

# Assessment of the effects of vegetational cover on the long-term stability of a waste rock dump

<http://dx.doi.org/10.1590/0370-44672018720176>

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## Abstract

Waste rock dump reclamation includes re-grading the banks to flatten the overall slope followed by re-vegetation. These measures are required to reduce surface erosion, provide physical stability, and meet future use goals. In some cases, the presence of trees can cause localized shear zones in a waste rock dump by the load of the tree mass and the action of wind, the presence of roots in the soil and the death of some species. In geotechnical assessment, however, the influence of trees on slope stability of waste rock dumps is only qualitative. This article assesses the effects of trees on slope stability of a reclaimed waste rock dump of an iron ore mine. Historic data of construction, laboratory tests, and stability analysis were evaluated. Slope stability assessments were performed after an extensive geotechnical and forest campaign, considering two scenarios - the final waste rock dump without the vegetational cover and the final waste rock with vegetational cover. Tree vegetation in slopes produces both positive and negative effects on slope stability. For scenario 1, the lowest factor of safety occurs for a potential failure surface near the bottom of the waste dump, passing by a layer 6 m depth from the surface. In scenario 2 the factor of safety is increased by 10%. For deeper failure surface, the smallest factor of safety occurs 30 m deep. However, very tall trees at the top of the waste dump are subject to wind action, which has a negative impact on slope stability.

**keywords:** waste rock dump, slope stability, trees, wind force, mine closure.

## 1. Introduction

Waste rock is the unprocessed overburden material that is excavated and disposed of in order to access valuable ore bodies. In the median and large Brazilian iron ore mines, waste rock dumps represent large structures and, therefore, their construction, reclamation, and closure process have to be managed following the

best practices available and the legal requirements. The practice shows that even after the closure is complete, waste rock dumps are subject to long-term changes due to the invasion of tree species, the growth of existing ones, soil densification, translocation of nutrients, etc (Zang *et al.* 2015). The Brazilian Norm NBR 13029

(ABNT, 2017) describes that an executive design of a waste rock dump must contain all the geometric characteristics, ranging from the design of the internal and surface drainage to the final protection of the berms and the landscape finish. However, the stability assessment of a dump is based on data obtained during the preliminary

studies with freshly-mined waste rock. Several hypotheses of rupture are evaluated for different situations and under different hydrogeological conditions. Geotechnical stability of a waste rock dump is a major concern and must be ensured for long-term (Aragão and Filho, 2008; Hudson-Edwards *et al.*, 2011).

Construction process of a waste rock dump occurs in the upward direction (ABNT, 2017). And, after finalizing the first banks in their final geometric configuration, the vegetation process

should take place. In general, the vegetation process is conducted, bank by bank at distinct times, by different people, with a distinct concept and in parallel, invasion of neighbouring species can occur, resulting in a quite heterogeneous reforestation along the waste dump.

In general, the overall geotechnical stability on a waste rock dump is assessed based upon the final geometric configuration planned and the geotechnical parameters of the different types of waste rock that compose the dump.

However, the stability assessment considering the final vegetation cover is, usually, not performed. This article presents the geotechnical stability assessment, conducted on a 20-year old reclaimed waste rock dump (Figure 1). The dump is 190 m in height, contains about 50 Mm<sup>3</sup> of waste rock and occupies an area large than 270 ha. Historic data of construction, laboratory tests, and stability analysis were evaluated, as well as a more detailed characterization of the vegetation.



Figure 1  
The waste rock dump during the construction phase (a) and current (b).

## 2. Materials and methods

### 2.1 Geotechnical and forest characterization of the waste dump

The waste rock dump object of this study has no as-built information. Therefore, in order to know the geotechnical, drainage and vegetation characteristics,

a comprehensive investigation was conducted, including geophysical, installation of instruments, boreholes tests, and the evaluation of arboreal behaviour,

from the aerial part to the rooting system. Figure 2 shows the drilling and the geotechnical instrument locations.

Although the top of the dump was

composed of waste rock, a very soft to a compact layer of non-plastic soil was identified in the first 6 meters depth. This layer is highlighted in this study because it is the place where the root system develops.

No internal drainage structures

were identified in the boreholes tests. The surface drainage system is composed of horizontal ditches along the berms, discharging in a peripheral ditch, which directs surface waters towards a sediment containment basin.

