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### Hydrodynamic model of the Great Bay estuarine system. Part 1

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# **Hydrodynamic Model of The Great Bay Estuarine System**

**UNH Sea Grant  
Technical Report UNH-SG-153**

HYDRODYNAMIC MODEL OF THE GREAT BAY ESTUARINE SYSTEM  
PART I

by

Barbaros Celikkol  
Ronald Reichard

Report No.: UNH-SG-153

This publication is a result of research sponsored by NOAA Office of Sea Grant,  
Department of Commerce, under Grant No. 04-5-158-50 (SGR/CZ-2).

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University of New Hampshire  
Mechanics Research Laboratory  
August 1976



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## LIST OF NOTATION

A	-	Tidal amplitude
C	-	Generalized bottom friction coefficient
$C_f$	-	Connor and Wang bottom friction coefficient
$C_h$	-	Chezy bottom friction coefficient (Leendertse)
f	-	Coriolis effect ( $f = 2\Omega \sin \phi$ , where $\phi$ is the latitude)
$F_{xx, yy, xy}$	-	Constitutive relations for eddy viscosity terms
$F_p$	-	Constitutive relation for pressure effects
g	-	Gravity
h	-	Water depth with respect to mean water level
H	-	Total water depth ( $H = h + \eta$ )
L	-	Tidal wavelength
p	-	Pressure
$p'$	-	Pressure fluctuation with respect to the ensemble average pressure
$p''$	-	Pressure fluctuation with respect to the vertical average pressure
$\bar{p}$	-	Ensemble average pressure
P	-	Vertical average of ensemble average pressure
$P_a$	-	Atmospheric pressure
$q_{x, y}$	-	Vertical average velocity times water depth ( $q_x = HU$ )
$r_{ij}$	-	Reynolds stress ( $r_{ij} = \overline{\rho_0 u_i' u_j'}$ )
$r_{ij}''$	-	Fluctuation of Reynolds stress with respect to the vertical average Reynolds stress
$R_{ij}$	-	Vertical average Reynolds stress
t	-	Time
$u_i$	-	Tensor notation velocity component
$u_i'$	-	Fluctuation of the velocity component with respect to the ensemble average velocity component
$u_i''$	-	Fluctuation of the velocity component with respect to the vertical average velocity component
$\bar{u}_i$	-	Ensemble average velocity component
$U_i$	-	Vertical average of the ensemble average velocity component
U	-	Vertical average velocity component in the x direction
V	-	Vertical average velocity component in the y direction

- $x_i$  - Cartesian direction in tensor notation
- $x$  - Cartesian horizontal direction
- $y$  - Cartesian horizontal direction perpendicular to  $x$
- $\epsilon_{ij}$  - Eddy viscosity coefficient
- $\nu$  - Viscosity coefficient in the Navier-Stokes Equation
- $\eta$  - Water surface elevation with respect to mean water level
- $\rho$  - Water density
- $\rho_0$  - Average water density
- $\tau_{ij, xx, yy, xy}$  - Internal stress term (vertical average of products of vertical average velocity fluctuations)
- $\tau^b$  - Bottom stress term
- $\tau^s$  - Surface stress term
- $\omega$  - Tidal frequency
- $\Omega$  - Earth's frequency of rotation
- $\xi$  - Viscosity coefficient in the Navier-Stokes Equation

#### ABSTRACT

A numerical hydrodynamic model developed by Connor and Wang has been applied to the Great Bay Estuary system. The model, using the finite element method, was found to be better suited to the complexities of the Great Bay Estuary system than Leendertse's finite difference model. Initial model development has been completed, and qualitatively acceptable results are presented. A numerical experiment was conducted to develop a procedure to be used for selection of critical model parameters in the calibration process. The general scheme for model calibration is presented, and is ready to be implemented, pending receipt of current data collected last summer by the National Ocean Survey.





## INTRODUCTION

The Great Bay Estuary system and the surrounding area is a large part of the New Hampshire seacoast area. Much of the area immediately adjacent to the estuary is underdeveloped, and plans for future development and protection of the estuary are being considered at this time. The Great Bay Estuary System Modeling Project is an attempt to describe the dynamics of the estuary and to predict the effect on the estuary of possible development schemes, thus providing a quantitative basis for the decision-makers. The initial step in this program is the development of a computer-based numerical hydrodynamic model to predict the water movements in the estuary. Two mathematical models, based on the conservation of mass and momentum equations, have been studied in detail. Connor and Wang's two-dimensional finite element model was selected over Leendertse's two-dimensional finite difference model as being more suited for this particular application. Initial model application has been carried out, including a series of tests to establish a basis for selecting appropriate values of the various model parameters when calibrating the models. The models are qualitatively acceptable, and are ready to be calibrated quantitatively. A complete set of field data, from the UNH/NOS (National Ocean Survey) cooperative field program, will soon be available for use in the calibration process.

## A PHYSICAL DESCRIPTION OF THE GREAT BAY ESTUARY SYSTEM

The Great Bay Estuary System is located in the New Hampshire seacoast region, and forms part of the New Hampshire-Maine boundary. Its area of 45 square kilometers makes it one of the largest estuaries opening on the Gulf of Maine. The Great Bay estuary system and its tributaries have a drainage area of about 2,410 kilometers.

The geometry of the estuary is complex, but lends itself to division of the estuary into segments (see Figure 1). Portsmouth Harbor is the mouth of the estuary and, together with the lower Piscataqua River, serves as the only seaport for New Hampshire. This area can be described as a channel, with several islands bordering the Portsmouth Harbor section. The channel is dredged in places to maintain a minimum 10.5 M (35 feet) depth, although it is 12 to 15 M deep for most of its length. This section's tidal prism is the lowest in the system, but the section is dominated by the tidal flow of the entire system. The currents are large, approaching a maximum of two to three meters per second at mid-ebb and mid-flood in the narrower sections. The average tidal range at Portsmouth is about 2.5 M, falling to 1.9 M at Dover Point.

The upper Piscataqua River is formed by the convergence of the Cocheco and Salmon Falls Rivers in Dover. It is the shallowest area in the estuary, with a mean depth of about two meters. The upper Piscataqua River is characterized by a channel approximately five meters deep, with tidal flats on both sides. The tidal currents are much weaker than the lower Piscataqua, as it is only affected by its own tidal prism. Little Bay is an L-shaped segment of the estuary joining the Piscataqua River at Dover Point, and the Great Bay at Adams Point. It is characterized by a channel with tidal flats on both sides. Two of the system's tributaries, the Bellamy and Oyster Rivers, flow into Little Bay. Little Bay turns sharply at Fox Point, creating complex flow patterns and a great deal of turbulence. It is dominated by tidal flow (including Great Bay effects), and has currents of two-three m/s at Dover Point due to the combined effect of large tidal prism and shallow depth at this point.

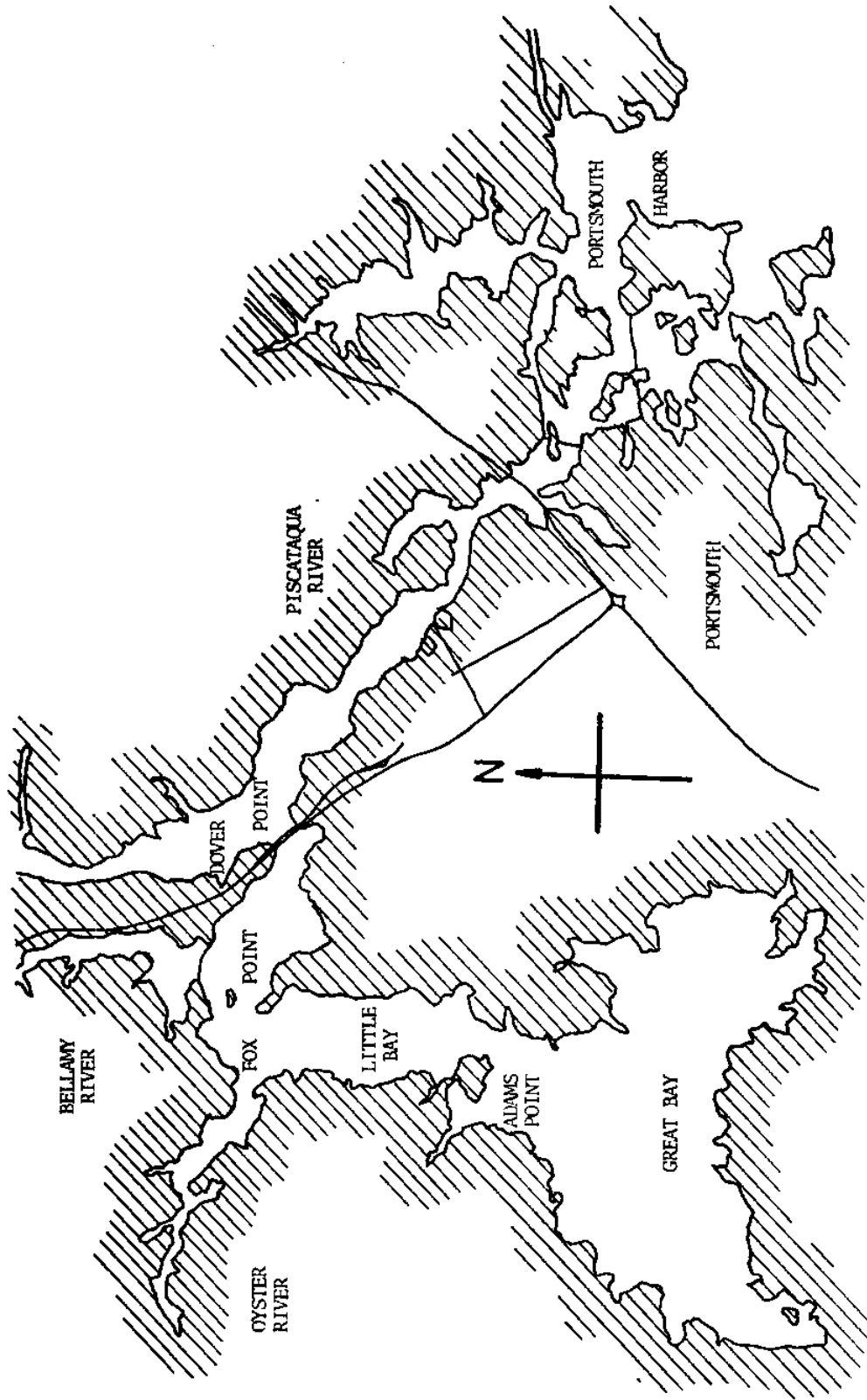
The last segment is Great Bay proper, a wide, shallow (2.7 M average) bay characterized by tidal flats, and a network of channels.

A small channel from the Winnicut River, and a larger one from the Squamscott and Lamprey Rivers join in the center of the Bay and form the main channel, which connects with Little Bay at Adams Point.

The freshwater discharge into the estuary varies greatly, with about half of the runoff in the months of March and April. Consequently, for the remainder of the year, freshwater discharge is of relatively minor importance when compared with tidal effects.

Appendix I contains details of the bathymetry of the Great Bay Estuary System (Shevenell). The data was obtained from U.S. Coast and Geodetic Survey maps 210 and 211. Areas of the segments and their depth contours were obtained through use of a planimeter. The bathymetry presented by these charts is the best complete set available, but a comprehensive bathymetric survey is needed.

FIGURE 1  
GREAT BAY ESTUARY SYSTEM



## PROJECT FORMULATION

The estuary is one of the most important recreational areas for the State of New Hampshire and southern Maine. Its relatively unpolluted waters are abundant with fish and other wildlife. Recently, a great deal of political and economic pressure has been brought to bear on the state to industrialize the land surrounding the estuary. The University of New Hampshire undertook a comprehensive study on the impact of an oil refinery located on the land adjacent to the estuary. One of the greatest problems the study team faced was the absence of a quantitative basis to determine the effects of such a development. Furthermore, the state is preparing a master plan for sewage control in the seacoast region. The plan deals with prediction of future sewage levels and treatment methods, but is based on qualitative understanding of the estuary, rather than a quantitative study.

To meet the needs of the state for better information on environmental effects of future development plans for the Great Bay Estuary system, a quantitative study is needed. The study must include description of present conditions, as well as predictive capabilities. Comprehensive data collection can be used to prescribe present conditions, but a model of the estuary is needed in order to predict changes resulting from environmental alterations.

At present, two types of models can be constructed for physical and chemical predictions. The first is a scale physical model of the estuary. Boundary conditions are specified by pumping in water with appropriate physical and chemical properties. Data is then collected in the model by measuring various parameters with laboratory instruments. The model is modified until it can predict present conditions, and then altered to reflect development plans for prediction of the impact of these plans on the estuary. The physical model has been well-developed over the years, and its limitations, which are considerable, are well known.

The second type of model is a mathematical model. The geometry of the estuary is simulated through a series of three-dimensional, geometrically regular shapes called a grid. The equations representing the process are solved numerically for this grid. The model is calibrated to simulate present conditions; future conditions are simulated by altering the model in accordance with future development plans. Mathematical modeling of estuaries has developed rapidly in the last decade, and is now gaining acceptance. There is still much work to be done in this area, but mathematical models are already replacing physical models for several types of applications in estuarine and coastal waters.

Several mathematical models are currently available for estuarine processes. Hydrodynamic models, predicting the tides and currents, are the most advanced, and are used as a basis for dispersion models. Some hydrodynamic models have dispersion equations built in, while others have a companion dispersion model sharing the same grid and using the output of the hydrodynamic model. Most dispersion models predict concentrations of conservative and non-conservative substances. The theoretical basis of hydrodynamic models is reviewed in the next section.



## GOVERNING EQUATIONS

The equations governing the motion in an estuary are the three momentum equations and the continuity equation. In their general form, the analytical solutions are not available. The differences in theory among investigators are the simplifying assumptions imposed to obtain a solvable set of equations.

In tensor notation the generalized equations of momentum and continuity for estuaries can be expressed by the following two equations:

a) The time rate of change of momentum of a moving fluid particle is equal to the sum of the forces acting on it:

$$\frac{\partial}{\partial t} (\rho u_i) + \frac{\partial}{\partial x_j} (\rho u_i u_j) = - \frac{\partial p}{\partial x_i} - 2 \xi_{ijl} \Omega_j \rho u_l + \rho g \delta_{i3} + \frac{\partial}{\partial x_j} \left[ \mu \left( \frac{\partial u_i}{\partial x_j} - \frac{2}{3} \delta_{ij} \frac{\partial u_k}{\partial x_k} \right) \right] + \frac{\partial}{\partial x_j} \left( \xi \frac{\partial u_k}{\partial x_k} \right)$$

b) The mass of a moving element of fluid remains constant:

$$\frac{\partial \rho}{\partial t} + \frac{\partial}{\partial x_i} (\rho u_i) = 0$$

Where  $t$  is time,  $\rho$  density,  $u$  a velocity component,  $x$  a direction,  $P$  pressure,  $\Omega$  the earth's rotation,  $g$  gravity,  $\mu$  and  $\xi$  viscous coefficients.

Applying the following assumptions:

- 1) Incompressible flow ( $\frac{\partial \rho}{\partial p} = 0$ )
- 2)  $\rho$  (density) =  $\rho_0$  (constant) +  $\delta \rho$  (a small perturbation term)
- 3) Viscosity coefficient is constant ( $\mu = \mu_0$ ).
- 4) The second derivative of velocity with respect to perpendicular coordinates is small:

$$\left( \frac{\partial^2 u_i}{\partial x_j^2} = 0 \neq j \right)$$

The equations are simplified to obtain:

$$\frac{\partial u_i}{\partial t} + \frac{\partial}{\partial x_j} (u_i u_j) = - \frac{1}{\rho_0} \frac{\partial p}{\partial x_i} - 2 \xi_{ijl} \Omega_j u_l + g \delta_{i3}$$

$$\frac{\partial u_i}{\partial x_i} = 0$$

Representing the variables as the sum of their ensemble average and a fluctuation about the ensemble average,

$$u_i = \bar{u}_i + u'$$

$$p = \bar{p} + p'$$

Where the overbar denotes the ensemble average, and the prime denotes the fluctuation term, the equations can be ensemble averaged to obtain:

$$\frac{\partial \bar{u}_i}{\partial t} + \frac{\partial}{\partial x_j} (\bar{u}_i \bar{u}_j) = - \frac{1}{\rho_0} \frac{\partial \bar{p}}{\partial x_i} - 2\epsilon_{ijl} \Omega_j \bar{u}_l + g \delta_{i3}$$

$$- \frac{1}{\rho_0} \frac{\partial}{\partial x_j} \overline{\rho_0 u_i' u_j'}$$

$$\frac{\partial \bar{u}_i}{\partial x_i} = 0$$

This averaging technique smooths the stochastic processes while retaining the deterministic processes. The additional term is called the Reynolds stress,  $r$ , and is the ensemble average of the product of velocity fluctuations with respect to the ensemble average velocity, multiplied by density:

$$r_{ij} = \overline{\rho_0 u_i' u_j'}$$

Assuming vertical variations of the various parameters are small, the equations may be vertically averaged, and the vertical momentum equation reduced to the hydrostatic relation, without loss of meaning. Representing the variables as the sum of their vertical average and a fluctuation about the vertical average:

$$\bar{u}_i = U_i + u_i''$$

$$\bar{p} = P + p''$$

$$r_{ij} = R_{ij} + r_{ij}''$$

Where capital letters indicate the vertical average values, and double prime denotes the fluctuation about the vertical average, the vertically averaged equations are:

$$\frac{\partial U}{\partial t} + \frac{\partial U^2}{\partial x} + \frac{\partial(UV)}{\partial y} = - \frac{1}{\rho_0} \frac{\partial P_a}{\partial x} + fV + g \frac{\partial \eta}{\partial x}$$

$$- \frac{1}{\rho_0} \frac{\partial R_{xx}}{\partial x} - \frac{1}{\rho_0} \frac{\partial R_{xy}}{\partial y} + \frac{1}{\rho_0} \frac{\partial \tau_{xx}}{\partial x} + \frac{1}{\rho_0} \frac{\partial \tau_{yx}}{\partial y} + \frac{(\tau_x^b + \tau_x^s)}{\rho_0 h}$$

$$\frac{\partial V}{\partial t} + \frac{\partial(UV)}{\partial x} + \frac{\partial V^2}{\partial y} = - \frac{1}{\rho_0} \frac{\partial P_a}{\partial y} - fU + g \frac{\partial \eta}{\partial y}$$

$$- \frac{1}{\rho_0} \frac{\partial R_{yy}}{\partial y} - \frac{1}{\rho_0} \frac{\partial R_{xy}}{\partial x} + \frac{1}{\rho_0} \frac{\partial \tau_{yy}}{\partial y} + \frac{1}{\rho_0} \frac{\partial \tau_{yx}}{\partial x} + \frac{(\tau_y^b + \tau_y^s)}{\rho_0 h}$$

$$\frac{1}{H} \frac{\partial \eta}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = 0$$

Where  $U$  and  $V$  are horizontal velocity components,  $P_a$  atmospheric pressure,  $f$  the coriolis effect,  $\eta$  the water height above mean water level (MNL),  $R$  the Reynolds stress, and  $H$  the total water depth. The product of two velocity fluctuations (with respect to the vertical average) are represented as the internal stress terms  $\tau_{xx}$ ,  $\tau_{yy}$ ,  $\tau_{xy}$ , and the bottom stress  $\tau_x^s$ ,  $\tau_y^s$ . Reynolds and internal stress terms cannot be directly included in the equations, and are usually neglected as being small. These effects can be included by assuming a functional



relationship with the horizontal velocity gradient as follows:

$$\frac{1}{\rho_0} (\tau_{ij} - R_{ij}) = \epsilon_{ij} \left( \frac{\partial U_i}{\partial x_j} + \frac{\partial U_j}{\partial x_i} \right)$$

Where the coefficient  $\epsilon$  is called the eddy viscosity coefficient. The bottom stress  $\tau_x^b$ ,  $\tau_y^b$  is assumed proportional to a quadratic function of velocity:

$$\frac{\tau_x^b}{\rho_0 h} = \frac{CV (U^2 + V^2)^{\frac{1}{2}}}{h}$$

$$\frac{\tau_y^b}{\rho_0 h} = \frac{CV (U^2 + V^2)^{\frac{1}{2}}}{h}$$

Where C is the bottom friction coefficient. The equations may now be expressed in the following form:

$$\begin{aligned} \frac{\partial U}{\partial t} + \frac{\partial U^2}{\partial x} + \frac{\partial(UV)}{\partial y} &= fV + g \frac{\partial n}{\partial x} \\ &+ \frac{\partial}{\partial x} (2\epsilon_{xx} \frac{\partial U}{\partial x}) + \frac{\partial}{\partial y} [\epsilon_{yx} (\frac{\partial V}{\partial x} + \frac{\partial U}{\partial y})] \\ &+ \frac{C U (U^2 + V^2)^{\frac{1}{2}}}{h} \\ \frac{\partial V}{\partial t} + \frac{\partial(UV)}{\partial x} + \frac{\partial V^2}{\partial y} &= -fU + g \frac{\partial n}{\partial y} \\ &+ \frac{\partial}{\partial y} (2\epsilon_{yy} \frac{\partial V}{\partial y}) + \frac{\partial}{\partial x} [\epsilon_{xy} (\frac{\partial U}{\partial y} + \frac{\partial V}{\partial x})] \\ &+ \frac{CV (U^2 + V^2)^{\frac{1}{2}}}{h} \\ \frac{1}{H} \frac{\partial n}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} &= 0 \end{aligned}$$

The atmospheric pressure gradient and the surface stress terms have been omitted. They can be important for specific atmospheric conditions, but in general they are not important, and the difficulty in specifying these terms makes their inclusion questionable at best.

