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Recent Advancement in Proportioning of No-fine Concrete - Review

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ARTICLE INFORMATION	ABSTRACT
DOI: 10.15415/jotitt.2019.71001	The paper highlight characteristics of no fine concrete (NFC) by evaluating and interpreting the research work undertaken by research scholars around world in past. It explains the approach for better performances. It helps to understand and identify the difference between knowledge and actual performance on ground which stops it from wide and acceptable application throughout the globe. A comprehensive investigation of proportioning of no fine Concrete has been discussed. Identification of relationship between mechanical properties with its design
<i>Keywords</i> : Durability, Supplementary cementitious material, Mechanical strength, Microstructure, Porosity, Permeability	and pore structure of NFC will help us in studying ways of its better utilization. The specific reference has been made for India to understand level of research & development in laboratories and status of different applications at different levels.

1. Introduction

No fine concrete (NFC) is composed of large aggregates which allow water to infiltrate into ground from surface. In cities, every major road and shoulder drains are topped with normal concrete. The rainwater has no space to percolate down to the soil and ground water is not being recharged. The pavement tiling projects enclose trees completely, unable to spread their roots, the trees do not last long enough and also weaken the footpath. Not only are the trees put to strain but also the water heads off as storm water through the drains instead of seeping into the ground. The drying up of groundwater affects the trees and strains their growth. Storm water drain overflow is leading to urban floods and water logging. Considerable volume of rain water flow on impervious roadway, parkways and street roads is not absorbed by soil. Such waste of water disturbs the natural balance of ecosystem and results in frequent floods, thinning of ground water table and pollution of rivers. The obvious solution is to use NFC on broad scale. The traditional concrete should be converted to porous or NFC to the extent possible. Such NFCs function as natural filters by absorbing pollutant loads and stopping them from flowing into water channels. NFC has low unit weight compared to normal concrete. It is unique in terms of porosity and cement matrix is the dominating factor to provide strength [1]. The voids in macrostructure allow water to percolate through. It is cheap as far as costs

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are concerned. It has high thermal insulation value as reflected in Fig. 1.

The potentiality of NFC in allowing water to flow rapidly is top characteristic followed by its strength to carry loads. Until now, the term permeability has not been specified leading to confusion of how much permeable, permeable concrete is. Initial good permeability might reduce with time due to clogging. Although, the industry is moving toward design permeability but lack of comprehensive data and understanding about behavior of porosity and infiltration test is a hindrance [2]. NFC absorbs UV & heat rays and help in reduction in temperature [3]; however it is subject to regular care for long term benefits.

The present review paper helps us in

understanding the proportioning stated by different authors and looking out for avenues for application of NFC. Fig. 2 shows the observed results.

2. Challenges in application of permeable pavements

Permeable pavement systems (PPS) have wide applications ranging from residential houses to commercial factories but due to below mentioned challenges the usage is very less and irregular. These have very low strength if compared with normal concrete pavement systems. The amount of damage and time period of damage makes builders reluctant to use it. Due to this the usage has been for light utility only. The early strength development is essential in case of PC as the



Figure 1: Benefits of using NFC



Figure 2: Photographic view of no-fine concrete

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road need to be open for traffic at a time limit of 7 days [4]. Some exercise need to be done to achieve early strength, adding mineral admixtures or polymer modification with sufficient water drainage capacity.

The comprehensive data to access particular strength is not available. A full proof system for measuring level and quantum of permeability is still not available. The pavement tends to clog very early compromising its applicability to a large extent. The pollutants percolate through the voids and reach ground water and affect the ecosystem negatively. The maintenance efforts and cost are very high which reduces economic viability.

Many researchers [5,6] have done detailed study and [7-9] have proposed technical guidelines still lack of authentic central technical warehouse and access to it has resulted in some failures [10] of NFC in practical applications. [11,12] .Have also tried to understand traffic loads for NFC with falling weight deflectometer, but further research is required in this. The wide-range of research on (PPS) was also summarized [13] which highlight the current scenario in research and industry and recommends the future avenues of research and development. In one of the available pilot study [14], higher raveling was observed in turning zones compared to driving and parking ones but the permeability was observed to be satisfactory although the compressive strength was low. Haselbach and Freeman [15] investigated vertical porosity distribution within a field placed NFC slab to understand clogging and ways of maintenance and inferred that porosity reduction was highest at top. The property is beneficial since clogging at top provide an opportunity to clean it through vacuuming for regular maintenance. There are many problems surrounding the measurement of quality in NFC pavements. The problems exist because the material properties and their sensitivity to variations, which occur in consolidating and curing $\lceil 16 \rceil$.

3. History of no-fine concrete application

Pervious concrete has been used in the past in the various parts of the world for building constructions before World War II. After 1946 NFC was utilized for variety of applications [1]. Brief history of no fine concrete is shown with the help of diagram in Fig. 3.



Figure 3: History of application of NFC

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4. Materials and its properties used in making NFC

4.1 Aggregates

In accordance with ASTM D448 [17], single size or gradation in between (19-9.5mm) aggregate are generally utilized in the making of NFC from NA and LWA. The NA to be used in NFC should be free from dust or clay particles and any type of coating of chemical that can decrease the strength of bond between cement matrix and aggregates.

