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Kejun Wen, Yang Li, Wei Huang, Catherine Armwood, Farshad Amini, and Lin Li

1 Mechanical behaviors of hydrogel-impregnated sand

2 Kejun Wen^a, Yang Li^b, Wei Huang^c, Catherine Armwood^d, Farshad Amini^e, Lin Li^f

3 **Abstract:** Hydrogel has been widely used in medical studies due to their unique integration of
4 solid and liquid properties. There is limited studies of using hydrogel in construction materials.
5 The goal of this study was to investigate the effect of hydrogel on mechanical behaviors of sandy
6 materials. The effects of reaction time, sodium alginate content, and curing temperature on
7 mechanical behaviors of hydrogel-impregnated sand were studied through unconfined
8 compression tests, falling head permeability tests, consolidated and undrained triaxial tests,
9 scanning electron microscopy, and durability tests. The unconfined compression strength (UCS)
10 increased with sodium alginate content, but the hydraulic conductivity of hydrogel-impregnated
11 sand decreased with sodium alginate content. The optimum reaction time and curing temperature
12 were found to be 3 days and 50°C, respectively, for the hydrogel-impregnated sand. The stress-
13 strain curves of hydrogel-impregnated sand indicated that the ductility of hydrogel-impregnated
14 sand was significantly improved compared with the traditional cementitious method. Moreover,
15 the results of durability tests indicated that approximately 60% of the original UCS of hydrogel-
16 impregnated sand still remained after 12 wet-dry and freeze-thaw cycles.

17 **Keywords:** Hydrogel, Ductility, Curing condition, Wet-dry, Freeze-thaw

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18 **Introduction**

19 Mechanical and chemical treatment methods have been developed to improve soil properties of
20 strength, erosion, dynamic resistance, and stability [1]. The mechanical treatment methods
21 include densification, dewatering, and structural reinforcement [2-4]. The chemical treatment
22 methods involve chemical stabilizers such as cement, lime, fly ash, gypsum, and bituminous
23 materials [5-7]. The stabilization mechanisms of chemical treatments have been extensively
24 studied [8-11]. However, these materials are costly and may have environmental concerns [1].
25 These additives may cause the increase of soil pH after treatment, which have negative effects on
26 surrounding groundwater and plants. Moreover, the treated soils commonly exhibit brittle
27 behavior that potentially affect their stability for structures [11,12]. Meanwhile, the production of
28 traditional additives, such as cement and lime, consume a large amount of natural resources and
29 energy [1]. Therefore, the development of eco-friendly materials for soil improvement is
30 essential.

31 Non-traditional additives including enzymes, microbials, resins, acids, and polymers have
32 gained much attention in recent years [13-15]. The bio-geochemical processes that induces
33 mineral precipitation, has been utilized as an alternative to traditional chemical grouting [16].
34 Microbial induced calcite precipitation (MICP) is the most common bio-process that has been
35 studied. The MICP treatment requires the existence of ureolytic bacteria, urea, and calcium-rich
36 solution to drive the bio-geochemical reaction [13,17,18,19]. The MICP treatment helps bond the
37 soil particles of sand to improve the mechanical behavior of sandy soil. The MICP-treated sand
38 materials have been applied as alternative construction materials in the development of bio-
39 bricks and bio-beams [11,16,19]. Wen et al. [19] found that the flexure strength of bio-beams
40 could achieve around 3.0 MPa, which is equivalent to a plain concrete beams. However, the

41 MICP-treated sand materials exhibit brittle performance which is opposite of the desired ductile
42 performance needed for beams [11,19]. Recent studies have shown that using gel-type bio-
43 polymers can possibly improve the soil strength and the ductility of sandy soil [20]. Previous
44 studies mostly focused on the use of thermo-gelation bio-polymer such as gellan and agar
45 biopolymers, which require a high temperature ($\sim 100^{\circ}\text{C}$) during the reaction. Chang et al. [20]
46 mixed sand with gellan gum at 100°C and tested the UCS after sample cooling. The test results
47 showed that the 2% gellan gum-treated sand gained around 400 kPa UCS and failed at 7% strain
48 when the sample fully dried. However, the sample lost almost 90% strength after re-submerging
49 the sample into water. The durability of biopolymer-treated soil presents to be unstable in
50 aqueous environments.

51 Hydrogel, a class of three-dimensional (3D) networks formed through the cross-linking
52 of hydrophilic polymer chains embedded in a water-rich environment, possesses broadly tunable
53 physical and chemical properties [21,22]. Hydrogel is abundant in plant and animal tissue, with
54 examples ranging from xylems and phloems to muscles and cartilages [23]. Due to their unique
55 integration of solid and liquid properties, hydrogel has been widely explored in diverse
56 application such as drug delivery, biomedicine, soft electronics, sensors, tissue engineering, and
57 coating for medical devices [22,23,24]. Gong et al. [24] developed a strong hydrogel by inducing
58 the double-network structure method. The double-network structure with poly (2-acrylamido-2-
59 methylpropanesulfonic acid) and poly (acrylamide) hydrogel can sustain a compression stress of
60 17.2 MPa and recover immediately after unloading. Sun et al. [25] developed a synthetic
61 hydrogel by mixing Ca-alginate and polyacrylamide to achieve tough and stretchable properties.
62 The alginate-polyacrylamide hybrid gel can be stretched to exceed 20 times its original length

63 without rupture. The superior toughness and mechanical strength of hydrogel has the potential to
64 improve the ductility and dynamic loading resistance of construction materials.

65 The goal of this study was to apply Ca-alginate hydrogel for improving the ductility and
66 mechanical behavior of sandy soil. In this study, the Ca-alginate hydrogel was selected due to its
67 environmentally friendly properties [26,27]. The effects of reaction time, sodium alginate content,
68 and curing temperature on mechanical behaviors of hydrogel-impregnated sand were studied
69 through unconfined compression tests, falling head permeability tests, consolidated and
70 undrained triaxial tests, scanning electron microscopy, and durability tests.

71 **Materials and Methods**

72 *Sand*

73 Mississippi local sand was used in this study. The sieve analysis method was used to determine
74 the sand particle size distributions according to ASTM C136 [28]. The standard U.S. sieve was
75 used in this study. The sand particle distribution curve is shown in Figure 1. The coefficient of
76 uniformity (C_u) and gradation (C_c) were determined as 2.05 and 1.21, respectively. It was
77 classified as a poorly graded sand (SP) according to Unified Soil Classification System (USCS).

