Interactions between aquatic plants and turbulent flow: A field study using stereoscopic PIV

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A stereoscopic particle image velocimetry (PIV) system for use in shallow (~ 0.5 m deep) 8 rivers was developed and deployed in the Urie River, Scotland, to study the interactions 9 between turbulent flow and a Ranunculus penicillatus plant patch in its native environ-10 ment. Statistical moments of the velocity field were calculated utilising a new method 11 of reducing the contribution of measurement noise, based on the measurement redun-12 dancy inherent to the stereoscopic PIV method. Reynolds normal and shear stresses, 13 their budget terms, and higher order moments of the velocity probability distribution 14 in the wake of the plant patch were found to be dominated by the presence of a free 15 shear layer induced by the plant drag. Plant motion, estimated from the PIV images, 16 was characterised by travelling waves that propagate along the plant with a velocity sim-17 ilar to the eddy convection velocity, suggesting a direct coupling between turbulence and 18 the plant motion. The characteristic frequency of the plant velocity fluctuations (~ 1 Hz) 19 may suggest that the plant motion is dominated by large eddies with scale similar to the 20 flow depth or plant length. Plant and fluid velocity fluctuations were, in contrast, found 21 to be strongly correlated only over a narrow ($\sim 30 \text{ mm}$) elevation range above the top of 22 the plant, supporting a contribution of the shear layer turbulence to the plant motion. 23 Many aspects of flow-aquatic plant interactions remain to be clarified, and the newly 24 developed stereoscopic field PIV system should prove valuable in future studies. 25

26 Key words: ...

27 1. Introduction

Aquatic plants play a vital role in the management and healthy functioning of river 28 ecosystems. They provide habitat, refuge, and food for periphyton, invertebrates, and 29 fish; they produce oxygen and sink carbon through photosynthesis; they regulate sedi-30 ment transport and mixing, and they contribute to hydraulic resistance (e.g. Naden et al. 31 2006; Bornette & Puijalon 2011; Folkard 2011b; Nepf 2012). Understanding of these pro-32 cesses is important for the successful management of river systems (mitigating flood risk, 33 preserving biodiversity, maintaining water quality) but is still limited by a lack of fun-34 damental knowledge of the interactions between plants and flowing water. One of the reasons for this is that flow-plant interactions are scale dependant, covering a wide range 36 of scales from the sub-leaf to the plant patch and larger scales, and are thus controlled by 37 several complex and interlinked phenomena such as turbulence, viscous and pressure drag 38

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forces, plant biomechanical properties, and plant motion (Nikora 2010). Another reason 30 is that experimental measurement of these phenomena remains challenging. Many of the 40 experimental investigations on aspects of flow-plant interactions have been carried out 41 in laboratory flumes using artificial plant replicas or plant surrogates (e.g. Ghisalberti & 42 Nepf 2002; Nezu & Sanjou 2008; Siniscalchi et al. 2012), or using real plants attached to 43 the bed in some artificial way (e.g. Sand-Jensen 2003; O'Hare et al. 2007; Siniscalchi & 44 Nikora 2012). Although these studies allow systematic manipulation of flow conditions 45 and deployment of a full array of experimental technologies, it remains an open question 46 as to whether they are truly representative of real plants in their natural habitats. A 47 number of field studies have also been carried out (e.g. Koehl & Alberte 1988; Sand-48 Jensen & Mebus 1996; Green 2005; Naden et al. 2006; Sukhodolova & Sukhodolov 2012). 49 These studies, however, inevitably resort to point velocity measurement techniques (of-50 ten involving only time averaged velocities) which miss much of the detailed structure of 51 the flow field. 52

In the study reported here, the need for more extensive field data on flow-aquatic 53 plant interactions is addressed by developing a stereoscopic PIV system for field use and 54 deploying it in the Urie River, Scotland. The PIV technique has been previously used 55 outside of a laboratory (e.g. Nimmo Smith et al. 2002; Zhu et al. 2006; Tritico et al. 56 2007; Katija & Dabiri 2008; Liao et al. 2009). This study, however, is the first time the 57 stereoscopic PIV method has been used in the field, allowing all three components of the 58 velocity vector to be captured. The system is utilised to study the interactions between 59 river turbulence and the motion of a Ranunculus penicillatus plant patch in its natural 60 61 environment.

The structure of the paper is as follows. First, the design of a stereoscopic PIV system 62 that can be deployed in small rivers (~ 0.5 m flow depth) is discussed, including system 63 calibration, analysis algorithms, and a new method of reducing the contribution of mea-64 surement errors to certain velocity statistics. Second, features of the field site selected 65 for the study are identified, and measurement errors are analysed. Third, statistics of the 66 flow field in the wake of the *Ranunculus* plant patch are evaluated including terms of 67 the Reynolds stress budget equation, spectra, and convection velocity. Fourth, statistics 68 of the plant motion are evaluated along with correlations between turbulence and plant 69 movement. Finally, potential interaction mechanisms between the plants and the flow are 70 discussed. 71

72 2. In-situ stereoscopic PIV system

The in-situ stereoscopic PIV system was designed to utilise existing components from 73 a custom-made laboratory PIV system including the laser (Oxford Lasers Nano-L-50/100 74 PIV, twin Nd:YAG, 100 mJ at 50 Hz) and cameras (Dalsa 4M60, CMOS, 2352×1728 75 pixels at 60 frames per second, 7.4 micron pixel pitch, 60% effective fill factor, 532 nm 76 bandpass optical filter, 60 mm lens at f/5.6) and direct to disk image recording setup 77 $(4 \times 7200 \text{ rpm SATA disks in RAID 0 per camera)}$. At the core of the design is a glass bot-78 tomed 'boat' shaped structure which sits at the water surface and allows a pair of cameras 79 and the laser light sheet stable optical access through the fluctuating water surface of the 80 river (figure 1). The streamlined design of the 'boat' limits the disturbed region of the 81 flow-field to a thin boundary layer near the water surface estimated to be approximately 82 5 mm thick (based on previous experience with similar structures and approximate es-83 timates using conventional relationships). The 'boat' incorporates a trapezoidal shaped 84 water prism (e.g. Prasad 2000) to minimise both optical distortion caused by refraction 85 and internal reflections that occur at the water-glass-air interfaces. The 'boat', cameras, 86

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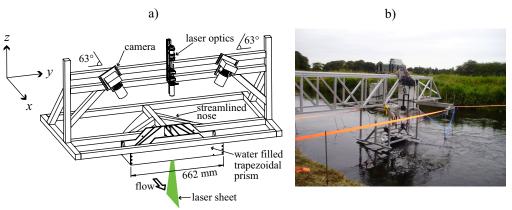


FIGURE 1. Schematic of glass bottomed 'boat' attached to camera and laser mount sub-frame (a). System deployed in the Urie River, Scotland (b).

and laser optics sit on a rigid sub-frame that allows the cameras and laser optics to be 87 aligned, focussed and calibrated in the laboratory prior to field deployment. In the field, 88 the sub-frame is attached to a specially designed frame (bridge) and carriage assembly 89 that allows the PIV system to be traversed in the streamwise (0.5 m) and transverse 90 (5.0 m) directions. The bridge is constructed of aluminium extrusions (Kanya PVS), it 91 spans 7.5 m, weighs 150 kg, and is designed so that at least one end of the bridge is 92 anchored on the river bank. The other end of the bridge can be supported mid river 93 on stainless steel poles with tension straps tied to the far river bank to ensure stability. 94 The turbulent wakes created by the bridge support elements are well clear of the mea-95 surement area (figure 1b). The laser, cameras and computer are powered by a portable 96 5 kVA generator. Seeding (conifer pollen, 60-80 micron diameter, 800-1000 kg/m³ den-97 sity) is mixed with water at a concentration of 100 g/l and injected into the river by a 98 pump approximately 5 m upstream of the test section at a solids rate of 100 grams per 99 minute. The entire bridge, carriage, and laser and camera assembly can be installed at a 100 field site by an 8 person team in around 7 hours. Disassembly is faster (around 2 hours) 101 leaving several hours for measurements during a single day deployment. The orientation 102 of our coordinate system is shown in figure 1. We will refer to the x, y, and z axis and 103 their associated velocity components u, v, and w as the nominal streamwise, transverse 104 and bed normal (or vertical) directions and velocities, respectively. In practice, the laser 105 light sheet was aligned visually to be parallel to the local mean flow direction by making 106 use of the visible stream of tracer particles injected upstream. 107

A stereoscopic camera configuration was selected because it offers a number of bene-108 fits over a single orthogonal camera setup. Firstly, all three components of the velocity 109 vector are resolved compared to just two components for a single camera configuration. 110 The additional velocity component provides valuable information on the structure and 111 dynamics of the flow field, particularly in the highly three-dimensional flow regions near 112 the bed of open channels and in the wake of aquatic plants. Secondly, the stereoscopic 113 configuration allows all cameras and laser optics to be placed above the river surface. 114 This minimises the disturbance to the flow field and limits camera vibration which can 115 introduce additional error into the velocity measurements. The need to waterproof the 116 camera and laser components is also removed. Thirdly, perspective errors which occur 117 in single camera systems due to the unresolved out of plane velocity component (Raffel 118 et al. 2007) are eliminated by the stereoscopic configuration. Finally, by taking advantage 119 of the redundancy inherent in stereoscopic PIV, some velocity statistics can be calculated 120

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with a significantly reduced contribution of random measurement noise. In the following section, details of our implementation of the stereoscopic PIV method are outlined, including: calibration and stereoscopic reconstruction, cross correlation algorithms, method of extracting the velocity of plant motion, and analysis of measurement errors.