The effect of shapes and size of aggregates was determined on the permeability of NFC [18]. The result found indicates that aggregate with less angularity number produce mix having low permeability. Neptune and Putman [19] reported that unit weights of aggregates should be determined in accordance with ASTM C29/C29M [20]. The mechanical strengths of NFC increases with the coefficient of uniformity (Cu) to a point after which it decrease. In one of the studies [21] it was reported that, if we use high quantity of small aggregates (4-8 mm), then we can achieve higher density and flexural strength of concrete.

Gaedicke et al. [22] investigated three types of aggregates and found their effect on porosity and abrasion resistance. Use of RCA in NFC decreases the unit weight as reported by Kim and lee [23], but increases its environmental benefits [24, 25]. Previous studies indicated that waste material can also be successfully utilized as a substitute of natural aggregate to produce NFC. For example Nguyen et al. [26] utilized sea shell by-products in partial replacement of aggregates in NFC both as an environment friendly building material and a potential component.

Kuo *et al.* [27] reported that the Washed municipal solid waste incinerator bottom ash (MSWIBA) of maximum size 12.5 mm used as a replacement of NA does not cause any significant difference in connected porosities, compressive strength and permeability coefficient. Different approaches have been used by the researchers in making NFC with Recycle aggregate in different replacement levels are shown in Table 1.

Replacement of NA with RCA, increase the porosity and permeability. Bhutta et al $\lfloor 28 \rfloor$ reported that by polymer modification, the compressive strength of NA and RCA increase by 1.57 and 1.79 times respectively.

The weak cement matrix and interfacial transition zone among gravels in recycled aggregate concrete which causes compressive strength to decrease in case of traditional concrete [29].

It is also found that the use of LWA in NFC, decrease the unit weight and thermal conductivity which makes it suitable for use as insulating concrete; however, compressive strength improved significantly [30,31].

Author	RCA % / Miner- al Admixture	Major findings
Li [34] & Rizvi <i>et</i> <i>al.</i> [35]	RCA 15%	No substantial difference visible between two natural and Recycle Coarse Aggregate.
Bhutta et al. [60]	RCA 100% + Polymer	Gain in void ratio and water permeability, augmentation of mechanical strength by use of polymer.

Table 1: No-fine concrete made with recycle aggregate

Table 1 [Continued]			
Author	RCA % / Miner- al Admixture	Major findings	
F. Tittarelli <i>et al.</i> [38]	RCA 100%	Capillary water absorption increased by 100% but mechanical performance decreased by 30%.	
Gaedicke <i>et al.</i> [9]	RCA 50%	Porosity increased by 20% but mechanical per- formance decreased by 8%.	
Guneyisi <i>et al.</i> [142]	RCA 25%, 50%, 75%, 100%	Augmentation of permeability coefficient, de- cline in unit weight and mechanical properties.	
Gaedicke <i>et al.</i> [9]	RCA 50%+ GGBFS	RCA and GGBFS have no impact on abrasion resistance.	
Z. Zhang <i>et al.</i> [145]	RCA 100% + Fly Ash	Addition of Recycle clay brick aggregate in- creased crushing index but decreased compres- sive, flexure strength and elasticity modulus by 36%, 28% and 21% respectively (28 days).	

4.2 Cementitious materials

Ordinary Portland cement has been generally used as major binder in making concrete. Industrial wastes like FA, slag and SF can also be used in addition to Portland cement and should meet the requirements of ASTM standards. Aspects such as rate of strength development, setting time and permeability also need to be studied for respective SCM.

Partially replacing OPC with MA like FA, SF, and RHA is also well researched. The thickness of cementitious layer is in direct relationship with mechanical and hydrological properties of NFC. Yang and Jiang [32] found that compressive strength of more than 45 MPa and flexural strength of more than 5 MPa could be achieved by the addition of Silica fume, and using smaller size aggregate. Yang [33] found that there is increase in performance against Freeze-thaw résistance with the addition of silica fume. The RHA and fiber has been observed to increase the mechanical properties $\lceil 34 \rceil$. It was also reported by Zhong & wille [35] that the strength of cement, size of aggregates and aggregate to binder (a/b) ratio largely modify the strength properties. Also, the film forming ability was determined [36] in terms of paste thickness, and reported that the large size aggregate make thicker film compared to small aggregate size. The required amount of paste in NFC can also be estimated from the paste thickness. Large size aggregate has more paste thickness as compared to the small size aggregates. The strength properties increase as the size of aggregate decrease which increases the binding area.

4.3 Admixtures

Water reducing admixtures depend on the w/c for mixing [1]. Retarding admixtures are generally required while making stiff paste like in case of NFC. They are specifically helpful in hot weather application. Some admixture like Retarding admixtures are used during moulding process to act as lubricant and helping in taking the mix out easily. Accelerators are helpful when NFC is placed in cold environment. Many studies have reported the use of viscosity modifying agent (VMA) (in accordance with ASTM C494) with water reducing agent to modify the fresh properties of NFC [22, 142, 143].

