900.58	2	3	1	1000	Tansig
Dez	January and February	March	54	5787.051	7639.613	32.01	1852.56	2	3	1	1000	Logsig
		April		10296.36	7992.78	22.37	2303.58	2	3	1	1000	Tansig

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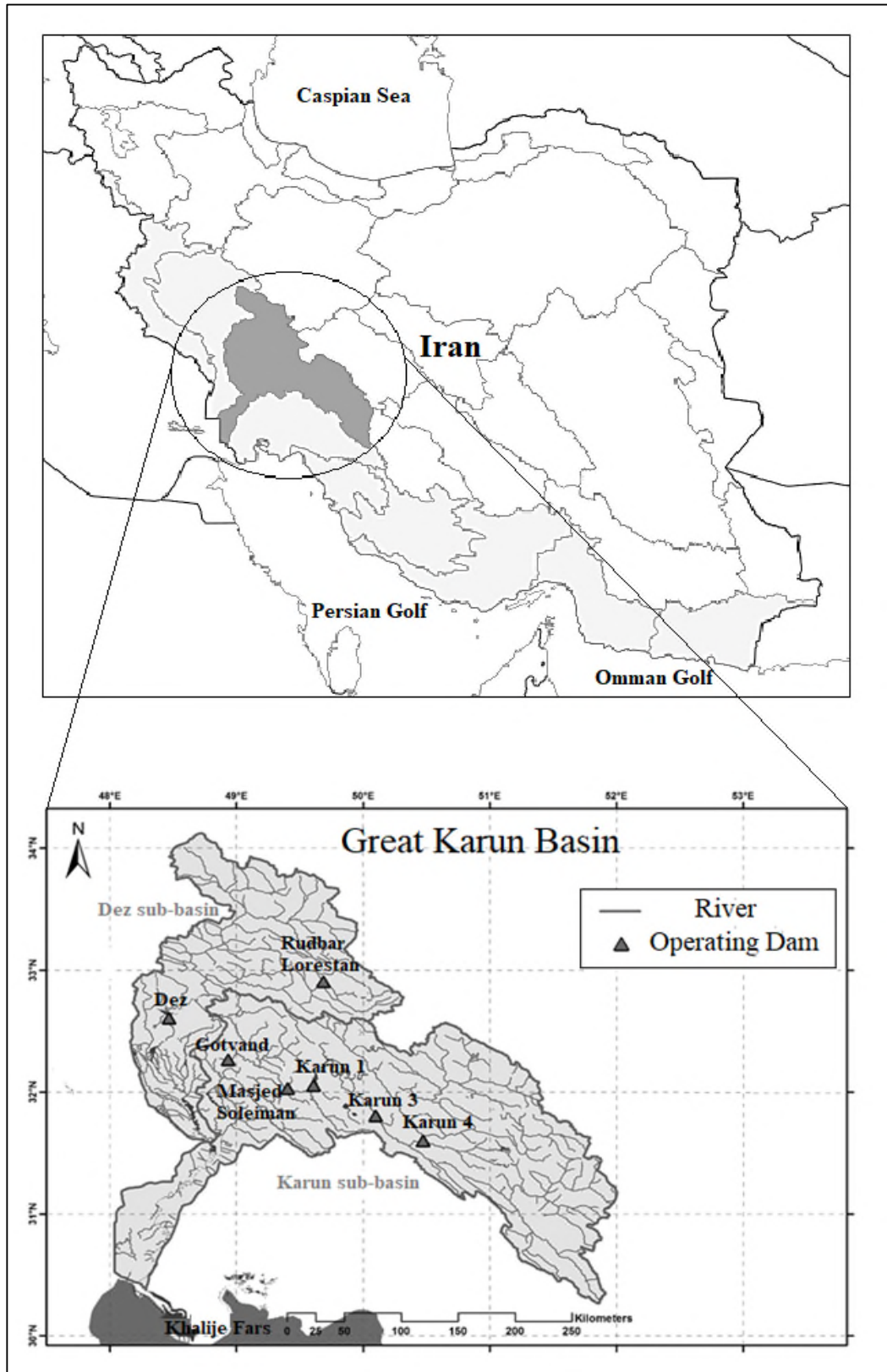
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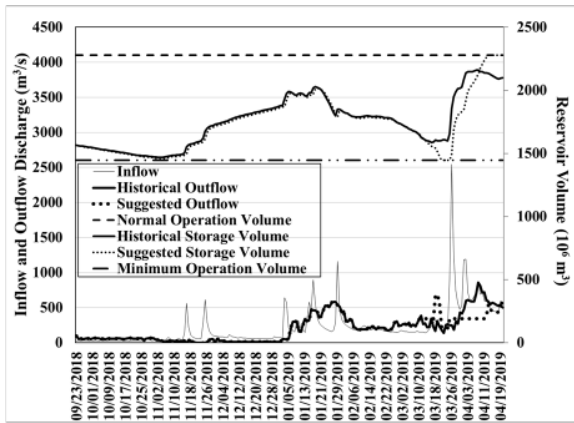
**Figure 1-** Schematic of parallel and series reservoirs systems.



