

Force-derivative feedback semi-active control of base-isolated buildings using large-scale MR fluid dampers

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ABSTRACT: The combination of passive and active schemes has been increasingly considered in the structural control community as a promising way to design efficient smart hybrid base isolation systems for seismic protection. This paper considers a hybrid system in which an active feedback control law is derived to be applied in parallel with a passive isolation device. The active control uses the restoring force supplied by the passive isolator as the main feedback signal. This paper can be divided in two main parts: in the first one, the paper presents the theoretical formulation and stability analysis in the active control strategy; in the second part, a set of numerical simulations is performed when the force is supplied in a semi-active way to validate and discuss the efficiency of the approach in a more realistic scenario. Moreover, the performance of the proposed semi-active control algorithm is compared with passive-off, passive-on and clipped-optimal controllers. The proposed control scheme reduces the base displacement without increasing the floor accelerations.

KEY WORDS: structural control; semi-active; MR dampers.

1 INTRODUCTION

Structural control systems have shown great interest in the last decades for hazard mitigation in civil structures [9]. Passive control systems have been widely used to mitigate vibrations due to external dynamic loadings [15], [22]. The basic concept of base isolation is to make the structure behave like a rigid body through a certain degree of decoupling from the ground motion. However, for the purpose of maintaining the seismic response of structures within safety, service and comfort limits, the combination of passive base isolators and feedback controllers (applying forces to the base) has been proposed in recent years. Applications of hybrid control systems consisting on active ([4], [16], [17], [18]) or semi-active ([3], [7], [14]) systems installed in parallel to base isolators have the capability of reducing response quantities of base-isolated structures more significantly than passive dampers. The idea of adding a feedback control is based on the premise that a control action is to be applied at the base with force magnitudes which are not excessive due to the high flexibility of the isolators. The main benefit of the inclusion of the control is that the assistance of such a force can help prevent large displacements of the base isolator, which could endanger the integrity of the scheme.

In this paper we firstly consider this kind of hybrid systems in which an active feedback control law is derived to be applied in parallel with a passive isolation device. The active feedback

control law uses the restoring force supplied by the passive isolator as the main feedback signal.

Since semi-active controllers in hybrid base-isolation systems can achieve almost the same performance as an active base isolation system in protecting the safety of building against strong earthquakes [13], we also present the semi-active realization of the proposed active scheme. In this sense, the magnetorheological (MR) fluid dampers –considered as semi-active devices– are represented using the normalized Bouc–Wen model [11]. Because the force generated in the MR dampers is dependent on the local responses of the structural system, the desired control force cannot always be produced by the devices. Only the control current or voltage can be directly controlled to increase or decrease the force produced by this devices. In this work, a new practical method [2], [3] is used to compute the command current of the MR dampers. The whole method is finally simulated by considering a three-dimensional smart base-isolated benchmark building [16].

The paper is structured as follows. Section 2 is dedicated to designing the force-derivative feedback control law and it is divided into two subsections: Subsection 2.1 presents the dynamic model of the base-isolated structure; Subsection 2.2 describes the desired control force which is based on a force-derivative feedback controller for hysteretic base-isolated structures. The semi-active realization of the proposed control scheme is developed in Section 3. The inverse model that

provides a suitable context to compute the command current of MR dampers analytically is described in Subsection 3.1; meanwhile the algorithm for selecting the command signal is concisely stated in Subsection 3.2. The smart base-isolated structure that serves as a benchmark problem for numerical testing is presented in Section 4 together with some numerical simulations to analyze the performance of the proposed semi-active scheme. Final comments are given in Section 5.

2 FORCE-DERIVATIVE FEEDBACK CONTROL LAW DEVELOPMENT

There exists a wide range of control algorithms that are applied to base-isolated buildings: clipped-optimal control [7], [12], [26]; maximum energy dissipation algorithms [14]; and modulated homogeneous friction algorithms, among others. Each of these controllers is able to reduce the structural response to some degree. From a structural point of view, a reasonable controller has to reduce the base displacement while decreases or slightly increases the accelerations. Li and Ou [13] showed that the active control forces in base-isolated structures have damping characteristics. In this study, an active force-derivative feedback controller will be applied in a semi-active way to the eighth-storied base-isolated benchmark building [16]. The control forces will be applied at the base through manipulation of the command current i at the MR dampers.