The geophysical investigations

conducted on three cell counts suggest a preferential flow path for a tailings pond located above the waste rock dump, which functions as a large water box, reloaded every rainy season. The water flowing into the dump is traceable near the bottom of the waste dump.

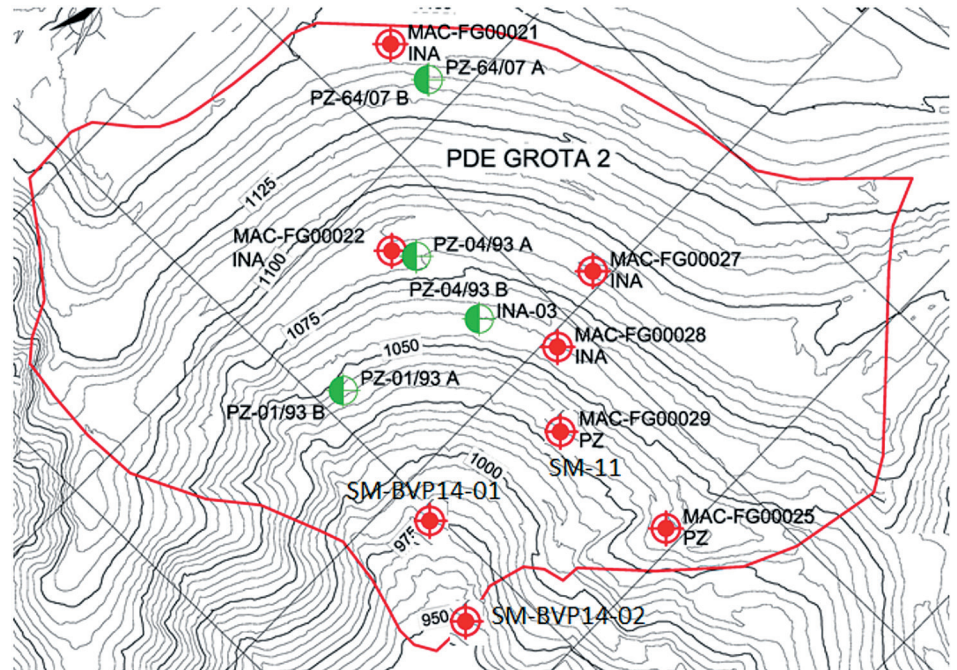


Figure 2  
Drilling and the geotechnical instruments locations on the waste rock dump.

Disturbed and non-disturbed soil samples were taken in trenches and later sent to the UFOP laboratories, in order to conduct soil fertility analysis and

geotechnical characterization. In order to obtain the geotechnical parameters of the surface layer, large-scale shear tests were conducted (Figure 3), as indicated

by Paria *et al.* (2017) for granular waste rock. This experiment differs from conventional by the box of 20 cm wide and 5 cm high.



Figure 3  
Large direct shear test equipment - UFOP Geotechnical Laboratory.

In general terms, the waste rock dump vegetation is divided into three areas, which generates the basis for consideration in the geotechnical assessment. The waste rock dump's upper area presents an approximate average canopy of 5 m, with a predominant vegetative cover of native species with the low part well closed, formed by undergrowth and

shrub with the areas of clearances occupied by exotic grasses. Exotic grasses with isolated shrubs and trees form the central area, approximately 50 m in length. The lower part vegetation is predominantly of Eucalyptus, with an average canopy of 20 m with a well-formed sub-forest of native species naturally regenerated.

To understand the root system, trenches 2 m in depth and 1 m in width, staged in a window, were opened on the face of the slope (Figure 4). The trenches were opened just in front of the tree's trunk, with the purpose of assessing the substrate layers, the behavior of the root system and the development of the roots in different layers.



Figure 4  
The trench for assessing substrate layers and the roots system.

## 2.2 Parameter characterization

Geotechnical and “vegetation” parameters resulted of investigations and tests conducted were adopted to support the numerical model. Table

1 presents the geotechnical data. The superficial layer parameters were determined by geotechnical characterization conducted in the undisturbed samples.

Additional boreholes tests, laboratory analysis, and correlation with SPT tests for deeper layers are described in VALE (2011).

