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- Two stage BSDP is developed
- Considering varying levels of forecast uncertainty improves performance
- Benefit reduction from use of longer inflow forecasts is mitigated

Correspondence to:

C. Zhang,
czhang@dlut.edu.cn

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A two stage Bayesian stochastic optimization model for cascaded hydropower systems considering varying uncertainty of flow forecasts

Wei Xu^{1,2}, Chi Zhang¹, Yong Peng¹, Guangtao Fu³, and Huicheng Zhou¹

¹School of Hydraulic Engineering, Dalian University of Technology, Dalian, China, ²College of River and Ocean Engineering, Chongqing Jiaotong University, Chongqing, China, ³Centre for Water Systems, College of Engineering, Mathematics, and Physical Sciences, University of Exeter, Exeter, UK

Abstract This paper presents a new Two Stage Bayesian Stochastic Dynamic Programming (TS-BSDP) model for real time operation of cascaded hydropower systems to handle varying uncertainty of inflow forecasts from Quantitative Precipitation Forecasts. In this model, the inflow forecasts are considered as having increasing uncertainty with extending lead time, thus the forecast horizon is divided into two periods: the inflows in the first period are assumed to be accurate, and the inflows in the second period assumed to be of high uncertainty. Two operation strategies are developed to derive hydropower operation policies for the first and the entire forecast horizon using TS-BSDP. In this paper, the newly developed model is tested on China's Hun River cascade hydropower system and is compared with three popular stochastic dynamic programming models. Comparative results show that the TS-BSDP model exhibits significantly improved system performance in terms of power generation and system reliability due to its explicit and effective utilization of varying degrees of inflow forecast uncertainty. The results also show that the decision strategies should be determined considering the magnitude of uncertainty in inflow forecasts. Further, this study confirms the previous finding that the benefit in hydropower generation gained from the use of a longer horizon of inflow forecasts is diminished due to higher uncertainty and further reveals that the benefit reduction can be substantially mitigated through explicit consideration of varying magnitudes of forecast uncertainties in the decision-making process.

1. Introduction

With recent improvements in weather forecasting technologies, there is increasing attention on the use of weather forecasts to improve reservoir operation [e.g., Niewiadomska-Szynkiewicz *et al.*, 1996; Chang *et al.*, 2005; Hsu and Wei, 2007; Ngo *et al.*, 2007; Saavedra Valeriano *et al.*, 2010a, 2010b]. Weather forecasts are generally available at different time scales, for example, nowcasts (0–6 h), short range forecasts (1–3 days), and medium range forecasts (3–10 days), which are related to different degrees of predictive accuracy. Short range Quantitative Precipitation Forecasts (QPFs) have long been used for water system management, for example, real time flood control [e.g., Niewiadomska-Szynkiewicz *et al.*, 1996; Hsu and Wei, 2007; Ngo *et al.*, 2007; Saavedra Valeriano *et al.*, 2010a, 2010b] and short term hydroelectric system operation [Zhou *et al.*, 2005; Wang *et al.*, 2005]. Although medium range QPFs have a reduced accuracy and reliability compared to short range QPFs, they have been proven to be useful for reservoir operation [Collischonn *et al.*, 2005, 2007; Bravo *et al.*, 2009; Tang *et al.*, 2010]. For example, medium range QPFs have been used to forecast reservoir inflow for hydroelectric system operation [e.g., Zhou *et al.*, 2005; Wang *et al.*, 2005; Collischonn *et al.*, 2007; Zhou *et al.*, 2009, 2010; Tang *et al.*, 2010] and interbasin water transfer operation [e.g., Liang *et al.*, 2009; Wang *et al.*, 2010; Xi *et al.*, 2010]. At the same time, the limitations of use of these forecasts have been highlighted due to high levels of predictive uncertainty [Meneguzzo *et al.*, 2004; Bartholmes and Todini, 2005]. Uncertainty in QPFs is a major factor affecting the accuracy of inflow forecasts and thus the development of reservoir operating policies [Roulin and Vannitsem, 2005; Mascaro *et al.*, 2010; Saavedra Valeriano *et al.*, 2010b]. Thus, effectively addressing uncertainties in QPFs is critical to support informed decision making of reservoir operation [Cui *et al.*, 2011; Fan and van den Dool, 2011]. This study will investigate the explicit use of varying magnitudes of uncertainty in QPFs to improve the performance of cascaded hydropower systems, which, to the best of our knowledge, has not been attempted in the literature.

Stochastic Dynamic Programming (SDP) has been used to develop reservoir operation policies with the consideration of inflow uncertainties. *Dutta and Houck* [1984] and *Dutta and Burges* [1984] assumed that the distribution of forecast uncertainties remains unchanged during the entire forecast horizon. The efficiency and reliability of hydropower operation are substantially affected by the assumption of forecast uncertainties [*Tejada-Guibert et al.*, 1995]. In order to reduce the negative influence on decision making caused by high inflow uncertainty in a long forecast horizon (FH), *Stedinger et al.* [1984] suggested to employ only the best inflow forecasts within the beginning period of FH and discard the inflow forecasts in the remaining periods of FH with the intent of deriving improved reservoir operating policies. Recent research demonstrated that uncertainty in QPFs and inflow forecasts generally increases with an extending lead time [*Simonovic and Burn*, 1989; *Maurer and Lettenmaier*, 2003, 2004; *You and Cai*, 2008; *Zhao et al.*, 2012]. To effectively quantify inflow forecast uncertainty, *Karamouz* [1990] suggested the use of Bayesian decision theory to interpret inflow uncertainty by incorporating new information associated with inflow probabilities. Further, a Bayesian Stochastic Dynamic Programming (BSDP) model was developed by embedding Bayesian theory in SDP to represent inflow uncertainty [*Karamouz and Vasiliadis*, 1992]. *Kim and Palmer* [1997] used a BSDP model to investigate the potential benefits of using seasonal flow forecasts in hydropower generation. *Mujumdar and Nirmala* [2007] aggregated individual inflows of reservoirs in a cascaded hydropower system, and represented the forecast uncertainty of the aggregated inflow using a BSDP model. *Zhou et al.* [2009] and *Tang et al.* [2010] proposed a hybrid method that employs a SDP model for dry seasons and BSDP model for wet seasons. On one hand, increasing FH may provide more information for decision making in a longer time framework, on the other hand the benefit gained from a longer horizon may be offset by increasing forecast uncertainty [*You and Cai*, 2008; *Zhao et al.*, 2012]. However, in the BSDP models mentioned above, inflow forecast is treated as having the same level of uncertainty over the entire FH and the varying magnitudes of uncertainty are not explicitly considered for the development of reservoir operations.

The primary purpose of this paper is to present a new Two Stage Bayesian Stochastic Dynamic Programming (TS-BSDP) methodology for real time operation of cascaded hydropower systems. The newly proposed methodology in this study consists of a new Bayesian method which uses inflow forecasts for real-time reservoir operation in a more effective way, and also an aggregation-disaggregation approach that allows the proposed method to be implemented for a complex hydropower system in a more efficient way. To utilize the varying magnitudes of uncertainty in inflow forecasts with a medium-range forecast horizon, this methodology partitions the forecast horizon into two periods and consequently two different strategies are developed to incorporate the inflow uncertainties of the two periods through Bayesian theory for reservoir operation. The two strategies are designed to investigate the best way to utilize forecast inflows with different magnitudes of uncertainty in the two periods; they also allow for comparison of the impacts of different lengths of decision horizon on system performance regarding hydropower generation and reliability. Further, an aggregation-disaggregation approach is used to reduce the dimensional complexity of cascaded reservoir operation problems. The newly developed methodology is demonstrated using the Hun River cascaded hydropower reservoirs in China.

2. Two Stage Bayesian Stochastic Optimization Model

For multireservoir systems, the SDP and BSDP models become computationally intractable due to the curse of dimensionality problem, thus simplification is usually needed. An aggregation approach has been used to improve computational efficiency, in which a multireservoir system with cascaded hydropower stations is simplified as a single virtual reservoir through aggregation of the storage and inflow of individual reservoirs [*Turgeon*, 1980; *Turgeon and Charbonneau*, 1998; *Valdés et al.*, 1992; *Liu et al.*, 2009]. *Hall and Dracup* [1970] originally aggregated the storages of all reservoirs in a hydropower system into an equivalent reservoir to solve the dimensionality problem of deterministic dynamic programming, and optimal policies for the aggregated reservoir were decomposed into individual policies for each reservoir through the prescribed relationships between the reservoirs. An energy-based approach was later developed for decomposition by converting the inflows and storages of individual reservoirs to the potential energy available, which can be taken as a state variable to handle the nonlinear relationship between the volume of water release and the power generated [*Turgeon*, 1980; *Turgeon and Charbonneau*, 1998; *Liu et al.*, 2011]. However, the energy approach did not consider the individual characteristics of the reservoirs and the amount of

water to be released from the aggregated reservoir [Archibald *et al.*, 2006]. An alternative approach was provided to construct the relationship between the state variables of individual reservoirs using historical data [Saad *et al.*, 1994; Ponnambalam and Adams, 1996; Archibald *et al.*, 1997, 2006; Serrat-Capdevila and Valdés, 2007]. In the optimization process, the aggregate variables are disaggregated to estimate the optimal objective value according to the relationship. In this study, the conditional expectation method, developed by Loucks *et al.* [1981], is selected as the disaggregation model. The conditional expectation method has been successfully applied to disaggregate the aggregate inflow for cascaded hydropower reservoirs by Mujumdar and Nirmala [2007].