2.1 System description

The system description considers a nonlinear base-isolated building structure as shown in Figure 1. More precisely, the control design is based on a dynamic model composed of two coupled subsystems, namely, the main structure or superstructure (S_r) and the base isolation (S_c):

$$S_r : \mathbf{M}\ddot{\mathbf{x}} = -\mathbf{M}\mathbf{J}\ddot{\mathbf{x}}_g - \mathbf{C}\dot{\mathbf{r}} - \mathbf{K}\mathbf{r} \quad (1)$$

$$S_c : m\ddot{x} + c\dot{x} + kx = c_1\dot{r}_1 + k_1r_1 - m\ddot{x}_g - \Phi + u \quad (2)$$

where \ddot{x}_g is the absolute ground acceleration, $\mathbf{x} = [x_1, x_2, \dots, x_8]^T \in \mathbb{R}^8$ represents the horizontal displacements of each floor with respect to the ground. The mass, damping and stiffness of the i th storey is denoted by m_i, c_i and k_i , respectively, $\mathbf{r} = [r_1, \dots, r_8]^T \in \mathbb{R}^8$, represents the horizontal displacements of the i -th floor relative to the $(i-1)$ -th floor. The base isolation is described as a single degree of freedom with horizontal displacement x . It is assumed to exhibit a linear behavior characterized by mass, damping and stiffness m, c and k , respectively, plus a nonlinear behavior represented by a hysteretic restoring force Φ . This restoring force can be supplied by some base isolation device. The matrices $\mathbf{M}, \mathbf{C}, \mathbf{K}$ and \mathbf{J} of the structure have the following form:

$$\mathbf{M} = \text{diag}(m_1, m_2, \dots, m_8) \in \mathbb{R}^{8 \times 8}$$

$$\mathbf{J} = [1, \dots, 1]^T \in \mathbb{R}^8$$

$$\mathbf{C} = (c_{ij}) \in \mathbb{R}^{8 \times 8}, c_{ij} = \begin{cases} c_i, & i = j \\ -c_{i+1}, & j - i = 1 \\ 0, & \text{otherwise} \end{cases}$$

$$\mathbf{K} = (k_{ij}) \in \mathbb{R}^{8 \times 8}, k_{ij} = \begin{cases} k_i, & i = j \\ -k_{i+1}, & j - i = 1 \\ 0, & \text{otherwise} \end{cases}$$

Finally, u is the active control force supplied by an appropriate actuator.

The model in equations (1)-(2) is used to design an appropriate control law. The applicability and efficiency of the proposed controller will be then shown using a more realistic and complex model.

The equation of motion of the base (2) can be written in the form

$$S_c : m\ddot{x} + c\dot{x} + kx = \underbrace{c_1(\dot{x}_1 - \dot{x}) + k_1(x_1 - x)}_{\delta[x, \dot{x}, x_1, \dot{x}_1]} - \underbrace{\Phi - m\ddot{x}_g}_{\Delta(t)} + u.$$

It is well accepted that the movement of the superstructure S_r is very close to the one of a rigid body due to the base isolation [18]. Then it is reasonable to assume that the motion of the first floor relative to the base will be very small. Therefore, it is also reasonable that the interaction force $\delta[x, \dot{x}, x_1, \dot{x}_1]$ will be small in comparison with the rest of the forces acting on the base [14]. Consequently, the following simplified equation of motion of the base can be used in the subsequent controller design:

$$\tilde{S}_c : m\ddot{x} + c\dot{x} + kx = -\Phi - m\ddot{x}_g + u. \quad (3)$$

2.2 Force-derivative feedback controller

Force-feedback controllers have received great attention in the active cable control of cable-stayed bridges [6], where the concept of *integral force feedback* is introduced, offering a sufficient increase of structure damping by suitable sensor-actuator-pairs integrated between the stay-cables and the bridge deck. The static loads are compensated by hydraulic accumulators without permanent power supply. Moreover, force-feedback controllers have been seen in active vibration isolation [5], [19], where it is shown that the force feedback implementation benefits from alternating poles and zeros which allows a control law with guaranteed stability, making it very attractive when the payload to be isolated from the disturbance source is very flexible, such as in large space structures. At the same time, other applications have been seen in piezoelectric actuators for vibration control of civil structures used (decentralized) *integral force feedback* control [21], vehicle vibration control using MR dampers [23], and hybrid structural simulation [1] and [25].