The left hand side of the momentum equations is composed of the temporal and convective acceleration terms. The right hand side of the equations is composed of the forces acting on a fluid particle. The surface slope and bottom friction terms are the dominant forces, while the coriolis force and eddy viscosity term are secondary effects. The surface slope, acting as a hydraulic head, forces the flow, while the bottom friction is the primary resisting force.



### LEENDERTSE'S FINITE DIFFERENCE MODEL

The two-dimensional finite difference solution technique developed by Leendertse is generally accepted and has broad application. The conservation of momentum and mass equations are reduced to a two-dimensional, vertically averaged form for solution:

$$\frac{\partial U}{\partial t} + U \frac{\partial U}{\partial x} + V \frac{\partial U}{\partial y} = fV + g \frac{\partial \eta}{\partial x} + g \frac{U(U^2 + V^2)^{1/2}}{C_h^2 H} + \frac{\tau_x^s}{\rho h}$$

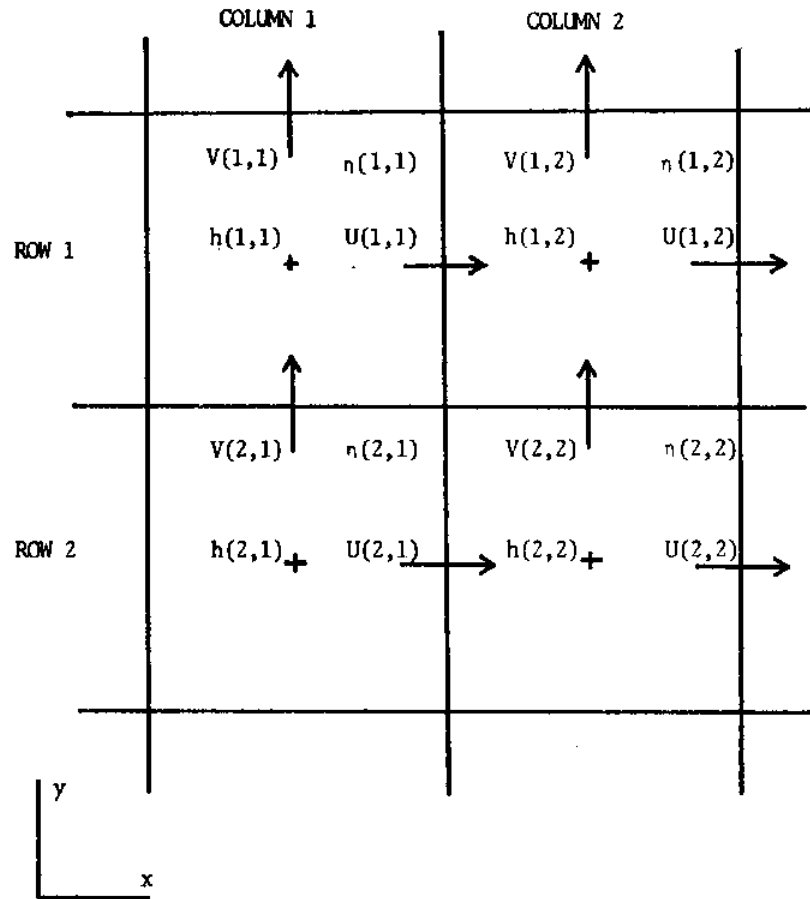
$$\frac{\partial V}{\partial t} + U \frac{\partial V}{\partial x} + V \frac{\partial V}{\partial y} = -fU + g \frac{\partial \eta}{\partial y} + g \frac{V(U^2 + V^2)^{1/2}}{C_h^2 H} + \frac{\tau_y^s}{\rho h}$$

$$\frac{\partial \eta}{\partial t} + \frac{\partial}{\partial x} (HU) + \frac{\partial}{\partial y} (HV) = 0$$

U and V are the vertically averaged velocities in the x and y directions respectively,  $\eta$  is the elevation of the water surface above the mean water level (MWL), h is the depth of the water from MWL, H the total water depth which equals  $h + \eta$ ,  $C_h$  the Chezy friction coefficient,  $\tau_x^s$  and  $\tau_y^s$  are the wind stress components.

A computational grid is selected to represent the geometry of the estuary. The various parameters are staggered on the grid as shown in Figure 2. The surface elevation is computed at the grid point, the U and V velocity components are computed midway between grid points, and the MWL depth h is selected for the center of every four grid points. The time step  $\Delta t$  is divided in half, with a solution obtained for each half time step. In the first solution, the U velocities are computed along each row using an implicit finite difference technique, and then the V velocities are computed explicitly along each column. The process is reversed in the second half time step, computing V velocities implicitly, and U velocities explicitly. The final result for the time step is obtained by averaging the explicit and implicit solution for velocity components at each point.

FIGURE 2  
 THE FINITE DIFFERENCE GRID AND THE PARAMETER  
 STAGGERING SCHEME OF LEENDERTSE



Four elements, numbered by rows and columns, and the location of the variables in each element, are presented. Element (1,2) is complete, and shows the variables associated with it. In addition to its own variables, it is affected by all three of the other surface elevations, the U velocity of element (1,1), and the V velocity of element (2,2).

### CONNOR AND WANG'S FINITE ELEMENT MODEL

The two-dimensional finite element solution technique developed by Connor and Wang is a unique and promising new approach. The two-dimensional vertically averaged conservation of momentum and mass equations used in the model are:

$$\begin{aligned} \frac{\partial q_x}{\partial t} + \frac{\partial(Uq_x)}{\partial x} + \frac{\partial(Vq_y)}{\partial y} &= -\frac{\partial F_p}{\partial x} + g \frac{\partial(h\eta)}{\partial x} \\ &+ fq_y + \frac{\partial F_{xx}}{\partial x} + \frac{\partial F_{yx}}{\partial y} + (\tau_x^b - \tau_x^s) \\ \frac{\partial q_y}{\partial t} + \frac{\partial}{\partial x}(Vq_x) + \frac{\partial}{\partial y}(Uq_y) &= -\frac{\partial F_p}{\partial y} + g \frac{\partial}{\partial y}(h\eta) \\ &-fq_x + \frac{\partial F_{yy}}{\partial y} + \frac{\partial F_{xy}}{\partial x} + (\tau_y^b - \tau_y^s) \\ \frac{\partial \eta}{\partial t} + \frac{\partial q_x}{\partial x} + \frac{\partial q_y}{\partial y} &= q_i \end{aligned}$$

with the constitutive relations:

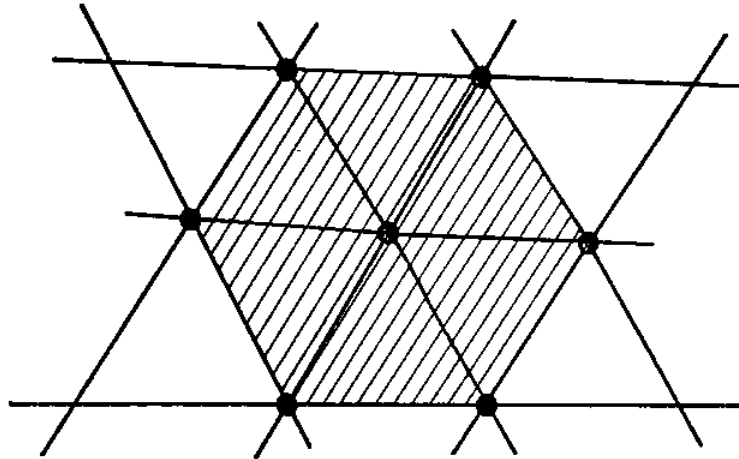
$$\begin{aligned} F_{xx} &= \int_{-h}^{\eta} (\tau_{xx} - \rho(u')^2) dz = \epsilon_{xx} \frac{\partial q_x}{\partial x} \\ F_{yy} &= \int_{-h}^{\eta} (\tau_{yy} - \rho(v')^2) dz = \epsilon_{yy} \frac{\partial q_y}{\partial y} \\ F_{xy} = F_{yx} &= \int_{-h}^{\eta} (\tau_{xy} - \rho(u'v')) dz = \epsilon_{xy} \left( \frac{\partial q_y}{\partial x} + \frac{\partial q_x}{\partial y} \right) \\ F_p &= gh\eta + 1/2 g\eta^2 + \frac{\Delta\rho}{2\rho_0} gH^2 + \frac{p^s}{\rho_0} H \\ \tau_x^b &= \frac{C_f q_x (q_x^2 + q_y^2)^{1/2}}{\rho H^2} \\ \tau_y^b &= \frac{C_f q_y (q_x^2 + q_y^2)^{1/2}}{\rho H^2} \end{aligned}$$

Where H is the total water depth, U and V the vertically averaged velocities, u' and v' the velocity fluctuations with respect to the ensemble average, q<sub>x</sub> and q<sub>y</sub> equal HU and HV respectively, h the depth of the water at MWL, η the height of the water surface above MWL, ρ<sub>0</sub> the average density, Δρ the density fluctuation, C<sub>f</sub> the bottom friction coefficient, τ<sub>xx</sub>, τ<sub>yy</sub>, τ<sub>xy</sub> are internal stresses, and ε<sub>xx</sub>, ε<sub>yy</sub>, ε<sub>xy</sub> are eddy viscosity coefficients.

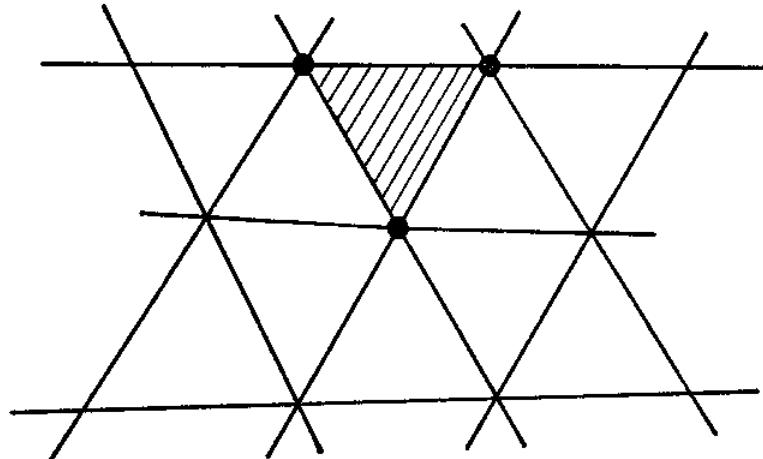
The finite element method approximates the solution of a boundary value problem with a function of piece-wise continuous polynomials. It is based on discretization of the continuum into an equivalent system of finite elements. Connor and Wang selected the simplest configuration, triangles with nodes at the angles. The values of the variables within the element have been assumed a linear function of the values at the nodes. The equations are transformed for application to an element using this linear polynomial representation. Treatment of the entire continuum is accomplished through summation of the contributions of

each element. The domain of influence of a nodal value and an element value are graphically displayed in Figure 3. Solutions for  $q_x$ ,  $q_y$  and  $n$  are obtained at each node. Depth is selected at each node point, while bottom friction and eddy viscosity are selected for each element.

FIGURE 3  
CONNOR AND WANG'S DOMAIN OF INFLUENCE FOR NODE AND ELEMENT VALUES



Domain for Influence for Node Values



Domain of Influence for Element Values

A node variable or parameter affects all of the adjacent elements (six in this example), as the value of the variable or parameter within each element is a function of the values at the nodes. An element parameter affects the three nodes of the element.

## MODEL APPLICATION

Connor and Wang's finite element model was chosen over Leendertse's finite difference model for this particular application because of the complexity of the geometry in the Great Bay Estuary system, and the need to model the estuary in segments, resulting from computational limitations. Connor and Wang's model has been applied to the Portsmouth Harbor, Piscataqua River, Little Bay and Great Bay segments of the estuary. A listing of the input data and a plot of the grid for each model are presented in Appendices III through VI respectively.

The grid was selected to coincide with the coastal and bathymetric features of the estuary as presented by United States Coast and Geodetic Survey chart No. 210. The model is not presently capable of handling mud flats, and these are, for the most part, neglected. The average distance between adjacent nodes is the characteristic length of the grid. The features included in the grid are at least as large as two characteristic lengths.

Several other factors, involving numerical stability, were considered when developing the grid. The size and shape of the elements may vary, but the most stable configuration is equilateral triangles of equal size. To preserve the grid stability, the general guidelines followed for these considerations are: (1) area of adjacent elements should vary less than 20 percent, and (2) angles of elements should be greater than 30 and less than 90 degrees. Another factor considered in selecting the grid is that an element may not have more than two of its three nodes on a land boundary. The final consideration is the number of nodes and elements to be included. The larger the number of nodes, the greater the detail, but computer core and time requirements are also increased. The maximum size of the time step is related to the characteristic length, and for this particular application the small characteristic lengths constrained the size of the time steps. As a result of these computational constraints, the grid was selected with as large a characteristic length as possible. The grids were modified several times to obtain an acceptable result.

Boundary conditions were set up as specified for the model. Land boundary conditions are handled internally by the model, forcing the flow to be tangential to the land. To accomplish this, normal angles (with respect to the land boundary) must be specified for each node on a land boundary. This task was simplified through the use of a simple auxiliary program listed in Appendix XI. Rivers are not considered in this preliminary development as their effect is secondary to the tidal prism, and are therefore treated as land boundaries. The open boundaries are treated by specifying the tidal amplitude, the tidal frequency and the phase lag. The tidal frequency was assumed standard, and is the same for all open boundaries. The tidal amplitude was selected for each open boundary, and the phase for each node on each open boundary. The values for these parameters were obtained from the Tide Tables of North America.

The bottom friction coefficient was assumed constant throughout the grid, and a medium value was chosen for preliminary runs. The eddy viscosity coefficient was assumed constant throughout the grid, with an arbitrary value selected for the  $\epsilon_{xx}$  and  $\epsilon_{yy}$  coefficients, and a smaller value for the  $\epsilon_{xy}$  coefficient.

With the data for the model set up, the model was run using a constant depth (representative of the depth of the estuary) for initial runs, as the model is sensitive to this parameter. This allowed the stability of the grid to be tested, and approximate values

for the friction and eddy viscosity coefficients to be found. After the grid had been evaluated and modified, and the coefficients evaluated, the depth of the estuary at each node was entered, again using chart No. 210. The chart data was supplemented for the Piscataqua River channel with data from a University of New Hampshire bathymetric survey. To preserve the cross sectional areas and volume characteristics of the estuary segment, slight modification of the depths was necessary. The model was run again, with varying depths, to test the stability of the fully three-dimensional grid. Large changes in depth within an element, and very shallow depths, cause numerical instabilities, requiring slight modifications. This initial development and testing has been completed, and the model is ready for calibration.



## EVALUATION OF CRITICAL MODEL PARAMETERS

The model uses two parameters which must be evaluated for the estuary: the bottom friction coefficient and the eddy viscosity coefficient. Bottom friction plays a major role in estuary dynamics. This frictional effect is expressed in the model in the form  $CU^2/h$ , where  $C$  is the friction coefficient,  $U$  the vertically averaged velocity, and  $h$  the water depth. The eddy viscosity term is a combination of Reynolds Stress terms, resulting from ensemble averaging, and internal stress terms, resulting from the vertical averaging. It is expressed in the form  $\frac{\partial}{\partial x} (\epsilon_{xx} \frac{\partial u}{\partial x})$  where  $\epsilon_{xx}$  is the eddy viscosity coefficient and  $\partial u/\partial x$  is the horizontal current gradient. Both of the coefficients are assumed constant in time, and constant for an element. They may be specified for each element individually, however, allowing spatial changes in the coefficients from element to element. The values of these parameters are selected, in a process called model calibration, to cause model results to compare favorably with field data. A procedure for selection of these parameters would greatly assist in model calibration.

The initial step in the development of such a procedure is a review of the physical processes involved. Current velocity is a function of the tidal amplitude, bottom friction coefficient, and eddy viscosity coefficient. The tidal elevation at open boundaries is used as the forcing function for the model. It is specified as  $A \cos(\omega t)$ , where  $A$  is the tidal amplitude,  $\omega$  the frequency of the tide, and  $t$  the time. The current speed is proportional to the tidal elevation, and inversely proportional to the bottom friction coefficient. The primary effect of the eddy viscosity term on current velocity is to cause a phase shift, while its effect on the current magnitude is secondary.

A numerical experiment was conducted, consisting of a series of model simulations, to determine the effects of these three parameters on the model results. A simulation consisted of running the Little Bay model for one and one-half tidal cycles, allowing half a tidal cycle for the model to settle down numerically. Three sets of simulations were run, one for each of the parameters to be tested. Each set consisted of four simulations, holding two of the parameters constant while specifying a different value of the third parameter for each simulation. One simulation was common to all three sets, so a total of ten different simulations were carried out. Table 1 presents a list of all the coefficients used for the various simulations. Both  $u$  and  $v$  components of the velocity at a specific node were output every 20 minutes of simulation time, for a total of 37 current vectors over the tidal cycle for each simulation.

This data is presented as a series of graphs of current speed and direction plotted against time. Figure 4 presents these two plots for the simulation set in which amplitude was varied. The speed increases with increasing tidal amplitude, while slight variations in curve shape indicate the presence of phase variations. The direction plot indicates that the current direction is well-behaved, and the phase variation present. This confirms that current speed is proportional to tidal amplitude, and also indicates that phase varies with tidal amplitude.

The variation of friction coefficient simulation set results are presented in Figure 5. It can be seen from the speed plot that current velocity varies inversely with respect to the friction coefficient as expected. The direction plot indicates little or no phase variation for changing friction coefficients.

TABLE 1  
SUMMARY OF PARAMETER VALUES FOR NUMERICAL EXPERIMENTS

	A	C	$\epsilon_{xx}$	$\epsilon_{yy}$	$\epsilon_{xy}$
Amplitude	0.8	0.020	18.0	18.0	8.0
	1.0	0.020	18.0	18.0	8.0
	1.2*	0.020	18.0	18.0	8.0
	1.4	0.020	18.0	18.0	8.0
Friction	1.2	0.012	18.0	18.0	8.0
Coefficient	1.2	0.016	18.0	18.0	8.0
	1.2*	0.020	18.0	18.0	8.0
	1.2	0.025	18.0	18.0	8.0
Eddy	1.2	0.020	9.0	9.0	4.0
Viscosity	1.2*	0.020	18.0	18.0	8.0
Coefficient	1.2	0.020	27.0	27.0	12.0
	1.2	0.020	36.0	36.0	16.0

\*Simulation common to all three sets

The eddy viscosity simulation set results, presented in Figure 6, include one case ( $\epsilon_{xx} = 36.0$ ) which is numerically unstable. Neglecting this case, the plots indicate a phase variation in speed with respect to the eddy viscosity coefficient.