Building on the aggregation-disaggregation concept described above, the TS-BSDP model is developed in this study. This model is compared with the following three models: the Aggregate Flow Stochastic Dynamic Programming (AF-SDP), the Aggregation-Disaggregation Stochastic Dynamic Programming (AD-SDP) and the Aggregation-Disaggregation Bayesian Stochastic Dynamic Programming (AD-BSDP). The AF-SDP considers only the aggregation of individual reservoir inflows, and a single equivalent reservoir is not constructed. The other models consider the aggregation of both inflows and storages, and simplify the hydropower system to a single virtual equivalent reservoir for optimization. The aggregate storage and inflow are decomposed into values for each reservoir based on the storage-discharge relationships derived using historical data.

In this section, the aggregation-disaggregation approach is first introduced to construct a virtual equivalent reservoir. The uncertainties of the aggregate inflow are then addressed using Bayesian theory. Finally the structure and recursive equations of the TS-BSDP model are introduced.

2.1. Aggregation-Disaggregation Approach

2.1.1. Construction of an Equivalent Reservoir

The single equivalent reservoir is constructed by aggregating the inflows and storages of individual reservoirs to reduce the complexity of the optimal reservoir operation problem. The aggregate inflow is used as a state variable, and is calculated as $Q_t = \sum_{z=1}^Z q_t^z$, where q_t^z is the inflow of reservoir z at time step t and Z is the total number of reservoirs. The aggregate inflow is discretized into a number of adjoining intervals, each of which is represented by an element in the set $\{Q_t^1, \dots, Q_t^\mu\}$, where μ is the total number of discretization levels.

According to Mujumdar and Nirmala [2007], the AF-SDP model aggregates only the inflows of the individual reservoirs. The storages of individual reservoirs are discretized into a number of adjoining intervals, represented by the elements in the set $\{v_1^z, \dots, v_{\beta^z}^z\}$, where β^z is the number of storage intervals for reservoir z . The number of possible combinations of storage intervals of individual reservoirs is $\beta^1 \times \dots \times \beta^z$. Thus the AF-SDP model can result in the curse of dimensionality problem when the number of reservoirs or/and number of storage discretizations are large.

Apart from AF-SDP, all other models used in this study consider aggregation of the storages of individual reservoirs. The minimum and maximum reservoir storages of the equivalent reservoir are represented by

$$\begin{cases} V_{\max} = \sum_{z=1}^Z v_{\max}^z \\ V_{\min} = \sum_{z=1}^Z v_{\min}^z \end{cases} \quad (1)$$

where v_{\min}^z and v_{\max}^z represent the minimum and maximum storages of reservoir z , respectively. The equivalent reservoir storage is discretized into adjoining intervals, which are represented by $\{V_1, \dots, V_\phi\}$, where the subscript ϕ is the number of storage discretizations.

2.1.2. Disaggregation of Aggregate Variables

Once the optimal problem is solved based on the equivalent reservoir, the aggregate variables (inflow and storage) have to be disaggregated to the relevant variables of each individual reservoir. The conditional expectation method is used for disaggregation in this study [Loucks *et al.*, 1981; Mujumdar and Nirmala 2007]. When aggregate variable R_t at time step t belongs to interval r , the conditional expectation of the related variable for reservoir z (denoted by R_t^z) is calculated as

$$R_t^z = f(R_t^z | R_t) = E[R_t^z | R_t = r] = \sum_{iz=1}^{n_r} P[R_t^z = iz | R_t = r] \cdot nr_{iz}^t = \sum_{iz=1}^{n_r} P_{riz}^t \cdot nr_{iz}^t \tag{2}$$

where iz represents an interval of R_t^z for reservoir z ; n_r is the number of intervals (iz) at time step t ; nr_{iz}^t is the expected value of R_t^z in interval iz ; P_{riz}^t represents the probability that individual variable (R_t^z) at time step t belongs to the interval iz , given the aggregate variable (R_t) at time step t belongs to interval r .

The inflow of each individual reservoir at time step t , represented using q_t^z ($z=1, \dots, Z$), can be calculated by replacing the variables in equation (2) with relevant flow variables. Similarly, the storages at the beginning and end of time step t , i.e., s_{t-1}^z and s_t^z ($z=1, \dots, Z$), can also be calculated.

With the equivalent reservoir, the optimization problem is solved by taking the aggregated inflow and storage as state variables. In developing the operation policies of the equivalent reservoir, the decision variables are converted to the aggregate output. The aggregate output is then disaggregated with the objectives of minimizing the spillages and maximizing the amount of water at the end of the operation period as below:

$$F_t(h_t) = \max\{f_{endlev}(h_t^d | s_{t-1}, q_t, H_t) - \eta f_{spill}(h_t^d | s_{t-1}, q_t, H_t)\} \quad d \in D \tag{3}$$

where $f_{endlev}(h_t^d | s_{t-1}, q_t, H_t)$ and $f_{spill}(h_t^d | s_{t-1}, q_t, H_t)$ are the functions of storage at the end of time step t and spillage at time step t , respectively. H_t is the aggregate output obtained according to operation policies. h_t^d is the d th output combination of individual reservoirs by disaggregating H_t at time step t . D is the total number of combinations. h_t is the output combination, which is selected for operation. s_{t-1} is the storage combination of individual reservoirs at beginning of time step t , and q_t is the inflow of individual reservoirs at time step t . η is a penalty factor, typically in the interval [1, 10], and is used to direct the search to reduce the overall spillage from all reservoirs [Tang et al., 2010].

2.2. Uncertainties of Aggregate Inflow

In a simple SDP model, the reservoir inflow is normally represented using a simple Markov process. The randomness of the inflow at time step $t+1$ is addressed through a prior inflow transition probability $P[Q_{t+1}|Q_t]$, given inflow Q_t at time step t .

In a BSDP model, the inflow forecast uncertainty at time step t is represented in equation (4) using the posterior inflow transition probability $P[Q_t|F_t, Q_{t-1}]$, given the observed inflow (Q_{t-1}) at time step $t-1$ and inflow forecast (F_t) at time step t . The predictive probability of forecasts is represented by $P[F_{t+1}|Q_t]$, as described in equation (5) below.

$$P[Q_t|F_t, Q_{t-1}] = \frac{P[F_t|Q_t] \times P[Q_t|Q_{t-1}]}{\sum_{Q_t} P[F_t|Q_t] \times P[Q_t|Q_{t-1}]} \tag{4}$$

$$P[F_{t+1}|Q_t] = \sum_{Q_{t+1}} P[F_{t+1}|Q_{t+1}] \times P[Q_{t+1}|Q_t] \tag{5}$$

2.3. Optimization Problem Formulation

2.3.1. Objective Function

The operation policy in this study is to maximize the total power production as well as to minimize the deviation from the required system output to guarantee the stability of power supply. Thus, the objective function consists of two components: power production and penalty for deviation from system requirements as below

$$f_t(s_{t-1}, q_t) = \max\left\{\sum_{t=1}^T E[B(s_{t-1}, q_t, s_t)]\right\} \quad \forall s_{t-1}, q_t, s_t \tag{6}$$

where $B(s_{t-1}, q_t, s_t) = \left\{b(s_{t-1}, q_t, s_t) - \alpha[\max(e - b(s_{t-1}, q_t, s_t), 0)]^\beta\right\} \cdot \Delta t$

where $f_t(\cdot)$ represents the maximum expected hydropower generation for the entire planning horizon (T) and $t=1, \dots, T$. $b(\cdot)$ (MW) is a function of power generation given storage variables s_{t-1} and s_t at the beginning and end of time step t and flow variable q_t at time step t , and e is the required firm output, which is 76 MW for the case study. α and β are penalty factors; Δt (h) is decision interval.

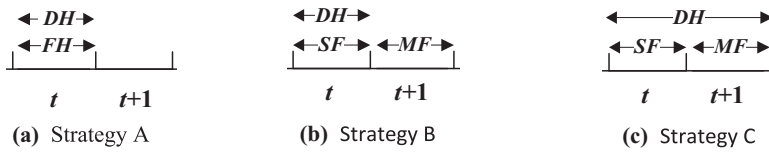


Figure 1. Three different decision-making strategies used by the four models. DH and FH represent decision horizon and forecast horizon, respectively.

2.3.2. State and Decision Variables

In this study, there are four models with different variables considered. In AF-SDP, the aggregate inflow at time step t and storage combination of individual reservoirs at the beginning of time step t are taken as state variables, and the storage combination of individual reservoirs at end of time step t is taken as a decision variable. In AD-SDP, the aggregate inflow at time step t and aggregate storage of the equivalent reservoir at the beginning of time step t are taken as state variables, and the aggregate storage at the end of time step t is taken as decision variable. In addition to the variables in AD-SDP, AD-BSDP considers the aggregate inflow at the previous time step $t-1$ as an additional state variable. While in TS-BSDP, the aggregate inflow at time step t is partitioned into two periods. Thus the state variables are aggregate inflows of two periods and aggregate storages at the beginning of time step t , and the decision variable is aggregate storage at the end of time step t .

2.4. Decision Strategies and Recursive Equations

2.4.1. Decision Strategies

It is important to determine the decision horizon in terms of the FH in the reservoir operation optimization problems. Figure 1a describes a general decision-making strategy in which the inflow forecast in the entire FH is used to determine the operation policies in the same period. This strategy is defined as Strategy A. This strategy is employed by AF-SDP, AD-SDP and AD-BSDP models in this study. In this way, the inflow forecast uncertainties in the entire horizon are treated identically. The longest FH available in this study is 10 days. Two cases with different time steps are used for comparison of the different models, i.e., $t = 5$ or 10 days.

In TS-BSDP model, the entire inflow FH (10 days) is partitioned into two periods: Short-range Forecast (SF, 1–5 days) and Medium-range Forecast (MF, 6–10 days). The time step t is 5 days in this model and two decision-making strategies, shown in Figures 1b and 1c, are developed to compare the impacts of different lengths of decision horizon on system performance. Figure 1b shows Strategy B, which uses the inflow forecasts at time steps t and $t+1$ to determine the operation policies for time step t . Figure 1c shows Strategy C, which uses the inflow forecasts at time steps t and $t+1$ to determine the operation policies for time steps t and $t+1$.

