University of Southern Queensland Faculty of Health, Engineering and Sciences

# Improving infiltration modelling for crusting soils

A dissertation submitted by

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#### Abstract

Infiltration and surface runoff modelling is important in many disciplines including environmental engineering, mine site rehabilitation, ecology and agronomy. Major errors in modelling can occur when the throttling effect of soil surface crusts, which reduce infiltration and subsequently increase surface runoff, are not considered. The aim of this project was to improve the modelling of infiltration on crusting soils by measuring the density of the soil surface crust.

A rainfall simulator was used to create a surface crust on soils (Sodosol and Chromosol) susceptible to surface crusting. The surface crusts resulted in greater than 90 per cent of applied rainfall becoming runoff. Several methods of measuring the crust density were trialled, with X-ray micro Computed Tomography (X-ray CT), when combined with the traditional soil core method, found to be the most accurate and reliable.

The HYDRUS-1D software application was used to model infiltration. Measured soil parameters, including crust density, and applied rainfall rates were used as model inputs. The inclusion of crust density into HYDRUS-1D resulted in insignificant improvements to modelling accuracy. Inverse modelling identified that this was as a result of HYDRUS-1D predicted saturated hydraulic conductivity values being three to four orders of magnitude larger than obtained from the inverse solution. The application of average crust hydraulic parameters, obtained from the set of inverse solutions, was found to provide a close approximation of infiltration rates— once surface runoff had commenced — and cumulative infiltration.

The findings of this project indicate that the use of average surface crust hydraulic parameters could provide major improvements in infiltration modelling accuracy using HYDRUS-1D without the requirement for additional sampling and analysis during field surveys. Further experimentation on a broader range of soils under differing vegetation regimes is required to validate the conclusions of this project.

# University of Southern Queensland Faculty of Health, Engineering and Sciences ENG4111/ENG4112 Research Project

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#### SYMBOLS AND ABBREVIATIONS

Table 1 details the symbols and abbreviations that have been used in this project.

Table 1 Abbreviations		
Description		
Bulk Density		
Matric potential		
Moisture content		
Saturated hydraulic conductivity		
Porosity		
Computed Tomography		
Hydraulic Conductivity Function		
Measuring Cylinder		
Pedotransfer Function		
Soil Water Characteristic		
Van Genuchten		

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#### 1. CHAPTER ONE – INTRODUCTION

I love a sunburnt country, A land of sweeping plains, Of ragged mountain ranges, Of droughts and flooding rains. Dorothea Mackellar, My Country Mackellar (2016)

All models are wrong, but some are useful.

George Box (Box & Draper 2007)

Managing the flow and storage of water, whether for flood mitigation, irrigation or provision of drinking water, has been of great importance since at least Sumerian times (Sprague De Camp 1963). Both ancient and contemporary civilisations have invested significant effort in managing their water resources. For Australia, a land of weather extremes, as famously noted by Dorothea Mackellar, managing water resources remains of particular importance. Modelling is one of the primary tools that support the management of water resources.

Until the mid-nineteenth century experientially based rules of thumb and large factors of safety were used to guide water management requirements (Linsley et al. 1992). These approaches have been replaced over the last 150 years by scientific/engineering processes. The prediction of infiltration and surface runoff, critical in water engineering, mine-site rehabilitation, agricultural and environmental disciplines, has become increasingly sophisticated. A range of empirical and physically based models have been developed to assist in the prediction of infiltration and runoff. Some of many models include Darcy's law (Darcy 1856), the Green and Ampt equation (Rawls et al. 1990), Horton's equation (Horton 1941) and the Philip equation (Philip 1954). The 21st century has seen further advances through the application of advanced computing, Geographic Information Systems (GIS) and remote sensing to improve watershed modelling (Migliaccio & Srivastava 2007).

Despite the significant advances in modelling infiltration and surface runoff, the spatial and temporal variability of precipitation and soil still result in model error, confirming George Box's observation that 'all models are wrong.' One of the soil based sources of modelling error is the impact of surface crusts. Surface crusts are thin compacted layers of soil that have higher bulk density and reduced porosity compared to the underlying soil. Surface crusts act as a throttle to infiltration and thus increase surface runoff. When predictive models fail to consider the impact of the surface crust on infiltration this reduces the accuracy of model predictions.

This project seeks to improve the accuracy of infiltration modelling, and subsequently the prediction of surface runoff, by incorporating the density of the surface crust into the commonly used infiltration modelling application, HYDRUS-1D.

#### 1.1. Project aim

The aim of this project is to improve the modelling capability for infiltration and surface runoff on soils that form a surface crust.

#### **1.2. Project objectives**

The objectives required to achieve the aim are as follows:

- 1. Review the literature on modelling infiltration and surface runoff on crusting soils.
- 2. Use previous experimental data to determine the potential contribution of surface crust density to model error.
- 3. Identify and/or develop a reliable method of collecting surface crust/seal samples for the purposes of accurately calculating crust bulk density and/or porosity.
- 4. Optimise an infiltration and/or surface runoff model that incorporates surface crust parameters such as bulk density and porosity.
- 5. Validate whether incorporation of soil crust parameters into the infiltration/surface runoff models improves model accuracy.
- 6. Make recommendations on changes to current soil survey processes to improve infiltration and surface runoff predictions.

The project specification can be found in Appendix A.

#### **1.3. Dissertation overview**

#### **1.3.1. Literature review**

This chapter presents the background literature relevant to this project including sections on infiltration, surface crusts, related soil properties, infiltration modelling and pedotransfer functions (PTF). The chapter concludes by identifying the knowledge gap.

#### 1.3.2. Modelling

This chapter details the modelling completed as part of the project. The modelling includes investigating the impact of various parameters and soil crust profiles on infiltration modelling using HYDRUS-1D using arbitrarily selected input data. Subsequent sections include the modelling of the Soil Water Characteristic (SWC) as  $\rho_b$  increases using externally sourced observed water retention data and the RETC and HYDRUS-1D software applications to develop a PTF. The chapter concludes by modelling infiltration as soil  $\rho_b$  increases.

#### 1.3.3. Methods and materials

This chapter explains the experimental methodology applied to address the projects aims and objectives.

#### 1.3.4. Results

This chapter details the results from the various experimental activities completed during the project including. This includes both the results from the physical experiments required as input data for subsequent modelling and the results of the modelling using HYDRUS-1D.

#### 1.3.5. Discussion

This chapter draws together the key observations and findings from the initial modelling activity and the experimental phase of the project. Conclusions are drawn on whether measuring the  $\rho_b$  of the surface crust improves the accuracy of infiltration modelling.

#### **1.3.6.** Conclusion

This chapter summarises the major findings of the project and a final evaluation of the results against the project aim.

#### 2. CHAPTER TWO – LITERATURE REVIEW

#### 2.1. Introduction

The aim of this chapter is to review the literature related to infiltration/surface runoff and surface seals/crusts. The literature review consists of eight sections. The first section provides a general overview of infiltration and surface runoff. Section two defines surface seals/crusts and associated characteristics. Section three describes soil physical and chemical properties that influence, or are influenced by, surface sealing/crusting. Relevant soil hydraulic properties are defined in section four. Section five reviews some of the commonly used approaches to model infiltration on soils affected by surface seals/crusts. The sixth section describes modelling approaches to infiltration relevant to this project. Section seven introduces Pedotransfer functions (PTFs) whilst the final section defines the knowledge gap and draws conclusions relevant to this project.

#### 2.2. Section One - Infiltration and Surface Runoff

#### 2.2.1.Infiltration Overview

Infiltration is defined as the movement of water through the soil surface and into soil (Linsley et al. 1992). Infiltration capacity is the 'maximum rate at which a given soil when in a given condition can absorb rain as it falls' (Horton 1933, p. 399). For a soil's infiltration capacity to be reached requires water to be ponded on the soil surface otherwise water will not always be available at all times for infiltration. The infiltration capacity starts with a high rate and decays to a steady level which in many soils approximates the saturated hydraulic conductivity ( $K_s$ ) of the soil as shown in Figure 1. The shape of this curve will vary depending upon factors including the soil porosity, colloidal properties and the antecedent moisture content (Turner et al. 1984).





The infiltration rate is the actual rate or flux of water passing through the soil surface at a point in time. The maximum possible infiltration rate at a specified time is equal to the infiltration capacity at that time. Often, under light rainfall for example, the infiltration rate will be less than the infiltration capacity. Both infiltration rate and infiltration capacity use the dimension of length per unit time  $(LT^{-1})$ .

Cameron Leckie ( ENG4112 Research Project The hydraulic properties of the soil surface control the portioning of precipitation between infiltration and surface runoff (Badorreck et al. 2013, p. 1) which will now be described.

#### 2.2.2.Surface runoff overview

Water falling as precipitation follows several paths (Linsley et al. 1992). Some of this water is intercepted by vegetation (interception) and does not reach the ground. Some of the water is retained in depressions (depression storage) on the soil surface forming puddles. Much of the water is infiltrated into the soil as previously identified. Evapotranspiration results in some water returning to the atmosphere as water vapour. Surface runoff (or overland flow) results when the intensity of the precipitation is greater than the infiltration capacity of the soil or the soil surface is saturated. A summary of the physical processes involved in runoff generation is detailed in Figure 2.



Figure 2 Physical processes involved in runoff generation (Tarboton 2003)

The measurement of surface runoff and infiltration is required in many fields including hydrology, ecology, agriculture and civil engineering. Common methods of measuring infiltration and surface runoff will now be reviewed.

#### 2.2.3.Measurement of infiltration - infiltrometers

Physically based models of infiltration and runoff processes require the ability to obtain accurate estimates of soil hydraulic properties, such as sorptivity (S) and hydraulic conductivity (K) (Latorre et al. 2013, p. 581). Infiltrometers are devices that can be used to directly estimate soil hydraulic properties.

There are multiple types of infiltrometers such as single ring, double ring and tension (also known as disc permeameters) infiltrometers. Soil hydraulic conductivity is often determined in situ using double-ring infiltrometers (Lai & Ren 2007). These consist of two concentric metal rings that are inserted into the soil surface. Both the inner and outer rings are filled with water with the water level being measured from which the infiltration rate and saturated hydraulic conductivity can be

determined. Tension infiltrometers consist of a disc base in contact with the underlying soil connected to a graduated water-supply reservoir and a bubble tower. The soil hydraulic properties are calculated from the cumulative infiltration curves which are measured from the drop in the reservoir level (Latorre et al. 2013).

Despite intensive research over time, a large variability in results, particularly for  $K_s$ , is a common occurrence. As an example, Lai and Ren (2007) report that  $K_s$  can change by an order of magnitude or more within a short distance. Factors leading to this variability include spatial variability and heterogeneity of soils, measurement errors (Lai & Ren 2007) and the calculation method used (Verbist et al. 2010).

#### 2.2.4.Measurement of infiltration – rainfall simulators

Rainfall simulators have been used successfully for over 90 years to conduct research on infiltration, surface runoff, soil erosion (Humphry et al. 2002) and soil crusting (Abudi et al. 2012). An advantage of rainfall simulators is their ability to create controlled and repeatable artificial rainfall. This allows for expeditious data collection, particularly when compared to natural rainfall, and comparisons between different soil types and soil management approaches (Humphry et al. 2002). Rainfall simulators consist of a method of delivering simulated rainfall (either via a nozzle or drop forming tubes (Ogden et al. 1997)) onto a plot of specified dimensions and a mechanism to collect and measure the runoff from the plot (Bertrand 1965). Rainfall simulators also have the ability to specify rainfall characteristics including drop–size distribution, raindrop velocity and kinetic energy, rainfall intensity and uniformity and whether rainfall is continuous or not (Humphry et al. 2002). Infiltration is determined by calculating the difference between the known application rate and runoff using a water balance approach.

Rainfall simulators, like infiltrometers have disadvantages. For example many rainfall simulators have difficulty in applying an energy flux to the soil similar to that of natural rainfall which is a major disadvantage for runoff studies (Abudi et al. 2012).

#### 2.2.5.Measurement and modelling of surface runoff - catchment scale

Rainfall simulators and infiltrometers provide measurement of hydraulic variables at a point/small plot scale. Due to the heterogeneity of soils and surface conditions (Blöschl & Sivapalan 1995), both because of the nature of the soil itself (e.g. mineral composition, texture and structure) and macrostructures (such as root systems, root perforations, sun-checks, earthworm perforations and other biologic structures) (Horton 1941) point values maybe inaccurate by orders of magnitude if applied at field and/or catchment level. In most cases the ability to collect a statistically valid number of point measurements is unachievable. The use of sub-catchment/catchment level hydrologic rainfall-runoff models offers a method to address the deficiency of point measurements for hydrologic studies.

There are currently hundreds of hydrologic models based on catchment water balance methods in use (Boughton 2005). The Conservation of Mass principle is adopted in these models (accounting for all water entering, leaving and stored in a catchment) which limits the potential for error (Boughton 2005). Infiltration and other water losses (e.g. vegetation and depression storage) are treated in a simplified manner using conceptual loss models such as Initial Loss – Continuing Loss (Hill et al. 1998). Whilst these models may not be theoretically correct, for catchment level hydrologic modelling purposes they are sufficiently accurate. One study compared the use of a point infiltration equation (Green & Ampt) against simplified catchment level conceptual loss models. The results indicated that on average the results using the simplified loss models to the point infiltration equation were superior (Hill et al. 1998).

#### 2.3. Section Two - Surface Crusts

#### 2.3.1.Description

Under some conditions soils form a seal (seals develop in wet soil) or crust (crusts develop as the soil dries) at the soil surface. There is much debate and sometimes contradictory/interchangeable use of these terms in the literature however it is generally agreed that a seal relates to the wet soil state and a crust the dry soil state (Nciizah & Wakindiki 2015). This terminology will be used throughout this project.

A soil crust is a layer of increased  $\rho_b$  of the top few millimetres of soil (Singer & Munns 2006). The International Society of Soil Science defines a soil crust as thin surface layer which is much more compact, hard and brittle when dry than the material immediately underneath the crust with a thickness of up to three centimetres (Nciizah & Wakindiki 2015). Additionally crusts exhibit reduced porosity and changes to pore size distribution (Bajracharya & Lal 1999).

Surface crusts significantly reduce the infiltration rate and cumulative infiltration (acting as a throttle on infiltration), decrease water storage in the underlying soil, increase runoff volumes (and thus soil erosion) whilst reducing or preventing seedling emergence (Valentin & Bresson 1992). Loch (1989) describes the behaviour of the surface seal under rainfall as a rigid, impenetrable surface.

The significance of the negative impact of surface seals is demonstrated by Moore and Eigel (1981) whose research found that both the infiltration rate and cumulative infiltration can be reduced by up to 80 percent. Other studies show results of similar magnitude. Whilst the impacts of soil crusts in agricultural settings are primarily deleterious, crusts also play an important ecological role including stabilising soils, regulating the balance between infiltration and runoff as well as fixation of carbon and nitrogen (Read et al. 2008).

It should be noted that the scientific study of the uppermost surface layer of the soil is much younger than the study of soil science; as a result general agreement on terminology, definitions and classification systems is still a work in progress (Patrick 2002).

#### 2.3.2.Types

Patrick (2002) classifies three major crust types being physical, chemical and biological. Physical crusts, which Patrick (2002) argues should be described as physico-chemical crusts due to the interaction of soil and water physical and chemical properties, are formed when the soil surface is reorganised due to the kinetic energy imparted by raindrops. Chemical crusts, generally found in hot arid areas, are formed by the precipitation of salts. Biological crusts are formed by a variety of organisms including cyanobacteria, green algae, mosses, bryophytes, lichens and fungi and are prominent in arid and semi-arid ecosystems globally (Read et al. 2008). This project focuses on physical crusts, as such biological and chemical crusts will not be considered any further.

Valentin and Bresson (1992) classified three classes of physical crust (each with sub-classes) being structural, erosion and depositional. Structural crusts are formed by the in situ rearrangement of soil particles; depositional crusts are formed by the micro-bedding (sorting, packing and orientation) of both coarse and fine particles; erosion crusts consist of a thin single, rigid and smooth surface layer of fine particles. Moss (1991) describes two physically distinct crust types. The first comprises of a thin 'skin seal' overlying a 'washed in layer' that form where ponding occurs under turbid water conditions. The second, which aligns with Valentin and Bresson's structural crust, comprises of a surface soil layer that has been compacted with little to no addition of surface solids. Above this layer, in patches, can form thin densely packed layers of silt. Moss describes this type as a 'rain-impact soil crust is the focus of this project.

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#### **2.3.3.Crust formation**

The formation of a surface crust/seal is a complex process. Three different descriptions of seal/crust formation are provided, each of which assists in developing an understanding of crust formation.

Bajracharya and Lal (1999) identified five overlapping and simultaneous sub processes that occur during soil crust formation. These are (1) aggregate disruption and slaking, (2) void filling and illuviation, (3) compaction and particle rearrangement, (4) smoothing and lowering of the surface and (5) drying and consolidation.

Loch (1989) describes the visually observable formation of surface seal formation, for an initially dry soil, as having three stages, being:

- 1. Penetration of raindrops into the soil forming large craters. The soil surface is wetted and aggregate breakdown commences
- 2. The soil surface flattens, rather than being cratered, with free water becoming visible on the surface
- 3. The soil surface becomes distinctly flat. Surface ponding occurs as the infiltration rate is reduced.

Moss (1991) identified three temporal stages in the formation of a surface seal. During Stage One (which only occurs on dry soil) raindrops and lateral outflow from raindrop impact enter the soil immediately by hydraulic penetration with little soil disturbance. Stage Two sees less water entering the soil directly, rates of lateral outflow increasing and surface water taking longer to enter the soil. Craters are formed penetrating up to 5 mm below the soil surface. Air splashing of soil particles becomes intense and aggregate breakdown rapid. During Stage Three the craters become shallower and eventually the surface develops a planar geometry.

Seal formation can occur quickly and with little rain. On a pre-wetted soil, Moss (1991) using rainfall of 40 mm/h intensity, found that the transition from Stage Two to Three had commenced after one minute of rain (0.67 mm of rainfall applied) with a fully developed rain-impact soil crust becoming fully developed between eight (5.3 mm rain) and 16 minutes (10.7 mm rain) of rainfall commencement. A more recent study (Armenise et al. 2018) found that less than nine minutes of rain was required to develop a seal greater than 2.5 mm in thickness in three different soils seeing infiltration being reduced by 60 per cent.

Whilst the physical processes leading to surface seal formation are not directly important in this project, the time required to form a seal and the associated rainfall intensity are important for informing experimental design.

#### **2.3.4.Crust thickness**

A wide range of crust thicknesses are reported in the literature. Bajracharya and Lal (1999) describe a crust from being a few millimetres to a few centimetres thick whilst Valentin and Bresson (1992) identified erosion crusts that were well less than a millimetre thick.

Crust thickness is of importance in many hydraulic models. For example Loch (1989) provides a version of the Darcy equation relevant to a surface crust (Equation 1) where, K is the hydraulic conductivity (LT<sup>-1</sup>), i is the infiltration rate (LT<sup>-1</sup>),  $H_c$  is the pressure head at the bottom of the seal (L), and  $L_c$  is the thickness of the seal (L). The denominator is the crust depth implying that relatively small inaccuracies can result in significantly different values for hydraulic conductivity.

$$K = i \frac{H_c}{L_c}$$
 Equation 1

Until relatively recently visual observation methods using tools such as Vernier callipers or microphotographs have been used to measure crust thickness (Armenise et al. 2018). These methods can be imprecise and thus error prone. X-ray techniques (Valentin & Bresson 1992; Bresson et al. 2004) have been used to characterise crust  $\rho_b$  (from which crust thickness can be determined). X-ray Computed Tomography (CT) has also been used to enable a non-subjective assessment of seal formation and depth (Armenise et al. 2018).

There are a number of difficulties in determining the thickness of the surface crust. The first difficulty is that whilst some transitions from the crust to the underlying soil are abrupt (providing a clearly defined surface crust), some crusts are gradual, increasing the difficulty of determining the actual depth of the crust (see for example Roth (1997)). A second difficulty is that the surface crust can be spatially variable even over small distances (Loch 1989). This has led some researchers to generalise the depths of surface crusts.

#### 2.3.5.Soils prone to crusting

Soil surface seal/crust formation is influenced by many factors. These include soil texture, aggregate stability, organic matter, tillage practices, cropping history, cultivation methods, rainfall intensity and duration (Moore 1981; Moore & Eigel 1981). Cultivated soils are often particularly susceptible to crusting. This is due to cultivated soils not having any protection from the impact of rainfall which vegetation and surface covers provide and often having reduced levels of soil organic matter that aids in aggregate stability.

Each of the three previous soil crust formation descriptions identify soil aggregate breakdown as being a key factor in crust formation. A soil aggregate is defined as 'a group of primary soil particles that cohere to each other more strongly than to other surrounding particles' (Nimmo & Perkins 2002). Aggregate stability influences the physical behaviour of the soil and is thus related to infiltration and erosion. Soils with low aggregate stability are susceptible to crusting and erosion (Le Bissonnais 2016).

Le Bissonnais (2016) describes four major mechanisms that cause aggregate breakdown being (1) the slaking of aggregates pressure of compressed air due to wetting being greater than the aggregates mechanical strength, (2) differential swelling of clay particles, (3) raindrop impact mechanically breaking down aggregates and (4) dispersion by osmotic stress.

These mechanisms vary in multiple ways including the forces involved, soil properties that control the mechanism, the resulting fragments and the intensity of the disaggregation (Le Bissonnais 2016). Whilst many soil properties influence aggregate stability, three properties play a major role. These are the Exchangeable Sodium Percentage (ESP), iron and aluminium oxides and oxyhydroxides (sesquioxides) and organic matter (Le Bissonnais 2016). Sodic soils, soils with an ESP greater than 6 (Isbell 2016) tend to be dispersive and susceptible to crust formation. Sesquioxides cement soil particles together whilst organic matter protects the soil surface from the impact of rain drops and improves infiltration. Knowledge of these properties is required to assist in selecting appropriate soils for this project.

#### **2.3.6. Rainfall characteristics**

Rainfall characteristics play a major role in the formation of surface crusts. These characteristics include the kinetic energy of falling raindrops, cumulative rainfall depth, rainfall intensity and raindrop size (Nciizah & Wakindiki 2015).

The kinetic energy of a raindrop is calculated using Equation 2 where KE is the kinetic energy (Joules), m is the mass (M) and v is the velocity  $(LT^{-1})$ . Clearly velocity plays a major role in the kinetic energy of a raindrop. When a raindrop impacts the soil surface the absorption of the raindrops kinetic energy can break down soil aggregates into smaller aggregates or singular grains (Nciizah & Wakindiki 2015). Surface cover (e.g. plants and plant residue) can significantly reduce the velocity of raindrops and hence the kinetic energy impacting the soil surface resulting in much slower rates of surface sealing (Geeves 1997). Raindrop size (which influences raindrop mass) also impacts seal formation with larger drops forming deeper and denser crusts (Moss 1991). Rainfall kinetic energy flux tends towards constant values in the range of 25 to 29 J m<sup>-2</sup> mm<sup>-1</sup> at rainfall intensities of 40 mm h<sup>-1</sup>.

$$KE = \frac{1}{2} \times m \times v^2$$
 Equation 2

Rainfall intensity influences the rate of soil wetting and thus aggregate breakdown due to slaking caused by rapid wetting (Geeves 1997). As previously identified seal formation under high intensity rain can occur rapidly. Cumulative rainfall positively influences crust thickness meaning that the longer the applied rainfall the thicker the surface crust (Armenise et al. 2018). To artificially create a surface crust thus requires high intensity rainfall for a period of time which can be achieved through the use of a rainfall simulator.

#### 2.3.7. Crusting and Australian Soils

Large areas of Australia are affected by soil crusting, including both physical and biological crusts. Biological crusts are common in arid and semi-arid areas (Eldridge et al. 2000). The soils most commonly affected by surface sealing/crusting are Chromosols, Sodosols and Kandosols, particularly where the soil surface is degraded, as wells as Vertosols with sodic surface conditions (Murphy 2015).

Sodic soils readily disperse and are prone to surface crusting. Approximately 28 per cent of the Australian continent is assessed as having sodic soils (Rengasamy & Olsson 1991). Raine and Loch (2003) report that in Queensland 25% of soils are strongly sodic and 20% are variably sodic. Sodicity occurs when there are a high proportion of sodium cations in comparison to other cations. Raine and Loch (2003) argue that the term dispersive soils is a more appropriate term to use than sodic due to the fact that higher clay soils, soils with no mulch/vegetation and cultivated soils can all tend to disperse even if the Exchangeable Sodium Percentage (ESP) results do not classify the soil as sodic. Dispersive soils can develop surface crusts under a number of conditions (Raine & Loch 2003).

The wide scale susceptibility of Australian soils to surface crusting has potentially large impacts on hydrologic/agronomic modelling and subsequently the prediction of infiltration, runoff and crop growth highlighting the importance of being able to accurately model the impacts of surface crusting crusts.

#### 2.4. Section Three - Soil Properties

A number of soil properties are relevant to the study soil crusts and this project. These include bulk density, porosity and soil texture. Soil hydraulic properties will be considered in the next section.

#### 2.4.1.Bulk density

Bulk density ( $\rho_b$ ) is a measure of the level of compaction within a soil. It is defined as the ratio of the oven dried soil mass to its bulk volume and is defined by Equation 3 (Singer & Munns 2006) where  $\rho_b$  is the bulk density (ML<sup>-3</sup>), M<sub>s</sub> is the mass of the sample (M) and V<sub>t</sub> is the bulk volume (L<sup>3</sup>) of the sample.

$$\rho_b = \frac{M_s}{V_t}$$
 Equation 3

Typical  $\rho_b$  values in course textured soils typically range from 1.2 to 1.8 g/cm<sup>3</sup> and in finer textured soils range from 1.0 to 1.6g/cm<sup>3</sup> (Tan 1996). Low bulk densities from 1 – 1.5 g/cm<sup>3</sup> indicate favourable conditions for plant root growth whilst high bulk densities from 1.8 – 2 g/cm<sup>3</sup> indicate compacted soils with few pore spaces that are unfavourable for plant root growth (Tan 1996). In compacted soils plant root growth is restricted due to the difficulty of plant roots penetrating the soil, thus limiting water and nutrient uptake (Stirzaker et al. 1996).

Two main approaches to measuring bulk density have been developed; direct and indirect methods. Direct methods include the core, clod and excavation methods. The core method involves removal of a soil core of known volume from the soil mass using a soil core sampler. The soil core is dried at 105°C until the soil reach a constant weight at which point Equation 3 is applied to calculate the bulk density (Blake 1965). The clod method involves covering an air dry clod or soil ped in a resin to provide a waterproof covering and weighing the clod whilst in the air and in the water from which the volume can be determined (Blake 1965). The water replacement (also known as compliant cavity) method involves using a water filled plastic liner to measure the volume of soil excavated (Cresswell & Hamilton 2002).

Indirect methods use radiation to determine  $\rho_b$ . Gamma radiation determination is based on empirical relationships between radiation and the  $\rho_b$  of soil (Yin Chan 2002). High resolution X-radiography has also been to determine the bulk density of soil crusts (Bresson et al. 2004) and will be discussed later.

Direct methods of determining  $\rho_b$  are typically used for land resource survey activities as they are relatively fast and use simple equipment that is portable whilst being accurate (Cresswell & Hamilton 2002). The soil core method is recommended for laboratory determination of  $\rho_b$  whilst the excavation method is recommended for field determination (Cresswell & Hamilton 2002).

#### 2.4.2.Bulk density of surface crusts

The methods previously described all have limitations when it comes to determining the  $\rho_b$  of a soil crust. Soil core sampling devices produce a soil core sample of much greater depth than the thickness of a soil crust. For example a commonly used 100 cc soil sample ring with an internal diameter of 50 mm has a depth of 51 mm, many times the depth of a surface crust. Use of the soil core method will therefore underestimate crust  $\rho_b$ .

The primary disadvantage of the soil clod method for crust  $\rho_b$  determination relates to sample size and obtaining samples. Larger samples (fist sized according to Grossman and Reinsch (2002) and greater

than 100 g oven dry weight according to (Cresswell & Hamilton 2002) are recommended as they provide a more representative representation of  $\rho_b$  than smaller samples. For thin surface crusts this is not practical and/or difficult to obtain due to the thinness/fragility of the crust and the ability to separate it from the underlying soil.

The water replacement method also has limitations for measuring crust  $\rho_b$ . Grossman and Reinsch (2002) indicate that the sample thickness should exceed 2 cm whilst (Cresswell & Hamilton 2002) state that accurate measurements occur when the excavation is deep in proportion to the excavation diameter. Both of these requirements are problematic for determining surface crust  $\rho_b$ . Recent research by Hardie and Almajmaie (2019) however recommends the use of the water replacement method as an in situ method of obtaining crust porosity/ $\rho_b$ .

Researchers have also used a number of other methods to determine surface crust  $\rho_b$ . For example, Fohrer et al. (1999) collected micro soil cores, used an oil immersion method and X-ray Computerassisted Tomograph (CT) to determine crust  $\rho_b$ . The micro soil cores provided an average  $\rho_b$  value for the 0 – 10 mm depth layer (Fohrer et al. 1999) and whilst likely to be more accurate than traditional soil cores will underestimate crust  $\rho_b$  where crust depth is less than 10 mm. Roth (1997) used an oil immersion method (similar in concept to the clod method) without raising concerns on sample size.

Determination of surface crust  $\rho_b$  is further complicated by the changing  $\rho_b$  of the crust with depth. Until relatively recently, soil seals/crusts were considered to be discrete layers (Roth 1997). Recent studies have however identified that the  $\rho_b$  of surface crusts demonstrate a gradual decrease with depth (Roth 1997), as highlighted in Figure 3, and should be considered in terms of non-uniform layers (Bresson et al. 2004). Further complications are the lateral heterogeneity of surface crusts and the process by which a crust is formed also influences  $\rho_b$  (Bresson et al. 2004).



Figure 3 Decline of surface crust bulk density with depth (Roth 1997)

X-ray CT has been used to measure, with high resolution, several soil crust properties including  $\rho_b$  (Fohrer et al. 1999) and depth (Armenise et al. 2018). Whilst producing accurate data, X-ray CT scanners are very expensive and might not be easily accessible (Bresson et al. 2004), indicating that they are not suitable for routine determination of soil crust  $\rho_b$ .

X-ray CT has however been used to validate  $\rho_b$  depth functions that can be used to determine the  $\rho_b$  of a crust at various depths (Bresson et al. 2004). An example of a bulk density depth function for structural crusts is shown in Equation 4 (from (Roth 1997)) where  $\rho_c$  is the surface crust bulk density (ML<sup>-3</sup>),  $\rho$  is the bulk density of the undisturbed soil (ML<sup>-3</sup>),  $\Delta \rho_o$  is the maximum change in the bulk density at the soil surface (ML<sup>-3</sup>),  $\gamma$  is a shape factor describing soil-rain interaction (dimensionless) and z is the depth from soil surface (L).

$$\rho_c = \rho + \Delta \rho_o \times e^{(-\gamma,z)}$$
Equation 4

Some modelling applications can include different values of  $\rho_b$  throughout the soil profile. The use of X-ray CT is assessed as the only practical method of obtaining this data.

The determination of surface crust  $\rho_b$  using commonly practiced methods remains problematic. Sophisticated techniques such as X-ray CT can provide accurate data but are unlikely to be available for routine use. This highlights the need to:

- Develop a new method, or adapt a current method, for the determination of soil crust  $\rho_b$ , and
- Determine to what level of accuracy is the  $\rho_b$  of the surface crust required for the purposes of modelling infiltration and runoff.

#### 2.4.3.Porosity

Soil is a three phase system consisting of solids, liquids and gases. The spaces in between the solid phase of the soil matrix are known as pores and are occupied by either liquids or gases. Approximately 50 per cent of the volume of an uncompacted volume of soil consists of pores (Singer & Munns 2006).

Mathematically the porosity ( $\varphi$ ) of the soil is defined as the ratio of the pore volume to the total volume as detailed in Equation 5 (from Singer and Munns (2006)) where  $\varphi$  is the soil porosity (dimensionless),  $V_P$  is the pore volume (L<sup>3</sup>) and  $V_T$  is the total volume (L<sup>3</sup>) of the soil.

$$\varphi = \frac{V_P}{V_t}$$
 Equation 5

The porosity of the soil can be used to calculate the soil bulk density and vice versa as detailed in Equation 6 where  $\rho_s$  is the particle density (ML-<sup>3</sup>) and the other terms have been previously defined.

Laboratory (Flint and Flint (2002)) and X-Ray Computed Tomography (CT) methods (Armenise et al. 2018) can be used to calculate particle density however 2.65 g/cm<sup>3</sup> is generally used as the particle density for most mineral soils (Cresswell & Hamilton 2002).

$$\varphi = 1 - \frac{\rho_b}{\rho_s}$$
 Equation 6

One of the characteristics of soil crusts is a low porosity (Nciizah & Wakindiki 2015) leaving less pore space available for plant root growth and water movement/storage.

#### **2.4.4.Measuring** $\rho_b$ and $\varphi$ using X-Ray Computed Tomography

Over the last few decades advanced techniques have been developed to quantitatively measure soil properties at the micro scale. One of these is X-Ray Computed Tomography (CT). X-ray CT is a non-destructive method of visualising the interior of objects including their three dimensional properties by measuring the attenuation of X-ray radiation (Ketcham 2017). This is achieved by taking images of a thin slice of an object. Whilst normal digital images are composed of pixels, or picture elements, an X-ray CT image is composed of a three dimensional volume element known as a voxel (Ketcham 2017).

X-ray CT has been used to support research across a range of soil science topics including: mineral grains and constituents, soil physical properties ( $\rho_b$  and  $\varphi$ ), solute transport and soil biota (Taina et al. 2008). Of relevance to this project, X-ray CT has recently been used to accurately measure the thickness of surface seals. This was achieved by selecting a reference porosity at a depth beneath the soil surface not affected by the surface crust. Porosity was determined from this depth to the soil surface. The thickness of the crust was equal to the depth where the porosity equalled the reference porosity (Armenise et al. 2018). This approach of determining seal thickness appears to be far more accurate than traditional methods of measurement by visual observation and the uses of calipers/rulers. X-ray CT has also been used to measure the change in  $\rho_b$  with depth below the soil surface (Bresson et al. 2004).