2.1. Stereoscopic PIV calibration

Our stereoscopic PIV implementation is based on the 'mapping' method introduced by 126 Willert (1997), where cross correlation is performed on images that have been 'dewarped' 127 to obtain a constant magnification across the image. The 2-component vector fields from 128 a pair of cameras are subsequently combined to reconstruct the three-component velocity 129 field. Critical to both the image dewarping and velocity field reconstruction steps is a 130 function which relates three-dimensional (x, y, z) - streamwise, transverse, bed-normal re-131 spectively) 'world' coordinates to corresponding two-dimensional image coordinates. To 132 obtain it, we use a pinhole camera model (e.g. Calluaud & David 2004) combined with a 133 2-media refraction model (neglecting the contribution of the glass elements of the water 134 prism) based on Maas (1996) and a misalignment correction based on Wieneke (2005). 135 In total, 13 model parameters need to be estimated for each camera using a calibra-136 tion procedure, including four intrinsic camera parameters (f_x, f_y, i_0, j_0) , six extrinsic 137 camera parameters (α , β , γ , t_x , t_y , t_z), and three parameters for the refraction model 138 $(\alpha_g, \beta_g, t_{zg})$. Three additional parameters (α_m, β_m, t_m) apply to all cameras and are 139 used to correct any misalignment between the laser light sheet and the calibration target. 140 Here f_x and f_y are camera focal lengths, i_0 and j_0 are image origin coordinates, α , β , γ 141 and t_x, t_y, t_z are the three Euler rotation angles and three translations, respectively, that 142 define the position and viewing direction of the camera. The refraction model parame-143 ters α_q , β_q and t_{zq} are two rotation angles and one translation that give the position 144 and orientation of the water-air interface, while misalignment parameters α_m , β_m and 145 t_m map the light sheet plane onto the calibration plane. Other parameters are available 146 to incorporate lens distortions or to incorporate the refraction caused by the glass win-147 dows of the 'prism boat', but these parameters are not used in this study as they were 148 found to not improve the calibration. The calibration procedure is carried out in a lab-149 oratory tank after final alignment and focussing of the cameras. The intrinsic, extrinsic, 150 and refraction parameters are estimated for each camera based on a set of images of a 151 two-sided calibration plate (3 mm diameter dots spaced at 20 mm) which is translated 152 to different positions using a precision machined baseplate. The calibration images pro-153 vide a set of point coordinates (the centres of each dot on the calibration plate image) 154 and corresponding world coordinates (based on the known calibration plate geometry) 155 allowing the model parameters to be optimised using an iterative least square fit. Finally, 156 the misalignment correction parameters are estimated from the experimental PIV images 157 by ensemble cross correlation between the dewarped images from the first and second 158 cameras (Wieneke 2005). In this way, the precise position of the light sheet relative to 159 the cameras does not need to be fixed in the laboratory and some adjustment in the field 160 is possible (as long as the light sheet remains within the camera depth of field). 161

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2.2. Cross correlation algorithm

A detailed description of our cross correlation algorithm and evaluation of its performance
 is available in Cameron (2011). Some modifications were necessary to optimise for the
 field PIV images; these are described in this section.

Our PIV algorithm can be classed as an iterative deformation method (IDM) with windowed Fourier transform based cross correlation. Two key features of the algorithm, which directly influence measurement noise, measurement resolution, number of outliers,

and the number of iterations required to reach convergence are 1) the size and weighting 169 of the interrogation regions (image subsections used for cross correlation analysis, e.g. 170 Raffel et al. 2007), and 2) the low pass filtering of the velocity field after each iteration. To 171 analyse the field PIV images, which have a scale factor of 12 pixels/mm, we have selected 172 Blackman weighted 96×96 pixel (8×8 mm) interrogation regions (BL96) with a 12 pixel 173 (1 mm) grid spacing, and a low pass filter based on a windowed sinc function (Sinc2.5). 174 The modulation transfer function (MTF) for this algorithm (IDM-BL96-Sinc2.5) has 175 been estimated following Astarita (2007) and is given in figure 2. The MTF reflects 176 the spatial averaging (low-pass filtering) of the velocity field associated with the cross 177 correlation algorithm. For a MTF value of 0.9, figure 2 indicates the cut-off wavelength 178 (resolution) for IDM-BL96-Sinc2.5 along the $k_x = 1/\lambda_x$ wavenumber axis is 92 pixels 179 (7.7 mm), where k_x is the wavenumber and λ_x is the wavelength in the streamwise di-180 rection. This algorithm trades in some resolution relative to IDM-BL64-TH6 (Cameron 181 2011, figure 2) in return for improved robustness against outliers due to the larger in-182 terrogation regions. In comparison to the classic PIV method with 32 pixel unweighted 183 interrogation regions (IDS-TH32, figure 2), IDM-BL96-Sinc2.5 has slightly increased res-184 olution, improved flatness in the pass band, efficient anti-aliasing due to steep roll off 185 and negligible side lobes, and significantly increased robustness due to having nine times 186 more pixels in each interrogation region. Theoretical convergence for IDM-BL96-Sinc2.5 187 is eight iterations, defined here as the number of iterations required for the equivalent 188 noise bandwidth $(ENBW_q)$ to reach 99.9% of its ultimate value. $ENBW_q$ is calculated 189 for each iteration (q) by integrating the squared transfer function predicted after each 190 191 iteration:

$$ENBW_q = \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} \left[MTF_q\left(k_x, k_z\right)\right]^2 dk_x dk_z$$
(2.1)

where MTF_q is the modulation transfer function estimated for q iterations of the algo-192 rithm using the method of Astarita (2007), and k_z is the wavenumber in the vertical 193 direction. For comparison, theoretical convergence of IDM-BL64-TH6 is 36 iterations. 194 There is additional filtering of the velocity field associated with the finite thickness of 195 the light sheet (~ 1.5 mm), but in this case the light sheet is quite thin and the effect is 196 small relative to the filtering associated with the cross correlation algorithm. The effect 197 of the finite resolution of the measurement system is to reduce the contribution of high 198 wavenumber (small) eddies to the measured velocity variance. The magnitude of this 199 effect depends on the flow field and cannot be easily quantified. For the present experi-200 ments, however, the cut-off wavelength of the measurements (7.7 mm) is small compared 201 to the flow depth (390 mm) and therefore it is likely that the missing velocity variance 202 is small. 203

A feature of the field PIV images is that some of the interrogation regions were inter-204 mittently occupied by plant material or void of sufficient seeding particles such that valid 205 velocity vectors could not be obtained. In order to pre-empt these problems we introduce 206 a measurement 'clipping' function, $\phi_M(x, z, t_n)$, $(t_n \text{ is the time step})$ defined as $\phi_M = 1$ 207 for valid interrogation regions (with sufficient seeding and absent of any plant), otherwise 208 $\phi_M = 0$. The mean value of $\phi_M(x, z, t_n)$ can be defined as the measurement porosity. 200 The measurement clipping function serves two purposes, first it is passed to the cross 210 correlation algorithm so that bad interrogation regions can be handled appropriately by 211 the algorithm, and second it is passed to velocity field post processing routines so that 212 velocity statistics are correctly calculated only over valid data. Regions where $\phi_M = 0$ 213 are identified in an image pre-processing stage using a type of signal to noise ratio. Each 214 image is first decomposed into two parts, a plant image (by applying a median filter to 215

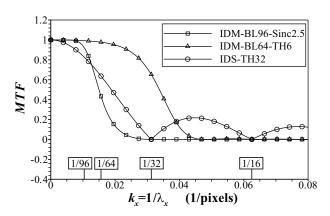


FIGURE 2. Comparison of transfer functions for different PIV algorithms: IDM-BL96-Sinc2.5 is employed in this study; other algorithms are discussed in Cameron (2011).

the original image) and a seeding image (by subtracting the plant image from the original 216 image). The 'signal' is then defined as the sum of the pixel intensities within an interroga-217 tion region for the seeding image, and the 'noise' is obtained as the sum of pixel intensities 218 within an interrogation region for the plant image. For each interrogation region, if the 219 signal divided by the noise is above a threshold value, ϕ_M is set to 1, otherwise it is 0. 220 The threshold value was optimised by visual assessment and a trial and error approach 221 on a subset of the PIV images. Once determined, it was applied globally throughout the 222 image set. Within the PIV algorithm, regions of $\phi_M = 0$ are replaced with interpolated 223 or extrapolated velocity values from neighbouring valid data. This step is important with 224 IDM PIV algorithms as it allows vector field low-pass filtering and interpolation to be 225 performed in each iteration without inadvertently propagating bad vectors to adjacent 226 interrogation regions. We emphasise that the interpolated /extrapolated values are only 227 used within the cross correlation algorithm. Time averaged statistics of the velocity field 228 are calculated incorporating only valid data as, for example, in the case of the first order 220 statistics: 230

$$\overline{\theta}(x,z) = \frac{1}{\sum_{t_n=1}^{t_n=T} [\phi_M(x,z,t_n)]} \sum_{t_n=1}^{t_n=T} [\theta(x,z,t_n) \times \phi_M(x,z,t_n)]$$
(2.2)

where θ is a flow variable, t_n is the time step, and T is the total number of time steps. The ϕ_M parameter is also used in calculating correlation functions and spectra; equations are given where appropriate in the following sections. Potential measurement uncertainties associated with flow regions having small values of measurement porosity are limited by only presenting data for which $\overline{\phi_M} > 0.75$.

2.3. Calculating plant velocity

The field PIV images contained enough detail of the fluctuating plant to extract estimates 237 of the vertical $w_p(x)$ and transverse $v_p(x)$ plant velocity components. We selected rect-238 angular interrogation regions (96×1024 pixels, 8×85 mm) which were sufficiently high 239 to cover the entire visible plant cross section, but narrow enough that plant velocities 240 could be measured as a function of streamwise position. Standard cross correlation ap-241 plied to median filtered PIV images (to remove seeding particles) resulted in a rather 242 wide peak in the correlation function (proportional to the width of the plant) and there-243 fore poor accuracy in estimating the displacement. To improve cross correlation perfor-244 mance, we have employed Wernet's (2005) symmetric phase only filtering (SPOF) which 245

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makes the correlation function more sensitive to the high wave number content of the 246 image (i.e., the sharply defined edges of the plant stems and leaves). The SPOF takes 247 the form of a weighting function $C(k_m, k_n)$ (2.3) applied to the cross spectral density 248 $G(k_m, k_n) H^*(k_m, k_n)$ between a pair of interrogation regions, where G and H are the 249 Fourier transforms of the first and second interrogation regions, H^* is the complex con-250 jugate of H, and k_m and k_n are wavenumbers in the m and n dewarped image directions. 251 The cross correlation function φ (2.4) is then calculated as the inverse Fourier transform 252 (FFT^{-1}) of the weighted cross spectral density, which for C = 1 is the standard Fourier 253 transform based cross correlation: 254

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$$C(k_m, k_n) = \frac{1}{\sqrt{|G(k_m, k_n)|}\sqrt{|H(k_m, k_n)|}}$$
(2.3)

$$\varphi = FFT^{-1} \left[C(k_m, k_n) G(k_m, k_n) H^*(k_m, k_n) \right]$$
(2.4)

The displacement of the correlation peak is estimated only in the vertical (n) image direction which is sensitive to vertical and transverse displacements of the plant. By combining the displacements estimated from a pair of stereoscopic cameras, the vertical $w_p(x)$ and transverse $v_p(x)$ plant velocity components are recovered. Due to the extended interrogation regions, the measured velocities approximate the cross-sectional average of plant velocity fluctuations.