Summarizing the results of these graphs, shows that the magnitude of the current velocity is a function of tidal amplitude and bottom friction coefficient, while the phase shift of the current is a function of tidal amplitude and the eddy viscosity coefficient. Since the tidal amplitude is determined by the physical conditions present at the boundary, calibration of the model with respect to current velocity magnitude is accomplished by changing the bottom friction coefficient, while calibration with respect to phase is carried out through modifications of the eddy viscosity coefficient.

To quantify the relations between tidal amplitude, bottom friction and current velocity, the data was normalized by dividing by a characteristic velocity. A characteristic velocity can be developed from the conservation of momentum equation for channel flow. For a one-dimensional, constant depth channel, neglecting the eddy viscosity term, the conservation of momentum equation can be written:

$$\frac{\partial U}{\partial t} + U \frac{\partial U}{\partial x} = -g \frac{\partial \eta}{\partial x} + \frac{CU^2}{h}$$

Where U is the vertically averaged current velocity,  $\eta$  the surface elevation, h the water depth, and C the friction coefficient. Evaluating the order of magnitude for each term in the equation for the Great Bay Estuary system:

$$\begin{array}{cccc} \frac{\partial U}{\partial t} & + & U \frac{\partial U}{\partial x} & = & -g \frac{\partial \eta}{\partial x} & + & \frac{CU^2}{h} \\ 10^{-4} & & 10^{-4} & & 10^{-3} & & 10^{-3} \end{array}$$

Neglecting the acceleration terms because they are an order of magnitude smaller yields:

$$g \frac{\partial \eta}{\partial x} = \frac{CU^2}{h}$$

or

$$U = \left( \frac{gh}{C} \frac{\partial \eta}{\partial x} \right)^{1/2}$$

Where U is the characteristic velocity. For  $\Delta x = L/4$ , where L is the tidal wave length,  $\Delta \eta = A$ , the tidal amplitude:

$$U = \left( \frac{4ghA}{CL} \right)^{1/2} = \left( \frac{4gh}{L} \right)^{1/2} \left( \frac{A}{C} \right)^{1/2}$$

Evaluating to the order of magnitude:

$$\left( \frac{4gh}{L} \right)^{1/2} = 10^{-1}$$

and the characteristic velocity U can be expressed as:

$$U = 0.1 \left( \frac{A}{C} \right)^{1/2}$$

The variation of the maximum normalized velocity among the simulations (with the exception of the unstable case) is less than five percent. Therefore, for a given amplitude, this formula can be used in the selection of appropriate values for the bottom friction coefficient when calibrating the model.

The relations between tidal amplitude, eddy viscosity coefficient, and current phase are very complex. A qualitative, graphic approach was taken to provide insight into the selection of proper eddy viscosity coefficients. The numerical experiment data, normalized using the above procedure, is presented as phase plane plots, or hodographs. These are plots of one current component versus the other current component over the tidal cycle. The plots for the eddy viscosity coefficient set (Figure 7) show the increasing width of the curve as eddy viscosity increases, clearly delineating the phase effects of the eddy viscosity term. Note the plot shape for the unstable simulation ( $\xi_{xx} = 36.0$ ). It can be seen that the eddy viscosity effects vary for the tidal amplitude (Figure 8) and bottom friction coefficient sets (Figure 9), despite the fact that the eddy viscosity coefficient is held constant. This occurs as the eddy viscosity term is a function of the second spatial derivative of the horizontal velocity and not the horizontal velocity itself. This indicates that the bottom friction coefficients must be evaluated first; then the eddy viscosity coefficients can be evaluated from comparison of phase plane plots of normalized model results and field data.

The results of these tests will facilitate the calibration of the models. The quantitative aspect in the relation of the tidal amplitude and friction coefficient to current velocity will eliminate much of the trial and error work previously associated with this step in the model development. The eddy viscosity coefficient must still be selected qualitatively, but this is the only parameter selected this way. Therefore, simple comparison of the shape of hodograph curves of normalized field data and model results at corresponding locations in the estuary should prove adequate.

FIGURE 4  
 CURRENT SPEED AND DIRECTION PLOTS FOR THE TIDAL AMPLITUDE EXPERIMENT

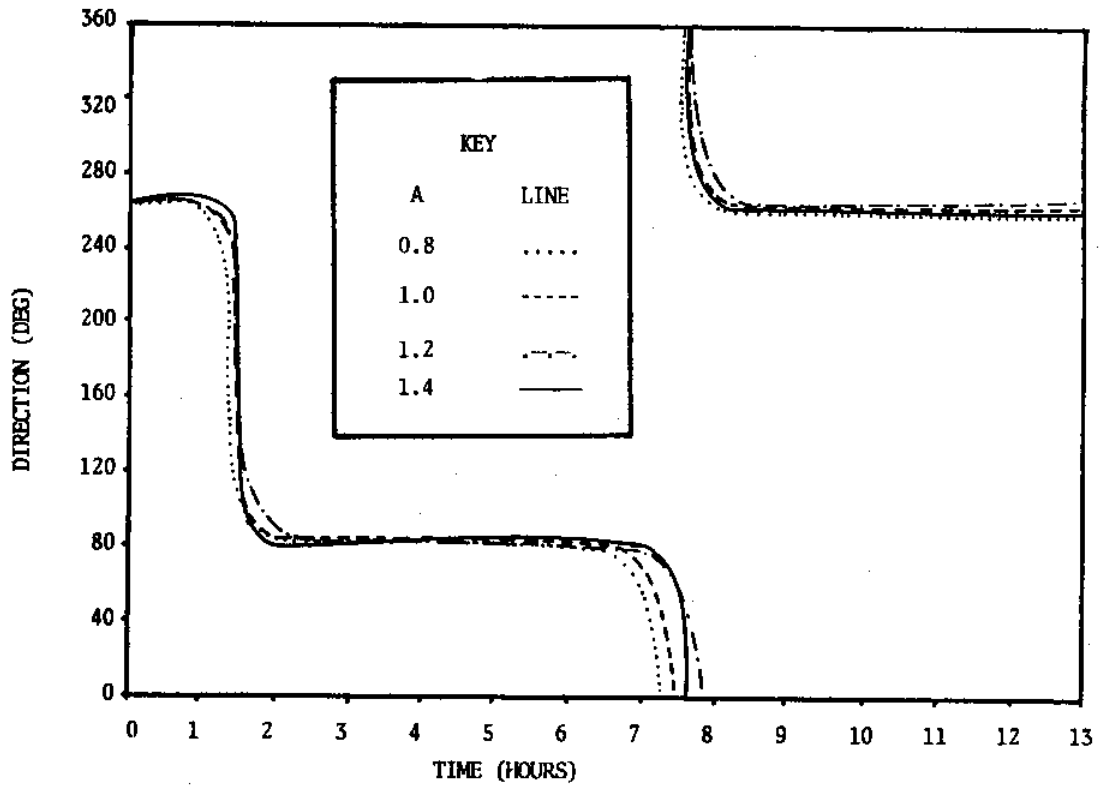
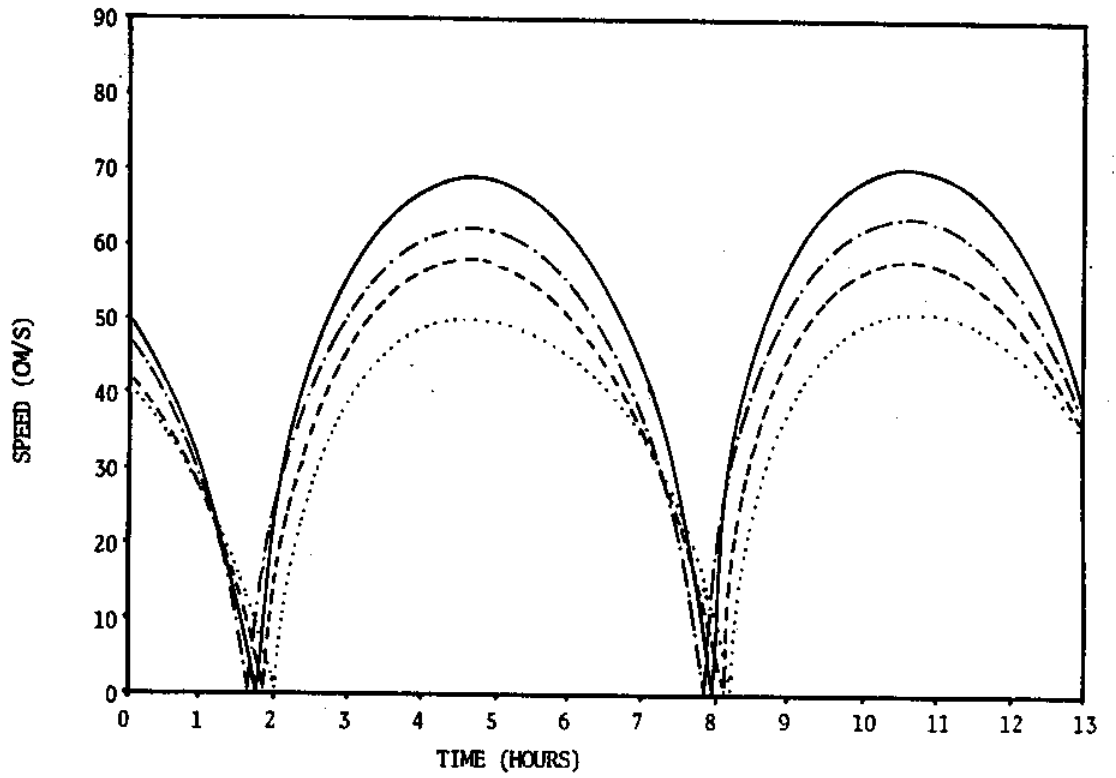


FIGURE 5  
CURRENT SPEED AND DIRECTION PLOTS FOR THE BOTTOM FRICTION EXPERIMENT

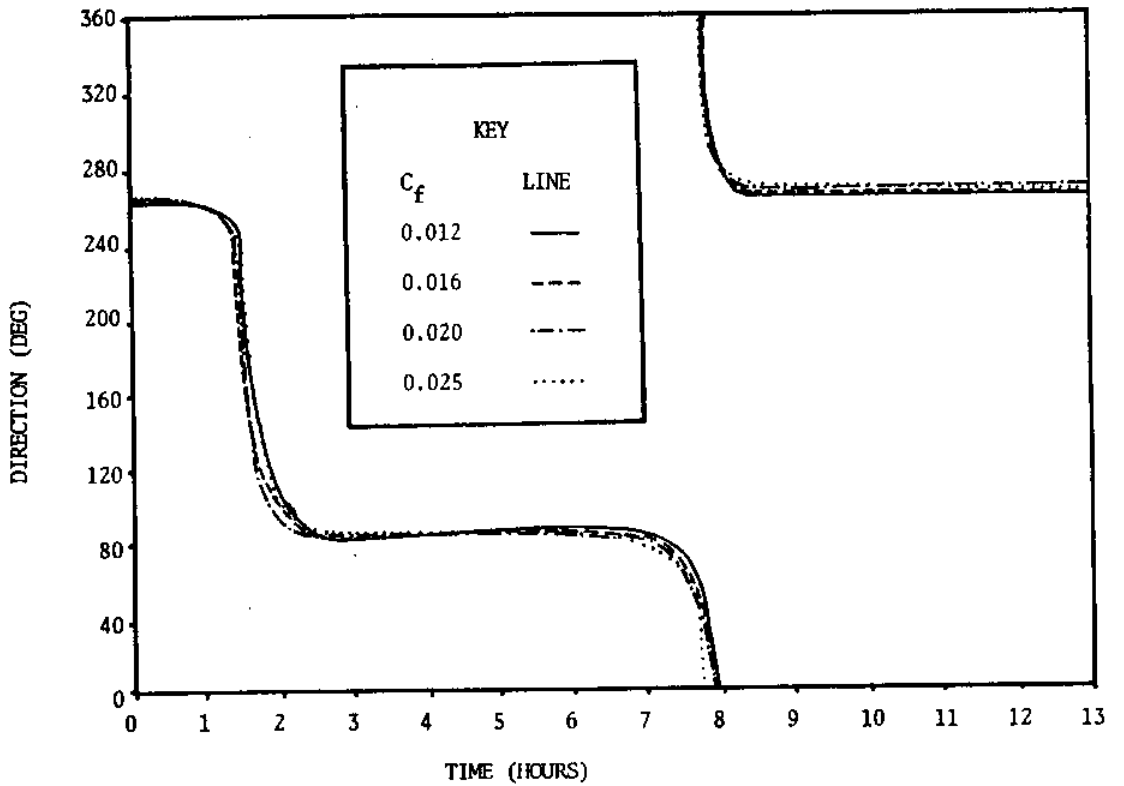
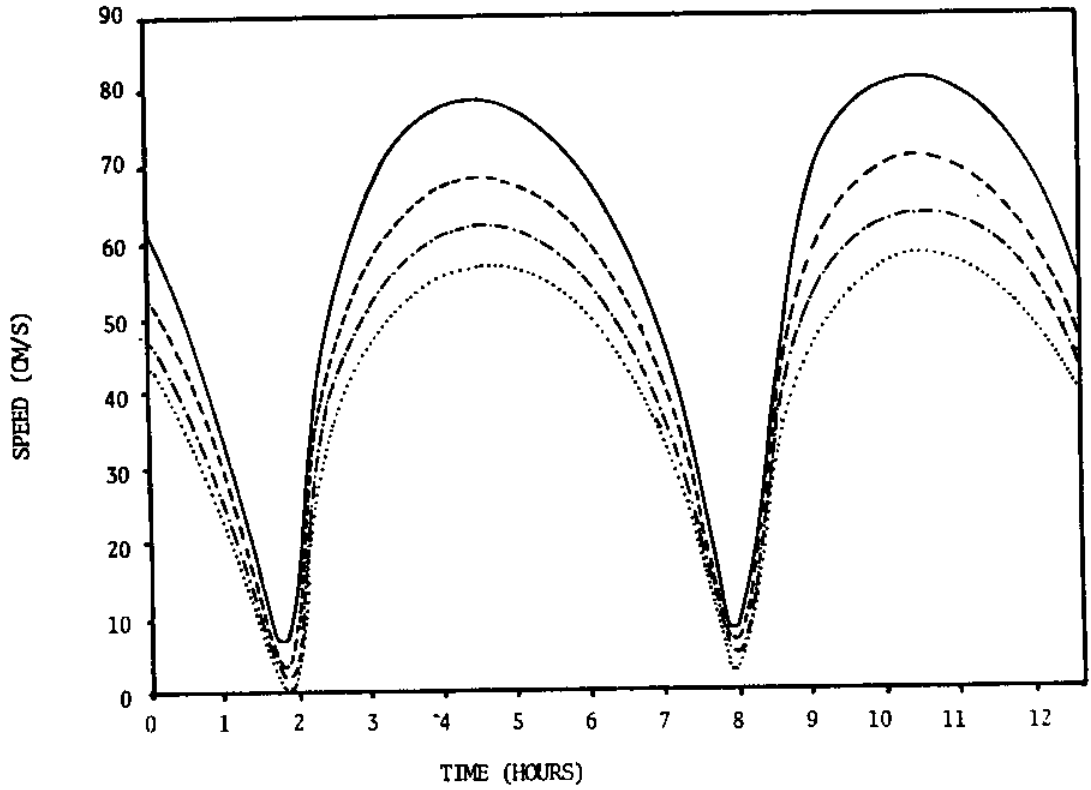


FIGURE 6  
CURRENT SPEED AND DIRECTION PLOTS FOR THE EDDY VISCOSITY EXPERIMENT

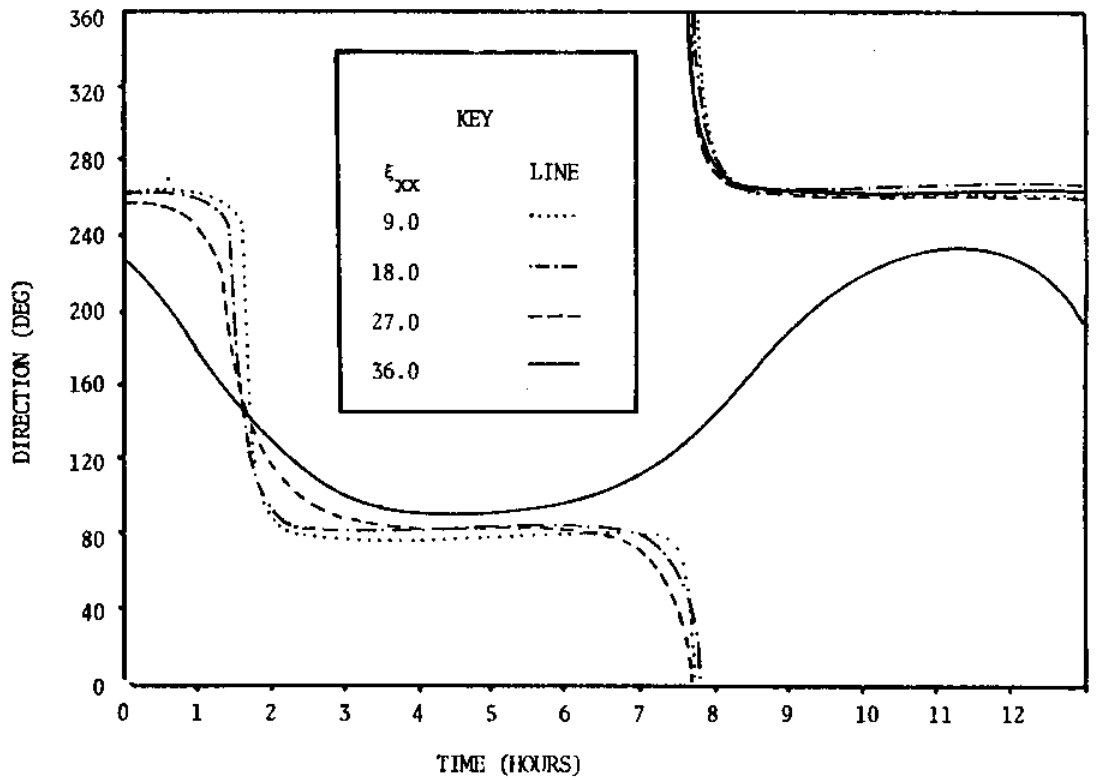
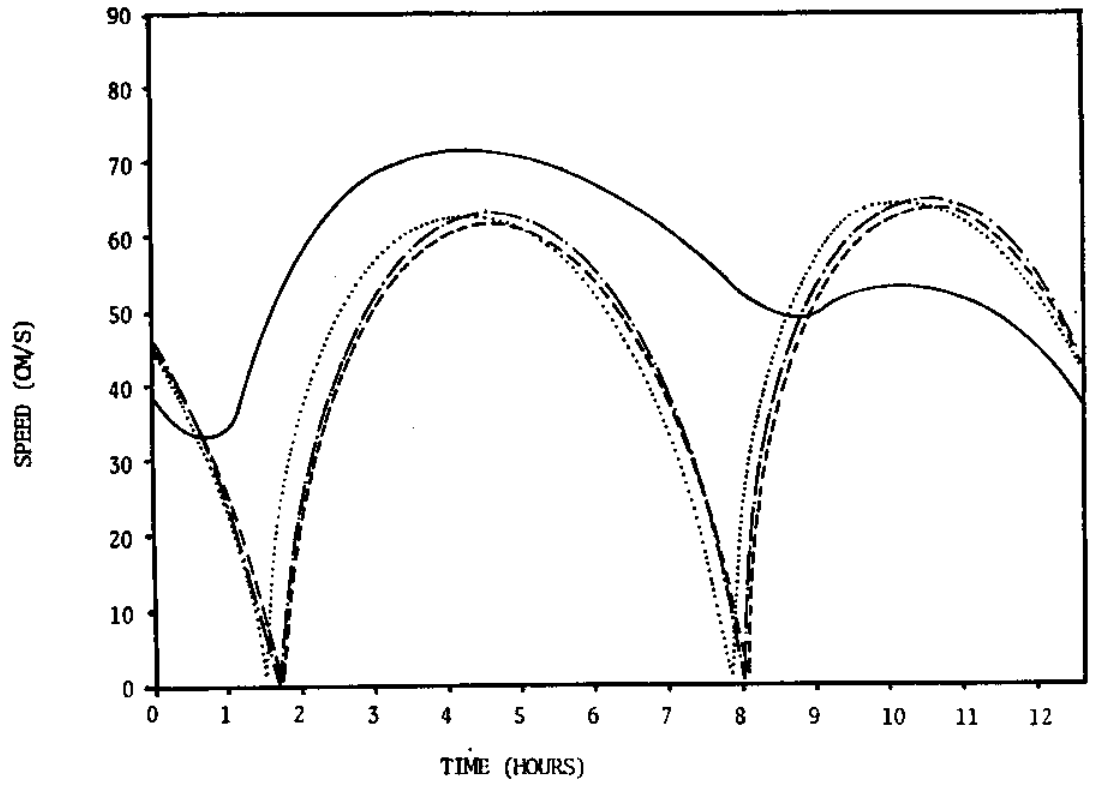