The use of CT can provide an extremely accurate quantification of key soil seal/crust physical properties such as  $\varphi$  and  $\rho_b$ , particularly when compared to more traditional methods. Whilst CT is unlikely to be available for routine quantification of soil crusts properties, it does provide the opportunity in this project to compare the accuracy of different surface crust soil  $\rho_b$  sampling techniques and investigate the impact, in conjunction with  $\rho_b$  depth functions, of inaccuracies in measuring surface crust  $\rho_b$  on infiltration modelling.

#### 2.4.5.Soil texture and textural class

Soil texture is an important parameter as it influences a broad range of soil properties including fertility, bearing strength, erosivity and hydraulic conductivity (Bowman & Hutka 2002).

The mineralogical parts of the soils solid phase consists of three types of particle differentiated by size; being sand, silt and clay. In Australia clay particles are defined as being smaller than 0.002 mm in diameter, silt particles between 0.02 and 0.02 mm and clay particles are between 0.02 mm and 2 mm (National Committee on Soil and Terrain 2009). Other classification systems exist that define particle sizes differently. Each soil sample can be classified into a soil textural class based upon the percentage of sand, silt and clay. Commonly a soil textural triangle is used to assign a soil sample to a textural class.

Field and laboratory evaluation of textural class can vary (National Committee on Soil and Terrain 2009) as a result of factors other than percentages of sand, silt and clay influencing determination of texture in the field. This indicates the requirement for laboratory determination of soil texture as many pedotransfer functions (PTF) use soil textural percentages for prediction of other soil properties (Nemes & Rawls 2004). A number of laboratory techniques are available for accurate particle size analysis (Gee & Or) including sieving, pipette and hydrometer methods. Particle Size Analysis laboratory procedures recommended for Australia are detailed in Bowman and Hutka (2002).

#### **2.5. Section Four - Soil Hydraulic Properties**

The prediction of water movement through soil is of critical importance to modelling infiltration and surface runoff. Two key soil moisture parameters (moisture content and matric potential) will be described followed by the Soil Water Characteristic (SWC) (or Water Retention Curve). Hydraulic conductivity will then be defined followed by the Hydraulic Conductivity Function (HCF). The SWC describes the ability of the soil to store water and the HCF describes the soils ability to transmit water (Bristow et al. 1995). These two functions allow the complete description of soil hydraulic behaviour.

#### **2.5.1.Moisture Content**

Soil is a three phase system consisting of solids, water and gases (Figure 4). The moisture content ( $\theta$ ) of a soil is required for many purposes in soil physics. The  $\theta$  of a soil can be defined gravimetrically ( $\theta_g$ ) (Equation 7) or volumetrically ( $\theta_v$ ) (Equation 8).  $\theta_v$  and  $\theta_g$  are related by Equation 9.  $M_w$ ,  $M_s$ ,  $V_w$ ,  $W_s$  and  $\rho_w$  refer to the mass of water, mass of soil, volume of water, volume of soil and density of water respectively.

$$\theta_g = \frac{M_W}{M_S}$$
Equation 7

$$\theta_{v} = \frac{V_{W}}{V_{S}}$$
Equation 8

$$\theta_v = \theta_g \times \frac{\rho_b}{\rho_w}$$
Equation 9

#### Mass relations



Volume relations

Figure 4 Soil mass and volume relationships for the solid, water and air phases within a soil. M denotes mass (M) and V denotes volume ( $L^3$ ). Subscripts a, w, s and p respectively denote air, water, solids and pores respectively (Cresswell & Hamilton 2002).

There are a number of landmark values for  $\theta$  that are of importance in soil physics. These are (Singer & Munns 2006):

- Saturation ( $\theta_s$ ) where all soil pores are filled by water.
- Field Capacity ( $\theta_{FC}$ ) where gravity has emptied the largest soil pores.
- Permanent Wilting Point ( $\theta_{PWP}$ ) where plants can no longer draw moisture from the soil pores.
- Oven Dry ( $\theta_{OD}$ ) where almost all soil pores have been emptied/evaporated after drying in an oven for 24 hours at 105 °C.

#### 2.5.2.Matric Potential

The adhesive and cohesive properties of water and capillary forces give rise to a negative pressure (or potential) within the soil matrix. This is known as the matric potential ( $\psi_m$ ). The matric potential is measured in a number of different units including kPa, Bar or metres head of water. As  $\psi_m$  is a negative quantity, small negative values represent a small suction and large negative values represent a large suction.

When suction (negative pressure) is applied to a saturated soil, water begins to empty from the soil pores. The largest pores will empty first with progressively smaller pores emptying as the suction increases. At very high suctions (large values of  $\psi_m$ ) only the smallest of pores will retain water, held tightly to soil particles via adsorption. Thus as suction increases water content decreases (Hillel 2003).

The landmark values of  $\theta$  described above are related to  $\psi_m$  values. For example Permanent Wilting Point (PWP) corresponds to the soil water content ( $\theta_{PWP}$ ) at  $\psi_m = -153.30$  m and Field Capacity

corresponds to soil water content ( $\theta_{FC}$ ) at  $\psi_m = -1$  m for sandy soils, -3.5 m for medium textured soils and -5.0 m for clayey soils (Radcliffe & Simunek 2010). Plant Available Water is the difference between these two values multiplied by the depth of soil.

The relationship between  $\theta$  and  $\psi_m$  is described by the Soil Water Characteristic curve.

#### 2.5.3.Soil Water Characteristic

The Soil Water Characteristic (SWC) is extremely important in modelling the storage of water in soils. By measuring  $\theta$  at varying values of  $\psi_m$  a curve can be developed that is known as the Soil Water Characteristic (SWC) (Tuller & Or 2004). An example SWC for a clay soil and a sandy soil is shown in Figure 5.

The difference between the two soils can be explained via soil texture and structure. At low suction (between saturation and around  $\psi_m = -10$  kPa), water content is related primarily to capillarity and pore size distribution both of which are linked to soil structure (Cresswell et al. 1992). At high suctions water content is mainly due to the adsorption of water to soil particles which is linked to soil texture (Hillel 2003, p. 114). Sandy soils tend to have larger and more regular pore sizes thus at even relatively low suctions will have a lower water content than a clay soil. The negative charge on clay particles adsorbs water which when combined with a range of pore sizes means that the slope of the curve is less steep than that of a sandy soil.



#### Water content

Figure 5 Soil Water Characteristic (Hillel 2003, p. 115). Note that the axes are reversed in many instances.

Many equations have been developed to describe the relationship between  $\theta$  and  $\psi_m$ . A commonly used model is the van Genuchten (VG) retention curve model (Equation 10) (Tuller & Or 2004) where  $\theta$ ,  $\theta_s$  and  $\theta_r$  are the water content, water content at saturation and residual water content respectively and  $\psi_m$  is the matric potential. Parameters  $\alpha$ , *m* and *n* are parameters related to the shape of the soil water characteristic.

Effective saturation 
$$(S_e) = \frac{\theta - \theta_r}{\theta_s - \theta_r} = \left[\frac{1}{1 + (\alpha \psi_m)^n}\right]^m$$
 Equation 10

Software applications have been developed that use equations such as Equation 8 to determine soil hydraulic parameters. The RETC (Retention Curve) software application (van Genuchten et al. 1991) is an example that can derive both the SWC and HCF.

#### 2.5.4.Hydraulic conductivity

Saturated hydraulic conductivity ( $K_s$ ) is a measure of the soils ability to transmit water under saturated conditions (Jabro 1992).  $K_s$  is a function of a soils particle size distribution, pore size distribution, pore configuration and bulk density (Jabro 1992).  $K_s$  is a key soil hydraulic variable that controls the partition of rainfall between infiltration and runoff as well as being a primary variable in all models related to the hydrological cycle (Sobieraj et al. 2001).  $K_s$  is one of the most sensitive input parameters used in hydrological models (Zhao et al. 2016).

Estimating  $K_s$  values directly, either in the field or laboratory is time consuming and labour intensive (Rawls et al. 1998). There are numerous methods of directly determining saturated hydraulic conductivity including; the Guelph permeameter, constant-head well permeameter, double tube method, Amoozemeter, small detached cores using falling head or constant head, large attached columns, instantaneous profile, cylindrical infiltrometers, dripper method, piezometer and detached core heat-shrinkable plastic (Rawls et al. 1998). (Rawls et al. 1998) found that over 80% of the measurements were obtained used the falling head or constant head on small detached cores or instantaneous profile methods. Typical values of  $K_s$  are detailed in Table 2.

Table 2 Typical saturated hydraulic conductivity ( $K_s$ ) values (Hiller 2005)		
Soil texture	Ks (m/sec)	
Clay	$10^{-10} - 10^{-8}$	
Silt	$10^{-8} - 10^{-6}$	
Sand	$10^{-5} - 10^{-3}$	
Gravel	$10^{-2} - 10^{-1}$	

Table 2 Typical saturated hydraulic conductivity  $(K_s)$  values (Hillel 2003)

The importance of saturated hydraulic conductivity and the difficulties of calculating it directly have led to the development of indirect methods via pore-size distribution, inverse methods or Pedo-Transfer Functions (PTF) (Sobieraj et al. 2001).

Despite the importance of  $K_s$ , in the majority of cases the soil is not saturated. Water still flows in unsaturated soil. The driving force of water movement in unsaturated soils is differences in soil matric potential. This creates a gradient causing water to flow from where the suction is higher (large  $\psi_m$ ) to where it is lower (small  $\psi_m$ ) (Hillel 2003). Hydraulic conductivity drops steeply in the transition from saturated to unsaturated conditions as the largest pores which conduct the most water are emptied first (Hillel 2003). The relationship between K and  $\psi_m$ , or  $\theta$ , is described by the Hydraulic Conductivity Function (HCF).

#### **2.5.5.Hydraulic Conductivity Function**

Figure 6 depicts examples of the HCF for a sandy soil and a clayey soil. The HCF is normally displayed on log – log scales as the values for both *K* and  $\psi_m$  can vary by orders of magnitude. Under saturated conditions, the sandy soil conducts more water due to the distribution of larger pore sizes within the soil. However as the suction is increased, these pores are rapidly emptied of water resulting

in a rapid decline in K. For the clay soil, which has a much higher proportion of small pores, the rate of decline in K is much less.



Figure 6 Graphical depiction of example Hydraulic Conductivity Function's for a clay soil and a sandy soil (log – log scale)

Multiple empirical equations have been proposed to relate K to  $\psi_m$  or  $\theta$  (Hillel 2003). These equations are often depicted as  $K(\psi)$  or  $K(\theta)$ . Mualem's hydraulic conductivity model is most commonly coupled with the previously described VG retention curve model (Equation 10) (Radcliffe & Simunek 2010). Mualem's model in terms of both effective saturation ( $S_e$ ) and  $\psi_m$  are detailed in Equation 11 (Radcliffe & Simunek 2010) and Equation 12 (van Genuchten et al. 1991) where all terms have been previously defined.

$$K(S_e) = K_s \times S_e^{\ l} \times \left[1 - \left(1 - S_e^{\frac{1}{m}}\right)^m\right]^2$$
 Equation 11

$$K(\psi) = \frac{K_s \times \{(1 - (\alpha\psi)^{mn}) \times [1 + (\alpha\psi)^n]^{-m}\}^2}{[1 + (\alpha\psi)^n]^{ml}}$$
Equation 12

The RETC software application can derive the parameters for the Mualem HCF (van Genuchten et al. 1991).

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#### 2.6. Section Six - Modelling infiltration

#### 2.6.1. Background

The measurement and understanding of the factors controlling the infiltration process has proven exceedingly difficult over a long period of time (Betson 1964). This may explain the large number of models that have been developed, tested and modified to predict infiltration. These models vary in complexity from simple empirical equations, such as the modified Kostiakov equation, to physically based models such as the Green and Ampt equation, through to computationally intensive numerical solutions to the Richards' equation (Souchere et al. 2001). The commonly used HYDRUS-1D software application solves the Richards equation to model infiltration.

#### 2.6.2. Darcy's law

The one dimensional flow of water through a porous medium, such as soil can be mathematically defined by the commonly used Darcy's law. Equation 13 details Darcy's law where Q is the flow of water per unit time (L<sup>3</sup>T<sup>-1</sup>) through an area A (L<sup>2</sup>), K is the hydraulic conductivity and  $d\psi/dx$  is the water potential gradient (L/L) (Singer & Munns 2006).

$$Q = A \times K \times \frac{d\psi}{dx}$$
 Equation 13

Darcy's law is used within the Richards equation. Saturated hydraulic conductivity can be calculated by rearranging Darcy's law to solve for K once the Q has reached a constant value.

#### 2.6.3. Richards Equation

The Richards' equation can be described as a combination of the unsaturated Darcy's law combined with the equation of continuity that results in a second-order non-linear partial differential equation for unsaturated flow in porous media (Moore & Eigel 1981).

Equation 14 details the Richards' equation where *h* is the pressure head (L),  $\theta$  is the volumetric water content (L<sup>3</sup>L<sup>-3</sup>), *t* is time (T), *x* is a spatial coordinate (L), *S* is a sink term (L<sup>3</sup>L<sup>-3</sup>T<sup>-1</sup>),  $\alpha$  denotes the flow direction relative to the vertical axis and *K* is the HCF (LT<sup>-1</sup>) (Šimůnek et al. 2009).

$$\frac{\delta\theta}{\delta t} = \frac{\delta}{x} \left[ K \left( \frac{\delta h}{\delta x} + \cos \alpha \right) \right] - S$$
 Equation 14

In most circumstances the Richard's equation can only be solved by numerical methods and is much more computationally intensive than other methods such as the Green and Ampt model. As a result it is best applied using a software application such as HYDRUS-1D.

#### 2.6.4. HYDRUS-1D

A software package known as HYDRUS-1D has been developed to model one dimensional flow in a variably saturated porous medium using the Richards' equation (Šimůnek et al. 2009). HYDRUS-1D is freely available and widely used for simulating infiltration, heat and solute movement in soils.

HYDRUS-1D has a graphical interface which simplifies the setup and execution of simulations. Inbuilt within HYDRUS-1D is the ROSETTA database (see Section 2.7.2 for more information on ROSETTA) which is a hierarchical PTF that enables the prediction of soil hydraulic properties based on user inputted data (Schaap et al. 2001). The advantage of this approach is that readily available soil properties such as texture and  $\rho_b$  can then be used as inputs for the determination of the parameters required to solve the Richards equation.

#### HYDRUS-1D is used for the modelling of infiltration throughout this project.

# 2.7. Section Seven - Pedotransfer functions and the prediction of soil hydraulic properties

#### 2.7.1. Overview of Pedotransfer Functions

There is a requirement in many soil related disciplines for data on soil hydraulic properties to quantify plant available water and to model the movement of water and solutes through the soil (Contreras & Bonilla 2018). In many instances, due to time and cost, there will be limits on how many measurements of soil hydraulic properties are made (McKenzie & Cresswell 2002). To address this deficiency a significant research effort has been made over time to establish relationships between soil hydraulic properties and other more readily measurable soil properties. These relationships are known as Pedotransfer Functions (PTF).

There are multiple different types of PTF from simple look up tables to regression equations and neural network analysis (Radcliffe & Simunek 2010). PTFs are used in different ways. Some PTFs predict soil hydraulic properties directly (such as the moisture content at a specified matric potential) or as input parameters into analytical models that estimate soil hydraulic properties (Radcliffe & Simunek 2010).

Soil physical properties that are commonly used as inputs into PTFs include soil texture/particle size distribution, organic matter content and  $\rho b$  (Contreras & Bonilla 2018). Some of the numerous examples of PTF that have been developed include Jabro (1992) who identified that an empirical relationship existed between K<sub>s</sub>, soil texture and  $\rho_b$ , and (Rawls et al. 1998) who included  $\rho_b$  values as a discriminator in developing mean values for K<sub>s</sub>.

PTF are not necessarily a panacea for a lack of soil hydraulic data. Sobieraj et al. (2001) reviewed nine PTFs that used various soil properties to predict  $K_s$ . Their conclusion after applying PTFs to predict soil hydraulic parameters and hydrological outputs found that they were inadequate. Another paper studied six PTFs and found they are an inaccurate method of predicting  $K_s$  due to the inherent variability of  $K_s$  whilst noting that including  $\rho_b$  as a parameter can improve error ratios (Tietje & Hennings 1996). Selle et al. (2011) concluded that water transmission properties using PTFs were poorly correlated with basic soils data, including  $\rho_b$ , in the Shepparton irrigation region.

Not all studies have had negative results however. Multiple studies have shown that systematic variation in K<sub>s</sub> can be explained by soil texture, porosity and  $\rho_b$  (Zhao et al. 2016). Zhao et al. (2016) also identified that PTFs are not easily transferrable between bioclimatic zones and require optimisation using local data if they are to be reliable. Paydar and Ringrose-Voase (2003) and Minasny and McBratney (2000) reported similar findings identifying that local calibration is required as well as ensuring that standardised sample volumes and measurement techniques are considered.

#### 2.7.2. Bulk density as a predictor of soil hydraulic properties

Bulk density is a commonly used input parameter in PTFs. For example Patil and Singh (2016) found that  $\rho_b$  was used as a parameter in 15 of 19 PTFs they reviewed that were used to estimate saturated hydraulic conductivity. A recent review by Zhang and Schaap (2019) also identified that  $\rho_b$  is used in many PTFs.

The ROSETTA application, included in HYDRUS-1D and RETC, provides five hierarchical PTF to predict water retention parameters and saturated hydraulic conductivity based on available input data as detailed in Table 3 (Schaap et al. 2001). The models that use more input parameters perform better
than those that use less (Schaap et al. 2001). The three PTF at the top end of the hierarchy include  $\rho_b$  as a parameter. Zhang and Schaap (2019) reported an increase in r<sup>2</sup> values between estimated and measured  $K_s$  values from 0.45 to 0.55 when  $\rho_b$  was included as a predictor.

Model	Input Parameters	Comment
H1	Textural class	USDA textural classes
H2	Sand, Silt, Clay (SSC)	Percentages
H3	SSC, Bulk Density (BD)	
H4	SSCBD, water content at 3.3 m suction ( $\theta_{3,3}$ )	
H5	SSCBD, $\theta_{3,3}$ , water content at 150 m suction ( $\theta_{150}$ )	

 Table 3 Input Parameters for ROSETTA PTF Models

Few studies have however discussed the impact of the increased  $\rho_b$  of surface crusts when using PTFs. Paydar and Ringrose-Voase (2003) found that a Kozeny-Carman model based PTF had poor predictive capability and failed to predict  $K_s$  values for soils that hard set, had surface crusts or high levels of organic matter. Jarvis et al. (2002) identified that one source of error in measuring hydraulic conductivity could be attributed to different approaches in dealing with surface seals. The increased  $\rho_b$ of the seal is a likely contributor to this error. As the first few millimetres of the soil surface under sealing conditions control infiltration (Di Prima et al. 2018), these errors could be propagated via PTFs and affect their accuracy and reliability.

Bulk density is commonly used as an input into PTF used to predict saturated hydraulic conductivity and subsequently infiltration. There are mixed reviews on the effectiveness of PTF, including the inclusion of  $\rho_b$  as a predictive variable. The literature on the impact of surface seals when using PTFs is sparse. The  $\rho_b$  of the soil surface is generally determined from samples with a depth that is many times greater than the depth of a surface seal/crust. This suggests that values of  $\rho_b$  being used in PTFs to predict  $K_s$  will be systematically lower, potentially by a significant margin, than the  $\rho_b$  of the surface crust. This is a potential, and perhaps major, source of error in predicting soil hydraulic properties on soils susceptible to crusting. The development of a PTF that predicts the hydraulic behaviour of a surface crust thus offers an opportunity to improve infiltration modelling.

# 2.8. Section Eight - Conclusions and knowledge gap

Accurate values of soil hydraulic properties are required in a broad range of disciplines including water engineering, mine-site rehabilitation, ecology and agronomy, for the purposes of predicting infiltration, drainage, plant available water, modelling the impact of management actions, and watershed modelling (Timlin et al. 2004). Despite intensive efforts over a prolonged period of time; developing models of infiltration and surface runoff that are both accurate and widely applicable remains a challenge. This is largely due to the spatial heterogeneity of soil characteristics as well as surface seals/crusts which significantly reduce infiltration whilst increasing runoff. Combined these characteristics have resulted in no particular model of infiltration or runoff being generally accepted (Nciizah & Wakindiki 2015).

The literature review has identified that whilst  $\rho_b$ , and other basic soil properties such as texture, have been used to varying degrees of success to predict soil hydraulic parameters such as  $K_s$ ; very limited research has been conducted on incorporating the impact of surface crust properties such as  $\rho_b$  upon infiltration modelling. This is evidenced by the use of  $\rho_b$  values throughout the literature measured from surface horizons of a depth that are many times the depth of a surface crust. The resultant underestimation of  $\rho_b$  values is a source of error in infiltration modelling. Noting that surface crusts can reduce infiltration by 80 per cent (Moore & Eigel 1981) the resulting errors can be large with potential consequential impacts.

The literature indicates that traditional methods of  $\rho_b$  determination are not suited or practical for surface crusts. X-ray CT has been identified as a method that can be used to accurately measure surface crust porosity and/or  $\rho_b$ . These values can subsequently be incorporated into an infiltration model such as HYDRUS-1D to determine whether measuring the  $\rho_b$  of the surface crust improves model accuracy.

Rainfall simulators have been identified as a reliable method of applying rainfall to a soil surface that will result in a surface crust being formed. Rainfall simulators also enable the collection of surface runoff/infiltration observed data that can be compared to model outputs. Combined, the use of a rainfall simulator, the ability to accurately measure crust  $\rho_b$  and the use of a modelling application such as HYDRUS-1D enable an experiment to be designed and implemented that will determine whether measuring surface crust  $\rho_b$  will improve the accuracy of infiltration modelling.

There is an increasing demand for accurate data on soil hydraulic properties that are required for use in hydrological models that inform policy making at both the catchment and regional scale (Paydar & Ringrose-Voase 2003) and crop simulation models. When combined with the large land area in Queensland and Australia susceptible to soil crusting, this indicates the requirement to determine whether incorporating the impact of surface crusts in infiltration modelling improves model accuracy. The literature review has identified a gap in the research related to the impact of surface crusts upon infiltration as well as methods that can be applied to fill this knowledge gap.

# 3. CHAPTER THREE – MODELLING THE IMPACT OF SURFACE SEALS/CRUSTS ON SOIL HYDRAULIC PROPERTIES AND INFILTRATION

# **3.1. Introduction**

Due to the difficulty, time and resource requirements required to accurately measure soil hydraulic properties, infiltration and surface runoff; modelling plays a critical role in most hydrologic investigations. The aim of this chapter is thus to investigate the impact of increasing  $\rho_b$  on soil hydraulic properties and infiltration with commonly used modelling applications (RETC and HYDRUS-1D).

The modelling approach is summarised below:

- 1. Initial modelling using HYDRUS-1D to determine whether increasing the soil  $\rho_b$ , representing a soil crust, has an impact on infiltration.
- 2. Obtain observed water retention and hydraulic data for a number of soils across a range of soil textures.
- 3. Determine the Soil Water Characteristic (SWC) and Hydraulic Conductivity Function (HCF) using the observed water retention and hydraulic conductivity data for the selected soils using RETC.
- 4. Increase the  $\rho_b$  of the soils (representing a seal/crust) and determine predicted SWC and HCF using RETC.
- 5. Use the observed and predicted soil hydraulic properties to model infiltration using HYDRUS-1D under different scenarios.
- 6. Develop PTF/s for modelling impact of increased  $\rho_b$  on soil hydraulic properties.

## 3.2. Automating modelling activities

To reduce the time taken to complete modelling activities using RETC and HYDRUS-1D, MATLAB scripts and functions were developed to automate a number of repetitive tasks. These tasks included:

- Changing parameter values in HYDRUS-1D input files
- Executing HYDRUS-1D simulations
- Importing HYDRUS-1D (and RETC) output files for analysis and charting.

Selected MATLAB code developed for this project is detailed in Appendix B.

#### 3.3. Impact of increasing bulk density on infiltration

To confirm the importance of considering surface crust  $\rho_b$  as a parameter in modelling infiltration (and subsequently runoff) a number of simulations were conducted to calculate the infiltration rate and cumulative infiltration under different crust conditions using HYDRUS-1D.

A 100 mm deep soil profile was used for the simulations allowing for 1 mm depth segmentation within HYDRUS-1D. Four crust conditions were modelled, being: no crust, 2 mm, 6 mm and 10 mm crusts with the crust being a constant  $\rho_b$  throughout its depth (Figure 7). A constant crust  $\rho_b$  was selected for the initial simulations for simplicity although this does not align with the findings of Roth (1997), Bresson et al. (2004) and others, which indicate that surface crust  $\rho_b$  should be considered in terms of non-uniform layers. A uniform matric potential was applied to the soil profile.



Figure 7 Soil profiles used in HYDRUS-1D simulations. Black rectangles at the top of the soil profile represent uniform surface crusts. White rectangles represent underlying soil. The profile on the right represents a non-uniform crust with bulk density decreasing each mm until the underlying soil profile is reached.

The underlying soil was assigned a constant but lower  $\rho_b$  than the crust.  $\rho_b$  figures were arbitrarily selected based on ranges discussed by (Tan 1996). A coarse (Sandy Loam), medium (Clay Loam) and fine (Medium Clay) texture class were selected as representative soils. The simulations were run for 10 minutes. A summary of the soil properties is detailed in Table 4. Water retention data was not used to derive soil hydraulic parameters from the ROSETTA database within HYDRUS-1D.

				0	
Texture	% Sand	% Silt	% Clay	Soil p <sub>b</sub>	Crust ρ <sub>b</sub>
Sandy Loam	60	30	10	1.4	1.8
Clay Loam	35	35	30	1.4	1.8
Medium Clay	25	25	50	1.4	1.8

Table 4 Soil properties used in HYDRUS-1D to predict parameters for soil hydraulic modelling

#### **3.3.1.Simulation One: Ponded Head**

The first simulation involved a ponded head of 10 mm being applied to each soil type (matric potential at the surface was changed to 10 mm) with the underlying soil having a constant head value of -1000 mm (Field Capacity). The results are detailed in Table 5, which highlights major reductions in cumulative infiltration under all crusts with the reduction increasing with crust thickness. Figure 8 provides a graphical representation of infiltration rate and cumulative infiltration under the different crust scenario's for the Clay Loam, which was representative of all three soils. The infiltration rate is higher at all times for the un-crusted soil than the crusted soils, with the initial and final infiltration rates declining as crust thickness increases.

	Medium Cla	y	Clay Loam		Sandy Loam		
Crust thickness	Cumulative Infiltration (mm)	Percentage reduction	Cumulative Infiltration (mm)	Percentage reduction	Cumulative Infiltration (mm)	Percentage reduction	
No Crust	3.42		6.51		9.89		
2 mm crust	2.67	22%	3.59	45%	8.85	11%	
6 mm crust	1.11	67%	1.27	80%	4.77	52%	
10 mm crust	0.78	77%	0.78	88%	3.28	67%	

Table 5 Cumulative infiltration results from ponded head scenario



log(Time) - (s)

Figure 8 Simulation One: Cumulative Infiltration Under 10mm Ponded Head Conditions for a Clay Loam. The infiltration rate is plotted on a log – log axis.

#### **3.3.2.Simulation Two: Rainfall**

A second simulation, using the same depth, texture and crust parameters, was modelled that involved rainfall. A rainfall intensity of 100 mm/h was selected (0.02778 mm/s). The cumulative infiltration results are detailed in Table 6 and follow a similar pattern to that identified in Simulation Two.

	Medium Cla	У	Clay Loam		Sandy Loam	
Crust thickness	Cumulative Infiltration (mm)	Percentage reduction	Cumulative Infiltration (mm)	Percentage reduction	Cumulative Infiltration (mm)	Percentage reduction
No Crust	3.11		4.10		11.41	
2 mm crust	2.57	17%	3.53	14%	10.87	5%
6 mm crust	0.90	71%	1.18	71%	8.64	24%
10 mm crust	0.64	79%	0.81	80%	6.59	42%

Table 6 Cumulative infiltration results from 100 mm/h rainfall intensity simulation

Figure 9 provides a graphical representation of cumulative infiltration and infiltration rate for the Clay Loam soil. The differences in the infiltration rate profile, compared to the smooth curves for the

ponded head scenario, are assessed as being a result of the additional time required for saturation of the soil surface under rainfall before the infiltration rates decline.



Figure 9 Simulation Two: Cumulative infiltration under 100 mm/h rainfall for a Clay Loam.

#### **3.3.3.Simulation Three: Changing matric potential**

The matric potential of the soil profile was changed to investigate the impact that this parameter, reflecting varying degrees of soil wetness, had on infiltration. Four values of matric potential were simulated being: -100 mm (default value in HYDRUS-1D), -1000 mm (Field Capacity), -10,000 mm (an intermediate value between Field Capacity and Permanent Wilting Point) and -150,000 mm (Permanent Wilting Point). A 10 mm ponded head condition was used for all simulations.

Figure 10 provides an example of infiltration rates and cumulative infiltration under changing values of matric potential for the Medium Clay profile with a 6 mm crust. All simulations provided similar infiltration and cumulative infiltration profiles. It is clear from Figure 10 that the drier the soil the greater the infiltration rate and cumulative infiltration.

Figure 11 and Figure 12 provide comparisons across the texture classes of cumulative infiltration at matric potential values of -100 mm and -150000 mm respectively. These Figures highlight that the impact of matric potential upon infiltration and cumulative infiltration extend across the texture classes.



Figure 10 Infiltration Rate and Cumulative Infiltration for Medium Clay Soil with 6 mm crust



Figure 11 Comparison of cumulative infiltration at h = -100mm



Figure 12 Comparison of cumulative infiltration at h = -150,000 mm (PWP)

Matric potential of the soil profile is a required input parameter for HYDRUS-1D simulations. Whilst it is clear that large changes in  $\psi$  have a substantial impact on infiltration, quantifying the impact of smaller scale changes is important to identify how accurately the input values for  $\psi$  must be. To address this, a simulation was conducted using  $\psi$  values of – 2000 mm, -3000 mm, -4000 mm and – 5000 mm. The results are detailed in Table 7 and identify that small changes in  $\psi$  have an insignificant impact upon the final infiltration rate and cumulative infiltration, over the short time period of the simulation. Similar results were obtained for simulations using other textures and crust thicknesses.

min crust under ponded neud conditions							
Ψ(mm)	Final infiltration rate (mm/h)	<b>Cumulative infiltration (mm)</b>					
-2000	26.3	5.1					
-3000	26.8	5.3					
-4000	27.2	5.3					
-5000	27.5	5.4					

Table 7 Impact of small changes in  $\psi$  on final infiltration rate and cumulative infiltration for Sandy Loam with a 6 mm crust under ponded head conditions

The finding that small changes in  $\psi$  result in insignificant changes in infiltration will be important in the modelling of infiltration during the experimental phase of this project.

## **3.3.4.Simulation Four: Changing time**

Simulations to this point have been run for a short period of time. To investigate the impact of time on infiltration the parameters used in Simulation One were retained however the time was extended to 24 h (86400 s).

Table 8 details the results for both final infiltration rates and cumulative infiltration. For two of the three textures the final infiltration rates for the no crust and 2 mm crust reach the same value with cumulative infiltration rates also reaching a similar value. For the 6 mm and 10 mm crusts there is a marked reduction in final infiltration rates and cumulative infiltration compared to the un-crusted soil.

Final infiltration rate (mm/h)									
Soil	No crust	2 mm crust	6 mm crust	10 crust	mm				
Sandy Loam	20.52	20.52	14.4	11.52					
Clay Loam	13.68	4.32	2.88	0.72					
Medium	5.4	5.4	2.88	2.16					
Clay									
Cumulative i	nfiltration (mi	<b>n</b> )							
Soil	No crust	2 mm crust	6 mm crust	10	mm				
				crust					
Sandy Loam	489.97	498.08	364.51	281.39					
Clay Loam	330.7	105.37	70.963	17.663					
Medium Clay	129.85	128.75	68.234	50.877					

 Table 8 Summary of results for final infiltration rates and cumulative infiltration rates after 24 h simulation under ponded head conditions



Figure 13 Infiltration rates over 24 h for the Sandy Loam on a log - log scale

Figure 13 is an example of the infiltration rates over a 24 h period for the Sandy Loam soil. The other soils had a similar profile. All soils reached a steady infiltration rate within the 24 h time period of the simulation. The time taken to reach the steady infiltration rate was roughly similar for the no crust and 2 mm crust simulations and progressively longer for the 6 mm and 10 mm crust across all simulations.

#### 3.3.5.Simulation Five: Non-uniform crusts

The impact of non-uniform crusts was simulated by changing the soil profile to have five soil layers. The top four layers (with a thickness of 1 mm) had densities descending from 1.8 g/cm<sup>3</sup> in 0.1 g/cm<sup>3</sup> increments until the underlying soil  $\rho_b$  was reached (the right hand most profile in Figure 7 provides a visual representation of the profile). The ponded head scenario was selected as it ensures that the infiltration capacity of the soil was exceeded throughout the simulation with a simulation time of 600 s. All other parameters remained the same as detailed in Simulation One.

Cumulative Infiltration (mm)									
	No	2 mm	6 mm	Variable					
Soil	crust	Crust	Crust	Crust					
Sandy Loam	9.8928	8.8522	4.7709	8.6848					
Clay Loam	6.5078	3.5861	1.2744	3.5168					
Medium									
Clay	3.4166	2.6691	1.1146	2.3978					

Table 9 Comparison of cumulative infiltration for no crust, 2mm crust, 6 mm crust and variable crust simulations

Table 9 details the cumulative infiltration values for the no crust, 2 mm crust, 6 mm crust and variable crust simulations. Cumulative infiltration under the variable crust simulation approximates cumulative infiltration with a 2 mm crust for all three soils. Figure 14 provides a comparison of infiltration rates and cumulative infiltration for the Medium Clay soil. Similar profiles were obtained for the other two textures. Infiltration rates in all cases under the variable crust condition were similar to the 2 mm crust.



Figure 14 Infiltration and cumulative infiltration rate comparison for medium clay

## 3.3.6. Conclusion

A number of simulations testing different parameters have been completed to assess the effectiveness of HYDRUS-1D in modelling infiltration on crusted soils.