Air entraining admixtures are often used in NFC. When used in environment susceptible to freezing and thawing, it has shown good resistance. As widely reported $\lceil 38-40 \rceil$, the experimental investigation explored the effect of air entraining admixtures and reported that water to cement (w/c) ratio less than 0.40, entrained air bubbles are not required since the paste matrix are more resistant to freeze-thaw action but further corroboration is required to validate this.

4.4 Mix proportions

The material used for mix proportioning of NFC is combination of Portland cement, with uniform or binary combination of coarse aggregate, miniscule fine aggregates by weight of total aggregate and water [41]. As widely reported by many researches [4, 21, 32, 34, 143, 42-467, with the increase of little amount of fine aggregate as sand, the compression strength, durability and resistance against F/T tend to enhance. Generally PC consists of 270-415 kg/m3 of cement, 1190-1480 kg/ m3 of aggregate and w/c ratio from 0.26-0.40. The 28 days compressive strength varies from 3.5-28 MPa, hydraulic conductivity in the range 0.2-5.4mm/s and pore size 2-8mm, depending on the type of aggregate and compaction technique used.

From past research studies it is seen that NFC can be produced by varying the w/c ratio, a/c ratio, size of aggregate and binder material type shown in Table 2. From the past studies it is found that NFC mix proportions are selected from experimental basis; however, the numbers of proposed studies on mix design of NFC are very less. The design methods reported in past literature have many disadvantages [1, 47-49] for e.g. they do not indicate how to determine the w/c ratio or effect of compaction on properties of concrete has not been taken into consideration as well there are no appropriated technique to determine the mix design of NFC.

Nguyen et al. [26] proposed the mix design for NFC based on the presumption that the cementitious paste acts only as a coating. Based on the hypothesis, required w/c, aggregate volume, cement paste volume were calculated. Binder drainage test was also proposed to evaluate the w/c ratio so that the cement paste does not go down the lower layers of concrete. To obtain empirical equation for mixture design, Zhangi et al [48] recommended orthogonal test. According to the Talbot's formula and previous experience, the orthogonality test was designed with consideration of three factors - cement dose, water cement ratio and aggregate gradation with four levels each.

References	Year	Cementitious Material (kg/m³)	Aggregates (kg/m³)	W/CM	Agg./cm ratio	Water (kg/m³)
Khankhaje <i>et</i> <i>al.</i> [50]	2016	340	1460	0.32	-	109
Chandrappa <i>et al.</i> [51]	2016	321-487	1373-1692	0.25-0.35	3.0-5.0	84-161
Yahia and Kabagire ∑52]	2014	195-535	1500-1700	0.30	-	-

Table 2: Summary of mix proportions of NFC from past literature (recreated as in [16])

Table 2 [Continued]						
References	Year	Cementitious Material (kg/m³)	Aggregates (kg/m³)	W/CM	Agg./cm ratio	Water (kg/m³)
Nguyen et al.[46]	2013	309	1525	0.30	4.9	93
Lim et al. [53]	2013	242-495	1560	0.20-0.30	3.15-6.44	73.4 - 148.5
Bassuoni and Sonebi [54]	2010	315-415	1200-1400	0.28-0.40	4-6	125-154
Huang et al. [42]	2010	320-353	1440-1587	0.35	4.5	112.1 - 123.4
Neithalath et al. [55]	2010	309-312	1544-1568	0.33	5	101-104
Kevern et al. [56]	2010	340	1540	0.27	4.53	100
Crouch et al. [57]	2007	287-345	1542-1620	0.30	4.5-5.6	87-105
Ghaffori et al. [58]	1995	300-413	1651-1800	0.37-0.42	4-6	125-154

Joshaghani *et al.* [59] utilized the Taguchi method for designing the NFC pavement to optimize the mix design. The study determined that strength is mainly governed by the void ratio and increase in maximum size of aggregates, as the void ratio surge, the strength of single size aggregate decline. As stated in the study, the most dominating factors in compressive strength are the quantity of paste and in tensile and flexural strength is the size of aggregate.

Several successful attempts have been made on the mix design for NFC (provided in Table 2) and concluded that adequate combination of mix ingredients in NFC gives the required strength and durability. The network of interconnected voids of aggregate coated with stiff paste should be maintained.

5. Fresh properties of NFC

5.1 Unit weight

Fresh state of NFC is a function of workability,

unit weight and porosity and it is affected mainly by water to cement ratio, workability and properties of aggregates used. Aggregate in concrete is a big factor with type, size and water absorption playing crucial roles. Sometimes, it is very difficult to obtain workable NFC of required porosity due to zero slump loss in it. Physical properties of aggregate like size, shape, texture affect workability in major way.

Unit weight of NFC is imperative instrument which helps in deciding the plastic state. There are no standard methods available to determine the unit weight; moreover, the unit weight is one of the best available measures for quality control of NFC mixtures. It is common to compact NFC by jigging in accordance with ASTM C29 [20] and the National Ready Mix Concrete Association (NRMCA) [60]. In a study by Tennis et al. [61] reported that field practices can have the acceptance of a NFC mixture of unit weight 80 kg/m³

or +/- 5% tolerance factor is acceptable for field practices. The effect of two methods of compaction namely rodding and jigging on unit weight were also analyzed. Neptune and putman [19] observed that jigging resulted in higher unit weight for all the mixtures within 9.5 mm gradation and mixtures with Cu closer to 3 and above. On the other hand, the rodding procedure produced higher values for the mixtures with lower Cu values in the 12.5mm and 19.0mm. Also, Gaedicke et al. [62] calculated the unit weight described in ASTM C1754 and used proctor hammer for compaction.