FIGURE 7  
HODOGRAPH OF NORMALIZED CURRENT DATA FOR THE EDDY VISCOSITY COEFFICIENT EXPERIMENT

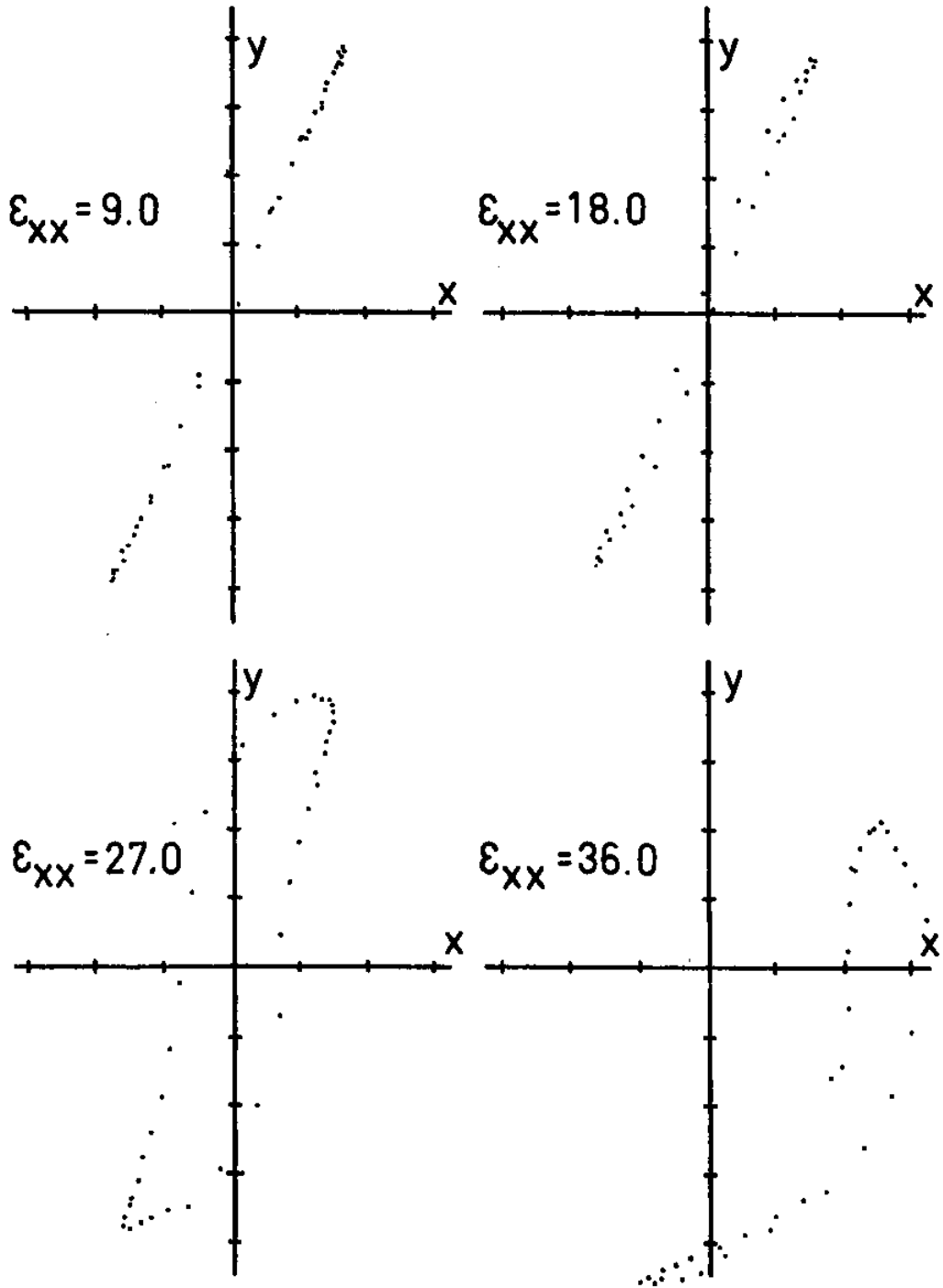


FIGURE 8  
HODOGRAPH OF NORMALIZED CURRENT DATA FOR THE TIDAL AMPLITUDE EXPERIMENT

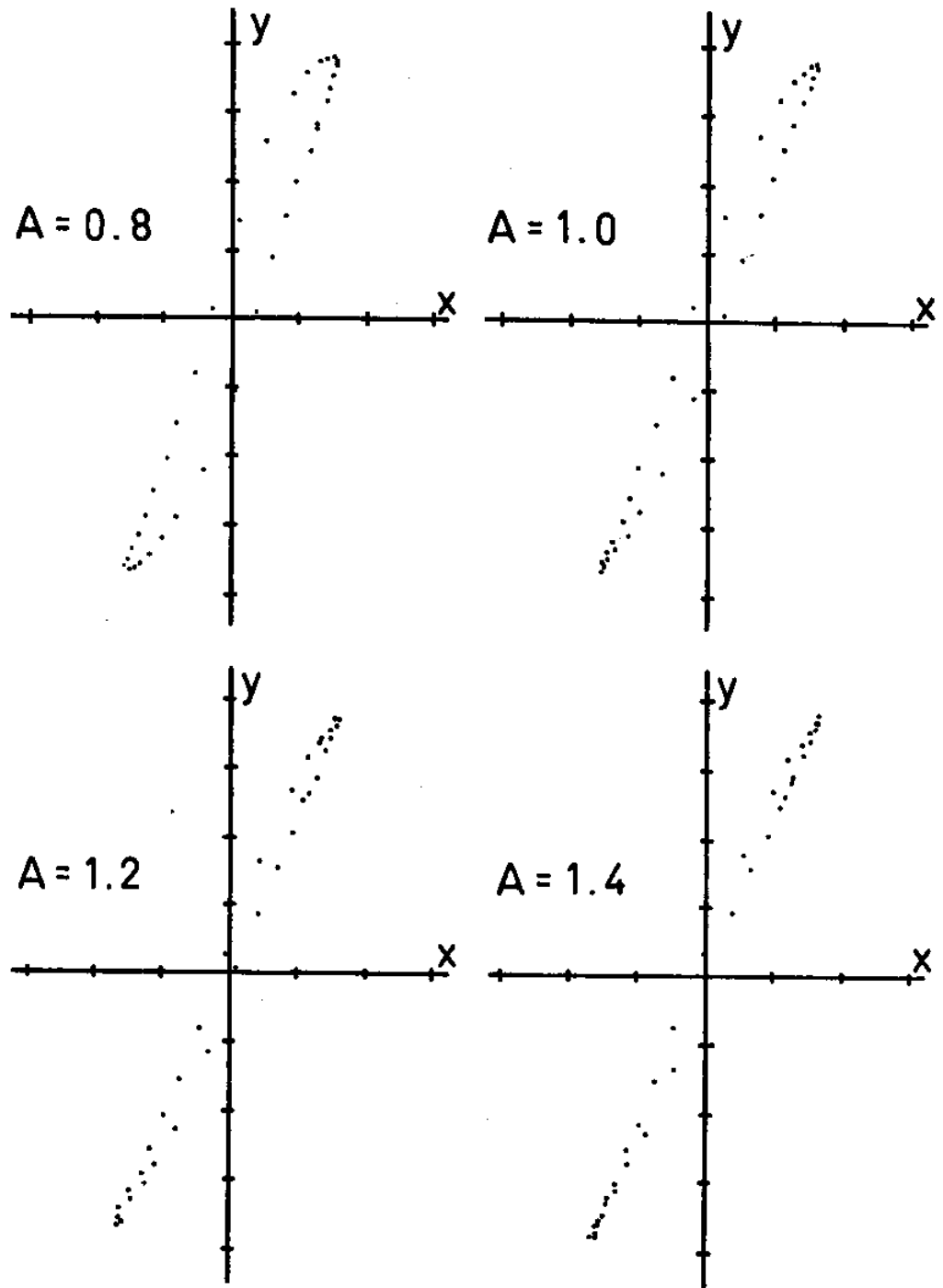
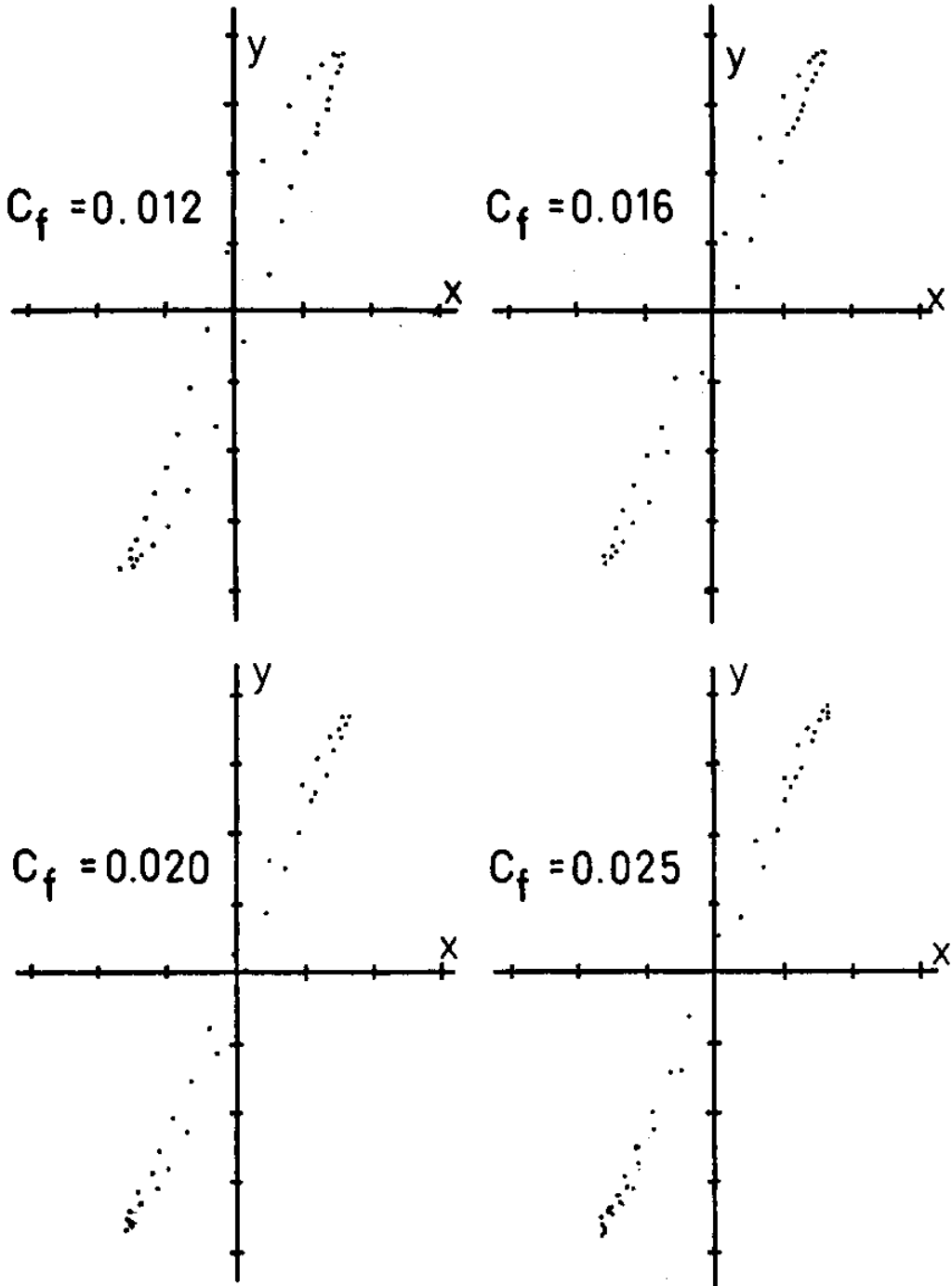




FIGURE 9  
HODOGRAPH OF NORMALIZED CURRENT DATA FOR THE BOTTOM FRICTION COEFFICIENT EXPERIMENT





## PRELIMINARY RESULTS

The model output is in the form of tidal elevation above mean water level and current velocity components at each node. Although the data in this form is important, it is difficult to develop an understanding of the overall flow characteristics from it. Therefore, graphical techniques have been applied to the model results to present a general picture of the characteristics. Surface elevation isoplots for individual time steps have been developed to illustrate the spatial gradients of surface elevation. Current vector plots for individual time steps illustrate the current patterns predicted by the model for any particular time in the tidal cycle. These plots are useful for evaluation of the model as well as presentation of final results for various applications.

Results for the Portsmouth Harbor, Piscataqua River, Little Bay and Great Bay models are presented in tabular and graphic form in Appendices VII through X respectively. These are only preliminary results, as the models have not been calibrated.

Qualitatively, the results of the models are acceptable for this stage of development. Current patterns and surface gradients are reasonable and compare favorably with the geometry of the basin. Higher currents and surface gradients are present in narrower and shallower parts of the estuary where the cross-sectional area is less. Comparison with preliminary field data indicates that appropriate current velocities can be predicted by the model. The models have been developed as completely as possible without detailed comparison with field data.



## MODEL CALIBRATION

The model calibration is expected to be undertaken soon. The UNH/NOS cooperative field program was carried out successfully last summer for the majority of the Great Bay Estuary system, and the remainder of the program (the Portsmouth Harbor area) will be carried out this summer. Data analysis for the UNH portion of the program is nearly complete. Tide data has been received from NOS for the locations of importance for model calibration. The NOS current data is expected soon and will allow the model calibration process to begin.

Calibration of the model is the adjustment of bottom friction and eddy viscosity coefficients to cause the model results to compare more favorably with the data base. The comparisons of model results with the data base will take the following forms:

- 1) Comparison of tidal elevation and phase at points in the model corresponding to UNH/NOS tide stations.
- 2) Comparison of cross-sectional area mass flux at sections in the model corresponding to the UNH current data transects.
- 3) Comparison of current amplitude and phase at points in the model corresponding to NOS current stations and UNH transect stations.
- 4) Comparison of current hodographs at points in the model corresponding to NOS current stations and UNH transect stations.

The tidal elevation comparison will be helpful in evaluating spatial change in the bottom friction coefficient. Comparisons of cross-sectional mass flux and current data, together with tidal elevation data, will be used to calculate bottom friction coefficients, through the use of the parameter evaluation technique developed earlier. Comparison of hodographs will be used to evaluate eddy viscosity coefficients.



APPENDIX I  
 BATHYMETRY OF THE GREAT BAY ESTUARY SYSTEM

VOLUME DETERMINATIONS  
 Great Bay Estuary System

	Volume		Tidal Prism	
	$10^6 m^3$	%	$10^6 m^3$	%
Great Bay	42.8	16.2	28.2	29.3
Little Bay	59.7	22.6	22.6	23.5
Upper Piscataqua River	13.4	5.1	7.5	7.8
Lower Piscataqua River	147.7	56.0	37.8	39.3
Total	263.6	99.9	96.1	99.9

HYPSONOMETRIC DATA  
(Based on Area)

Area Between	Great Bay		Little Bay		Upper Piscataqua		Lower Piscataqua		Estuarine System	
	%	Cum%	%	Cum%	%	Cum%	%	Cum%	%	Cum%
Tidal Flat	46.2		29.4		48.1		27.2		35.6	
Low Tide	53.8		70.6		59.1		72.8		64.4	
0 - 1	52.8*	52.8								
1 - 6	25.2	78.0	36.8*	36.8	55.3*	55.3	21.9*	21.9	42.4*	42.4
6 - 12	13.2	91.2	12.9	49.7	27.0	82.3	12.1	34.0	13.6	56.0
12 - 18	3.8	95.0	8.6	58.3	11.3	93.6	10.1	44.1	8.2	64.2
18 - 24	1.7	96.7	9.7	68.0	6.3	99.9	16.7	60.8	12.4	76.6
24 - 30	0.7	97.4	8.7	76.7						
30 - 40	1.5	98.9	12.7	89.4			14.3	75.1	9.5	86.1
40 - 50	0.9	99.8	9.4	98.8			13.2	88.3	8.1	94.2
50 - 60	0.2	100.0	1.0	99.8			8.9	97.2	4.4	98.6
> 60							2.8	100.0	1.3	99.9

\*% of low tide area



HYPSONETRIC DATA  
(Based on Volume)\*

Interval	Great Bay		Little Bay		Upper Piscataqua		Lower Piscataqua		Estuarine System	
Frequency	%	Cum%	%	Cum%	%	Cum%	%	Cum%	%	Cum%
Tidal Prism	66.2		37.9		55.3		25.5		36.5	
Mean Low Tide	33.8		62.1		44.7		74.5		63.5	
0 - 1	19.4	19.4								
1 - 6	37.0	56.4								
6 - 12	19.8	76.2	18.9	46.1	26.3	87.2	17.3	38.6	18.1	44.9
12 - 18	8.9	85.1	15.3	61.4	10.1	97.3	14.6	53.2	14.1	59.0
18 - 24	5.3	90.4	17.2	78.6	2.7	100.0				
24 - 30	3.8	94.2	9.2	87.8			22.8	76.0	21.8	80.8
30 - 40	4.0	98.2	9.3	97.1			12.8	88.8	10.9	91.7
40 - 50	1.4	99.6	2.7	99.8			7.3	96.1	5.6	97.3
50 - 60	0.4	100.0	0.3	100.1			2.9	99.0	2.0	99.3
> 60							1.0	100.0	0.7	100.0

\*% of volume at mean low tide



APPENDIX II  
ENSEMBLE AVERAGE DEFINITION

The major part of the dynamics of the estuary is deterministic, but part of the motion of the fluid is random in nature. The basic equations include all of the dynamics, both stochastic and deterministic, but for a deterministic solution, the stochastic processes must be handled statistically.

Let us assume that the appropriate instantaneous motions and properties can be measured in detail for a period of time and that the external forcing processes are known. Assuming that the measurements have been taken for a number of these periods having the same external forces, the records can be analyzed to obtain the deterministic and stochastic modes. The deterministic value of a parameter at time  $t_0$  is obtained by averaging all the records of the parameter at time  $t_0$ . This averaging process is called the ensemble average.