78 *Ca-alginate Hydrogel*

79 The Ca-alginate hydrogel was prepared from sodium alginate solution that mixing the solution
80 with CaCl_2 agents [29]. The sodium alginate was delivered as powder and the gel solution was
81 created when the powders were mixed with water. The sodium alginate powder used in this study
82 was supplied by ACRON (CAS No. 9005-38-3). The sodium alginate solutions were prepared in
83 DI water at room temperature. Four different sodium alginate contents (0.1%, 0.2%, 0.3% and
84 0.4% by weight of dry sand) were used for this study.

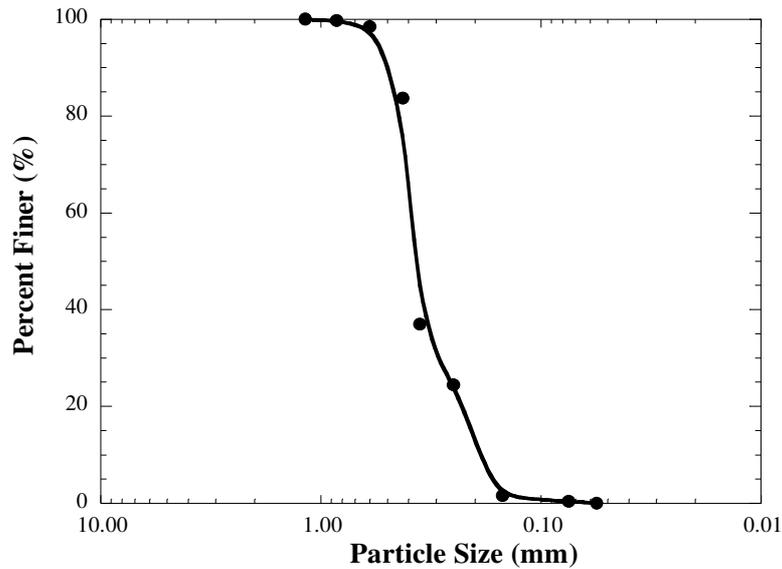


Figure 1. Particle distribution curve of Mississippi sand.

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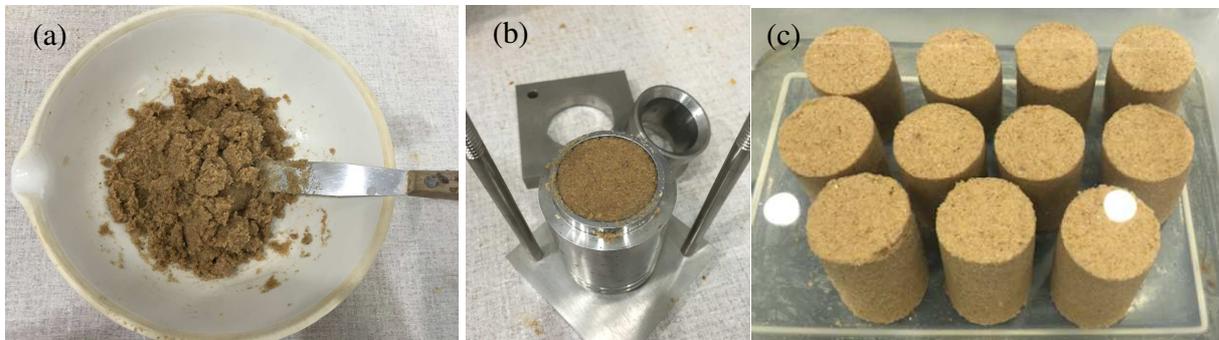
87

88 *Hydrogel-impregnated Sample Preparation*

89 All hydrogel-impregnated sand samples were prepared using the following sample preparation
 90 method. Sodium alginate solution (~20 mL) was mixed with sand (100 g) to a workable status at
 91 room temperature as shown in Figure 2 (a). The mixture was compacted in a mini compaction
 92 mold with a diameter of 33.0 mm (1.3 in.) and a height of 71.1 mm (2.8 in.) (Figure 2 (b)). After
 93 compaction, the sample was extruded out and merged into 0.5 M CaCl₂ solution as shown in
 94 Figure 2 (c). The CaCl₂ solution was used as ionic cross-linking agent with sodium alginate to
 95 form the Ca-alginate hydrogel. The formatted Ca-alginate hydrogel can cement the sand particles
 96 together and improve the mechanical performance of sand. Different reaction times (1, 3, 5, 7, 14,
 97 28 days) in the CaCl₂ solution were investigated to explore the optimum performance of
 98 hydrogel-impregnated sand. All testing samples were prepared in triplicate.

99 Four different curing conditions were selected to investigate the effect of curing temperature on
100 the properties of hydrogel-impregnated sand. After removing the samples from the reaction tank,
101 the hydrogel-impregnated sand was either (1) air-dried at room temperature (25⁰C) for 28 days,
102 (2) oven-dried in 50⁰C for 24 h, (3) oven-dried in 80⁰C for 24 h, or (4) oven-dried in 100⁰C for
103 24 h.

104



105

106

Figure 2. Images of sample preparation.

107

108 ***Unconfined Compression Test***

109 The hydrogel-impregnated samples for the unconfined compression test were cylinder-shaped
110 with 2H:1D ratio (diameter of 33.0 mm and height of 71.1 mm). The unconfined compression
111 test was conducted under strain-controlled conditions at a uniform loading rate of 1.5%/min in
112 accordance with ASTM D2166 [30].

113 ***Permeability Test***

114 The falling head permeability testing method was used to test the hydraulic conductivity of the
115 hydrogel-impregnated sand following ASTM D5084-16a [31]. For untreated sand, the constant
116 head permeability testing method was used following ASTM D2434-68 [32].

117 ***Consolidated and Undrained Triaxial Test***

118 Consolidation undrained triaxial compression tests were conducted under 100, 200 and 400 kPa
119 cell pressure at a constant axial strain rate of 1.0% strain/min. The tests terminated after the strain
120 reached 15%.

121 ***Durability tests***

122 Durability tests of hydrogel-impregnated sample were conducted in accordance with ASTM D
123 560 [33] for freeze-thaw cycles and ASTM D 559 [34] for wet-dry cycles. The UCS tests were
124 conducted on these samples after every 3 cycles.

125 ***Freeze-thaw***

126 Every freeze-thaw cycle began by introducing specimens in a freezing cabinet with constant
127 temperature of -23°C for 24 h. Next, the samples were placed in the moist room at temperature of
128 25°C and a relative humidity of 100% for 24 h. The number of freeze-thaw cycles was up to 12
129 times in this study. The mass loss after each freeze-thaw cycle were measured. After each freeze-
130 thaw cycle, the hydrogel-impregnated samples were thawed at 50°C for 24 h before testing.