Assuming that the earthquake disturbance is unknown but bounded, the following force-derivative feedback controller is proposed:

$$u = -\zeta \dot{\Phi}, \quad (4)$$

where ζ is a positive real number. From an active control point of view, the force which has to be supplied by the actuator is

based on the measured force which is currently supplied by the isolator device.

More precisely, the following assumptions are stated for system (3):

Assumption 1: The force Φ is described by the normalized version of the Bouc–Wen model [10]:

$$\Phi(t) = \kappa_{\dot{x}}\dot{x}(t) + \kappa_w w(t), \quad (5)$$

$$\begin{aligned} \dot{w}(t) = & \rho(\dot{x}(t) - \sigma|\dot{x}(t)||w(t)|^{n-1}w(t) \\ & + (\sigma - 1)\dot{x}(t)|w(t)|^n), \end{aligned} \quad (6)$$

where \dot{x} is the velocity and $\kappa_{\dot{x}}, \kappa_w, \rho, \sigma$ and n are the system parameters.

Assumption 2: The earthquake disturbance $-m\ddot{x}_g(t)$ is unknown but bounded; i.e., there exists a known constant G such that $|\ddot{x}_g(t)| \leq G, \forall t \geq 0$.

Moreover, Theorem 1 in [10] guarantees the existence of a computable upper bound ρ_w on the internal dynamic variable $w(t)$, i.e., $|w(t)| \leq \rho_w, \forall t \geq 0$, independently on the boundedness of $x(t)$.

The following theorem states the bounded-input bounded-output stability of the proposed controller.

Theorem 1: Consider the nonlinear system in equation (3) and the force-derivative feedback control law

$$u = -\zeta \dot{\Phi}, \quad (7)$$

under the assumption of the boundedness of the earthquake disturbance. Then, the closed-loop system in equations (3) and (7) is bounded-input bounded-output stable.

Proof. This proof is based on the boundedness of the earthquake disturbance $-m\ddot{x}_g$.

The closed-loop system in equations (3) and (7) yields

$$m\ddot{x} + c\dot{x} + kx = -\underbrace{(\kappa_{\dot{x}}\dot{x} + \kappa_w w)}_{\Phi} - m\ddot{x}_g - \zeta \underbrace{(\kappa_{\dot{x}}\dot{x} + \kappa_w \dot{w})}_{\dot{\Phi}}.$$

Applying the Laplace transform to the equation of motion of the base, we obtain

$$\begin{aligned} [(m + \zeta \kappa_{\dot{x}})s^2 + (c + \kappa_{\dot{x}})s + k]x(s) = \\ -m\ddot{x}_g(s) - [\zeta \kappa_w s + \kappa_w]w(s), \end{aligned}$$

where $x(s), w(s)$ and $\ddot{x}_g(s)$ are the Laplace transform of the signals $x(t), w(t)$ and $\ddot{x}_g(t)$, respectively. The direct transfer function between the ground motion \ddot{x}_g , the hysteretic variable w , and the controlled base displacement x is

$$\begin{aligned} x(s) &= \frac{\overbrace{(m + \zeta \kappa_{\dot{x}})s^2 + (c + \kappa_{\dot{x}})s + k}^{T_g(s)}}{-m} \ddot{x}_g(s) \\ &\quad - \frac{\zeta \kappa_w s + \kappa_w}{\underbrace{(m + \zeta \kappa_{\dot{x}})s^2 + (c + \kappa_{\dot{x}})s + k}_{T_w(s)}} w(s) \\ &= T_g(s)\ddot{x}_g(s) - T_w(s)w(s) \end{aligned}$$

It can be shown, using the Nyquist stability criterion, that the transfer functions $T_g(s)$ and $T_w(s)$ are stable. Therefore, the boundedness of the *input* signals $\ddot{x}_g(t)$ and $w(t)$ guarantees the boundedness of the *output* signal $x(t)$, that is, the base displacement. ■