Table 1  
Geotechnical Parameters of the Waste Rock Dump

Waste dump area	Upper	Middle	Botton	Comments
Average height of trees (m)	5.0	___	20.0	
Wind speed (m/s)	30.0	___	30.0	Basic velocity isopleths from the map of Quadrilátero Ferrífero region, MG.
Topographic Factor (S1)	1.0	___	0.9	1 for trees at the top and cosine (30 °)
Roughness factor (S2)	0.8	___	1.0	Category III = flat or wavy terrain with obstacles, such as hedges and walls, few tree-fringes, low and sparse buildings. (Ex: farms and country houses (except parts with weeds), farms with hedges and / or walls, suburbs at considerable distance from the center, with low and sparse houses). The average level of the top of the obstacles is considered equal to 3,0 m;
				Class C = group of trees for which the horizontal or vertical dimension of the frontal surface exceeds 50 m
Statistical factor (S3)	1.0	___	1.0	With significant environmental impact, restricted to the limit of the enterprise or locality, affecting anthropic and/or natural areas that requires project to mitigate and repair adverse effects to the environment.
Characteristic speed	23.4	___	23.8	Calculated $V_k = VS1S2S3$
Density of the barrier	0.7	___	0.4	
Effective area (Ae)	3.5	___	8.0	Expected values according to medium canopy
Drag coefficient (Ca)	0.5	___	0.5	According to graph related to the effective area
Dynamic pressure (q)	34.1	___	35.4	Calculated $q = V_k^2/16$
Wind force for trees at the top of the dump - Fve (kN/m <sup>2</sup> )	0.5	___	1.4	Calculated $Fve = q.A.Ca$
Vertical pressure (kN/m <sup>2</sup> )	3.0	0.0	5.0	Values between 2 and 5kPa, according to bibliography
Additional cohesion of the roots - C' r (kPa)	3.3	2.0	5.0	Estimated according to bibliography
Depth of roots (m)	1.0	0.5	2.0	Used 10% of the height of the tree, according to investigations

Tree influence considered in this study include the tree's load in each area of the waste rock dump, the depth of the rooting system and its contribution for

increasing the soil cohesion, as well as the wind force, horizontally applied, in the direction of the potential landslide. To calculate the wind strength ( $F_{ve}$ ), the au-

thors adopted the Brazilian Norm - NBR 6123 (ABNT, 1988) for geotechnical purposes. According to NBR 6123, the wind strength can be calculated by Equation 1.

$$F_{ve} = q \times A_e \times C_a \quad (1)$$

Where:  $q$  = Dynamic pressure in ( $N/m^2$ );  $A_e$  = effective area in  $m^2$ ; and  $C_a$  = drag coefficient

The vegetational vertical permeability acts as a barrier of wind and transmits the load to the soil. Topographical and roughness parameters as well as the statistical factors were established considering the characteristics of the surrounding areas.

Dense vegetation with an average height of 5 m covers the waste rock dump surface. At the top of the dump, there is an estimated 70% of the vertical area, followed by a low growing vegetation in the middle, and on the bottom, eucalyptus trees (about 20 m high) with a permeability of 40% considering the riparian woods. On the upper part of the waste

rock dump the vegetation distribution does not work as a windbreaker to the lower part of the dump.

A load of 3 kPa for the native vegetation area and 5 kPa for the eucalyptus area were considered based on studies of Wolle (1986); Kozciak (2005); and Fiori and Carmignani (2009). These authors consider the variation of 2 to 5.2 kPa, between capoeira and forests. A vertical load was not considered for the intermediary area.

Additional cohesions of 3.3 kPa for the native vegetation established on surface pebble soil to soils under varied forests of evergreen leaves; 4 kPa for the low

growing vegetation area, and 5 kPa for eucalyptus trees with the riparian woods areas were considered according to Norris and Greenwood (2006). These additional cohesions were assumed to the depths of 1 m, 0.5 m, and 2 m, respectively, according to field observations. Table 2 presents the wind force calculation, the parameters and associated factors as well as the comments according to the adapted NBR 6123 (ABNT, 1988) for the upper, middle and bottom areas, respectively.

For the numerical analysis, the Morgenstern & Price method was applied using SLIDE 7.0 software.