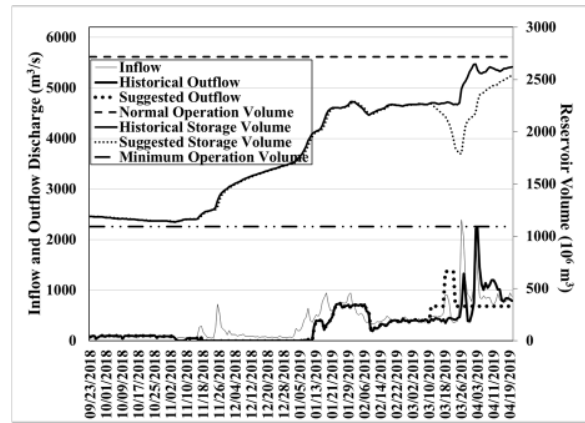


**Figure 2-** Map of the Great Karun Basin and its operating reservoirs.

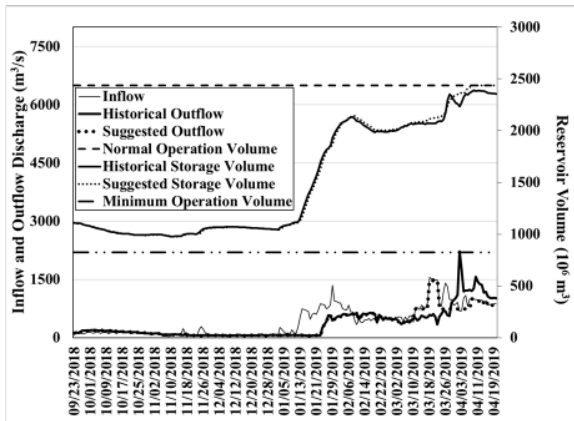
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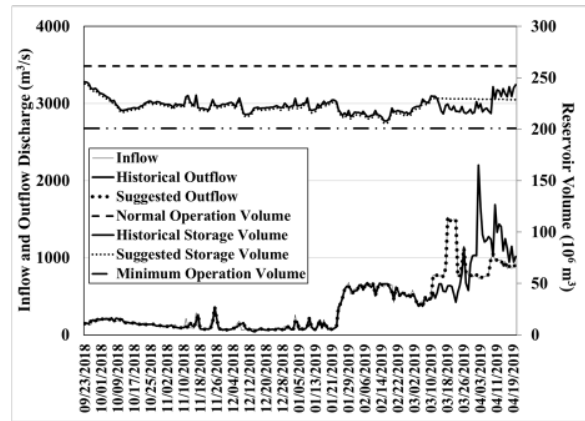
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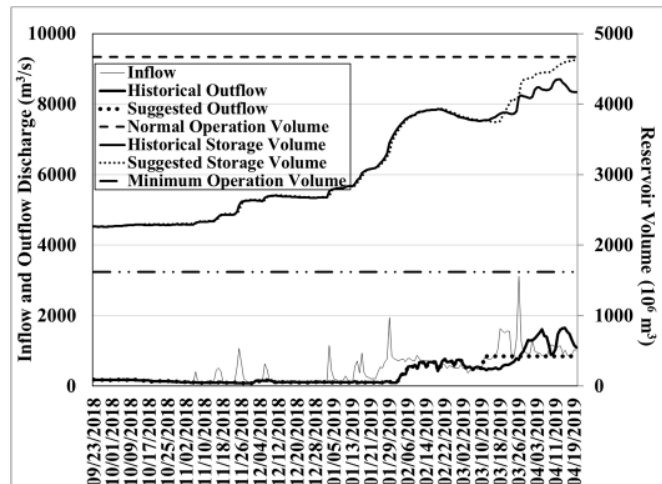
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(c)