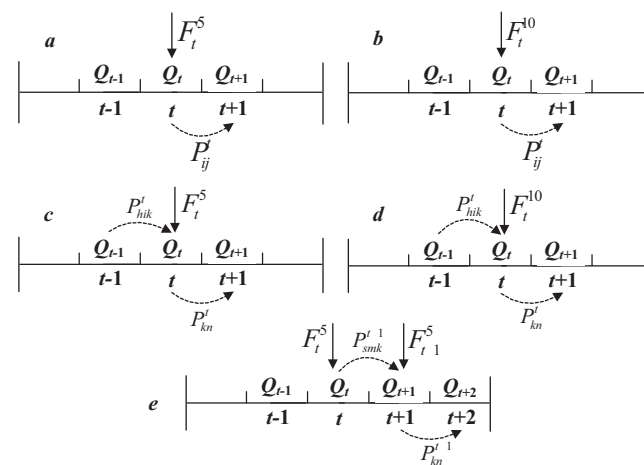


Figure 2. Representation of inflow uncertainties of various models. (a) and (b) SDP; (c) and (d) BSDP; and (e) TS-BSDP.

2.4.2. AF-SDP Model Recursive Equation

The AF-SDP model takes the storages of individual reservoirs and the aggregate inflow as state variables. In this model, the randomness of aggregate inflow for forecast horizons of 5 days and 10 days are depicted in Figures 2a and 2b respectively. The randomness of inflow is addressed by the prior inflow transition probability $P[Q_{t+1}|F_t]$ (denoted by P_{ij}^t), where the subscript i represents the i th interval of aggregate inflow forecast (F_t) at t , and j represents the j th interval of aggregate inflow (Q_{t+1}) at $t+1$. The recursive equation is defined below

$$f_t(S_{t-1}, i) = \max_{s_t} [B(S_{t-1}, i, S_t) + \sum_j P_{ij}^t f_{t+1}(S_t, j)] \quad (7)$$

where $f_t(S_{t-1}, i)$ represents the maximum expected hydropower generation from time step t to T .

2.4.3. AD-SDP Model Recursive Equation

Recall that a single equivalent reservoir is constructed to reduce the dimension of the optimal reservoir operation problem in AD-SDP model. Thus, the AD-SDP model uses both the aggregate storage and aggregate inflow as state variables. Comparing AD-SDP and AF-SDP can reveal the efficiency in the use of the equivalent reservoir in the optimization process.

Assuming that S_{t-1} and S_t represent the aggregate storages at the beginning and end of time step t , these variables are disaggregated using the method of conditional expectation, i.e., equation (2). The recursive equation is defined as

$$f_t(S_{t-1}, i) = \max_{S_t} [B(S_{t-1}, i, S_t) + \sum_j P_{ij}^t f_{t+1}(S_t, j)] \quad (8)$$

where i represents the interval of inflow forecast (F_t) at time step t , and j represents the interval of aggregate inflow (Q_{t+1}) at $t+1$.

2.4.4. AD-BSDP Model Recursive Equation

In AF-SDP and AD-SDP, the randomness of aggregate inflows is addressed as a simple Markov Process; the inflow forecasts are considered as accurate information. In the BSDP model, however, the inflow forecast uncertainty is addressed by Bayesian theory. Comparing the performance of AD-SDP and AD-BSDP, the impact of inflow forecast uncertainties on decision making can be evaluated.

Figures 2c and 2d show how the inflows are updated for two forecast horizons, i.e., 5 days and 10 days, respectively. The P_{hik}^t represents the posterior flow transition probability ($P[Q_t|F_t, Q_{t-1}]$); P_{kn}^t represents predictive probability of inflow forecasts ($P[F_{t+1}|Q_t]$). The recursive equation for AD-BSDP is defined as

$$f_t(S_{t-1}, h, i) = \max_{S_t} \{ \sum_k P_{hik}^t [B(S_{t-1}, k, S_t) + \sum_n P_{kn}^t f_{t+1}(S_t, k, n)] \} \quad (9)$$

where h represents the interval of inflow (Q_{t-1}) at time step $t-1$; i represents the interval of inflow forecast (F_t) at time step t ; k represents the interval of inflow (Q_t) at time step t and n represents the interval of inflow forecast (F_{t+1}) at time step $t+1$.

2.4.5. TS-BSDP Model Recursive Equation

Although AD-BSDP considers the inflow forecast uncertainties, they are treated indifferently across the entire FH. Thus, the TS-BSDP model is constructed to explicitly utilize the varying magnitudes of inflow forecast uncertainty and make the best of the information available to improve system performance.

Figure 2e shows the representation of inflow uncertainties in TS-BSDP. The inflow forecast horizon (10 days) is partitioned into two parts equally: the inflow forecasts in SF (1–5 days) are assumed to be accurate. The forecast uncertainties in MF (6–10 days) are addressed by Bayesian theory. It should be noted that similar to AF-SDP and AD-SDP inflow forecast in SF is assumed to be accurate. Note that the errors involved in Stage 1 can be propagated into Stage 2 and eventually affect the reservoir operation decisions. However this depends on the length of the first stage. In this particular case study, preliminary investigations showed that there is little benefit of considering the uncertainties in SF in terms of improving system performance. This is due to the relatively high accuracy of inflow forecasts in SF since the length (5 days) of the first period allows a quite accurate forecast in the study area [Chu *et al.*, 2010]. Future work remains for the cases with a long period for Stage 1 where the errors may have a considerable impact.

A backward recursive relationship is developed starting from the final time step T . $f_t^{SF}(\cdot)$, $f_t^{MF}(\cdot)$ and $f_t^{FH}(\cdot)$ are used to represent the maximum expected generation of hydropower for the SF, the MF and the entire FH, respectively. An inflow forecast interval in SF and MF are represented by s and m , respectively. The operation policies of decision Strategies B and Strategy C are derived from $f_t^{SF}(\cdot)$ and $f_t^{FH}(\cdot)$ respectively. The recursive equations for different time steps are described below.

When $t = T$: Since there is no forecast for the MF at time step T , which is represented as φ , a null value. In order to compare the performance at time step T , the aggregate storages at the end of time step T is set to L_{\min} , which is the minimum reservoir storage of the equivalent reservoir. The value of L_{\min} does not affect

the optimization results when the iteration reaches a steady state. So the recursive equation of MF is equal to that of SF.

$$\begin{cases} f_T^{SF}(S_{T-1}, s, \varphi) = \max [B(S_{T-1}, i, L_{\min})] \\ f_T^{MF}(S_{T-1}, s, \varphi) = \max [B(S_{T-1}, i, L_{\min})] \end{cases} \quad (10)$$

When $t = T-1$: When the backward recursion moves one step backward to $t=T-1$, there are two periods for calculation: $T-1$ and T . The system performance measure is $B(S_{T-2}, s, S_{T-1})$ at the current time step $t=T-1$. The expected value of system performance for SF at time step $T-1$ is determined by the performance measure at time step $T-1$ and the expected value of system performance for SF at time step T as well as inflow transition from s to k , given the inflow forecasts (s) and (m) at SF and MF, respectively, updated by the posterior flow transition probability (P_{smk}^T). The expected value of system performance for MF at $T-1$ is determined by the posterior flow transition probability (P_{smk}^T) and the expected value of the system performance for SF at time step T . The system performance measure of the entire FH at time step $T-1$ is determined by the system performance and inflow forecast of SF and MF. The recursive equation of SF, MF and FH are defined below.

$$\begin{cases} f_{T-1}^{SF}(S_{T-2}, s, m) = \max [B(S_{T-2}, s, S_{T-1}) + \sum_k P_{smk}^T f_T^{SF}(S_{T-1}, k, \varphi)] \\ f_{T-1}^{MF}(S_{T-2}, s, m) = \max [P_{smk}^T f_T^{SF}(S_{T-1}, k, \varphi)] \\ f_{T-1}^{FH}(S_{T-2}, s, m) = \frac{s}{s+m} f_{T-1}^{SF}(S_{T-2}, s, m) + \frac{m}{s+m} f_{T-1}^{MF}(S_{T-2}, s, m) \end{cases} \quad (11)$$

When $t \leq T-2$: When the iteration moves backward to $T-2$ or beyond, there are at least two periods that should be considered for calculation. The expected value of the system performance for SF at time step t is determined by four factors: system performance measure of SF at time step t $B(S_{t-1}, s, S_t)$, the expected value of system performance for SF at time step $t+1$, the posterior flow transition probability (P_{smk}^{t+1}) and the predictive probability of forecasts (P_{kn}^{t+1}), which represents the predictive probability of inflow forecasts $P[F_{t+2}|Q_{t+1}]$ where k represents the interval of inflow Q_{t+1} at time step $t+1$ and n represents the interval of inflow forecast F_{t+2} at time step $t+2$. The expected value of system performance for MF is determined by the last three terms. Thus, the recursive equations of SF, MF and FH are defined below.

$$\begin{cases} f_t^{SF}(S_{t-1}, s, m) = \max \{ B(S_{t-1}, s, S_t) + \sum_k P_{smk}^{t+1} \times [\sum_n P_{kn}^{t+1} f_{t+1}^{SF}(S_t, k, n)] \} \\ f_t^{MF}(S_{t-1}, s, m) = \max \{ P_{smk}^{t+1} \times [\sum_n P_{kn}^{t+1} f_{t+1}^{SF}(S_t, k, n)] \} \\ f_t^{FH}(S_{t-1}, s, m) = \frac{s}{s+m} f_t^{SF}(S_{t-1}, s, m) + \frac{m}{s+m} f_t^{MF}(S_{t-1}, s, m) \end{cases} \quad (12)$$

3. Case study

3.1. Hun River Cascade Hydropower Reservoirs

The Hun River cascaded hydropower reservoir system is used to test the newly developed methodology in this study. It is located in the lower reaches of the Hun River basin in the northeast of China. Figure 3 shows the location of the hydropower reservoirs and gauging stations. The basin covers $40^\circ 40' \sim 42^\circ 15' N$ and $124^\circ 43' \sim 126^\circ 50' E$ with an approximate area of $15,000 \text{ km}^2$. Precipitation in dry and wet seasons varies significantly. The mean annual rainfall is about 860 mm, and about 70% to 80% of the precipitation occurs in the wet seasons (from May to September).