Unit weight and porosity are directly related to each other. As the porosity increases, density decreases and vice versa, also if the density of NFCs goes up, the porosity and water permeability goes down [63, 64]. Moreover, Murray et al. [65] reported that material properties are function of porosity of concrete mix. The final concrete unit weight is highly impacted by unrodded unit weight of aggregate. ASTM C1688 is commonly used to calculate the density and voids content in NFC. The effective voids present in NFC decrease significantly by increasing fine aggregate and water content, ultimately increases the average compressive strength [66].

5.2 Porosity/ Pore sizes

Porosity is also one of the main properties of NFC and directly affects the mechanical and durability properties [67]. In NFC the total porosity/void content can be determined as per ASTM C1688 [68]. It can also be measured by using the method developed by Montes *et al.* [69] conforming to ASTM C1754 [70] and this method is widely reported by many researchers [34, 51, 52, 62, 63, 71, 72] while several researchers [19, 72-75] have also used the method described in ASTM D7063 [76] to calculate the effective porosity. Zhong and wille [35] used difference between saturated and oven dried weight to calculate effective porosity. It was also reported that higher percentage of coarse aggregate resulted in a higher porosity [21]. This may be due to fact that void by FA cannot be filled by CA.

Deo and Neithalath [77] reported a reduction in porosity of the NFC mixtures with the increase in the compressive energy absorbed. As per Kim and lee [23], the total void ratios of specimen are more when smaller size aggregates are used. Moreover, Meulenyzer et al. [78] reported that paste volume and aggregate size have strong influence on pore parameters like pore size diameter and image analysis is helpful to understand the association between pore structure and permeability of the materials. The porosity is the decisive factor for quality control in pavement design and for material comparison. The error between different equipment's readings in separate labs but using similar Archimedes principle was 2.2% [69]. The porosity is an important parameter and mainly depends on field placement techniques so it was also recommended to calculate the porosity of field-placed specimens based on the aggregate size, core size and porosity [67]. A study also reported that porosity is greatly affected by recycle aggregate.

Bhutta *et al.* [79] in their study mentioned that the porosity is influenced by the addition of super plasticizer and a thickening agent for high performance NFC.

Vertical porosity distribution was also determined but earlier it was assumed to be constant along the depth [73, 80]. Their experimental investigation reported that the porosity increases significantly from top to bottom being lowest porosities in the top quarter, average porosities in the center half, and the higher porosities near the bottom.

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Various stereological and morphological techniques were used to determine the distribution of pores in NFC [55]. Lian *et al.* [81] proposed mathematical model that provide relation between porosity and compressive strength. Zhong and wille [82] reported the pore size distribution derived by linear path function follows image analysis correctly.

the mathematical model to predict compressive strength based on porosity.

Many researchers are delved into evaporation rate studies from different types of pavements. H. Li *et al.* [85] provided an easy technique to determine evaporation rate with adequate size of pore and capillary effect for different types of pavements. It was reported that to increase the evaporation and cooling effect,



Figure 4: Images of 2D sections of NFC specimens made with: (a) 100% 4.75mm(# 8); (b) 100% 9.5mm(#4); and (c) 100% 12.5mm(#3/8) aggregates (black spots are the pores)[83].

Deo et al. [83] found reduction in porosity with an increase in the compressive energy absorbed by compactive efforts in NFC mixtures. The size of pores increase with increase in size of aggregate. The mixes can have the similar porosity or identical porosity regardless of the nominal maximum size of aggregate used. The size of pores will increase with large size aggregate (12.5) (Fig. 4) and large no of small pores in small size aggregate (4.75mm) and combination of large and small pores in (9.5mm). Cosic et al. [21] reported that to produce large size pores, large aggregate could be used resulting into higher value of overall porosity but it also reduce the chance of clogging. Large amount of smaller size fraction (4-8 mm) increase the density and strength properties in PC. Lian and Zhuge [84] developed mathematical model to correlate compressive strength and porosity and studied through empirical and theoretical derivations. Experimental data was in good agreement with the pavement should have sufficient permeability and the evaporation rate was 0.1-0.3mm/hr although earlier Syrrakou *et al.* [86] reported evaporation rates of magnitude (10⁻² mm/h).

6. Hardened properties of NFC

6.1 Compressive strength

The compressive strength is inversely correlated to permeability [34, 57, 132]. It is also observed that the compressive strength of the NFC increases linearly with the increase of the tensile strength [30]. It has been observed that addition of small amount of sand was efficient in increasing mechanical property [2, 6, 92]. It is also reported that addition of sand and latex enhance the mechanic strength but reduce permeability of NFC [131]. The mixes with only sand, had higher increase in strength than the mixtures with sand and latex.