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2.4. Stereoscopic velocity field reconstruction

Two-component velocity fields estimated using cross correlation on dewarped images are combined from two cameras to reconstruct the three-component velocity field according to Raffel *et al.* (2007) as:

$$\begin{bmatrix} 1 & 0 & \psi_{1m} \\ 0 & 1 & \psi_{1n} \\ 1 & 0 & \psi_{2m} \\ 0 & 1 & \psi_{2n} \end{bmatrix} \begin{bmatrix} \Delta_x \\ \Delta_z \\ \Delta_y \end{bmatrix} = \begin{bmatrix} \Delta_{1m} \\ \Delta_{1n} \\ \Delta_{2m} \\ \Delta_{2n} \end{bmatrix}$$
(2.5)

where Δ_{cm} and Δ_{cn} are the two displacement components estimated from images from the *c* camera (*c* = 1, 2), Δ_x , Δ_y and Δ_z (pixels) are the three displacement components in the *x*, *y*, *z* directions (figure 1a) and ψ_{cm} and ψ_{cn} are calibration factors calculated at the centre of each interrogation region using the camera calibration model. The ψ_{cm} and ψ_{cn} values indicate the shift in dewarped image coordinate (respectively in the *m* and *n* directions) corresponding to a unit displacement in the *y* (out-of-plane) direction and are equivalent to the tangents of the local view angles.

Equation (2.5) is an overdetermined system of linear equations (4 equations, 3 un-273 knowns). It can be solved using a least squares method (Raffel et al. 2007) or by calcu-274 lating exact solutions to subsets of the four equations (e.g. Prasad 2000). In the latter 275 case, a redundant estimate for one of the velocity components may be obtained and it 276 is standard practice to average together the redundant estimates to reduce the variance 277 of the measurement noise in that component by a factor of two (Prasad 2000). More 278 efficient use of the redundancy in (2.5) can be made by storing the redundant estimates 279 separately rather than averaging them together. Following the method introduced for 280 acoustic Doppler velocimeters by Hurther & Lemmin (2001), some velocity statistics can 281 then be calculated which have significantly reduced noise contribution. This approach has 282 not previously been tested with stereoscopic PIV data and so a brief evaluation is given 283 in the following section. For the present camera configuration, the redundancy inherent 284 in (2.5) falls substantially on the Δ_x displacement component, although in the general 285

case, it may be shared between all of the displacement components. Based on (2.5), we can write:

$$v = \Delta_y (M\Delta_{ls})^{-1} = (\Delta_{1n} - \Delta_{2n}) (\psi_{1n} - \psi_{2n})^{-1} (M\Delta_{ls})^{-1}$$
(2.6)

$$w = \Delta_z (M\Delta_{ls})^{-1} = (\Delta_{1n}) (M\Delta_{ls})^{-1} - \psi_{1n}v = (\Delta_{2n}) (M\Delta_{ls})^{-1} - \psi_{2n}v$$
(2.7)

$$u_{[1]} = (\Delta_{1m}) (M\Delta_{ls})^{-1} - \psi_{1m} v$$
(2.8)

$$u_{[2]} = (\Delta_{2m}) \left(M \Delta_{ls} \right)^{-1} - \psi_{2m} v \tag{2.9}$$

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$$u = 0.5 \left(u_{[1]} + u_{[2]} \right) \tag{2.10}$$

where M is a scale factor of the dewarped images (pixels/mm), $\Delta_{ls}(ms)$ is the time separation between laser pulses, $u_{[1]}$ and $u_{[2]}$ are redundant estimates of the u velocity component, and u, v, and w are the velocity components (m/s) in the x, y, and zdirections respectively.

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2.5. Noise reduction

The redundancy in the streamwise velocity measurement can be used to calculate velocity variance with a substantially reduced contribution of measurement noise. The instantaneous measured velocity fluctuation $(u' = u - \overline{u})$ can be decomposed into the sum of the actual velocity fluctuation (u_a') and the measurement error (ε_u') as $u' = u_a' + \varepsilon_u'$. The measured velocity variance can then be written:

$$\overline{u'u'} = \overline{(u_a' + \varepsilon_u')(u_a' + \varepsilon_u')} = \overline{u_a'u_a'} + \overline{\varepsilon_u'\varepsilon_u'} + 2\overline{u_a'\varepsilon_u'}$$
(2.11)

where the term $2\overline{u_a'\varepsilon_{u'}}$ vanishes if the measurement error is not correlated with the actual velocity fluctuation. The measured velocity variance therefore includes contributions from the actual velocity variance and the variance of the random measurement error. If redundant estimates of the velocity fluctuation $(u_{[1]}', u_{[2]}')$ are available, (2.11) can be rewritten as:

$$\overline{u_{[1]}'u_{[2]}'} = \overline{\left(u_a' + \varepsilon_{u_{[1]}}'\right)\left(u_a' + \varepsilon_{u_{[2]}}'\right)} = \overline{u_a'u_a'} + \overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[2]}}'} + \overline{u_a'\varepsilon_{u_{[1]}}'} + \overline{u_a'\varepsilon_{u_{[2]}}'} \quad (2.12)$$

where $\varepsilon_{u_{[1]}}'$ and $\varepsilon_{u_{[2]}}'$ are the measurement errors associated with $u_{[1]}'$ and $u_{[2]}'$ respectively. The third and fourth terms on the right vanish if the measurement error is not correlated with the velocity fluctuation, leaving $\overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[2]}}}'$ as the noise contribution to the measured velocity variance. The magnitude of $\overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[2]}}}'$ depends on the degree of correlation $(-1 \leq C_{\varepsilon_{u_{[12]}}} \leq 1)$ between the two noise terms, i.e.:

$$\overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[2]}}'} = C_{\varepsilon_{u_{[12]}}}\sqrt{\left(\overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[1]}}'}\right)\left(\overline{\varepsilon_{u_{[2]}}'\varepsilon_{u_{[2]}}'}\right)}$$
(2.13)

In the ideal case $C_{\varepsilon_{u_{[12]}}}$ approaches zero, and if additionally the measurement error is 312 uncorrelated with the velocity fluctuation, then $\overline{u_{[1]}'u_{[2]}'}$ can be considered a 'noise free' estimate of the velocity variance. In practice, although $u_{[1]}$ and $u_{[2]}$ are measured by 313 314 different cameras, some correlation between the noise terms might be expected as the 315 same particles are imaged by both cameras, albeit from different angles. Furthermore 316 the equations for $u_{_{[1]}}$ and $u_{_{[2]}}$ (2.8, 2.9) both include the transverse velocity v. In the 317 present study, however, the multiplying factors ψ_{1m} and ψ_{2m} are quite small, increasing 318 from zero at the centre of the image to around $|\psi_{cm}| = 0.2$ at the left and right edges. 319 Nevertheless, part of the measurement error in v will contribute to $u_{_{[1]}}'u_{_{[2]}}'$. 320

For the camera configuration used in our field experiments, this approach is limited to reducing the noise in statistics of the streamwise velocity component. The noise level in the other components can, however, still be estimated. Based on (2.5), by assuming $\psi_{1n} = -\psi_{2n}$ (for a symmetric camera system) and $\psi_{1m} = \psi_{2m} = 0$, and applying standard equations for error propagation, it can be shown that

$$\overline{\varepsilon_w'\varepsilon_w'} = \psi_{1n}^2 \overline{\varepsilon_v'\varepsilon_v'} = 0.5 N_{nm} \overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[1]}}'} = 0.5 N_{nm} \overline{\varepsilon_{u_{[2]}}'\varepsilon_{u_{[2]}}'}$$
(2.14)

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$$\overline{\varepsilon_v'\varepsilon_w'} = \overline{\varepsilon_u'\varepsilon_w'} = \overline{\varepsilon_u'\varepsilon_v'} \sim 0 \tag{2.15}$$

where $N_{nm} = \overline{\varepsilon_{\Delta_{cn}} \varepsilon_{\Delta_{cn}}} / \overline{\varepsilon_{\Delta_{cm}} \varepsilon_{\Delta_{cm}}}$, $\varepsilon_{\Delta_{cm}} \varepsilon_{\Delta_{cm}}$ is the random error in Δ_{cm} , and $\varepsilon_{\Delta_{cn}}$ is the random error in Δ_{cn} , and it is assumed that the error variance is the same for each camera (i.e. $\overline{\varepsilon_{\Delta_{1n}}} \varepsilon_{\Delta_{1n}} = \overline{\varepsilon_{\Delta_{2n}}} \varepsilon_{\Delta_{2n}}$ and $\overline{\varepsilon_{\Delta_{1m}}} = \overline{\varepsilon_{\Delta_{2m}}} \varepsilon_{\Delta_{2m}}$). For the present camera configuration, the value of N_{nm} is likely to be greater than one due to the elongation of particle images induced by the image dewarping process. Its value can be estimated with the help of computer generated PIV images.