3 SEMI-ACTIVE IMPLEMENTATION

In this work, the active control force is not applied directly to the base through an active actuator. Contrarily, we use MR dampers to perform a semi-active realization. The first step used to design a semi-active control strategy is the selection of an *active* control law that makes the closed-loop system stable. In our case, this active control law is based on the derivative of the force which is currently supplied by the isolator device. The second step is the use of a semi-active MR damper, with force F_{MR_p} , to try to follow the active *desired* force given by equation (7). That is, we use: (a) a passive isolator device described by equations (5)-(6) with fixed parameters and (b) a semi-active MR damper that tries, by updating the current i , to apply a force equal to the derivative of the force of the passive isolator device times a design parameter $-\zeta$. Even though this strategy can seem redundant, in the event of a fault, the system continues acting in a passive way. The next sections are concerned on how to compute the command current to carry out this objective.

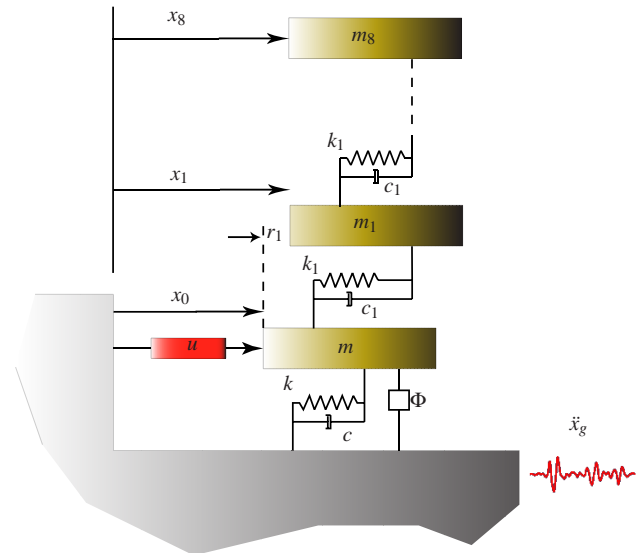


Figure 1. Two degree-of-freedom (2DOF) model.

3.1 The inverse model

The inverse model will provide a suitable tool to compute the command current of MR dampers analytically. Consider the normalized form of the Bouc–Wen model for MR dampers (see the Appendix for further details):

$$\begin{aligned} F_{MR}(t) = & [\kappa_{\dot{x},a}i(t) + \kappa_{\dot{x},b}] \dot{x}(t) \\ & + [\kappa_{w,a}i^2(t) + \kappa_{w,b}i(t) + \kappa_{w,c}] w(t), \end{aligned}$$

where $F_{MR}(t)$ is the output force of the MR damper. The inverse model, that is, the computation of the current i as a function of the velocity and force, is based on the following simplification: (a) the internal dynamic variable $w(t)$, which is unmeasurable, is replaced by the sign of the velocity:

$$w(t) = \text{sgn}(\dot{x}) \in \{-1, 1\}.$$

We remark that, in the normalized version of the Bouc–Wen model, the value of this internal dynamic variable lies within the range $[-1, 1]$.

As a result of this simplification, the MR damper model is

$$\begin{aligned} F_{MR}(t) &= [\kappa_{\dot{x},a}i(t) + \kappa_{\dot{x},b}] \dot{x}(t) \\ &+ [\kappa_{w,a}i^2(t) + \kappa_{w,b}i(t) + \kappa_{w,c}] \operatorname{sgn}(\dot{x}) \\ &= [\kappa_{w,a} \operatorname{sgn}(\dot{x})] i^2(t) + [\kappa_{\dot{x},a} \dot{x}(t) + \kappa_{w,b} \operatorname{sgn}(\dot{x})] i(t) \\ &+ [\kappa_{\dot{x},b} \dot{x}(t) + \kappa_{w,c} \operatorname{sgn}(\dot{x})] \end{aligned}$$

The current $i(t)$ can be found by solving the quadratic equation

$$\underbrace{\kappa_{w,a} \operatorname{sgn}(\dot{x})}_{a_2} i^2(t) + \underbrace{[\kappa_{\dot{x},a} \dot{x}(t) + \kappa_{w,b} \operatorname{sgn}(\dot{x})]}_{a_1} i(t) + \underbrace{[\kappa_{\dot{x},b} \dot{x}(t) + \kappa_{w,c} \operatorname{sgn}(\dot{x}) - F_{MR}(t)]}_{a_0} = 0.$$