APPENDIX III  
PORTSMOUTH HARBOR INPUT DATA AND GRID

MIT	FINITE	ELEMENT	HYDRODYNAMIC	MODEL	-	PORTSMOUTH	HARBOR		
1	3	294.64	2540.00	16.74	1.25	1200.0	0.0	210.00	
2	2	431.80	2590.80	16.74	1.25	1200.0	0.0	0.0	
3	3	584.20	2646.68	3.33	1.25	1200.0	0.0	225.0	
4	1	254.00	2392.68	9.12	0.0	193.48	0.0	0.0	
5	0	406.40	2438.40	13.69	0.0	0.0	0.0	0.0	
6	0	552.80	2484.12	10.64	0.0	0.0	0.0	0.0	
7	1	731.52	2529.84	3.33	0.0	48.29	0.0	0.0	
8	1	360.68	2275.84	7.60	0.0	229.58	0.0	0.0	
9	0	523.32	2336.80	15.22	0.0	0.0	0.0	0.0	
10	0	685.80	2372.36	7.60	0.0	0.0	0.0	0.0	
11	1	863.60	2397.76	3.33	0.0	47.19	0.0	0.0	
12	1	508.00	2179.32	5.16	0.0	228.39	0.0	0.0	
13	0	650.24	2219.96	16.74	0.0	0.0	0.0	0.0	
14	0	812.80	2240.28	5.16	0.0	0.0	0.0	0.0	
15	1	1005.84	2275.84	3.94	0.0	15.73	0.0	0.0	
16	1	614.68	2052.32	6.07	0.0	214.32	0.0	0.0	
17	0	767.08	2077.72	18.87	0.0	0.0	0.0	0.0	
18	1	949.96	2103.12	3.33	0.0	17.25	0.0	0.0	
19	1	706.12	1864.68	12.17	0.0	213.17	0.0	0.0	
20	0	914.40	1899.92	16.74	0.0	0.0	0.0	0.0	
21	1	1082.04	2001.52	3.33	0.0	62.53	0.0	0.0	
22	1	828.04	1727.20	6.07	0.0	241.96	0.0	0.0	
23	1	980.44	1717.04	6.07	0.0	262.78	0.0	0.0	
24	0	1107.44	1838.96	16.74	0.0	0.0	0.0	0.0	
25	1	1244.60	1950.72	3.94	0.0	76.01	0.0	0.0	
26	1	1143.00	1686.56	3.94	0.0	258.25	0.0	0.0	
27	0	1280.16	1808.48	19.79	0.0	0.0	0.0	0.0	
28	1	1407.16	1920.24	5.16	0.0	105.88	0.0	0.0	
29	1	1524.00	2026.92	5.16	0.0	103.57	0.0	0.0	
30	1	1320.80	1645.92	3.33	0.0	257.76	0.0	0.0	
31	0	1457.96	1767.84	19.79	0.0	0.0	0.0	0.0	
32	0	1574.80	1879.60	10.64	0.0	0.0	0.0	0.0	
33	1	1691.64	1981.20	3.33	0.0	77.64	0.0	0.0	
34	1	1518.92	1605.28	5.16	0.0	256.79	0.0	0.0	
35	0	1635.76	1732.28	16.74	0.0	0.0	0.0	0.0	
36	0	1757.68	1844.04	12.47	0.0	0.0	0.0	0.0	
37	1	1874.52	1950.72	6.99	0.0	78.05	0.0	0.0	
38	1	1691.64	1559.56	5.16	0.0	254.23	0.0	0.0	
39	0	1818.64	1676.40	6.99	0.0	0.0	0.0	0.0	
40	0	1935.48	1793.24	16.74	0.0	0.0	0.0	0.0	
41	1	2052.32	1905.00	16.57	0.0	74.59	0.0	0.0	
42	1	1844.04	1513.84	6.07	0.0	252.43	0.0	0.0	

43	0	1976.12	1625.60	7.60	0.0	0.0	0.0	0.0
44	0	2098.04	1737.36	14.91	0.0	0.0	0.0	0.0
45	1	2225.04	1854.20	21.01	0.0	73.95	0.0	0.0
46	1	2011.68	1457.96	6.07	0.0	251.84	0.0	0.0
47	0	2133.60	1569.72	7.60	0.0	0.0	0.0	0.0
48	0	2255.52	1686.56	13.69	0.0	0.0	0.0	0.0
49	1	2387.60	1808.48	18.26	0.0	42.80	0.0	0.0
50	1	2169.16	1407.16	3.33	0.0	249.07	0.0	0.0
51	0	2286.00	1524.00	22.84	0.0	0.0	0.0	0.0
52	1	2423.16	1625.60	16.74	0.0	29.4	0.0	0.0
53	1	2306.32	1346.20	6.07	0.0	239.17	0.0	0.0
54	0	2438.40	1437.64	23.75	0.0	0.0	0.0	0.0
55	1	2590.80	1473.20	6.07	0.0	72.80	0.0	0.0
56	1	2418.08	1259.84	6.07	0.0	222.06	0.0	0.0
57	0	2570.48	1285.24	23.45	0.0	0.0	0.0	0.0
58	1	2509.52	1112.52	19.79	0.0	246.73	0.0	0.0
59	1	2682.24	1148.08	16.74	0.0	285.80	0.0	0.0
60	0	2727.96	1325.88	23.45	0.0	0.0	0.0	0.0
61	1	2773.68	1498.60	13.69	0.0	98.26	0.0	0.0
62	1	2849.88	1209.04	6.07	0.0	285.64	0.0	0.0
63	0	2895.60	1361.44	23.45	0.0	0.0	0.0	0.0
64	1	2941.32	1524.00	8.21	0.0	98.49	0.0	0.0
65	1	3027.68	1244.60	6.07	0.0	282.86	0.0	0.0
66	0	3073.40	1402.08	23.45	0.0	0.0	0.0	0.0
67	1	3114.04	1549.40	7.60	0.0	100.99	0.0	0.0
68	1	3205.48	1290.32	6.07	0.0	287.48	0.0	0.0
69	0	3246.12	1442.72	18.26	0.0	0.0	0.0	0.0
70	1	3281.68	1590.04	13.69	0.0	106.03	0.0	0.0
71	1	3368.04	1351.28	6.07	0.0	289.24	0.0	0.0
72	0	3403.60	1498.60	22.84	0.0	0.0	0.0	0.0
73	1	3449.32	1645.92	15.22	0.0	109.67	0.0	0.0
74	1	3540.76	1407.16	6.07	0.0	286.59	0.0	0.0
75	0	3576.32	1549.40	19.79	0.0	0.0	0.0	0.0
76	1	3622.04	1711.96	6.07	0.0	114.76	0.0	0.0
77	1	3657.60	1981.20	9.73	0.0	196.35	0.0	0.0
78	1	3672.84	2204.72	3.94	0.0	148.08	0.0	0.0
79	1	3708.40	1452.88	6.07	0.0	280.41	0.0	0.0
80	0	3759.20	1620.52	19.79	0.0	0.0	0.0	0.0
81	1	3789.68	1803.40	14.61	0.0	167.60	0.0	0.0
82	0	3815.08	2062.48	10.64	0.0	0.0	0.0	0.0
83	1	3865.88	2316.48	10.64	0.0	110.68	0.0	0.0
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86	0	3997.96	1915.16	18.87	0.0	0.0	0.0	0.0
87	0	4048.76	2153.92	6.07	0.0	0.0	0.0	0.0
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90	0	4165.60	1742.44	19.79	0.0	0.0	0.0	0.0
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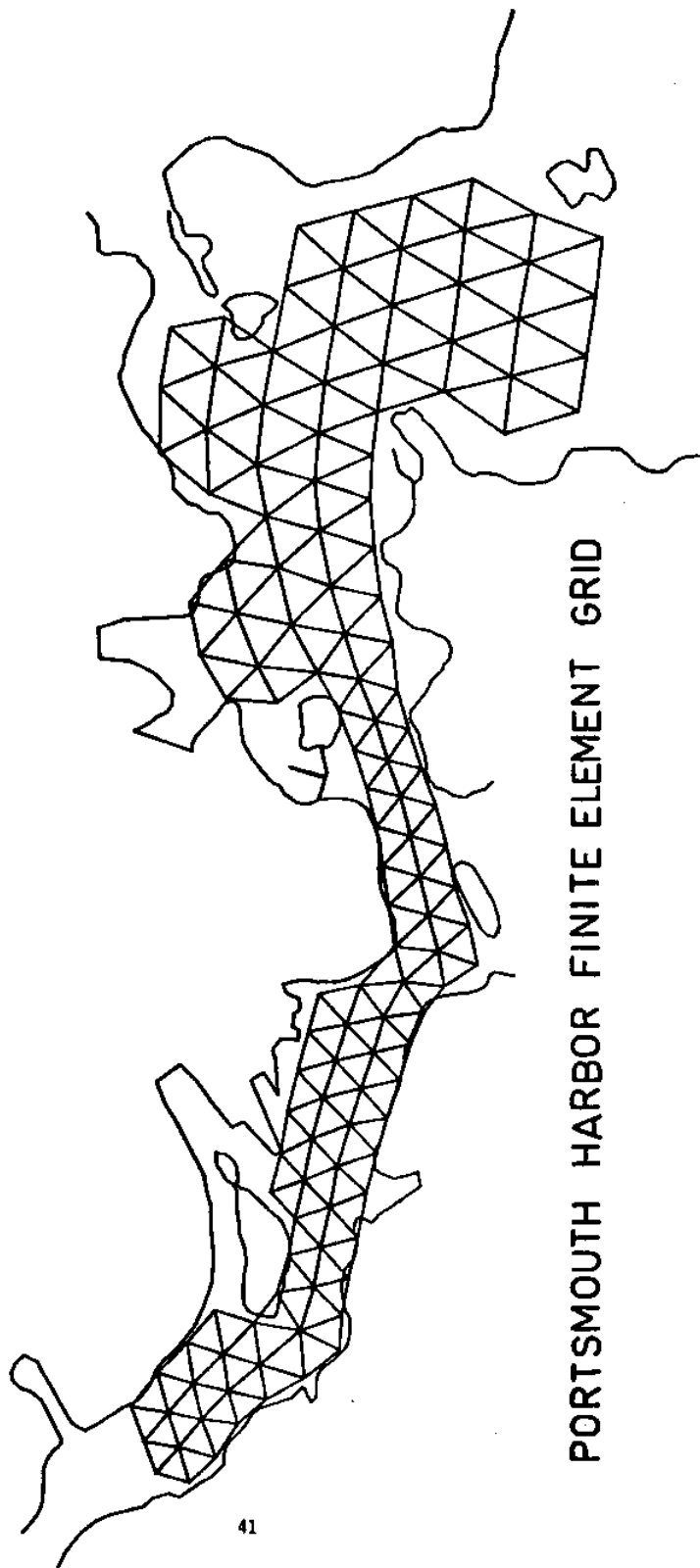
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PORTSMOUTH HARBOR FINITE ELEMENT GRID



APPENDIX IV  
PISCATAQUA RIVER INPUT DATA AND GRID

2	128	94	2	1	2	2	2	1	1	240	0		
MIT FINITE ELEMENT HYDRODYNAMIC MODEL - PISCATAQUA RIVER													
1	3	152.54	507.95	15.00	1.25	3600.00	0.0	-92.00					
2	2	66.03	685.73	11.00	1.25	3600.00	0.0	0.0					
3	3	152.38	863.51	5.52	1.25	3600.00	0.0	88.00					
4	0	253.97	685.73	13.44	0.0	0.0	0.0	0.0					
5	1	330.17	507.95	7.35	0.0	-114.00	0.0	0.0					
6	0	436.84	695.89	11.92	0.0	0.0	0.0	0.0					
7	1	344.77	863.51	5.52	0.0	90.00	0.0	0.0					
8	1	451.60	356.36	3.08	0.0	-130.00	0.0	0.0					
9	0	507.95	507.95	10.39	0.0	0.0	0.0	0.0					
10	1	523.19	863.51	4.30	0.0	97.00	0.0	0.0					
11	1	609.54	341.92	3.08	0.0	-98.00	0.0	0.0					
12	0	700.97	507.95	10.39	0.0	0.0	0.0	0.0					
13	0	624.78	706.05	11.92	0.0	0.0	0.0	0.0					
14	1	721.29	914.31	4.30	0.0	98.00	0.0	0.0					
15	1	802.56	345.41	3.08	0.0	-109.00	0.0	0.0					
16	0	919.39	507.95	7.35	0.0	0.0	0.0	0.0					
17	0	822.88	711.13	11.92	0.0	0.0	0.0	0.0					
18	1	944.79	924.47	4.30	0.0	89.00	0.0	0.0					
19	1	1026.06	299.69	3.08	0.0	-89.00	0.0	0.0					
20	0	1147.96	507.95	7.35	0.0	0.0	0.0	0.0					
21	0	1046.38	711.13	11.92	0.0	0.0	0.0	0.0					
22	1	1168.28	904.15	4.30	0.0	85.00	0.0	0.0					
23	1	1224.16	335.25	3.08	0.0	-85.00	0.0	0.0					
24	0	1356.22	507.95	8.87	0.0	0.0	0.0	0.0					
25	0	1264.79	706.84	11.92	0.0	0.0	0.0	0.0					
26	1	1391.78	888.91	4.30	0.0	88.00	0.0	0.0					
27	1	1422.26	335.25	4.30	0.0	-92.00	0.0	0.0					
28	0	1569.56	507.95	8.87	0.0	0.0	0.0	0.0					
29	0	1513.69	695.89	11.92	0.0	0.0	0.0	0.0					
30	1	1671.15	888.91	3.08	0.0	84.00	0.0	0.0					
31	1	1615.28	320.01	4.30	0.0	-95.00	0.0	0.0					
32	0	1762.52	497.79	9.48	0.0	0.0	0.0	0.0					
33	0	1727.03	690.81	11.92	0.0	0.0	0.0	0.0					
34	1	1899.73	843.20	3.08	0.0	70.00	0.0	0.0					
35	1	1813.38	299.69	3.08	0.0	-87.00	0.0	0.0					
36	0	1909.89	452.07	11.92	0.0	0.0	0.0	0.0					
37	0	1930.21	645.10	11.92	0.0	0.0	0.0	0.0					
38	1	2041.95	767.00	4.30	0.0	46.00	0.0	0.0					
39	1	2036.88	345.41	7.35	0.0	-96.00	0.0	0.0					
40	0	2067.35	492.71	11.92	0.0	0.0	0.0	0.0					
41	1	2123.23	629.86	7.35	0.0	35.00	0.0	0.0					
42	1	2163.86	289.53	7.35	0.0	-112.00	0.0	0.0					

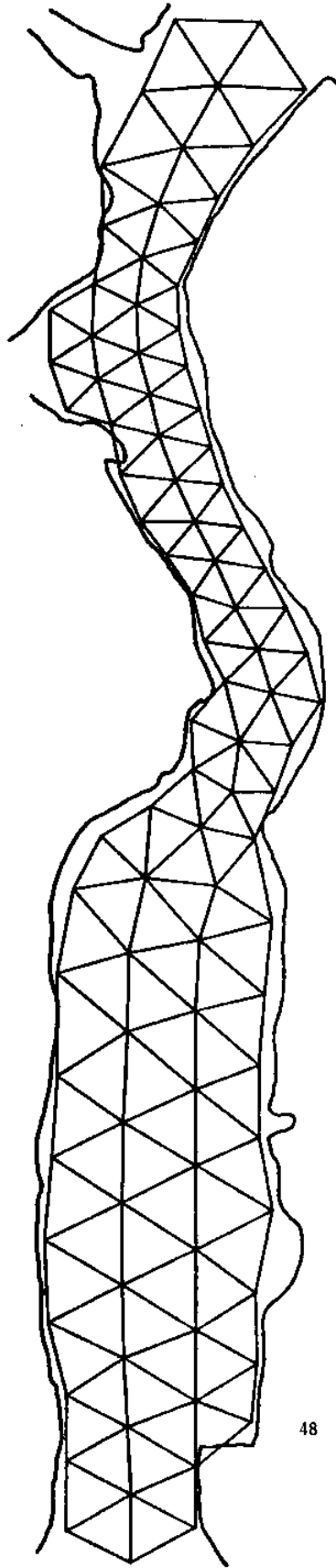
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44	1	2224.82	513.03	7.35	0.0	67.00	0.0	0.0
45	1	2290.85	243.82	7.35	0.0	-116.00	0.0	0.0
46	0	2301.01	386.04	11.92	0.0	0.0	0.0	0.0
47	1	2356.88	518.11	7.35	0.0	71.00	0.0	0.0
48	1	2407.68	167.62	7.35	0.0	-99.00	0.0	0.0
49	0	2433.08	299.69	11.92	0.0	0.0	0.0	0.0
50	1	2463.55	426.68	7.35	0.0	84.00	0.0	0.0
51	1	2539.75	203.18	7.35	0.0	-70.00	0.0	0.0
52	0	2549.90	335.25	11.92	0.0	0.0	0.0	0.0
53	1	2570.22	482.55	7.35	0.0	111.00	0.0	0.0
54	1	2661.65	259.05	7.35	0.0	-64.00	0.0	0.0
55	0	2661.65	396.20	11.92	0.0	0.0	0.0	0.0
56	1	2697.21	513.03	7.35	0.0	114.00	0.0	0.0
57	1	2768.32	314.93	7.35	0.0	-62.00	0.0	0.0
58	0	2788.64	452.07	11.92	0.0	0.0	0.0	0.0
59	1	2803.88	584.14	7.35	0.0	126.00	0.0	0.0
60	1	2880.07	375.88	7.35	0.0	-63.00	0.0	0.0
61	0	2895.31	507.95	11.92	0.0	0.0	0.0	0.0
62	1	2895.31	655.25	7.35	0.0	121.00	0.0	0.0
63	1	2996.90	431.76	7.35	0.0	-63.00	0.0	0.0
64	0	3007.06	558.74	11.92	0.0	0.0	0.0	0.0
65	1	3022.30	711.13	3.08	0.0	107.00	0.0	0.0
66	1	3098.90	462.26	7.35	0.0	-68.00	0.0	0.0
67	0	3128.97	604.46	11.92	0.0	0.0	0.0	0.0
68	1	3159.44	736.53	3.08	0.0	132.00	0.0	0.0
69	1	3195.00	858.43	3.08	0.0	136.00	0.0	0.0
70	1	3200.08	492.71	7.35	0.0	-73.00	0.0	0.0
71	0	3240.71	629.86	11.92	0.0	0.0	0.0	0.0
72	0	3281.35	772.08	8.87	0.0	0.0	0.0	0.0
73	1	3327.07	904.15	3.08	0.0	100.00	0.0	0.0
74	1	3316.91	528.27	7.35	0.0	-76.00	0.0	0.0
75	0	3352.46	660.33	11.92	0.0	0.0	0.0	0.0
76	0	3398.18	787.32	13.44	0.0	0.0	0.0	0.0
77	1	3469.29	904.15	3.08	0.0	61.00	0.0	0.0
78	1	3418.50	548.58	7.35	0.0	-83.00	0.0	0.0
79	0	3484.53	665.41	11.92	0.0	0.0	0.0	0.0
80	1	3540.40	782.24	7.35	0.0	56.00	0.0	0.0
81	1	3535.33	553.66	7.35	0.0	-101.00	0.0	0.0
82	0	3611.52	650.17	11.92	0.0	0.0	0.0	0.0
83	1	3697.87	756.84	7.35	0.0	86.00	0.0	0.0
84	1	3657.23	497.79	7.35	0.0	-115.00	0.0	0.0
85	0	3758.82	604.46	11.92	0.0	0.0	0.0	0.0
86	1	3860.41	761.92	7.35	0.0	76.00	0.0	0.0
87	1	3774.06	441.92	7.35	0.0	-120.00	0.0	0.0
88	0	3911.21	538.43	15.92	0.0	0.0	0.0	0.0
89	1	4063.59	650.17	7.35	0.0	62.00	0.0	0.0
90	1	3916.29	345.41	7.35	0.0	-129.00	0.0	0.0
91	0	4078.83	441.92	15.92	0.0	0.0	0.0	0.0
92	3	4255.61	558.74	7.35	1.25	0.0	0.0	60.00
93	3	4068.67	203.18	7.35	1.25	0.0	0.0	-135.00
94	2	4251.53	325.09	51.92	1.25	0.0	0.0	0.0
	1	1	4	2	0.028	18.0	18.0	3.0
	2	2	4	3	0.028	18.0	18.0	3.0
	3	1	5	4	0.028	18.0	18.0	3.0
	4	4	7	3	0.028	18.0	18.0	3.0
	5	5	6	4	0.028	18.0	18.0	3.0
	6	4	6	7	0.028	18.0	18.0	3.0
	7	8	9	5	0.028	18.0	18.0	3.0
	8	5	9	6	0.028	18.0	18.0	3.0