131 ***Wet-dry***

132 Every wet-dry cycle began with oven drying for 24 h at 50°C. Then, specimens were immersed
133 underwater for 24 h at 25°C. The number of wet-dry cycles was up to 12 times in this study. The
134 mass loss after each wet-dry cycle were measured.

135 ***Scanning Electron Microscopy (SEM) Analysis***

136 SEM images were taken to observe the micro-scale connections between hydrogel and sand
137 particles. Selected samples including untreated sand and hydrogel-impregnated sand under wet

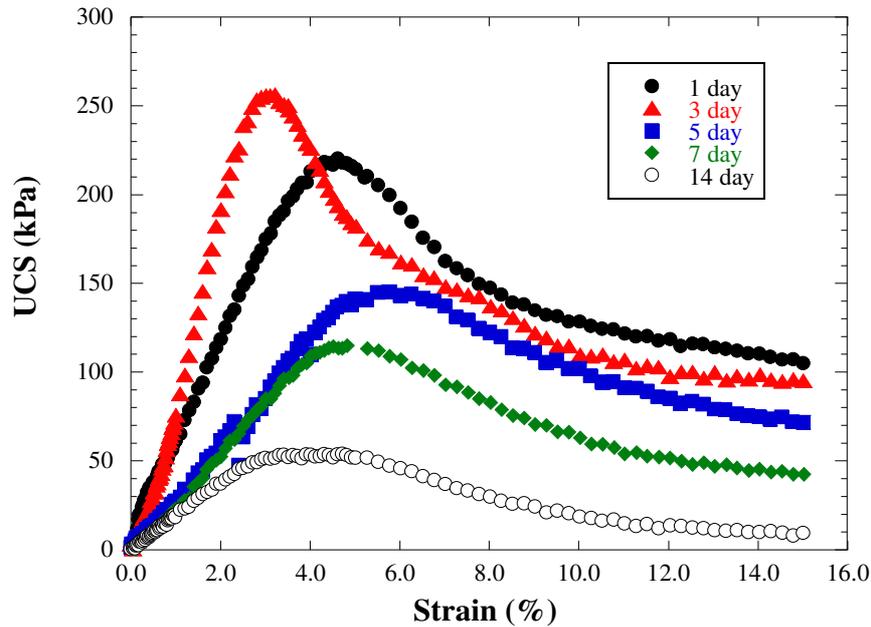
138 and dry conditions were mounted on the stubs with adhesive carbon conductive tabs. The
139 prepared samples were observed by secondary electron detection in SEM (TESCAN LYRA3).

140

141 **Results and Discussion**

142 *Effect of reaction time on strength improvement of hydrogel-impregnated sand*

143 The stress-strain relationship of hydrogel-impregnated sand with 0.4% sodium alginate content at
144 different reaction times (1 day, 3 day, 5 day, 7 day, and 14 days) in the CaCl₂ solution is shown
145 in Figure 3. The conventionally-reinforced sandy soil commonly exhibited a brittle behavior
146 [11,17]. In contrast, the failure strain of hydrogel-impregnated sand could reach up to 6%,
147 indicating a good ductility behavior. Meanwhile, all samples exhibited residual strength after the
148 peak strength, which demonstrated better elastic behavior. The UCS of hydrogel-impregnated
149 sand increased with reaction time up to 3 days, and then the UCS started to reduce. This could be
150 caused by the degradation or decrosslinking of hydrogel. Shoichet et al. [35] found that increased
151 exposure of calcium-crosslinked alginate to sodium citrate can result in decreased gel strength
152 because sodium citrate chelates calcium, thereby decrosslinking calcium alginate. Rowley et al.
153 [36] also indicated that the ionically crosslinked alginates lost its mechanical properties over
154 time due to an outward flux of crosslinking ions into the surrounding medium. The exchange
155 between divalent crosslinking ions (e.g., Ca²⁺) with monovalent ions from the surrounding
156 environment causes alginate hydrogels to degrade [37]. In this study, the sodium ions in the
157 solution may have degraded the Ca-alginate over time. Therefore, the study proposed the
158 optimum reaction time for Ca-alginate was 3 days.



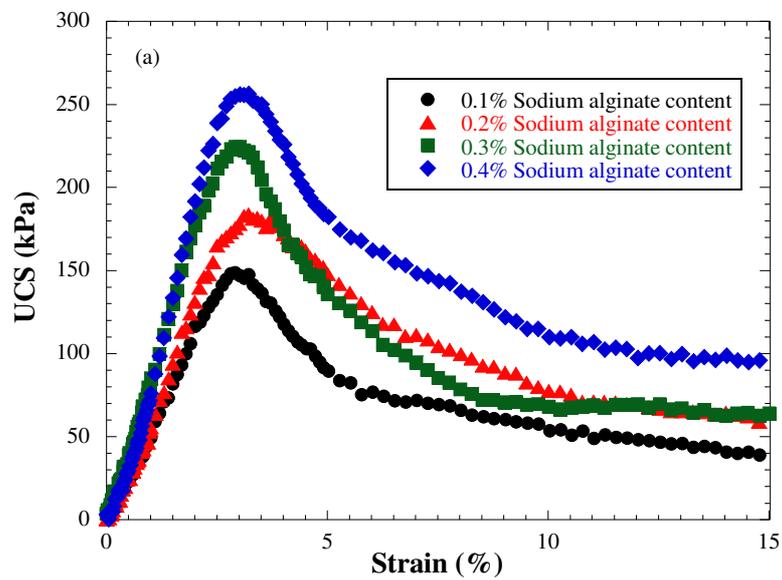
159

160 Figure 3. The UCS of hydrogel-impregnated sand with 0.4% sodium alginate content at different
 161 reaction times.

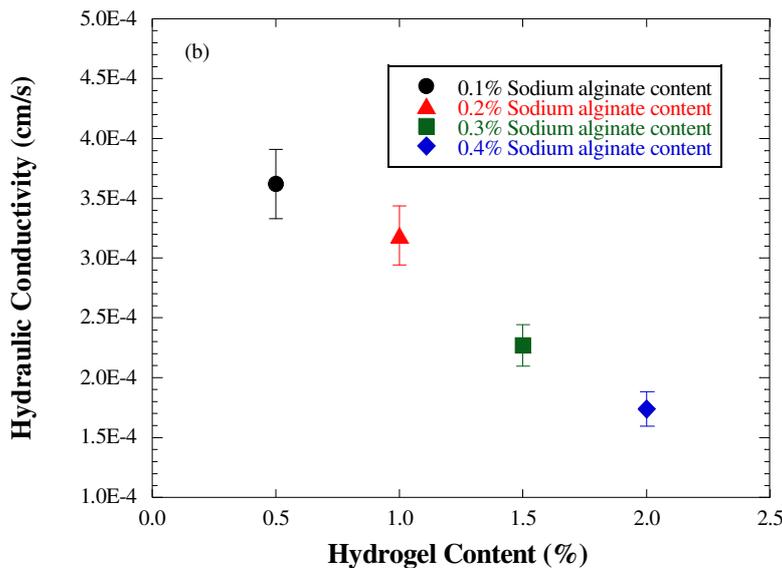
162 ***Effect of sodium alginate content on strength improvement of hydrogel-impregnated sand***