The simulations indicate that a surface seal/crust of higher  $\rho_b$  than the underlying soil results in substantially less cumulative infiltration than an un-crusted soil. These results align with what has been identified in the literature and confirm the importance of considering surface crust  $\rho_b$  for hydrologic modelling. One potential issue was identified in the results, under the ponded head simulation, where there was very little difference in cumulative infiltration between the no crust and 2 mm crust. This result does not align with the literature which indicates that even thin crusts can result in a large reduction in infiltration. The results from the non-uniform crust simulation suggest that there is little advantage in modelling a crust as a non-uniform layer, which suggests a limitation in the model parameters given the literature observations of naturally occurring crusts.

Having identified/confirmed that increasing the  $\rho_b$  has an impact on infiltration, more realistic modelling of soil hydraulic properties and infiltration based on observed data will now be examined.

# 3.4. Determining the Soil Water Characteristic from the observed data

The parameters for the SWC were obtained by using the RETC software application (van Genuchten et al. 1991). RETC can be used to derive the input parameters required by a number of models that generate a SWC and HCF. These models include the Brooks and Corey and VG equations for soil water retention and Burdine and Mualem pore-size distribution models (van Genuchten et al. 1991).

# 3.5. Observed soil water retention data

Modelling in the previous section used arbitrarily selected  $\rho_b$  values to identify the impact of a surface crust upon infiltration. The modelling conducted in the remainder of this chapter makes use of measured soil properties as input data. The data has been obtained from the US Department of Agriculture's (USDA) UNsaturated SOil hydraulic DAtabase (UNSODA) database (Nemes et al. 2015). UNSODA is a database of soil hydraulic properties from nearly 800 soil samples across the world (Nemes et al. 2015). Data included in UNSODA includes:

- Site details
- Profile information
- Lab and field data for hydraulic conductivity, water retention and other soil hydraulic properties
- Soil texture and bulk density data.

Data from soils representing fine, medium and coarse grained textures were selected that met the following characteristics:

- Data was for the surface horizon.
- A minimum of eight soil water characteristic data points measured for drying of the soil (very few samples had soil water characteristic data for wetting).
- Where possible, soil water retention data was available to a suction of 153 m (Permanent Wilting Point). For a couple of soils the retention data was not available at or near the 153 m suction.

The aim was to select four soils per texture class however there were only two clay textured soils that met these requirements. A summary of the selected soils is shown in Table 10.

Soil						
code	Texture	Location	Sand	Silt	Clay	ρ <sub>b</sub>
		Rotenbach, Schwarzsee,				
2620	clay	Switzerland	17%	36%	48%	1.07
4120	clay	Kalloo (Antwerp), Belgium	14%	35%	51%	1.24
2580	loam	Burgdorf, Switzerland	36%	50%	14%	1.08
2590	loam	Marthalen, Switzerland	34%	48%	18%	1.07
2650	loam	Marthalen, Switzerland	49%	33%	19%	1.22
2750	loam	Zugerberg, Switzerland	52%	30%	19%	1.01
	loamy					
1090	sand	Lillington, NC, USA	81%	10%	9%	1.13

Table	10	Observed	Soil	Data	Summary	v
THOIC	10	Objet fea	2011	Dutu	Summer	

2570	loamy sand	Rheinau, Switzerland	87%	10%	3%	1.5
3130	loamy sand	Dickey Co., ND, USA	76%	19%	6%	1.44
4010	loamy sand	Booischot (Mechelen), Belgium	79%	17%	5%	1.59

#### **3.5.1.RETC modelling without retention data**

Initially the RETC 'Direct' (or forward fitting) option was selected to determine the SWC. The VG Retention Curve Model (with m = 1 - 1/n) and Mualem Conductivity Model were selected, see Equation 10 and Equation 11. The Mualem Conductivity Model is most commonly paired with the VG Retention Curve Model (Radcliffe & Simunek 2010). Soil hydraulic parameters were calculated by entering the Sand, Silt, Clay and  $\rho_b$  (SSCBD) data into the ROSETTA PTF which is inbuilt into RETC (and HYDRUS-1D).

The predicted (RETC) and observed (UNSODA) data were plotted against each other for each of the selected soils. The Direct fitting option in RETC does not calculate the parameters required to apply the VG equation, therefore linear regression in Microsoft Excel using the LINEST function was used to develop a function to relate the water content to the matric potential. In all instances a logarithmic function was found to be the best fit.



Figure 15 Soil Water Characteristic - Loam Soil Code 2590

Figure 15 is an example of the predicted versus observed soil water characteristic. The points represent the predicted or observed data points from RETC and UNSODA respectively whilst the lines represent the curve generated from the LINEST function ( $r^2 = 0.94$  for the RETC data curve fit and  $r^2 = 0.96$  for the UNSODA data curve fit).

Figure 16 compares the predicted versus observed results and the 1:1 line. If the predicted (P) and observed (O) data closely aligned the 1:1 and P-O lines would be very close to one another. In this instance it can be seen that there is not good alignment between the 1:1 and P-O lines indicating that the predicted data does not align closely to the observed data. This result was seen to a greater or lesser with all of the soils examined. This is an indication that the selected fitting model (Direct fitting) within RETC does not provide the required level of accuracy.



Figure 16 Comparison of Predicted vs Observed Results - Loam Soil Code 2590

#### **3.5.2.RETC modelling incorporating retention data**

The next modelling activity used the 'Retention data only' fitting type within RETC. All other inputs remained the same with the exception of using the observed hydraulic retention data parameters from the UNSODA database.

Figure 17 provides examples of the Soil Water Characteristic (Figure 17a, c & e) and Predicted versus Observed results (Figure 17b, d & f) respectively for three of the soils. The results are substantially better than those achieved from the 'Direct' fitting type within RETC. The close alignment of the results indicates that the logarithmic curve fitted equation from the RETC modelling can be used as a proxy for the SWC. These equations will be used in the next modelling step where the  $\rho_b$  will be increased to investigate the impact on the SWC.



Figure 17 Example Retention Parameter Modelling using RETC

The relationship between these logarithmic curve fitting functions is defined by Equation 15 where  $\theta$  (%) is the moisture content and  $\psi$  is the matric potential or suction (cm). 'a' and 'b' are fitted parameters determined from the LINEST function within Microsoft Excel.

$$\theta = a \times \log_{10} \psi + b$$
 Equation 15

The parameters 'a' and 'b' and associated  $r^2$  value for each of the soils are detailed in Table 11. The parameters for soils code 4010 has not been provided due to spurious results in RETC/the LINEST function, which has not been resolved. The minimum  $r^2$  value is 0.843 (Loamy Sand 3130) with all other values being greater than 0.9 indicating that the developed functions are a good fit to the data.

Soil code	Texture	а	b	r <sup>2</sup>
2620	clay	-0.0893	0.6223	0.964
4120	clay	-0.0643	0.5637	0.969
2580	loam	-0.0785	0.6443	0.994
2590	loam	-0.0875	0.5523	0.939
2650	loam	-0.1131	0.5808	0.988
2750	loam	-0.1047	0.6518	0.992
1090	loamy sand	-0.1144	0.4312	0.933
2570	loamy sand	-0.1742	0.6322	0.904
3130	loamy sand	-0.2289	0.7039	0.843

Table 11 'a' and 'b' parameters for Soil Water Characteristic

# **3.6.** Modelling the Soil Water Characteristic as $\rho_b$ increases

#### 3.6.1. Retention data only

The first attempt at modelling the changes in the SWC resulting from increased  $\rho_b$  used the 'Retention data only' fit option in RETC. The soil hydraulic properties were recalculated using the inbuilt ROSETTA PTF with the  $\rho_b$  of the soil samples being increased by 0.1 g/cm<sup>3</sup> per iteration. Soil texture and the observed water retention data points remained the same.

The Clay soil code 2620 was modelled with four bulk densities (1.07 (initial value), 1.17, 1.27 and  $1.37 \text{ g/cm}^3$ ). The results of the RETC predicted soil water characteristic are detailed in Figure 18. The results from each scenario are the same.



Figure 18 Clay soil 2620 - Impact of changed  $\rho b$  on SWC

Initially this result caused some confusion. The RETC output file (RETC.OUT) for each scenario was compared. The initial hydraulic properties were different for each scenario. However the iterative approach used by RETC to develop the soil water characteristic when combined with the selected curve fitting option (using observed retention data) resulted in the output for each scenario converging

to the same result. In hindsight this became clear and indicated that the 'Retention data only' fit option was inappropriate for this modelling.

### 3.6.2. Forward problem and correction factor function

The second attempt used the 'Forward problem' fitting option (i.e. no retention data was used in predicting the soil water characteristic). The substantial difference between the RETC predicted results using the 'Forward problem' and the observed retention points, as visually demonstrated in Figure 15, was identified as a problem. To overcome this, a correction factor function was determined by:

- Determining the difference between the RETC predicted forward problem and UNSODA observed functions for 100 equally spaced water potential data points ( $\psi$  range 1 cm to 15,000 cm).
- Use the LINEST function to complete a curve fit of the difference and derive the 'a' and 'b' parameters to develop a correction factor function.
- The correction factor function was added to the curve fit function derived from the RETC forward problem solution.

An example of the correction factor function is detailed in Figure 19. The blue line represents the curve fitted function of the observed water retention data from the UNSODA data base. The red line is the RETC predicted SWC function using the forward problem fit type. The difference between the observed and predicted functions is the purple line. The green points are the total of the RETC predicted SWC and correction factor functions, which provides a very good approximation of the original UNSODA observed function. Equation 3 represents the SWC for the clay soil code 2620 with the original  $\rho_b$ .

$$\theta = (-0.136 \times \log_{10} \psi) + 0.703 + (0.055 \times \log_{10} \psi) - 0.081$$
 Equation 16

The results demonstrated in Figure 19 validate the correction factor approach to using the RETC forward problem fitting option. This approach was taken for all of the soils with similar results.



Figure 19 Example development of the correction factor function for clay soil code 2620 (logarithmic x scale)

With this approach validated, five bulk densities were simulated for each soil sample being the original  $\rho_b$  value and subsequent increments of 0.1 g / cm<sup>3</sup>. For example, the following  $\rho_b$  values were used for clay soil code 2620: 1.07, 1.17, 1.27, 1.37 and 1.47 g/ cm<sup>3</sup>.

The results for the clay, loam and loamy sand soils are detailed in Figure 20, Figure 21 and Figure 22 respectively with a logarithmic x-axis. For all soils the change in  $\rho_b$  results in a change to the SWC. As  $\rho_b$  increases,  $\theta$  decreases for a given  $\psi$ . This is particularly the case for smaller  $\psi$  values. As  $\psi$  increases  $\theta$ , particularly for the clay and loam textured soils either converge or come close to converging. This is not the case for the loamy sand soils where the results appear less reliable with the SWC intersecting and in some instances negative  $\theta$  values being predicted.



Figure 20 Soil Water Characteristic with increasing  $\rho_b$  for two clay soils







Figure 22 Soil Water Characteristic with increasing  $\rho_b$  for four loamy sand soils

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## 3.7. Soil Water Characteristic Pedotransfer development

PTF enable the prediction of soil hydraulic properties without requiring extensive measurement of these properties. This section, based on the modelling completed to date aims to determine whether a PTF can be developed to predict the SWC of a soil as the  $\rho_b$  increases. A reliable PTF that achieves this aim could have significant benefits across a number of agronomic and hydrologic applications relating to soil water.

#### 3.7.1. Relationship between SWC 'a' and 'b' regression coefficients

The correction factor approach taken in modelling the SWC as  $\rho_b$  increases results in two regression coefficients for 'a' and 'b.' The SWC regression coefficients have been determined for all soils and all  $\rho_b$  values.

Relationships between these coefficients were analysed to determine their predictive value for an individual soil, for soils of a single texture class and for all soils. This was achieved by obtaining the regression equation from a scatter plot of the 'b' versus the 'a' coefficients.

Figure 23 details the scatter plots for each of the soils grouped by textural class and highlights that for all soils there is a linear relationship between the coefficients. Figure 24 includes a line of best fit and regression equation for the 'a' and 'b' coefficients grouped by textural class. There is a strong relationship between these coefficients for clay ( $r^2 = 0.93$ ) and loam ( $r^2 = 0.99$ ) soils and a moderate relationship for the loamy sand textured soils ( $r^2 = 0.76$ ).







Figure 23 Relationship between 'a' and 'b' coefficients for clay, loam and loamy sand soils

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Figure 24 Relationship between 'a' and 'b' coefficients by soil textural class

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Figure 25 demonstrates that the global relationship for all soils is weak ( $r^2 = 0.20$ ) indicating that the relationship between the coefficients is potentially useful at the soil textural class level but not for all soils.



Figure 25 Relationship between 'a' and 'b' coefficients for all soils

For the relationship between the coefficients to be practically useful requires a relationship to be established between the coefficients and a measurable soil property such as  $\rho_b$ . Figure 26 details the relationships between the 'b' coefficient and  $\rho_b$ . Moderate relationships exist between the coefficients with r<sup>2</sup> values of 0.55, 0.75 and 0.58 for the clay, loam and loamy sand textural classes respectively.





To determine whether the 'a' and 'b' coefficients are useful at the soil textural class level the regression equations relating the 'b' coefficient to the initial  $\rho_b$  value and subsequently the 'a' coefficient to the 'b' coefficient were applied. The resulting coefficients along with the coefficients obtained from the RETC Forward (Direct) data fitting option are detailed in Table 12. The average difference between the modelled and RETC 'a' and 'b' coefficients are -3.9% and -4.1% respectively. Soil 3130 (loamy sand) can be considered as an outlier because the highest observed  $\psi$  data point is 834 cm compared to the other soils with the highest  $\psi$  data point being at or around 15,000 cm. If the

coefficient for soil 3130 is thus ignored the average difference between the modelled and RETC 'a' and 'b' coefficients reduces to 1.3% and -1.2% respectively.

			Modelled	RETC	% diff	Modelled	RETC	% diff
Texture	Code	$\rho_b$	'a'	'a'	<b>Δ'a'</b>	'b'	'b'	Δ'b'
				-				
Clay	2620	1.07	-0.1468	0.1362	7.8%	0.7089	0.7031	0.8%
				-				
Clay	4120	1.24	-0.1115	0.1205	-7.4%	0.6253	0.6458	-3.2%
				-				
Loam	2650	1.08	-0.1529	0.1514	1.0%	0.6754	0.6726	0.4%
				-				
Loam	2580	1.07	-0.1538	0.1514	1.6%	0.6790	0.6836	-0.7%
				-				
Loam	2590	1.22	-0.1408	0.1356	3.8%	0.6258	0.6016	4.0%
				-				
Loam	2750	1.01	-0.1590	0.1514	5.0%	0.7003	0.6774	3.4%
Loamy				-				
Sand	2570	1.13	-0.1518	0.1560	-2.7%	0.5105	0.5900	-13.5%
Loamy				-				
Sand	3130	1.5	-0.1229	0.1883	-34.8%	0.4150	0.5183	-19.9%
Loamy				-				
Sand	1090	1.59	-0.1158	0.1279	-9.4%	0.3917	0.4264	-8.1%

Table 12 RETC generated and model predicted values for 'a' and 'b' coefficients

These coefficients were used to generate a SWC for each soil using Equation 1. The results for a representative soil from each texture class are detailed in Figure 27 and compared with the SWC curve fitted function developed from the RETC Direct fit data output (i.e. using the RETC coefficients detailed in Table 12) as well as the observed data from the UNSODA database. The results across all soils (less the outlier soil code 3130) were similar and indicate that the approach of obtaining coefficients at the textural class level and then applying the coefficients to the  $\rho_b$  of an individual soil result in a SWC that matches closely with the SWC predicted from RETC using the Direct fitting option. The predicted SWC does differ considerably from the observed values however the addition of a correction factor function (as was applied to the direct fitting option) could enable a close match to the observed data.



Figure 27 SWC comparison by textural class

To achieve this, regression coefficients of the correction factor function coefficients (Table 13) for each soil were averaged by textural class (Table 14).

Soil code	Texture	'a' coefficient	'b' coefficient
2620	clay	0.054967	-0.080731
4120	clay	0.062084	-0.099439
2580	loam	0.070317	-0.036020
2590	loam	0.052848	-0.064232
2650	loam	0.053620	-0.113744
2750	loam	0.050584	-0.034962
1090	loamy sand	0.025544	-0.019199
2570	loamy sand	0.033149	-0.033427
3130	loamy sand	-0.024440	0.158535
4010	loamy sand	0.010645	0.046412

# Table 13 Correction factor coefficients

#### Table 14 Average 'a' and 'b' coefficient values

Texture	'a' Average	'a' STDEV	'b' average	'b' STDEV
Clay	0.0585256	0.003558	-0.090085	0.009354
Loam	0.056842	0.03197	-0.062239	0.03197
Loamy sand	0.0112242	0.075784	0.0380802	0.075784

The correction factor function was then added to the predicted SWC and plotted. The results for six soils are detailed in Figure 28. For most of the soils this approach produced a SWC that was similar to the observed water retention data points however was not as consistently accurate as the SWC developed from the individual soils modelling.





This modelling indicates that a PTF can be developed that enables the SWC to be predicted reasonably accurately as  $\rho_b$  increases. Further modelling using a larger sample size and more soil types should be completed to refine the PTF and test its usefulness. More sophisticated analysis techniques, such as regression trees, could also be applied to identify whether other soil parameters, such as clay content, could be incorporated to improve the reliability/applicability in predicting the SWC.

#### **3.8. Modelling the Hydraulic Conductivity Function**

The Hydraulic Conductivity Function (HCF) is the second function required to fully describe soil hydraulic behaviour. Mualem's hydraulic conductivity model is most commonly coupled with the VG retention equation (Radcliffe & Simunek 2010). The HCF can be expressed in terms of  $\psi$  or  $\theta$ . Mualem's model in terms of both effective saturation ( $S_e$ ) and  $\psi$  are detailed in Equation 11 (Radcliffe & Simunek 2010) and Equation 12 (van Genuchten et al. 1991) where the effective saturation ( $S_e$ ) = ( $\theta - \theta_r$ )/( $\theta_s - \theta_r$ ) and m = 1 - 1/n.

The parameters required to model these functions are provided when the Retention data fit option is used in RETC. Figure 29 details a plot the HCF for loam soil 2580 from the log(K) values in the



RETC output file and log(K) values calculated using Equation 12. As would be expected the results align exactly.

Figure 29 Plot of Mualem equation against RETC Predicted log(K) output

The RETC predicted values for HCF for each value of  $\rho_b$  were plotted. An example is provided in Figure 30 (plotted on a log – log scale). In a corollary to the SWC, as the  $\rho_b$  of the soil increases the hydraulic conductivity reduces for a given matric potential. The results were similar for all soils across the texture classes.



Figure 30 Example HCF for loam soil (Code 2580)

The similar shape of the curves suggests that there is a relationship between the hydraulic conductivity functions. Regression equations for each of the HCFs were developed. Polynomials of degree six were found to be the best fit with the  $r^2$  value approaching 1 in all instances.

Whilst developing a relationship between increasing  $\rho_b$  and the HCF is important, further modelling was not required to meet the objectives of the project. As such this aspect of the modelling was not progressed any further but would be a useful extension of the project.

# **3.9. Modelling infiltration**

The modelling completed using RETC from both the observed and predicted soil hydraulic properties provide the soil hydraulic parameters required in HYDRUS-1D. Whilst these parameters can be predicted within HYDRUS-1D using the ROSETTA PTF, the accuracy of the results are poor compared to those obtained from the Retention fitting option in RETC which is calculated from observed data. Five independent soil hydraulic parameters ( $\theta_r$ ,  $\theta_s$ ,  $\alpha$ , *n*, *l* and  $K_s$ ) are required by HYDRUS-1D. The first four parameters have been obtained from RETC. '1' has been assumed to be a constant value of 0.5 which is common practice (Šimůnek et al. 2009) and  $K_s$  has been derived from the ROSETTA PTF.

A number of scenarios were modelled to investigate the impact of increased  $\rho_b$  on infiltration. The scenarios were:

- Infiltration as  $\rho_b$  increases on a single layered soil under ponding conditions
- Infiltration on a two layered soil (with the top layer representing a surface crust as a uniform layer) under ponding conditions

• Infiltration on a multi layered soil (with the top layers representing a surface crust of nonuniform  $\rho_b$ ) under ponding conditions.

For all scenario's a soil profile segment of 100 mm depth was selected which allows 1 mm segmentation within HYDRUS-1D. Additionally the weighting of the segments was changed to 0.1 to allow much greater fidelity in modelling infiltration through the surface crust. The  $\psi$  value was kept at constant value of 100 mm throughout the soil profile segment (the default value in HYDRUS-1D) except for the top of the soil surface which was assigned a value of 0 to represent ponding conditions.

## 3.9.1. Infiltration on a single layered soil under ponding conditions

The initial modelling of infiltration into a single layered soil under ponding conditions was modelled in HYDRUS-1D for two scenarios. The first scenario used the soil hydraulic parameters obtained from the ROSETTA PTF using the SSCBD data only (i.e. the Direct fit option in RETC). The second scenario used the soil hydraulic parameters obtained by using the SSCBD, water retention and hydraulic conductivity (where available) data (i.e. the Retention or Retention and Conductivity fit options in RETC).

Figure 31 details the results for the clay soil 2620 highlighting that the infiltration predicted using the Direct fit derived soil hydraulic properties is significantly higher at all times, including the steady infiltration rate. The steady infiltration rate is 157% higher in the Direct Fit modelled infiltration.



Figure 31 Infiltration rate comparison using different Soil Hydraulic Properties

To model the impact of increasing  $\rho_b$  on infiltration the 'correction factor' SWC obtained for each of the values of  $\rho_b$  was used to provide water retention data. This data was then used as an input into RETC from which the VG parameters could subsequently be obtained. Only the  $\theta_s$ ,  $\theta_r$ ,  $\alpha$  and nparameters were fitted using the 'Retention data only' fitting option in RETC. The *l* parameter was left as the default value of 0.5 and the  $K_s$  value was that predicted by the ROSETTA PTF using the SSCBD data. Retention Curve data was added for 10 values of  $\psi$  ( $\psi = 1, 151, 601, 1201, 2401, 5101, 6601, 8250, 10050$  and 15000 cm). The RETC determined VG parameters were subsequently used as the soil hydraulic parameters in a 20 minute infiltration simulation on HYDRUS-1D. All other parameters remained constant except the soil hydraulic parameters between the simulations. The results for three soils (Clay 2620, Loam 2750 and Loamy Sand 4010) are detailed in Figure 32, Figure 33 and Figure 34.



Figure 32 Impact of increasing  $\rho_b$  on infiltration for clay soil (2620)



Figure 33 Impact of increasing  $\rho_b$  on infiltration for Loam soil (2750)



Figure 34 Impact of increasing  $\rho_b$  on infiltration for Loamy Sand soil (4010)

The results highlight that as  $\rho_b$  increases both the initial infiltration rate and steady infiltration rate reduce. The water retention data used in determining the soil hydraulic properties for the initial  $\rho_b$  was observed data whereas for the other simulations the predicted data from the derived SWC. This explains why the infiltration curve is not as smooth for the initial value of  $\rho_b$  when compared to the predicted values. The reduction in infiltration rates is substantial in all instances, as shown in Table 15 and highlights the impact of increasing  $\rho_b$  on infiltration.

Clay - 2620		Loam - 2750		Loamy Sand - 401	0
	Reduction		Reduction		Reduction
	in final		in final		in final
	infiltration		infiltration		infiltration
Density	rate	Density	rate	Density	rate
$\rho b = 1.07 \text{ g/cm}^3$		$\rho b = 1.01 \text{ g/cm}^3$		$\rho b = 1.44 \text{ g/cm}^3$	
$\rho b = 1.17 \text{ g/cm}^3$	44%	$\rho b = 1.11 \text{ g/cm}^3$	50%	$\rho b = 1.54 \text{ g/cm}^3$	33%
$\rho b = 1.27 \text{ g/cm}^3$	56%	$\rho b = 1.21 \text{ g/cm}^3$	56%	$\rho b = 1.64 \text{ g/cm}^3$	36%
$\rho b = 1.37 \text{ g/cm}^3$	72%	$\rho b = 1.31 \text{ g/cm}^3$	71%	$\rho b = 1.74 \text{ g/cm}^3$	56%
$\rho b = 1.47 \text{ g/cm}^3$	83%	$\rho b = 1.41 \text{ g/cm}^3$	81%	$\rho b = 1.84 \text{ g/cm}^3$	69%

Table 15 Reduction in steady infiltration rate from initial  $\rho_b$  value

The reduction in infiltration as  $\rho_b$  increases predicted in the modelling aligns with the findings in the literature.

#### 3.9.2. Infiltration on a two layered soil with a uniform crust under ponding conditions

The first crusted soil infiltration simulation considered the crust as being uniform, that is the  $\rho_b$  of the crust was uniform throughout the depth of the crust. A two layer profile was set up in HYDRUS-1D with the first layer (representing the crust) ending 5.339 mm below the soil surface and the second layer ending 100 mm below the surface. A crust depth of 5 mm was assumed, being in the middle range of crust depths reported in the literature. HYDRUS-1D allows the distribution of nodes to be increased near the surface compared to the bottom of the soil profile. This option was selected which resulted in the surface layer being changed to a depth of 5.339 mm.

The crust layer was assigned the VG parameters obtained for the highest value of  $\rho_b$  that had been modelled (i.e. the initial  $\rho_b$  plus 0.4 g/cm<sup>3</sup>). The VG parameters developed from the initial  $\rho_b$  was applied to the underlying soil layer. The value for  $K_s$  was obtained for both layers using the ROSETTA PTF.

A 20 minute infiltration simulation was run under ponding conditions represented by a pressure head of zero at the soil surface. The initial pressure head throughout the remainder of the soil profile was kept at the default value of -100 cm.

# **3.9.3.** Infiltration on a multilayered layered soil with a non-uniform crust under ponding conditions

The literature (Roth 1997) indicates that surface crusts are not of uniform density and should be considered as non-uniform layers with a  $\rho_b$  that gradually reduces until the  $\rho_b$  of the underlying soil is reached. A non-uniform crust was modelled by splitting the surface crust into four layers each with a lower  $\rho_b$  than the  $\rho_b$  of the layer above it as detailed in Table 16. The VG parameters developed from the correction factor function in RETC was applied to the crust layers. The  $K_s$  values calculated by the ROSETTA PTF were used. All other conditions were kept the same.

Tuble 10 bon prome set up for non uniform crust				
Layer	ρ <sub>b</sub>	Depth below surface	Comment	
		(mm)		
1 (top of profile)	Initial $\rho_b + 0.4 \text{ g/cm}^3$	0.0 - 0.826	Crust	
2	Initial $\rho_b + 0.3 \text{ g/cm}^3$	0.826 - 1.917	Crust	
3	Initial $\rho_b + 0.2 \text{ g/cm}^3$	1.917 - 3.273	Crust	
4	Initial $\rho_b + 0.1 \text{ g/cm}^3$	3.273 - 5.339	Crust	
5 (bottom of profile)	Initial $\rho_b$	5.339 - 100.0	Underlying soil	

Table 16 Soil profile set up for non-uniform crust

#### 3.9.4. Results from infiltration modelling on crusted soils

Simulations for the uniform and non-uniform crusts were modelled for one soil from each of the three texture classes (Clay 2620, Loam 2750 and Loamy Sand 4010). Example results for infiltration into the Clay soil 2620 are detailed in Figure 35.



Figure 35 Comparison of infiltration with a soil crust - Clay soil 2620

The difference in infiltration rates and cumulative infiltration rates are not as great as could be expected. The reason for this was considered to be that the  $K_s$  predicted by ROSETTA were greater than has been observed elsewhere in surface seals. For example the  $K_s$  value predicted by ROSETTA for the Loam soil was 0.980 cm/h. This compares with a measured hydraulic conductivity value for a seal on a loam soil of 0.062 cm/h (Moore 1981). The potential for a substantial overestimation of infiltration using the ROSETTA PTF derived  $K_s$  value is thus possible.

#### 3.9.5. Comparing ROSETTA predicted K<sub>s</sub> values

To investigate whether ROSETTA predicted *Ks* values are appropriate for application to a surface crust the  $K_s$  values for the same three soils were predicted using all five options within the ROSETTA PTF (textural class, Sand, Silt and Clay percentages (SSC), SSC plus  $\rho_b$ , (SSCBD), SSCBD plus  $\theta$  at 33 kPa and the same plus  $\theta$  at 1500 kPa). The  $\theta$  values were obtained from the SWC curve fit (including the correction factor function) of the modelled data.

ROSETTA uses a hierarchical model to determine parameters such that the more input data that is provided the more accurate the prediction of soil hydraulic properties including  $K_s$  (Schaap et al. 2001).



Figure 36 Comparison of ROSETTA PTF predicted  $K_s$  values for three soils. (min) is for the lowest modelled  $\rho_b$  and (max) is for the highest modelled  $\rho_b$  value.

The results are detailed in Figure 36. The texture class and SSC values are the same for both the minimum and maximum  $\rho_b$  values for each soil as they rely upon the same data. It is clear that there are major differences in predicted  $K_s$  values depending upon what data is inputted into ROSETTA. Some *Ks* values change by one to two orders of magnitude when  $\rho_b$  and water retention data is included. Based on the design of the ROSETTA PTF, the  $K_s$  values predicted using  $\rho_b$  and water retention data should be more accurate than the values determined using the SSCBD data only (Schaap et al. 2001) thus providing more accurate predictions of infiltration.

#### 3.9.6. Modelling infiltration with new K<sub>s</sub> values

Based on the difference in  $K_s$  values identified in Figure 36 the infiltration simulations were repeated with  $K_s$  values obtained from ROSETTA using all of the available data (SSCBD and  $\vartheta$  at 33 and 1500 kPa).

The impact upon infiltration rate for a homogenous soil profile as  $\rho_b$  increases are detailed in Figure 37 (Clay 2620), Figure 38 (Loam 2750) and Figure 39 (Loamy Sand). The results indicate that there is a substantial difference in infiltration rates as  $\rho_b$  increases. Similar results were obtained for cumulative infiltration.

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Figure 37 Impact of increasing  $\rho_b$  on infiltration rate - Clay 2620



Figure 38 - Impact of increasing  $\rho_b$  on infiltration rate - Loam 2750


Figure 39 - Impact of increasing  $\rho_b$  on infiltration rate - Loamy Sand 010

Infiltration simulations for an un-crusted, uniform crust and non-uniform crust were repeated. The results are detailed in Figure 40 (Clay 2620), Figure 41 (Loam 2750) and Figure 42 (Loamy Sand 4010).

Table 17 details the differences in cumulative infiltration compared to the un-crusted soil. The results highlight a substantial difference in infiltration rates between the different crust scenarios for the Clay and Loam soils. The Loamy Sand soil appears as an anomaly with infiltration increasing under the uncrusted soil scenario. All input data has been checked for this simulation and the results remain the same. The result for the Clay and Loam soils are a more accurate reflection of the expected impact of surface crusts upon infiltration based upon the findings from the literature review than the modelling results using the less accurate SSCBD derived  $\rho_b$  values. Experimental data will be compared with these results to determine the validity of this modelling approach.

	2620 Clay		2750 Loan	n	4010 Loamy Sand	
	Infiltration (mm)	% difference from uncrusted	Infiltration (mm)	% difference from uncrusted	Infiltrati on (mm)	% difference from uncrusted
Uncrusted	7.3	unerusteu	18.7	unerusteu	11.8	unorusteu
Uniform Crust	2.2	-70%	12.9	-31%	19.6	66%
Non Uniform						
Crust	6.0	-18%	17.1	-9%	10.3	-13%

Table 17 Comparison of cumulative infiltration differences for three soils



Figure 40 Infiltration under three crusting scenarios - Clay 2620



Figure 41 Infiltration under three crust scenarios - Loam 2750



Figure 42 - Infiltration under three crust scenarios - Loamy Sand 4010

## 3.10. Summary

A number of conclusions can be drawn from the modelling that has been completed.

Firstly the accuracy of the RETC predicted SWC improved considerably when both observed water retention data and SSCBD data was used. The fitted SWC function using Equation 1 resulted in mean  $r^2$  values of 0.967, 0.978 and 0.894 for the clay, loam and loamy sand textural classes, respectively. This indicates that the developed SWC functions provide an accurate representation of the observed data. As a result the developed SWC functions were used as a baseline against which the impact of increased  $\rho_b$  was compared.

The observed soil water retention data could not be used to model the SWC as  $\rho_b$  increased. This required the use of the substantially less accurate forward fitting option in RETC where only SSCBD data was used to predict the hydraulic properties. To improve the accuracy of the direct fit SWC a correction factor was applied which resulted in a very good alignment with the observed data.

An assumption was made that the correction factor could be applied to the SWC functions as  $\rho_b$  was iteratively increased. Whether this assumption is valid needs to tested experimentally. The modelled SWC for each soil changed as the  $\rho_b$  was increased with, in the main,  $\theta$  being less for a given value of  $\psi$  as the  $\rho_b$  increased until  $\theta$  values converged/nearly converged at large matric potentials. This behaviour was expected.

The SWC function (Equation 1) required two fitted coefficients ('a' and 'b') which were obtained through linear regression. There was a strong linear relationship between these coefficients for each soil, a moderate relationship at the textural class level and a weak relationship when all soils were considered. A moderate relationship also existed between the  $\rho_b$  and the coefficients. To determine whether these relationships were of practical use values for the coefficients were developed using the regression equations developed for each textural class. These coefficients were then applied to all soils and compared with the SWC functions developed using the RETC Retention Data fit option. In all instances the SWC obtained through the regression equations compared very poorly with the RETC developed SWC. This indicates, based on the small dataset and limited textural classes considered that this global approach does not assist in developing an accurate SWC functions. The approach did however develop reasonable predictions of the SWC at the textural class level.

Textural class has been identified as having an impact upon the modelling results. The coarse grained textural class (loamy sand) had consistently less reliable results than the fine and medium class soils examined. This indicates that the modelling approach taken here may vary in accuracy/applicability dependent upon the textural class.

Further modelling is required to extend the work already completed on the development of the SWC to the Hydraulic Conductivity Function ( $K(\psi)$ ) which will enable a complete description of the impact of increasing  $\rho_b$  on soil water. Expanding the modelling to cover different soil textural classes would also be of use, particularly to examine the relative reliability of the modelling approach for the coarse grained classes.

Validating the identified impact of increased  $\rho_b$  on the SWC as identified during the modelling requires experimentation. Experimentation will require obtaining water retention data ( $\theta$  and  $\psi$ ) for soils at several  $\rho_b$  values. This will enable a comparison of the predicted versus observed SWC and subsequently validation of the modelling that has been completed. This experimentation could be extended to investigate whether a compacted soil behaves the same when it includes a surface crust.