Three reservoirs, Huanren, Huilong and Taipingshao, constitute the Hun River cascaded hydropower system and their basic characteristics are listed in Table 1. The upstream reservoir, Huanren reservoir, is the main water source for hydroelectric production. The downstream reservoirs of Huilong and Taipingshao are daily regulation reservoirs. During operation, the Huilong reservoir is most prone to spillage due to its limited regulation ability and small installed capacity.

In the case study, the operator needs to plan the hydropower generation operation policies for the electrical power grid system for 5 days, 10 days, 20 days and a month ahead respectively. In order to guarantee the stability of the electrical power grid system, the operation policies can only be changed at least 5 days ahead, during the normal operating periods except flood periods.

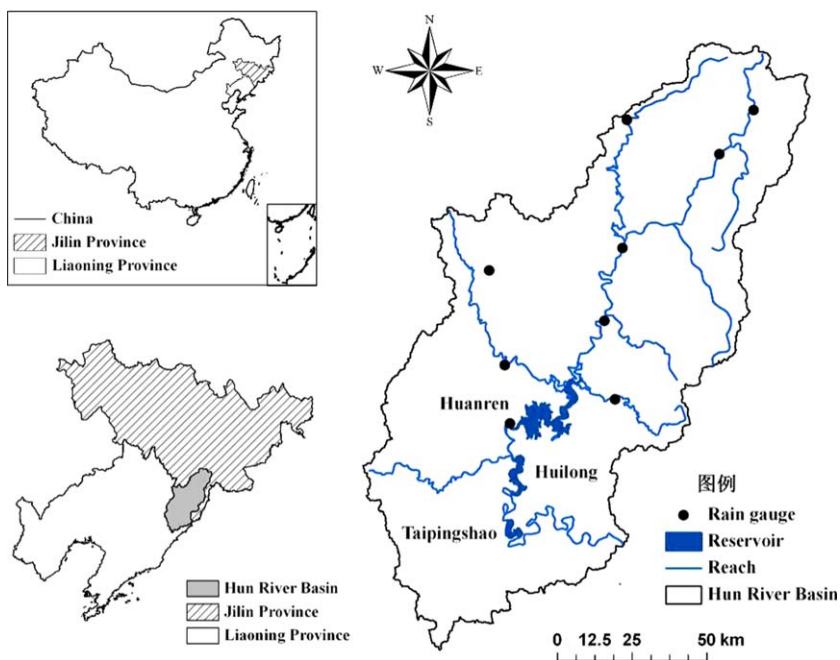


Figure 3. The Hun River cascade reservoirs system.

3.2. Forecasted and Observed Data

The Global Forecast System (GFS) developed by the U.S. National Centers for Environmental Prediction, is a global Numerical Weather Prediction (NWP) computer model. The QPFs used in this study are derived from the precipitation forecasts over the entire East Asia region. For research purposes, the GRIB1 data sets are collected from the NOAA server from 2001 to 2010. The QPFs for the 10 days forecast made at the first data assimilation cycle of 0000 UTC every day have been extracted from the GRIB1 data sets for all gauging stations of the Hun river basin shown in Figure 3, by a GRIB1 encoder/decoder program. Finally, the average forecasts of precipitation in the Hun river basin are estimated by the Thiessen polygon method [McCuen, 1998].

The observed precipitation data are available from 1968 to 2010 in the upstream of Huanren. The observed inflow data of each reservoir were obtained from 1968 to 2010. The observed data are used to calibrate and verify the inflow forecasting models.

3.3. Rainfall-Runoff Models

Many hydrological models can be used to predict reservoir inflows using QPFs, such as conceptual rainfall-runoff models and distributed hydrological models [Nijssen et al., 1997; Collischonn et al., 2005, 2007; Tang et al., 2007]. In the case study catchment, the inflow is low and stable during dry seasons (from October to April), however, it becomes high and unstable due to frequent, heavy rainfall during wet seasons (from May to September). Thus, the inflows in this case study are predicted using two models: a multiple linear regression model for dry seasons and Xinanjiang model for wet seasons, with the intent of improving predictive accuracy.

Table 1. Characteristics of Hun River Cascaded Hydropower Reservoirs

Characteristic	Huanren	Huilong	Taipingshao
Basin area (km ²)	10400	2100	528
Total storage (Mm ³)	3460	123	182
Usable storage (Mm ³)	2199	90	164
Dead storage (Mm ³)	1380	72	145
Normal water level (m)	300	221	191.5
Dead water level (m)	290	219	190
Installed capacity (MW)	222	72	161
Firm output (MW)	33	18	25
Turbine capacity (m ³ /s)	416	330	534

with the intent of improving predictive accuracy.

The multiple linear regression model is based on the correlation relationship between a number of important basin factors and inflow forecasts. Building on previous studies [Liang et al., 2009; Zhou et al., 2010; Tang et al., 2010; Wang et al., 2010], four factors, i.e.,

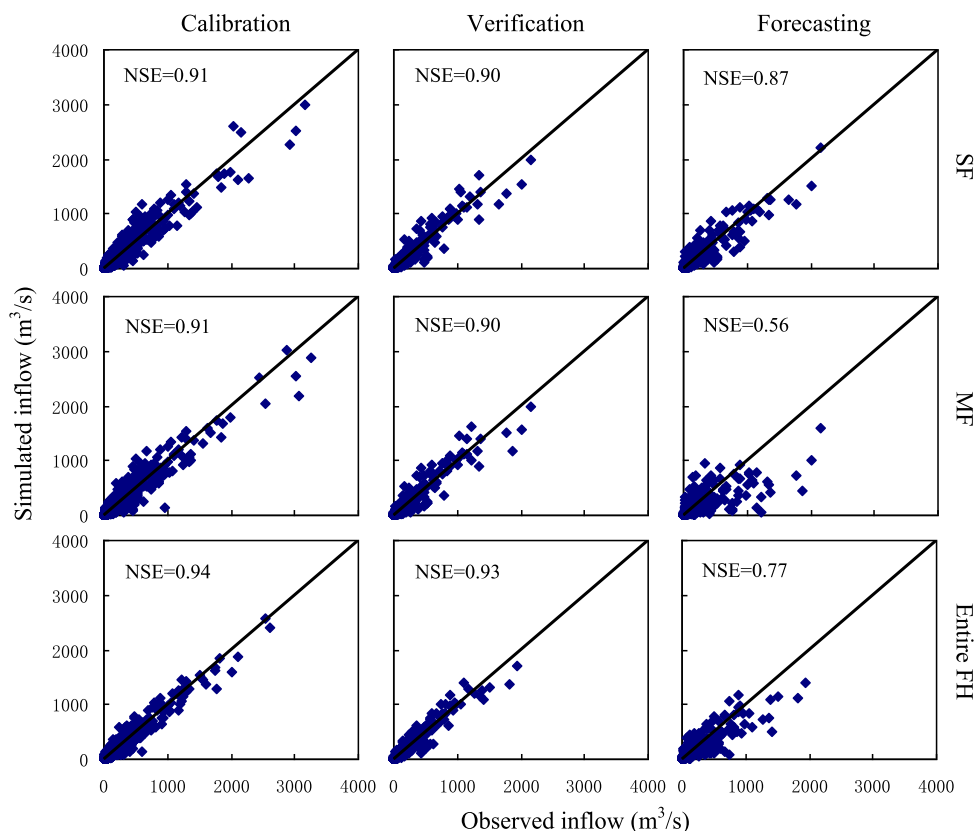


Figure 4. The comparison of observed and estimated inflows.

precipitation and soil moisture of the current period, average inflow of the previous period, and QPFs of the next period, are selected to forecast the inflow at the next period (forecast horizon). The data of soil moisture are obtained through continuous simulation using observed daily inflow and precipitation. The parameters of the multiple linear regression model are determined using the least squares technique [Tang et al., 2010; Wang et al., 2010].

The Xinanjiang model is a conceptual rainfall-runoff model, which was developed by Zhao [1992]. This model has been widely used in China, particularly in humid and semihumid regions [e.g., Cheng et al., 2006]. The inputs to the model are daily precipitation from QPFs and soil moisture indices, and the model output is daily runoff. A Genetic Algorithm (GA) is used to calibrate the Xinanjiang model parameters [Cheng et al., 2006]. The Nash–Sutcliffe Efficiency (NSE) and Root Mean Square Error (RMSE) are used to assess predictive accuracy of the two chosen models.

The inflow forecasting models introduced above were applied to estimate the average inflow for SF (days 1–5), MF (days 6–10) and FH (days 1–10). The respective parameters of the Xinanjiang model and regression model were calibrated against observed data from 1968 to 2000, and were verified with observed data from 2001 to 2010. Based on the calibrated models, the medium-range QPFs of GFS from 2001 to 2010 were applied to forecast the inflows, which were used to assess the performance of the newly developed TS-BSDP model against other SDP models described above. It should be noted that due to the availability of QPF forecasts the performance of the new model discussed above is calculated using the 10 year QPF data, which is used to derive the operational rules. Thus, its performance should be further tested on more hydro-power systems with different system characteristics and different sets of QPF data.