Artificial PIV images were generated using a procedure described in Cameron (2011), 333 but extended here to generate a stereoscopic pair of images by applying the camera 334 calibration model to transform simulated three-dimensional particle coordinates to image 335 coordinates for a pair of cameras. The simulated cameras were positioned similar to the 336 real cameras used in the field experiments (63 degree viewing angle). Other parameters of 337 the simulation were: seeding concentration of 9×10^{-3} particles per pixel, particle image 338 diameter of 2.1 pixels, background intensity of 6 grey levels (8 bit quantization), random 339 additive noise with standard deviation 1.4 grev levels, maximum particle brightness of 340 500 grey levels (reflecting some saturation of the 8 bit image), and fill factor of 0.6. These 341 parameters were selected to approximate the experimental PIV images obtained in the 342 field. A series of 256×256 pixel images were generated, each with a uniform displacement 343 field across the image, but with the displacement systematically varied over a set of 344 4×10^5 images to uniformly cover the range $0 < \Delta_{cm} < 2$ and $0 < \Delta_{cn} < 4.4$ pixels 345 which corresponds to two full cycles of the peak locking error (Raffel et al. 2007) in each 346 direction. Note that the peak locking error typically has a period of 1 pixel, but when 347 images are dewarped, by in this case stretching the image by a factor of 2.2 in the n348 direction, the peak locking period is stretched by the same factor. The simulated images 349 were analysed using the same algorithm as was used for the field experiment images, and 350 the error variance for each component was obtained as: 351

$$\overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[1]}}'} = \overline{\varepsilon_{u_{[2]}}'\varepsilon_{u_{[2]}}'} = 1.06\overline{\varepsilon_{v}'\varepsilon_{v}'} = 0.27\overline{\varepsilon_{w}'\varepsilon_{w}'}$$
(2.16)

indicating a value of $N_{nm} = 7.4$. This relationship is used in section 3.1 to estimate the variance of the errors in the vertical and transverse velocity components. From the simulation, the correlation coefficient between the errors in $u_{[1]}$ and $u_{[2]}$ was found to be very small $(C_{\varepsilon_{u_{[12]}}} = 1.4 \times 10^{-3})$. The simulation data also indicates that the ratio $\overline{u_a'\varepsilon_{u_{[1]}}}'/\overline{u_a'u_a'} \approx \overline{u_a'\varepsilon_{u_{[2]}}}'/\overline{u_a'u_a'}$ is of the order 10^{-6} for the present experiments, confirming that the third and fourth terms on the right hand side of (2.12) can be safely neglected. It is therefore reasonable to assume that $\overline{u_{[1]}'u_{[2]}}'$ has significantly reduced noise contribution compared to $\overline{u_{[1]}'u_{[1]}}'$ and $\overline{u_{[2]}'u_{[2]}}'$

Higher order statistics can also be estimated using redundant velocity estimates to reduce noise contribution. For example, by calculating the velocity skewness (S) and kurtosis (K) as:

$$S_u = \frac{\overline{u'u'u'}}{\left(\overline{u_{[1]}}'u_{[2]}'\right)^{3/2}}$$
(2.17)

$$K_{u} = \frac{\overline{u_{[1]}' u_{[2]}' u_{[2]}' u_{[2]}'} + \left(\overline{u_{[1]}' u_{[2]}'}\right) \left(\overline{u_{[1]}' u_{[2]}'}\right) - \left(\overline{u_{[1]}' u_{[1]}'}\right) \left(\overline{u_{[2]}' u_{[2]}'}\right)}{\left(\overline{u_{[1]}' u_{[2]}'}\right)^{2}} - 3 \qquad (2.18)$$

The measurement noise is eliminated if it is uncorrelated with the velocity fluctuation and if the noise correlation $C_{\varepsilon_{u_{[12]}}}$ is zero. Again, (2.17) and (2.18) can only be applied for the streamwise velocity component for the present camera configuration. The noise contribution to the measured skewness and kurtosis for other velocity components can be estimated assuming that the random errors have a Gaussian distribution, using:

$$S_{va} = \frac{\overline{v_a' v_a' v_a'}}{\overline{v_a' v_a'}^{3/2}} = (1 + N_v)^{3/2} S_v$$
(2.19)

369

$$S_{wa} = \frac{\overline{w_a' w_a' w_a'}}{\overline{w_a' w_a'}^{3/2}} = (1 + N_w)^{3/2} S_w$$
(2.20)

370

$$K_{va} = \frac{\overline{v_a' v_a' v_a' v_a'}}{\overline{v_a' v_a'}^2} - 3 = (1 + N_v)^2 K_v$$
(2.21)

371

$$K_{wa} = \frac{\overline{w_a' w_a' w_a' w_a'}}{\overline{w_a' w_a'}^2} - 3 = (1 + N_w)^2 K_w$$
(2.22)

where S_v and S_w are the measured transverse and vertical velocity skewness, K_v and K_w are the measured transverse and vertical velocity kurtosis. Actual (or noise free) velocity fluctuations (v_a', w_a') , skewness (S_{va}, S_{wa}) and kurtosis (K_{va}, K_{wa}) are denoted with the subscript 'a'. The terms N_v and N_w are the noise to signal ratios defined as:

$$N_v = \frac{\overline{\varepsilon_v' \varepsilon_v'}}{\overline{v_a' v_a'}} \quad \text{and} \quad N_w = \frac{\overline{\varepsilon_w' \varepsilon_w'}}{\overline{w_a' w_a'}}$$
(2.23)

which can be estimated from (2.16) and the experimentally measured velocity variance.
These relationships are used in section 4.3 to estimate noise contributions to measured velocity skewness and kurtosis.

379 3. Field site and experiments

The site selected for the field deployment was on the Urie River, near the town of 380 Invertie and 26 km from Aberdeen City. An approximately straight section of the River 381 was identified (figure 3a) with convenient vehicle access and a rich abundance of aquatic 382 plants, including species from the Myriophyllum, Ranunculus, Potamogeton, and Cal-383 *litriche* genera, along with various aquatic mosses (figure 3b). The gravel bed at this 384 River reach had a median particle size of 35 mm (estimated from a random sample of 385 117 particles) and featured intermittent sandy patches and occasional large boulders. 386 At the test section the River was 12.9 m wide (figure 4), the average flow depth was 387 0.39 m, the flow rate was 2.7 m³/s, and the water surface slope was $1.5\pm0.4 \times 10^{-3}$. The 388 Reynolds number based on flow depth and mean velocity was 1.52×10^5 and the Froude 389 number was 0.28. Assessment of velocity time series (not shown) and observations of the 390 river water surface elevation throughout the deployment suggest that the flow conditions 391 were steady. 392

A set of PIV measurements were made of the flow field around a *Ranunculus penicillatus* plant patch located 2.8 m from the right river bank. The maximum dimensions of the patch were approximately 400 mm long, 200 mm wide, and 100 mm high. This particular

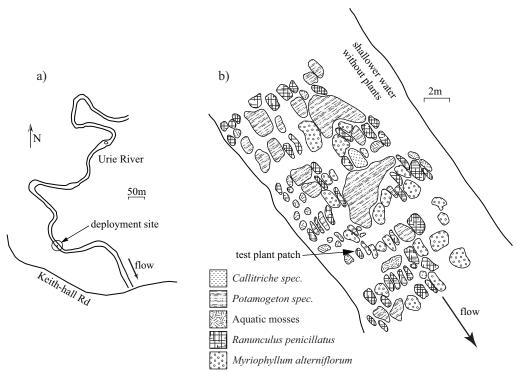


FIGURE 3. Sketch of the field deployment site (a). Aquatic plant species near the test section of the Urie River (b).

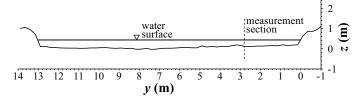


FIGURE 4. Cross section of the Urie River near the test section.

patch was selected because of its size in relation to the PIV field of view and its isolation 396 from other plants and large boulders. Three-minute PIV recordings were made at three 397 measurement locations, starting near the free end of the plant patch and subsequently 398 incremented by 130 mm in the downstream direction. The total measurement coverage 399 was a planar region 400 mm in the streamwise direction and 320 mm in the vertical 400 direction and aligned with the centreline of the plant (figure 5a). The recording rate was 401 30 image pairs per second, but due to a technical issue, some of the frames were later 402 found to not be viable, resulting in an average of 20 image pairs per second. Missing time 403 steps are assigned $\phi_M = 0$ allowing statistical quantities to be estimated using only valid 404 data. 405

3.1. Measurement noise

Based on the redundant estimates of the streamwise velocity component, the variance of
 the noise can be estimated as:

$$0.5\left(\overline{\varepsilon_{u_{[1]}}'\varepsilon_{u_{[1]}}'} + \overline{\varepsilon_{u_{[2]}}'\varepsilon_{u_{[2]}}'}\right) = 0.5\left(\overline{u_{[1]}'u_{[1]}'} + \overline{u_{[2]}'u_{[2]}'} - 2\overline{u_{[1]}'u_{[2]}'}\right)$$
(3.1)

11

⁴⁰⁶

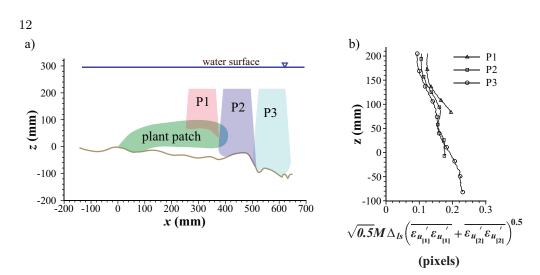


FIGURE 5. PIV measurement coverage (P1, P2, P3) relative to the plant patch and local bed topography (a). Standard deviation of measurement noise for the three measurement positions (b).

This noise term is plotted in the form $\sqrt{0.5}M\Delta_{ls}\left(\overline{\varepsilon_{u_{[1]}}}'\varepsilon_{u_{[1]}}'+\overline{\varepsilon_{u_{[2]}}}'\varepsilon_{u_{[2]}}'\right)^{0.5}$ in figure 5b, which is the standard deviation of the error in displacement units (pixels), allowing com-409 410 parison with previous studies of PIV error (here M = 12 pixels/mm and $\Delta_{ls} = 1$ ms). 411 Figure 5b indicates that the standard deviation of the measurement noise in the stream-412 wise displacement component is approximately the same for each of the three measure-413 ment positions and increases from around 0.1 pixels for large z (near the free surface) 414 to around 0.2 pixels near the bed. The increase in error approaching the bed reflects 415 the varying magnification of the source images and deteriorating image conditions with 416 distance from the cameras due to light sheet intensity falloff. The magnitude of the error 417 is comparable to that obtained from computer simulations when considering a significant 418 out of plane displacement component (e.g. Nobach & Bodenschatz 2009; Cameron 2011). 419 Values of the noise to signal ratio terms (2.23) for the transverse and vertical velocity 420 components can be estimated as $N_v = 0.02$ and $N_w = 0.07$ in the wake of the plant 421 patch. 422