Infiltration modelling demonstrated that as the  $\rho_b$  increased the infiltration rate decreased, supporting what has been identified in the literature. The initial infiltration results into a crusted soil profile, particularly a non-uniform crust, were somewhat surprising. Whilst the uniform crust results in a lower infiltration rate than the other two scenarios the final infiltration rate of the non-uniform crust is approximately the non-crusted soil.

These results led to a comparison of predicted  $K_s$  values using the various options in the ROSETTA PTF. This identified that there are differences, some substantial, in the  $K_s$  values between the different data input options. The ROSETTA PTF is designed such that the more data provided, the more accurate the predicted  $K_s$  value. As such for all modelling requiring  $K_s$  the SSCBD and two water retention point option predicted  $K_s$  values was applied to one soil from each texture class.

The infiltration modelling was repeated using the new  $K_s$  values. For the clay and loam soils the results were as was expected with substantial differences as  $\rho_b$  increased and in the three crusting scenarios. The results from the loamy sand however indicated an increase in the infiltration rate under a uniform crust compared to an un-crusted soil. Whether this is anomaly for that particular soil or representative of modelling limitations on coarser grained soils is yet to be determined.

# 4. CHAPTER FOUR – METHODS AND MATERIALS

# 4.1. Overview and Experimental Approach

The aim of the experimental component of this project was to collect experimental data that could be analysed to determine whether measuring the  $\rho_b$  of the surface crust would lead to improved modelling predictions of infiltration and/or runoff. This experimental design was based around the conclusions drawn from the literature review.

To achieve this aim the following experimental work was planned:

- Source soils for use in rainfall simulation experiments and soil hydraulic property determination.
- Measure the relevant soil physical and chemical properties.
- Conduct rainfall simulation experiments that would provide two outputs: 1) runoff and infiltration data, and 2) a soil crust.
- Measure the  $\rho_b$  of the surface crust using multiple methods.
- Determine the hydraulic properties of soil samples of varying  $\rho_b$  to provide  $\theta$  data required for HYDRUS-1D modelling and validate predicted soil hydraulic properties.
- Use measured soil properties to model infiltration using HYDRUS-1D and compare against observed infiltration from the rainfall simulation experiments.

It was assessed that this experimental approach would enable the aim of the project to be met.

## **4.2. Soils**

Four soils were used during the project (summarised in Table 18). The A horizon from a Sodosol sourced from near Millmerran, Queensland, was the primary soil used, with approximately 0.6 m<sup>3</sup> of the A horizon (0–20 cm depth) being collected. This was sufficient soil to fill three large rainfall simulator trays (see Rainfall simulation section) for the rainfall simulation experiments. The Sodosol was selected based on their tendency to form surface seals/crusts (Murphy 2015). Surface sealing was in evidence throughout the area surrounding the sampling site (Figure 43). Smaller quantities of three other soils were also collected to enable a comparison to be made against the Sodosol as well as providing different soil textures for soil hydraulic property modelling. It was expected that the Chromosol and Ferrosol samples would form a crust during the rainfall simulation experiments based on observations of crusting in the vicinity of the sampling sites.

Soil	Australian Soil Classification	Sourced from
1	Sodosol	Millmerran QLD
		Lat: 27.85553
		Long: 151.23521
2	Chromosol	Riverhills QLD
		Lat: -27.563494
		Long: 152.919188
3	Ferrosol	Sumner QLD
		Lat: -27.564151
		Long: 152.930637
4	Hydrosol	Riverhills QLD
		Lat: -27.561364
		Long: 152.898057

Table 18 Project Soil Overview



Figure 43 Surface seal in close proximity to the Sodosol sampling site at Millmerran Queensland

The soils were air dried, screened to remove large pieces of organic matter, stones and break up large aggregates. Manual mixing was used to homogenise the soil. Approximately 1 kg of soil of each sample was obtained from the homogenised soils for physical and chemical analysis. These samples were dried in an oven at 40  $^{\circ}$ C for three days and ground to pass a 2 mm sieve.

## **4.3.** Soil physical and chemical properties

Soil chemical and physical data was required primarily as input data for HYDRUS-1D modelling and assistance with soil classification. The soil physical and chemical data requirements and the applicable methods are detailed in Table 19.

Table 19 Soil Physical and Chemical Data R	Requirements and Standard Methods
--	-----------------------------------

Serial	Characteristic	Method /Reference	Comments/Reason
		Physical	
1	Particle Size Analysis	Hydrometer method (Burt 2014). This method was	Define soil texture
		selected as it would enable determination of both	Input for HYDRUS modelling
		Australian and US particle sizes. HYDRUS-1D uses	
		US particles sizes for sand, silt and clay.	
2	Moisture Content	Weighing of wet soil and oven dried soil	Input for calculation of $\rho_b$
3	Bulk density	503.01 Intact Core Method	Input for HYDRUS modelling
	(bulk soil)	(Cresswell & Hamilton)	
4	Aggregate Stability	Aggregate Stability in Water (ASWAT), (Field et al.	Results analysis/discussion (aggregate stability)
		1997)	
		Chemical	
5	pH	4A1 pH of 1:5 soil/water suspension	Soil classification
		(Rayment & Lyons 2011)	
6	Electrical Conductivity	3A1 EC of 1:5 Soil/water extract	Results analysis/discussion
	(EC)	Soil Chemical Methods Australasia (Rayment &	Determine EC of solution for saturated hydraulic
		Lyons 2011, pp. 20 - 2).	conductivity experiments

# 4.4. Rainfall simulation

Rainfall simulation was used for two primary purposes:

- Development of a soil seal/crust that could be sampled to determine crust  $\rho_b$  and associated soil parameters.
- To provide infiltration and runoff data to compare modelled infiltration results against.

The rainfall simulation activities generally followed the guidance provided in Connolly et al. (2002) adapted as required based on the particulars of the Landloch rainfall simulator used for experimental work and the requirements of this project.

## 4.4.1. Rainfall Simulator

A laboratory rainfall simulator (see Figure 44) from the environmental consultancy Landloch was used in this project. Landloch primarily uses this simulator for runoff and erosional studies. The simulator consisted of:

- An A-frame supporting three horizontally rotating Veejet nozzles.
- A control system enabling control of simulated rainfall application.
- A water supply system including rainwater tanks, a pump and associated plumbing.
- A mounting frame to hold two soil sample trays at a selected angle. The trays can be set at three different angles from the horizontal (0, 20 and 30 degrees) to simulate different slope angles. Two tray sizes were available. The dimensions of the large tray are 750 mm x 750 mm x 220 mm or 0.124 m<sup>3</sup>. The dimensions of the small tray are 400 mm x 400 mm x 115 mm or 0.0184 m<sup>3</sup>.



Figure 44 Landloch Rainfall Simulator with two large trays sitting on the mounting frame at a 20 degree angle.

The following settings were used for the rainfall simulator:

- Water supply pressure. 60 kPa +/- 1 kPa. Advice provided by *Landloch* staff indicated that this was the pressure required to provide simulated rainfall that is similar to natural rainfall in drop size and energy.
- **Dwell time**. When the simulator is operating the nozzles emit water continuously. At either extreme of the nozzles rotation the water is captured in baffles and returned to the water

supply system. The length of time that the nozzles remain at these extremes is known as the dwell time. Increased dwell time results in lower rainfall being applied to the soil samples.

- **Sweep speed**. Sweep speed is the time taken for the nozzles to rotate from one extreme to the opposite extreme of its axis. This is the time when the simulated rainfall is applied to the soil samples. Increasing the sweep speed increases the rainfall applied to the soil samples.
- **Rainfall intensity**. Varying the dwell time and sweep speed changes the rainfall intensity (mm/h). Simulator settings aimed to produce a rainfall intensity of 100 mm/h. After reviewing the literature (Armenise et al. 2018), (Hyväluoma et al. 2012) and (Moss 1991) a high intensity was selected as it was assessed that this would provide a high certainty of a surface seal developing. Each soil tray had rain gauges on each side, front and rear (four gauges total). The applied rainfall was determined by averaging the depth of rainfall in the gauges.
- **Tray angle**. Trays were mounted at a 20 degree angle to the horizontal based on the advice of Landloch staff.

## **4.4.2. Preparation for rainfall simulator experiments**

The rainfall simulator experiments included three parts being preparation, conduct and sampling.

Soil samples were prepared to ensure that they approximated the soil in a field condition (e.g.  $\rho_b$  and surface condition) and that there were no macro-pores or anomalies that would result in misleading results. Sample preparation consisted of the following steps:

- A layer of air dried soil was placed into the sampling tray.
- The layer was compacted manually using a ram (block of timber with a handle).
- The tray was filled with soil and compacted again.
- The soil surface was smoothed to align to the top of the tray.
- Additionally soil fines were added to the front lip of the tray and each corner until the soil was level with the lip on the tray. This step is critical otherwise runoff ponds on the soil surface and cannot be measured as runoff.
- The soil surface was gently wetted with a fine mist and allowed to air dry. This cycle was repeated several times.

An image of a packed soil tray is detailed in Figure 45.



Figure 45 Packed soil Tray

The layout of the tray (top view) is presented in Figure 46 Sample tray layout. A sampling exclusion zone of 100 mm around the edge of the sample was applied to minimise any edge effects.



Figure 46 Sample tray layout

**4.4.3. Conduct of rainfall simulator experiments** Experimental conduct consisted of the following steps:

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- A sample of the pre-simulation soil surface was obtained by scraping approximately 50 g of soil from the surface from multiple points across the sample surface. The surface was then gently smoothed by hand.
- Photographs were taken of the soil surface prior to rainfall application.
- The rainfall simulator was powered on and tested to ensure that the nozzles rotated freely.
- The pump to the rainfall simulator was turned on and the main valve adjusted until the correct pressure was obtained.
- A final check was made to ensure that all requirements had been completed prior to commencing the simulation.
- The rainfall simulator was activated and timing, using a stop watch, commenced.
- The outlet from the soil tray was observed to identify runoff commencement. Some water comes through the outlet almost immediately (some water enters the runoff collection chamber on each sweep of the simulator). This is not considered runoff. Runoff was determined once there was evidence of water ponding at the front of the soil tray and a continuous flow of water was flowing through the runoff outlet. This time was recorded.
- Approximately one minute after runoff commencement was determined the first sample was taken. This involved placing a 500 mL jar under the runoff outlet for approximately 15 seconds (exact time was recorded). Measurements were taken at one minute intervals, two minute intervals and five minutes intervals with the interval increasing as time progressed from runoff commencement.
- Rainfall simulation experiments were run for 25 minutes (for soils when runoff commenced within a couple of minutes of rainfall commencement) to 30 minutes (for soils that required longer time periods for runoff to commence. This gave a minimum of 20 minutes of runoff data.
- At the completion of the simulation all sample jars were weighed. Flocculent was added to jars containing greater than 50 mL of water and subsequently excess water drained. All samples were then placed into an oven set a 105 °C for 24 hours. The dry weights were then measured. This allowed calculation of runoff and sediment from each sampling.

## 4.4.4. Sampling

- Upon completion of each rainfall simulation experiment the soil within the trays was sampled. The sampling locations and method are detailed in Figure 47 Sampling locations and method
- Bulk density measurements were taken using the soil core method and the water retention method (Cresswell & Hamilton 2002). Three samples from each tray were taken for each method in the exclusion zone on the sides and rear of the tray. Samples were not taken from the front of the tray in case this impacted upon subsequent runoff results. Apart from small amounts of soil required to determine θ, the remainder of the soil was replaced in the trays and tamped until the soil surface was smooth.



Figure 47 Sampling locations and method

Sampling was conducted as follows:

- A PVC soil core ring (with a  $30^{\circ}$  bevel at one end) was gently inserted into the soil using a mallet and cork block until the soil surface aligned to the top of the ring. The PVC rings were left in place until all three rainfall simulation experiments were complete. To prevent further development of a surface seal a flywire mesh cover was gently inserted over the soil core ring (Figure 48). The fly-wire mesh was used to reduce the kinetic energy of incoming raindrops to a level that was less than what is required to create a seal. Upon completion of the rainfall simulation experiments the soil around the rings was gently excavated. The rings were removed, bottoms trimmed and weighed to allow  $\rho_b$  determination of the entire sample. The cores were then wrapped in plastic cling wrap and placed into a refrigerator/esky for storage.
- Three surface seal  $\rho_b$  samples were taken using the water retention method.
- Once the seal sample had been taken, a further sample to the depth of the wetting front was also taken to allow determination of  $\theta$ .



Figure 48 Flyscreen mesh placed over soil core ring to prevent thickening of the soil crust during subsequent rainfall simulations. Small pebbles on top of screen provide an indication of how rainfall can move particles.

# 4.5. Bulk density determination

The literature review identified that:

- Current soil survey methods do not determine bulk density of surface crusts.
- Most commonly used methods of determining  $\rho_b$  are not suitable for determining the bulk density of thin surface seals/crusts.
- Bulk density can be calculated indirectly via relationship between porosity and bulk density using an assumed (2.65 g/cm<sup>3</sup>) or measured value of particle density.
- There are advanced methods available to determine crust  $\rho_b$  such as X-Ray CT however they are not practical for routine use.

Five methods were used to obtain  $\rho_b$  being:

- The soil core method for the bulk soil underlying the surface crust
- The water retention method
- The thin slice method
- Modified intact clod method
- X-ray Computed Tomography.

These methods are explained in detail below.

## 4.5.1. Soil core method

A similar procedure as described in (Cresswell & Hamilton 2002) was used for determining the underlying soil  $\rho_b$ . Four  $\rho_b$  samples from the underlying soil were taken using a 50mm PVC soil core ring. The cores were inserted into the soil approximately 50 mm below the soil surface. The surrounding soil was excavated from around the core before it was removed. The top and bottom of the core were gently trimmed with a blade to ensure that they were flat. The cores were weighed and then a sample placed in an oven for 24 h at 105 °C to determine the moisture content. The dry weight of the sample (minus the mass of the core ring) was calculated using Equation 17 where  $\rho$  is the density of the moist sample and  $\theta$  is the gravimetric moisture content of the sample as a percentage.

$$\rho_b = \frac{\rho}{1+\theta}$$
 Equation 17

#### 4.5.2. Water Retention Method

The water replacement method is described in (Cresswell & Hamilton 2002). A schematic of a device to determine  $\rho_b$  using this approach is displayed in Figure 49.



Figure 49 Water Replacement Method (Cresswell & Hamilton 2002)

An apparatus similar to that shown in Figure 49 was constructed and is shown in Figure 50. The internal diameter of the annulus was 110 mm.



Figure 50 Water Replacement Method Apparatus

The following procedure was used to determine  $\rho_b$  using this apparatus:

- The apparatus was placed on the soil surface such that the three metal rods were inserted for stability and the underlying foam was flush with the soil surface.
- A thin plastic bag was inserted into the annulus and filled with water until the water level just touched the screw (forming a meniscus with the tip of the screw, a syringe was used to assist with this). This was the control volume.
- The plastic bag with the control volume was carefully removed and weighed.
- A sharp knife was used to cut around the circumference of the annulus to a shallow depth. A criss-cross pattern was then cut across the top of the seal. A bent teaspoon was used to carefully excavate the surface seal with the aim of achieving a relatively level excavation of the surface seal.
- The removed soil was weighed, placed in an oven at 105 °C for 24 hours and reweighed to obtain the dry mass of soil.
- A plastic bag was re-inserted into the annulus and water added until the water just touched the screw tip. The plastic bag was removed and weighed.
- The volume of the crust was calculated by subtracting the control volume from final volume enabling  $\rho_b$  to be calculated with the measured oven dry soil mass.

## **4.5.3. Thin Slice Method**

Surface seals/crusts are generally only millimetres in thickness. The aim of the thin slice method was to remove the surface crust from the underlying soil mass. This was to be achieved by inserting the 50 mm PVC soil core into the soil such that the surface crust was kept within the core. Once removed from the soil mass the core was placed on a timber spacer and the soil core forced downward until the

soil surface was just exposed above the soil core (see Figure 51). The crust was removed by slicing using a thin wire (a guitar string).



Figure 51 Thin slice method explanation. The left hand diagram shows the soil core once it is removed from the soil. The right hand diagram demonstrates how the soil surface was exposed above the core in preparation for removal using a thin wire.

Prior to removal of the surface crust from the soil, the height above the core was measured using callipers from which the volume of the soil can be calculated. The oven dry weight was then determined to allow calculation of surface crust  $\rho_b$ .

#### 4.5.4. Modified intact clod method

The thin slice method described in the section above, as is described in the next Chapter, was unsuccessful. As a result a modified version of the standard immersion method (Cresswell & Hamilton 2002) was used. The method was modified due to the small size and fragility of the soil crust fragments.

Samples of the Sodosols and Chromosol were taken at the completion of the rainfall simulation experiment (Figure 52). After air drying for a week, sandpaper was gently applied to the bottom of the crusts to achieve the thinnest crust section possible. These crust fragments were then oven dried.



Figure 52 Soil crust examples from a Sodosol prior to thinning

Crust  $\rho_b$  was determined, using both 10 mL and 100 mL measuring cylinders, as follows:

- The dry measuring cylinder was weighed using laboratory scales.
- Water was added to the measuring cylinder. The cylinder was weighed again.
- The crust fragment was weighed.
- The crust fragment was gently lowered into the measuring cylinder.
- The new volume was measured immediately after the crust was placed inside the cylinder by reading of the scale. The immediate measurement was required to minimise crust volume changes caused by rapid wetting/crust disintegration.
- The initial volume of water was calculated by determined by subtracting the cylinder mass from the recorded value. The difference between the initial and final volume is the volume of the sample.
- The crust fragment  $\rho_b$  was calculated.

#### 4.5.5. X-ray Computed Tomography (CT) method

The University of New England X-ray CT apparatus (GE Sensing & Inspection Technologies GmbH, Model v|tome|x s 240/180) was used to determine the porosity of soil core samples from which the crust and subsoil  $\rho_b$  were calculated. 50mm PVC soil core rings containing the soil core samples were placed on the sample mount within the CT apparatus (Figure 53).



Figure 53 Soil core on mount in X-ray CT scanner

The CT apparatus was calibrated prior to each scan. Scan settings are detailed in Table 20. The CT apparatus rotates the sample 360 degrees taking a total of 3200 images. Once the images have been completed a Beam Hardening Correction factor of two was applied before the three dimensional volume was reconstructed using the GE Sensing & Inspection Technologies GmbH phoenix datos|x 2 reconstruction software application (Version 2.2.1 RTM). The reconstructed scans were reviewed with the Volume Graphics VGStudio MAX 2.0.5 64 bit software application to check that the scan had been successful.

Setting	Value	Comment
Voltage	160 kV	Based on standard settings used
Current	120 µA	by UNE.
Power	19.2 W	
Resolution	30.121 μm	
Image Capture Timing	200 mS	
Number of images per 360	3200	
degree scan		
Mode of operation	Continuous	
Scan length	21 min 20 sec	

#### Table 20 X-Ray CT Scan Settings

Reconstructed volumes were prepared for analysis using the ImageJ software as follows:

- Each scan was imported into ImageJ.
- A circle with a diameter approximately 2.7 mm smaller than the internal diameter of the soil core ring was centred over an image slice (a top view of the scanned data). This was to filter out edge affects which were apparent in most scans. A check was made to ensure that this circle remained central over all image slices. The slice representing the top of the soil profile was identified (this varied from approximately the 100<sup>th</sup> slice to the 300<sup>th</sup> horizontal slice of the profile. A total of 1000 horizontal slices were scanned). The 900<sup>th</sup> slice was selected as the bottom of the profile to remove artefacts present in the bottom slices.
- This selection of data was saved as a 'stack' in .tif format for analysis using ImageJ.

90

Three outputs were required from the image analysis of the soil core stacks. These were:

- Determination of soil crust porosity (from which  $\rho_b$  can be calculated)
- Determination of bulk soil porosity (from which  $\rho_b$  can be calculated)
- Determination of soil crust thickness.

ImageJ scripts were developed to semi automate each of these tasks and are included in Appendix D.

The general approach to determining soil porosity for both the crust and bulk soil using ImageJ was as follows:

- Generate a vertical slice of the soil core image stack using the 'Reslice' function
- Forming a solid soil surface using Gaussian blurring
- Create a mask of the blurred image
- Create a Euclidean Distance Map (EDM) that enabled the quantification of the distance of a pixel from the soil surface.
- Application of the EDM and Mask to a binary (black and white) image of the soil slice.
- Calculation of the proportion of black pixels within the selected Region of Interest (ROI) using the 'Measure' function. This value was assumed to be porosity of the selected ROI. The bulk soil ROI was manually selected for each soil as a rectangular area from the bottom the image stack to a visually determined level below the soil crust. The ROI for the soil crust was selected as being between 10 and 40 pixels below the soil surface (0.3 and 1.2 mm). These settings were selected after testing multiple iterations with the aim of minimising the impact of unconsolidated soil material being incorporated into the ROI whilst maximising the crust thickness for all soil cores.

Porosity values were subsequently converted to  $\rho_b$  using Equation 6. The value for  $\rho_s$  was assumed to be 2.65 g/cm<sup>3</sup>.

Research by (Roth 1997) and others has identified that soil crusts should be considered as nonuniform layers where  $\rho_b$  changes with depth. To determine whether this conclusion is valid for these soils as well as determining crust thickness, the ImageJ script for determining the soil crust porosity was modified to measure porosity over thin sections parallel to the soil surface. Forty measurements were taken using 10 pixel depth increments (0.3 mm) commencing at one pixel below the soil surface. Values were calculated for two perpendicular vertical image slices per soil core and the results averaged.

This method is not as sophisticated as that used by Bresson et al. (2004), where horizontal sections were smoothed with increasing depth, however was assessed as being sufficient for the purposes of this project.

# **4.6.** Soil hydraulic properties

Measurement of the soil hydraulic properties was required for the following purposes:

- Determination of saturated hydraulic conductivity ( $K_s$ ) of soil cores of known  $\rho_b$  to compare with those predicted by HYDRUS-1D.
- Determination of the Soil Water Characteristic of soil cores of known  $\rho_b$  in order to obtain values of  $\theta$  at  $\psi$  values of 33 kPa and 1500 kPa. These values of  $\psi$  are required for subsequent HYDRUS-1D modelling.

• Provision of data to compare the Soil Water Characteristic (SWC) and Hydraulic Conductivity Function modelling completed during the modelling phase of the project with measured soil hydraulic property data.

## 4.6.1. Application of the modified Standard Compaction Test

HYDRUS-1D enables segmentation of a soil profile into layers. Each layer can be assigned a different  $\rho_b$  value. As it is very difficult to create thin crusts with a specified density, it was assessed that soil cores could be compacted to provide an analogue of the soil crust for the purposes of determining crust hydraulic properties.

A modified version of the Standard Compaction test (Vickers 1987) was used to create soil core samples of increasing  $\rho_b$ . The Standard Compaction Test is used to determine the optimum moisture content and corresponding soil density required for compaction during civil engineering works. The outcome required for this project was the creation of samples of increasing  $\rho_b$  such that soil hydraulic properties could be determined for each sample.

The following modifications were made to the Standard Compaction Test as follows:

- The air dried soil was sieved using a sieve with a 2 mm aperture to ensure that only the fine earth fraction was included in the sample.
- Water was added to the air dried sample to reach the approximate optimum moisture content required for maximum compaction as detailed in Table 21 Optimum moisture content range for different soil types . The soil was then manually mixed until the moisture content was consistent.

Table 21 Optimum moisture content range for different soil types (Arjun 2019)					
Sand	Sand silt or silty sand	Silt	Clay		
6 to 10%	8 to 12%	12 to 16%	14 to 20%		

• The number of blows with the 2.5 kg hammer per layer of soil was changed to result in differing  $\rho_b$ . The number of applied blows is described in the Results chapter.

## 4.6.2. Determination of saturated hydraulic conductivity

The aim of the saturated hydraulic conductivity experiments was to allow comparison of HYDRUS-1D predicted  $K_s$  values to observed data under different treatments. Saturated hydraulic conductivity of prepared soil cores was obtained using the constant head method (Das & Sobhan 2014). The experimental set up is shown in Figure 54 Hydraulic Conductivity experimental set up. The soil cores were placed in a Buchner funnel. The upturned plastic bottles had a polyethylene pipe segment with a sprinkler head attached inserted into the neck which enabled a constant head to be maintained when upturned. Plastic catch cups underneath the funnels were used to collect outflow which was weighed periodically. Two pore volumes were allowed to infiltrate through the soil core and measurements continued until five similar readings were obtained. The infiltration rate was calculated using a rearranged form of Darcy's law (Equation 13).



Figure 54 Hydraulic Conductivity experimental set up

A 2 dS/m Calcium Chloride solution was used for the infiltration experiments (including overnight soaking) based upon the EC measurement of the soil. This was to ensure that the calculated results were not impacted by the chemical composition of the soils (e.g. to prevent dispersion).

Experiments were completed on the following soil core treatments for both the Chromosol and the Sodosol with three replicates per soil:

- a. A loosely hand tamped soil core (low  $\rho_b$ ). To achieve consistency the mass of soil in the first core was measured and the same mass of soil placed into the other cores. The soil was then gently compacted until the depth of soil was the same as the first core.
- b. Cores compacted using the modified standard compaction test (high  $\rho_b$ ). Once the core was removed from the Proctor Device and the ends trimmed, a spacer was placed underneath the core and the soil core ring gently moved downward to expose approximately 2 cm of the soil core. This was removed using a sharp blade. The core was then inverted leaving 2 cm above the soil within the soil core ring where water for the constant head method could be ponded.
- c. Compacted cores with a surface crust (high  $\rho_b$ ). After drying for several weeks, the compacted cores from the previous step (b) were removed from the PVC soil core ring. The cores were subjected to artificial rain from a 9L garden watering can from a height of approximately 2 m with the aim of forming a surface crust. The core ring was replaced and the cores were allowed to dry for several days in the sun prior to the hydraulic conductivity tests. Petroleum gel was applied around the perimeter of the core, both top and bottom, to prevent preferential flow which could have occurred due to core shrinkage and damage from the artificial rainfall.

#### 4.6.3. Determination of the soil water characteristic

The SWC was obtained using the HYPROP apparatus (UMS GmbH nd). HYPROP uses an evaporation method to determine the SWC (UMS GmbH nd). 100 cc soil cores were extracted from larger soil cores prepared using the modified Standard Compaction test to produce samples with differing values of  $\rho_b$ . A sample of the soil from the larger core was oven dried to enable  $\theta$  determination and subsequently core  $\rho_b$  calculation. The samples were soaked until saturated in distilled water before being placed into the HYPROP sensor unit for determination of the SWC. Four

samples were run simultaneously with each measurement run being around five days in length. The SWC and VG parameters were obtained from each sample using the HYPROP-FIT software (Meter Group nd).

As 1500 kPa is outside of the tensiometer range of the HYPROP apparatus, it was planned to use a 1500 kPa pressure plate apparatus to obtain  $\theta_{1500}$ . This data point could then be incorporated into the HYPROP-FIT derived SWC. Due to issues with the results obtained from the HYPROP measurements (discussed in the Results Chapter) these measurements were not required.

# 4.7. Comparing observed infiltration to HYDRUS-1D predicted infiltration

The culmination of the experimental work is to compare observed infiltration rates from the rainfall simulator experiments with HYDRUS-1D predicted infiltration based on the measured soil parameters and in particular the  $\rho_b$  of the surface crust. This comparison enables an assessment to be made on whether measuring crust  $\rho_b$  improves the accuracy of infiltration modelling.

The following steps were required to enable this comparison:

- Observed infiltration data and parameters are prepared from the second and third rainfall simulation experiments for each soil type. The first simulation is not used as the soil crust had not developed and HYDRUS-1D does not support dynamic changes to the soil profile (e.g. changing crust ρ<sub>b</sub> with time) within a simulation. An infiltration rate-time regression equation was calculated for each soil.
- HYDRUS-1D simulations were completed using measured and derived parameters for the selected soils. Simulations were completed using the H3 and H4 models within ROSETTA (Schaap et al. 2001), as detailed in Table 22, to derive the VG soil hydraulic parameters.

Model	Input parameters	Comments
H1	Soil textural class	
H2	Sand, Silt, Clay (SSC)	USDA soil particle dimensions
		used
НЗ	SSC, Bulk Density (BD)	
H4	SSC, BD, water content at 3.3 m	
	suction ( $\theta_{3,3}$ )	
H5	SSCBD, $\theta_{33}$ , water content at	
	150 m suction ( $\theta_{150}$ )	

 Table 22 Models used to obtain VG equation parameters for use in HYDRUS-1 simulations.

• Modelled infiltration data from each simulation is compared with the observed infiltration regression equation. Statistical comparisons of observed and modelled infiltration rates are made using the Nash-Sutcliffe Criterion objective function. This objective function provides a numerical indication of the goodness of fit around the 1:1 line in predicted versus observed data comparisons (Hall 2001). Comparisons of cumulative infiltration between the observed and modelled data are also made.

After completion of these steps for each soil an assessment could be made as to whether measuring the crust  $\rho_b$  has improved the accuracy of infiltration modelling and which parameters are the most important in modelling infiltration on a crusted soil.

In the event that the modelling did not produce results similar to observed infiltration the inverse modelling capability (Radcliffe & Simunek 2010) within HYDRUS-1D could be used to obtain surface crust hydraulic properties.

# 5. CHAPTER FIVE - RESULTS

# 5.1. Soil physical and chemical properties

## **5.1.1. Physical Properties**

Soil texture and textural class details (using both the Australian and US particle sizes) are detailed in Table 23.

Table 23 Soil texture and textural classes using both the Australian and US particle sizes/classes. HYDRUS-1D uses the US particle size system.

	1	Australian	dimension	ns	US dimensions			
	Sand	Silt	Clay		Sand	Silt	Clay	
Soil	20 - 2000 μm	2 - 20 μm	<2 µm	Textural Class	50 - 2000 μm	2 - 50 μm	<2 µm	Textural Class
Sodosol A horizon	78%	8%	15%	Sandy Loam	72%	12.9%	15.5%	Sandy Loam
Sodosol B horizon	58%	4%	39%	Clay	56%	5.9%	38.1%	Sandy Clay
Chromosol	65%	8%	28%	Clay Loam	63%	14.7%	22.1%	Sandy Clay Loam
Hydrosol	43%	28%	30%	Silty Clay Loam	35%	40.6%	24.2%	Loam
Ferrosol	38%	13%	50%	Clay	33%	24.3%	43.1%	Clay

#### **5.1.2.** Chemical Properties

Soil chemical properties are detailed in Table 24.  $EC_e$ , salinity classification and aggregate dispersibility classifications have been obtained from Hazelton and Murphy (2016, pp. 189-91).

Soil	pН	EC (dS/m)	EC <sub>e</sub> (dS/m)	Salinity classification	ASWAT score	Aggregate dispersibility
Sodosol A horizon	5.7	0.08	1.12	Non Saline	0	Negligible/aggregated
Sodosol B horizon	5.7	0.14	1.05	Non Saline	13	Very high
Chromosol	5.8	0.08	0.69	Non Saline	0	Negligible/aggregated
				Highly		
Hydrosol	7.4	1.34	11.5	Saline	9	High to moderate
Ferrosol	6.4	0.19	1.43	Non Saline	0	Negligible/aggregated

#### **Table 24 Soil Chemical Properties**

## **5.2. Soil hydraulic properties**

### 5.2.1. Saturated hydraulic conductivity

Observed  $K_s$  values as well as a comparison with the ROSETTA predicted values (using the H3 model within ROSETTA (soil texture and  $\rho_b$  data only)) are provided in Figure 55 with the corresponding  $\rho_b$  of the soil cores detailed in Table 25.



Figure 55 Comparison of HYDRUS-1D predicted and observed saturated hydraulic conductivity. Error bars represent +/- one standard deviation

Soil	Uncompacted $\rho_b$ (g/cm <sup>3</sup> )	Compacted $\rho_b$ (g/cm <sup>3</sup> )
Chromosol	1.05	1.46
Sodosol	1.03	1.82

Table 25 Densities of soil cores used in K<sub>s</sub> experiments

The results detailed in Figure 55 highlight major differences in the observed and HYDRUS-1D predicted  $K_s$  values. Whilst the results are of the same magnitude the experimentally determined  $K_s$  values were on average double the HYDRUS-1D predicted values. The creation of a rainfall impact crust on the compacted soil cores resulted in a 72 % reduction in  $K_s$  for both soils highlighting the impact of surface crusts upon infiltration, as identified in the literature (Moore 1981).

HYDRUS-1D modelling was attempted using the soil texture, core  $\rho_b$ , HYDRUS-1D predicted and experimental  $K_s$  values to allow a comparison to be made. The numerical solution did not converge however for the compacted and crusted treatment (a problem that was found during subsequent modelling, see Section 5.6.2) so a comparison could not be made.

#### 5.2.2. Soil water characteristic van Genuchten parameters

Results from HYPROP soil water retention measurements are detailed in Table 26. The right hand most column details the content at 33 kPa calculated using Equation 10 and the HYPROP derived soil hydraulic parameters. This value was required for use in HYDRUS-1D to obtain soil hydraulic parameters from the ROSETTA H4 model.

Soil	$ ho_b$	α	n	$\theta_r$	$\theta_s$	$K_s$	$\theta_{33kPa}$
Chromosol	1.33	0.000131	1.439	0.4	0.889	1.62037E-05	0.85
Chromosol	1.41	0.0045	1.399	0.4	0.598	1.15741E-06	0.47
Chromosol	1.42	0.00368	1.655	0.325	0.560	7.07176E-05	0.37
Chromosol	1.48	0.00337	1.153	0.300	0.657	0.001469907	0.54
Chromosol	1.61	0.00117	1.952	0.198	0.305	2.43056E-05	0.23
Sodosol	1.63	0.01346	1.116	0	0.286	0.004085648	0.18
Sodosol	1.77	0.00769	1.126	0.250	0.483	0.000383102	0.40
Sodosol	1.78	0.00273	1.508	0.394	0.489	2.32639E-06	0.42
Sodosol	1.79	0.0027	1.644	0.034	0.223	0.00012963	0.08
Sodosol	1.79	0.0015	1.141	0.4	0.717	3.40278E-05	0.65
Sodosol	1.87	0.00381	1.108	0	0.350	0.000572917	0.26

 Table 26 HYPROP derived VG equation parameters

The soil hydraulic properties detailed in Table 26 show a great degree of variability even where the  $\rho_b$  values are the same or similar. This indicates that the results may not be reliable. The  $\theta_{33}$  data point was incorporated into several HYDRUS-1D simulations to determine whether this value had a significant impact upon predicted infiltration (see Section 5.6). There was little improvement when this water retention point was incorporated. As a result it was decided not to obtain the  $\theta_{1500}$  water retention values due to the difficulty in creating  $\rho_b$  samples of the same density, the time to obtain this data and the assessment that this data would make little difference in predicted infiltration results.