Layers	gnat / gsat	$c'$	$\phi'$
	( $kN/m^3$ )	( $kPa$ )	( $^\circ$ )
surface layer of the dump	19/20	1	30
Sterile	20/21	13	34
Waste rock	22/23	2	38
Embankment SPT $\leq 4$	18/19	6	23
Embankment SPT 23 to 27	18/19	19	35
silt	18/19	10	20
Residual shale soil and xisto	18/19	30	25
Sericite shale R0 (Saprolite)	18/19	40	28
Sericite shale R1_R2	19/20	60	30
Sericite shale R3	20/21	100	32

Table 2  
Geotechnical parameters used for analysis.

### 3. Results and discussions

A stability analysis was conducted on the longitudinal section of the waste rock dump since this section is considered the most critical due to the overall slope angle and the section position in relation to the dump shoulders'. Longitudinal section of the waste rock dump resulted in a profile with approximately 300 m of horizontal projection and an overall slope angle of  $30^\circ$ . Figure 5 presents the profile of the analysed dump considering the water table according to piezometers and water level indicator data; the loads and the material characteristics described in Table 2; and the geotechnical parameters of the waste rock.

The vegetational cover present today on the waste rock dump is 20 years old. To evaluate the influence of the vegetational layer, 2 different scenarios were considered in the computational models. Scenario 1 represents the final waste rock dump without the vegetational cover, and scenario 2 is the final waste rock with vegetational cover.

Figure 6 shows the results for the 2 scenarios. For scenario 1, the lowest factor of safety occurs for a potential failure surface near the bottom of the waste dump, passing over a layer 6 m in depth below the surface in contact with the two different layers of the waste rock, the "surface" and the "sterile"

layer, respectively. Such surface of rupture can be observed in the field from small mass movements occurring over time.

For the final waste rock dump, the factor of safety presented in the analysis is at the limit where the shear strength to resist failure is equal to the driving force to initiate failure. In the current configuration (scenario 2), the factor of safety is increased by 10%, corroborating the studies of Coppin & Richards (1990) and Perry *et al.* (2003). In the field, one can also observe that with the development of the vegetation, the layer stability increased and the mass movements reduced.

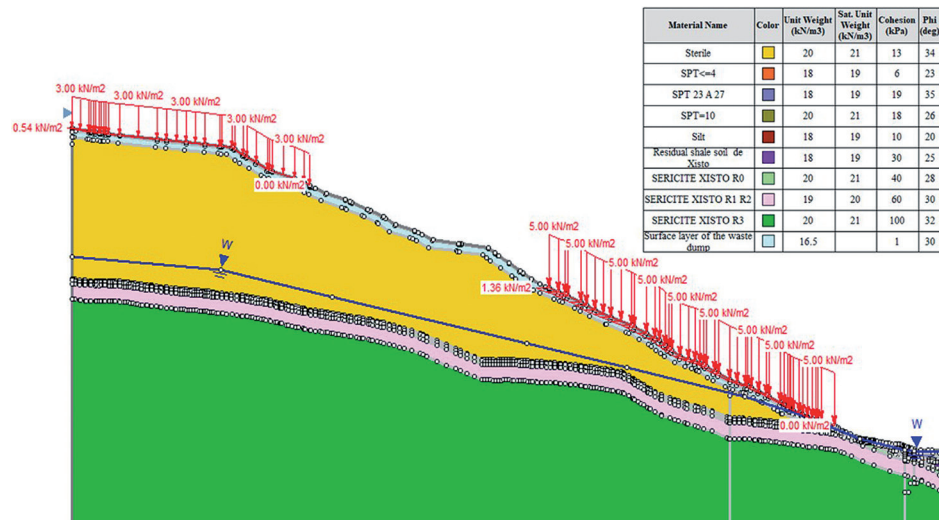


Figure 5 - The waste rock dump section for numerical analysis.