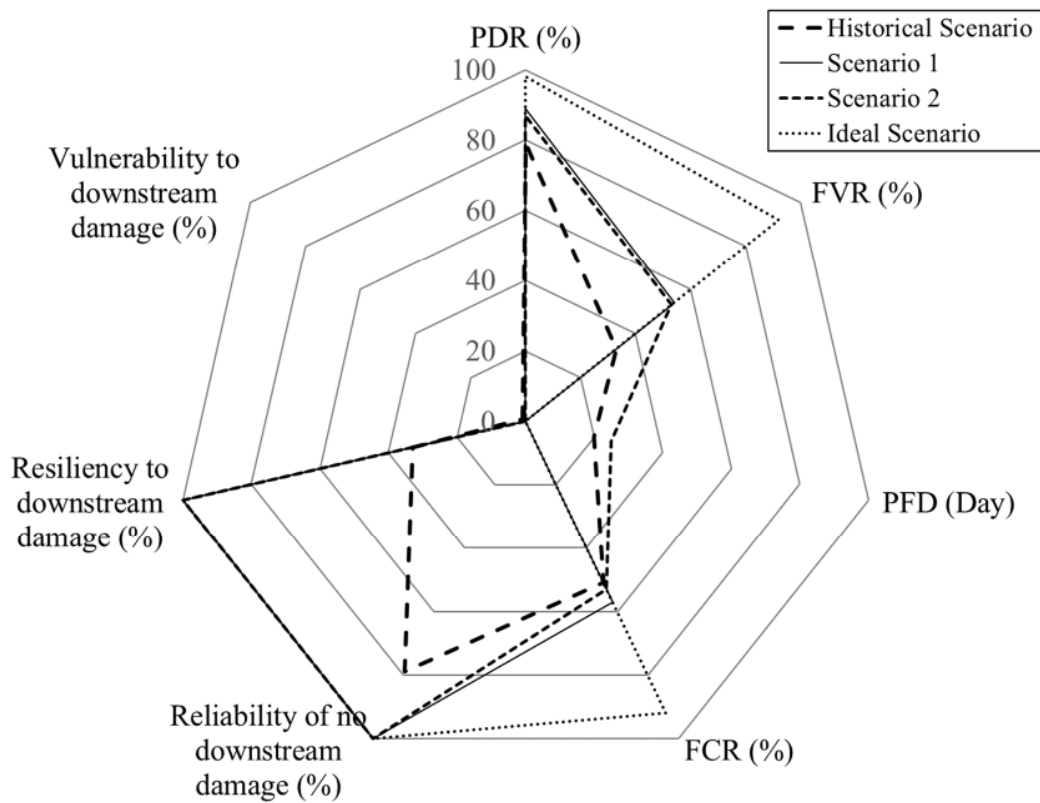


(d)



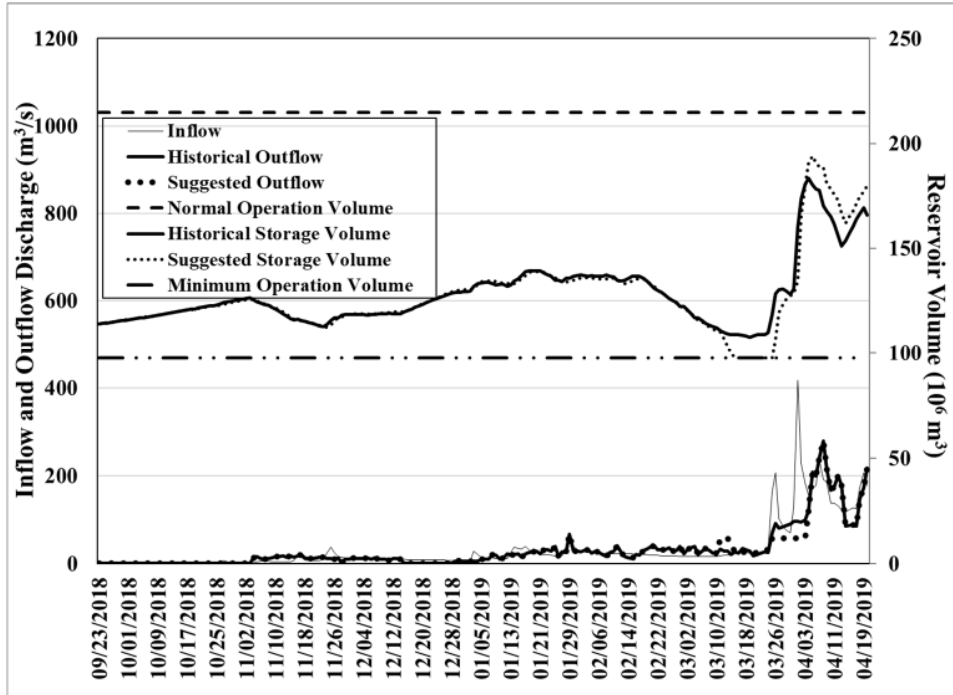
(e)

673 **Figure 3-** Daily changes in the water volume and discharges of (a) Karun 4 (b) Karun 3 (c)  
674 Karun 1 (d) Masjed-Soleiman and (e) Gotvand reservoirs under Scenario 1.