As the aggregate size increase, the mechanical strengths decline, but these differences were not statically significant $\lceil 57, 132 \rceil$. The mechanical strength is strongly related to mix proportion $\lceil 23 \rceil$ and porosity of NFC $\lceil 140 \rceil$. Shu et al. [62] reported higher compressive strength using limestone aggregate and incorporation of latex. Also, reported by H. Wu et al [64] adding latex desirably improved the strength whereas addition of fiber did not had visible effect on mechanical properties of NFC. Huang et al. [42] mentioned in their study that addition of polymer, sand, fiber enhance the mechanical strength. Giustozzi [87] mentioned in their study that polymer modified mixes showed delayed curing but the mechanical strength is significantly improved. It was also observed that NFC reached 80-90 % of compressive strength after 7 days of curing as observed after 28 days of curing [88]. Widely reported by many researchers [52, 89, 90] that the increase in paste volume resulted in improving the mechanical properties regardless of aggregate size and for a given paste volume the use of lower maximum size aggregate resulted in higher strength values. Deo and Neithalath [83] used image analysis method to study material structure and compressive response. The result indicate that using the large size aggregate and increase in paste volume content in NFC are observed to be increase the compressive strength and it is mainly influenced by the pore sizes, their distribution and spacing. Moreover, small size fraction of aggregate produce small size pores in NFC [90]. Many researchers have reported that higher compressive strength could be achieved for mixtures containing smaller size aggregate [91, 21, 35, 32,59,92] and increase in cement paste [30,52,89]. It is also observed that compressive strength increases with decrease in porosity $\lceil 23 \rceil$. Also, compressive strength of 35 MPa was reported by Chang *et al.* [93] using electric arc furnace slag and alkali activated slag cement.

Suozzo and Dewoolkar [94] investigated the effect of sulphur mortar capping and elastomeric pad capping on the compressive strength measurements and found that there is no statically significant difference in compressive strength measurement. Rehder et al. [95] and Huang et al. [42] from their study reported that fibers generally not found to influence the compressive strength to any significant degree, as is expected for conventional concretes also. Among the pore structure features, porosity exerted the maximum influence on compressive strength. However, Rangelov et al. [96] did air curing followed by wet curing and stated that 28 days compressive strength improved. More than 28 days moist curing did not gave significant gain in strength.

Attempts have been made to make the NFC using locally available coarse aggregates i.e. 1st class brick aggregate, crushed stone aggregate and recycled brick aggregate and found that NFC with compressive strength range from 4.5 to 11.72 MPa and permeability from 60 to 15 mm/sec can be made [97]. Wu *et al.* [98] made permeable bricks using incineration bottom ash and found to achieve 20 MPa of compressive strength. From the experimental work it was observed that the permeability coefficient was low as compared to the traditional bricks. Kevern *et al.* [99] studied 17 different types of aggregates and showed corresponding strengths.

Bhutta *et al.* [28] from their experimental investigation reported the reduction in compressive strength using recycle aggregate, but the compressive strength significantly increase due to polymer modification for both normal and recycle aggregate. It is believed that the addition of polymer have improved the internal cohesion

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and water retention between cement matrix and aggregate and increased the bonding force between neighboring aggregate particles. Gaedicke et al. [62] found out that compressive strength of RCA found to be 8% lower than pea gravel and 15% lower than limestone aggregate for porosity of 20%. Sata et al. [100] used crushed structural concrete and crushed clay bricks aggregates (both are recycle aggregates) to make geopolymer concrete and analyzed that these can be used but strength loose significantly. Although Compressive strength greatly affected by RCA (92). Moreover, Nguyen et *al.* [41] reported that by partially replacing natural aggregates with sea shell by products, a compressive strength of 15 MPa could be achieved.

Hence, it can be summarized that the strength of NFC can be increased (with compromise in permeability) by factors such as paste volume, small size aggregates, addition of sand, mineral admixture and mix design. Variation of compressive strength with porosity by using natural aggregate from 11 studies is shown in Fig. 5 and by using recycle aggregate is shown in Fig. 6.



w/c- 0.3- 0.42

Figure 5: Variation in 28 days compressive strength v/s porosity reported in previous studies using natural coarse aggregate



Figure 6: Variation in 28 days compressive strength v/s porosity reported in previous studies using recycle coarse aggregate

6.2 Split tensile strength and flexural strength

Crouch et al. [57] mentioned that the strength properties are related to of mixture proportions and are more sensitive to a/c ratio than w/c ratio. Several past investigations have been made on split tensile strength and flexural strength [19, 52, 63, 71]. Shu et al. [38] reported higher split tensile strength using limestone aggregate and incorporation of latex. Neptune and Putman [19] studied fifteen different aggregate gradations to determine the effect of aggregate on NFC properties. The result indicates that split and flexural strength decreased with single size aggregate gradation as the nominal maximum aggregate size increased. In addition to this Maguesvari and Narasimha [44] from their experimental investigation found that the flexural strength and split tensile strength increases with increase in percentage of fine aggregate. Gesoglu et al. [101] studied the effect of rubber aggregate on NFC and found the reduction in split tensile strength mainly due to isolation of aggregate by rubber aggregate from cement paste. The flexural strength of range between 1 to 3.5 MPa (28, 34,79). Multiple equation exist from past studies correlating splitting tensile strength, flexural strength and compressive strength for NFC as provided in Table 3; however, very few have developed equation which relate the two properties correctly. These authors have provided a very limited number of specimens for analysis and have taken into account the porosity variation. Although, the behavior of split tensile strength is similar to compressive strength ant its variation with porosity is shown in Fig. 7.