## 5.3. Rainfall simulation experiments

Rainfall simulations were conducted on three consecutive days using the Landloch rainfall simulator. A summary of the rainfall simulations is detailed in Table 27.

Tuble 1, Summing of Fundamental		
Soil description	Tray size	Number of simulations
Sodosol #1 A horizon	Large	3
Sodosol #2 A horizon	Large	3
Sodosol #3 A horizon	Large	3
Chromosol A horizon	Small	2
Ferrosol A horizon	Small	2
Hydrosol A horizon	Small	3

#### Table 27 Summary of rainfall simulation experiments

#### 5.3.1. Initial data

A spreadsheet was developed to record all data associated with the rainfall simulation (including soil details, timings for the simulation and sampling, applied rainfall, runoff as well as sampled wet and dry weights). The spreadsheet was also used to calculate the instantaneous runoff and infiltration rates. Infiltration curves were plotted from the infiltration rate data points. The original data from the rainfall simulation experiments is detailed in Appendix C.

The infiltration curves are detailed in Figure 56 through to Figure 61.



Figure 56 Infiltration curves for Sodosol #1



Figure 57 Infiltration curves for Sodosol #2



Figure 58 Infiltration curves for Sodosol #3

The Sodosol's demonstrated a general trend of infiltration rates declining with each cycle. When the soil was sampled at the end of the rainfall experiments the bulk soil felt dry (average measured  $\theta$  was 13%) indicating that most water had runoff and a crust had formed.





The infiltration rate on the second run of the Chromosol was initially less than the first run, as would be expected, however increased towards the end of the simulation. This may have been as a result of a preferential flow path being formed in the tray. The bulk soil of the Chromosol felt drier than the Sodosol's with an average measured  $\theta$  of 11.7 %. This indicated that most of the water had runoff as a result of a crust forming.



Figure 60 Infiltration curves for Ferrosol

The extremely high infiltration rate on the first run of the Ferrosol is as at least partially a result of insufficient packing of the front lip of the tray resulting in water pooling near the front edge but not running off. The  $\theta$  of the Ferrosol was not measured after the experiments due to an oversight however the soil felt saturated. There was no evidence of crust formation.



Figure 61 Infiltration curves for Hydrosol

The Hydrosol sample felt saturated after the rainfall simulation experiments with an average measured  $\theta$  of 54.5%. There was no evidence of crust formation.

Whilst samples demonstrated a general trend of infiltration rates declining overtime there was a substantial 'noise' within the data. Landloch staff indicated that this is normal from rainfall simulation experiments. As a result the data was cleaned before further analysis was completed.

## 5.3.2. Data cleaning

The rainfall simulation data was cleaned by removing unwanted outliers. The process applied was as follows:

- A data entry check was completed to ensure that hand written records from the rainfall simulation had been accurately transcribed into the spreadsheet.
- A zero value was applied to negative infiltration rates on the basis that a negative infiltration rates is physically impossible. It is assessed that negative infiltration rates occurred as a result of temporary ponding on the soil surface caused by rainfall impact reorganising the soil surface.
- Major increases in infiltration rate towards the end of the simulation were removed. This decision was made as it was assessed that the increases are likely to have resulted from preferential flow around the edges of the plot rather than infiltration into the soil mass. There was some evidence of this identified when emptying the trays were it was obvious in some of the trays that the moisture content on the sides of the plot was greater than in the middle of the plot.

## **5.3.3. Infiltration and runoff results**

Infiltration curves were replotted using the final data set for each simulation. A regression equation was derived using Microsoft Excel with the equation type (e.g. linear, polynomial or logarithmic) having the highest  $r^2$  value being selected. These equations are subsequently used to compare the observed infiltration rates to the HYDRUS-1D modelled infiltration rates. In general the  $r^2$  values for the first run were close to 1 and declined substantially on subsequent runs.

Soil	Run	Regression equation	$\mathbf{R}^2$
Sodosol #1	1	$I(t) = 1E-08t^2 - 2E-05t + 0.0142$	0.9455
Sodosol #1	2	$I(t) = 3E - 09t^2 - 6E - 06t + 0.0046$	0.7562
Sodosol #1	3	$I(t) = -6E - 04\ln(t) + 0.0043$	0.5203
Sodosol #2	1	$I(t) = 7E-09t^2 - 2E-05t + 0.0089$	0.9852
Sodosol #2	2	$I(t) = 3E - 09t^2 - 9E - 06t + 0.0075$	0.94
Sodosol #2	3	$I(t) = -5E - 04\ln(t) + 0.0036$	0.3426
Sodosol #3	1	$I(t) = 6E - 09t^2 - 1E - 05t + 0.0096$	0.9694
Sodosol #3	2	$I(t) = 3E - 09t^2 - 6E - 06t + 0.0046$	0.7562
Sodosol #3	3	$I(t) = -8E - 04\ln(t) + 0.0051$	0.2851
Chromosol	1	$I(t) = -7E - 04\ln(t) + 0.008$	0.6226
Chromosol	2	$I(t) = 4E-09t^2 - 5E-06t + 0.0038$	0.3947
Ferrosol	1	$I(t) = 3E-09t^2 - 8E-06t + 0.0297$	0.8823
Ferrosol	2	$I(t) = 3E-09t^2 - 9E-06t + 0.0082$	0.8267
Hydrosol	1	I(t) = 2E - 08t2 - 4E - 05t + 0.0266	0.774
Hydrosol	2	$I(t) = -0.00\overline{6}\ln(t) + 0.0445$	0.7129
Hydrosol	3	$I(t) = -2E - 09t^2 + 4E - 06t + 0.0059$	0.7624

Table 28 Regression equations and  $r^2$  values for observed infiltration where I(t) is the infiltration rate at time (t) and t is time in seconds

Cumulative infiltration was determined as follows:

- It was assumed that all rainfall applied prior to runoff commencement infiltrated into the soil. This was calculated by multiplying the average applied rainfall rate (mm/s) by the time until runoff commencement.
- Integrating the regression equation over the time period from runoff commencement until simulation end.
- Adding the two results together.

The cumulative infiltration results are displayed in Table 29. A trend across all soils is that total infiltration is the highest in the first run and declines in subsequent runs.

								Post runoff	Total
		Rainfall	Time to	Time to last	Pre runoff	First	Second	infiltration	infiltration
		intensity	runoff	measurement	infiltration (mm)	integration	integration	(mm)	(mm)
Soil	Run	(mm/s)	(s)	(s)	[Col C x Col D]	term	term	[Col G -Col H]	[Col F + Col I]
Sodosol #1	1	0.0317	50	1815	1.58	12.761	0.685	12.08	13.66
Sodosol #1	2	0.0336	40	1520	1.34	3.573	0.179	3.39	4.74
Sodosol #1	3	0.0294	45	1528	1.32	0.765	0.118	0.65	1.97
Sodosol #2	1	0.0289	39	1510	1.13	-1.328	0.332	-1.66	-0.53
Sodosol #2	2	0.0321	309	1512	9.92	4.509	1.917	2.59	12.51
Sodosol #2	3	0.0292	40	1542	1.17	0.662	0.090	0.57	1.74
Sodosol #3	1	0.0308	39	1215	1.20	7.870	0.367	7.50	8.70
Sodosol #3	2	0.0310	40	1520	1.24	3.573	0.179	3.39	4.63
Sodosol #3	3	0.0281	45	768	1.27	0.449	0.128	0.32	1.59
Chromosol	1	0.0282	78	1512	2.20	3.110	0.272	2.84	5.04
Chromosol	2	0.0272	76	933	2.06	2.452	0.275	2.18	4.24
Ferrosol	1	0.0293	669	1825	19.63	46.958	18.378	28.58	48.21
Ferrosol	2	0.0260	344	1605	8.95	5.703	2.329	3.37	12.33
Hydrosol	1	0.0285	400	1815	11.42	22.255	7.867	14.39	25.81
Hydrosol	2	0.0275	440	1825	12.08	9.941	0.410	9.53	21.62
Hydrosol	3	0.0288	344	1605	9.90	11.865	2.239	9.63	19.53

Table 29 Cumulative infiltration calculations from rainfall simulation experiments

## 5.3.4. Sources of error

As demonstrated in Figure 56 Infiltration curves for Sodosol #1through to Figure 61 there is significant variance in the recorded runoff data. This variance can be attributed to a number of factors, including the rainfall simulator, measurement of runoff and the soil/soil trays.

Potential sources of error from the rainfall simulator include:

• An average rainfall value based on the runoff collected from four rain gauges surrounding the runoff plot is used as the value for applied rainfall. There is substantial variance in the spatial distribution of the applied rainfall as highlighted in Table 30 which details the average rainfall in mm/h collected from each of the four gauges from each plot.

		Back					Back		
		114.74					108.30		
Left	115.32	Plot 1	108.53	Right	Left	105.17	Plot 2	100.17	Right
		102.36					92.17		
		Front					Front	]	

Table 30 Average rainfall (mm/h) collected in rain gauges

- The rain gauges located at the front and back of the trays were mounted by a magnet to the tray such that the top of the rain gauge was at the same angle to the horizontal as the soil tray (20 per cent gradient). The result is that the effective aperture of the rain gauge, particularly for the gauge located at the front of the tray is substantially reduced resulting in fewer rain drops entering the gauge. This effect can be noticed by the smaller values of recorded rainfall in the front gauge compared to the other gauges. The net effect is that the averaged applied rainfall per plot used in the calculations is less than the actual rainfall applied to the plot. This is estimated to be a difference in the applied rainfall average to a three gauge annual rainfall using the side and back gauges.
- The rainfall simulator is located outside in a semi-protected area but is still exposed to weather conditions, particularly wind. There were no noticeable breezes present during the simulations however, as wind was not measured, there is the potential that wind had a temporal and/or spatial impact upon the applied rainfall but this cannot be quantified.

The major potential source of error from the measurement of runoff is the manual nature of the recording method. Measuring the runoff sample requires two jars to be simultaneously placed under the runoff outlet for each of the plots. Errors can result from one of the jars being placed under the outlet at slightly different times and/or the jar not being centrally placed under the outlet in the first instance resulting in some runoff not being captured. There is also the potential for error in the timing by the recorder. One data point was selected and the sampling time changed by +/- 0.2 s. This resulted in a change to the instantaneous infiltration rate (mm/h) from the original value of 7.5 mm/h to 6.2 mm/h (-0.2 s) or 8.7 mm/h (+0.2 s).

There are several sources of potential error relating to the soils/soil trays. These are:

- Inconsistent packing of the trays. The trays were packed manually with special attention placed on ensuring that the edges and corners of the trays were well packed to minimise any preferential flow that would reduce runoff. However the manual nature of this process implies that inconsistencies could result.
- Insufficient packing of the front lip of the tray. When this occurs water that would be runoff pools at the lip, reducing the runoff and increasing the apparent infiltration. It is possible that this occurred on the Ferrosol run number one where ponding water was visible along the front lip. Additional soil was packed into this tray prior to the second run and the resultant infiltration rate was substantially lower. How much of this change was due to insufficient packing cannot be determined.
- All of the soil was screened prior to being placed into the trays and large stones and pieces of organic matter were removed. This resulted in a reasonably homogenous soil material. It is assessed that the state of the soils used in the simulation contributed little to the potential errors from the rainfall simulation experiments.
- The nature of the soil surface changed over time as a result of crust formation and raindrop impact. The changes impacted upon surface ponding/detention and thus measured runoff at various stages throughout the rainfall simulation. Whilst these changes could be visually observed during the simulations there was no practical measurement. It is assessed that these changes contributed to the noise within the runoff data.

A number of sources of error have been identified that have impacted upon the results from the rainfall simulation experiments. An exact quantification of the error is not possible with the data that

is available however an assessment is that the infiltration rate through time could vary by values of +/-5 mm/h. This needs to be considered when comparing the results of the rainfall simulation experiments with the HYDRUS-1D modelled results.

# 5.4. Bulk density measurements (less X-Ray CT)

## 5.4.1. Bulk soil

The average  $\rho_b$  and associated standard deviation for each of the soils are detailed in Table 31. The standard deviation values indicate that the soil was packed relatively evenly in the trays.

The of th					
Soil	$\rho_{\rm b}$ (g/cm <sup>3</sup> )	Standard deviation			
Sodosol #1	1.24	0.055			
Sodosol #2	1.30	0.051			
Sodosol #3	1.36	0.058			
Chromosol	1.13	0.053			
Ferrosol	0.98	0.014			
Hydrosol	0.85	0.015			

Table 31 Average bulk densities and standard deviation for underlying soil based on a sample of four

## 5.4.2. Water Retention Method

A summary of the results from the Water Retention Method for determining crust  $\rho_b$  are detailed in Table 32.

Soil	Mean $\rho_b$ (g/cm <sup>3</sup> )	Standard Deviation
Sodosol #1	1.32	0.10
Sodosol #2	1.33	0.47
Sodosol #3	1.56	0.20
Chromosol	0.93	0.20
Ferrosol	0.87	0.06
Hydrosol	0.78	0.20

 Table 32 Summary of water retention method crust bulk densities

The obtained values for the Sodosol's were relatively consistent with the exception of two values from Tray #2 which appeared to be outliers. Removing these changed the average density for this sample to 1.21 g/cm<sup>3</sup> from 1.33 g/cm<sup>3</sup>. The Chromosol formed a distinct crust so the crust density is lower than would be expected for a surface crust. The Ferrosol did not appear to form a crust and the low value for density is perhaps an indication that the tray was insufficiently compacted. The low density of the Hydrosol could be related to high levels of organic matter in that sample.

The Water Retention Method was finicky and time consuming (approximately 10 minutes required per sample). It proved very difficult to remove the crust without removing some of the underlying uncrusted soil. Based on an effective area of excavation of 100mm and the average volume of soil removed (38.8 mL) an approximate average depth of excavation using this method was 5 mm. Visual analysis of images of the surface crusts suggests that thicknesses of 1 - 3 mm (Figure 62) whilst micro X-ray CT indicated crusts approximately 2 mm thick (see Section 5.5). This indicates that the water retention method sampled to a depth several times thicker than the thickness of the surface crust leading to an underestimation of the crust  $\rho_b$ .



Figure 62 Close up of example Sodosol crust. Visual examination suggests a crust of 1 - 3 mm. Pencil lead is 2 mm thick.

### 5.4.3. Thin Slice Method

This method proved ineffective at removing the crust for two reasons. The first was that in virtually all instances the soil surface was uneven making it difficult to calculate an accurate sample volume. The second reason was that whilst the wire could remove a sample from the top of the core quite effectively, it tended to do so along a plane of weakness within the core that was much thicker than the surface crust. As a result of the uneven segments of crust that were removed it proved impossible to obtain a sample that was shaped sufficiently even to obtain an accurate density measurement. As a result the modified intact clod method was used.

#### 5.4.4. Modified intact clod method

The summarised results of the modified intact clod method are detailed in Table 33.

	10 mL		100 mL		
Summary	Mean density	Standard Deviation	Mean density	Standard Deviation	Percentage difference
Sodosol #1	1.99	0.19	1.75	0.28	14%
Sodosol #2	1.94	0.28	1.16	0.06	68%
Sodosol #3	1.99	0.22	1.46	0.26	36%
Sodosol					
Average	1.97		1.46		35%
Chromosol	1.62	0.19	1.15	0.18	40%

 Table 33 Results from modified intact clod method for bulk density determination

Substantial differences in crust  $\rho_b$  are evident based on the size of measuring cylinder (MC) used to determine displaced volume with the 10 mL MC producing higher density values (but more consistent) for all soils. The standard deviations are broadly comparable between the sizes but more consistent for the 10 mL MC. As the soil oven dry mass and initial volume were measured using laboratory scales; it appears that the final volume reading obtained by reading the scale on the MC is a likely source of error, particularly given the small size of the crust fragments used to obtain  $\rho_b$ . The average mass of the crust fragments used in the 10 mL and 100mL MC were 0.7 g and 2.4 g respectively. Another potential source of error was air bubbles that formed when the crust fragments were placed into the cylinder. The larger bubbles were popped using a pencil before a reading was

obtained but smaller bubbles may have remained that would result in a larger final volume and hence lower calculated density.

To determine how sensitive the calculated densities were to the final volume measurement a sensitivity analysis was conducted. The gradations were 0.25 mL and 1 mL apart on the 10 and 100 mL MC respectively. A conservative estimate of the largest possible measurement error was half the difference between the gradations (i.e. 0.125 mL and 0.5 mL for the 10 mL and 100 mL measuring cylinders respectively). It was further assumed that this error could be either higher or lower than the actual reading resulting in assumed accuracies of +/- 0.0625 mL and +/- 0.25 for the 10 mL and 100 mL MC respectively. Densities were re-calculated using the Negative Maximum Error (NME, subtracting the maximum volume error) and Positive Maximum Error (PME, adding the maximum volume error) and the range between the NME and PME calculated. The results are detailed in Table 34.

	10 mL			100 mL		
Soil	NME	PME	Range	NME	PME	Range
Sodosol #1	2.53	1.26	1.26	2.10	1.20	0.91
Sodosol #2	2.57	1.32	1.25	1.35	1.16	0.19
Sodosol #3	2.41	1.21	1.20	1.76	1.20	0.56
Chromosol	1.88	1.16	0.72	1.32	1.15	0.17
		Average	1.11		Average	0.46

Table 34 Average  $\rho_b$  results for the modified clod immersion method sensitivity analysis. Units are in g/cm<sup>3</sup>. Average range results between the 10 mL and 100 mL measuring cylinder are also provided.

Some of the individual NME results for the 10 mL MC exceeded a  $\rho_b$  of 2.65 g/cm<sup>3</sup> which is physically impossible. The average range of the 10 mL MC results is more than twice that of the 100 mL MC results indicating that the accuracy of  $\rho_b$  calculated from the 100 mL MC is likely to be better than results from the 10 mL MC. It was decided to use the 100 mL MC calculated  $\rho_b$  values for comparison to other methods of  $\rho_b$  determination due to its assessed increased accuracy. Additional measurements were made with the average results detailed in Table 35 Bulk density results from surface crust method

Table 2	- DII-	diameters.		<b>c</b>			
Table 5	э дшк	density	results	ILOIII	surface	crust	methoa

Soil	Mean $\rho_b$ (g/cm <sup>3</sup> )	Standard Deviation
Sodosol #1	1.49	0.26
Sodosol #2	1.27	0.13
Sodosol #3	1.33	0.30
Chromosol	1.25	0.29

## 5.5. Bulk density measurements (X-ray CT)

## 5.5.1. Introduction

Imagery produced from the micro X-Ray CT of soil cores was analysed using ImageJ (Schneider et al. 2012) image analysis software. The first step of the analysis process was to obtain a vertical slice of a soil core, an example of which is detailed in Figure 63.



Figure 63 Example soil core profile from core number 2 (Sodosol). The black within the soil mass represents voids (or organic matter as annotated). The grey represents the soil particles.

A visual analysis of Figure 63 highlights the following:

- At the top of the soil surface there are sections of unconsolidated material, likely resulting from the detachment and subsequent deposition of soil particles from raindrop impact.
- Below the unconsolidated material is a thin layer of compacted material with relatively few visible pores of small size. This is the surface crust.
- The bulk soil underneath the soil crust has both a greater number of pores and pores of larger size.

A crust, similar to that identified in Figure 63, was visually identifiable on all of the soil cores.

### 5.5.2. Crust porosity

For each image stack, the 'Radial Reslice' function in ImageJ was executed over a 180 degree arc with 10 degrees rotation between each slice. This resulted in 18 vertical slices per soil core. To determine if there was a loss in accuracy by only using 18 images the 'Radial Reslice' function was applied using 5 degrees rotation between each slice (i.e. 36 vertical slices per soil core). A comparison of the two settings (Table 36) indicates that the difference is negligible. This comparison was made to minimise the image processing time with an acceptable level of accuracy.

	Calculated porosity	Calculated bulk density
18 slices	0.876931667	2.323868917
36 slices	0.870652778	2.307229861

Table 36 Comparison of porosity and bulk density results for 18 and 36 vertical slices per core

The initial ImageJ script was amended to enable automatic porosity calculation for each of the 18 slices. The resulting ImageJ determined porosity and resultant  $\rho_b$  values are detailed in Table 37.

		Crust		
Soil	Core	Porosity	$\rho_b(\mathrm{g/cm}^3)$	
Chromosol	16	0.09	2.42	
Chromosol	11	0.13	2.30	
Chromosol	12	0.13	2.32	
Sodosol #1	2	0.12	2.32	
Sodosol #1	3	0.1	2.39	
Sodosol #1	4	0.09	2.42	

Table 37 ImageJ calculated crust porosity and  $\rho_b$ 

Soil	Core	Crust	
Sodosol #1	13	0.23	2.03
Sodosol #2	7	0.15	2.25
Sodosol #2	8	0.22	2.06
Sodosol #2	9	0.17	2.21
Sodosol #2	14	0.17	2.20
Sodosol #3	5	0.09	2.40
Sodosol #3	6	0.18	2.16
Sodosol #3	10	0.14	2.28
Sodosol #3	15	0.14	2.28

The results indicate a porosity that is substantially lower than normally observed (and as a result  $\rho_b$  is higher). Some of the crust density values are approaching the maximum possible density of soil (assumed to be 2.65 g/cm<sup>3</sup>). This will be explored further in the next section.

## 5.5.3. Determination of bulk soil porosity

The bulk soil porosity results for the average of 18 vertical slices per core are detailed in Table 38.

		Bulk soil		
Soil	Core	Porosity	$\rho_b (\mathrm{g/cm}^3)$	
Chromosol	16	0.2	2.13	
Chromosol	11	0.27	1.94	
Chromosol	12	0.32	1.80	
Sodosol #1	2	0.27	1.93	
Sodosol #1	3	0.28	1.92	
Sodosol #1	4	0.35	1.72	
Sodosol #1	13	0.28	1.91	
Sodosol #2	7	0.31	1.83	
Sodosol #2	8	0.24	2.00	
Sodosol #2	9	0.24	2.02	
Sodosol #2	14	0.24	2.02	
Sodosol #3	5	0.2	2.11	
Sodosol #3	6	0.28	1.90	
Sodosol #3	10	0.26	1.97	
Sodosol #3	15	0.31	1.83	

Table 38 ImageJ calculated bulk soil porosity and  $\rho_b$ 

The porosity and  $\rho_b$  values calculated by image analysis for both the crust and sub-soil are substantially lower than those reported in the literature. This suggests that the ImageJ calculated values are different than the actual values. There are a number of reasons why this could occur:

- Resolution. The resolution of the imagery is  $30 \mu m$ . As a result pores of smaller size than this can not be identified and are treated as soil solids. This reduces the apparent porosity.
- Thresholding. The 'Make Binary' function in ImageJ was used to threshold the images. There may be a bias in this algorithm that reduces the apparent porosity.
• Soil carbon. Pieces of charcoal were present in many of the soil cores. As demonstrated by Figure 63 soil carbon appears black. When thresholding is applied to the image, pieces of soil carbon can appear as a pore. This would have the effect of increasing the apparent porosity.

The ImageJ derived  $\rho_b$  values were considered to be positively biased when compared to the actual values. To correct this bias the average bulk soil measured value for  $\rho_b$  using the soil core method (Cresswell & Hamilton 2002) was assumed to be the correct value. The adjusted values for crust  $\rho_b$  were then calculated using Equation 18 and are detailed in Table 39.

Table 39 details the adjusted crust  $\rho_b$  values and compares them to the measured bulk soil  $\rho_b$  values. This indicates that in all instances  $\rho_b$  of the crust was greater than the underlying soil as is to be expected if a crust was formed. There is however a large range of differences in densities with the minimum increase being 3 % (Core 8) and the maximum increase being 41 % (Core 4).

Soil	Core	Adjusted crust $\rho_b$ (g/cm <sup>3</sup> )	Average measured bulk soil $\rho_b$ (g/cm <sup>3</sup> )	Absolute difference between crust and subsoil density (g/cm <sup>3</sup> )	Percentage difference (%)
Chromosol	16	1.28		0.15	14%
Chromosol	11	1.34		0.21	19%
Chromosol	12	1.45	1.13	0.32	28%
Sodosol #1	2	1.49		0.25	20%
Sodosol #1	3	1.55		0.31	25%
Sodosol #1	4	1.74		0.50	41%
Sodosol #1	13	1.32	1.24	0.08	6%
Sodosol #2	7	1.59		0.29	22%
Sodosol #2	8	1.34		0.04	3%
Sodosol #2	9	1.42		0.12	9%
Sodosol #2	14	1.41	1.30	0.11	9%
Sodosol #3	5	1.55		0.19	14%
Sodosol #3	6	1.55		0.19	14%
Sodosol #3	10	1.57		0.21	16%
Sodosol #3	15	1.69	1.36	0.33	24%

Table 39 Comparison of adjusted crust  $\rho_b$  to measured bulk soil  $\rho_b$ 

The purpose of deriving density values was for use as an input into HYDRUS-1D modelling for both the surface crust layer and the underlying soil. The average of the adjusted crust  $\rho_b$  value for each soil was used for the surface crust  $\rho_b$  and the average measured  $\rho_b$  using the soil core ring method used for the soil underlying the crust.

#### 5.5.4. Identification of changes in soil $\rho_b$ with depth

The average  $\rho_b$  with increasing depth for each of the soils is shown in Figure 64. The impact of unconsolidated material at the top of the soil surface is shown by a lower density than the crust immediately below the soil surface for all soils. The increased density below the soil surface is a result of the surface crust. The density declines with increasing depth below the surface however there is substantial variation of  $\rho_b$  with depth for all soils.



Figure 64 ImageJ derived average  $\rho_b$  (not adjusted) with depth below soil surface.

Given the variation in density throughout the depth of the image stack there was no distinct depth where the surface crust ended. To determine a crust depth value that could be incorporated into HYDRUS-1D for modelling purposes the following approach was applied:

- The average ρ<sub>b</sub> and standard deviation of the last 25 measurements was calculated. This was assumed to be the subsoil density ρ<sub>b</sub>.
- A five point moving average was calculated to smooth out the measured  $\rho_b$  values.
- The averages and the original data were plotted. After reviewing the plots it was decided to include the average  $\rho_b$  plus one standard deviation to account for the variability within the data. An example of the results from Sodosol #3 are detailed in Figure 65.



Figure 65 Determining the thickness of the surface crust (Sodosol #3)

The thickness of the crust was then calculated using two methods. The first was subtracting depth of maximum density (0.3 mm in all instances) from the depth where the moving average intercepted the subsoil average. The second approach was similar but used the depth where the moving average intercepted the subsoil average plus one standard deviation. The results are detailed in Table 40.

Soil	Depth subsoil average interception (mm)	Crust thickness (mm)	Depth subsoil average plus one sigma interception (mm)	Crust thickness (mm)
Sodosol #1	4.83	4.53	3.93	3.63
Sodosol #2	3.03	2.73	2.43	2.13
Sodosol #3	4.23	3.93	2.13	1.83
Chromosol	6.93	6.63	3.03	2.73

Table 40 Crust thickness determination

The initial HYDRUS-1D modelling proved that small variations in crust thickness made little difference in the predicted infiltration. As a result after reviewing the density with depth profile for each soil, the results in Table 41, photographic images of the crusts as well as ImageJ images, it was decided that a 2 mm crust would be used for all soils.

### 5.5.5.Summary of bulk density results

Bulk density results from the various methods are detailed in Table 41 and compared by method in Figure 66.

Soil	Bulk Soil	Crust - Water	Crust - Intact	Crust –
	(Core)	Retention	clod	Adjusted X-ray
				СТ
Sodosol #1	1.24	1.32	1.49	1.36
Sodosol #2	1.30	1.33	1.27	1.52
Sodosol #3	1.36	1.56	1.33	1.44
Chromosol	1.13	0.93	1.25	1.59

Table 41 Summary of bulk density crust results

The X-ray crust CT method of determining  $\rho_b$  is the only method which results in the crust having a higher density than the underlying soil for all soils. The water retention method and intact clod method results in one and two of the four soils respectively having a crust density that is lower than the bulk soil crust. As a result of this the crust  $\rho_b$  values determined by the adjusted X-ray CT method have been used for the subsequent modelling using HYDRUS-1D.



Figure 66 Comparison of bulk density results by different methods

### 5.6. Modelling observed infiltration using HYDRUS-1D

#### 5.6.1.Introduction

The purpose of this project is to determine whether measuring the  $\rho_b$  of a surface crust improves the accuracy of modelled infiltration. In this section infiltration is modelled using HYDRUS-1D with the same parameters as used in the rainfall simulation experiments (e.g. simulation time, rainfall intensity) and measured soil data (e.g. soil texture, bulk density). HYDRUS-1Ds inverse modelling capability has also been used to derive soil hydraulic parameters from the observed rainfall simulation

infiltration data. The results of these methods are subsequently compared to the observed infiltration rates from the rainfall simulation experiments.

#### **5.6.2.Modelling using SSCBD inputs**

Key model parameter values for the SSCBD modelling are detailed in Table 42. The soil profile was set to 100 mm and a 2 mm crust thickness incorporated to approximate the thickness of the surface crusts created during rainfall simulation. The value for  $\psi$  was set to -10,000 mm throughout the profile to replicate a partially dry soil.

Soil	% Sand	% Silt	%Clay	Soil mass $\rho_b$	Crust p <sub>b</sub>
Chromosol	63	15	22	1.13	1.36
Sodosol #1	72	11	17	1.24	1.52
Sodosol #2	72	11	17	1.30	1.44
Sodosol #3	72	11	17	1.36	1.59

Table 42 SSCBD parameter values for HYDRUS-1D modelling comparison to observed infiltration

The second run of each soil was used to provide the observed infiltration data using the regression equations detailed in Table 28. The infiltration from the first run for each soil was ignored as this included the time where the crust was being formed. It is not possible to model infiltration during crust formation using HYDRUS-1D.

An example of the results is provided in Figure 67 for the Chromosol. The horizontal lines during the initial part of the simulation represent the infiltration before runoff commenced. The results indicate that incorporating the crust  $\rho_b$  makes relatively difference to the modelled infiltration. Similar results were obtained for the Sodosol simulations.



Figure 67 Comparison of approximate observed infiltration against modelled infiltration with and without a surface crust for the Chromosol

A comparison was made between modelled versus observed infiltration. The range of values for determining the criteria was constrained by only using infiltration values that occurred after the first run off measurement. This decision was made as for larger rainfall events, which are of more importance from an infiltration/runoff modelling perspective, the first few minutes is only a small proportion of the total rainfall. The values for the Nash Sutcliffe objective function for each soil are detailed in

Table 43. In all instances a large negative value results indicates very poor fits between modelled and observed results.

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Table 43 Objective function values	
Soil	Nash Sutcliffe Criteria
Chromosol	-4998
Sodosol #1	-1743
Sodosol #2	-31
Sodosol #3	-85

An optimisation process was completed for each soil with the aim of increasing the increasing the closeness of the modelled versus observed data. The process was to select the 'Air Entry Value of -2cm' option in the Soil Hydraulic Model dialog box within HYDRUS-1D and then increase the  $\rho_b$ value for the crust until the simulation did not converge. The final step was to reduce K<sub>s</sub> until the simulation results did not converge. The results (Table 44) were compared to the original 2 mm crust simulation and the Nash-Sutcliffe criterion value calculated to determine how closely the modelled results approached the observed results.

Soil	Highest $\rho_b$ (g/cm <sup>3</sup> )	Lowest K <sub>s</sub> (mm/s)	Nash-Sutcliffe Criteria
Chromosol	1.6	0.00035	-5291
Sodosol #1	1.7	0.0019	-1614
Sodosol #2	1.7	0.0017	-86
Sodosol #3	1.6	0.0031	-186

Table 44 Values obtained from optimisation activity

As demonstrated in Table 44 the optimisation activity did not enhance the accuracy of the modelling results.

### **5.6.3.** Modelling using SSCBD and $\theta_{33}$ inputs

This modelling used the same input parameters as detailed in the previous section with one change. The change was the use of the H5 model within HYDRUS-1D when predicting the soil hydraulic properties which requires  $\theta_{33}$ . The  $\theta_{33}$  value was obtained from the HYPROP soil water retention measurements obtained from a repacked soil core of similar  $\rho_b$  to the surface crust. Three problems were encountered using this approach, being:

- The  $\rho_b$  values for the soil cores did not align to the  $\rho_b$  of the surface crust.
- There was a major difference in derived soil hydraulic properties even when the soil core  $\rho_b$ values were very similar.
- For the Sodosol HYPROP measurements, the  $\rho_b$  of the soil cores were much higher than that • measured for the crust.

As a result the SSCBD and  $\theta_{33}$  modelling was limited to the Chromosol with two values of  $\theta_{33}$  being used (0.85 and 0.47) that were obtained from soil core  $\rho_b$  values of 1.33 and 1.41 g/cm<sup>3</sup>. These  $\rho_b$ values bounded the measured crust  $\rho_b$  of 1.36 g/cm<sup>3</sup>.

The modelling results are detailed in Figure 68 and Figure 69. In both instances the incorporation of the  $\theta_{33}$  water retention point improves the modelling results compared to the observed infiltration however there is still a marked difference and the water retention values are not considered reliable.



Figure 68 HYDRUS-1D modelling including incorporation of  $\theta_{33}$  (value of 0.47) compared to observed infiltration



Figure 69 HYDRUS-1D modelling including incorporation of  $\theta_{33}$  (value of 0.85) compared to observed infiltration

#### **5.6.4. Inverse solution**

The direct method of predicting infiltration through the use of measured soil properties provided a very poor comparison to the observed infiltration. As a result the inverse modelling capability of HYDRUS-1D was used to determine the crust soil hydraulic parameters.

HYDRUS-1D enables the user to choose which parameters are optimised when calculating the inverse solution. After trialling different input data options the rainfall simulation 'Time – Flux' option was selected as it resulted in predicted infiltration that was a close match to the observed infiltration. The  $\alpha$ , n and  $K_s$  parameters were optimised for the surface crust horizon. When  $\theta_s$  and  $\theta_r$  were also optimised the modelled infiltration was a close match to the observed infiltration however this resulted in  $\theta_r$  being greater than  $\theta_s$  which from a practical perspective is impossible (this is not

necessarily an issue however as  $\theta_r$  can be thought of as a fitting parameter according to Radcliffe and Simunek (2010). No parameters for the underlying soil were optimised as it was found that this resulted in the solution not converging.