163 The stress-strain relationship of hydrogel-impregnated sand at different sodium alginate contents
 164 after 3 days of reaction time was shown in Figure 4 (a). It can be seen that the UCS of hydrogel-
 165 impregnated sand increased with the increase of sodium alginate content. The strength of
 166 hydrogel-impregnated sand at 0.4% sodium alginate content (260 kPa) was around two times
 167 higher than that at 0.1% sodium alginate content (140 kPa). Bu et al. [11] found that the UCS of
 168 the optimum lime-treated sand (15% by weight of dry sand) was 140 kPa, which is similar to
 169 hydrogel-impregnated sand with 0.1% sodium alginate. The UCS of hydrogel-impregnated sand
 170 was lower than that of cement-treated sand, but is still comparable with a lower percentage
 171 additive at 0.4%. Consoli et al. [38] reported that the UCS of 2% cement-treated sand was
 172 around 250 kPa which was similar to that of hydrogel-impregnated sand with 0.4% sodium
 173 alginate. Chang et al. [12] mixed different contents of gellan gum with sand to improve the

174 strength behavior of sand, and the results indicated that the UCS increased with gellan gum
 175 content. The UCS of 2% gellan gum-treated sand was around 180 kPa. Meanwhile, the residual
 176 strength of hydrogel-impregnated sand increased with the increase of sodium alginate content,
 177 and hydrogel-impregnated sand with 0.4% sodium alginate content had a residual strength of 100
 178 kPa. This is in agreement with the fact that the gellan gum contents had a positive effect on
 179 residual strength of gellan gum-treated sand [12].



180



181

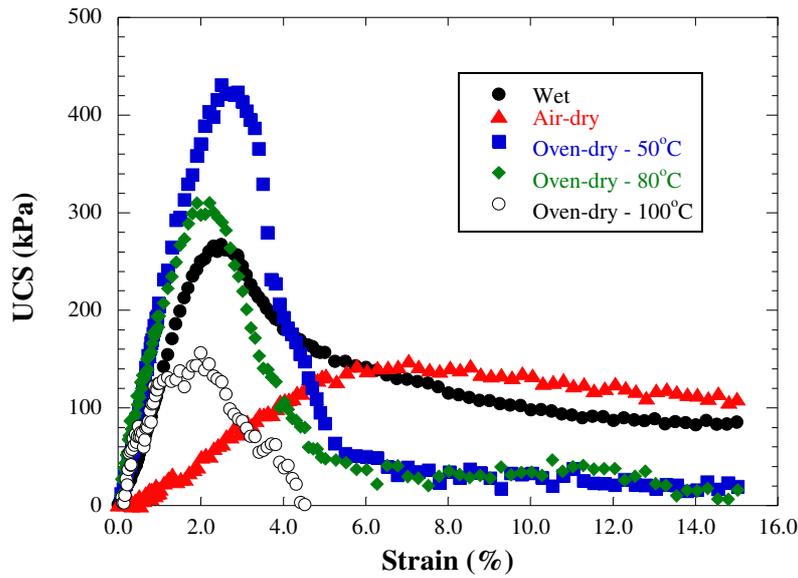
182 Figure 4. The effect of sodium alginate content on (a) stress-strain curve; (b) hydraulic
183 conductivity of hydrogel-impregnated sand.

184

185 The hydraulic conductivity of hydrogel-impregnated sand at different sodium alginate
186 contents was investigated in this study. As shown in Figure 4 (b), the higher the sodium alginate
187 content, the lower the hydraulic conductivity of hydrogel-impregnated sand. The hydraulic
188 conductivity decreased from an untreated condition ($\sim 10^{-2}$ cm/s) to hydrogel-impregnated sand
189 with 0.4% sodium alginate content ($\sim 1.8 \times 10^{-4}$ cm/s), indicating that the void spaces between
190 sand particles were filled and cemented by the inclusion of hydrogel.

191 ***Effect of curing temperature on strength improvement of hydrogel-impregnated sand***

192 Four different curing conditions were selected to investigate the effect of curing temperature on
193 the UCS of hydrogel-impregnated sand. Figure 5 shows the stress-strain curve of hydrogel-
194 impregnated sand with 0.4% sodium alginate content at different curing temperatures. The wet
195 condition means that the samples were tested without curing. It can be shown that the
196 performance of stress-strain hydrogel-impregnated sand was significantly affected by the curing
197 temperature. The highest UCS was around 430 kPa at 50⁰C oven-dried curing condition, and the
198 lowest one was 160 kPa at 100⁰C oven-dried curing condition. Chang et al. [12] studied the
199 strength behaviors of gellan gum-treated sand under different curing conditions. They reported
200 that the UCS of the air-dried sample was higher than that of the wet samples. This is related to
201 the remaining sodium ions in the wet sample degrading/decrosslinking the Ca-alginate over time
202 during the air-dry process over 28 days.



203

204 Figure 5. The stress-strain curve of hydrogel-impregnated sand under different curing conditions.

205

206 The failure modes of hydrogel-impregnated sand at different curing temperatures are
 207 shown in Figure 6. In the case of the wet condition, the sample did not completely break after
 208 peak strength and was still cemented by hydrogel, achieving a residual strength of 90 kPa at 15%
 209 strain. The failure sample exhibited a shear zone failure mode as shown in Figure 6 (a). An X-
 210 shape shear band and several small cracks appeared in the failure sample. Asghari et al. [39]
 211 found this similar failure mode for lime-cemented sand. Figure 6 (b) shows the failure sample
 212 after 28 days of air-dried conditions. The sample failed in a barreling or drum shape and no
 213 cracks were identified, indicating a uniform status of the sample. This is consistent with the
 214 result from the stress-strain curve in Figure 5. The air-dried sample presented a superior ductility,
 215 and the failure strain was around 7%, with 100 kPa residual strength at 15% strain. Plé and Lê
 216 [40] reported that the fiber-reinforced silty clay soil presented a drum shape failure mode, which
 217 indicated that the strain localization is prevented by the presence of the fibers. When the

218 hydrogel-impregnated sample was cured at higher temperatures, the ductility and strength
219 significantly changed. Figure 6 (c) shows the failure sample at 50°C curing temperature, and the
220 sample exhibited a shear failure mode. Meanwhile, the peak strength reached around 430 kPa,
221 but the stress reduced dramatically after peak stress. The residual strength was around 20 kPa at
222 15% failure strain. When the curing temperature increased to 100°C, the failure strain reduced to
223 around 2.0% and the peak stress reduced to 160 kPa. The sample was brittle and weak with no
224 residual stress. The top of the sample was broken and the failed portion became loose sand as
225 shown in Figure 6 (d).