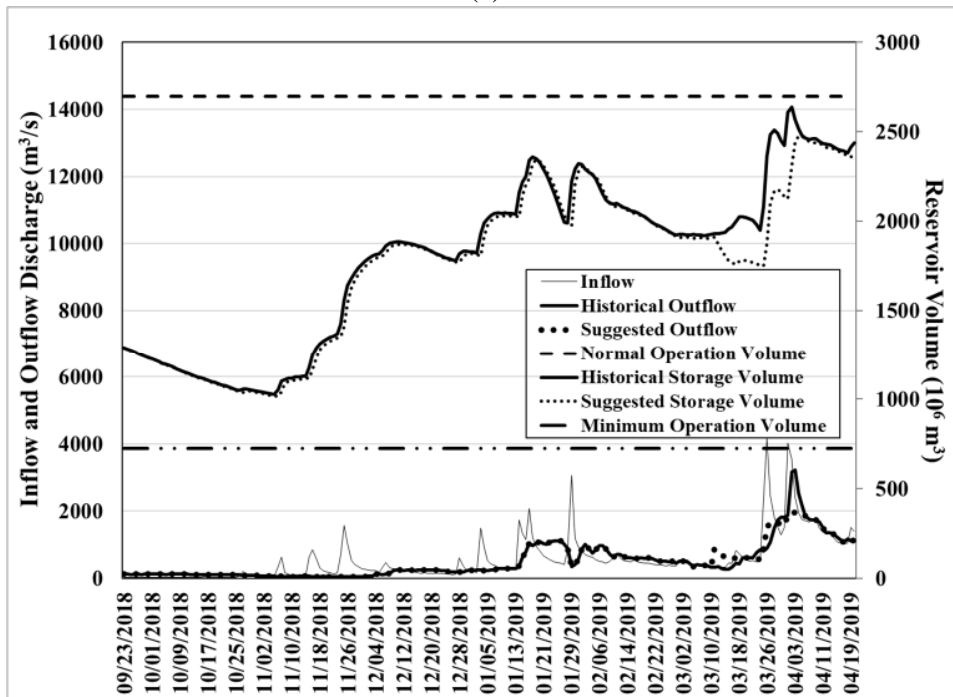


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**Figure 4-** Comparison radar graph of the developed basin criteria for historical and developed prerelease scenarios of the Karun sub-basin

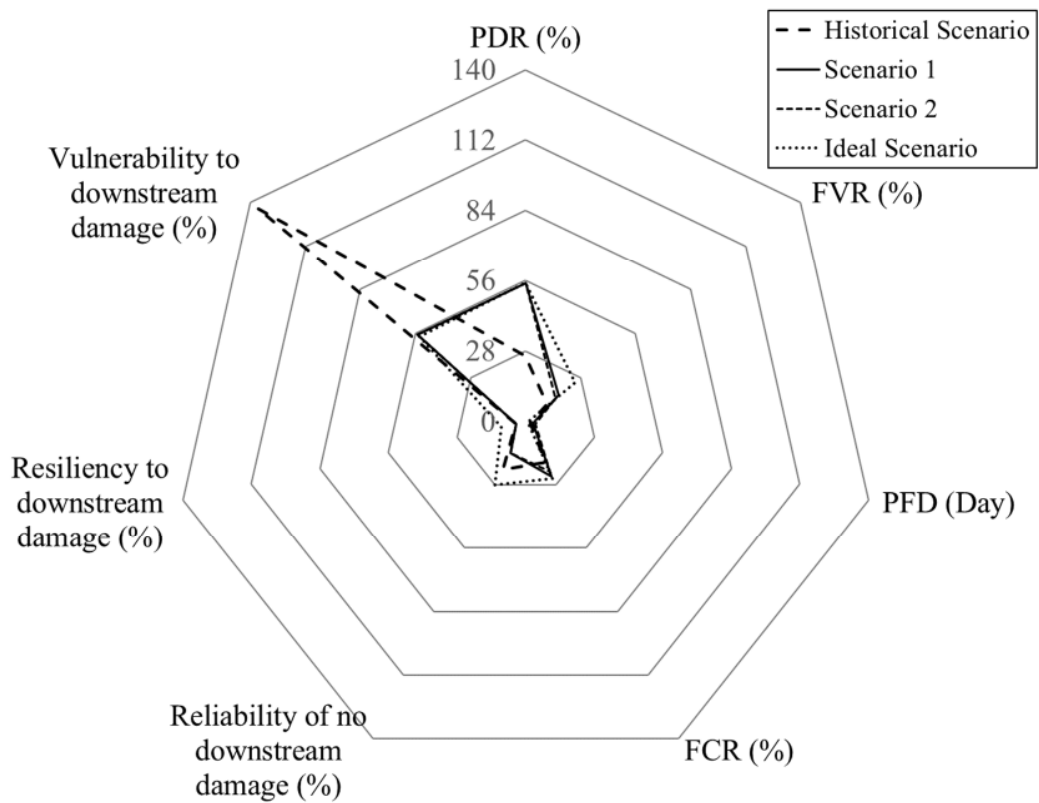


(a)