Figure 4 shows the simulated versus observed flows of Huanren reservoir during calibration, verification and forecasting periods. During the periods of calibration and verification, the flow forecasting models perform well, and the values of NSE equal to or greater than 0.9. During forecasting periods, the value of NSE is reduced to 0.77 for the 10 days forecasts. The accuracy for SF is high, with a NSE value of 0.87. But for MF the

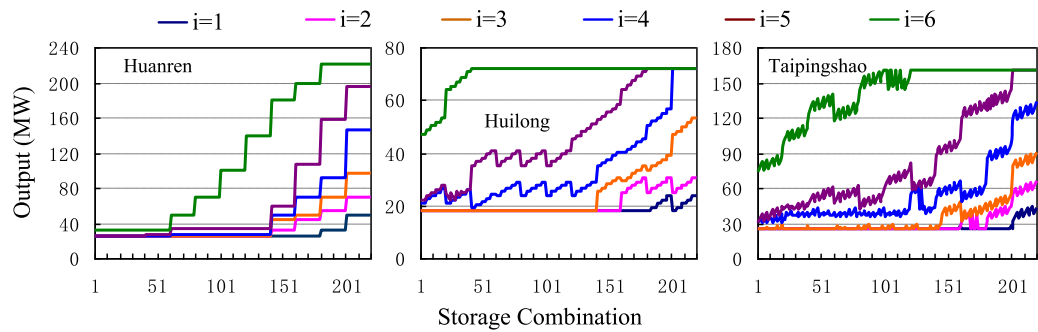


Figure 5. The operation policies derived using AF-SDP.

value of NSE is only 0.56. The forecasts for high flows, larger than 1000 m³/s, are generally lower than the observed inflows mainly because high QPFs are generally underestimated, i.e., lower than observed precipitations. This clearly shows that the inflow forecasts of SF and MF have different degrees of uncertainty, and thus should have different strategies for use in reservoir operation.

4. Results and Discussion

4.1. Operation Policies

Operation policies from various models are derived using relevant backward recursive equations by iterating until the ending storage reaches a steady state [Su and Deininger, 1972; Serrat-Capdevila and valdés, 2007; Mujumdar and Nirmala, 2007], with inflow data from 1968 to 2010. The penalty factors of α and β in the objective function are set to 1 and 2 respectively, according to Tang et al. [2010]. The penalty factor of η in equation (3) is set to 5 from the interval [1, 10] through trial and error in terms of the performance of disaggregation.

In the optimization process, the aggregate inflows are discretized into 6 intervals ($\mu=6$), representing 15%, 30%, 45%, 60%, 75% and 90% percentiles. In AF-SDP model, the storages of Huanren, Huilong and Taipingshao are discretized into 25, 3 and 3 intervals respectively ($\beta^1=25; \beta^2=3; \beta^3=3$). The number of possible combinations of storage is $25 \times 3 \times 3 = 225$. In other models where an equivalent reservoir is constructed, the storage of the equivalent reservoir is discretized into 30 intervals ($\phi=30$) with an increment of 30 Mm³, resulting in a total number of storage combinations $30 \times 3 \times 3 = 270$. It should be noted that a higher level of discretization can be used for reservoir inflow and storage; however, it might become impractical when coming to real time reservoir operations in practice, as it results in a large number of operating policies for a multireservoir system.

4.1.1. Operation Policies of AF-SDP

Figure 5 shows the operation policies for 10 days derived using the AF-SDP model at time step 22 (from 1 to 10 August). In order to simplify the operation policies, the combinations of individual storages are represented

Number	Storage Combination (Mm ³) (HR/HL/TPS) ^a
1	1380/72/145
2	1380/72/155
3	1380/72/164
4	1380/81/145
5	1380/81/155
6	1380/81/164
.....
223	2199/90/145
224	2199/90/155
225	2199/90/164

^aHR, HL, and TPS represent the storage of Huanren, Huilong and Taipingshao respectively.

by numbers, as shown in Table 2. Thus, the x axis represents the number of storage combinations and y axis represents the output of individual reservoirs. The different curves represent the results of different intervals (i) in which aggregate inflow forecast lies. The variable of i represents the forecasting aggregate inflow in current period.

Figure 5 can be interpreted using an operation condition as an example, where the storages of Huanren, Huilong and Taipingshao are 1380, 90 and 155 Mm³ respectively and the aggregate inflow belongs to the 6th interval. The storage combination (1380/90/155) is represented by the number of 8 as shown in Table 2. The outputs of Huanren,

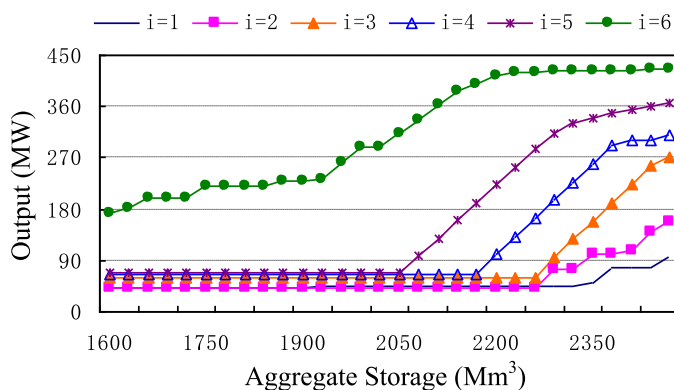


Figure 6. The operation policies of the equivalent reservoir derived by AD-SDP.

Huilong and Taipingshao for the FH of 10 days are obtained directly from the individual operation policies as 37 MW, 52 MW and 85 MW, respectively.

It can be noted that there are some fluctuations in the output curves of Huilong and Taipingshao, which are caused by the ordering of storage combination. As shown in Table 2, the order of reservoirs is Huanren, Huilong, and Taipingshao; this results in the highest level of fluctuation for the last reservoir—Taipingshao, followed by Huilong.

4.1.2. Operation Policies of AD-SDP

Figure 6 shows the operation policies from AD-SDP for 10 days from 1 to 10 August. Recall that the AD-SDP model uses the aggregated inflow and storage of the equivalent reservoir as state variables and the decision variable is converted to the aggregate output of equivalent reservoir. Thus, the x axis represents the storage of the equivalent reservoir, and the y axis represents the aggregate output.

Taking the same example as in AF-SDP, the storages of Huanren, Huilong and Taipingshao are 1380, 90, and 155 Mm³ respectively and the aggregate inflow belongs to the 6th interval. Recall that the outputs of individual reservoirs, are 37 MW, 52 MW and 85 MW respectively, using the operation policies of AF-SDP in Figure 5. These outputs are expected values, which are the optimal policies for the entire planning horizon (T) under the conditional expectation of inflows instead of the optimal policies for every combination of inflows. Huilong reservoir has the least regulation potential due to its design storage, so it is most prone to spillage. However, in AF-SDP, the release of Huanren is not controlled to avoid or reduce the spillage at Huilong. While in AD-SDP, the outputs of individual reservoirs are obtained as 33 MW, 60 MW and 88 MW for Huanren, Huilong and Taipingshao, respectively, from the aggregate output of the equivalent reservoir (181 MW). It can be seen that the performances of Huilong and Taipingshao are improved while the performance of Huanren is reduced as a result of reduced release from Huanren. In this way, the performance of AD-SDP is better than that of AF-SDP by storing more water in Huanren reservoir for hydropower generation.

4.1.3. Operation Policies of AD-BSDP

Figure 7 shows the policies derived by AD-BSDP for 10 days from 1 to 10 August. The x axis and y axis have the same meaning as those in Figure 6. In Figure 7, h and i represent the observed inflow (h) at the previous time step and the forecasting inflow (i) at the current time step. The operation policies of AD-BSDP include 36 inflow scenarios. In real-time operation at August 1, the aggregate output for the next 10 days can be obtained according to inflow information of h and i as well as the aggregate storage on 1 August.

Comparing the policies in Figures 6 and 7, it can be seen that the operation policies derived by AD-BSDP are affected by observed inflows at the previous time step. For the same inflow forecast intervals in Figure 6, there are six different policy scenarios (represented by h) due to different inflows at the previous time step in Figure 7. The observed inflows at the previous time step are used to reduce the uncertainty of inflow forecast. It can be seen that with an increase in inflow forecast (from $i=1$ to 6), the influence of the observed inflows is diminished, with no difference between the six policy scenarios ($h=1, \dots, 6$) when $i=6$. The results also demonstrate that the observed inflows at the previous time step have a substantially more significant effect in the cases of high storage than low storage, as revealed by the different trajectories at the high storage end in each inflow forecast case in Figure 7.

In comparison with AD-SDP, AD-BSDP addresses the uncertainty of inflow forecast. When the inflow forecast at current time step is high or the reservoir storage is high, the reservoir is prone to spillage. This information is used for disaggregation to avoid or reduce spillage. As a result, the aggregate output of AD-BSDP is larger than that of AD-SDP. Using the same operating example, i.e., when inflow forecast $i=6$ and aggregate

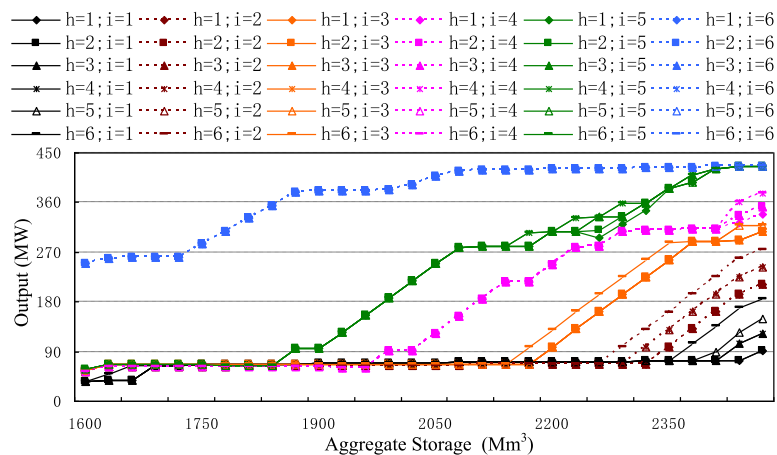


Figure 7. The operation policies of equivalent reservoir derived by AD-BSDP.

storage is 1600 Mm³, the output from AD-BSDP is 250 MW, representing a significant increase compared to 181 MW from AD-SDP.