Endeavors have been put on flexural strength to exhibit the effect of recycle aggregate and polymer modification on NFC. The result have shown that the use of recycle aggregate

Table 3: Proposed equ	ation for relationship bet	ween compressive	e strength,	split tensile
str	ength and flexural stren	gth for concrete [[9]	

Reference	Proposed equation	Observation
Delatte <i>et al.</i> [102]	$ \begin{aligned} f_{st} &= 5e^{-0.0522v} \\ fc &= 62e^{-0.0677v} \end{aligned} $	Correlating f_{st} and f_c to the void content of the concrete.
Crouch <i>et al.</i> [103]	$f_{st} = 0.28 \ (fc)^{0.55}$	For NFC, utilized gravel and limestone
Ibrahim <i>et al</i> . [63]	$f_{\rm st} = 0.0478 + 0.1706 (f_{\rm c})$	For NFC, utilized crushed lime- stone
Gaedicke <i>et al.</i> [71]	$f_{st} = 0.181 \ (fc)^{0.875}$ $f_{st} = 0.153 \ (fc)^{0.979}$	For all types of aggregates and for effect of specimen type.
Neptune and Putman [19]	$f_{st} = 0.22 (fc)^{0.84}$ $f_r = 0.63 (fc)^{0.47}$	For NFC.
Ghafoori and Dutta [58]	$ \begin{aligned} f_{\rm st} &= 5.67 \ (f_{\rm c})^{0.5} \\ f_{\rm st} &= 5.9 \ (f_{\rm c})^{0.5} \\ f_{\rm st} &= 6.15 \ (f_{\rm c})^{0.5} \end{aligned} $	For NFC at 28, 60 and 90 days of curing respectively.
ACI 318 [104]	$f_{\rm st} = 0.56 (f_{\rm c})^{0.5}$	For conventional concrete.
ACI 363 [105]	$f_{\rm st} = 0.59 (f_{\rm c})^{0.5}$	For high performance concrete.

decreased the flexural strength than that of normal aggregate but the polymer modification enhanced the flexural strength in all NFC [79]. Cosic *et al.* [21] found that flexural strength and modulus of elasticity were higher for dolomite aggregate than steel slag aggregate and increased with higher amount of small aggregate fraction. Brake *et al.* [37] results indicate the flexural strength depend on size in NFC than in conventional concrete [106]. Kevern *et al.* [107] reported addition of FA and 5% sand could increase flexural strength in NFC.

W.T Kuo *et al.* [27] reported that split tensile and flexure strength is 1:9 and 1:4 of the compressive strength by using MSWIBA. Chen at al. [106] reported that flexure strength is predominated by porosity than compressive strength in NFC.

7. Durability properties

Generally durability is defined as the surface life under given environmental conditions.

Exposure to chemicals like sulphate, acids etc. and adverse climate condition are the main factors which affects the durability. The microstructure of ITZ depends upon the physical properties of aggregate [108,109]. Cement matrix is the only contact between the aggregates strength and durability depends on it. It is well documented in the past literature that contact surface is the weakest point in the concrete from which the propagation of cracks starts. If the surface of aggregate is rough and porous, the bonds will be strong with cement paste [110]. In a study by Vancura et al. [39] Fig. 7, the behavior of NFC was tested as per ASTM C666 and ASTM C457 standard. The crack propagation starts from the contact area between the aggregate and cement paste by separating from surface as shown in Fig. 8.





W/c- 0.3- 0.42





Figure 8: Crack propagation primarily through the ITZ in NFC [39].



Figure 9: Air-entrained concrete [39] and without entrained air bubbles

7.1 Freeze thaw resistance

The resistance to Freeze-Thaw (F-T) cycle is measured as loss in mass after successive number of F-T cycles. The ASTM C666 is used to measure F-T resistance due to lack of standard laboratory practice and more appropriate test is required to assess the durability under F-T condition for NFC [111]. Aggregate properties have the greatest effect on F-T durability. Many experimental studies has reported that use of air-entering admixture improve the F-T durability [38, 39, 58, 99]. Sample field were studied in order to evaluate the F-T. Fig. 9 shows NFC with and without air entering admixture. It was also found that the NFC with w/c below 0.4 may not require entraining bubbles as the paste matrix provides better resistance against F-T cycles. NFC mixes with aggregate having higher specific gravity also increase the resistance against F-T [99, 81]. Yang & Jiang [32] reported that addition of SF and SP increase the F-T resistance. In addition to this addition of small amount FA and synthetic fiber also increase F-T resistance. [112]. Yang et al. [113] reported that partially saturated samples compared to submerged samples have improved F-T resistance, although salt application increase the disintegration and addition of polypropylene fiber improve the resistance against F-T cycles [33].

7.2 Abrasion resistance

Raveling remains one of the prime challenges towards broader application of concrete technology [114]. Portland cement NFC have interconnected voids providing high porosity but at the same time decreasing strength and abrasion resistance.