226



227

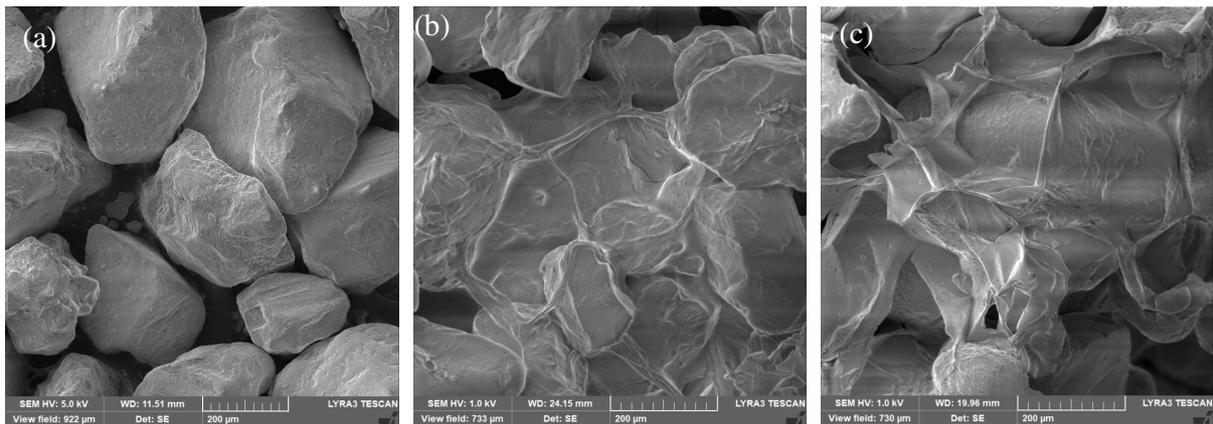
228 Figure 6. The failure mode of hydrogel-impregnated sand under (a) wet condition; (b) air-dry
229 condition; (c) 50°C oven-dried condition; (d) 100°C oven-dried condition.

230

231 The SEM images of untreated sand and hydrogel-impregnated sand are shown in Figure 7.
232 The untreated sand did not have cohesion, and the shape of untreated sand was irregular as
233 shown in Figure 7 (a). The image of hydrogel impregnated sand under wet condition is shown in
234 Figure 7 (b). The hydrogel uniformly warped the sand particles and cemented them together,
235 resulting in the reduction of void space. This is consistent with the results of hydraulic
236 conductivity in Figure 4 (b). The hydrogel connection shrank in size after the hydrogel-

237 impregnated sample cured at 50°C as shown in Figure 7 (c). However, the strength of 50°C dried
238 sample increased which may be because the hydrogel becomes a solid material during the drying
239 process.

240



241

242 Figure 7. SEM images of a) Untreated Mississippi sand; b) hydrogel-impregnated sand under wet
243 condition; c) hydrogel-impregnated sand under dry condition (50°C oven dried).

244

245 ***Shear strength of hydrogel-impregnated sand***

246 The natural repose angle of Mississippi sand is shown in Figure 8 (a), which was around 32°.

247 The consolidated and undrained triaxial tests were conducted on hydrogel-impregnated sand with

248 0.4% sodium alginate, and three different confining pressures (100 kPa, 200 kPa and 400 kPa)

249 were selected. Figure 8 (b) shows the Mohr circle curves and failure envelopes obtained from

250 triaxial compression strength tests on hydrogel-impregnated samples. The test results show that

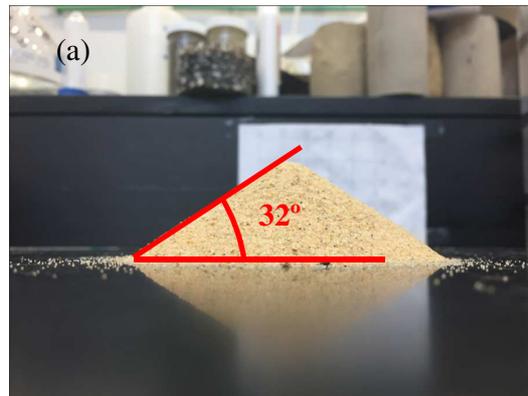
251 the cohesion and friction angle of hydrogel-impregnated sand were 150 kPa and 16°, respectively.

252 Sandy soil is well-known as cohesionless soil. The gelation connection provided by hydrogel

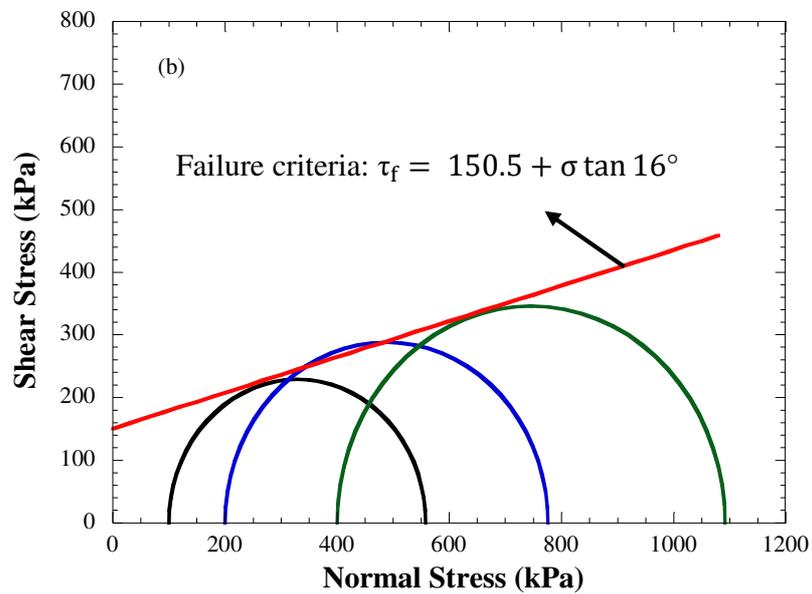
253 enhanced the connection between sand particles and improved the cohesion of sandy soil. Li et al.