4.1.4. Operation Policies of TS-BSDP

Figure 8a shows the complete operation policies derived by TS-BSDP using Strategy C for 10 days from 1 to 10 August and the operation policies of different inflow forecasts of SF from s=1 to s=6 are shown in Figures 8b–8g. The variables s and m in Figure 8 represent inflow forecast of SF (1–5 days) and MF (6–10 days) respectively.

Figures 8b–8g represent the operation policies for one single inflow forecast interval of SF from 1 to 6, respectively. In each figure, there are six different policy scenarios due to different inflows of MF. The 10 day operation policies are obtained by weighting the operation policies of SF and MF based on the inflow intervals of s and m. The operation policies of SF are determined by $f_t^{SF}(\cdot)$, in which inflow forecast (m) is used to forecast the inflow interval (reflecting inflow trend during MF approximately) and the expected value of remaining time steps are affected by m through the posterior flow transition probability. The operation policies of MF are determined by $f_t^{MF}(\cdot)$, in which the inflow forecast (s) is used to reduce the uncertainty of m.

While in AD-SDP and AD-BSDP, the 10 day inflow forecast at the current time step is only represented by a representative value of interval. Thus the variation of inflows is not reflected in the operation policies. For example, assuming that the 10 day inflow forecast is 500 m³/s, it belongs to interval 3 in AD-BSDP. Dividing the 10 days inflow into SF and MF, there are several possible combinations such as SF=300 m³/s (s=2) and MF=700 m³/s (m=4) or SF=700 m³/s (s=4) and MF=300 m³/s (m=2). These two combinations have different operation policies under the same water storage as shown in Figures 8c and 8e, respectively.

Similar to Figure 8a, Figure 9a shows the complete operation policies derived by TS-BSDP using Strategy B for 5 days from 1 to 5 August. The operation policies of the inflow forecast interval of SF from s=1 to s=6 are shown in Figures 9b–9g respectively.

The operation policies in Figures 8a and 9a have different meanings in terms of decision horizon. In real-time operation, the operation policies in Figure 8a represent the 10 days operation decision in future according to the scenarios of s and m. The operation policies in Figure 9a determine the first 5 days operation using the inflow forecasts of s and m. The operation policies in Figure 9a are more effective in terms of hydropower generation, and are able to adapt to varying conditions for the next 5 days' operation decision by updating the inflow forecasts of s and m.

In Figures 9b–9g, the aggregate outputs, in the case of the same inflow forecasts of SF, fluctuate with varying inflow forecasts of SM. Because in the recursive equation $f_t^{SF}(\cdot)$, the operation policies during SF are affected by the inflow forecasts s and m. The inflow forecasts (m) only affect the expected value of remaining time steps through the posterior flow transition probability.

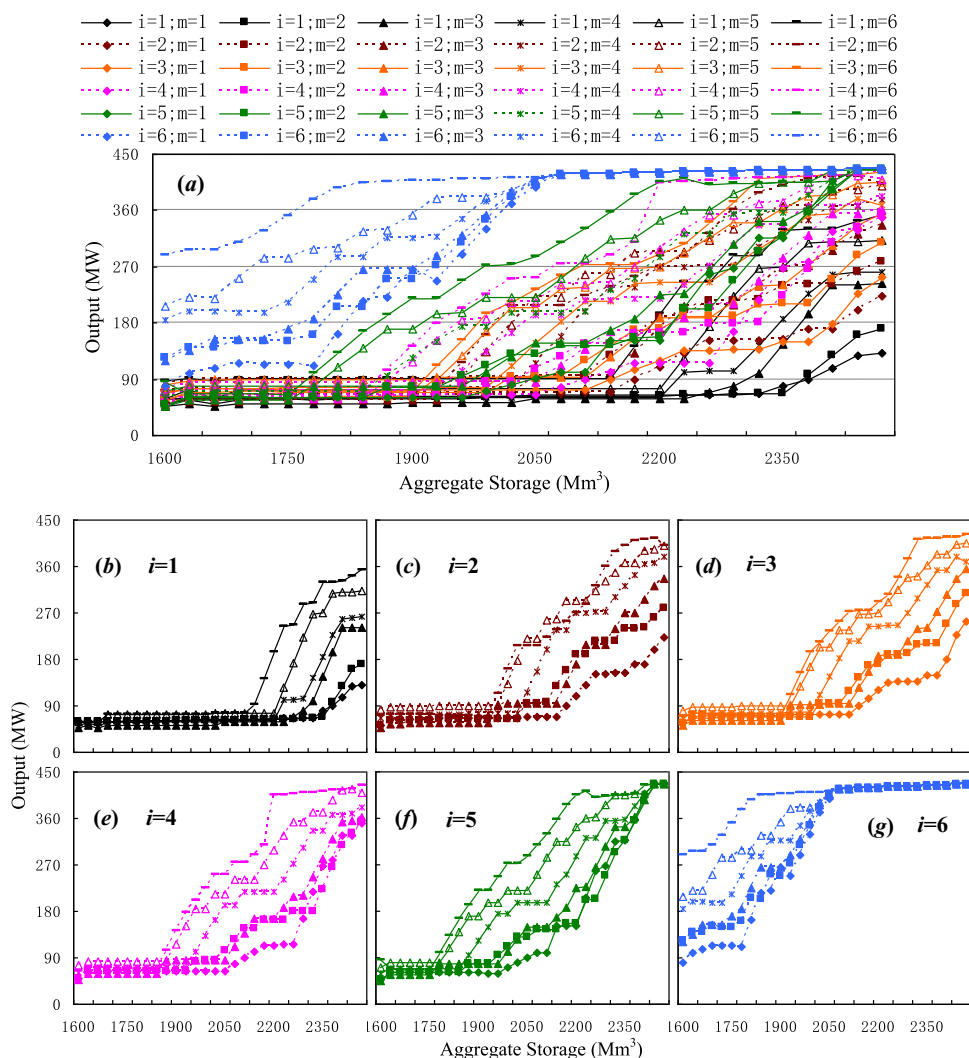


Figure 8. The operation policies of the equivalent reservoir for Strategy C between 1 and 10 August.

While in Figures 8b–8g, the operation policies are obtained by weighting the operation policies of SF and MF based on the inflow forecast intervals (s and m). The operation policies during SF are affected not only by the expected value of remaining time steps but also by the operation policies of MF based on the weight of s and m . Comparing with 5 day policies derived from AF-SDP, AD-SDP, and AD-BSDP, the policies in Figure 9a can make the best use of the forecast information to improve the efficiency and reliability as discussed below.

4.2. Performance Evaluation With Observed Data

The Annual Hydropower Generation and reliability are chosen as metrics to compare the performance of the operation policies derived from AF-SDP, AD-SDP, AD-BSDP, and TS-BSDP. Annual Hydropower Generation is an important indicator of the hydropower system in terms of use of water resources. System reliability is defined as the probability that the system output is not lower than the minimum demand, i.e., the required firm output in this study [Hashimoto *et al.* 1982]. Note that reliability measures the frequency of failures only and in some cases the magnitude of failures needs to be considered particularly when the deficit situations for hydropower generation become severe [Raje and Mujumdar, 2010].

The performance metrics were calculated using the observed inflows from 2001 to 2010, which implies that forecast inflows are assumed to be known perfectly at the time of operation. It is expected that the results can reveal the maximum efficiency and reliability that could be achieved based on accurate information, and also show TS-BSDP can handle the different degrees of forecast uncertainty. The performance of AF-SDP,