423 4. Flow turbulence and plant fluctuations

424

4.1. Mean velocity field

Mean velocity streamlines combined for the three measurement planes (figure 6) indicate 425 that the flow does not separate from the plant patch and no recirculation zone forms. 426 Folkard (2011a) defines this as the 'canopy through-flow' regime, but the flow and patch 427 conditions for its existence are yet to be identified for real plants. In contrast, the small 428 rock immediately behind the plant patch shows clear signs of separation and recircu-429 lation, highlighting the potentially different mechanisms of drag for these two objects. 430 Bluff bodies, such as the rock behind the plant patch, produce drag mainly through the 431 differential pressure between their upstream and downstream surfaces which occurs due 432 to flow separation. Drag on aquatic plants, however, due to their flexibility, porosity, and 433 large wetted surface area, may be dominated by viscous drag (Nikora & Nikora 2007) 434 which forms due to the velocity gradient at the plant surfaces. Although figure 6 is con-435 sistent with the proposed conjecture, this hypothesis is difficult to test experimentally 436 as flow separation and pressure drag may occur at several different plant scales (plant 437

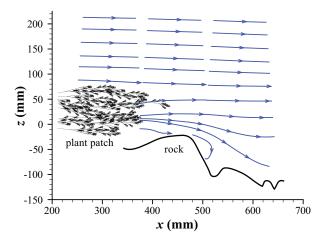


FIGURE 6. Velocity streamlines in the flow region around a Ranunculus plant patch.

patch, individual plant, stem, leaf). Recent experimental studies have measured drag 438 forces at each of these scales (e.g. Albayrak et al. 2012; Nikora et al. 2012; Siniscalchi & 439 Nikora 2012; Siniscalchi et al. 2012; Siniscalchi & Nikora 2013), however, separating vis-440 cous/pressure drag contributions directly still exceeds experimental capability. Further 441 complicating the viscous/pressure drag argument is that simple scaling relationships such 442 as $F_D \propto u^2$ for pressure drag and $F_D \propto u^1$ for viscous drag (where F_D is drag force on 443 the plant) cannot easily be applied to aquatic plants as they have the tendency to change 444 their structure in response to the velocity field. This so called 'reconfiguration' (Vogel 445 1994; de Langre 2008) can change the wetted surface area, the effective frontal area and 446 the drag coefficient (through streamlining) of the plant as a function of flow velocity, 447 thereby complicating interpretation of force scaling with flow velocity. 448

The plant drag, whether viscous or pressure dominated, is a sink of momentum and 440 introduces a free shear layer (and associated inflection in the $\overline{u}(z)$ profile) at the interface 450 between the retarded flow in the wake of the plant and the background channel flow 451 (figure 7). The inflectional form of the mean velocity profile is suggested to lead to 452 the Monami phenomenon in aquatic plant canopies (Ghisalberti & Nepf 2002; Nezu & 453 Sanjou 2008; Nepf 2012) and dominate local turbulence characteristics due to a periodic 454 production of vortices (Kelvin Helmholtz instability). For a single isolated plant patch, 455 however, the mean flow in the wake is distinctly three-dimensional and exposed to high 456 background turbulence levels which would tend to disrupt any periodic vortex formation 457 mechanisms. The shear layer may nevertheless be associated with high levels of turbulence 458 production; the distribution of the Reynolds stresses and their budget terms are examined 459 in the following section. Potential periodicity of the velocity in the plant wake is examined 460 in section 4.5. Figure 7b illustrates the streamwise momentum recovery in the wake of 461 the plant for the x coordinates marked by circles in figure 7a. The streamwise velocity 462 in the wake is steadily increasing with increasing x, and the corresponding decay of the 463 maximum velocity gradient is apparent. 464

4.2. Reynolds stresses and their budget terms

The normal Reynolds stresses $(\overline{u'u'}, \overline{v'v'}, \overline{w'w'})$ and the primary Reynolds shear stress ($-\overline{u'w'}$) all attain maximum values near the shear layer in the wake of the plant (figure 8). In general, the distribution of Reynolds stresses in the patch wake may depend on a variety of patch and approach flow conditions such as patch length and width, the

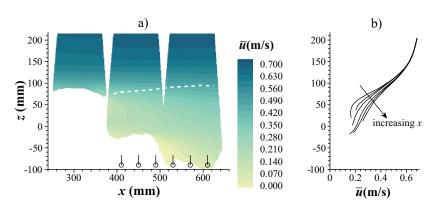


FIGURE 7. Time-averaged streamwise velocity distribution: a) around a *Ranunculus* plant patch, and b) in the wake of the plant patch for x coordinates corresponding to circle symbols in 'a'. Dashed line indicates local maximum in the $\partial \overline{u}/\partial z$ distribution.

distribution and shape of plant stems and leaves within the patch, the flexibility of 470 the plants, the approach flow Reynolds number, and the flow depth to patch height 471 ratio. For example, in contrast to our study, the peak Reynolds stress for Folkard's 472 (2005) model seagrass canopy formed several patch heights downstream of the patch 473 and near the reattachment point of the separated flow. It is not yet clear if natural 474 patches of *Ranunculus penicillatus* form similar wake features under different flow and 475 patch conditions. Secondary Reynolds shear stresses (not shown) were found to be an 476 order of magnitude smaller than the primary Reynolds shear stress as might be expected 477 (due to symmetry) near the centreline of the plant. The Reynolds stress correlation 478 coefficient $-\overline{u'w'}/(\overline{u'u'}\ \overline{w'w'})^{0.5}$, which reflects the efficiency of the turbulent fluctuations 479 at redistributing momentum, has a maximum value of 0.61 in the plant wake (x =480 400, z = 75), slightly larger than the 0.4-0.5 typical for open channel flows (Nezu & 481 Nakagawa 1993), the 0.5 found for terrestrial canopies (Raupach et al. 1996), and the 0.5 482 found in the wake of a cylinder (Cantwell & Coles 1983). 483

The transverse and vertical normal stresses have similar magnitudes to each other in 484 the plant wake (v'v'/w'w' = 1 - 1.2) which is smaller than the ratio 1.65 typical for open 485 channel flows (Nezu & Nakagawa 1993), but closer to the ratio of 1.2 measured for a 486 plane mixing layer by Wygnanski & Fiedler (1970). The streamwise normal stress $\overline{u'u'}$ 487 is found to decay with increasing x much faster than the other components. In the far 488 wake $(x = 600 \text{ mm}) \overline{u'u'}$ has reduced to 71% of its near wake (x = 400 mm) maximum. 489 Corresponding values for $\overline{v'v'}$ and $\overline{w'w'}$ are 91% and 85% respectively. Subtle differences 490 in the elevations where the maximum variance occurs can be seen between the different 491 components of the Reynolds stress tensor. Local maximums in both u'u'(z) and u'w'(z)492 tend to higher elevations with increasing x following the mean shear layer and reflecting 493 the expansion of the wake region into the outer flow. The trend for $\overline{w'w'}(z)$ is nearly 494 horizontal, and for $\overline{v'v'}(z)$ it is downward. The reason for these different trends is not 495 clear, but further understanding might be gained by considering the budget equation for 496

14

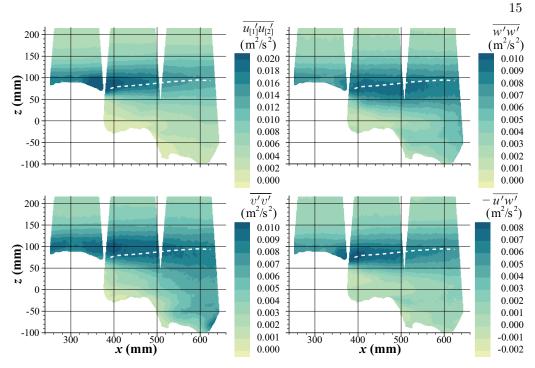


FIGURE 8. Reynolds normal stresses and the primary Reynolds shear stress. Dashed lines indicate local maximum in the $\partial \overline{u}/\partial z$ distribution.

⁴⁹⁷ the Reynolds stresses:

$$\underbrace{\frac{\partial \overline{u_k'u_i'}}{\partial t} + \overline{u_j} \frac{\partial \overline{u_i'u_k'}}{\partial x_j}}_{ij} = -\underbrace{\frac{\partial \overline{u_k'u_j'}}{\partial \overline{u_j}}}_{ij} = -\underbrace{\frac{\partial \overline{u_k'u_j'}}{\partial \overline{u_j}} - \overline{u_i'u_j'} \frac{\partial \overline{u_k}}{\partial x_j}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_k'u_j'}}{\partial x_j}}_{ij} + \underbrace{\frac{\partial \overline{u_k'u_k'}}{\partial x_j}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial x_j}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial x_j}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial x_j}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial x_j}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}{\partial \overline{u_i'u_k'}}}_{ij}}_{ij}} + \underbrace{\frac{\partial \overline{u_i'u_k'}}}{\partial \overline{u_i'u$$

where ρ is fluid density, ν is kinematic fluid viscosity, and p is fluid pressure. The free indices (*i* and *k*) can take the values 1, 2 or 3 where u_1 , u_2 , u_3 correspond to the velocity components u, v and w and x_1 , x_2 , x_3 , correspond to the x, y, and z directions respectively (figure 1a). The dummy index j implies summation over all possible values of j (j = 1, 2, 3) in accordance with the Einstein summation convention. Overbars indicate time (ensemble) averaged values and the prime symbol defines the deviation of an instantaneous variable from its time averaged value (e.g. $u' = u - \overline{u}$).

The Reynolds stress budget equation can be derived from the Navier-Stokes (NS) momentum conservation equation in three steps. First derive an equation for the fluctuating velocity by subtracting the time average of the NS equation from the NS equation. Second, multiply the equation for the velocity fluctuation by u_k and time average the

resulting equation. Third, exchange the free indices (i and k) in the equation developed in step 2 and add this new equation to the original equation in step 2 to give (4.1). It can be noted that the budget equation for turbulent kinetic energy is obtained by taking half the trace of (4.1). Equation (4.1) has received considerable attention as a framework to develop closure models for the Reynolds averaged Navier-Stokes equations. Distribution of the terms in (4.1) can also provide some insight into the turbulence in the wake of the plant patch, and in the present study this is our primary interest.