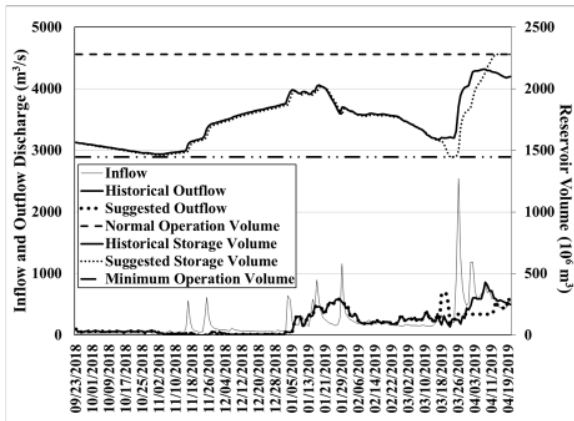
(b)

679 **Figure 5-** Daily changes in the water volume and discharges of (a) Rudbar-Lorestan (b) Dez  
680 reservoirs under Scenario 1

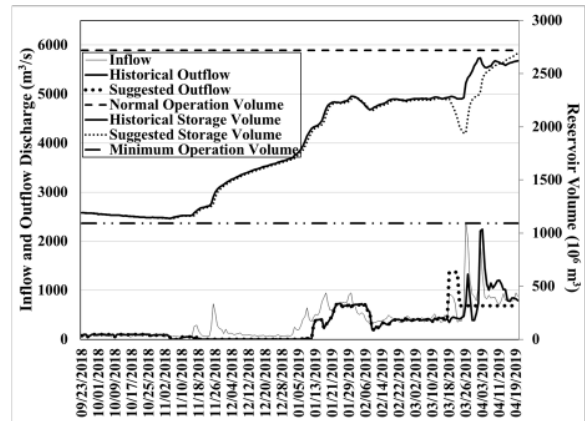


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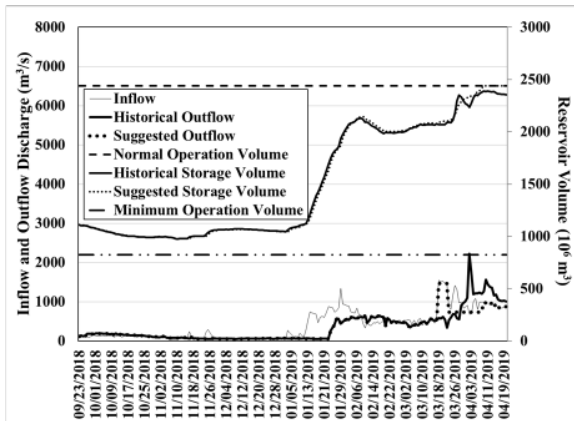
**Figure 6-** Comparison radar graph of the developed basin criteria for historical and developed prerelease scenarios of the Dez sub-basin.



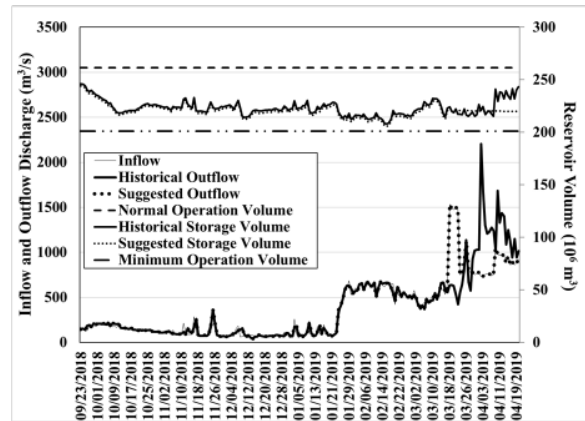
(a)



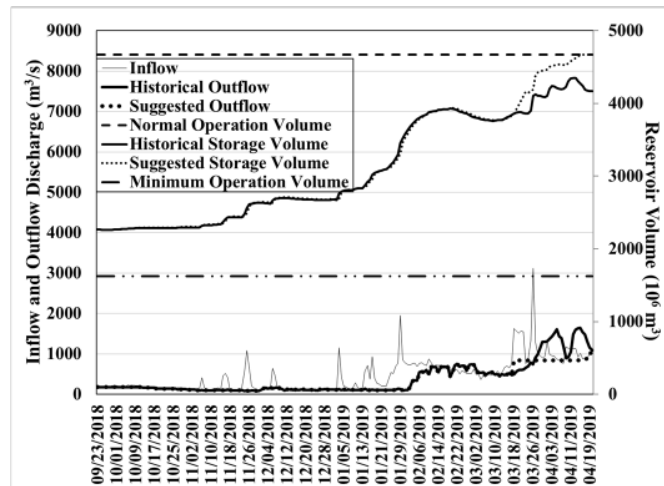
(b)