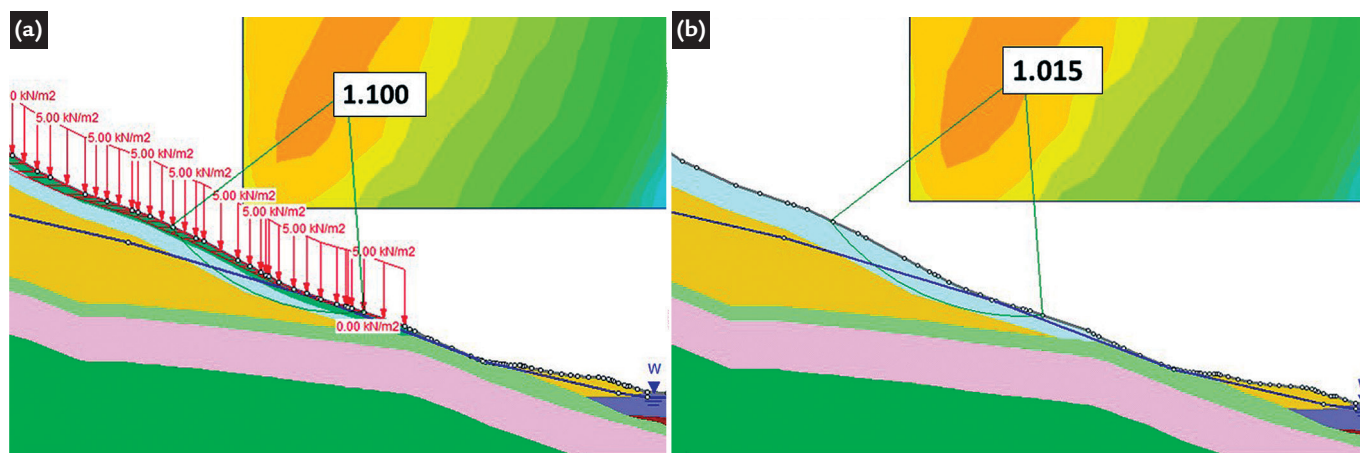


Figure 6 - Local Analysis – the base of the waste rock dump - Scenarios 1 (a) and 2 (b).

By forcing the potential failure surface deeper, the smallest factor of safety

occurs in the contact with the sericite xisto layer, 30 m deep. However, for this

potential mass, the vegetation no longer has any influence as shown in Figure 7.

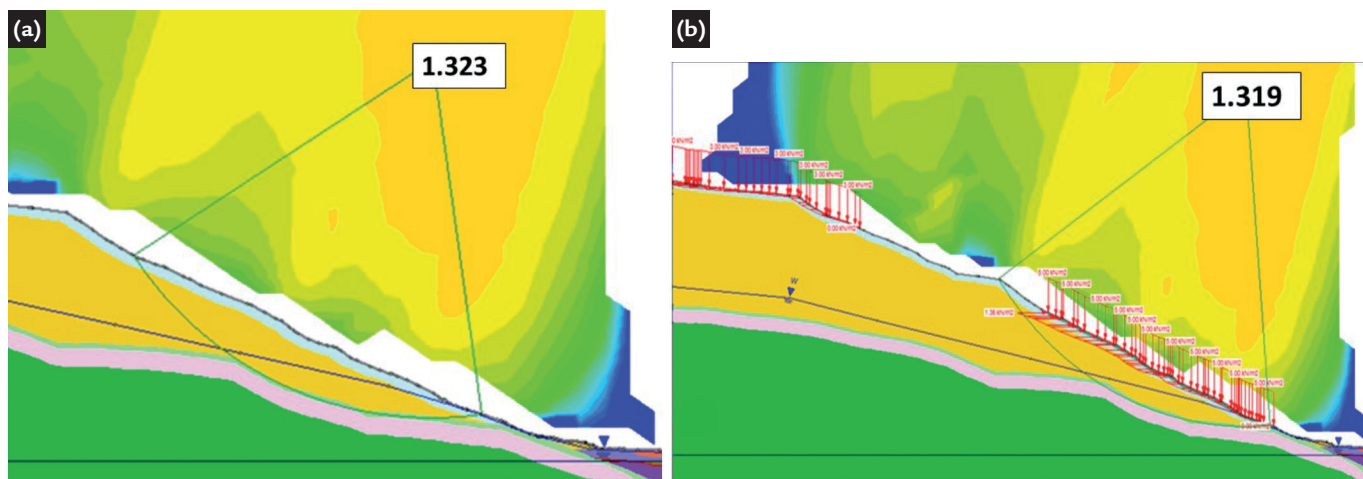


Figure 7 - Local analysis - the base of the waste rock dump - Scenes 1 (a) and 2 (b) with deep failure surface.

The top of the waste rock dump was also locally evaluated for both scenarios.

In this case, vegetation development generated a small negative impact, below

1%, on the factor of safety as shown in Figure 8.



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Received: 19 November 2018 - Accepted: 23 July 2019.