Thereby, the final form of the inverse model will be:

$$i(\dot{x}, F_{MR}) = \frac{-a_1 \pm \sqrt{a_1^2 - 4a_2a_0}}{2a_2} \quad (8)$$

where

$$\begin{aligned} a_0 &= \kappa_{\dot{x},b} \dot{x}(t) + \kappa_{w,c} \operatorname{sgn}(\dot{x}) - F_{MR}(t) \\ a_1 &= [\kappa_{\dot{x},a} \dot{x}(t) + \kappa_{w,b} \operatorname{sgn}(\dot{x})] \\ a_2 &= \kappa_{w,a} \operatorname{sgn}(\dot{x}) \end{aligned}$$

3.2 The selection of the command current i

It is well known that the force generated by the MR damper cannot be commanded; only the voltage v or the current i applied to the MR damper can be directly changed [7]. In the clipped-optimal control algorithm [7], the command voltage takes the values zero or the maximum, according to

$$v = V_{\max} H\{(f_d - F_{MR})F_{MR}\}, \quad (9)$$

where V_{\max} is the maximum voltage to the current driver associated with saturation of the magnetic field in the MR damper, $H(\cdot)$ is the Heaviside step function, f_d is the desired control force and F_{MR} is the measured force of the MR damper. In some situations, when the dominant frequencies of the system under control are low, large changes in the forces applied to the structure may result in high local acceleration [26]. In this sense, a modification to the original clipped-optimal control algorithm in which the control voltage can be any value between zero and a V_{\max} , was proposed in [26], where the control voltage is determined using a linear relationship between the applied voltage and the maximum force of MR damper. A similar approach can be found in [8], where a force-feedback control scheme is employed to overcome the difficulty of commanding the MR damper to produce an arbitrary force. In this paper we consider the same idea of changing the voltage but changing the current according to the inverse model in equation (8). More precisely, to induce the MR damper to generate approximately the desired control force f_d , the algorithm for selecting the command signal can be concisely stated as

$$i = \frac{-a_1 \pm \sqrt{a_1^2 - 4a_2a_0}}{2a_2} \quad (10)$$

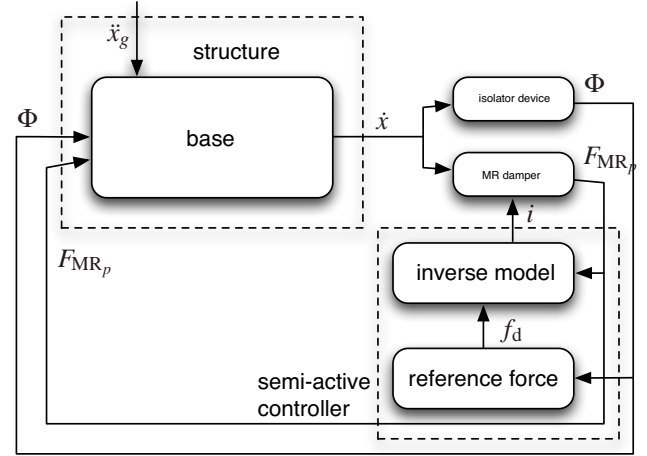


Figure 2. Block diagram of the semi-active control system for a single MR damper.

where

$$\begin{aligned} a_0 &= \kappa_{\dot{x},b} \dot{x}(t) + \kappa_{w,c} \operatorname{sgn}(\dot{x}) - f_d(t) \\ a_1 &= [\kappa_{\dot{x},a} \dot{x}(t) + \kappa_{w,b} \operatorname{sgn}(\dot{x})] \\ a_2 &= \kappa_{w,a} \operatorname{sgn}(\dot{x}) \end{aligned}$$

and f_d is computed according to

$$f_d = -\zeta \dot{\Phi}. \quad (11)$$

Both equations (10)-(11) define a semi-active controller. Figure 2 illustrates the corresponding closed-loop system.