In a uniform, two-dimensional channel flow $\overline{v} = \overline{w} = \partial/\partial x = \partial/\partial y = 0$ and the only 516 non-zero normal stress production term is in the $\overline{u'u'}$ budget. Variance is redistributed 517 from u'u' to the other normal stress components by the pressure-strain correlation term 518 which is traceless and therefore does not appear in the total turbulent kinetic energy 519 balance. Away from boundaries, the dissipation rate is expected to be approximately 520 equal in each of the normal stress budgets due to local isotropy, if the Reynolds number 521 is reasonably high (Davidson 2004). Dissipation in the $\overline{u'w'}$ budget is typically small, and 522 production in $\overline{u'w'}$ is balanced largely by the pressure-strain term (Mansour *et al.* 1988; 523 Pope 2000). The mean convection, turbulent transport, and pressure transport terms 524 act to redistribute the Reynolds stresses in space and each of the transport terms inte-525 grate to zero over the flow depth in two-dimensional channel flow. The viscous transport 526 term is expected to be negligible away from boundaries compared to other transport 527 mechanisms if the Reynolds number is large. In the wake of an aquatic plant patch, the 528 time-averaged flow field is three-dimensional and some departure from the distributions 529 of the budget terms for two-dimensional flow may be expected. Some of the terms in 530 (4.1) cannot be evaluated from the experimental data. The pressure field is not avail-531 able, terms involving transverse derivatives cannot be calculated, and there is insufficient 532 spatial resolution to resolve the dissipation rate tensor that would require resolution of 533 the order of the Kolmogorov microscale (~ 0.1 mm). We can, however, estimate con-534 tributions from streamwise and vertical derivatives to the mean convection, turbulent 535 transport, and production terms as (4.2)-(4.5), where the terms in brackets highlight the 536 transverse derivatives that could not be calculated in this study. The effect of random 537 measurement errors should be negligible for the terms involving third moments (all tur-538 bulent transport terms) and terms involving the fluid stresses $\overline{u_{_{[1]}}'u_{_{[2]}}}', \overline{u'w'}, \overline{u'v'}, \text{ or } \overline{v'w'}$. 539 Terms involving $\overline{v'v'}$ or $\overline{w'w'}$ will be biased by the measurement noise, but evaluation 540 of the magnitude of the noise contribution to each of these terms suggests that in all 541 cases it is much smaller than the sampling error. Sampling errors were estimated using 542 a resampling technique (Garcia et al. 2006) and associated confidence intervals are indi-543 cated in figure 9. In general, the sampling error varies with z, but in order to reduce the 544 clutter in figure 9, an average value is given. Derivatives were estimated by convolving 545 the time averaged moments of the velocity field with a 21×21 grid point (21×21 mm) 546 2^{nd} order least squares kernel. The size of the filter was sufficiently large to smooth over 547 sampling errors (due to finite measurement duration), but still sufficiently small so as 548 not to significantly reduce the amplitude of the measured derivatives. 549

 $-\rho \overline{u} \frac{\partial \overline{u_{[1]}}' u_{[2]}}{\partial x} - \rho \overline{w} \frac{\partial \overline{u_{[1]}}' u_{[2]}}{\partial z} - \left(\rho \overline{v} \frac{\partial \overline{u'u'}}{\partial u}\right)$ mean conv. $-\rho \frac{\partial \overline{u'u'u'}}{\partial x} - \rho \frac{\partial \overline{w'u'u'}}{\partial z} - \left(\rho \frac{\partial \overline{v'u'u'}}{\partial y}\right) \qquad \qquad \Big\} \rho \overline{u'u'} \text{ budget}$ (4.2)turb. trans. $-2\rho \overline{u_{_{[1]}}}' u_{_{[2]}}' \frac{\partial \overline{u}}{\partial x} - 2\rho \overline{u'w'} \frac{\partial \overline{u}}{\partial z} - \left(2\rho \overline{u'v'} \frac{\partial \overline{u}}{\partial y}\right)$ production $-\rho \overline{u} \frac{\partial \overline{v'v'}}{\partial x} - \rho \overline{w} \frac{\partial \overline{v'v'}}{\partial z} - \left(\rho \overline{v} \frac{\partial \overline{v'v'}}{\partial y}\right)$ mean conv. $-\rho \frac{\partial \overline{u'v'v'}}{\partial x} - \rho \frac{\partial \overline{w'v'v'}}{\partial z} - \left(\rho \frac{\partial \overline{v'v'v'}}{\partial y}\right) \quad \left\{ \rho \overline{v'v'} \text{ budget} \right.$ (4.3)turb. trans. $-2\rho \overline{u'v'}\frac{\partial \overline{v}}{\partial x} - 2\rho \overline{v'w'}\frac{\partial \overline{v}}{\partial z} - \left(2\rho \overline{v'v'}\frac{\partial \overline{v}}{\partial y}\right)$ production $-\rho \overline{u} \frac{\partial \overline{w'w'}}{\partial x} - \rho \overline{w} \frac{\partial \overline{w'w'}}{\partial z} - \left(\rho \overline{v} \frac{\partial w'w'}{\partial y}\right)$ mean conv. $-\rho \frac{\partial \overline{u'w'w'}}{\partial x} - \rho \frac{\partial \overline{w'w'w'}}{\partial z} - \left(\rho \frac{\partial \overline{v'w'w'}}{\partial u}\right) \quad \left\{ \rho \overline{w'w'} \text{ budget} \right.$ (4.4)turb. trans. $-2\rho \overline{u'w'}\frac{\partial \overline{w}}{\partial r} - 2\rho \overline{w'w'}\frac{\partial \overline{w}}{\partial z} - \left(2\rho \overline{v'w'}\frac{\partial \overline{w}}{\partial u}\right)$ production $-\rho \overline{u} \frac{\partial \overline{u'w'}}{\partial x} - \rho \overline{w} \frac{\partial \overline{u'w'}}{\partial z} - \left(\rho \overline{v} \frac{\partial \overline{u'w'}}{\partial y}\right)$ mean conv.

turb. trans.

production

 $-\rho\frac{\partial\overline{u'u'w'}}{\partial x}-\rho\frac{\partial\overline{u'w'w'}}{\partial z}-\left(\rho\frac{\partial\overline{u'w'v'}}{\partial u}\right)$ $-\rho \overline{u'w'} \frac{\partial \overline{u}}{\partial x} - \rho \overline{w'w'} \frac{\partial \overline{u}}{\partial z} - \rho \overline{u_{[1]}}' u_{[2]}' \frac{\partial \overline{w}}{\partial x} - \rho \overline{u'w'} \frac{\partial \overline{w}}{\partial z} \\ - \left(\rho \overline{w'v'} \frac{\partial \overline{u}}{\partial y} - \rho \overline{u'v'} \frac{\partial \overline{w}}{\partial y}\right)$

(4.5)

 $\rho \overline{u'w'}$ budget

Distributions of the available Reynolds stress budget terms together with the Reynolds 550 stresses are shown in figure 9 for the near wake (x = 430 mm) and for the far wake 551 (x = 600 mm). The distribution of the production term in the $\overline{u'u'}$ budget forms a peak 552 at z = 85 mm in the near wake and z = 99 mm in the far wake which closely matches the 553 peaks in the corresponding Reynolds normal stress. The mean convection term is also a 554 gain at this elevation but is much smaller than the production term. The ratio between 555 the local production and the mean convection terms, 3.8 in the near wake, suggests that 556 the u'u' field near the shear layer is dominated by local rather than upstream generation 557 processes. This result is consistent with a plane mixing layer (Wygnanski & Fiedler 558 1970), but differs from many separated flows which feature a region where convection is 559 the dominant gain term, for example the axisymmetric wake (Uberoi & Freymuth 1970) 560

and the wake of a surface mounted cube (Hussein & Martinuzzi 1996). The turbulent 561 transport term in the u'u' budget is a loss near the shear layer and a gain at both higher 562 and lower elevations. The effect of this term is therefore to diffuse turbulence away 563 from the shear layer where it is produced. It is interesting to note that both turbulent 564 transport and mean convection terms cross zero above and below the shear layer at 565 about the same elevations (z = 114 mm and z = 64 mm respectively). Further, these 566 elevations correspond to measurement porosity values measured at $x = 350 \text{ mm of } \phi_M =$ 567 0.98 and 0.04 respectively, i.e. near the extreme upper and lower elevations of the top of 568 the fluctuating plant (considering that $\overline{\phi}_M$ in this region is dominated by the presence or 569 absence of plant within PIV interrogation regions). The alignment between these three 570 statistics may indicate that the fluctuating plant (and corresponding fluctuation of the 571 shear layer elevation) plays a role in regulating the distribution of u'u' in the wake. 572 Production in the $\overline{v'v'}$ and $\overline{w'w'}$ budgets is small compared to the production in the 573 $\overline{u'u'}$ budget and does not appear to explain why the maximums in the three Reynolds 574 normal stress distributions do not coincide. The reason for this is likely contained in the 575 pressure-strain and pressure transport terms (which are not available from experimental 576 data) and also in upstream production such as in the wakes of individual plant stems and 577 leaves. Both the turbulent transport and mean convection terms in the transverse and 578 vertical normal stress budgets have similar characteristics to the corresponding terms in 579 the streamwise normal stress budget. The transport terms are a loss where the velocity 580 variance is high and a gain in both the higher and lower flow layers. Convection terms 581 follow the same pattern, but are smaller and have opposite sign. The $\overline{u'w'}$ budget has 582 similar characteristics to the $\overline{u'u'}$ budget, but each term has opposite sign because the 583 primary Reynolds shear stress is negative. We note again a correlation between the 584 distribution of the production term and the corresponding Reynolds stress distribution 585 with the local peaks in these distributions forming at the same elevation. The production 586 is 7.3 times larger than the convection term indicating that the primary Reynolds shear 587 stress distribution is dominated by local rather than upstream production. The turbulent 588 transport and mean convection terms have opposite signs and similar to the $\overline{u'u'}$ budget, 589 each crosses zero near the same elevation. 590

591

4.3. Higher order moments

Skewness $S_i = \overline{u'_i u'_i u'_i} / \overline{u'_i u'_i}^{3/2}$ and kurtosis $K_i = \overline{u'_i u'_i u'_i u'_i} / \overline{u'_i u'_i}^2 - 3$ (repeated 592 index does not imply summation) distributions provide further indication of the nature 593 of the turbulence in the wake of the plant patch. Equations (2.19)-(2.22) indicate that 594 measured skewness and kurtosis are biased towards zero by the measurement noise. The 595 relative error in S_w is around 10% and in K_w is around 15% in the shear zone behind the 596 plant patch. Relative errors for S_v and K_v are 3% and 4% respectively and for S_u and K_u 597 the error contribution is minimised using (2.17) and (2.18). Skewness is an indicator of 598 the asymmetry of the velocity probability distribution, with negative skewness associated 599 with a left-tailed distribution (rare high magnitude velocity fluctuations tend to have a 600 negative sign) while positive skewness indicates a right-tailed distribution (rare high 601 magnitude events tend to have a positive sign). Figure 9 indicates that S_v in the wake 602 of the plant is near zero over much of the flow depth which is expected due to the 603 approximate symmetry of the time averaged flow field near the plant centreline. Skewness of the streamwise and bed-normal velocity components have opposite signs over most 605 of the flow depth. A transition from an 'ejection' dominated upper flow region ($S_u <$ 606 0, $S_w > 0$) to a 'sweep' dominated lower flow region $(S_u > 0, S_w < 0)$ is evident 607 around z = 88 mm in the near wake which corresponds to the location of the mean 608 shear layer. Such antisymmetric distributions of S_u and S_w are typical of mixing layers 609