9	6	10	7 0.028	18.0	18.0	3.0
10	8	11	9 0.028	18.0	18.0	3.0
11	9	13	6 0.028	18.0	18.0	3.0
12	6	13	10 0.028	18.0	18.0	3.0
13	11	12	9 0.028	18.0	18.0	3.0
14	9	12	13 0.028	18.0	18.0	3.0
15	13	14	10 0.028	18.0	18.0	3.0
16	11	15	12 0.028	18.0	18.0	3.0
17	12	17	13 0.028	18.0	18.0	3.0
18	13	17	14 0.028	18.0	18.0	3.0
19	15	16	12 0.028	18.0	18.0	3.0
20	12	16	17 0.028	18.0	18.0	3.0
21	17	18	14 0.028	18.0	18.0	3.0
22	15	19	16 0.028	18.0	18.0	3.0
23	16	21	17 0.028	18.0	18.0	3.0
24	17	21	18 0.028	18.0	18.0	3.0
25	19	20	16 0.028	18.0	18.0	3.0
26	16	20	21 0.028	18.0	18.0	3.0
27	21	22	18 0.028	18.0	18.0	3.0
28	19	23	20 0.028	18.0	18.0	3.0
29	20	25	21 0.028	18.0	18.0	3.0
30	21	25	22 0.028	18.0	18.0	3.0
31	23	24	20 0.028	18.0	18.0	3.0
32	20	24	25 0.028	18.0	18.0	3.0
33	25	26	22 0.028	18.0	18.0	3.0
34	23	27	24 0.028	18.0	18.0	3.0
35	24	29	25 0.028	18.0	18.0	3.0
36	25	29	26 0.028	18.0	18.0	3.0
37	27	28	24 0.028	18.0	18.0	3.0
38	24	28	29 0.028	18.0	18.0	3.0
39	29	30	26 0.028	18.0	18.0	3.0
40	27	31	28 0.028	18.0	18.0	3.0
41	28	33	29 0.028	18.0	18.0	3.0
42	29	33	30 0.028	18.0	18.0	3.0
43	31	32	28 0.028	18.0	18.0	3.0
44	28	32	33 0.028	18.0	18.0	3.0
45	33	34	30 0.028	18.0	18.0	3.0
46	31	35	32 0.028	18.0	18.0	3.0
47	32	37	33 0.028	18.0	18.0	3.0
48	33	37	34 0.028	18.0	18.0	3.0
49	35	36	32 0.028	18.0	18.0	3.0
50	32	36	37 0.028	18.0	18.0	3.0
51	37	38	34 0.028	18.0	18.0	3.0
52	35	39	36 0.028	18.0	18.0	3.0
53	39	40	36 0.028	18.0	18.0	3.0
54	36	40	37 0.028	18.0	18.0	3.0
55	40	41	37 0.028	18.0	18.0	3.0
56	37	41	38 0.028	18.0	18.0	3.0
57	39	42	43 0.028	18.0	18.0	3.0
58	39	43	40 0.028	18.0	18.0	3.0
59	43	44	40 0.028	18.0	18.0	3.0
60	40	44	41 0.028	18.0	18.0	3.0
61	42	45	46 0.028	18.0	18.0	3.0
62	42	46	43 0.028	18.0	18.0	3.0
63	43	46	44 0.028	18.0	18.0	3.0
64	46	47	44 0.028	18.0	18.0	3.0
65	45	48	49 0.028	18.0	18.0	3.0
66	45	49	46 0.028	18.0	18.0	3.0
67	49	50	46 0.028	18.0	18.0	3.0
68	46	50	47 0.028	18.0	18.0	3.0

69	48	51	49 0.028	18.0	18.0	3.0
70	51	52	49 0.028	18.0	18.0	3.0
71	49	52	50 0.028	18.0	18.0	3.0
72	52	53	50 0.028	18.0	18.0	3.0
73	51	54	52 0.028	18.0	18.0	3.0
74	54	55	52 0.028	18.0	18.0	3.0
75	52	55	53 0.028	18.0	18.0	3.0
76	55	56	53 0.028	18.0	18.0	3.0
77	54	57	55 0.028	18.0	18.0	3.0
78	57	58	55 0.028	18.0	18.0	3.0
79	55	58	56 0.028	18.0	18.0	3.0
80	58	59	56 0.028	18.0	18.0	3.0
81	57	60	58 0.028	18.0	18.0	3.0
82	60	61	58 0.028	18.0	18.0	3.0
83	58	61	59 0.028	18.0	18.0	3.0
84	61	62	59 0.028	18.0	18.0	3.0
85	60	63	61 0.028	18.0	18.0	3.0
86	63	64	61 0.028	18.0	18.0	3.0
87	61	64	62 0.028	18.0	18.0	3.0
88	64	65	62 0.028	18.0	18.0	3.0
89	63	66	64 0.028	18.0	18.0	3.0
90	66	67	64 0.028	18.0	18.0	3.0
91	64	67	65 0.028	18.0	18.0	3.0
92	67	68	65 0.028	18.0	18.0	3.0
93	66	70	67 0.028	18.0	18.0	3.0
94	70	71	67 0.028	18.0	18.0	3.0
95	67	71	68 0.028	18.0	18.0	3.0
96	71	72	68 0.028	18.0	18.0	3.0
97	68	72	69 0.028	18.0	18.0	3.0
98	72	73	69 0.028	18.0	18.0	3.0
99	70	74	71 0.028	18.0	18.0	3.0
100	74	75	71 0.028	18.0	18.0	3.0
101	71	75	72 0.028	18.0	18.0	3.0
102	75	76	72 0.028	18.0	18.0	3.0
103	72	76	73 0.028	18.0	18.0	3.0
104	76	77	73 0.028	18.0	18.0	3.0
105	74	78	75 0.028	18.0	18.0	3.0
106	78	79	75 0.028	18.0	18.0	3.0
107	75	79	76 0.028	18.0	18.0	3.0
108	79	80	76 0.028	18.0	18.0	3.0
109	76	80	77 0.028	18.0	18.0	3.0
110	78	81	79 0.028	18.0	18.0	3.0
111	81	82	79 0.028	18.0	18.0	3.0
112	79	82	80 0.028	18.0	18.0	3.0
113	82	83	80 0.028	18.0	18.0	3.0
114	84	82	81 0.028	18.0	18.0	3.0
115	84	85	82 0.028	18.0	18.0	3.0
116	85	83	82 0.028	18.0	18.0	3.0
117	85	86	83 0.028	18.0	18.0	3.0
118	87	85	84 0.028	18.0	18.0	3.0
119	87	88	85 0.028	18.0	18.0	3.0
120	88	86	85 0.028	18.0	18.0	3.0
121	90	88	87 0.028	18.0	18.0	3.0
122	88	89	86 0.028	18.0	18.0	3.0
123	90	91	88 0.028	18.0	18.0	3.0
124	91	89	88 0.028	18.0	18.0	3.0
125	93	91	90 0.028	18.0	18.0	3.0
126	91	92	89 0.028	18.0	18.0	3.0
127	93	94	91 0.028	18.0	18.0	3.0
128	94	92	91 0.028	18.0	18.0	3.0



43.1	.72722E-04	9.81	44640.0	1030.0															
11160.0	33480.0	05.0		18	5.0							1					240		
	2		28			28													
1	5	8	11	15	19	23	27	31	35	39	42	45	48	51	54	57	60	63	66
70	74	78	81	84	87	90	93												
92	89	86	83	80	77	73	69	68	65	62	59	56	53	50	47	44	41	38	34
30	26	22	18	14	10	7	3												
		2		3			3												
93	94	92																	
3	2	1																	



PISCATAQUA RIVER FINITE ELEMENT GRID

APPENDIX V  
LITTLE BAY INPUT DATA AND GRID

2	171	118	2	1	2	2	2	1	1	360		
17	FINITE	ELEMENT	HYDRODYNAMIC	MODEL	-	LITTLE	BAY					
1	2	1697.81	3440.25	9.0	1.20	0.0	0.0	0.0	0.0	0.0		0.0
2	3	1921.33	3764.05	7.0	1.20	0.0	0.0	0.0	0.0	-2.00		
3	3	1489.53	3277.69	7.0	1.20	0.0	0.0	0.0	0.0	177.00		
4	0	1697.81	3201.49	9.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
5	1	1916.25	3099.89	7.0	0.0	-2.00	0.0	0.0	0.0	0.0		0.0
6	1	1474.29	3038.93	7.03	0.0	158.00	0.0	0.0	0.0	0.0		0.0
7	0	1687.65	2947.49	11.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
8	1	1906.09	2891.61	7.0	0.0	-19.00	0.0	0.0	0.0	0.0		0.0
9	1	1291.41	2830.65	9.0	0.0	140.00	0.0	0.0	0.0	0.0		0.0
10	0	1514.93	2749.37	9.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
11	1	1774.01	2698.57	7.0	0.0	-4.00	0.0	0.0	0.0	0.0		0.0
12	1	1144.09	2642.69	9.0	0.0	135.00	0.0	0.0	0.0	0.0		0.0
13	0	1372.69	2581.73	13.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
14	0	1606.37	2510.61	9.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
15	1	1290.85	2469.97	6.0	0.0	-1.00	0.0	0.0	0.0	0.0		0.0
16	1	915.49	2459.81	8.0	0.0	131.00	0.0	0.0	0.0	0.0		0.0
17	0	1220.29	2388.69	11.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
18	0	1469.21	2317.57	13.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
19	1	1763.85	2236.29	9.0	0.0	-34.00	0.0	0.0	0.0	0.0		0.0
20	1	707.21	2256.61	7.0	0.0	142.00	0.0	0.0	0.0	0.0		0.0
21	0	1027.25	2195.65	9.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
22	0	1321.89	2094.05	16.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
23	1	1601.29	2043.25	10.0	0.0	-36.00	0.0	0.0	0.0	0.0		0.0
24	1	310.97	1880.69	5.0	0.0	141.00	0.0	0.0	0.0	0.0		0.0
25	1	539.57	1961.97	7.0	0.0	130.00	0.0	0.0	0.0	0.0		0.0
26	0	834.21	1997.53	10.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
27	0	1149.17	1926.41	14.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
28	1	1464.13	1819.73	10.0	0.0	-42.00	0.0	0.0	0.0	0.0		0.0
29	1	275.41	1621.61	7.0	0.0	175.00	0.0	0.0	0.0	0.0		0.0
30	0	498.93	1713.05	7.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
31	0	752.93	1774.01	11.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
32	0	1006.93	1768.93	18.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
33	1	1220.29	1626.69	11.0	0.0	-36.00	0.0	0.0	0.0	0.0		0.0
34	1	275.41	1377.77	8.0	0.0	178.00	0.0	0.0	0.0	0.0		0.0
35	0	463.37	1453.97	11.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
36	0	691.97	1540.33	9.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
37	0	935.81	1560.65	15.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
38	1	1126.85	1377.77	10.0	0.0	-17.00	0.0	0.0	0.0	0.0		0.0
39	1	265.25	1169.49	8.0	0.0	-172.00	0.0	0.0	0.0	0.0		0.0
40	0	458.29	1250.77	12.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0
41	0	646.25	1326.97	10.0	0.0	100.00	0.0	0.0	0.0	0.0		0.0
42	0	864.69	1321.89	12.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0

43	1	1078.05	1169.49	8.0	0.0	-13.00	0.0	0.0
44	1	326.21	981.53	8.0	0.0	-146.00	0.0	0.0
45	0	468.45	1078.05	12.0	0.0	0.0	0.0	0.0
47	0	663.97	1165.49	7.0	0.0	10.00	0.0	0.0
48	0	874.85	1067.89	12.0	0.0	0.0	0.0	0.0
49	1	1037.41	966.29	8.0	0.0	-4.00	0.0	0.0
50	1	458.29	869.77	9.0	0.0	-159.00	0.0	0.0
51	0	671.65	976.45	8.0	0.0	-87.00	0.0	0.0
52	0	859.61	864.69	13.0	0.0	0.0	0.0	0.0
53	0	656.41	742.77	14.0	0.0	0.0	0.0	0.0
54	1	427.81	625.53	12.0	0.0	180.00	0.0	0.0
55	1	1047.57	803.73	8.0	0.0	19.00	0.0	0.0
56	0	879.93	661.49	11.0	0.0	0.0	0.0	0.0
57	0	717.37	458.29	12.0	0.0	0.0	0.0	0.0
58	1	458.29	356.69	9.0	0.0	-155.00	0.0	0.0
59	1	620.85	183.97	8.0	0.0	-141.00	0.0	0.0
60	1	1276.17	869.77	9.0	0.0	130.00	0.0	0.0
61	1	1144.09	661.49	8.0	0.0	91.00	0.0	0.0
62	0	1017.09	448.13	11.0	0.0	0.0	0.0	0.0
63	0	874.85	234.77	14.0	0.0	0.0	0.0	0.0
64	1	747.85	6.17	8.0	0.0	-115.00	0.0	0.0
65	1	1514.93	966.29	9.0	0.0	107.00	0.0	0.0
66	0	1413.33	691.97	11.0	0.0	0.0	0.0	0.0
67	0	1245.69	458.29	11.0	0.0	0.0	0.0	0.0
68	0	1164.41	229.69	11.0	0.0	0.0	0.0	0.0
69	1	996.77	21.41	8.0	0.0	-89.00	0.0	0.0
70	1	1763.85	1017.09	8.0	0.0	101.00	0.0	0.0
71	0	1672.41	752.93	13.0	0.0	0.0	0.0	0.0
72	0	1550.49	504.01	6.0	0.0	0.0	0.0	0.0
73	1	1393.01	260.17	6.0	0.0	-55.00	0.0	0.0
74	1	1286.33	16.33	7.0	0.0	-57.00	0.0	0.0
75	1	2032.09	1067.89	7.0	0.0	94.00	0.0	0.0
76	0	1931.49	773.25	12.0	0.0	0.0	0.0	0.0
77	0	1814.65	529.41	12.0	0.0	0.0	0.0	0.0
78	1	1713.05	285.57	7.0	0.0	-89.00	0.0	0.0
79	1	2322.65	1057.73	7.0	0.0	85.00	0.0	0.0
80	0	2210.89	803.73	11.0	0.0	0.0	0.0	0.0
81	0	2094.05	534.49	11.0	0.0	0.0	0.0	0.0
82	1	1987.37	270.33	8.0	0.0	-93.00	0.0	0.0
83	1	2607.13	1022.17	6.0	0.0	84.00	0.0	0.0
84	0	2485.21	763.09	9.0	0.0	0.0	0.0	0.0
85	0	2363.29	524.33	11.0	0.0	0.0	0.0	0.0
86	1	2226.13	255.09	7.0	0.0	-93.00	0.0	0.0
87	1	2947.49	991.69	6.0	0.0	84.00	0.0	0.0
88	0	2749.37	727.53	9.0	0.0	0.0	0.0	0.0
89	0	2617.29	504.01	11.0	0.0	0.0	0.0	0.0
90	1	2495.37	244.93	7.0	0.0	-94.00	0.0	0.0
91	1	3226.89	956.13	6.0	0.0	83.00	0.0	0.0
92	0	3059.25	702.13	9.0	0.0	0.0	0.0	0.0
93	0	2891.61	473.53	11.0	0.0	0.0	0.0	0.0
94	1	2764.61	219.53	7.0	0.0	-95.00	0.0	0.0
95	1	3501.21	920.57	6.0	0.0	78.00	0.0	0.0
96	0	3353.89	686.89	9.0	0.0	0.0	0.0	0.0
97	0	3186.25	453.21	11.0	0.0	0.0	0.0	0.0
98	1	3008.45	199.21	7.0	0.0	-94.00	0.0	0.0
99	1	3775.53	839.29	6.0	0.0	70.00	0.0	0.0
100	0	3582.49	625.53	9.0	0.0	0.0	0.0	0.0
101	0	3440.25	407.49	11.0	0.0	0.0	0.0	0.0
102	1	3252.29	183.97	7.0	0.0	-94.00	0.0	0.0
103	1	4065.09	712.29	8.0	0.0	59.00	0.0	0.0

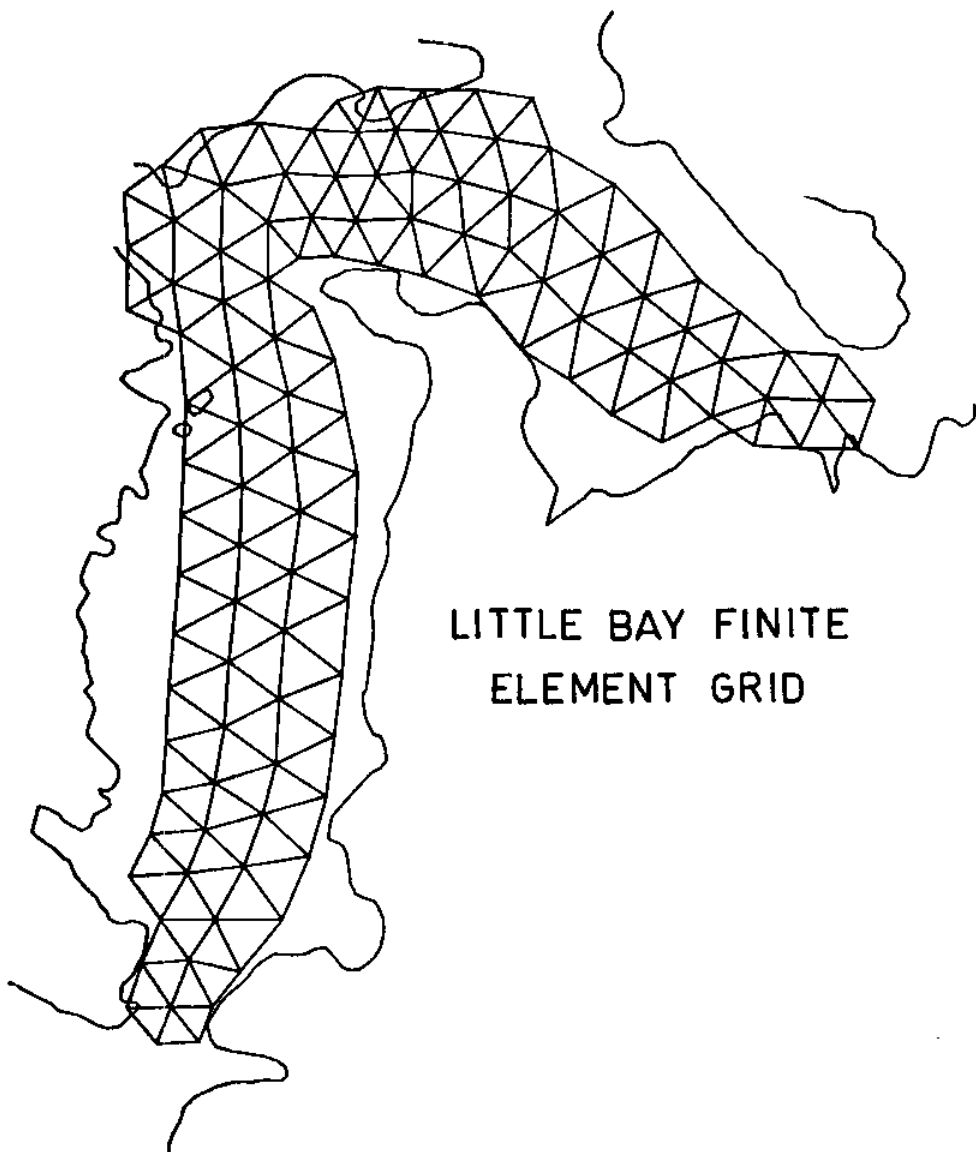
104	0	3821.25	539.57	9.0	0.0	0.0	0.0	0.0
105	0	3653.61	361.77	11.0	0.0	0.0	0.0	0.0
106	1	3485.97	168.73	7.0	0.0	-100.00	0.0	0.0
107	1	4303.84	519.25	8.0	0.0	52.00	0.0	0.0
108	0	4065.09	407.49	13.0	0.0	0.0	0.0	0.0
109	0	3851.73	280.49	13.0	0.0	0.0	0.0	0.0
110	1	3673.93	112.85	8.0	0.0	-112.00	0.0	0.0
111	1	4466.40	392.25	7.0	0.0	61.00	0.0	0.0
112	0	4253.04	285.57	14.0	0.0	0.0	0.0	0.0
113	1	4065.09	158.57	10.0	0.0	-84.00	0.0	0.0
114	1	3866.97	16.33	8.0	0.0	-85.00	0.0	0.0
115	3	4628.96	331.29	8.0	1.10	3600.00	0.0	69.00
116	0	4466.40	204.29	12.0	0.0	0.0	0.0	0.0
117	1	4268.29	72.21	7.0	0.0	-111.00	0.0	0.0
118	2	4634.04	138.25	12.0	1.10	3600.00	0.0	0.0
119	3	4471.48	1.09	8.0	1.10	3600.00	0.0	-108.00
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163	109	114	113
164	109	110	114