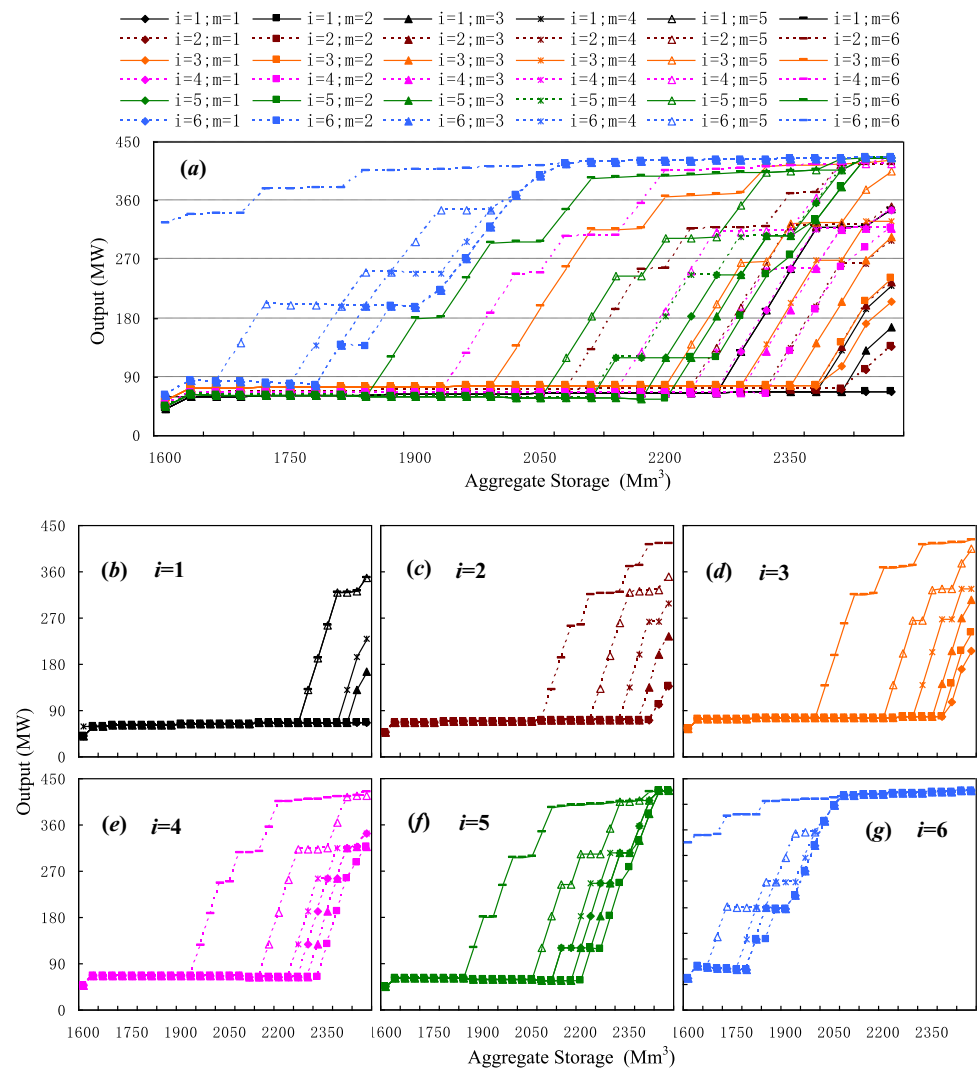


Figure 9. The operation policies for Strategy B between 1 and 5 August.

AD-SDP, and AD-BSDP is simulated using Strategy A with two decision horizons of 5 and 10 days, while the TS-BSDP model is assessed using Strategy B with a decision horizon of 5 days and Strategy C with a decision horizon of 10 days, as shown in Figure 1.

It can be seen from Table 3 that the performance indicators from the 10 days decision horizon are better than those from 5 days using observed inflows. The results confirm that use of longer-range inflows improves system performance when the inflows are accurate (in the case of observed inflows).

Comparing the results from AD-SDP and AF-SDP confirms the effectiveness of the Aggregation-Disaggregation method [Mujumdar and Nirmala 2007]. With both decision horizons, the Annual Hydropower Generation values of Huilong and Taipingshao are improved significantly with AD-SDP, resulting in a greater Annual Hydropower Generation value from the hydropower system, although Huanren's performance is reduced slightly. This is because the discharges of the three reservoirs are optimized to produce a maximum amount of water at the end of a decision horizon.

In the case study, about 70% to 80% of the annual precipitation occurs in wet seasons. Thus the indicator of Annual Hydropower Generation is mainly affected by the operation in wet seasons. The results obtained in Table 3 show that AD-BSDP performs better than AD-SDP in terms of Annual Hydropower Generation. Conversely, the system reliability indicator is mainly affected by the operation in dry seasons due to low inflows. It can be noted that the system reliability of AD-BSDP with the 5 day decision horizon is reduced when

Table 3. System Performance From Various Optimization Models Using Observed Inflow Information^a

Models	DH (days)	Annual Hydropower Generation (MWH)			System AHG	Reliability (%)			System Reliability
		Huanren	Huilong	Taipingshao		Huanren	Huilong	Taipingshao	
AF-SDP	5	420.96	235.12	378.45	1034.53	82.36	86.53	86.67	85.19
AD-SDP	5	417.65	249.98	407.43	1075.06	84.03	87	89	93.47
AD-BSDP	5	426.86	253.56	414.15	1094.56	86.53	88.47	88.61	87.36
TS-BSDP ^{5B}	5	434.45	258.29	422.33	1115.06	85.28	92.42	84.17	94.02
AF-SDP	10	428.93	231.78	382.85	1043.56	84.44	83.83	86.39	88.89
AD-SDP	10	423.03	247.19	408.19	1078.41	85	86.44	89.17	93.61
AD-BSDP	10	429.12	252.04	418.6	1099.76	85.56	87.17	88.61	92.78
TS-BSDP ^{5C}	10	434.74	258.97	429.96	1123.68	87.5	91.17	86.67	93.05

^{a5B} represents Strategy B and ^{5C} Strategy C.

compared to the 10 day horizon. Through the analysis, the posterior flow transition probability (P_{hik}^t) is mainly affected by observed inflow (h) at the previous time step with a shorter decision horizon. In AD-BSDP model, h is used to reduce the uncertainty of inflow forecast (i). With a shortened forecast horizon, the relationship between h and k is enhanced, in a sense that k is forecasted by h , instead of i . When the forecast horizon is 10 days, the relationship is weakened by the uncertainty in inflow forecast (i).

The overall performance of TS-BSDP is better than that of AD-BSDP. With the decision Strategy B, TS-BSDP uses the inflow of 10 days to determine the operation policy of the SF, while AD-BSDP uses only the inflow of SF. In order to illustrate the influence of considering the MF, the operation processes from time step 35 to 48 in the wet season of 2001 are shown as an example in Figure 10. Figure 10a shows the inflows and the releases for hydroelectric generation and Figure 10b shows the water levels and spillages of Huanren. There is a significant difference in the release at time step 41 between AD-BSDP and TS-BSDP. TS-BSDP model increases the release to reduce the storage to the dead storage volume due to the high inflow forecasted at time step 42. Similarly, the release is reduced at time step 45 to store water for a low inflow

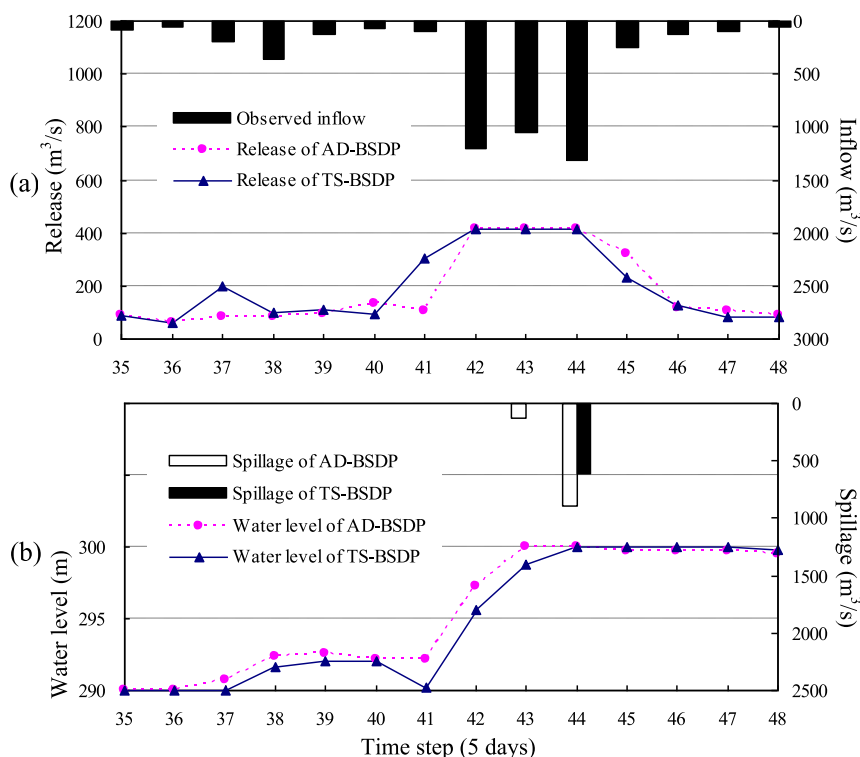


Figure 10. Comparison of operation processes between AD-BSDP and TS-BSDP.

Table 4. System Performance From Various Optimization Models Using Inflow Forecasts^a

Models	DH (days)	Annual Hydropower Generation (MWH)			System AHG	Reliability (%)			System Reliability
		Huanren	Huilong	Taipingshao		Huanren	Huilong	Taipingshao	
AF-SDP	5	414.92	233.2	375.5	1023.62	80.42	85.14	84.72	83.47
AD-SDP	5	407.61	249.92	404.6	1062.14	82	86	85	93.05
AD-BSDP	5	421.52	252.51	411.7	1085.73	85.14	87.08	87.5	85.97
TS-BSDP ^{5B}	5	421.06	258.07	416.95	1096.08	85.28	89.61	88.72	93.75
AF-SDP	10	411.78	229.13	372.74	1013.65	81.94	82.17	81.94	86.67
AD-SDP	10	404.07	240.33	401.69	1046.09	79.17	85.61	84.21	92.25
AD-BSDP	10	419.58	248.87	413.79	1082.24	83.33	87.17	87.78	92.77
TS-BSDP ^{5C}	10	419.75	254.15	414.37	1088.26	83.89	89.46	86.94	92.25

^{a5B} represents Strategy B and ^{5C} Strategy C.

forecasted at the next time step. During the operation from time step 35 to 48, the hydropower generation based on the decision Strategy B of TS-BSDP is increased by 4.53 MWH compared to that from the AD-BSDP.