Several studies have shown that small sized coarse aggregate, fine aggregates, polymer and other chemical admixture improve NFC properties [35, 32, 115]. Spalling and raveling are attributed as two biggest reason of abrasion are tested by cantabro test, loaded wheel abrasion test and surface abrasion tests ASTM C944 [64,116]. Addition of latex enhances the strength in abrasion.

Similarly, Shu *et al.* [38] also came up with the same investigation by using cantabro method to study abrasion resistance with latex. Incorporating RCA and GGBS did not reduce the abrasion resistance [22]; However, Dong *et al.* [115] found that surface abrasion test fail to distinguish between latex and fiber incorporated NFC but successful in control mix. Chen *et al.* [117] studied with help of accelerated abrasion test and inferred that NFC pavement is more durable than normal concrete. The results may vary if the cement type, coarse aggregate type, gradation and mix proportion change. Kevern *et al.* [107] found the better abrasion with surface cur-

ing compound but the best resistance against abrasion was obtained with soya bean oil as curing compound.

7.3 Fracture toughness

The incorporation of fiber can be selected to increase toughness. In accordance with ASTM C1399 [118], addition of fiber of length 1.5-2 inch provide extra toughness to the concrete. Chen et al. [106] reported that the polymer modification increases the concrete resistance to cracking. This happens due to particles polymerizing in the ITZ region by developing strong bond between cement paste matrix and aggregate. Brake et al. $\lceil 37 \rceil$ found that the fracture toughness also to be dependent on size of specimen. Gesoglu et al. $\lceil 101 \rceil$ mentioned in their study that fracture toughness increases when replacing fine aggregate & coarse aggregate with crump rubber and tire chips.

Rehder *et al.* [95] found that the fracture toughness was primarily dependent on the porosity, even though toughness was found to increase with increase in fiber volume fraction. Gain in pore size; reduce the toughness value for a specific porosity. Applications of tire chips augment the fracture energy but shows reverse results for crumb rubber.

7.4 Flexure fatigue

Fatigue strength of a concrete material is determined by a two parameter Weibull probability function. Chen *et al.* [33] from their investigation stated that flexure fatigue test shows that Polymer modified NFC has far longer flexure fatigue life than supplementary cementitious modified NFC at all stressed level, as polymer helps in reduction of cracking or delay the cracking generation; moreover the fatigue life decreases with the increasing porosity and the stress level sustained by these specimens.

7.5 Carbonation

NFC due to its macro structure is highly inclined to carbonation and long term preservation to embedded steel is not possible. The process may starts in few weeks after surface exposure. The electrochemical measurement exhibit that proliferation of corrosion would be high for material dipped in water [88]. Enough data is not available to understand the behavior.

7.6 Permeability

The main function of NFC is the ability to pass water through interconnected network of pores. As per ACI 522R-08 the permeability of NFC lies between (1.4-12.2) mm/s. The hydraulic conductivity mainly depends upon porosity and the pore sizes [40]. Tests show that a minimum porosity of approximately 15% is required to achieve significant percolation. The permeability is inversely proportional to the strength properties [19,59]. Permeability is significantly influenced by the porosity [4] and not by the RCA [92]. Regardless of aggregate type and polymer type the coefficient of permeability increase as the total void ratio increase for all porous concretes [28]. Huang et al. [42] reported decrease in permeability due to incorporation of latex and sand in NFC.

The permeability or Hydraulic conductivity is calculated from the well-known Darcy law [51]. The inertial and velocity effects were neglected in Darcy's law and these effects are prominent when the interconnected pores turn out to be larger in size. In order to take into account the nonlinearity, the Forcheimmer's equation and Izbash law/power law are useful [119, 120]. Kozeny gave an equation to calculate permeability which was a function of porosity and specific surface area (later modified by Carmen) [121].

Chandrappa and Biligiri [51] investigated to validate the Darcy's law and K-C

equation which provides correlation between permeability and pore structure features. Tortuosity and specific surface area are higher in smaller size aggregate and lesser in large size aggregate.

Later Sumanrooriya *et al.* [90] predicted permeability of NFC mixture through planar images of specimen and through experimental investigation and reported that they were in good agreement with each other. Since clogging is biggest threat for permeability, Alamd and Haselbach [122] studies different specifications and designs to make clogging resistant NFC.

The permeability of NFC can be measured by a simple apparatus shown in Fig. 10. In the absence of standard test method, different researches have used different apparatus based on their understanding [123].



Figure 10: Falling head permeability apparatus [85]



Figure 11: Variation in water permeability v/s porosity from previous studies [124]

From the trend obtained (Fig.11), it is seen that permeability increases with increase in porosity. It is also reported that the permeability of concrete decreases with the increase of RHA amount until the optimum RHA amount is reached [34]. Also, constant head permeability appears to be a function of pasted drain down, effective air void content and void size in NFC specimen. The reserve of the falling head and constant head methods agree reasonably well for laboratory sample as widely reported [91, 63, 125, 101, 124]. In order to understand the microstructure, Bentz [144] through virtual model compared the percolation characteristics and transport properties of actual NFC and found to be closest to the actual PC. Also CT image (Computed tomography) and probalistic description method were utilized to analyze the microstructure and found that void distribution strongly affects strength and percolation [124].