Based on this desired force, the corresponding command current that has to be applied to the damper will be calculated according to equation (10). Let i_α and i_β be the two roots of this equation and let $[0, i_{\max}]$ be the range of admissible current. The applied current i_a will finally be

$$i_a = \begin{cases} \max\{i_\alpha, i_\beta\}, & i_\alpha, i_\beta \in [0, i_{\max}] \\ \min\{i_\alpha, i_\beta\}, & \min\{i_\alpha, i_\beta\} \in [0, i_{\max}], \\ & \max\{i_\alpha, i_\beta\} \notin [0, i_{\max}] \\ i_{\max}, & i_\alpha, i_\beta > 0, \min\{i_\alpha, i_\beta\} \notin [0, i_{\max}] \\ \max\{i_\alpha, i_\beta\}, & \min\{i_\alpha, i_\beta\} < 0, \\ & \max\{i_\alpha, i_\beta\} \in [0, i_{\max}] \\ i_{\max}, & \min\{i_\alpha, i_\beta\} < 0, \\ & \max\{i_\alpha, i_\beta\} \geq i_{\max} \\ 0, & \max\{i_\alpha, i_\beta\} < 0 \\ \Re(i_\alpha), & i_\alpha \in \mathbb{C} \setminus \mathbb{R}, 0 \leq \Re(i_\alpha) \leq i_{\max} \\ 0, & i_\alpha \in \mathbb{C} \setminus \mathbb{R}, \Re(i_\alpha) < 0 \\ i_{\max}, & i_\alpha \in \mathbb{C} \setminus \mathbb{R}, \Re(i_\alpha) > i_{\max} \end{cases}$$

In the implementation of this formula, the values are truncated between zero and i_{\max} when the current does not belong to the range of admissible values. When the roots are complex conjugates, we just consider the real part, and then we apply the same truncation.

This process helps the damping force generated by the MR dampers become more closer to the desired control force, f_d .

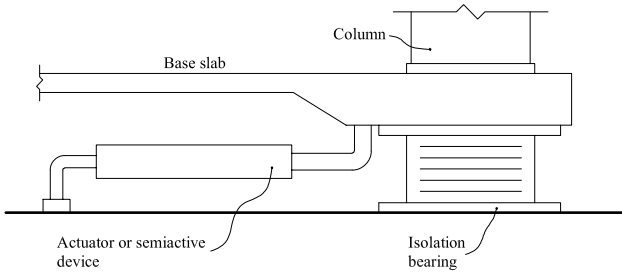


Figure 3. Elevation view with devices.

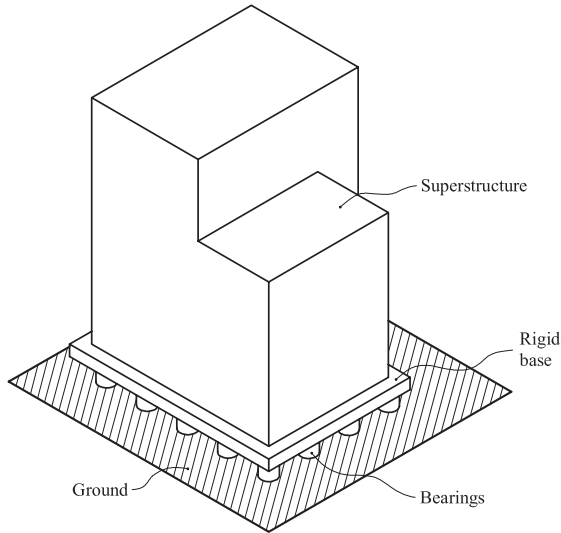


Figure 4. A representative figure of the benchmark structure.

4 NUMERICAL RESULTS

The performance of the semi-active control algorithm presented in Section 3 is now evaluated through numerical simulation using the smart base-isolated benchmark building. The smart base-isolated benchmark building [16] is employed as an interesting and more realistic example to further investigate the effectiveness of the proposed design approach. This base-isolated building is an eight-storey frame building, similar to existing building in Los Angeles, California. The results are also compared with the clipped-optimal control algorithm in equation (9) [7] and also with two limit cases: *passive off* and *passive on*, that corresponds to the cases of *zero* current applied to the damper and *maximum* current applied to the damper. The evaluation is reported in terms of the performance indices described in Table 1. The controlled benchmark structure is simulated for seven earthquake ground accelerations defined in the benchmark problem (Newhall, Sylmar, El Centro, Rinaldi, Kobe, Ji-Ji and Erzinkan). The performance indices larger than 1 indicate that the response of the controlled structure is bigger than that of the uncontrolled structure. Table 2 shows the average evaluation criteria for all 14 cases (the seven prescribed earthquakes in two orthogonal directions). The performance indices larger than one in Table 2 are underlined. Figure 5 show the time history response of the base-isolated building under the Erzinkan earthquake for different control cases. It is clear in