(c)

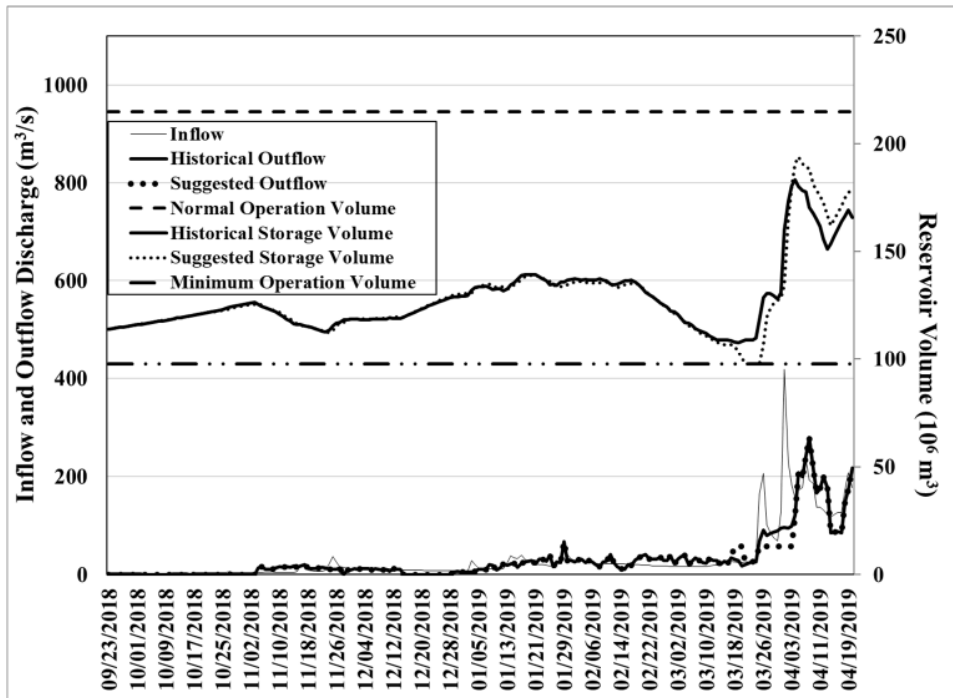


(d)

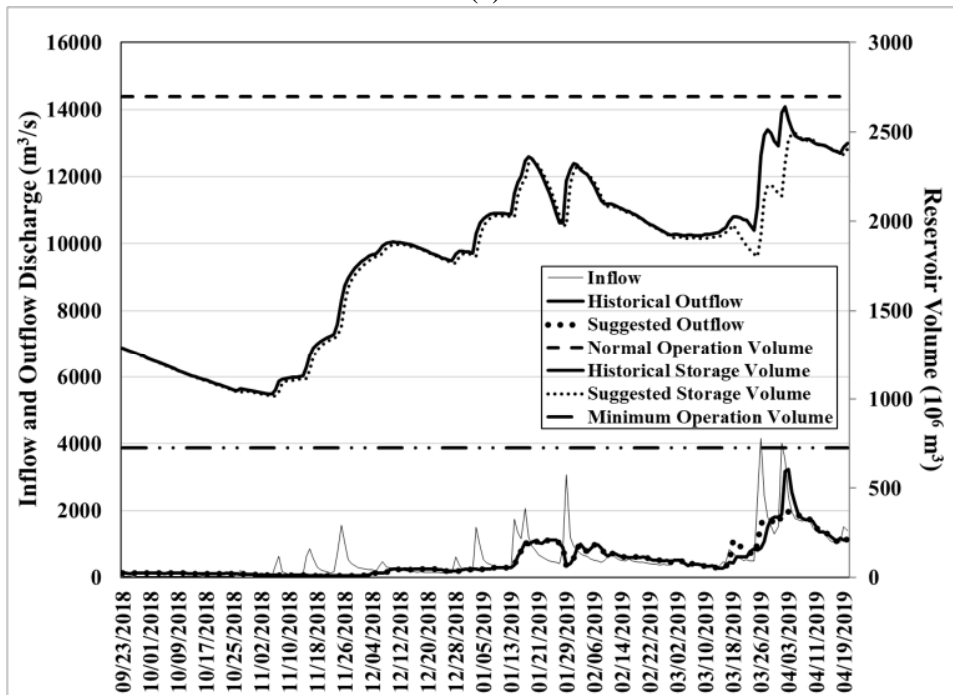


(e)

685 **Figure 7-** Daily changes in the water volume and discharges of (a) Karun 4 (b) Karun 3 (c)  
686 Karun 1 (d) Masjed-Soleiman and (e) Gotvand reservoirs under Scenario 2.