8. Water purification characteristics of NFC

The runoff water carries different types of faecal matters, heavy and light chemicals,

soil, oil, organic-inorganic residential and industrial wastes [126- 132]. The traditional pavements favor smooth runoff of water and as a result, the pollutants get passed on to water sources. If such pollutants pass on to ground then it pollutes ground water and imbalance the ecological system. The NFC reduces the quantity of such runoff water as well improve the quality of absorbed water reaching ground water resources.

High thickness of NFC acts as better purifier [133]. In one of studies, concentration of lead was negligible and copper and zinc were less in NFC [134, 135]. Dissolved heavy metal concentration was found to be very less in NFC [136]. NFC could be a very good option in industrial area to capture heavy metals. Ketcheson *et al* [129] reported that NFC can increase chloride contamination in salt vulnerable areas. Geotextile separate cadmium, zinc and copper [137, 138].

Table 4 shows different avenues considered for the advancement of properties of NFC.

Authors	Proposed methodology	Significance
Bhutta <i>et al.</i> [79]	Three different aggregate sizes along with SP and	High performance porous concrete (HPPC) with blending of SP and cohesive agent.
	cohesive agent	Fair workability and strength properties.
		Addition of cohesive agent decreases the total void ratio and permeability.
Zhifu Yang [33]	Variation in w/c with different combination of	An increase in cement content slightly increased the F-T Resistance of water-cured NFC.
	silica Fume, fiber, AEA	Increasing w/c from 0.25 to 0.35, increase in F-T resistance of both water and air cured specimen observed.
	Silica fume was observed to help increase the F-T resistance of water-cured in the presence of deicing salts whereas no significant effect	
		of polypropylene fibers.

Table 4: Different avenues considered for the advancement of properties of NFC

	Table 4 [Continued]			
Li <i>et al.</i> [139]	Silica fume, fly ash (se- lected as reactive powder), U-type expansion agent	High Strength NFC with Optimum mix pro- portion of Reactive powder Compressive strength greatert than 69 MPa		
	aluminum oxide and alumi- num potassium sulfate)	and flexural strength greater than 10 MPa with a 13.02 mm/s permeability coefficient.		
Shen <i>et al.</i> [140]	Application of TiO2 onto the surface of NFC	Excellent pollutant removal properties.		
Tho-in <i>et</i> <i>al.</i> [141]	(FA), Na₂SiO₃, NaOH solu- tion and coarse aggregate	Acceptable mechanical properties.		
Nguyen <i>et</i> <i>al.</i> [26]	60% mass of the natural aggregates was replaced by crushed seashells and	The crushed shells have a lower mechanical strength and freeze/thaw resistance than the control NFC.		
	about 6% of sand is used to make shell NFC	Significant decrease in permeability on loading with mixture of silty clay and sand.		
Murray et al. [65]	Chitosan (waste from sea food industry) and tire crumb.	Similar properties High sorption capacity.		
Sata <i>et al.</i> [100]	NA, RCA with fly ash (FA), (Na2SiO3) & (NaOH)	Both Recycle aggregate from Concrete and clay Bricks could be used.		
	solution	Compressive strength of 10.3 MPa with recy- cle aggregate produced.		
Zhong & Wille [45]	SF, silica powder and ag- gregates with 99% content of SiO_2 along with high range WR	HPPC made with appropriate aggregate size and a/b ratio.		

9. Summary and conclusion

The review paper summarizes the detailed study on various parameters depicting the properties of NFC and its vital use for infiltrating water into the ground. It can be inferred that admixture such as RHA, FA, SF when added to concrete, have potential to increase mechanical and durability properties of NFA. As a result of which the microstructure of cement matrix will be modified in order to enhance the quality of concrete as mentioned in the above section. It has been observed from the previous studies that the effect of w/c ratio is negligible on the strength of NFC which mainly depends on the mixed variables that are function of aggregate cement ratio. The resistance against aggressive chemicals has not been reported in past studies; hence it is required to investigate the pavement considering the long-term effect. The literature review reveals that a very few studies have been reported on carbonation of NFC.

So, it can be concluded that the proper selection of mix variable is a major concern which dominates the strength property of NFC. In India, its testing is limited to R&D labs only and practical application is still not on the ground. Regular maintenance of NFC pavement is required for the proper

functioning. More practical research need to be conducted for the efficient use of NFC in pavements. A few of the future research aspects are described as following:

Research Scope in the field of NFC

- The knowledge of perdurable behavior of NFC using binary and ternary combination of mineral admixtures is relatively unknown.
- Investigation of mechanical properties and durability of no fine concrete on long term basis.
- Carbonation of NFC is not well defined.
- The response towards diverse cement blends on mechanical and durability properties of NFC need to be further probed.
- Optimization of mineral admixtures in mix design of NFC.
- Established mix design for the application on low volume traffic as well as for high volume traffics pavements.
- Standardized method to determine the porosity and permeability of no fine concrete.
- The durability property such as resistance against salts, Sulphate and acids is not studied.

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