165	111	116	115																	
166	111	112	116																	
167	112	117	116																	
168	112	113	117																	
169	115	116	118																	
170	116	119	118																	
171	116	117	119																	
43.1	.72722E-04	9.81	44640.00	1030.0																
11160.0	78120.0	10.0		18	5.0								1						120	
	2		33																	
115	111	107	103	59	95	91	87	83	79	75	70	65	60	61	55	49	43	38	33	
28	23	19	15	11	8	5	2													
3	6	9	12	16	20	25	24	25	34	39	44	50	54	58	59	64	69	74	73	
78	82	86	90	94	98	102	106	110	114	113	117	119								
	2			3			3													
2	1	3																		
119	118	115																		
0																				





LITTLE BAY FINITE  
ELEMENT GRID



APPENDIX VI  
GREAT BAY INPUT DATA AND GRID

IT	FINITE	ELEMENT	HYDRODYNAMIC	MODEL	- GREAT BAY		240	1
1	1	6045.00	574.00	1.50			303.1	
2	1	6273.00	1143.00	1.50			6.8	
3	1	5588.00	589.00	1.50			269.3	
4	0	5760.00	1117.00	1.50			.	
5	1	5892.00	1676.00	1.50			38.0	
6	1	5110.00	584.00	1.50			269.3	
7	0	5283.00	1087.00	1.80			.	
8	0	5425.00	1605.00	1.80			.	
9	1	5537.00	2092.00	1.50			38.3	
10	1	4673.00	599.00	1.50			238.3	
11	0	4851.00	1066.00	2.11			.	
12	0	4978.00	1544.00	2.41			.	
13	0	5100.00	2001.00	2.11			.	
14	1	5232.00	2509.00	1.50			67.6	
15	1	4434.00	1036.00	1.80			241.0	
16	0	4577.00	1483.00	3.02			.	
17	0	4673.00	1925.00	4.55			.	
18	1	4785.00	2438.00	1.50			99.3	
19	1	4094.00	1016.00	1.80			246.9	
20	0	4185.00	1432.00	3.33			.	
21	0	4338.00	1854.00	2.41			.	
22	1	4389.00	2372.00	1.80			102.8	
23	1	3794.00	1356.00	3.02			249.1	
24	0	3891.00	1803.00	3.63			.	
25	1	3987.00	2255.00	2.72			43.8	
26	1	4140.00	2707.00	1.80			337.8	
27	1	4328.00	3098.00	1.50			1.1	
28	1	3352.00	1295.00	1.50			278.2	
29	0	3505.00	1752.00	3.94			.	
30	0	3606.00	2209.00	3.63			.	
31	0	3733.00	2692.00	2.72			.	
32	0	3937.00	3098.00	1.80			.	
33	1	4130.00	3474.00	1.50			58.5	
34	1	2951.00	1234.00	1.50			277.5	
35	0	3098.00	1696.00	1.50			.	
36	0	3215.00	2164.00	1.80			.	
37	0	3332.00	2656.00	5.16			.	
38	0	3525.00	3093.00	6.99			.	
39	1	3733.00	3479.00	10.64			28.7	
40	1	3942.00	3815.00	13.69			329.1	
41	1	4114.00	4114.00	15.52			330.9	
42	3	4277.00	4414.00	3.02	1.10		0.00	331.5

43	1	2540.00	1188.00	1.80		277.4		
44	0	2682.00	1656.00	1.80		.		
45	0	2824.00	2133.00	3.33		.		
46	0	2946.00	2600.00	5.16		.		
47	0	3129.00	3063.00	7.60		.		
48	0	3332.00	3469.00	10.64		.		
49	0	3556.00	3825.00	7.60		.		
50	0	3804.00	4150.00	10.64		.		
51	2	3982.00	4434.00	15.22	1.10	0.00		
52	1	2057.00	1117.00	1.80		277.8		
53	0	2245.00	1605.00	1.80		.		
54	0	2397.00	2077.00	3.33		.		
55	0	2560.00	2565.00	1.80		.		
56	0	2702.00	3027.00	1.80		.		
57	0	2987.00	3454.00	3.33		.		
58	0	3169.00	3830.00	3.33		.		
59	1	3423.00	4185.00	2.11		109.9		
60	3	3703.00	4455.00	3.02	1.10	0.00		133.9
61	1	1569.00	1056.00	1.50		277.3		
62	0	1752.00	1534.00	1.80		.		
63	0	1935.00	2026.00	3.33		.		
64	0	2108.00	2529.00	2.72		.		
65	0	2286.00	2981.00	2.11		.		
66	1	2463.00	3439.00	2.11		120.2		
67	1	2712.00	3835.00	1.80		145.5		
68	1	2997.00	4216.00	1.50		114.6		
69	1	756.00	482.00	1.50		304.5		
70	1	1021.00	985.00	1.50		304.8		
71	0	1259.00	1473.00	2.11		.		
72	0	1447.00	1955.00	3.02		.		
73	0	1630.00	2473.00	1.50		.		
74	0	1833.00	2931.00	1.50		.		
75	1	2006.00	3418.00	1.50		93.2		
76	1	233.00	421.00	1.50		245.9		
77	0	452.00	909.00	1.50		.		
78	0	670.00	1402.00	3.33		.		
79	0	934.00	1899.00	2.11		.		
80	0	1117.00	2413.00	1.50		.		
81	1	1336.00	2885.00	1.50		126.1		
82	1	1554.00	3388.00	1.50		125.2		
83	1	-71.00	853.00	3.63		185.2		
84	1	157.00	1346.00	3.33		155.2		
85	1	386.00	1844.00	1.50		156.2		
86	1	604.00	2362.00	1.50		156.4		
87	1	817.00	2834.00	1.50		125.6		
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129	80	81	87

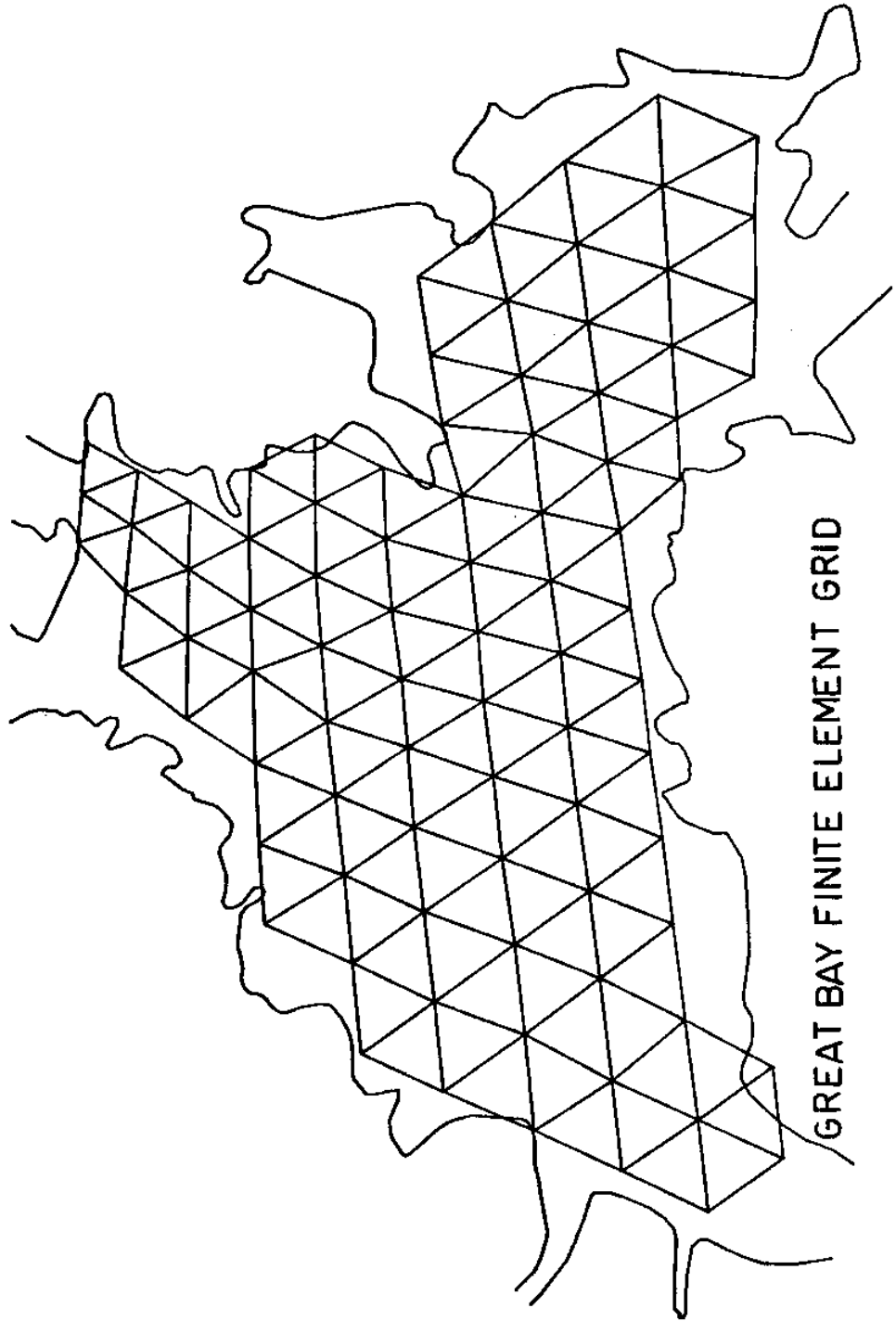
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11160.0	12480.0	10.0		18	5.0		1									240				
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41	42																			

42 51 1 60

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APPENDIX VII  
PORTSMOUTH HARBOR MODEL RESULTS

RESULTS OF THE MODEL AT 4.1 HOURS AFTER HIGH TIDE  
MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
1	16.46	-0.28	0.28	-0.49
2	16.46	-0.28	0.28	-0.50
3	3.05	-0.28	0.65	-0.65
4	8.85	-0.27	0.13	-0.52
5	13.40	-0.29	0.13	-0.36
6	10.36	-0.28	0.23	-0.36
7	3.04	-0.29	0.41	-0.37
8	7.31	-0.29	0.43	-0.37
9	14.94	-0.28	0.38	-0.40
10	7.30	-0.30	0.33	-0.37
11	3.05	-0.28	0.41	-0.39
12	4.88	-0.28	0.36	-0.32
13	16.45	-0.29	0.37	-0.36
14	4.87	-0.29	0.19	-0.34
15	3.63	-0.31	0.16	-0.57
16	5.77	-0.30	0.38	-0.55
17	18.57	-0.30	0.28	-0.55
18	3.03	-0.30	0.18	-0.58
19	11.85	-0.32	0.66	-0.38
20	16.45	-0.29	0.25	-0.38
21	3.02	-0.31	0.63	-0.33
22	5.75	-0.32	0.17	-0.09
23	5.76	-0.31	0.41	-0.05
24	15.41	-0.33	0.83	-0.26
25	3.61	-0.33	0.69	-0.17
26	3.61	-0.33	0.60	-0.12
27	19.45	-0.34	0.87	-0.23
28	4.82	-0.34	0.57	0.16
29	4.79	-0.37	0.35	0.09
30	2.99	-0.34	0.62	-0.13
31	19.44	-0.35	0.76	-0.04
32	10.29	-0.35	0.45	0.03
33	2.97	-0.36	0.52	-0.11
34	4.81	-0.35	0.59	-0.14
35	16.38	-0.36	0.57	-0.13
36	12.11	-0.35	0.46	-0.08
37	6.62	-0.37	0.48	-0.10
38	4.81	-0.35	0.53	-0.15
39	6.62	-0.37	0.59	-0.12
40	16.37	-0.37	0.44	-0.14

\*  
ETA = WATER SURFACE DEVIATION FROM MSL

## RESULTS OF THE MODEL AT 4.1 HOURS AFTER HIGH TIDE

MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
41	18.21	-0.36	0.48	-0.13
42	5.70	-0.37	0.46	-0.15
43	7.24	-0.36	0.56	-0.19
44	14.54	-0.37	0.44	-0.16
45	20.64	-0.37	0.37	-0.11
46	5.70	-0.37	0.47	-0.15
47	7.23	-0.37	0.58	-0.23
48	13.32	-0.37	0.39	-0.23
49	17.83	-0.38	0.29	-0.32
50	2.96	-0.37	0.50	-0.19
51	22.46	-0.38	0.58	-0.39
52	16.36	-0.38	0.27	-0.49
53	5.69	-0.38	0.58	-0.34
54	23.36	-0.39	0.72	-0.49
55	5.66	-0.41	0.77	-0.24
56	5.67	-0.40	0.35	-0.38
57	23.05	-0.40	0.57	-0.15
58	19.38	-0.41	0.27	-0.12
59	16.32	-0.42	0.39	0.11
60	23.03	-0.42	0.75	0.08
61	13.27	-0.42	0.74	0.11
62	5.65	-0.42	0.63	0.18
63	23.01	-0.44	0.79	0.11
64	7.78	-0.43	0.78	0.12
65	5.63	-0.44	0.73	0.17
66	23.00	-0.45	0.80	0.20
67	7.14	-0.46	0.83	0.16
68	5.63	-0.44	0.74	0.23
69	17.80	-0.46	0.88	0.23
70	13.24	-0.45	0.84	0.24
71	5.61	-0.46	0.78	0.27
72	22.35	-0.49	0.88	0.30
73	14.75	-0.47	0.81	0.29
74	5.62	-0.45	0.85	0.25
75	19.28	-0.51	0.86	0.24
76	5.58	-0.49	0.68	0.32
77	9.20	-0.53	-0.05	0.18
78	3.41	-0.52	0.02	0.03
79	5.57	-0.50	0.81	0.15
80	19.27	-0.52	0.73	0.19

\* ETA = WATER SURFACE DEVIATION FROM MSL

RESULTS OF THE MODEL AT 4.1 HOURS AFTER HIGH TIDE  
MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
81	14.09	-0.52	0.09	0.43
82	10.11	-0.53	0.09	0.17
83	10.12	-0.52	0.12	0.05
84	5.56	-0.51	0.81	0.14
85	17.73	-0.53	0.35	0.37
86	18.35	-0.52	0.23	0.16
87	5.54	-0.53	0.23	-0.04
88	2.80	-0.53	-0.06	0.02
89	5.54	-0.53	0.48	0.09
90	19.27	-0.52	0.34	0.05
91	22.31	-0.53	0.40	-0.09
92	2.80	-0.53	0.12	-0.11
93	5.54	-0.53	0.44	0.04
94	17.72	-0.54	0.43	0.06
95	20.46	-0.55	0.41	0.09
96	3.38	-0.56	0.09	0.16
97	2.77	-0.56	0.07	0.03
98	12.55	-0.53	0.39	0.00
99	15.20	-0.54	0.40	-0.02
100	20.76	-0.55	0.28	0.08
101	8.26	-0.56	0.13	0.04
102	4.62	-0.54	0.16	-0.02
103	5.52	-0.55	0.46	-0.04
104	18.31	-0.56	0.35	-0.05
105	15.80	-0.55	0.21	-0.08
106	4.59	-0.57	0.05	-0.07
107	3.40	-0.54	0.19	-0.19
108	11.01	-0.55	0.36	-0.31
109	19.01	-0.56	0.28	-0.29
110	14.05	-0.56	0.12	-0.29
111	2.77	-0.56	-0.06	-0.27
112	6.40	-0.59	-0.05	-0.33
113	8.26	-0.56	-0.05	-0.34
114	15.87	-0.57	0.29	-0.35
115	22.88	-0.57	0.27	-0.20
116	7.03	-0.57	0.22	-0.05
117	8.22	-0.60	-0.10	-0.36
118	7.02	-0.58	-0.05	-0.33
119	13.72	-0.58	-0.04	-0.34
120	17.38	-0.58	0.18	-0.27

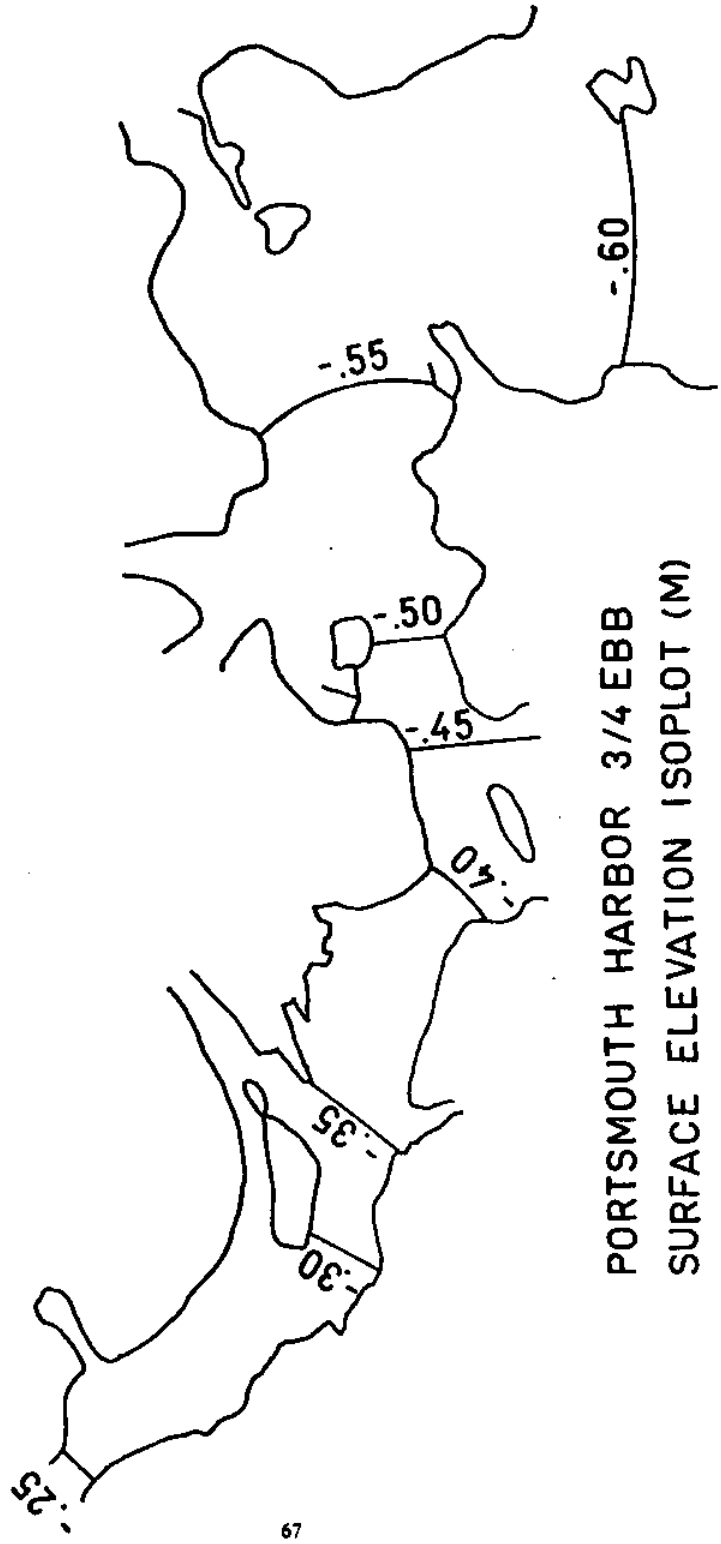
\*  
ETA = WATER SURFACE DEVIATION FROM MSL