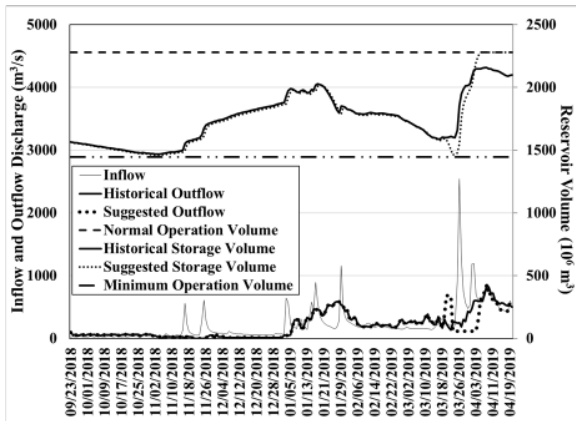
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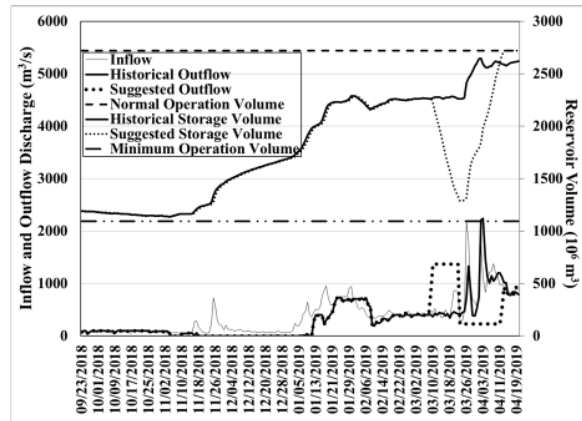
(b)

687 **Figure 8-** Daily changes in the water volume and discharges of (a) Rudbar-Lorestan (b) Dez  
 688 reservoirs under Scenario 2.

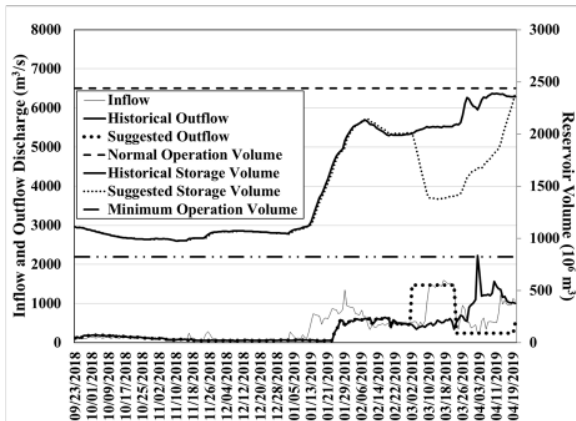
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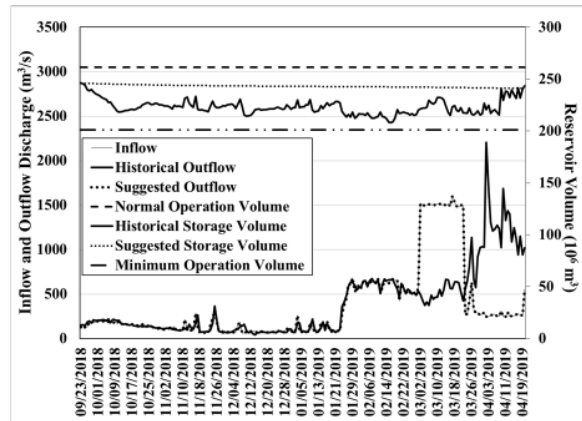
(a)



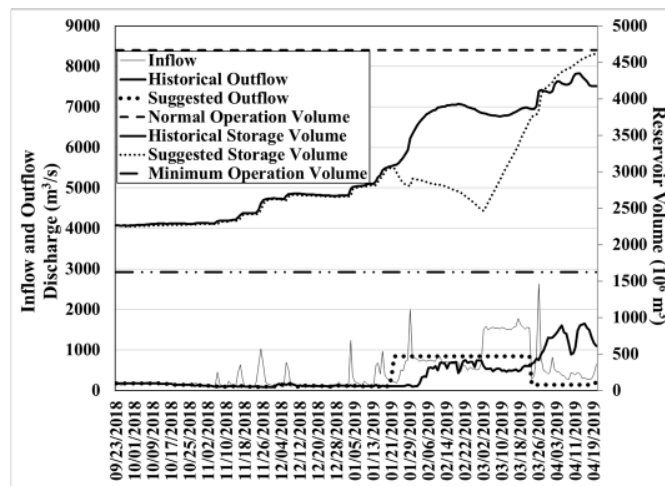
(b)



(c)