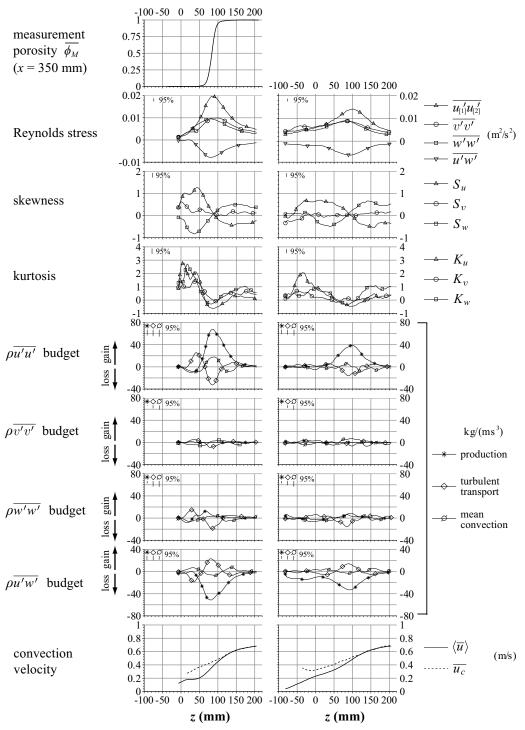


FIGURE 9. Velocity field statistics in the near wake (x = 430, left column except measurement porosity) and far wake (x = 600, right column) of a *Ranunculus penicillatus* plant patch.

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and flows over aquatic canopies (Raupach et al. 1996; Nezu & Sanjou 2008) but are also 610 found in open channel flows over gravel beds (Nikora & Goring 2000b). The kurtosis 611 of a probability distribution is an indicator of its 'peakedness' relative to a Gaussian 612 distribution. A high value of the kurtosis coefficient of a velocity signal indicates the 613 presence of rare (intermittent) high magnitude events, while a kurtosis coefficient that 614 is less than zero indicates that high magnitude events occur more frequently than for a 615 Gaussian distribution. Figure 9 indicates that the kurtosis coefficient for each velocity 616 component follows a similar distribution with regions of positive kurtosis in the higher 617 and lower flow layers and a region of low kurtosis near the shear layer. The largest values 618 of kurtosis are found behind the plant for elevations between z = 0 mm and z = 50 mm. 619 This indicates, in conjunction with $S_u > 0$, $S_w < 0$, that the flow field in this region 620 is characterised by rare high magnitude sweep events that likely originate from higher 621 flow layers and intermittently impinge into the low velocity region behind the plant. The 622 negative value of kurtosis near the mean shear layer ($K_u = -0.61$ at z = 88 mm) is 623 similar to the value of -0.63 measured by Wygnanski & Fiedler (1970) at the centre of 624 a plane mixing layer. Negative values of kurtosis have also been found in the near-bed 625 region of gravel bed open channel flows by Nikora & Goring (2000b) and for a smooth 626 wall boundary layer by Balachandar et al. (2001). It is interesting to note that in each 627 of these examples, and also for the present aquatic plant wake, the location of minimum 628 kurtosis corresponds to the location of maximum variance. 629

4.4. Convection Velocity

Eddy convection velocity $(\overline{u_c})$ has previously been studied primarily because of its rel-631 evance to Taylors 'frozen turbulence' approximation which can be applied to transform 632 velocity statistics (such as velocity spectra and correlation functions) between time and 633 space domains. Several studies have indicated surprising departures of the convection 634 velocity from the local mean velocity (with $\overline{u_c} > \overline{u}$) such as in terrestrial canopy flows 635 by Shaw et al. (1995), in aquatic canopies by Nezu & Sanjou (2008), and for gravel bed 636 open channel flows by Nikora & Goring (2000a). Understanding the reasons for this de-637 parture may provide some further insight into the turbulence structure, and this is our 638 motivation for examining the convection velocity in the wake of the *Ranunculus* plant 639 patch. 640

⁶⁴¹ Convection velocity in the wake of the plant patch can be estimated from the 2-point ⁶⁴² space-time correlation:

$$R(z, x_u, x_d, \Delta t_n) \left[\overline{u'u'}(x_u, z) \overline{u'u'}(x_d, z) \right]^{0.5} = \frac{\sum_{t_n=1}^{t_n=T} \left[u'(x_u, z, t_n) u'(x_d, z, t_n + \Delta t_n) \phi_M(x_u, z, t_n) \phi_M(x_d, z, t_n + \Delta t_n) \right]}{\sum_{t_n=1}^{t_n=T} \left[\phi_M(x_u, z, t_n) \phi_M(x_d, z, t_n + \Delta t_n) \right]}$$
(4.6)

where x_u and x_d identify 'upstream' and 'downstream' x coordinates, t_n is the time step, Δt_n is time step separation, T is the total number of time steps, and ϕ_M is the measurement clipping function described in section 2.2. The eddy convection velocity is then:

$$\overline{u_c}\left(z, x_u, x_d\right) = \frac{x_d - x_u}{\Delta t_{R_{\text{max}}}} f_s \tag{4.7}$$

where $\Delta t_{R_{\text{max}}}$ is the time separation (measured by time steps) that maximises R, and f_s is the sampling frequency (30 Hz). The mean velocity field in the wake of the plant is not homogeneous, so in order to make a meaningful comparison between the convection velocity and the local mean velocity, the latter is spatially averaged over the range 651 $x_u < x < x_d$:

670

$$\langle \overline{u} \rangle \left(z, x_u, x_d \right) = \frac{1}{\sum_{x=x_u}^{x=x_d} \sum_{t_n=1}^{t_n=T} \left[\phi_M \left(x, z, t_n \right) \right]} \sum_{x=x_u}^{x=x_d} \sum_{t_n=1}^{t_n=T} \left[u \left(x, z, t_n \right) \phi_M \left(x, z, t_n \right) \right]$$
(4.8)

Convection velocity and local average velocity are shown for the near wake $(x_u =$ 652 $410, x_d = 450$ mm) and for the far wake ($x_u = 580, x_d = 620$ mm) in figure 9. Below 653 z = 115 mm in the near wake and below z = 120 mm in the far wake, the convection 654 velocity deviates significantly from the local mean velocity. The result $\overline{u_c} > \langle \overline{u} \rangle$ might 655 be expected in the lower flow layers (z < 50 mm) as the turbulence in this region is 656 characterised by rare high magnitude velocity fluctuations which are generated near the 657 shear layer (where the mean velocity is higher) and periodically impinge into the low 658 velocity region. It is reasonable to assume that these eddies propagate with a velocity 659 close to the mean velocity where they are generated explaining the observed $\overline{u_c} > \langle \overline{u} \rangle$ 660 near the bed. We note, however, that near the shear layer (z = 85 mm in the near wake) 661 where the velocity fluctuations are dominated by local production, we can still observe 662 that the convection velocity is larger than the local mean velocity $\langle \overline{u_c} / \langle \overline{u} \rangle = 1.2$ at 663 z = 85 mm). Raupach et al. (1996) explains similar observations in terrestrial canopy 664 flows by suggesting that eddies which dominate the two-point correlation R are produced 665 mainly during wind gusts and therefore naturally propagate with the higher velocity of 666 the gust rather than the lower mean velocity. The relevance of this interaction mechanism 667 between the outer flow and the shear layer eddies to the present experiment, where scale 668 separation is much smaller, is not clear and remains to be clarified in future experiments. 669

4.5. Velocity Spectra

The structure of the velocity field in the wake of the plant is further examined by considering the power spectrum of velocity fluctuations, $F_{ii}(f)$. The spectrum can be evaluated for velocity data with missing samples using the Lomb-Scargle method (Lomb 1976; Scargle 1982) which can be written as:

$$2F_{ii}(\omega) = \frac{\left\{\sum_{t_n=1}^{t_n=T} u_i \phi_M \cos\left[\omega \left(t-\tau\right)\right]\right\}^2}{\sum_{t_n=1}^{t_n=T} \phi_M \cos^2\left[\omega \left(t-\tau\right)\right]} + \frac{\left\{\sum_{t_n=1}^{t_n=T} u_i \phi_M \sin\left[\omega \left(t-\tau\right)\right]\right\}^2}{\sum_{t_n=1}^{t_n=T} \phi_M \sin^2\left[\omega \left(t-\tau\right)\right]}$$
(4.9)

where, $t = t_n/f_s$ is the time corresponding to the t_n th measurement sample, $\omega = 2\pi f$ is the angular frequency, f is the linear frequency, and τ is a time lag adopted by (Scargle 1982) to enforce invariance of the spectrum to time translation of the data and simplify the statistical behaviour, with:

$$\tan(2\omega\tau) = \frac{\sum_{t_n=1}^{t_n=T} \phi_M \sin[2\omega t]}{\sum_{t_n=1}^{t_n=T} \phi_M \cos[2\omega t]}$$
(4.10)

The Lomb-Scargle method is equivalent to estimating the spectrum by a least squares fit of sine waves to the data and for regularly spaced data reduces to the conventional Fourier spectrum (Scargle 1982).

⁶⁶² Comparison of the velocity power spectrum near the shear layer in the wake of the ⁶⁶³ plant (x = 430, z = 85 mm, figure 10a) with the spectrum at a higher elevation (x = 430, ⁶⁸⁴ z = 200 mm, figure 10b), where the influence of the plant is reduced, indicates a broad ⁶⁶⁵ increase in energy across all resolved frequencies in the plant wake. Some flattening of ⁶⁶⁶ the spectrum is evident at higher frequencies due to the contribution of aliasing and ⁶⁶⁷ measurement noise. A subtle clustering of energy around f = 1 Hz can be seen in the

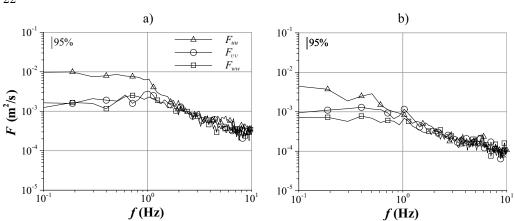


FIGURE 10. Flow velocity spectrum a) behind the plant patch at x = 430, z = 85 mm; b) above the plant patch at x = 430, z = 200 mm.

wake spectrum, but there is no indication of a highly periodic component that would suggest a Kelvin Helmholtz type instability of the shear layer.