Figure 70 is an example of the results from the inverse solution compared to the modelled results (with no crust and with a 2mm crust) and the idealised observed infiltration. The inverse solution provides a very good match to the observed infiltration once runoff has commenced (At t = 99 s in this example). Similar results were achieved using the inverse solution for all soils.



Figure 70 Comparison of observed infiltration with modelled infiltration and inverse solution - Chromosol

The Nash-Sutcliff Criteria for the inverse solution against the observed infiltration function are detailed in Table 45 indicating that the parameters derived from the inverse solution provide a much better approximation of observed infiltration compared to the direct attempts using SSCBD data.

Soil	Nash-Sutcliffe Criteria
Chromosol	0.691
Sodosol #1	0.655
Sodosol #2	-0.0623
Sodosol #3	0.137

Table 45 Nash-Sutcliffe criterion for inverse solution modelled versus observed infiltration data

The soil hydraulic properties from the inverse solution are detailed in Table 46, including the third rainfall simulation for the Sodosol soils. The optimised parameters are  $\alpha$ , n and K<sub>s</sub>. These parameters were compared to the soil hydraulic properties used in the direct solution. The key difference between the direct and inverse parameters is the  $K_s$  values. The inverse solution  $K_s$  values are three to four orders of magnitude smaller than the direct solution values as shown in Figure 71.

Soil	θ <sub>r</sub> mm <sup>3</sup> /mm <sup>3</sup>	θ <sub>s</sub> mm <sup>3</sup> /mm <sup>3</sup>	α (1/mm)	n	K <sub>s</sub> (mm/s)	$\mathbf{r}^2$	Air Entry (Y/N)
Chromosol	0.067	0.442	0.02351	0.7125	1.756E-06	0.2882	Y
Sodosol #1 Run 2	0.058	0.395	0.02352	0.7315	2.336E-06	0.7405	Y
Sodosol #2 Run 2	0.06	0.418	0.00357	0.8082	3.242E-07	NaN	Ν
Sodosol #3 Run 2	0.055	0.375	0.00280	0.7092	3.470E-06	0.7272	N
Sodosol #1 Run 3	0.058	0.395	0.00866	0.7315	9.962E-07	0.2450	Ν
Sodosol #2 Run 3	0.06	0.418	0.00133	0.8513	1.855E-07	0.2124	Y
Sodosol #3 Run 3	0.055	0.375	0.00250	0.7092	2.415E-07	0.2094	Ν





Figure 71 Comparison of  $K_s$  between inverse and direct HYDRUS-1D simulations using a log scale. Sodosol is abbreviated to 'Sod' and Chromosol to 'Chr'. (I) indicates inverse method and (D) direct method.

This enormous variation suggests that the major reason why the direct method of predicting infiltration on the crusted soils does not come close to the observed infiltration rates is because of the underestimation of  $K_s$ .

A check was made that the derived parameters from the inverse solution would produce the same results when used in the direct approach. Figure 72 is an example of this check for Sodosol #3 indicating that the derived parameters produce the same result. This occurred for all soils.

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When entering the derived parameters into HYDRUS-1D a warning message 'Retention Curve parameter is smaller than 1.001' was triggered. This did however prevent the simulation from being completed however a boundary condition for HYDRUS-1D is that n > 1 (Šimůnek et al. 2009). The implications of this are discussed in Chapter Six.



Figure 72 Check of inverse derived soil hydraulic properties - Sodosol #3

#### 5.6.5. Modelling using average parameters

Simulations using the inverse method in HYDRUS-1D have been able to provide a close approximation to the observed infiltration rates. An initial assessment of the parameters for  $\alpha$ , n and Ks (Table 46) indicated that they were generally similar. It was therefore investigated whether the average results (Table 47) from these parameters could be used in HYDRUS-1D simulations to provide an accurate approximation of the observed infiltration.

 Table 47 Comparison of average soil hydraulic parameters obtained from inverse and direct HYDRUS-1D simulations

		α (1/mm)	n	K <sub>s</sub> (mm/s)
	Mean	0.009412	0.75048	1.33E-06
Inverse	Std Dev	0.009911	0.0564	1.25E-06
	Mean	0.002783	1.4576	0.004777
Direct	Std Dev	0.000146	0.036898	0.001463

For each of the soils the 2 mm crusted soil simulation was copied. All parameter values were kept the same except for  $\alpha$ , n and  $K_s$ . These values were replaced with the average values detailed in Table 47. An example simulation for Sodosol #1 using the average parameters compared to the inverse solution is provided in Figure 73. Simulations for all soils produced a similar result.



Figure 73 Comparison of inverse solution, direct solution using average parameters and the observed infiltration for Sodosol #1

The Nash-Sutcliffe Criterion was calculated for all soils. The results are detailed in Table 48. Whilst the results are negative, indicating a poor fit, they are substantially better than the results obtained from simulations using the SSCBD input data (see

Table 43). Given the 'noise' within the original infiltration data resulting in only moderately high  $r^2$  values for the derived infiltration functions, the Nash Sutcliffe Criterion values are as assessed as being acceptable.

	Nash	Sutcliffe
Soil	Criterion	
Chromosol	-3.74	
Sodosol #1	-0.278	
Sodosol #2	-0.11	
Sodosol #3	-0.86	

 Table 48 Nash-Sutcliffe Criterion values for average parameter simulations

A sensitivity analysis was completed by changing the parameters around the mean plus or minus a multiple of the standard deviation. For parameters  $\alpha$  and n this had no effect on the modelled infiltration; that is the results from each simulation were the same regardless of the parameter value used. Adjusting  $K_s$  did however have an impact upon the modelled infiltration as demonstrated in Figure 74 for Sodosol #3. All three sensitivity analysis values for  $K_s$  (represented by Min, Mean and Max in Figure 74) provided a much better approximation of the observed infiltration that the SSCBD predicted infiltration. The results were similar for the other soils.



Figure 74 Results of sensitivity analysis when Ks is changed by plus or minus the standard deviation of inverse solution  $K_s$  values for Sodosol #3

That changing values for  $\alpha$  and *n* made no impact on the modelled infiltration indicates that the  $K_s$  is the predominate parameter in modelling crust infiltration.

#### 5.6.6. Comparing cumulative infiltration

Cumulative infiltration for all soils was calculated for the following simulations:

- SSCBD modelled (no crust)
- SSCBD modelled (2mm crust)
- Inverse solution modelled
- Average SHP parameter modelled
- Rainfall simulation observed.

Cumulative infiltration was approximated from the time that runoff commenced (for the previously stated reason) until the end of the simulation. This was achieved by averaging the infiltration rate between each time step, multiplying this value by the time step, repeating for all time steps and summing the results.

An example comparison is provided for Sodosol #3 in Figure 75. It clearly shows that the use of average soil hydraulic results in a close match to observed infiltration when compared to the SSCBD modelled infiltration.



Figure 75 Cumulative infiltration comparison – Sodosol 3

A summary of cumulative infiltration for each soil type along with the percentage difference from observed infiltration are detailed in Table 49. The conclusion that can be drawn from these results is that the use of average parameters for  $\alpha$ , n and  $K_s$  provides a reasonable approximation of observed infiltration.

minutation					
Soil	SSCBD modelled	% difference from observed	Inverse solution derived mean	% difference from observed	Observed
Chromosol	19.2	834%	1.54	-25%	2.05
Sodosol #1	33.9	1550%	2.37	15%	2.93
Sodosol #2	25.8	1159%	1.98	-4%	2.93
Sodosol #3	22.6	1003%	1.88	-8%	3.46

Table 49 Summary of cumulative infiltration results and percentage difference from observed rainfall simulation infiltration

## 6. CHAPTER SIX – DISCUSSION

### 6.1.Modelling

The modelling indicated, which was confirmed by the experimental work, that HYDRUS-1D does not accurately model the large reductions in infiltration that can occur with thin crusts. Whilst this was not the initial aim of the modelling aspect of the work, this has proven vital to moving our understanding and modelling of thin crust impact forward. The initial concept for modelling was to obtain experimental data on infiltration in crusting soils and use that data to quantify errors resulting from the impact of the surface crust. Whilst there has been extensive research and experimentation completed in this work toward that approach, it proved technically difficult with complexity beyond the scope of work to obtain this data. Hence, the change in focus to investigating how HYDRUS-1D modelling responds to surface crusts using simulated data. Both the literature and X-ray CT calculated  $\rho_b$  identify that surface crusts are non-uniform layers; the modelling indicated however that there was little benefit in modelling the crust with HYDRUS-1D as a non-uniform layer. As a result a constant crust density was used in subsequent modelling.

A significant portion of the modelling investigated the potential to develop PTFs to predict the SWC and HCF of a soil as its  $\rho_b$  changed. However, further experimental work beyond the current scope is required to be conclusive in validating the outcomes of this modelling.

## 6.2. Infiltration into a crusting soil

The crusts formed on the Sodosol and the Chromosol samples proved to be an effective 'throttle' to infiltration, with over 90 % of the applied rainfall becoming runoff. This was further evidenced by the low subsoil moisture content, even after multiple rainfall simulations (an average of 13 % for the Sodosol and 12 % for the Chromosol), indicating that very little water infiltrated below the surface crust. These findings reinforce the importance of accounting for the impact of surface crusts in infiltration and surface runoff modelling.

The observed infiltration for both the Sodosol and Chromosol occurred in two distinct phases. The first phase commenced with the start of the rainfall simulation and ceased when runoff commenced. The infiltration rate during this phase was relatively high. HYDRUS-1D modelling of this phase provided a good approximation to the observed infiltration rate; however, there was a significant time lag within the model before the infiltration rate commenced declining (Figure 67). After the occurrence of runoff, the second phase commenced with a significant decline in the infiltration rate. HYDRUS-1D modelled this phase very poorly. These infiltration phases align with the general findings of Moore (1981) who used the Richard's equation (the infiltration model used in HYDRUS-1D) to model infiltration under no seal, developing seal and constant surface seal conditions. The observed infiltration rate profile aligned with the infiltration under constant seal conditions calculated by Moore (1981), whilst the HYDRUS-1D predicted infiltration had a similar profile to the developing surface seal condition. This appears to suggest that while the seal is documented as a physical density gradient (Roth 1997), its effect on infiltration is abrupt. This is likely a function of the surface infiltration characteristic at sub-millimetre scale governing infiltration, while the gradient becomes redundant. In such a model, infiltration beyond the densest layer would render all other flow unsaturated by virtue of matric potential, effectively rendering the crust gradient redundant as flow becomes constant within the smaller pore sizes.

Based on the rainfall simulation experiments completed as part of this project it is concluded that complete infiltration modelling in HYDRUS-1D requires two simulations to cover pre and post runoff conditions. The results from the two soils used in this project indicate that the cumulative infiltration in the first phase (Table 29) only averaged around 2 mm. Given the surface runoff/infiltration measurement inaccuracies identified during the rainfall simulation experiments (assessed as +/- 5 mm/h) this small amount of rainfall is insignificant and unlikely to lead to major modelling errors. Thus, for practical purposes it is assessed that modelling of the second phase only is sufficiently accurate to represent the physical system function.

### 6.3.Measurement of crust bulk density

Four methods of measuring crust density were trialled during the project being: the water retention method, thin slice, modified intact clod (physical methods) and X-ray CT. The surface crusts created on the soils used in the project were thin with physical connection to the underlying soil. This made it practically impossible to separate just the soil crust to enable its measurement. The manual nature of the physical methods trialled resulted in samples incorporating the underlying soil as well as the crust resulting in inaccurate measurements of crust  $\rho_b$ . The manual nature of the physical method also resulted in inconsistent sampling and, thus, more variability in the results.

The results from the modified intact clod method could potentially have been improved if the full intact clod method was applied (Cresswell & Hamilton 2002) where the clods are coated in paraffin wax. However the underlying problem of sampling just the crust and not the underlying soil would have remained. For the type of crusts formed in this project it is assessed that the physical methods are not suitable for measuring crust  $\rho_b$ . These methods maybe suitable on soils that form thicker crusts (e.g. 5 mm or greater) but this has not been tested during this project.

X-ray CT was applied to enable measurement of the soil core porosity using image analysis techniques. The resolution provided through the use of this technique highlighted the heterogeneous nature of the soils, both between cores and within the core (i.e. when viewing vertical slices taken at different angles around the vertical axis). The advantage of using X-ray CT was it enabled repeatable measurements to be made of porosity and the averaging of results. The porosity values obtained were however lower than is generally reported in the literature. This is primarily attributed to the resolution of the imagery (30  $\mu$ m) which implied that pores smaller than this resolution could not be measured. As X-ray CT images were taken of the core and not just the crust, the measured bulk soil  $\rho_b$  values were used to derive crust  $\rho_b$ . It is assessed that the errors resulting from this approach are minor given the reliability of the soil core method and the consistency of X-ray CT derived porosity values.

### 6.4. Applications of X-ray CT for soil porosity profiling

X-ray CT was also used to identify the depth of the surface crust. Due to time constraints associated with image processing, only two vertical slices were analysed to develop the  $\rho_b$  versus depth profile. It is expected that the profiles would have been smoother if more slices had of been analysed. The profiles highlighted that  $\rho_b$  constantly changes throughout the depth of the soil mass. The general shape of the profile was similar in all instances. The shape can be explained by a thin layer of unconsolidated material on the soil surface (lower density than the crust) immediately underneath of which is the thin surface crust (high density). The density then generally decreases with depth into the underlying soil. This profile aligns with the findings of both Armenise et al. (2018) and Roth (1997).

Despite the high resolution of changes in  $\rho_b$  that were identified by X-ray CT, defining crust thickness was still somewhat arbitrary. The purpose of measuring crust  $\rho_b$  in this project was as an input into HYDRUS-1D modelling. The modelling of variable density crusts identified that incorporating

multiple crusts layers with differing  $\rho_b$  made little difference to predicted infiltration rates. As a result the crust thickness was defined as 2 mm, which aligned with the portion of the crust with the highest density. This was assessed as being sufficiently accurate for modelling purposes within HYDRUS-1D.

The reduced porosity of the surface crust relative to the underlying soil was obvious through both visual observation and image analysis of X-ray CT images. Whilst it was not fully investigated in this project, cursory visual observation revealed not only were there fewer and smaller pores in the surface crust, there appeared to be a large number of vesicular pores. This helps explain why the inverse solution  $K_s$  values of the surface crust are so small compared to the HYDRUS-1D predicted values. Further investigation using the 3D image analysis capabilities of ImageJ could define the pore networks within the surface crust. There is potential that this could subsequently be used to determine crust  $K_s$  as research by Elliot et al. (2010) has suggested.

#### 6.5. HYDRUS-1D predicted soil hydraulic properties

HYDRUS-1D uses the ROSETTA application to predict soil hydraulic properties. ROSETTA includes PTFs that estimate water retention and hydraulic conductivity in a hierarchical manner based on the provided input data (Schaap et al. 2001). The underlying data for ROSETTA is obtained from the UNSODA database (Nemes et al. 2015) which contains basic soil properties such as  $\rho_b$  as well as measured soil hydraulic data. There is no indication that the soil data included in the UNSODA database includes measurements taken from crusting soils/surface crusts. With crust data not included in the databases used to parameterise ROSETTA, it is not reasonable to expect ROSETTA to accurately predict for crust conditions; as such observations would sit outside the boundary conditions.

Inverse solution derived surface crust  $K_s$  values were three to four orders of magnitude smaller than those predicted from ROSETTA. Similar variances in  $K_s$  values between the surface crust and underlying soil have been identified in other studies. For example, Šimůnek et al. (1998) found that the  $K_s$  of the surface crust was two orders of magnitude less than the underlying soil. Experimental results for both the Sodosol and Chromosol demonstrated a 72 % reduction in  $K_s$  between the compacted soil core and the same core after a rainfall impact surface crust had been formed. When the short duration of the applied rainfall during the crust formation process (required as the cores began to breakdown under rainfall impact), and core shrinkage resulting in potential preferential flow paths around the circumference of the core are considered, a much greater reduction in  $K_s$  is possible. These experimental results indicate that the HYDRUS-1D predicted  $K_s$  values are not appropriate for use on crusting soils. However, inclusion of crust data in a specific parameterising module within HYDRUS could be the focus of future work to reach a truly predictive approach for a given soil.

### 6.6. HYDRUS-1D modelling of crusted soils

Modelling of artificially developed 'crusted soils' using HYDRUS-1D identified that incorporating an increased  $\rho_b$  of a surface crust does result in a reduction in the infiltration rate, as compared to an uncrusted soil. Increasing crust thickness resulted in a corresponding decrease in infiltration results. This outcome is logical; however it does not represent what has been observed in some soils, including those used in this project. For the Sodosol and Chromosol soils, thin crusts result in major reductions in infiltration. The crust hydraulic properties obtained from the inverse solution varied significantly (in particular  $K_s$ ) from those of the underlying soil. HYDRUS-1D does not appear to handle very well circumstances where there the hydraulic properties of a crust vary significantly from the underlying soil. This was demonstrated during optimisation modelling (Section 5.6.2) where only minor changes

could be made to soil hydraulic properties before simulations failed to converge. Admittedly this research has only focused on the commonly used VG – Mualem hydraulic model and some parameters remained unchanged ('l' for example). Further modelling using other hydraulic models and/or parameter values may result in improved infiltration modelling of crusted soils.

#### 6.7. Use of average crust parameters

The average soil hydraulic properties calculated from the inverse solution data set (sample n=7) are detailed in Table 50. When used with the HYDRUS-1D predicted values for  $\theta_s$  and  $\theta_r$  and the default value of '*l*' (0.5) these parameters, as demonstrated in Section 5.6.5, provided a good approximation of observed infiltration. The results provided a marked improvement compared to the direct method of predicting infiltration using soil texture and  $\rho_b$  data to predict soil hydraulic properties. The use of these parameters has thus achieved the project aim of improving the accuracy of infiltration modelling.

 Table 50 Average crust parameters

α		
(1/mm)	<i>n</i> (unitless)	$K_s$ (mm/s)
0.009412	0.75048	1.33E-06

There is, however, an issue with the *n* value in these parameters. The *m* parameter has been constrained by the common practice of using the relationship m=1-1/n (Radcliffe & Simunek 2010). For values of n < 1.01 this results in *m* becoming a negative number. The result is that these parameters cannot be used to model the SWC using the VG equation (Figure 76). The modelled SWC with an n < 1.01 is physically impossible which explains why *n* should be greater than one (Šimůnek et al. 2009). As such whilst the average value of *n* can be used to accurately model infiltration it cannot be used for modelling the SWC (and potentially the HCF although this has not been tested). It is interesting to note the inverse solution generates a HYDRUS-1D parameter that is outside of the stated boundary conditions. This appears to be a weakness in the inverse modelling capability of HYDRUS-1D that allows parameters to be estimated that are outside of the boundary conditions for the underlying model.



Figure 76 Soil Water Characteristic using inverse solution derived average hydraulic parameters with comparison where n =1.01.

Whilst the use of inverse solution derived average crust parameters has improved infiltration modelling accuracy, it is clear that the current parameters cannot be used to model all aspects of a soil crust's hydraulic behaviour. Further modelling using the inverse solution approach, where different parameter combinations are optimised, is required. The aim of this modelling would be to determine whether a set of parameters can be obtained that improve infiltration accuracy whilst also providing sensible solutions for the SWC and HCF.

Typical hydraulic parameters for use with the VG model are available across a range of soil textures (Tuller & Or 2004). The values provided by Tuller and Or (2004) have been obtained from the UNSODA database. This was developed to provide readily available and reliable soil data to avoid the time consuming and costly requirement to measure soil hydraulic properties (Nemes et al. 2001). With direct measurements of crust hydraulic properties being very difficult to obtain, inverse solutions offer an attractive alternative (Šimůnek et al. 1998). Surface crust hydraulic parameters derived from inverse solutions provided a very good approximation of observed infiltration rates and cumulative infiltration once runoff commenced during this project. Developing a database of surface crust hydraulic properties, similar in approach to UNSODA, would be useful for improving the modelling of infiltration on crusting soils.

### 6.8. Assessing the accuracy of laboratory based compaction

The modified Proctor test was used with the aim of creating soils cores with different  $\rho_b$  values. These cores, used as an analogue of a soil crust, were then to be used to derive soil hydraulic parameters for use as inputs into HYDRUS-1D simulations (water retention data at  $\theta_{33 \text{ kPa}}$  and  $\theta_{1500 \text{ kPa}}$ ) as well as enabling validation of the SWC and HCF modelling. Six different combinations of blows were used

on the Sodosol with resultant  $\rho_b$  values ranging over a relatively narrow range of 1.63 to 1.87 g/cm<sup>3</sup>. Despite differing force being applied to each core, four of these values, originally intended to be progressively increasing, were within +/- 0.01 g/cm<sup>3</sup> of each other. These results indicate that this approach did not achieve the desired range of soil cores with differing  $\rho_b$  values. Soil cores were prepared by wetting to the approximate moisture capacity required for optimum compaction. It is assessed that the force applied by each blow of the 2.5 kg Proctor device was such that each soil core approached a maximum density after a small number of blows. This indicates that a Proctor like device with a smaller mass is required to provide greater control over the density of each soil core or another method, such as the Triaxial Shear Test (Das & Sobhan 2014) be adapted to provide greater control over core density. Further experimental work on this would be beneficial, potentially enabling the creation of a PTF that would enable prediction of the SWC and HCF as  $\rho_b$  changes for a given soil without the requirement for physical measurement.

#### 6.9. Recommendations for field survey

One of the objectives of this project was to make recommendations on changes to current soil survey processes to improve infiltration and surface runoff predictions. It was originally envisaged that a method would be developed that enabled accurate surface crust  $\rho_b$  determination. This data could then be entered into HYDRUS-1D which would improve modelled infiltration accuracy. The experimental results however identified that this approach is not feasible for two reasons. The first being that physical methods (i.e. methods that could be used either in the field or with readily available laboratory equipment) of determining crust  $\rho_b$  are inaccurate/unreliable (at least for use the Chromosol and Sodosol soils used in this project); the second being HYDRUS-1D predicted soil hydraulic properties based upon crust  $\rho_b$  values provided a very small improvement in model accuracy. As a result directly measuring the crust  $\rho_b$  is not recommended as a change to current soil survey processes.

The Australian Soil and Land Survey Handbook Field Handbook (National Committee on Soil and Terrain 2009) is the primary reference that ensures consistent data on soil profiles is recorded during soil survey activities. The Handbook uses numerical and/or alphabetical codes to describe various properties of the soil profile. One of the properties is the condition of the surface soil when dry (National Committee on Soil and Terrain 2009, pp. 189-91). Surface crust is included as one of these conditions and is assigned the code 'c'. Whilst this information is of some general use, other information could be obtained that would help assist with improving infiltration modelling accuracy.

Perhaps the easiest crust parameter that could be included is crust thickness. Visual observation and measurement of the soil crusts used resulted in similar crust thicknesses being recorded as obtained from X-ray CT. Given the relatively small impact of minor differences (e.g. +/- 1 mm) in crust thickness when modelling infiltration in HYDRUS-1D, as identified during in Chapter Two, this data would be useful for infiltration modelling purposes.

The surface crusts created during this project appeared to be continuous across the extent of the soil surface. Whilst not considered in this project, cracks of various extents tend to form in surface crusts. The cracks are preferential flow paths for infiltrating water, which result in an increased infiltration rate compared to a continuous crust. The reporting of the extent of cracking would be of use in infiltration modelling. For example HYDRUS-1D includes a dual porosity model that enables the impact of macropores upon infiltration to be modelled.

The addition of crust thickness and extent of cracks in surface crusts to current soil survey practices would provide useful input data to assist in modelling infiltration without requiring major changes to the extant processes.

### 6.10. Further work

There are several areas where further work is required to complete/validate the findings of this project and/or expand upon the original scope.

The greatest limitation on the results of this project are the small number of experiments completed on only two soils that were susceptible to crusting. A larger dataset is required that includes a broader range of soils, crust morphologies and soil conditions, including field conditions, to provide a more robust dataset and validate the application of average crust parameter values to infiltration modelling.

The two functions that provide a complete description of a soil's hydraulic properties are the SWC, which describes the storage of water in soil and the HCF, which describes the movement of water in soil. These are incorporated into the Richard's equation. A major part of the modelling completed in Chapter Two was based upon the theoretical development of PTFs that would enable the prediction of the SWC and HCF as the  $\rho_b$  changes. An assumption was made that enabled the creation of a correction factor that could be applied to develop the SWC for a soil when its  $\rho_b$  changed. It was planned to complete experimental work to determine the nature of the correction factor which would have added further depth to the results from this project. Unfortunately due to several factors including time limitations and the identified issues with the modified Proctor test/measurement of the SWC using HYPROP, this work was not able to be completed within the limitations of the project resources. It is a further body of work that could be very useful in enabling the prediction of the SWC without requiring laborious measurements.

It is unlikely in the foreseeable future that the availability of X-ray CT will become widespread due to its cost. This is a key disadvantage of X-ray CT. Advancements in camera technology available in mobile phones, including high magnification lenses, offers a potential opportunity that could be applied to improve the paucity of available surface crust data. Microscope attachments are cheap, readily available and offer up to 1000 times magnification (Horsey 2019). These could be used to photograph surface crusts for subsequent analysis by image analysis software enabling the measurement of crust thickness, crust morphology to be described and potentially porosity calculated. Whilst the resolution would be less than that provided by X-ray CT, and only the exterior of the surface crust profile could be analysed, these disadvantages would be outweighed by the increased ability to collect and analyse surface crust data. This data could be applied to enhance relatively simple reference data, such as the 'infiltrability' values developed by Valentin and Bresson (1992), into more rigorous datasets that could be parameterised for use in infiltration modelling.

## 7. CHAPTER SEVEN - CONCLUSIONS

Surface crusts, a thin layer of higher density and reduced porosity on the soil surface, act as a throttle to infiltration resulting in increased surface runoff. Thin surface crusts (~2 mm in thickness) formed by rainfall simulation experiments on Sodosol and Chromosol soils resulted in greater than 90 % of applied rainfall becoming runoff, highlighting the importance of modelling crust impact upon infiltration. Outside of specific studies focusing on surface crusts, current approaches to the modelling of infiltration and surface runoff do not account for the impact of the increased density of the surface crust. This is a source of modelling error that this project aimed to address.

Specifically the project involved the conduct of rainfall simulation experiments on soils that were prone to surface crusting and subsequently using various methods to measure the  $\rho_b$  of the resulting crust. These experiments provided the input data required to model infiltration runoff using the commonly used HYDRUS-1D software application. This approach enabled a conclusion to be drawn on whether incorporating the surface crust  $\rho_b$  improved infiltration modelling accuracy.

Numerous HYDRUS-1D modelling activities were completed to identify how various parameters affected model outputs resulting in several key conclusions. The first was that HYDRUS-1D did not replicate the dramatic reduction in infiltration observed in numerous studies by thin surface crusts (e.g. crusts of 1 - 2 mm in thickness). The second conclusion is that HYDRUS-1D predicted values for saturated hydraulic conductivity, the key soil hydraulic property, varied by up to two orders of magnitude depending upon the input data provided. Finally whilst surface crusts consist of non-uniform layers, from a modelling perspective it was found that there was no advantage to separating the crust into multiple layers of differing  $\rho_b$  as it is the layer of highest  $\rho_b$  that controls infiltration. These conclusions provided an initial insight into the limitations of HYDRUS-1D in modelling infiltration on crusting soils.

Three physical methods of measuring crust  $\rho_b$  were tested and all were found incapable of providing accurate and reliable results. The key difficulty for physical methods is that it is practically impossible to separate the thin surface from the underlying soil. Thus for thin rain impact surface crusts it is recommended that physical methods of measuring crust  $\rho_b$  are not used.

X-ray CT combined with image analysis using ImageJ was the fourth method used to determine crust porosity and subsequently  $\rho_b$ . The calculated porosity values using this method for the underlying soil were found to be substantially less than expected resulting in unrealistically high values for  $\rho_b$ . This was assessed to be primarily caused by the image acquisition resolution (30 µm) which meant that smaller pores were not measured. This problem was addressed by normalising the X-ray CT  $\rho_b$  values against physically measured sub-soil densities using the traditional soil core method enabling crust  $\rho_b$ to be calculated. X-ray CT was also used to measure surface crust thickness. There was in all instances significant variation in  $\rho_b$  throughout the depth of the soil cores. Due to the relative insensitivity of HYDRUS-1D to this parameter, a crust thickness of 2 mm with uniform density was used in all simulations. X-ray CT proved to be the most accurate method of determining crust  $\rho_b$  and thickness.

The data obtained from the experiments was used to complete HYDRUS-1D modelling and enable a comparison to be made against observed infiltration. It was found in all instances that including crust  $\rho_b$  into HYDRUS-1D resulted in only marginal improvements to modelling accuracy. Inverse modelling was subsequently completed. It was found that the resultant soil hydraulic properties were substantially different to those predicted by HYDRUS-1D using the in-built ROSETTA database. In

particular predicted  $K_s$  values were three to four orders of magnitude greater than those determined by the inverse solution approach. This difference accounts for why HYDRUS-1D modelling incorporating crust  $\rho_b$  provided only a marginal improvement in modelling accuracy. The key conclusion to be drawn is that the ROSETTA database should not be used to predict soil hydraulic properties for surface crusts.

The averages of the soil hydraulic parameters obtained from the inverse solutions were used as inputs into further HYDRUS-1D modelling. The use of the average parameters in all instances proved to provide a close approximation to the observed infiltration. This indicates that when appropriate hydraulic parameters are used, HYDRUS-1D results can provide a good approximation to the observed infiltration, thus increasing modelling accuracy.

An initial set of parameter values (n,  $\alpha$  and  $K_s$ ) for use in HYDRUS-1D modelling have been identified that can be used to improve infiltration modelling accuracy on crusting soils. These values do however need to be used with caution for several reasons. These include the small dataset and the limited range of soil textural classes from which the parameters have been developed. Further experimentation on a broader range of soils under differing soil conditions is required to validate these parameter values. Ideally a database of soil hydraulic properties, similar to the existing ROSETTA database, should be created for crusting soils.

This project has identified that using HYDRUS-1D predicted soil hydraulic properties does not improve modelling accuracy. However, modelling accuracy can be greatly improved when appropriate soil crust hydraulic parameters are applied. An initial set of these parameters have been proposed which HYDRUS-1D modellers can apply for simulations of infiltration on crusting soils.

## **APPENDICES**

- A. Project Specification
- B. Selected MATLAB code
- C. Rainfall simulation results
- D. ImageJ script

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# **Appendix A – Project Specification**

### ENG4111/4112 Research Project **Project Specification**

For:	Cameron Leckie	
Title:	Improving the prediction of infiltration and surface run-off from crusted soils by	
	measuring the density of the discrete surface seal/crust.	
Major:	Agricultural Engineering	
Supervisors:	Dr John Bennett, USQ	Dr Rob Loch, Landloch
<b>Enrolment:</b>	ENG4111 – EXT S1, 2019	ENG4112 – EXT S2, 2019
Project	The aim of this project is to improve the modelling capability for infiltration and	
Aim:	surface runoff for crusting soils.	

### Programme: Version 1, 13 March 2019

- 1. Review the literature on modelling infiltration and surface runoff on crusting soils.
- 2. Use previous experimental data to determine the potential contribution of surface crust density to model error.
- 3. Identify and/or develop a reliable method of collecting surface crust/seal samples for the purposes of accurately calculating crust bulk density and/or porosity.
- 4. Optimise an infiltration and/or surface runoff model that incorporates surface crust parameters such as bulk density and porosity.
- 5. Validate whether incorporation of soil crust parameters into the infiltration/surface runoff models improves model accuracy.
- 6. Make recommendations on changes to current soil survey processes to improve infiltration and surface runoff predictions.

# **Appendix B - Example MATLAB Scripts**

## Introduction

MATLAB was used extensively during modelling activities with HYDRUS-1D. MATLAB was used to automate multiple activities including changing input data, executing HYDRUS-1D and plotting the results of HYDRUS-1D simulations. The use of these scripts saved a significant amount of time compared to manual/Microsoft Excel based methods.

Code from one function and three scripts are provided below. A brief description of each is as follows:

- **Execute HYDRUS**. This function is called in other script files to execute the HYDRUS-1D application. HYDRUS-1D will execute whichever simulation that is listed in the 'Level\_01.dir' file.
- **Plot\_part1\_modelling\_hydrus**. This script plots infiltration and cumulative infiltration results for four different HYDRUS-1D simulations on the same plot.
- **Run HYDRUS with different head values.** This script is an example of a script file that changes the HYDRUS-1D simulation input values. For the selected simulation values, four different  $\psi$  values are simulated, HYDRUS is executed for each of the values and the results stored in arrays. The results from each of the simulations are then plotted.
- Nash Sutcliffe. This script compares simulated infiltration outputs from HYDRUS against the regression equations developed for the observed infiltration. The Nash Sutcliffe objective function is applied and the results plotted on a Predicted Observed plot.

This code is provided 'as is.' To use the code users will need to update folder locations for their own computer system. They will also need to generate the import functions to import data from the HYDRUS-1D output files (e.g. TLevel.out).