254 [41] investigated the shear strength of MICP-treated sand (0.18 M Ca) and found that the
255 cohesion of MICP-treated sand increased to 20 kPa, which was much lower than that of
256 hydrogel-impregnated sand, and the friction angle of MICP-treated sand was similar to that of
257 hydrogel-impregnated sand.

258



259



260

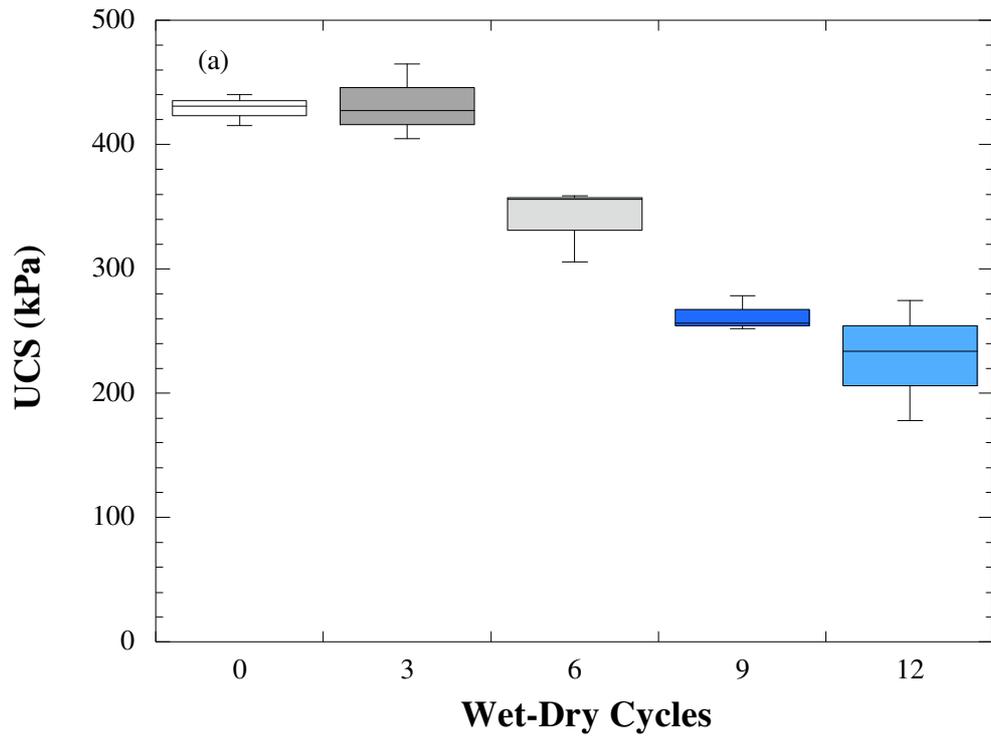
261 Figure 8. (a) The natural repose angle of Mississippi sand; (b) Mohr circle and failure envelope
262 of hydrogel-impregnated sand with 0.4% sodium alginate.

263

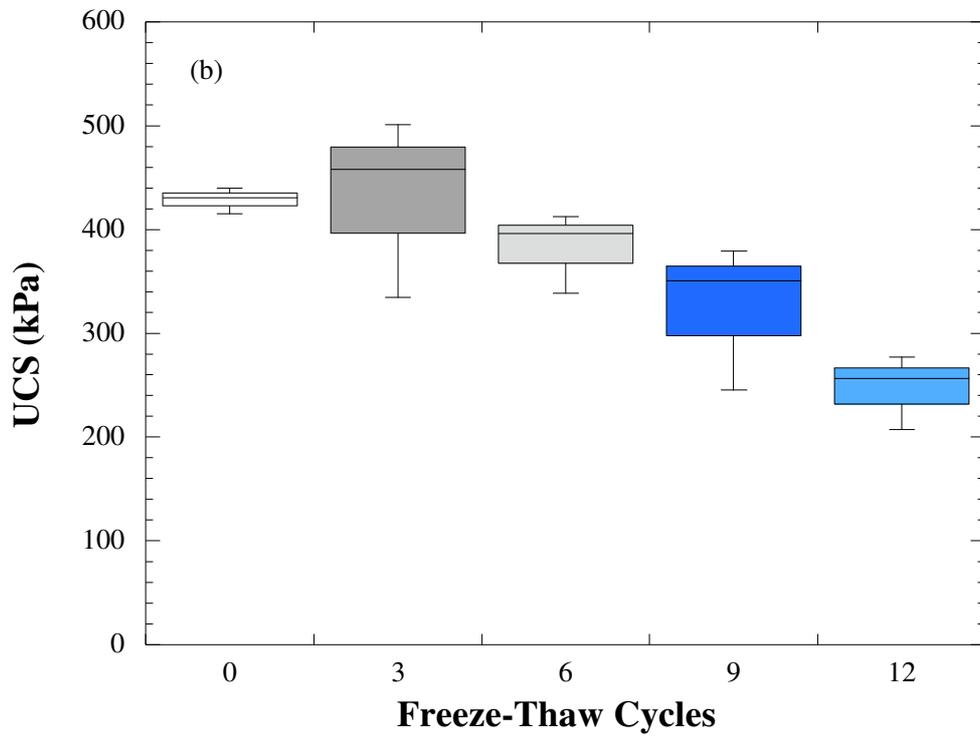
264 **Durability Test on Hydrogel-impregnated Sand**

265 The durability of hydrogel-impregnated sand was tested through wet-dry and freeze-thaw cycles.
266 The effect of wet-dry and freeze-thaw cycles on the UCS of hydrogel-impregnated sand is shown
267 in Figure 9. The samples used for the durability tests were hydrogel-impregnated samples with
268 0.4% sodium alginate content at a curing temperature of 50°C. It was found that no significant
269 difference for the samples subjected to 3 wet-dry or freeze-thaw cycles. After 3 cycles, the UCS
270 of hydrogel-impregnated sand started to decrease with the increase of wet-dry or freeze-thaw
271 cycles. After 12 wet-dry or freeze-thaw cycles, 60% of the original UCS of the hydrogel-
272 impregnated sand still remained. Kampala et al. [42] used fly-ash to reinforce clay soil, and the
273 UCS reduced over 50% after 6 wet-dry cycles. Chang et al. [12] studied the strength behavior of
274 2% gellan gum-treated sand and found that the UCS dropped dramatically from 435 kPa (dry
275 condition) to 45 kPa (re-submerged condition). This result indicated that the gellan gum-treated
276 sand was not recoverable once the gellan gum gel is condensed through dehydration. Eskişar et
277 al. [43] found that cement-treated clay reduced 50% in strength after 5 freeze-thaw cycles. With
278 these studies, it is indicated that the Ca-alginate hydrogel has a superior durability performance.