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4.6. Plant velocity fluctuations and plant-flow coupling

The fluctuating movements of aquatic plants are important for several reasons. First, 691 plant motion can enhance photosynthetic rate and nutrient uptake through increased 692 delivery of light and nutrients to leaf surfaces (Koehl & Alberte 1988; Nikora 2010). Sec-693 ond, plant drag forces (which determine plant survival during high flow periods) may be 694 regulated, to some extent, by plant motion. There is some evidence that waving plants 695 can experience less drag by aligning themselves with instantaneous velocity streamlines 696 ('dynamic reconfiguration', Siniscalchi & Nikora 2013), although in general the reverse 697 may also be true. Finally, plant movement can enhance turbulent kinetic energy in the 698 plant wake with implications for sediment transport and mixing processes. In the follow-699 ing, we study plant velocity fluctuations extracted from PIV images using the method 700 described in section 2.3, to examine the nature of the plant motion and potential inter-701 action mechanisms with the turbulent flow. 702

Measured vertical and transverse plant velocity variance (figure 11a) is found to in-703 crease rapidly approaching the free end of the plant, consistent with the similar measure-704 ments of Siniscalchi & Nikora (2013) for a variety of aquatic plant species in a laboratory 705 flume. The shape of the variance distribution reflects the structural dynamics of the 706 plant and the turbulent forcing due to the fluctuating viscous and pressure stresses at 707 plant surfaces. For simple structures undergoing free vibration (without external forcing) 708 analytical solutions to the equations of motion may be obtained to predict the relative 709 amplitude (and variance) of vibrations along the structure. The complex geometry of 710 aquatic plants and their as yet uncertain biomechanical properties still preclude such 711 analysis for the present case without dramatic simplifications. The ratio of transverse to 712 vertical plant velocity variance is in the range 1.15 to 1.35 over the resolved plant length. 713 quite similar to the corresponding ratio of fluid velocity variance in the wake of the plant 714 (1 to 1.2). Further information about the nature of plant velocity fluctuations can be 715 obtained from the 2-point correlation function (4.6). Figure 11b indicates that the time 716 $(\Delta t = f_s^{-1} \Delta t_n)$ corresponding to the maximum in the correlation function is increasing 717 with increasing point separation $(\Delta x = x_d - x_u)$. This suggests that the characteristic 718 plant motion is that of travelling waves rather than standing waves (vibration). These 719

two phenomena are, however, closely related as standing waves can be considered to 720 arise from the interference (constructive and destructive) of forward and backward prop-721 agating waves (Graff 1991). Païdoussis (2004), considering slender cylindrical structures 722 aligned axially with the flow, indicates that wave propagation rather than vibration is 723 typical for long structures. The propagation velocity estimated from the time lag that 724 maximises the correlation function (figure 11b) is 0.46 m/s for both v_p' and w_p' , which 725 is similar to the eddy convection velocity measured in the wake of the plant patch in 726 the shear zone (figure 9). The similarity between these two convection velocities suggests 727 that the waves propagating through the plant are dominated by the passage of turbulent 728 fluctuations (vortices). The plant velocity spectrum (figure 11c, x = 309 mm) indicates 729 maximum energy for frequencies around 1 Hz for both vertical and transverse compo-730 nents. The shape of the spectrum resembles that obtained in laboratory experiments 731 using the same species of plant (Ranunculus penicillatus, Siniscalchi & Nikora 2013) and 732 features a significant decay of energy towards both lower and higher frequencies. In com-733 parison, the transverse and vertical components of the fluid velocity spectrum measured 734 outside the flow region influenced by the plant (figure 10b) are constant (saturated) for 735 frequencies less than 1 Hz. If the plant velocity can be considered as a (linearly) filtered 736 response to the fluid velocity, figure 11c in comparison to figure 10b suggests that the 737 plant responds optimally to frequencies around 1 Hz (or wavelengths $\overline{u}/f \sim 0.5$ m, i.e. 738 of a similar scale to the patch length or flow depth). This observation may be related 739 to Naudascher & Rockwell's (1994) finding that for cylinders aligned axially with the 740 flow, each vibration mode of the structure is most efficiently excited by vortices of a cer-741 tain wavelength. Vibration modes for an aquatic plant are, however, yet to be identified. 742 Possible mechanisms of flow-plant interaction are further discussed in section 5. 743

Potential correlations between fluid (u_j') and plant $(u_{ip'})$ velocity fluctuations can be further examined using the normalised covariance function:

$$R_{0i_{pj}}(x,z) = \frac{u_{ip'}(x)}{\left(\overline{u_{ip'}}^{2} \overline{u_{j'}}^{2}\right)^{0.5}}$$
(4.11)

with i=2, 3 (v_p', w_p') and j=1, 2, 3 (u', v', w'). For x = 309 mm and z values approaching 746 the free surface, the correlation between plant and fluid motion for all components is 747 small $(R_{0i_pj} \sim 0.05)$, figure 11d). The R_{0v_pu} , R_{0v_pw} , R_{0w_pv} terms remain small for all z, 748 but the R_{0v_pv} , R_{0w_pw} , R_{0w_pu} terms increase rapidly approaching the top of the plant. 749 While it is not surprising to find a correlation between matching velocity components 750 (R_{0v_pv}, R_{0w_pw}) and the cross-component term R_{0w_pu} through the secondary correlation 751 $\overline{u'w'} < 0$, the narrowness of the correlated range $\Delta z \sim 30$ mm is unexpected. Given 752 the 1 Hz characteristic frequency (figure 11c) of plant velocity fluctuations, we might 753 reasonably be looking for characteristic eddy sizes of the order $\overline{u}/f \sim 0.5$ m and a 754 correspondingly larger correlation length. In the following section we consider potential 755 flow-plant interaction mechanisms that may help interpret the measured spectra and 756 correlation functions. 757

⁷⁵⁸ 5. Flow-plant interactions: concluding remarks

Naudascher & Rockwell (1994) indentified three general classes of flow-induced vibration mechanisms: extraneously induced excitation (EIE), instability induced excitation (IIE), and movement induced excitation (MIE). These classifications were developed to help identify and analyse the source of vibrations in engineering structures, but they are also relevant to the present case of flow-aquatic plant interactions, even if the charac-

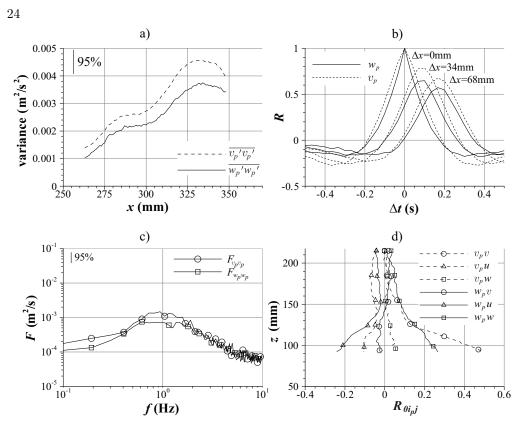


FIGURE 11. Variation of plant velocity variance along the length of the plant (a). Two-point correlation of plant velocity fluctuations (b). Spectrum of plant velocity fluctuations (x = 309 mm, c). Normalised covariance between plant and flow velocity fluctuations (x = 309 mm, d).

teristic plant motion is that of a propagating wave (figure 11b) rather than a vibration. 764 Extraneously induced excitation relates to structure (plant) motion caused by turbulence 765 in the flow, but independent of any local flow instability associated with the presence of 766 the plant. Instability induced excitation relates to motion induced by a flow instability 767 that appears due to the presence of a structure. For an aquatic plant, this instability 768 could, for example, be unsteady flow separation from the plant or a shear layer insta-769 bility. Flow separation at a plant scale seems unlikely based on figure 6, however, there 770 is significant turbulent kinetic energy associated with the shear layer in the wake of 771 the plant patch (figures 8, 9), and the short range of elevations over which plant and 772 fluid velocity fluctuations are correlated (figure 11d) support a contribution of IIE to the 773 plant motion. The absence of strong periodicity in the velocity spectrum measured in 774 the plant patch wake (figure 10a), however, does not support an instability of the Kelvin 775 Helmholtz type. We did observe a weak clustering of energy around 1 Hz (matching well 776 the dominant frequency of plant motion, figure 11c), however, this may simply reflect 777 the flapping elevation of the shear layer as the plant moves up and down. The relative 778 importance of EIE and IIE cannot be confirmed from the present experiments, but this 779 could be further investigated in a laboratory environment by, for example, towing plants 780 through stationary water to eliminate sources of EIE. Movement induced excitation is 781 a self-excited body vibration where the acceleration of a body in a fluid alters the flow 782 field in a way that can feed back to the body (via pressure and viscous stresses) to am-783 plify the initial movement. The 'flutter' of flags or aircraft wings are examples of MIE. 784

The correspondence between measured convection velocities of plant velocity fluctuations 785 and fluid velocity fluctuations in the plant patch wake suggests that the plant velocity 786 fluctuations are dominated by the passage of turbulent eddies (either EIE and IIE), and 787 MIE seems unlikely in the present case. Flexible cylinders aligned axially with the flow 788 (resembling aquatic plant stems to some extent) can exhibit MIE at certain critical flow 789 velocities, the dynamics for which have been studied extensively (e.g. Païdoussis 2004; 790 de Langre et al. 2007). Extension of this type of analysis to an aquatic plant is not yet 791 realistic due to the complex and changing plant geometry (reconfiguration) and the lack 792 of fundamental knowledge of the coupling between fluid flow and resulting lift and drag 793 forces acting on the plant. 794

Further experimentation in the laboratory and in the field is needed to clarify the nature of flow-plant interaction mechanisms. In this regard, we have demonstrated that the stereoscopic PIV method can be applied in field conditions and should prove to be valuable in further study of flow-aquatic plant interactions.

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