## Function: Execute HYDRUS

```
function [] = execute hydrus()
%Execute hydrus
   This function calls and executes Hydrus application. The simulation
8
8
   that is called is detailed in the Level Ol.dir file.
hydrus location = 'C:\Program Files (x86)\PC-Progress\Hydrus-1D 4.xx\'; %
Level 01.dir file is in this directory. This file needs to be changed to
choose a new simulation.
% hydrus location = 'C:\Users\Cameron\Documents\Engineering Degree - use
this one\19 S1 ENG4111 Research Project\Hydrus\';
cd(hydrus location);
hydrus executable = 'H1D CALC.exe';
call hydrus = fullfile(hydrus location, hydrus executable);
dos(call hydrus) % executes hydrus
end
```

# Script: Plot\_part1\_modelling\_hydrus

```
% Name: Plot_part1_modelling_hydrus.
```

```
% Purpose: To plot infiltration rate and cumulative infiltration for
% arbitarily created soil profile. Also present cumulative infiltration at
% the end of each simulation
% % Description: User selects which soils they are interested in (four are
% plotted). The data is imported from the T.OUT file for each simulation.
% The infiltration rate and cumulative infiltration versus time are plotted
% as well as the values for cumulative infiltration.
% I/O:
% Author: Cameron Leckie
% Date: 28 June 2019
% Test Log:
% Change Log:
응응
% Initialisation
clear all
clc
code dir = 'C:\Users\Cameron\Documents\MATLAB\ENG4111'; % All MATLAB code
relevant to the project is saved here.
hydrus dir = 'C:\Users\Cameron\Documents\Engineering Degree - use this
one\19 S1 ENG4111 Research Project\Hydrus Data Files'; % Hydrus data
directory
cd(hydrus dir);
22
% Find HYDRUS folders relevant to selected soil code within the current
directory.
list = ls; % assigns contents of folder to array list
list = cellstr(list); % converts list to a cell string
list(1:2) = []; % deletes the '.' and '..' entries from the array
88
% delete all .hld file extensions
idx hld = strfind(list, 'hld'); % row array to find which elements of list
have the extension hld in them'
find empty h1d = cellfun('isempty',idx h1d); % returns logical array
todelete = (size(idx h1d)); % vector that keeps track of which elements to
delete
i2 = 1; % reset second index for assigning values to todelete
for i = 1:length(idx h1d)
    % loop that checks each elemenent of idx to see if it is empty. If it
is
    % empty then the index of that value is assigned to the array todelete
  if find_empty_hld(i) == 0
   todelete(i2) = i;
   i2 = i2 + 1;
  end
end
list(todelete) = []; %delete all elements at once after the loop
```

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```
22
% Narrow down simulations you are searching for. Delete unwanted entries
% from list
disp(list);
prompt = 'Enter string to narrow down selection: ';
search string = input(prompt, 's');
idx search string = strfind(list, search string); % array to find which
elements of list include the selected soil code string. This will be used
to shorten array 'list'.
find empty search string = cellfun('isempty',idx search string); % returns
logical array
todelete search string = size(idx search string);
i2 = 1; % second index for assigning values to todelete
for i = 1:length(idx search string)
    % loop that checks each elemenent of idx soil code to see if it is
empty. If it is
   % empty then the index of that value is assigned to the array todelete
 if find empty search string(i) == 1
 todelete search string(i2) = i;
   i2 = i\overline{2} + 1;
 end
end
list(todelete search string) = []; %delete all elements at once after the
loop
88
% Select which simulations to import
disp('The following simulations meet your search criteria.');
disp(' ');
list index = 1:length(list);
simulation selector = table(list index', list)
simulation selector 1(1:4) = nan;
disp('Two options for chart legend are available');
disp('Option one: no crust, 2mm crust, 6mm crust, 10 mm crust');
disp('Option two: no crust, 2mm crust, 6mm crust, variable crust');
prompt = 'Enter 1 for option one and 2 for option two: ';
option selection = input(prompt);
for i = 1:4
    if option selection == 1
            simulation selector 1(i) = input('Enter index number in order:
no crust, 2mm crust, 6mm crust, 10 mm crust: ');
    else
        simulation selector 1(i) = input('Enter index number in order: no
crust, 2mm crust, 6mm crust, variable crust: ');
    end
```

Cameron Leckie ( ENG4112 Research Project end

```
22
% import selected data
cd(code dir)
[no crust, crust 2mm, crust 6mm, crust 10mm] =
plot part1 function (hydrus dir, list, simulation selector 1); % function
which imports data
응응
88
% Plot data in a subplot
% prompt = ('Enter soil texture type (e.g. Clay Loam) for chart title: ');
% chart title = input(prompt, 's');
subplot1 title = 'Infiltration Rate';
subplot2 title = 'Cumulative Infiltration';
clf % close any open plots
subplot(2,1,1)
loglog(no_crust(:,1), -3600 * no_crust(:,4),crust_2mm(:,1), -3600
*crust_2mm(:,4), '--', crust_6mm(:,1), -3600 * crust_6mm(:,4), 'k:',
crust_10mm(:,1), -3600 * crust_10mm(:,4), 'g-.', 'LineWidth', 1.5)
title(subplot1 title, 'FontSize',18);
if option selection == 1
    legend({'No Crust', '2mm Crust', '6mm Crust', '10mm Crust'},
'FontSize',14);
else
    legend({'No Crust', '2mm Crust', '6mm Crust', 'Variable Crust'},
'FontSize',14);
end
xlabel(' log(Time) - (s)', 'FontSize',18);
ylabel('log(Infiltration rate) - (mm/h)', 'FontSize',18);
subplot(2,1,2)
semilogx(no crust(:,1), no crust(:,18),crust 2mm(:,1), crust 2mm(:,18), '--
', crust_6mm(:,1), crust_6mm(:,18), 'k:', crust_10mm(:,1),
crust_10mm(:,18), 'g-.', 'LineWidth', 1.5)
title(subplot2_title, 'FontSize',18);
if option selection == 1
    legend({'No Crust', '2mm Crust', '6mm Crust', '10mm Crust'},
'FontSize',14);
else
    legend({'No Crust', '2mm Crust', '6mm Crust', 'Variable Crust'},
'FontSize',14);
end
xlabel('log(Time) - (s)', 'FontSize', 18);
ylabel('Cumulative infiltration - (mm)', 'FontSize',18);
```

```
Cameron Leckie (
ENG4112 Research Project
```

```
set(gcf, 'Position', get(0, 'Screensize')); % maximises figure on the
screen
save folder = 'C:\Users\Cameron\Documents\Engineering Degree - use this
one\19 S1 ENG4111 Research Project\Phase 2 - Modelling\RETC\Images';
cd(save folder) % changes folder to allow the figure to be saved in the
correct folder
22
% Cumulative infiltration values - identify the total cumulative
% infiltration at the end of the simulation
max no crust = max(no crust(:,18));
max crust 2mm = max(crust 2mm(:,18));
max crust 6mm = max(crust 6mm(:,18));
max crust 10mm = max(crust 10mm(:,18));
summary crust scenario = { 'max no crust'; 'max crust 2mm'; 'max crust 6mm';
'max crust 10mm'};
summary crust results = [max no crust; max crust 2mm; max crust 6mm;
max crust 10mm];
disp('Cumulative infiltration results');
table(summary crust scenario, summary crust results)
```

### Script: Run HYDRUS with different head values

```
% Name: Run Hydrus with different head values
% Purpose: Allow comparison of infiltration rates when different matric
% potentials are used.
% % Description: User selects a HYDRUS-1D simulation. HYDRUS is executed
using four different head values. The results are stored in arrays and
plotted.
% I/O:
% Author: Cameron Leckie
% Date: 30 June 2019
% Test Log:
% Change Log:
응응
% Initialisation
clear all
clc
code dir = 'C:\Users\Cameron\Documents\MATLAB\ENG4111'; % All MATLAB code
relevant to the project is saved here.
hydrus dir = 'C:\Users\Cameron\Documents\Engineering Degree - use this
one/19 S1 ENG4111 Research Project/Hydrus Data Files'; % Hydrus simulation
data directory
matlab dir = 'C:\Users\Cameron\Documents\MATLAB'; % directory where MATLAB
application files are stored
hydrus app dir = 'C:\Program Files (x86)\PC-Progress\Hydrus-1D 4.xx';
cd(hydrus dir);
22
% Find HYDRUS folders relevant to selected soil code within the current
directory.
```

Cameron Leckie (ENG4112 Research Project
```
list = ls; % assigns contents of folder to array list
list = cellstr(list); % converts list to a cell string
list(1:2) = []; % deletes the '.' and '..' entries from the array
22
% delete all .hld file extensions
idx hld = strfind(list, 'hld'); % row array to find which elements of list
have the extension hld in them'
find empty hld = cellfun('isempty',idx hld); % returns logical array
todelete = (size(idx h1d)); % vector that keeps track of which elements to
delete
i2 = 1; % reset second index for assigning values to todelete
for i = 1:length(idx h1d)
    % loop that checks each elemenent of idx to see if it is empty. If it
is
    % empty then the index of that value is assigned to the array todelete
  if find_empty_hld(i) == 0
  todelete(i2) = i;
   i2 = i2 + 1;
  end
end
list(todelete) = []; %delete all elements at once after the loop
22
% Narrow down simulations you are searching for. Delete unwanted entries
% from list
disp(list);
prompt = 'Enter string to narrow down selection: ';
search string = input(prompt, 's');
idx search string = strfind(list, search string); % array to find which
elements of list include the selected soil code string. This will be used
to shorten array 'list'.
find empty search string = cellfun('isempty',idx search string); % returns
logical array
todelete search string = size(idx search string);
i2 = 1; % second index for assigning values to todelete
for i = 1:length(idx search string)
    % loop that checks each elemenent of idx soil code to see if it is
empty. If it is
    % empty then the index of that value is assigned to the array todelete
 if find_empty_search_string(i) == 1
 todelete_search_string(i2) = i;
   i2 = i\overline{2} + 1;
 end
end
list(todelete search string) = []; %delete all elements at once after the
loop
```

22 % Select the simulation which will be run through Hydrus and update % Level 01.dir disp('The following simulations meet your search criteria.'); disp(' '); list index = 1:length(list); simulation selector = table(list index', list) prompt = 'Enter index number that aligns with selected simulation: '; simulation selector = input(prompt); % seeks user input on which simulation to run sim directory = cell2mat(list(simulation selector)); % the list which includes the simulation name is in a cell structure. Changes this to a string for selected simulation new level 01 = fullfile(hydrus dir, sim directory); % create file path to be inserted into Level 01. dir cd(code dir) level 01 format = '%s'; % format input into as a string fid = fopen('Level 01.dir', 'w'); fprintf(fid, level 01 format, new level 01); % print new file path to level 01.dir fclose(fid); copyfile('Level 01.dir', hydrus app dir) % copies new Level 01.dir file into Hydrus application folder 88 % Set up loop to simulation using four values of h and import that data. TLEVEL 1 = nan(100,22); % pre-allocate arrays to store T level.out  $TLEVEL^{2} = nan(100, 22);$ 3 = nan(100, 22);TLEVEL TLEVEL 4 = nan(100, 22);% provide user with choice to change head values % h values for use in sim in millimetres. % -100 is the default, -1000 is FC, -10000 is an intermediate, -150000 is % PWP default h values = [-100, -1000, -10000, - 150000]; % default values disp('Hydrus will now be called to execute four simulations using the following values of h (mm)'); disp(default h values'); prompt = 'Do you wish to use these values? '; h values choice = input(prompt, 's'); if h values choice == 'y' disp('You have selected the default h values');

```
elseif h_values_choice == 'n'
    disp('Enter four h values from least to most negative:')
    prompt = 'Enter h value: ';
    for i = 1 : length(default h values)
        default h values(i) = input(prompt);
    end
    disp('You have entered the following h values: ');
    disp(default_h_values');
end
disp('The default value for a ponded head scenario has been set at 10 mm.')
disp(' ');
prompt = 'Does this scenario have a ponded head? (y/n)? ';
ponded head = input(prompt, 's');
if ponded head == 'y'
   % prompt = 'Enter ponded head (positive value in mm): ';
   % ponded head value = input(prompt);
   ponded head value = 10;
else
    ponded head value = 0;
end
응응
8
for i = 1:4
    change head values in hydrus automatic (new level 01, ponded head value,
default h values(i))
    disp(default h values(i))
    execute hydrus % executes Hydrus using the simulation detailed in
Level 01.dir.
    cd(new level 01) % change directory to selected simulation
    if i == 1
        TLEVEL 1 = importTlevel('T Level.out'); % imports T Level.out from
selected simulation
    elseif i == 2
        TLEVEL 2 = importTlevel('T Level.out'); % imports T Level.out from
selected simulation
    elseif i == 3
        TLEVEL 3 = importTlevel('T Level.out'); % imports T Level.out from
selected simulation
    else
        TLEVEL_4 = importTlevel('T_Level.out'); % imports T_Level.out from
selected simulation
    end
end
응응
% Extract initial infiltration rate and final cumulative infiltration value
and save it in a text file that is appended to each iteration.
```

```
B - 9
```

```
% Extract initial infiltration rate
initial infiltration_rate = nan(1,4);
initial infiltration rate(1) = TLEVEL 1(1,4) * -3600;
initial infiltration rate(2) = TLEVEL 2(1,4) * -3600;
initial_infiltration_rate(3) = TLEVEL 3(1,4) * -3600;
initial infiltration rate(4) = TLEVEL 4(1,4) * -3600;
% Extract final infiltration rate
final infiltration rate = nan(1,4);
final infiltration_rate(1) = max(TLEVEL_1(:,4)) * -3600;
final infiltration rate(2) = max(TLEVEL 2(:,4)) * -3600;
final infiltration rate(3) = max(TLEVEL 3(:,4)) * -3600;
final infiltration rate(4) = max(TLEVEL 4(:,4)) * -3600;
% Extract cumulative infiltration
cumulative infiltration = nan(1,4);
cumulative infiltration(1) = max(TLEVEL 1(:,18));
cumulative infiltration(2) = max(TLEVEL 2(:,18));
cumulative infiltration(3) = max(TLEVEL 3(:,18));
cumulative infiltration(4) = max(TLEVEL 4(:,18));
% Summarise results
summary_of_simulations = cell(4,5); % create a cell array to store summary
data in.
for i = 1:4 % populate summary array
    summary of simulations(i,1) = {sim directory};
    summary of simulations(i,2) = {default h values(i)};
    summary of simulations(i,3) = {initial infiltration rate(i)};
    summary of simulations(i,4) = {final infiltration rate(i)};
    summary of simulations(i,5) = {cumulative infiltration(i)};
end
% disp(summary of simulations)
cd(hydrus dir)
sim sum format = '%s %i %f %f %f \r\n'; % format input
fid = fopen('Simulation summary.txt', 'a');
for i = 1:4
    fprintf(fid, sim sum format, summary of simulations{i,:});
end
fclose(fid);
응응
% Plot data in a subplot showing infiltration rate and cumulative
% infiltration
% chart_title = sim directory;
subplot1_title = strcat(' Infiltration Rate ');
subplot2 title = strcat(' Cumulative Infiltration ');
clf % close any open plots
```

```
B - 10
```

```
subplot(2,1,1)
loglog(TLEVEL_1(:,1), -3600 * TLEVEL_1(:,4), TLEVEL_2(:,1), -3600 *
TLEVEL_2(:,4), '--', TLEVEL 3(:,1), -3600 * TLEVEL 3(:,4), 'k:',
TLEVEL 4(:,1), -3600 * TLEVEL 4(:,4), 'g-.', 'LineWidth', 1.5)
title(subplot1 title, 'FontSize',18);
if h values choice == 'y'
    legend({'Default', 'FC', 'Intermediate', 'PWP'}, 'FontSize',14);
elseif h_values_choice == 'n'
    legend({'H1', 'H2', 'H3', 'H4'}, 'FontSize',14);
end
xlabel(' log(Time) - (s)', 'FontSize',18);
ylabel('log(Infiltration rate) - (mm/h)', 'FontSize',18);
subplot(2,1,2)
semilogx(TLEVEL 1(:,1), TLEVEL 1(:,18), TLEVEL 2(:,1), TLEVEL 2(:,18), '--
', TLEVEL 3(:,1), TLEVEL 3(:,18), 'k:', TLEVEL 4(:,1), TLEVEL 4(:,18), 'g-
.', 'LineWidth', 1.5)
title(subplot2 title, 'FontSize',18);
if h_values_choice == 'y'
    legend({'Default', 'FC', 'Intermediate', 'PWP'}, 'FontSize',14);
elseif h values choice == 'n'
    legend({'H1', 'H2', 'H3', 'H4'}, 'FontSize',14);
end
xlabel('log(Time) - (s)', 'FontSize',18);
ylabel('Cumulative infiltration - (mm)', 'FontSize',18);
set(gcf, 'Position', get(0, 'Screensize')); % maximises figure on the
screen
```

## Script: Nash Sutcliffe

```
% Name: Nash Sutcliffe
% Purpose: Compare observed versus modelled infiltration to determine the
closeness of fit
% Description:
% 1. Imports selected simulation results from HYDRUS.
% 2. Applies time values used in HYDRUS to observed infiltration function.
% 3. Plots observed versus modelled infiltration
% 4. Calculates objective function value.
% Author: Cameron Leckie
% Date: 13 August 2019
% Test Log:
% Change Log:
22
% Initialisation
clear all
clc
output data = 'C:\Users\Cameron\Documents\Engineering Degree - use this
one\19 S1 ENG4111 Research Project\Phase 4 - Results and
Analysis\Mod Obs data';
cd(output data);
```

```
code dir = 'C:\Users\Cameron\Documents\MATLAB\ENG4111'; % All MATLAB code
relevant to the project is saved here.
hydrus dir = 'C:\Users\Cameron\Documents\Engineering Degree - use this
one/19 S1 ENG4111 Research Project/Hydrus Data Files'; % Hydrus simulation
data directory
matlab dir = 'C:\Users\Cameron\Documents\MATLAB'; % directory where MATLAB
application files are stored
obs infil dir = 'C:\Users\Cameron\Documents\Engineering Degree - use this
one\19 S1 ENG4111 Research Project\Phase 4 - Results and
Analysis\Mod Obs data'
text file header format = '%s %s\n';
text file body format = '%5.0f %8.7f\r\n';
syms t
88
% 1. Imports selected simulation results from HYDRUS.
[simulation list] = Filter hydrus simulations(hydrus dir)
prompt = 'Enter index number of simulation you wish to select: ';
selected simulation index = input(prompt);
selected simulation = simulation list{selected simulation index, 2}; %
extract folder name from table as a cell format
selected simulation = char(selected simulation) % convert to string
folder_name = fullfile(hydrus_dir, selected_simulation); % full file path
to simulation
cd(folder name)
Modelled TLevel = importTlevel('T Level.out');
Modelled time values = Modelled TLevel(1:end-1, 1);
Modelled infiltration values = -Modelled TLevel(1:end-1, 4);
22
% plot(Modelled time values, Modelled infiltration values);
88
% Selected observed infiltration event
simulation title index = [1; 2; 3; 4; 5; 6; 7];
simulation title = {'Sod 1 R2'; 'Sod 1 R3'; 'Sod 2 R2'; 'Sod 2 R3'; 'Sod 3
R2'; 'Sod 3 R3'; 'Chr 1 R2'};
simulation selection table = table(simulation title index,
simulation title);
disp(simulation selection table);
prompt = 'Enter index number that aligns with selected simulation: ';
k = input(prompt);
88
% values required to calculate infiltration function
simulation_time_to_first_measurement = [128; 99; 130; 99; 128; 99; 99]; %
need to change this to first measurement
simulation time to last measurement = [1520; 1528; 1512; 1542; 1520; 768;
933];
simulation rainfall intensity = [0.033610148; 0.02940118; 0.032100543;
0.029236421; 0.031037122
; 0.028115882; 0.027170804];
```

```
simulation_inputs = table(simulation_title,
simulation_time_to_first_measurement, simulation_time_to_last_measurement,
simulation rainfall intensity)
time values = Modelled time values; % rename variable for next section
응응
\% Calculate observed infiltration values using time values from HYDRUS
% simulation
    for i = 1:length(time_values) % loop to assign infiltration rate values
for each function
        if time values(i) <= simulation_time_to_first_measurement(k)</pre>
            simulation infiltration rate(i) =
simulation rainfall intensity(k);
        else
            if k == 1
                simulation infiltration rate(i) = 3e-9 .* time values(i).^2
- 6e-6 .* time values(i) + 0.0046; % need to add if else statements here
for each regression equation
            elseif k == 2
                simulation_infiltration_rate(i) = -6e-4 .*
log(time values(i)) + 0.0043;
            elseif k == 3
                simulation infiltration rate(i) = 6e-12 .*
time values(i).<sup>^3</sup> - 2e-8 .* time values(i).<sup>^2</sup> + 8e-6 .* time values(i) +
0.00\overline{28};
            elseif k == 4
                simulation_infiltration_rate(i) = -5e-4 .*
log(time values(i)) + 0.0036;
            elseif k == 5
                simulation infiltration rate(i) = 3e-9 .* time values(i).^2
- 9e-6 .* time values(i) + 0.0075;
            elseif k == 6
                simulation_infiltration_rate(i) = -8e-4 .*
log(time values(i)) + 0.0051;
            else
                simulation infiltration rate(i) = 4e-9 .* time values(i).^2
- 5e-6 .* time values(i) + 0.0038;
            end
        end
    end
    % disp(simulation infiltration rate);
88
```

```
% use values that occur only after runoff has occurred
time_of_first_measurement = simulation_time_to_first_measurement(k); % time
in seconds when first measurement taken
counter = 1;
while Modelled time values(counter) < time of first measurement % while
loop to find the index (counter) when first measurement occurs
    counter = counter + 1;
    % disp(Modelled time values(counter));
end
% concatenate arrays
disp(Modelled time values(counter));
disp(counter);
a = table(simulation infiltration rate', Modelled infiltration values)
% counter
simulation infiltration rate(counter+1)
Modelled infiltration values (counter+1)
simulation infiltration rate = simulation infiltration rate(counter+1:end);
Modelled infiltration values = Modelled infiltration values(counter+1:end);
88
% Calculate Nash Sutcliffe (measure of fit to 1:1 line)
x bar = mean(simulation infiltration rate);
xi xbar = nan(1, length(simulation infiltration rate));
yi xi = nan(1, length(simulation infiltration rate));
for i = 1:length(simulation infiltration rate)
    xi_xbar(i) = (simulation_infiltration_rate(i) - x_bar).^2;
    yi_xi(i) = (Modelled_infiltration_values(i) -
simulation_infiltration_rate(i)).^2;
end
sum xi xbar = sum(xi xbar);
sum yi xi = sum(yi xi);
Nash sutcliffe E = (sum xi xbar - sum yi xi)/sum xi xbar;
22
% Plot modelled versus observed
% 1 to 1 line coordinates
x 1 1 = [0, 0.007];
y 1 1 = [0, 0.007];
Modelled infiltration values = Modelled infiltration values'; % to align
indexing (rows and columns)
clf % close any open plots
hold on
% swap observed and modelled around on the axis. Observed should be on the
```