279 However, the natural environment contains many micro-organisms that may have an
280 impacted the stability of Ca-alginate impregnated sand. Studies have concluded that the
281 decreased mechanical strength of Ca-alginate is due to the entrapped growing microorganism
282 [44,45]. Therefore, the effects of microorganisms on the mechanical properties of hydrogel-
283 impregnated samples need to be studied further.



284



285

286 Figure 9. The effect of wet-dry (a) and freeze-thaw (b) cycles on unconfined compressive
 287 strength of hydrogel-impregnated sand

288

289 **Conclusions**

290 As an environmentally friendly material used for sandy soil improvement, the hydrogel-
291 impregnated sand achieved a relatively high strength even at low concentrations of hydrogel. The
292 effects of reaction time, sodium alginate content, and curing temperature on mechanical
293 behaviors of hydrogel-impregnated sand were studied through unconfined compression tests,
294 falling head permeability tests, consolidated and undrained triaxial tests, scanning electron
295 microscopy, and durability tests. The optimum reaction time and curing temperature of hydrogel-
296 impregnated sand were found to be 3 days and 50°C, respectively. The UCS tended to increase
297 with more sodium alginate content, but hydraulic conductivity decreased with the sodium
298 alginate content. The UCS of hydrogel-impregnated sand at 0.4% sodium alginate content
299 reached 430 kPa and presented to be the optimum mixture ratio. The hydrogel-impregnated sand
300 showed a significant improvement in the cohesion of the sand particles. The results showed that
301 the cohesion and friction angle of hydrogel-impregnated sand at 0.4% sodium alginate content
302 were 150 kPa and 16°, respectively. In addition, the stress-strain curves of hydrogel-impregnated
303 sand indicated that the ductility of hydrogel-impregnated sand was significantly improved
304 compared to traditional cementitious methods. Moreover, the results of durability tests indicated
305 that approximately 60% of original UCS of hydrogel-impregnated sand still remained after 12
306 wet-dry and freeze-thaw cycles.

307 **Acknowledgments**

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311 **References**

312 [1] N. Latifi, S. Horpibulsuk, C.L. Meehan, M.Z. Abd Majid M, M.M. Tahir, E.T. Mohamad.
313 Improvement of problematic soils with biopolymer—an environmentally friendly soil stabilizer.
314 J. Mater. Civ. Eng., 29 (2) (2016), pp. 04016204.

315 [2] R. Horn, H. Domżzał, A. Słowińska-Jurkiewicz, C. Van Ouwerkerk. Soil compaction
316 processes and their effects on the structure of arable soils and the environment. Soil Till. Res.,
317 35(1-2) (1995), pp. 23-36.

318 [3] M.W. Bo, A. Arulrajah, S. Horpibulsuk, M. Leong, M.M. Disfani. Densification of Land
319 Reclamation Sands by Deep Vibratory Compaction Techniques. J. Mater. Civ. Eng., 26 (8)
320 (2013), pp. 06014016.

321 [4] Y. Chen. Study of Ground Treatment on Improvement of Pile Foundation Response in
322 Liquefiable Soils. Geotech. Geol. Eng., 35(5) (2017), pp.2219-2226.

323 [5] S.K. Dash, M. Hussain. Lime stabilization of soils: reappraisal. Journal of materials in civil
324 engineering, 24(6) (2011), pp. 707-714.

325 [6] S. Horpibulsuk, C. Phetchuay, A. Chinkulkijniwat. Soil stabilization by calcium carbide
326 residue and fly ash. J. Mater. Civ. Eng., 24(2) (2011), pp. 184-193.

327 [7] N. Cristelo, S. Glendinning, L. Fernandes, A.T. Pinto. Effects of alkaline-activated fly ash
328 and Portland cement on soft soil stabilisation. Acta Geotech., 8(4) (2013), pp. 395-405.

- 329 [8] S.H. Chew, A.H.M. Kamruzzaman, F.H. Lee. Physicochemical and engineering behavior of
330 cement treated clays. *J. Geotech. Geoenviron. Eng.*, 130(7) (2004), pp. 696-706.
- 331 [9] M. Al-Mukhtar, A. Lasledj, J.F. Alcover. Behaviour and mineralogy changes in lime-treated
332 expansive soil at 20 C. *Appl. Clay Sci.*, 50(2) (2010), pp. 191-198.
- 333 [10] C.A. Anagnostopoulos. Strength properties of an epoxy resin and cement-stabilized silty
334 clay soil. *Appl. Clay Sci.*, 114 (2015), pp. 517-529.
- 335 [11] C. Bu, K. Wen, S. Liu, U. Ogbonnaya, L. Li. Development of bio-cemented constructional
336 materials through microbial induced calcite precipitation. *Mater. Struct.*, 51(1) (2018), pp. 30.
- 337 [12] I. Chang, A.K. Prasadhi, J. Im, G.C. Cho. Soil strengthening using thermo-gelation
338 biopolymers. *Constr. Build. Mater.*, 77 (2015), pp. 430-438.
- 339 [13] J.T. DeJong, B.M. Mortensen, B.C. Martinez, D.C. Nelson. Bio-mediated soil improvement.
340 *Ecol. Eng.*, 36(2) (2010), pp. 197-210.
- 341 [14] G. Blanck, O. Cuisinier, F. Masrouri. Soil treatment with organic non-traditional additives
342 for the improvement of earthworks. *Acta Geotech.*, 9(6) (2014), pp. 1111-1122.
- 343 [15] N. Latifi, A. Marto, A. Eisazadeh. Physicochemical behavior of tropical laterite soil
344 stabilized with non-traditional additive. *Acta Geotech.*, 11(2) (2015), pp. 433-443.
- 345 [16] D. Bernardi, J.T. DeJong, B.M. Montoya, B.C. Martinez. Bio-bricks: Biologically cemented
346 sandstone bricks. *Constr. Build. Mater.*, 55 (2014), pp. 462-469.