Figure 5 that the proposed semi-active controller reduces the base displacements in 75% from the passive-off case.

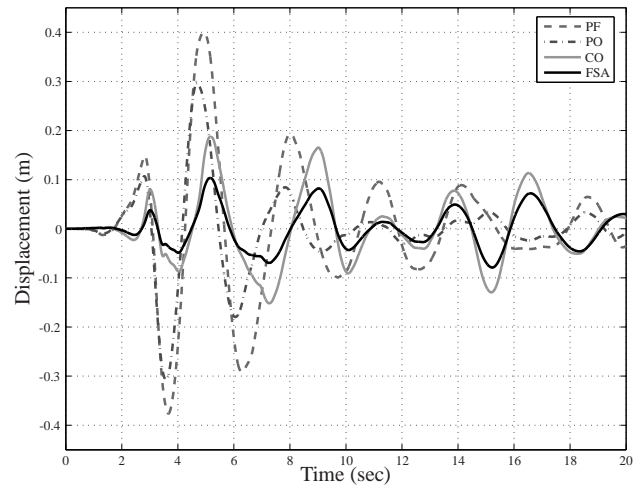


Figure 5. Response time history of the building subjected to Erzinkan earthquake (FP-y and FN-x) base displacement at the center of mass (2000kN MR damper)

Table 1. Performance indices, where, $i =$ isolator number, $1, \dots, n_{MR}$; $k =$ device number, $1, \dots, N_d$; $f =$ floor number, $1, \dots, N_f$; $q =$ earthquake number, $1, \dots, 7$; $t =$ time, $0 \leq t \leq T_q$; $\langle \cdot \rangle =$ inner product; $\| \cdot \| =$ vector magnitude incorporating NS and EW components.

Peak base shear	Peak structure shear
$J_1 = \frac{\max_t \ V_0(t,q)\ }{\max_t \ V_0(t,q)\ }$	$J_2 = \frac{\max_t \ V_1(t,q)\ }{\max_t \ V_1(t,q)\ }$
Peak base displacement	Peak inter-storey drift
$J_3 = \frac{\max_{t,i} \ d_i(t,q)\ }{\max_{t,i} \ d_i(t,q)\ }$	$J_4 = \frac{\max_{t,f} \ d_f(t,q)\ }{\max_{t,f} \ d_f(t,q)\ }$
Peak Floor acceleration	Peak control force
$J_5 = \frac{\max_{t,f} \ a_f(t,q)\ }{\max_{t,f} \ a_f(t,q)\ }$	$J_6 = \frac{\max_t \ \sum_k F_k(t,q)\ }{\max_t \ V_0(t,q)\ }$
RMS base displacement	RMS floor acceleration
$J_7 = \frac{\max_i \ \sigma_d(t,q)\ }{\max_i \ \sigma_d(t,q)\ }$	$J_8 = \frac{\max_f \ \sigma_a(t,q)\ }{\max_f \ \sigma_a(t,q)\ }$
Energy dissipated by MR damper	
$J_9 = \frac{\sum_k \int_0^{T_q} F_k(t,q) v_k(t,q) dt}{\int_0^{T_q} (V_0(t,q) \dot{U}_g(t,q) dt)}$	

5 CONCLUDING REMARKS

In this paper we have considered a hybrid system in which an active feedback control law has been derived to be applied in parallel with a passive isolation device. The active control uses the restoring force supplied by the passive isolator as the main feedback signal. A set of numerical simulations have been performed when the force is supplied in a semi-active way to validate and discuss the efficiency of the approach in a more realistic scenario. With respect to the implementation issues, a new practical method has been defined to compute the command voltage of the dampers according to the desired control force. The whole method is simulated by considering a three-dimensional smart base-isolated benchmark building which is used by the structural control community as a state-of-the-art model for numerical experiments of seismic control attenuation. The performance indices demonstrate that the

Table 2. Average evaluation criteria for different controllers using 2000 kN MR dampers for earthquake excitations in both directions.