```
% x axis
plot(x_1_1, y_1_1, 'r', 'LineWidth', 2);
plot(simulation_infiltration_rate, Modelled_infiltration_values,
'LineWidth', 2);
ylabel('Modelled infiltration rate', 'FontSize', 24)
xlabel('Observed infiltration rate', 'FontSize', 24);
text(0.0005, 0.005, sprintf('Nash Sutcliffe Criteria: %f',
Nash_sutcliffe_E), 'FontSize', 24)
```

hold off

## Appendix C – Rainfall simulation results

The data collected from each of the rainfall simulation experiments is detailed in this Appendix.

	Da	te		1-Jul-19	Plot 1				(Sodos	ol #3)			Plot 2				(Sodoso	#2)		
	P	Plot grad	dient						20%								20%			
	Rune	off initia	ation	(s)					39								39 sec			
				• •							Instantaneou	Instantaneous							Instantaneo	Instantaneous
					Bottle	Bottle	Wet	OD mass	Sediment	Runoff	s runoff	infiltration	Bottle	Bottle	Wet mass	OD mass	Sediment	Runoff	us runoff	infiltration
					number	mass (g)	mass (g)	(g)	mass (g)	mass (g)	(mm/h)	(mm/h)	number	mass (g)	(g)	(g)	mass (g)	mass (g)	(mm/h)	(mm/h)
Pr	e-rair	n moist	ure s	ample	771	66.864	157.2	151.1	84.236				772	66.834	143.7	139	72.166			
Tir	ne fro	om stat	D	uration																
(mi	n:sec	to min)	) (:	secs)																
1		30	90	16.38	773	66.785	306.5	82.3	15.5	208.7	81.5	29.4	805	68.801	289.9	80.2	11.399	209.7	81.9	29.0
2	3	30 1	50	16.19	774	66.768	301.1	81.3	14.5	205.3	81.1	29.8	804	68.736	300.4	80.4	11.664	220.0	87.0	23.9
3	3	37 2	17	16.25	775	66.861	311.4	82.9	16.0	212.5	83.7	27.2	803	66.78	310.5	79.4	12.62	231.1	91.0	19.9
5		8 3	80	16.59	776	66.933	331.7	84.5	17.6	229.6	88.6	22.3	802	68.781	327.1	81.9	13.119	245.2	94.6	16.3
7		7 4	27	16.41	777	66.817	347.0	86.2	19.4	241.4	94.2	16.8	801	68.731	336.9	83.2	14.469	253.7	98.9	12.0
9		7 5	47	15.75	778	66.453	341.5	86.1	19.6	235.8	95.8	15.1	800	66.829	330.4	81.6	14.771	248.8	101.1	9.8
12		5 7	25	15.82	779	66.675	350.4	86.3	19.6	244.5	98.9	12.0	799	68.627	351.7	85.1	16.473	266.6	107.9	3.1
15		5 9	05	16.13	780	66.802	355.6	86.7	19.9	249.0	98.8	12.1	798	66.851	359.2	86.1	19.249	273.1	108.4	2.5
20	1	15 12	15	15.72	781	66.583	356.1	85.8	19.2	251.1	102.2	8.7	797	66.739	351.5	85.2	18.461	266.3	108.4	2.5
25	1	10 15	10	16.09	782	66.971	344.2	82.9	15.9	245.4	97.6	13.3	796	66.862	354.0	84.5	17.638	269.5	107.2	3.7
Ро	st-rai	in moist	ure s	sample	783	66.953	175.1	156.5	89.547				795	66.903	226.3	156.5	89.597			
		Rain	off			Rai	n gauges		-											
			25 m	nin		48		47						Infiltrat	ion Da	to Vorc	uc Tim	•		
			58 s	ec	52	Plot 1	49	Plot 2	42					IIIIIIIIa		le vers		e		
			6 m	nsec		43	1	42			3	5.0								
		Time	e (s)	1558.06																
					Plot 1 Average	48	Plot 2 Ave	45												
				Rainfall ir	ntensity (mm/s)	0.031		0.029			3	0.0								
				Rainfall in	tensity (mm/h)	110.9		104.0												
					Moisture cont	Plot 1	Plot 2					5.0								
					Mass soil (g)	84.236	72.166													
					Mass water (g)	6.1	4.7				<u> </u>	0.0								
				Before	MC	7.2%	5.6%				ate									
					Mass soil (g)	89.547	89.597				n R									
					Mass water (g)	18.6	69.8				1	5.0								
				After	MC	20.8%	/7.9%													
											<u> </u>									
											1	0.0								
																	$\mathbf{N}$		•	
												5.0								
																	~			
												0.0 T	150	217 3	108 10	7 54	725	005	1215	1510
												50	100	/ T		nfall comm	encement la	)	1213	1310
														I	inte since fal		encement (S	1		
														Plot 1 (	Sodosol #3)	Plot	2 (Sodosol i	<b>#2)</b>		
											L						- ,500,00011	-,	1	
Came	ron L	eckie (											[		[					

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	Dat	te	1-Jul-19				Plot 1	(Sodosol #:	1)						Pl	ot 2 (Hydro	sol)		
		Plot grad	dient					20%								20%			
	Run	off initiat	tion (min)					50 sec								6 min 40 se	C		
				Bottle	Bottle	Wet mass	OD mass	Sediment	Runoff mass	Instantan eous runoff	Instantane ous infiltration	Bottle numb	Bottle mass	Wet mass	OD mass	Sediment	Runoff mass	Instantan eous runoff	Instan ous infiltra
				number	mass (g)	(g)	(g)	mass (g)	(g)	(mm/h)	(mm/h)	er	(g)	(g)	(g)	mass (g)	(g)	(mm/h)	(mm/
	Pre-ra	ain moist	ure sample	794	66.903	129.4	127	59.997	(0)			351	66.645	129.2	117.1	50.455	(0)	,	Ì
-	Time fro	m stat																	
(	min:sec	to sec)	Duration (secs)																
2	2 9	129	16.07	793	66.808	283.1	83.1	16.3	183.7	73.2	40.9	352	66.733	98.9	67.5	0.7	31.4	44.9	
3	8 9	189	16.21	792	68.69	292.7	84.5	15.8	192.4	76.0	38.1	353	66.818	112.5	68.2	1.3	44.3	62.8	
2	l 9	249	15.97	791	68.462	302.8	84.3	15.8	202.7	81.2	32.9	354	66.854	121.3	68.5	1.6	52.8	75.9	
5	5 9	309	16.10	790	66.825	311.5	83.5	16.7	211.3	84.0	30.1	355	66.89	127.6	68.9	2.0	58.7	84.7	
7	' 9	429	15.72	789	68.828	334.1	85.8	17.0	231.3	94.2	19.9	356	66.886	127.6	68.9	2.0	58.7	87.9	
8	3 7	487	15.88	788	66.798	342.9	85.3	18.5	239.1	96.4	17.7	357	66.886	133.8	69.0	2.1	64.8	96.5	
9	30	570	15.65	787	66.731	353.1	85.3	18.6	249.2	101.9	12.2	358	66.86	134.7	69.0	2.1	65.7	89.8	
11	. 8	668	15.60	786	66.85	338.4	84.7	17.9	235.9	96.8	17.3	359	66.88	126.1	68.7	1.8	57.4	85.9	
13	3 20	800	15.03	785	68.709	340.5	86.0	17.3	237.2	101.0	13.1	360	66.887	135.2	69.0	69.0	66.2	89.4	
16	5 19	979	15.11	784	66.706	368.3	87.4	20.7	260.2	110.2	3.9								
20	) 15	1215	16.47	385	66.9	378.4	86.7	19.8	271.9	105.7	8.4								
25	5 15	1515	15.03	384	66.886	328.4	80.6	13.7	234.1	99.7	14.4								
30	) 15	1815	16.66	383	66.845	365.4	81.4	14.6	269.4	103.5	10.6								
	Post-r	ain moist	ure sample	382	66.405	172.5	155	88.795				361	66.877	147.1	114.1	47.223			

	Date		1-Jul-19				Plot 1	(Sodosol #1	L)						Р	lot 2 (Hydro	sol)							
	P	lot gradie	ent					20%								20%								
	Runof	f initiatio	on (min)					50 sec		Instantan	Instantano					6 min 40 se	ec	Instantan	Instantano					
						Wet	OD		Runoff	eous		Bottle	Bottle	Wet	OD		Runoff	eous	ous					
				Bottle	Bottle	mass	mass	Sediment	mass	runoff	infiltration	numb	mass	mass	mass	Sediment	mass	runoff	infiltration					1
				number	mass (g)	(g)	(g)	mass (g)	(g)	(mm/h)	(mm/h)	er	(g)	(g)	(g)	mass (g)	(g)	(mm/h)	(mm/h)					
	Pre-rain	n moistur	re sample	794	66.903	129.4	127	59.997				351	66.645	129.2	117.1	50.455								
Tin	ne from	stat																		Tim	e from	stat	Duration	
(mi	n:sec to	sec) D	Duration (secs)																	(mir	sec to	min)	(s)	
2	9	129	16.07	793	66.808	283.1	83.1	16.3	183.7	73.2	40.9	352	66.733	98.9	67.5	0.7	31.4	44.9	57.8	7	9	429	15.7	
3	9	189	16.21	792	68.69	292.7	84.5	15.8	192.4	76.0	38.1	353	66.818	112.5	68.2	1.3	44.3	62.8	40.0	8	7	487	15.9	
4	9	249	15.97	791	68.462	302.8	84.3	15.8	202.7	81.2	32.9	354	66.854	121.3	68.5	1.6	52.8	/5.9	26.9	11	30	5/0	15.7	
5	9	309	16.10	790	62.825	311.5	83.5 85.0	10.7	211.3	84.0	30.1	355	66.89	127.6	68.9	2.0	58./	84./	18.1	12	8 2	608 רסד	15.0	
/ 8	7	429	15.72	785	66 798	34.1	85.3	17.0	231.3	94.2	19.9	350	66 886	133.8	69.0	2.0	64.8	96.5	63	16	∠ 19	02 979	15.0	
9	30	570	15.65	787	66.731	353.1	85.3	18.6	249.2	101.9	12.2	358	66.86	134.7	69.0	2.1	65.7	89.8	13.0	20	15	1215	16.5	
11	8	668	15.60	786	66.85	338.4	84.7	17.9	235.9	96.8	17.3	359	66.88	126.1	68.7	1.8	57.4	85.9	16.8	25	15	1515	15.0	
13	20	800	15.03	785	68.709	340.5	86.0	17.3	237.2	101.0	13.1	360	66.887	135.2	69.0	69.0	66.2	89.4	13.4	30	15	1815	16.7	
16	19	979	15.11	784	66.706	368.3	87.4	20.7	260.2	110.2	3.9													
20	15	1215	16.47	385	66.9	378.4	86.7	19.8	271.9	105.7	8.4													
25	15	1515	15.03	384	66.886	328.4	80.6	13.7	234.1	99.7	14.4													
30	15	1815	16.66	383	66.845	365.4	81.4	14.6	269.4	103.5	10.6													
F	Post-rai	n moistu	re sample	382	66.405	172.5	155	88.795				361	66.877	147.1	114.1	47.223								
	Pa	in off			Pain																			
	Γd	30 n	nin		Kain (	gauges	52								Infi	Itration	Rate	Versus	Time					
		30 s	ec	63	Plot 1	57	Plot 2	54																
		39 n	nsec		52		45		<u> </u>		70.0 -													
	Tir	ne (s)	1830.39			_																		
			Р	lot 1 Average	58	Plot 2	52.3				60.0 -												—	
			Rainfall int	ensity (mm/s)	0.032		0.029																	
			Rainfall int	ensity (mm/h)	114.1		102.8				_ 50.0 -			-+									_	
											4/u													
				N 4 - 1 -																			_	
						Plot 2					late		$\mathbf{i}$											
				Ividss soll (g)	עפט <u>א</u> יייייייייייייייייייייייייייייייייייי	12 1					uo ac a													
			Before	MC	<u> </u>	24.0%					- 0.0⊱ <b>rati</b>													
			BEIDIE	Mass soil (g)	88.795	47.22					nfilt			$\mathbf{N}$										
				lass water (g)	17.3	33					- 20.0 -												—	
			After	MC	19.5%	69.9%									$\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{\mathbf{$	$\frown$								
											10.0 -				*							-		
																	$\sim$							
											00-							1	1			1	_	
												)	200	400	600	0 800	100	0 1200	1400	1600	18	800	2000	
																Time sin	ce rainfall	commencer	nent (s)					

			1																	1
	Date	9	2-Jul-19	Plot 1				Sodos	ol #2			Plot 2				C	hromoso	bl		
	Plot	gradie	nt				20	)%								20%	0			
R	unoff	initiatio	on (s)		1	1	5	5		1	I			1		78	1	1		
							OD		Runoff	Instantaneous	Instantaneous		Bottle	Wet	OD		Runoff		Instantaneous	
					Bottle	Wet mass	mass	Sediment	mass	runoff	infiltration	Bottle	mass	mass	mass	Sediment	mass	Instantaneous	infiltration	
				Bottle number	mass (g)	(g)	(g)	mass (g)	(g)	(mm/h)	(mm/h)	number	(g)	(g)	(g)	mass (g)	(g)	runoff (mm/h)	(mm/h)	
Pre-	rain n	noisture	sample	141	67.062	117.3	111.8	44.738				175	67.089	104.4	100.6	33.511				0.1133
Tim	e fror	n stat	Duratio																	
(mir	n:sec t	o sec)	n (secs)																	Time f
2	10	130	16.69	142	67.051	352.5	79.4	12.4	260.7	100.0	15.6	5 174	67.023	131.3	68.6	1.6	61.1	82.4	19.2	
3	10	190	15.97	143	67.017	350.4	79.6	12.6	258.2	103.5	12.1	173	67.06	131.7	68.7	1.6	61.4	86.4	15.1	
4	11	251	14.59	144	67.023	332.4	79.4	12.4	240.6	105.6	10.0	172	67.012	127.7	68.5	1.5	57.7	89.0	12.6	
5	9	309	16.09	145	67.033	343.5	81.2	14.2	248.1	98.7	16.9	171	66.971	133.9	68.7	1.7	63.5	88.8	12.8	
7	7	427	17.32	146	67.002	364.3	82.6	15.6	266.1	98.3	17.2	170	67.013	138.7	68.9	1.9	67.9	88.2	13.3	
9	20	560	16.50	147	67.003	360.1	82.9	15.9	261.3	101.4	14.2	169	67.086	136.4	68.7	1.6	66.1	90.1	11.4	
12	8	728	16.07	148	67.032	365.4	82.3	15.3	267.8	106.7	8.9	168	67.095	132.8	68.5	1.4	62.9	88.1	13.5	
15	4	904	15.06	149	66.998	347.7	81.0	14.0	252.7	107.4	. 8.2	167	67.068	130	68.3	1.2	60.5	90.3	11.2	
20	16	1216	15.62	150	67.001	362.2	81.1	14.1	267.0	109.4	6.2	166	66.959	131.2	68.3	1.3	61.6	88.7	12.9	
25	12	1512	15.13	151	67.015	350	80.2	13.2	256.6	108.6	7.0	165	67.053	131.5	68.2	1.1	62.2	92.4	9.1	
		0																		
		0																		
		0																		
Post	-rain r	noisture	e sample	152	67.048	183.2	165.5	98.452				164	67.07	157.1	139.2	72.13				0.24816
		Rain off	f		Rain	gauges														
		25	min		52			48												
		42	sec	54	Plot 1	44	42	Plot 2	45											
		3	msec		48	1		39				In	filtrat	tion E	) ata	lorene	Timo			
		Time (s)	) 1542.03										mua		vale v	/ 1 3 4 3	iiiie			
				Plot 1 Average	49.5	Plot 2 Averag	43.5			25	.0									
			Rain	nfall intensity (mm/s)	0.032		0.028													
			Rain	fall intensity (mm/h)	115.6		101.6													
										20	0.0									
				Moisture content	Plot 1	Plot 2				(4/										
				Mass soil (g)	44.738	33.511				E 1		$\square$								
				Mass water (g)	5.5	3.8				<b>te</b>					~					
			Before	MC	12.3%	11.3%				I Ra		Y		$\checkmark$		<hr/>				
				Mass soil (g)	98.452	72.13				10 <b>tio</b>	.0	N		-						
				Mass water (g)	17.7	17.9				Itra										
			After	MC	18.0%	24.8%				Infi										
										5	.0 +									
										0	0.0 +	1	1	T	1			1 1		
											0 20	0 4	100	600	80	0 10	00	1200 1400	1600	
													Tiı	ne since	rainfall c	ommencem	ent (s)			
														- Sodosol	#2 —	- Chromoso	ol			
		_																		
Came	ron Le	eckie (																		•

ENG4112 Research Project

	Date	9	2-Jul-19	Plot 1			Fe	rrosol				Plot 2				Hyd	rosol							
	Plo	t gradien	nt				20%									20%								
	Runoff	f initiatio	on (s)				669									318								
								Sedim		Instant	Instanta					Sedim			Instantane					
							OD	ent	Runoff	aneous	neous	Bottle		Wet	OD	ent	Runoff	Instantane	ous					
				Bottle	Bottle mass	Wet	mass	mass	mass	runoff	infiltrati	numbe	Bottle	mass	mass	mass	mass	ous runoff	infiltration					
				number	(g)	mass (g)	(g)	(g)	(g)	(mm/h)	on	r	mass (g)	(g)	(g)	(g)	(g)	(mm/h)	(mm/h)					
Pre	-rain n	noisture	sample	770	66.763	100.1	94.6	27.837				163	67.099	<mark>138.7</mark>	113	45.9	)							
Tim	e fron	n stat	Duration																	Time from	m stat (mi	in:sec to		
(mi	n:sec t	o sec)	(secs)																		min)		Duration (s)	
11	20	680	14.91	769	66.617	98.7	66.9	0.3	31.5	13.5	92.1	162	67.051	128.4	68.5	1.4	58.5	82.0	16.9	6	20	380	16.04	
12	16	736	14.53	768	66.737	104.7	67.0	0.3	37.4	16.5	89.1	161	67.066	117	68.2	1.1	47.7	67.2	31.7	7	20	440	15.97	
13	19	799	17.09	767	66.801	115.4	67.2	0.4	47.8	17.9	87.7	160	66.986	131.3	68.6	1.6	61.1	79.5	19.3	8	20	500	17.28	
14	19	859	15.94	766	68.589	111.7	69.0	0.4	42.3	17.0	88.6	159	67.033	131.2	68.7	1.7	60.8	86.6	12.3	9	20	560	15.81	
16	18	978	15.87	765	66.879	113	67.4	0.5	45.1	18.2	87.4	158	67.004	129.1	68.6	1.6	5 58.9	88.9	10.0	11	20	680	14.91	
19	16	1156	14.69	764	66.918	120	67.3	0.4	52.3	22.8	82.8	157	66.977	125	68.4	1.4	55.2	85.4	13.4	12	16	736	14.53	
22	15	1335	16.25	763	66.911	122.8	67.4	0.5	54.9	21.6	84.0	156	67.036	137.5	68.9	1.9	66.7	87.9	11.0	13	19	799	17.09	
25	41	1541	16.41	762	66.884	123.5	67.4	0.5	55.6	21.7	83.9	155	67.052	132.7	68.8	1.7	62.2	87.7	11.1	14	19	859	15.94	
30	25	1825	15.44	761	66.867	123.2	67.3	0.4	55.5	23.0	82.6	154	67.02	131.8	68.7	1.7	61.4	87.1	11.8	16	18	978	15.87	
		0										153	67.029	134.9	68.9	1.9	64.1	98.2	0.7	19	16	1156	14.69	
		0										736	68.755	137.7	70.6	1.8	65.3	90.4	8.5	22	15	1335	16.25	
		0										737	66.648	134.9	68.5	1.9	64.5	88.5	10.4	25	41	1541	16.41	
		0										738	66.725	128.6	68.5	1.8	3 58.3	85.0	13.9	30	25	1825	15.44	
Pos	t-rain ı	moisture	sample	760	66.799	123.6	105.1	38.301				739	68.773	153.1	117.3	48.53	35.8							
	I	Rain off			Rain	gauges																		
		31	min		59			56																
		6	sec	58	Plot 1	53	52	Plot 2	51															
			msec		49			46																
	-	Time (s)	1866												Inf	iltrat	tion R	ate Ver	sus Tim	e				
			Plo	ot 1 Average	54.75	Plot 2 Ave	51.25				1(	00.0												
		R	ainfall inte	nsity (mm/s)	0.029		0.027				T(	00.0												
		Ra	ainfall inte	nsity (mm/h)	105.6		98.9				9	90.0 +				•	$\overline{}$							
											1	80.0 -												
			Moist	ture content	Plot 1	Plot 2					्रि ।	70 0												
				Mass soil (g)	27.837	45.901					ШШ Ц	, 0.0												
				ass water (g)	5.5	25.7					ie (r	60.0 +												
			Before	MC	19.8%	56.0%					Rat	50.0 🔶												
				Mass soil (g)	38.301	48.527					ion	40.0 ⊥												
				ass water (g)	18.5	35.8					trat	-0.0												
			After	MC	48.3%	73.8%					nfil	30.0 +			$\wedge$									
											-	20.0 🔶			$\frown$									
												100					$\sim$							
												10.0												
												0.0 +	1		1	1	1	1		1	1	1		
												0	200	40	00	600	800	1000	1200	1400	1600	1800	2000	
																Ti	me since	rainfall comm	nencement (s	5)				
																	_							
																_	- Ferros	ol — Hyd	drosol					

00	1.00	1000	20.00		
100	1600	1800	2000		

																			1
Da	te	2-Jul-19	Plot 1				Sodo	sol #1			Plot 2				Soc	losol #3			
Plo	ot gradient						20%								20%				
Runo	ff initiation	(s)					40								40				
		( )				OD				Instantaneous			Wet	OD		Runoff		Instantaneous	1
				Pottlo	Wot mass	marc	Sodimont	Dunoff	Instantanoous	infiltration	Pottlo	Pottlo	macc	macc	Sodimont	macc	Instantanoous	infiltration	
			I	bottle	vvet mass	111855	Seument	Kulloli			bottle	Bottle			Seument				
			Bottle number	mass (g)	(g)	(g)	mass (g)	mass (g)	runoff (mm/h)	(mm/h)	number	mass (g)	(g)	(g)	mass (g)	(g)	runoff (mm/h)	(mm/h)	-
Pre-rain	moisture s	ample	759	66.82	147.9	141.5	74.68				525	66.988	3 145	139.4	72.412				<u> </u>
Time fro	om stat	Duratio																	ļ
(min:sec	to sec)	n (secs)																	Time
2 8	128	15 50	758	66 831	353.6	81 7	14 9	257.0	106 1	14 9	524	1 66 379	325 3	81.9	15 5	227 9	94 1	17.6	
2 0	120	16 54	753	66.956	280.2	02.0	15.0	201.6	108.0	12.1	522	66 672	255.6	02.5	16 9	255.2	09.9	12.0	
3 9	109	10.54	757	00.850	360.3	02.0	15.9	201.0	108.9	12.1	523	00.072	355.0	03.3	10.0	255.5	90.0	13.0	
4 10	250	16.44	/56	66.866	369.4	80.9	14.0	274.5	106.8	14.1	522	66.546	352.2	81.4	14.9	255.9	99.6	12.1	ļ
5 11	311	14.81	755	66.807	348.3	79.8	13.0	255.5	110.4	10.6	521	L 66.617	7 336.8	81.1	14.5	241.2	104.2	7.5	<u> </u>
79	429	15.66	754	66.792	373.6	80.8	14.0	278.8	113.9	7.1	520	66.487	7 359.3	83.4	16.9	259.0	105.8	5.9	
96	546	16.00	753	68.749	383.3	81.9	13.2	288.2	115.3	5.7	519	66.682	363.7	83.7	17.0	263.0	105.2	6.5	
13 13	793	15.12	752	66.818	361.5	77.9	11.1	272.5	115.4	5.6	518	66.695	349.5	81.5	14.8	253.2	107.2	4.6	
16 10	970	16 19	751	66 739	375 3	79.0	12 3	284.0	112 3	87	517	7 66.803	372 5	81.2	14 4	276.9	109 5	23	
20 20	1220	10.15	751		373.3	70.0	12.5	204.0	112.3	<u> </u>	517	CC 827	2 250 7	70.0	12.1	270.5	105.5	2.5	
20 28	1228	15.50	750	66.669	3/3.3	79.9	13.2	280.2	115.7	5.3	510	00.83/	359.7	79.9	13.1	200.7	110.1	1.0	
25 20	1520	16.25	749	66.774	383.4	79.7	12.9	290.8	114.5	6.5	515	66.837	/ 359.6	80.1	13.3	266.2	104.9	6.9	ļ
	0																		
	0																		
	0																		
Post-rain	moisture	ample	748	68 734	153.8	139.9	71 166				514	1 66.875	176.8	159.6	92 725				1
i ost ium	moistare	ampic	/ 10	00.754	133.0	100.0	, 1.100				51	00.075	, 1,0.0	100.0	52.725				
	Dein off			Dei															
	Rain off			Rai	n gauges	1													
	25	min		52			50												Į
	54	sec	54	Plot 1	53	53	Plot 2	45											
	59	msec		50			45												
	Time (s)	1554.6																	
			Plot 1 Average	52 25	Plot 2 Avera	48 25													
		Dainfa	ll intoncity (mm/c)	0.024		0.021					Infilt	ration l	Rate V	'ersus	Time				
		Dainfa		121.0		0.031			20.0										
		Rainta	li intensity (mm/n)	121.0		111./			20.0										ļ
									18.0										
			Moisture content	Plot 1	Plot 2				16.0										
			Mass soil (g)	74.68	72.412			2	10.0										
			Mass water (g)	6.4	5.6			1/4	• 14.0 +										
		Before	<u>м</u> с	8.6%	7 7%			<u> </u>	. 12 0	$\times$									
			Mass coil (a)	71 166	<u>،,،،</u>			te	12.0										
				/1.100	32.723			Ra	10.0	-+									
			iviass water (g)	13.9	1/.2			ioi	80						$\sim$				Į
		After	MC	19.5%	18.5%			rat	0.0										
									6.0							$\overline{}$	/		1
									4.0										
																			1
									2.0										
									0.0	1									1
									0	200	400	600	800	)	1000	1200	1400	1600	1
									-			Timo cinco	rainfall co	mmonor	mont (c)	00	2.00		
												nine since		minencer	nent (s)				
												<u> </u>	1.114	<b>.</b>					
												Sodoso	1#1	- Sodosol i	# <b>3</b>				
																			1

	Da	te	3-Jul-19	Plot 1				Hydro	sol			Plot 2				Ferr	osol			
	Pl	ot gradier	nt					20%								20%				
	Runo	ff initiatio	on (s)		[	T	r –	344	1	1	1		[	r		344	[	1		
					<b>-</b>		OD	<b>.</b>	- "	Instantaneo	Instantane	<b>_</b>	<b>-</b>	Wet			- "		Instantaneous	
					Bottle	Wet	mass	Sediment	Runoff	us runoff	OUS	Bottle	Bottle	mass	OD mass	Sediment	Runoff	Instantaneous	Infiltration	
Dro	_rain	moisture	samnlo	Bottle number	mass (g)	115 2	(g) 07 1	mass (g)	mass (g)	(mm/n)	Inflitration	number 513	mass (g)	(g) 130 /	(g) 120 4	mass (g)	mass (g)	runoff (mm/n)	(mm/n)	
Tin	ne fro	m stat	Duration	<u> </u>	00.70	115.5	57.1	50.52				515	00.755	133.4	120.4	33.007				
(mi	n:sec	to sec)	(secs)																	Time f
3	40	220	15.03	492	66.744	125.5	68.0	1.3	56.2	23.9	79.7	512	66.703	113.9	67.0	0.3	46.6	69.8	23.9	
4	39	279	15.06	493	66.831	131.5	68.2	1.4	61.9	26.3	77.3	511	66.725	116.1	67.1	0.4	48.6	72.6	21.1	
5	38	338	14.97	494	66.851	126.4	68.2	1.3	56.9	24.3	79.3	510	66.829	117.8	67.1	0.3	50.4	75.8	17.9	
6	38	398	15.32	495	66.858	128.4	68.3	1.4	58.7	24.5	79.1	509	66.767	118.3	67.3	0.5	50.5	74.1	. 19.6	
8	36	516	15.88	496	66.856	133.6	68.4	1.5	63.7	25.7	78.0	508	66.776	118.1	67.1	0.3	50.7	71.8	21.9	
11	35	695	16.41	497	66.704	143.1	68.7	2.0	72.4	28.2	75.4	507	66.799	127.6	67.1	0.3	60.2	82.5	11.2	
20	30 47	1247	15.53	498	66 520	140.4	68.2	1.8	70.4	29.0	74.0	505	66 / 25	120.2	66.8	0.5	58.5	84.8	8.9	
20	47	1605	15.81	500	66 543	133.9	68.3	2.0	63.9	26.3	73.3	503	66 714	125.2	67.0	0.3	58.0	82.7	9.5	
20	13	0	10.00		00.515	133.5	00.5	1.7	03.5	20.1	11.2	501	00.711	125.5	07.0	0.3	50.0	0112		
		0																		
		0																		
		0																		
Pos	t-rain	moisture	e sample	501	66.632	166.1	124	57.368				503	66.716	164.5	133.6	66.884				
		Rain off	•			Rain gau	ges		1											
		27	min	45	51	10		46	42											
		12	sec	45		48	44	20 PIOT 2	42										_	
		70 Time (s)	1632 78		44															
		rine (3)	1052.70	Plot 1 Average	47	Plot 2 Av	42.5													
			Rainfall i	ntensity (mm/s)	0.029		0.026					Infiltrat	tion Ra	te Vei	r <mark>sus Ti</mark> i	ne				
			Rainfall i	ntensity (mm/h)	103.6		93.7			90.0										
										90.0										
				Moisture conte	Plot 1	Plot 2				80.0	$\sim$									
				Mass soil (g)	30.32	53.607			h/n	70.0										
				Mass water (g)	18.2	19			u	60.0								—		
			Before		60.0%	35.4%			ate	50.0										
				IVIASS SOII (g)	57.368	66.884				40.0										
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ENG4112 Research Project

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Runoff initiation (s)         Ob         Section (mass mass (g) (g)         Names (g)         Notation (mass (g)         Notation	
Image: series with the series withe series with the series with the series with the se	
Image: border         Bottle number         Bottle number         mass (g) mas	5
$ \begin{array}{                                    $	
The full modele supple         Duration         Solution         The full modele supple         Solution         The full modele supple         Solution         Solut	
Image of the local problem         Sector         Sector <td>_</td>	_
1       39       99       15.94       349       66.864       331.4       77.4       10.5       243.5       97.8       7.5       317       66.808       128       67.7       0.9       59.4       83.9         2       35       155       15.21       348       66.953       332.2       77.4       10.4       244.4       102.8       2.4       318       66.89       128       66.0       1.1       59.7       88.3         3       38       218       15.97       347       66.912       340.3       77.7       10.8       251.8       100.9       4.3       319       66.74       131.7       67.9       1.2       62.6       88.3         4       37       277       15.28       346       66.893       349.3       78.6       11.7       259.0       108.5       -3.2       320       66.937       135.4       68.2       1.3       65.9       97.1         6       37       397       15.66       345       66.903       336.2       77.6       10.7       247.9       101.3       3.9       321       66.875       129.6       67.9       1.0       60.7       87.2         8       36       <	Time f
2       35       155       15.21       348       66.953       332.2       77.4       10.4       244.4       102.8       2.4       318       66.89       128.8       68.0       1.1       59.7       88.3         3       38       218       15.97       347       66.912       340.3       77.7       10.8       251.8       100.9       4.3       319       66.74       131.7       67.9       1.2       62.6       88.3       97.1         6       37       397       15.66       346       66.93       349.3       78.6       11.7       259.0       108.5       -3.2       320       66.937       135.4       68.2       1.3       65.9       97.1         6       37       397       15.66       345       66.93       36.2       77.6       10.7       247.9       101.3       3.9       321       66.875       129.6       67.9       1.0       60.7       87.2         8       36       516       15.91       344       66.841       335.8       77.8       11.0       259.9       108.2       -30       323       66.874       128.9       68.0       1.1       59.8       87.5         15	4.0
3       38       218       15.97       347       66.912       340.3       77.7       10.8       251.8       100.9       4.3       319       66.74       131.7       67.9       1.2       62.6       88.3       97.1         4       37       277       15.28       346       66.893       349.3       78.6       11.7       259.0       108.5       -3.2       320       66.937       135.4       68.2       1.3       65.9       97.1       97.1         6       37       397       15.66       345       66.903       336.2       77.6       10.7       247.9       101.3       3.9       321       66.875       129.6       67.9       1.0       60.7       87.2         8       36       516       15.91       344       66.841       335.8       76.9       10.1       248.8       100.1       5.2       322       66.844       133.7       68.1       1.3       64.3       91.0       91.0       10.8       25.9       10.2       46.937       12.9       66.874       128.9       68.0       1.1       59.8       87.5       10.0       1.0       60.937       129.1       67.9       1.0       60.2       88.1       <	9.5
4       37       277       15.28       346       66.893       349.3       78.6       11.7       259.0       108.5       -3.2       320       66.937       135.4       68.2       1.3       65.9       97.1         6       37       397       15.66       345.6       66.903       336.2       77.6       10.7       247.9       101.3       3.9       321       66.875       129.6       67.9       1.0       60.7       87.2         8       36       516       15.91       3444       66.841       335.8       76.9       10.1       248.8       100.1       5.2       322       66.844       133.7       68.1       1.3       64.3       91.0       91.0         11       50       710       15.37       343       66.818       348.7       77.8       11.0       259.9       108.2       -3.0       323       66.874       128.9       68.0       1.1       59.8       87.5         15       33       933       15.38       342       66.87       345.9       77.4       10.6       257.9       107.3       2.1       324       66.97       129.1       67.9       1.0       60.2       88.1	9.6
63739715.6634664.03336.277.610.7247.9101.33.932166.875129.667.91.060.787.283651615.9134466.841335.876.910.1248.8100.15.232266.844133.768.11.364.391.091.0115071015.3734366.818348.777.811.0259.9108.2 $-3.0$ 32366.874128.968.01.159.887.5153393315.38343.934266.87345.977.410.6257.9107.3 $-2.1$ 32466.937129.167.91.060.288.110 $45.7$ 11.0259.9108.2 $-3.0$ 32366.874128.968.01.159.887.5115071015.3766.83344.777.811.0259.9107.3 $-2.1$ 32466.937129.167.91.060.288.1124515.0315.0334166.857337.677.410.6257.9107.3 $-2.1$ 32466.937129.167.91.060.288.1124215.0315.03341.066.857337.876.49.5251.9102.62.732666.98125.367.80.856.781.1124243 <td>).7</td>	).7
8       36       516       15.91       344       66.841       335.8       76.9       10.1       248.8       100.1       5.2       322       66.844       133.7       68.1       1.3       64.3       91.0         11       50       710       15.37       343       66.818       348.7       77.8       11.0       259.9       108.2       -3.0       323       66.874       128.9       68.0       1.1       59.8       87.5         15       33       933       15.38       342       66.837       345.9       77.4       10.6       257.9       107.3       -2.1       324       66.937       129.1       67.9       1.0       60.2       88.1         20       45       1245       15.03       341       66.856       337.6       78.4       11.5       247.7       105.5       -0.2       325       66.964       124.8       68.1       1.1       55.6       83.2         25       42       1542       15.72       340       66.95       337.8       76.4       9.5       251.9       102.6       2.7       326       66.98       125.3       67.8       0.8       56.7       81.1         25       4	).6
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	5.8
15       33       933       15.38       15.38       342       66.837       345.9       77.4       10.6       257.9       107.3       -2.1       324       66.937       129.1       67.9       1.0       60.2       88.1         20       45       1245       15.03       341       66.856       337.6       78.4       11.5       247.7       105.5       -0.2       325       66.964       124.8       68.1       1.1       55.6       83.2         25       42       1542       15.72       340       66.905       337.8       76.4       9.5       251.9       102.6       2.7       326       66.98       125.3       67.8       0.8       56.7       81.1         25       42       1542       15.72       340       66.905       337.8       76.4       9.5       251.9       102.6       2.7       326       66.98       125.3       67.8       0.8       56.7       81.1         40       0 <td>).3</td>	).3
20       45       1245       15.03       341       66.856       337.6       78.4       11.5       247.7       105.5       -0.2       325       66.964       124.8       68.1       1.1       55.6       83.2         25       42       1542       15.72       340       66.905       337.8       76.4       9.5       251.9       102.6       2.7       326       66.984       124.8       68.1       1.1       55.6       83.2         42       1542       15.72       340       66.905       337.8       76.4       9.5       251.9       102.6       2.7       326       66.984       124.8       68.1       1.1       55.6       83.2         4       0	).7
25       42       1542       15.72       340       66.905       337.8       76.4       9.5       251.9       102.6       2.7       326       66.98       125.3       67.8       0.8       56.7       81.1         0<	1.6
0       0	5.7
Post-rain moisture sample         339         66.899         172.7         155.5         88.601         327         66.728         140.6         123.8         57.072	
All Oli     Rail gauges       26 min     48	
13 sec 17 Plot 1 15 10 Plot 2 14	
38 msec     44     40	_
Time (s) 1573.38 Infiltration Rate Versus Time	
Plot 1 Average 46 Plot 2 Av 42.75	
Rainfall intensity (mm/s) 0.029 0.027	
Rainfall intensity (mm/h) 105.3 97.8	
15.0	
Moisture content Plot 1 Plot 2	
Mass soil (g) 50.216 37.342	
Mass water (g) 4.9 4.5	
Before MC 9.8% 12.1%	
Mass soil (g) 88.601 57.072	
Mass water (g) 17.2 16.8	
After MC 19.4% 29.4%	
-50	
Time since rainfall commencement (s)	
Sodosol #2 — Chromosol	1

																-			
Date	e	3-Jul-19	Plot 1				Sodosol	#1			Plot 2				Sodosol #	3			
PI	lot gradien	it .				20%									20%				
Runo	off initiatio	n (s)		1	I	45			Γ						45				
										Instantane							Instantan	Instantaneo	
									Instantaneo	ous			Wet				eous	us	
				Bottle	Wet	OD mass	Sediment	Runoff	us runoff	infiltration	Bottle	Bottle	mass	OD mass	Sediment	Runoff	runoff	infiltration	
			Bottle number	mass (g)	mass (g)	(g)	mass (g)	mass (g)	(mm/h)	(mm/h)	number	mass (g)	(g)	(g)	mass (g)	mass (g)	(mm/h)	(mm/h)	
Pre-rain	moisture	sample	328	66.842	125	120.1	53.258				701	68.8	141.5	135.3	66.5				
Time from	m stat	Duration																	
(min:sec	to sec)	(secs)																	Time
1 39	99	15.46	329	66.878	320.9	75.1	8.2	237.6	98.4	7.5	702	66.91	305.3	76.4	9.5	219.4	90.8	10.4	
2 39	159	15.56	330	66.82	333.3	76.0	9.2	248.1	102.1	3.8	703	66.818	329.7	77.7	10.9	241.1	99.2	2.0	
3 38	218	15.81	331	66.812	332.1	75.1	8.3	248.7	100.7	5.2	704	66.899	336.3	77.9	11.0	247.4	100.1	1.1	
4 38	278	15.44	332	66.832	353	76.9	10.1	266.0	110.3	-4.4	705	66.294	338.2	78.0	11.7	248.5	103.0	-1.8	
6 37	397	15.25	333	66.9	330.7	75.6	8.7	246.4	103.4	2.4	706	66.815	328.8	76.1	9.3	243.4	102.2	-0.9	
9 36	576	15.00	334	66.848	340	75.9	9.1	255.0	108.8	-3.0	707	66.795	332	76.3	9.5	246.2	105.0	-3.8	
12 48	768	15.25	335	66.85	324.2	74.9	8.1	241.3	101.2	4.6	708	66.826	315.4	75.3	8.5	231.6	97.2	4.0	
15 46	946	15.31	336	66.797	343.7	75.6	8.8	259.3	108.4	-2.5	709	68.834	332.4	79.5	10.7	242.2	101.3	0.0	
21 16	1276	15.13	337	66.843	333.7	74.9	8.1	250.7	106.1	-0.2	710	66.867	343.2	77.8	10.9	254.5	107.6	-6.4	
25 28	1528	15.88	338	66.902	346.6	75.6	8.7	262.3	105.7	0.1	711	68.82	353.0	79.7	10.9	262.4	105.8	-4.5	
Post-rair	moisture	samnla	502	66 779	179 1	160.3	93 521				712	68 767	15/1 3	1/1 1	72 333				
i ost i di	interstare	Sample	502	00.775	1/ 5.1	100.5	55.521				, 12	00.707	104.0	171.1	72.333				
	Rain off			Rain ga	IVES														
	25	min		48	]		47												
	56	sec	47	Plot 1	46	46	Plot 2	42											
	50	mene	1	1001		+0	1002	72			Inf	iltratio	n Rati	o Vorcu	is Timo				
	U Timo (s)	1556.06		42			40					intratio	mau						
	Time (S)	100.00	Blot 1 Average	1E 7E	Diot 2 Av	12 75			12.0										
			Pior I Average	45.75	PIUL Z AV	45.75			10.0 -										
			Painfall intensity (mm/b)	105.029		101.028			10.0	1									
			Rainian intensity (mm/m)	105.8		101.2			8.0 -									—	
			Moisture content	Dia± 1	Dict 2				<u>ج</u> 6.0 –										
									E				^						
			Iviass soli (g)	53.258	66.5				- 4.0 - e										
			iviass water (g)	4.9	6.2				2.0	$ \downarrow$ $\downarrow$	$- \wedge$		// _						
		Before		9.2%	9.3%														
			IVIASS SOIL (g)	93.521	/2.333				0.0 tral	200	200	600	800	1000	1200	1400	1600	1800	
			Mass water (g)	18.8	13.2				- Ē -2.0 -	200	100		000	1000	1200	1400	1000	1000	
		After	МС	20.1%	18.2%				-40-		V								
											•								
									-6.0									—	
									-8.0										
												Time s	ince rainfa	all commend	ement (s)				
									-			Plot 1 Sod	osol #1	-Plot 2	Sodosol #3				

## **Appendix D - Example ImageJ Script**

## Introduction

ImageJ was used to complete the image analysis of cores scanned by X-ray CT. To semi-automate the image analysis processes, a number of script files were developed. The script below is an example.

This script file identifies the soil surface through the use of the Gaussian blur function and creation of an EDM mask. Once the surface has been identified a for loop is used to measure the porosity of the image 40 times at incrementally increasing depths throughout the soil. This was used to determine the changes of porosity with depth.

The language used is an ImageJ scripting language.

```
Script
```

```
//
// Initialisation
// run("Close All")
print("New run");
```

// set the options to: // foreground = white // background = black run("Options...", "iterations=1 count=1 black edm=16-bit do=Nothing"); run("Colors...", "foreground=white background=black selection=yellow");

```
// Prepare for measurements
run("Clear Results");
run("Set Measurements...", "area bounding area_fraction redirect=None decimal=3");
roiManager("reset")
```

// open("C:/Users/Cameron/Documents/Engineering Degree - use this one/19 S1 ENG4111 Research Project/Created Images - ImageJ/Core2/Reslice of Core2.tif");

```
//image_directory = getInfo("image.directory");
//print(image_directory);
```

// number of times mask is applied at different depths below the soil surface n\_increments = 40;

```
orig_image = getTitle;
run("Invert");
run("Duplicate...", " ");
image = getTitle;
close(orig_image)
```

```
//
// create mask
```

C - 2

run("Invert"); setOption("BlackBackground", true); run("Make Binary"); binary\_image = getTitle(); // this binary image will have the mask applied to it once the mask is developed run("Duplicate...", " "); new\_binary\_image = getTitle(); // this image is used to develop the mask print(new\_binary\_image);

// Gaussian blur run to set up finding the soil surface
run("Gaussian Blur...", "sigma=10");

// threshold
// aim to get as a complete hole free soil as possible
setAutoThreshold("Default dark");
setOption("BlackBackground", true);
run("Convert to Mask");

// fill any remaining gaps
run("Fill Holes");

// Clean up any larger objects at the edge of the image // invert image to get holes at the edge as objects run("Invert");

// size restricted analyze particles with masks as output
run("Analyze Particles...", "size=50-Infinity show=Masks"); // change to 50 from 100 default

// reset the LUT and binary to: // foreground = white // background = black run("Invert LUT"); run("Invert");

mask\_image = getTitle();

// create EDM
run("Distance Map");
EDM\_map = getTitle();
print("EDM\_map original file name: " + EDM\_map);
// run("Duplicate...");
run("Duplicate...", " ");
EDM\_duplicate = getTitle();
print("Duplicate of EDM\_map filename: " + EDM\_duplicate);

// set initial values for threshold which are changed within the for loop
first\_point = 1;

```
second_point = 11;
lower_threshold = newArray(n_increments);
upper_threshold = newArray(n_increments);
for (i=1; i<=n_increments; i++) {
        lower_threshold[i-1] = first_point;
        first_point = first_point + 10;
        upper_threshold[i-1] = second_point;
        second_point = second_point + 10;
}
Array.print(lower_threshold);
Array.print(upper_threshold);
print("Loop begins - Loop begins - Loop begins");
//
// loop through binary image, apply threshold and record measurement
for (i=1; i<=n_increments; i++) {
        selectWindow(EDM_duplicate);
        run("Duplicate...", " ");
        // run("Duplicate..."); // this has fixed the problem but requires enter to be entered each time
```

// potentially use restore and back up to do the same as duplicate without having all the extra mak images created

// set a manual threshold
// fixed lower and upper threshold value
// i.e. size in pixels of submask

print("loop i value: " + i); print("first point: " + lower\_threshold[i-1]); print("second point: " + upper\_threshold[i-1]);

 $setThreshold(lower_threshold[i-1], upper_threshold[i-1]); \ensuremath{\,/\!/}\ changing \ values \ to \ apply threshold \ progressively \ deeper \ into \ the \ soil$ 

setOption("BlackBackground", true);
run("Convert to Mask");

active\_mask = getTitle(); print("Active mask " + active\_mask); // selectWindow(active\_mask);

// create selection based on mask, change to binary image and measure
run("Create Selection");
//roiManager("add");
selectWindow(binary\_image);

```
run("Restore Selection");
run("Measure");
// change threshold values for next iteration
first_point = first_point + 4;
second_point = second_point + 4;
// reset EDM_duplicate to original EDM
// EDM_duplicate = EDM_map;
resetThreshold;
```

```
}
```

selectWindow("Results");
run("Close All");

 $/\!/$  create a table to store the %Area data only in

// initially this is just for one column for one image. Need to be able to store results from other images
// in other columns so that the % Area results for each slice in a stack can be stored
// from this the average porosity with depth can be calculated.

// run("Close All")