In Table 3, the decision Strategy C performs better than Strategy B and Strategy A of other models using observed inflows. Using observed inflows, the flow increases in wet seasons can always be identified and represented by MF. This information is used in Strategy C, as a result, the outputs of decision Strategy C are better than those of decision Strategy A. Take an example of the 10 days inflow forecast being 500 m³/s, which belongs to interval 3 in AD-BSDP. The output of AD-BSDP is 80MW with Strategy B. Dividing the 10 days inflow into SF and MF, assuming that the inflow SF and MF are in the intervals $s=2$ and $m=4$, the average output of FH generated from Strategy C is 100 MW ($=\frac{2}{6} \times 60 + \frac{4}{6} \times 120$) using the aggregate outputs of SF (60 MW) and MF (120 MW) with inflow intervals $s=2$ and $m=4$ respectively.

4.3. Performance Evaluation by Forecasts

The performance of TS-BSDP is further tested under real time operation situations. In this case, the three reservoirs are operated under the operation policies developed from various models, using the forecasted inflows from 2001 to 2010 with the QPFs. The system performances of various models are provided in Table 4.

In general, the indicator values in Table 4 are reduced compared with those in Table 3, implying that the system performance is reduced with inflow forecasts in comparison to observed data. This is due to the uncertainties in inflow forecasts. Different from the results in Table 3, the values of Annual Hydropower Generation for 5 days are better than those of 10 days. This confirms the finding that use of longer-term inflow forecasts improves system performance when the forecasts are relatively accurate (in the case of observed inflows), however the improvement is diminished as inflow forecast uncertainty increases with decision horizon extending [Simonovic and Burn, 1989; Maurer and Lettenmaier, 2003, 2004, Zhao et al., 2012].

With the decision Strategy C, TS-BSDP generates more power by 23.92 MWH than AD-BSDP using observed data as shown in Table 3. When inflow forecasts are used, however, the performance of TS-BSDP is only slightly better than that of AD-BSDP (Table 4). With a decision strategy of 10 days, the operation policies are determined by balancing the inflow information of SF and MF. The inflow forecasts over 10 days are of high uncertainty (NSE=0.77), especially the inflow forecasts of MF (NSE=0.56). When the decision horizon increases to 10 days, the system performance is reduced.

With the decision Strategy B, the decision horizon is 5 days. The uncertainty of MF is incorporated in decision Strategy B through the posterior flow transition probabilities. While the operation policies of Strategy C are affected by not only the posterior flow transition probabilities, but also the operation policies of MF. Thus the uncertainty of MF affects the performance of Strategy C during SF more than that of Strategy B. In real-time operation, the decision horizon of Strategy B is rolled forward by 5 days, even if inflow forecasts at MF have a low accuracy, the decision Strategy B can correct the next operation decision by updating forecast information. The simulation results in Table 4 demonstrate that the decision Strategy B of TS-BSDP performs better than the others.

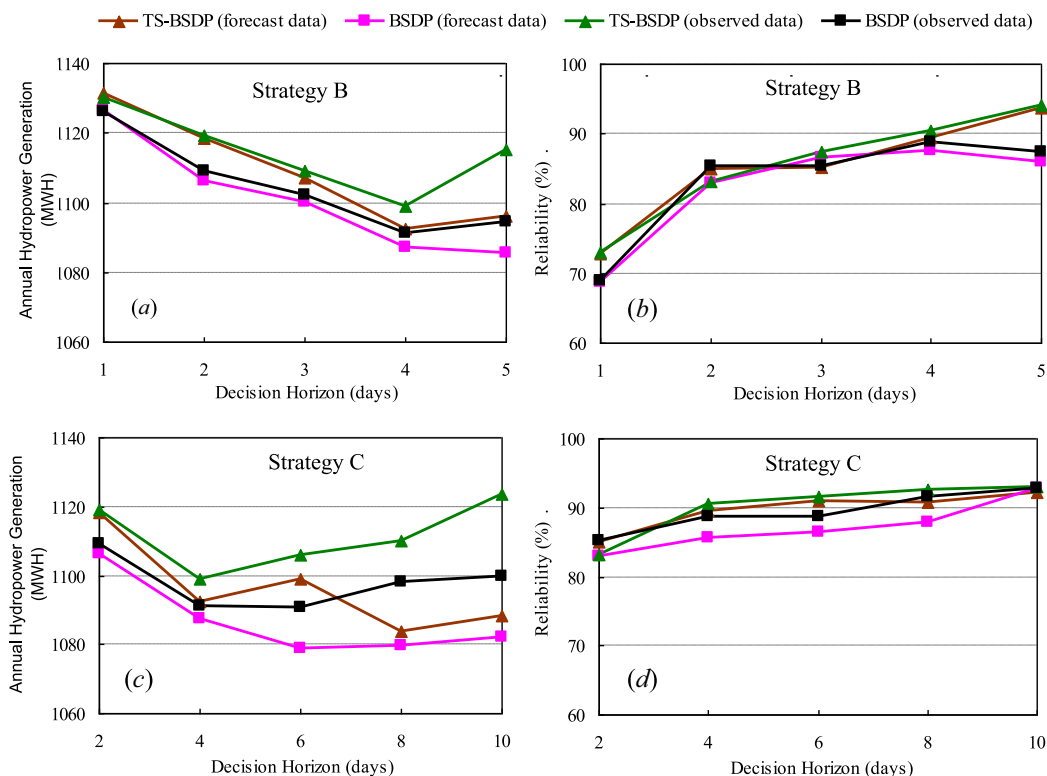


Figure 11. Performances of TS-BSDP and BSDP with different decision horizons.

4.4. Performance With Different Decision Horizons

The impact of different decision horizons on the performance of TS-BSDP is evaluated under different strategies. Under Strategy B, decision horizons arranging from 1 to 5 days are used to evaluate the system performance using both forecasted and observed data. Under Strategy C, decision horizons from 2 to 10 days are used. Figure 11 shows the Annual Hydropower Generation and reliability of TS-BSDP compared to those from BSDP under Strategy A.

With an increase in decision horizon from 1 to 5 days, the performance in Annual Hydropower Generation is generally reduced as shown in Figure 11a. Autocorrelation is used to represent a relation between the inflow in 1 day and the inflows in the preceding days. It is relatively strong in the first 5 days in the case study and can be sufficiently addressed using Markov process; thus considering a longer decision horizon (up to 5 days) does not provide valuable information for operation. However, it can be seen that the performance in Annual Hydropower Generation can be maintained or even improved when the decision horizon is greater than 5 days. This is due to the use of the flow forecasts from a longer horizon which reduces unnecessary spillage. Different from Annual Hydropower Generation, the results show that the reliability increases with a longer horizon from 1 to 10 days.

The above analyses confirm the previous finding that increasing FH may generate more hydropower; however, the benefits gained from a longer horizon may be offset by increasing forecast uncertainty [You and Cai, 2008; Zhao et al., 2012]. The results also reveal that it is more beneficial for system reliability to increase the inflow forecast horizon in comparison to hydropower generation. The above findings are supported with the results from all the models including TS-BSDP, which confirms the good agreement of TS-BSDP with previous models. Under both Strategies B and C, however, TS-BSDP outperforms BSDP at every horizon in terms of Annual Hydropower Generation whenever forecasted or observed data are used (Figures 11a and 11c). Similarly TS-BSDP generally outperforms BSDP in terms of reliability. It is expected that similar results could be obtained from other hydropower systems but TS-BSDP should be further tested on more complex hydropower systems and longer forecast (decision) horizons.

5. Conclusions

This paper investigates the use of QPFs to improve the overall system performance of cascaded hydropower systems in terms of hydropower generation and reliability. A new two-stage Bayesian model has been developed to quantify the uncertainties of inflow forecasts, which are considered as increasing with extending lead time, and two operation strategies have been developed to address the impact of the varying magnitudes of uncertainty. The newly developed TS-BSDP is tested on the Hun River cascade hydropower reservoirs system in China and is compared with three Stochastic Dynamic Programming models. Specifically, the findings obtained are summarized below.

1. The TS-BSDP model can effectively utilize inflow forecasts of varying uncertainty to improve system performance compared to three previously developed stochastic dynamic programming methods. Its computational efficiency is significantly improved using the virtual equivalent reservoir approach, although dividing the forecast into two periods (SF and MF) in TS-BSDP increases the complexity of the optimization problem by one dimension (which has the same dimensional complexity as AD-BSDP).
2. Splitting the forecast horizon into two periods in which inflow uncertainties are handled differently in response to the varying magnitudes of inflow forecast uncertainty is demonstrated to be an effective means of using the forecast information while addressing the uncertainty. The decision strategies based on the inflow forecast information can substantially influence the system performance. In particular, for the TS-BSDP model, the decision Strategy C performs better with observed inflows, and the decision Strategy B performs better with forecasted inflows. Further, the performance of decision Strategy C is strongly affected by inflow forecast uncertainty, and a shorter decision horizon is proven to be more effective in improving hydropower generation under this strategy.
3. This study shows that the benefit regarding hydropower generation gained from a longer horizon of inflow forecasts is diminished but the benefit reduction can be substantially mitigated through the explicit consideration of varying magnitudes of forecast uncertainties in the decision-making process. It also reveals that it is more beneficial for system reliability to increase the inflow forecast horizon in comparison to hydropower generation.

Overall, the newly developed two-stage Bayesian model is shown to have a clear advantage in quantifying the uncertainties of inflow forecasts to improve the overall system performance of cascaded hydropower systems. It should be noted that due to the availability of QPF forecasts the performance of the new model discussed above is calculated using the 10 year QPF data, which is used to derive the operational rules. Thus, its performance should be further tested on more hydropower systems with different system characteristics and different sets of QPF data. In particular, the assumption that inflow forecast in SF is accurate will be further investigated with different case studies and QPFs from different numerical weather prediction models; although it has little impact on the performance metrics in the case study according to some preliminary investigations. In the meantime, this model addresses only the uncertainty in inflow forecasts; hence, the impacts of other uncertainty sources such as QPFs and models need to be investigated in the future work.

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