Indices	F. Feedback	Clipped	Passive-On	Passive-Off
J_1	0.83	0.82	0.81	0.83
J_2	0.83	0.81	0.82	0.86
J_3	0.59	0.54	0.48	0.41
J_4	0.85	0.87	0.94	<u>1.09</u>
J_5	0.92	<u>1.00</u>	<u>1.15</u>	<u>1.40</u>
J_6	0.29	0.33	0.45	0.14
J_7	0.44	0.36	0.28	0.23
J_8	0.59	0.57	0.59	0.58
J_9	0.73	0.79	0.85	0.88

proposed semi-active method can effectively suppress structural vibration caused by earthquake loading and can provide a desirable effect of structural performance.

APPENDIX

THE MAGNETORHEOLOGICAL DAMPER MODEL

The normalized version of the Bouc–Wen model [10] is an equivalent representation of the original Bouc–Wen model [24]. For MR dampers in shear mode it takes the form:

$$F_{MR}(t) = \kappa_{\dot{x}}\dot{x}(t) + \kappa_w w(t), \tag{12}$$

$$\dot{w}(t) = \rho(\dot{x}(t) - \sigma|\dot{x}(t)||w(t)|^{n-1}w(t) + (\sigma - 1)\dot{x}(t)|w(t)|^n), \tag{13}$$

where $F_{MR}(t)$ is the output force of the MR damper, and $\dot{x}(t)$ is the velocity. The system parameters, which are current-dependent, are $\kappa_{\dot{x}}(i) > 0$, $\kappa_w(i) > 0$, $\rho(i) > 0$, $\sigma(i) > 1/2$, and $n(i) \geq 1$. These parameters control the shape of the hysteresis loop and their meaning can be found in [11]. The state variable $w(t)$ has not a physical meaning so that it is not accessible to measurements.

The MR damper model in equations (14)-(15), which is based on the normalized Bouc–Wen model in equations (12)-(13), was obtained from a model validation of a large-scale magnetorheological damper at Kinki University (Osaka, Japan) using both constant and varying current along with a varying displacement signal [20]. This normalized Bouc–Wen model depicts its current dependent parameters in equations (17)-(19):

$$F_{MR}(t) = [\kappa_{\dot{x},a}i(t) + \kappa_{\dot{x},b}] \dot{x}(t) + [\kappa_{w,a}i^2(t) + \kappa_{w,b}i(t) + \kappa_{w,c}] w(t) \tag{14}$$

$$\dot{w}(t) = [\rho_a i^2(t) + \rho_b i(t) + \rho_c] \cdot (\dot{x}(t) - \sigma|\dot{x}(t)||w(t)|^{n-1}w(t) + (\sigma - 1)\dot{x}(t)|w(t)|^n) \tag{15}$$

$$w(0) = \frac{F(0) - [\kappa_{\dot{x},a}i(0) + \kappa_{\dot{x},b}] \dot{x}(0)}{[\kappa_{w,a}i^2(0) + \kappa_{w,b}i(0) + \kappa_{w,c}]} \tag{16}$$

$$\kappa_{\dot{x}}(i) = \kappa_{\dot{x},a}i + \kappa_{\dot{x},b} = 328.47i + 35.14 \text{ N/mm} \tag{17}$$

$$\begin{aligned} \kappa_w(i) &= \kappa_{w,a}i^2 + \kappa_{w,b}i + \kappa_{w,c} \\ &= -136910i^2 + 62530i + 600 \text{ N} \end{aligned} \tag{18}$$

$$\begin{aligned} \rho(i) &= \rho_a i^2 + \rho_b i + \rho_c \\ &= 12.25i^2 - 3.8i + 0.93 \text{ mm}^{-1} \end{aligned} \tag{19}$$

The parameters n and σ barely show dependence on the current; therefore, constant average values are considered, $n = 1.63$ and $\sigma = 1.85$.

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