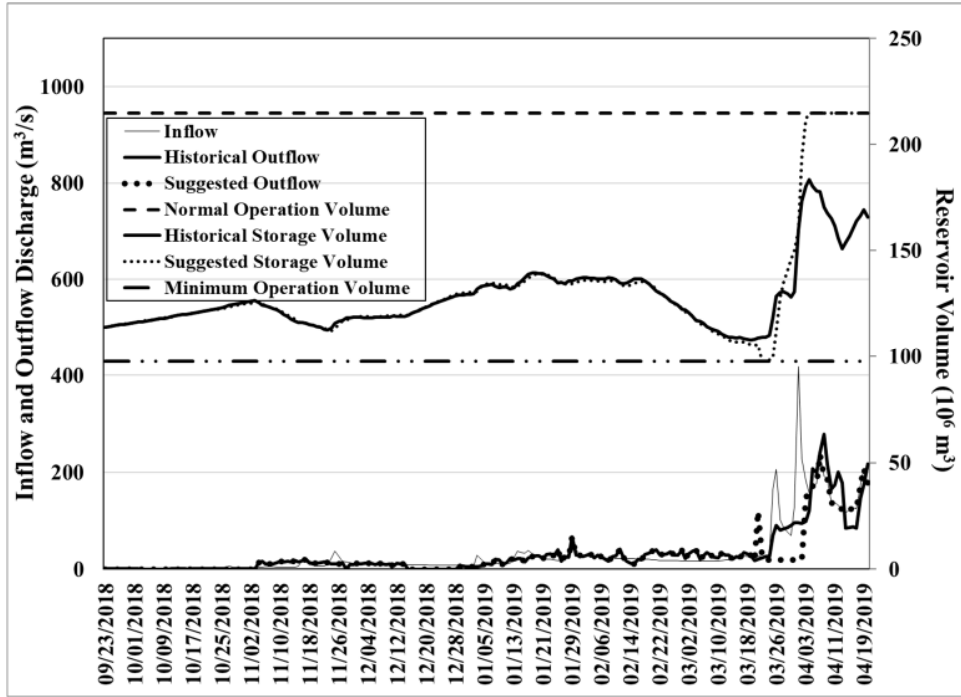
(d)



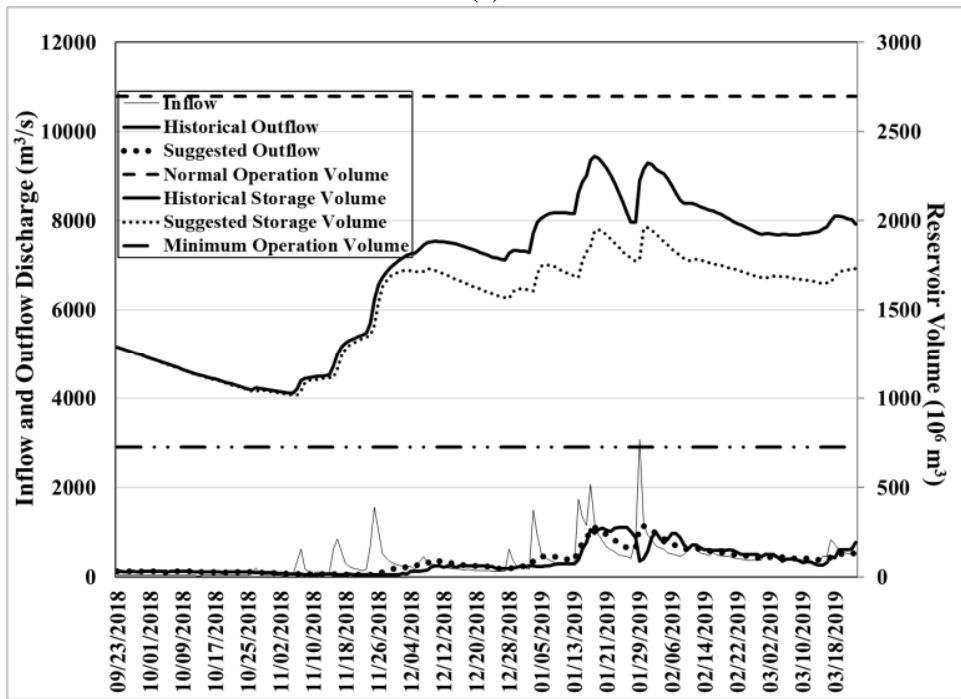
(e)

690 **Figure 9-** Daily changes in the water volume and discharges of (a) Karun 4 (b) Karun 3 (c)  
 691 Karun 1 (d) Masjed-Soleiman and (e) Gotvand reservoirs under Scenario 3.





(a)



(b)

692 **Figure 10-** Daily changes in the water volume and discharges of (a) Rudbar-Lorestan (b) Dez  
 693 reservoirs under Scenario 3.