- 347 [17] Q. Zhao, L. Li, C. Li, M. Li, F. Amini, H. Zhang. Factors affecting improvement of
348 engineering properties of MICP-treated soil catalyzed by bacteria and urease. *J. Mater. Civ. Eng.*,
349 26(12) (2014), pp. 04014094.
- 350 [18] M. Li, K. Wen, Y. Li, L. Zhu. Impact of Oxygen Availability on Microbially Induced
351 Calcite Precipitation (MICP) Treatment. *Geomicrobiol. J.*, 35(1) (2018), pp. 15-22.
- 352 [19] K. Wen, C. Bu, S. Liu, Y. Li, L. Li. Experimental investigation of flexure resistance
353 performance of bio-beams reinforced with discrete randomly distributed fiber and bamboo.
354 *Constr. Build. Mater.*, 176 (2018), pp. 241-249.
- 355 [20] I. Chang, J. Im, G.C. Cho. Geotechnical engineering behaviors of gellan gum biopolymer
356 treated sand. *Can. Geotech. J.*, 53(10) (2016), pp. 1658-1670.
- 357 [21] D. Seliktar. Designing cell-compatible hydrogels for biomedical applications. *Science*,
358 336(6085) (2012), pp. 1124-1128.
- 359 [22] Y.S. Zhang, A. Khademhosseini. Advances in engineering hydrogels. *Science*, 356(6337)
360 (2017), pp. eaaf3627.
- 361 [23] X. Zhao (2014). Multi-scale multi-mechanism design of tough hydrogels: building
362 dissipation into stretchy networks. *Soft Matter*, 10(5) (2014), pp. 672-687.
- 363 [24] J.P. Gong, Y. Katsuyama, T. Kurokawa, Y. Osada. Double - network hydrogels with
364 extremely high mechanical strength. *Adv. Mater.*, 15(14) (2003), pp. 1155-1158.
- 365 [25] J.Y. Sun, X. Zhao, W.R. Illeperuma, O. Chaudhuri, K.H. Oh, D.J. Mooney, J.J. Vlassak, Z.
366 Suo. Highly stretchable and tough hydrogels. *Nature*, 489(7414) (2012), pp. 133.

- 367 [26] K. Dey, P. Roy. Degradation of chloroform by immobilized cells of *Bacillus* sp. in calcium
368 alginate beads. *Biotechnol. Lett.*, 33 (2011), pp. 1101–1105.
- 369 [27] J. Wang, A. Mignon, D. Snoeck, V. Wiktor, S. Van Vliergerghe, N. Boon, N. De Belie.
370 Application of modified-alginate encapsulated carbonate producing bacteria in concrete: a
371 promising strategy for crack self-healing. *Front. Microbiol.*, 6 (2015), pp. 1088.
- 372 [28] ASTM C136/C136M-14 (2014) Standard Test Method for Sieve Analysis of Fine and
373 Coarse Aggregates, ASTM International, West Conshohocken, PA.
- 374 [29] K.Y. Lee, D.J. Mooney. Alginate: properties and biomedical applications. *Prog. Polym. Sci.*,
375 37(1) (2012), pp. 106-126.
- 376 [30] ASTM D2166/D2166M-13 (2013) Standard test method for unconfined compressive
377 strength
378 of cohesive soil. ASTM International, West Conshohocken, PA.
- 379 [31] ASTM-D5084-16a (2016) Standard test methods for measurement of hydraulic conductivity
380 of saturated porous materials using a flexible wall permeameter. ASTM International, West
381 Conshohocken, PA.
- 382 [32] ASTM D2434-68 (2006) Standard Test Method for Permeability of Granular Soils
383 (Constant Head). ASTM International, West Conshohocken, PA.
- 384 [33] ASTM-D560 (2016) Standard Test Methods for Freezing and Thawing Compacted Soil
385 Cement Mixtures. ASTM International, West Conshohocken, PA.
- 386 [34] ASTM-D559 (2015) Standard Test Methods for Wetting and Drying Compacted Soil-
387 Cement

388 Mixtures. ASTM International, West Conshohocken, PA.

389 [35] M.S. Shoichet, R.H. Li, M.L. White, S.R. Winn. Stability of hydrogels used in cell
390 encapsulation: An in vitro comparison of alginate and agarose. *Biotechnol. and bioeng.*, 50(4)
391 (1996), pp. 374-381.

392 [36] J.A. Rowley, G. Madlambayan, D.J. Mooney. Alginate hydrogels as synthetic extracellular
393 matrix materials. *Biomaterials*, 20(1) (1999), pp. 45-53.

394 [37] J.C. Breger, B. Fisher, R. Samy, S. Pollack, N.S. Wang, I. Isayeva. Synthesis of “click”
395 alginate hydrogel capsules and comparison of their stability, water swelling, and diffusion
396 properties with that of Ca^{+2} crosslinked alginate capsules. *J. Biomed. Mater. Res. B*, 103(5)
397 (2015), pp. 1120-1132.

398 [38] N.C. Consoli, R.C. Cruz, M.F. Floss, L. Festugato. Parameters controlling tensile and
399 compressive strength of artificially cemented sand. *J. Geotech. Geoenviron. Eng.*, 136(5) (2009),
400 759-763.

401 [39] E. Asghari, D.G. Toll, S.M. Haeri. Triaxial behaviour of a cemented gravely sand, Tehran
402 alluvium. *Geotech. Geol. Eng.*, 21(1) (2003), pp. 1-28.

403 [40] O. Plé, T.N.H. Lê. Effect of polypropylene fiber-reinforcement on the mechanical behavior
404 of silty clay. *Geotext. Geomembranes*, 32 (2012), pp. 111-116.

405 [41] M. Li, L. Li, U. Ogbonnaya, K. Wen, A. Tian, F. Amini. Influence of fiber addition on
406 mechanical properties of MICP-treated sand. *J. Mater. Civ. Eng.*, 28(4) (2015), pp. 04015166.

407 [42] A. Kampala, S. Horpibulsuk, N. Prongmanee, A. Chinkulkijniwat. Influence of wet-dry
408 cycles on compressive strength of calcium carbide residue–fly ash stabilized clay. *J. Mater. Civ.*
409 *Eng.*, 26(4) (2013), pp. 633-643.

410 [43] T. Eskişar, S. Altun, İ. Kalıpcılar. Assessment of strength development and freeze–thaw
411 performance of cement treated clays at different water contents. *Cold Reg. Sci. Technol.*, 111
412 (2015), pp. 50-59.

413 [44] K. Dey, P. Roy. Degradation of chloroform by immobilized cells of *Bacillus* sp. in calcium
414 alginate beads. *Biotechnol. Lett.*, 33 (2011), pp. 1101–1105.

415 [45] H. Eikmeier, H.J. Rehm. Stability of calcium-alginate during citric acid production of
416 immobilized *Aspergillus niger*. *Appl. Microbiol. Biotechnol.*, 26(2) (1987), pp.105-111.

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