RESULTS OF THE MODEL AT 4.1 HOURS AFTER HIGH TIDE

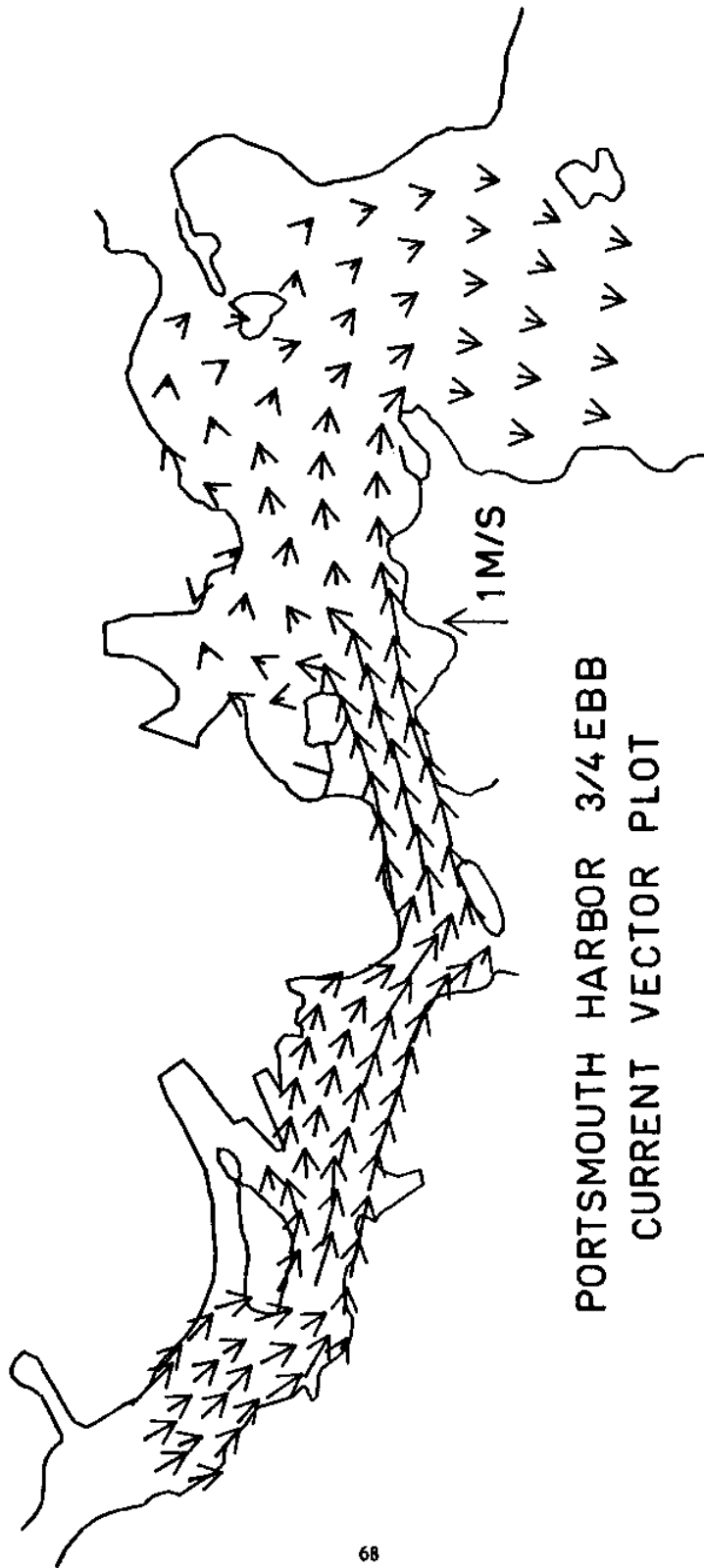
MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
121	14.03	-0.58	0.13	-0.15
122	2.75	-0.58	0.10	-0.10
123	11.57	-0.60	-0.08	-0.34
124	13.09	-0.60	-0.07	-0.29
125	19.20	-0.59	0.00	-0.30
125	11.59	-0.58	0.10	-0.23
127	2.74	-0.59	0.05	-0.20
128	14.01	-0.60	-0.05	-0.33
129	11.88	-0.59	-0.11	-0.26
130	10.68	-0.57	-0.01	-0.26
131	2.76	-0.57	0.06	-0.22
132	5.47	-0.60	-0.08	-0.31
133	19.21	-0.58	-0.11	-0.24
134	2.76	-0.57	-0.03	-0.28

\*  
ETA = WATER SURFACE DEVIATION FROM MSL



PORTSMOUTH HARBOR 3/4 EBB  
SURFACE ELEVATION ISOPLOT (M)



PORTSMOUTH HARBOR 3/4 EBB  
CURRENT VECTOR PLOT

APPENDIX VIII  
PISCATAQUA RIVER MODEL RESULTS

RESULTS OF THE MODEL AT 10.3 HOURS AFTER HIGH TIDE  
MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
1	14.82	-0.18	-0.43	0.01
2	10.81	-0.19	-0.69	0.13
3	5.34	-0.18	-0.30	0.01
4	13.26	-0.18	-0.52	0.02
5	7.18	-0.17	-0.53	0.23
6	11.76	-0.16	-0.61	0.00
7	5.35	-0.17	-0.35	0.00
8	2.94	-0.14	-0.18	0.15
9	10.23	-0.16	-0.43	0.07
10	4.14	-0.15	-0.39	-0.04
11	2.93	-0.15	-0.21	0.03
12	10.24	-0.15	-0.39	0.00
13	11.78	-0.14	-0.51	0.00
14	4.16	-0.14	-0.22	-0.03
15	2.95	-0.13	-0.40	0.13
16	7.21	-0.14	-0.41	0.02
17	11.78	-0.14	-0.54	-0.04
18	4.18	-0.12	-0.22	0.00
19	2.97	-0.11	-0.23	0.00
20	7.22	-0.13	-0.51	-0.05
21	11.31	-0.11	-0.52	0.07
22	4.16	-0.14	-0.39	0.03
23	3.00	-0.08	-0.05	0.00
24	8.77	-0.10	-0.46	0.06
25	11.83	-0.09	-0.53	-0.02
26	4.22	-0.08	-0.28	0.01
27	4.19	-0.11	-0.32	0.01
28	3.80	-0.07	-0.45	-0.01
29	11.83	-0.09	-0.54	0.12
30	3.00	-0.08	0.01	0.00
31	4.23	-0.07	-0.23	0.02
32	9.39	-0.09	-0.58	0.14
33	11.85	-0.07	-0.56	0.05
34	3.02	-0.06	0.40	-0.14
35	3.03	-0.05	-0.16	0.00
36	11.88	-0.04	-0.47	0.09
37	11.87	-0.05	-0.46	0.28
38	4.22	-0.08	0.04	-0.04
39	7.29	-0.06	-0.66	0.06
40	11.87	-0.05	-0.75	0.45

\* ETA = WATER SURFACE DEVIATION FROM MSL

RESULTS OF THE MODEL AT 10.3 HOURS AFTER HIGH TIDE

MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
41	7.30	-0.05	-0.12	0.17
42	7.32	-0.03	-0.51	0.20
43	11.93	0.01	-0.89	0.25
44	7.32	-0.03	-0.85	0.36
45	7.38	0.03	-0.39	0.19
46	11.92	0.00	-0.97	0.36
47	7.41	0.06	-0.28	0.09
48	7.32	-0.03	0.27	-0.04
49	11.97	0.05	-0.88	0.22
50	7.36	0.01	-0.91	0.09
51	7.42	0.07	-0.72	-0.26
52	11.96	0.04	-0.86	-0.30
53	7.46	0.11	-0.86	-0.33
54	7.40	0.05	-0.79	-0.38
55	12.03	0.11	-0.83	-0.40
56	7.38	0.03	-1.01	-0.45
57	7.42	0.07	-0.91	-0.43
58	12.09	0.17	-0.85	-0.37
59	7.51	0.16	-0.74	-0.54
60	7.44	0.09	-0.95	-0.48
61	12.07	0.15	-0.78	-0.51
62	7.51	0.16	-0.63	-0.38
63	7.56	0.21	-0.83	-0.42
64	12.14	0.22	-0.90	-0.19
65	3.24	0.16	-0.93	-0.28
66	7.47	0.12	-1.14	-0.46
67	12.20	0.28	-0.86	-0.32
68	3.31	0.23	-0.78	-0.71
69	3.34	0.26	-0.23	-0.24
70	7.61	0.26	-1.18	-0.36
71	12.16	0.24	-0.86	-0.29
72	9.23	0.36	-0.33	-0.23
73	3.33	0.25	-0.79	-0.14
74	7.65	0.30	-0.96	-0.24
75	12.21	0.29	-0.73	-0.50
76	13.71	0.27	-0.44	0.04
77	3.52	0.44	0.65	-0.36
78	7.63	0.28	-0.52	-0.06
79	12.29	0.37	-0.96	0.26
80	7.62	0.27	-0.72	0.48

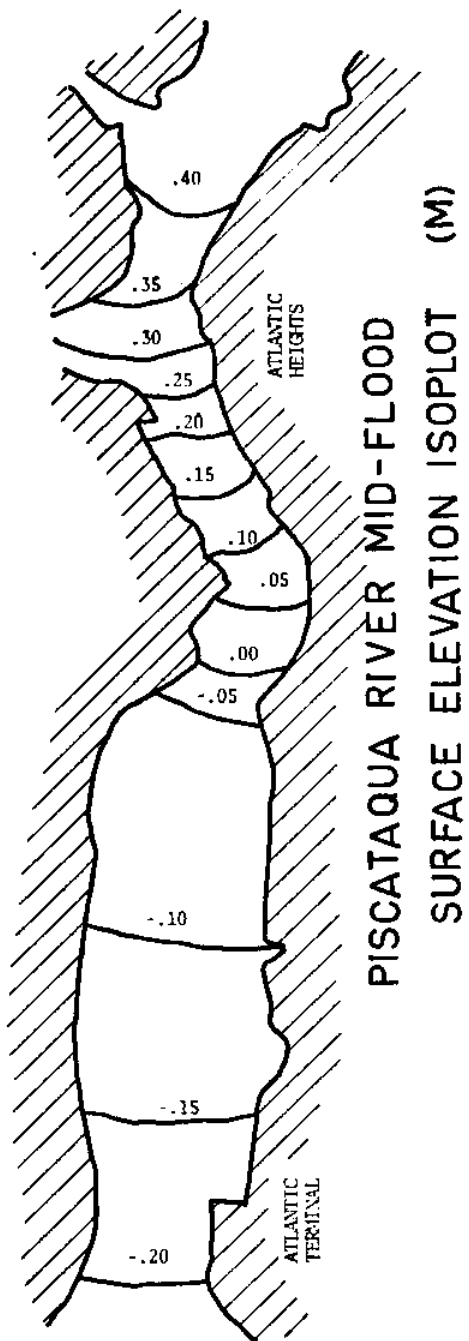
\* ETA = WATER SURFACE DEVIATION FROM MSL

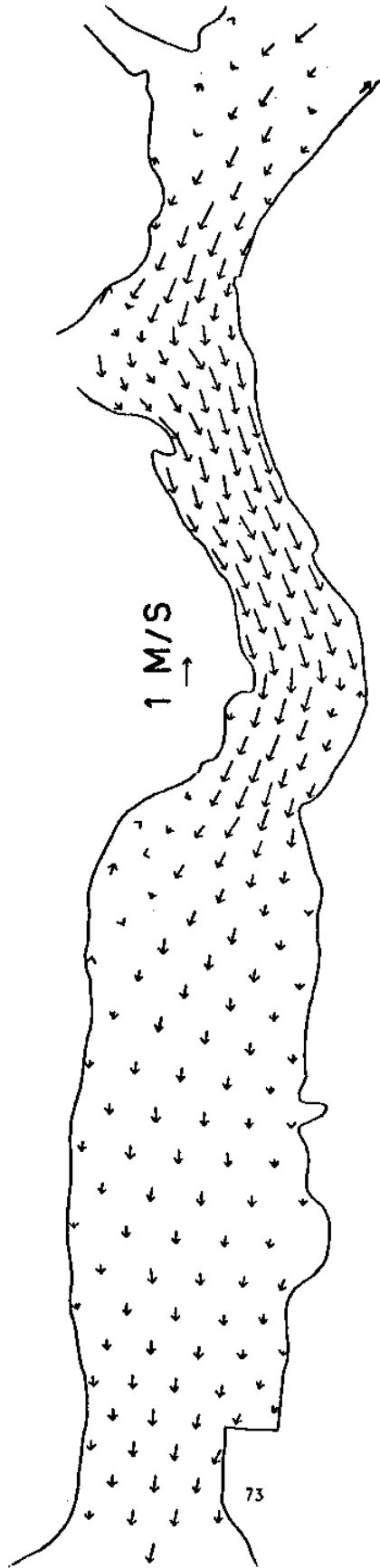


RESULTS OF THE MODEL AT 10.3 HOURS AFTER HIGH TIDE  
MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
81	7.59	0.24	-0.49	0.09
82	12.30	0.38	-1.12	0.29
83	7.68	0.33	-0.25	0.01
84	7.69	0.34	-0.62	0.28
85	12.31	0.39	-1.02	0.46
86	7.72	0.37	0.22	-0.05
87	7.71	0.36	-0.27	0.16
88	16.34	0.42	-0.77	0.34
89	7.74	0.39	0.25	-0.13
90	7.73	0.38	-0.21	0.17
91	16.35	0.43	-0.73	0.42
92	7.76	0.41	0.08	-0.04
93	7.76	0.41	0.39	-0.39
94	52.35	0.43	-0.19	0.21

\* ETA = WATER SURFACE DEVIATION FROM MSL





PISCATAQUA RIVER MID-FLOOD  
CURRENT VECTOR PLOT



APPENDIX IX  
LITTLE BAY MODEL RESULTS

RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE .

RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE		RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE .				
MKS UNITS		MKS UNITS				
NODE	WATER DEPTH	ETA *	VELOCITY U	VELOCITY V	VELOCITY U	VELOCITY V
1	5.06	0.06	-0.24	1.13	0.06	0.12
2	3.77	0.06	0.04	1.14	0.05	0.24
3	4.07	0.06	0.07	1.31	0.10	-0.05
4	12.67	0.12	-0.02	1.16	0.08	0.05
5	3.84	0.13	0.04	1.08	0.12	0.04
6	5.34	0.16	0.44	1.08	0.07	0.16
7	11.33	0.17	0.33	0.98	0.06	0.14
8	3.68	0.17	0.32	0.94	0.11	0.20
9	3.96	0.25	0.50	0.60	0.07	0.20
10	4.72	0.21	0.45	0.67	0.09	0.27
11	3.95	0.24	0.44	0.43	0.09	0.27
12	13.21	0.22	0.57	0.43	0.15	0.02
13	9.45	0.25	0.57	0.43	0.15	0.05
14	3.93	0.22	0.57	0.43	0.07	0.14
15	3.97	0.24	0.54	0.43	0.12	0.24
16	10.56	0.26	0.54	0.43	0.10	0.15
17	15.54	0.24	0.48	0.43	0.10	0.21
18	4.24	0.27	0.51	0.33	0.06	0.09
19	4.24	0.24	0.51	0.43	0.14	0.25
20	2.17	0.24	0.52	0.43	0.12	0.07
21	2.11	0.24	0.52	0.43	0.07	0.03
22	4.59	0.24	0.52	0.35	0.14	0.02
23	3.07	0.26	0.52	0.24	0.11	0.02
24	3.07	0.26	0.52	0.31	0.10	0.06
25	4.61	0.27	0.50	0.35	0.09	0.12
26	10.45	0.25	0.56	0.40	0.16	0.08
27	6.42	0.28	0.47	0.35	0.16	0.07
28	6.43	0.28	0.47	0.32	0.14	0.04
29	21.64	0.27	0.47	0.31	0.09	0.02
30	21.65	0.26	0.47	0.29	0.15	0.06
31	15.50	0.26	0.61	0.36	0.13	0.08
32	11.04	0.24	0.61	0.28	0.13	0.07
33	13.05	0.24	0.65	0.28	0.13	0.01
34	13.05	0.24	0.65	0.42	0.08	0.04
35	13.57	0.27	0.14	0.46	0.14	0.08
36	4.07	0.27	-0.07	0.46	0.07	0.09
37	4.07	0.28	-0.07	0.35	0.12	0.01

\* ETA = WATER SURFACE DEVIATION FROM MSL

RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE

MKS UNITS						
NODE	WATER DEPTH	ETA *	U VELOCITY	V VELOCITY	ETA *	U VELOCITY
81	9.56	0.33	-0.43	0.02	0.07	-0.08
82	4.12	0.11	-0.38	0.04	0.17	-0.15
83	4.16	0.45	-0.54	0.05	0.14	-0.00
84	18.74	0.40	-0.53	0.05	0.14	-0.01
85	7.59	0.32	-0.46	0.03	0.14	-0.05
86	4.19	0.43	-0.54	0.02	0.07	-0.01
87	4.14	0.43	-0.54	0.02	0.17	-0.03
88	10.65	0.43	-0.53	0.02	0.03	-0.14
89	12.69	0.45	-0.51	0.04	0.13	-0.11
90	4.15	0.44	-0.52	0.04	0.16	-0.05
91	4.16	0.45	-0.52	0.11	0.11	-0.02
92	12.37	0.46	-0.52	0.04	0.07	-0.02
93	4.17	0.43	-0.45	0.04	0.15	-0.14
94	4.17	0.46	-0.51	0.00	0.13	-0.05
95	4.22	0.51	-0.57	0.01	0.08	-0.07
96	10.26	0.45	-0.55	0.04	0.15	-0.05
97	11.09	0.43	-0.54	0.04	0.13	-0.05
98	4.20	0.49	-0.59	0.04	0.07	-0.11
99	4.22	0.49	-0.59	0.05	0.14	-0.05
100	10.87	0.45	-0.52	0.04	0.05	-0.07
101	12.74	0.50	-0.52	-0.01	0.12	-0.01
102	4.22	0.51	-0.55	0.05	0.13	-0.05
103	4.22	0.51	-0.55	0.05	0.13	-0.05
104	10.02	0.50	-0.54	0.10	0.08	-0.12
105	12.73	0.49	-0.52	0.10	0.12	-0.01
106	4.23	0.49	-0.52	0.13	0.05	-0.01
107	4.23	0.49	-0.52	0.13	0.05	-0.01
108	15.78	0.49	-0.52	0.17	0.14	-0.05
109	15.78	0.49	-0.52	0.17	0.14	-0.05
110	4.26	0.55	-0.57	0.42	0.10	-0.16
111	4.26	0.55	-0.57	0.42	0.10	-0.16
112	16.03	0.55	-0.57	0.42	0.04	-0.04
113	10.97	0.55	-0.57	0.42	0.07	-0.07
114	9.76	0.55	-0.57	0.24	0.04	0.01
115	4.27	0.50	-0.53	0.44	0.12	-0.04
116	16.27	0.52	-0.54	0.34	0.07	-0.03
117	4.26	0.58	-0.59	0.59	0.12	-0.03
118	15.97	0.58	-0.59	0.25	0.11	-0.07
119	5.73	0.58	-0.59	0.00	0.07	-0.07

\* ETA = WATER SURFACE DEVIATION FROM MSL

RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE

MKS UNITS

NODE	WATER DEPTH	ETA	U VELOCITY	V VELOCITY
81	1.52	0.14	0.06	0.04
82	1.95	0.12	0.05	0.02
83	3.46	0.13	-0.02	-0.00
84	1.59	0.00	0.02	-0.02
85	1.55	0.13	-0.04	-0.05
86	1.64	0.14	0.09	-0.05

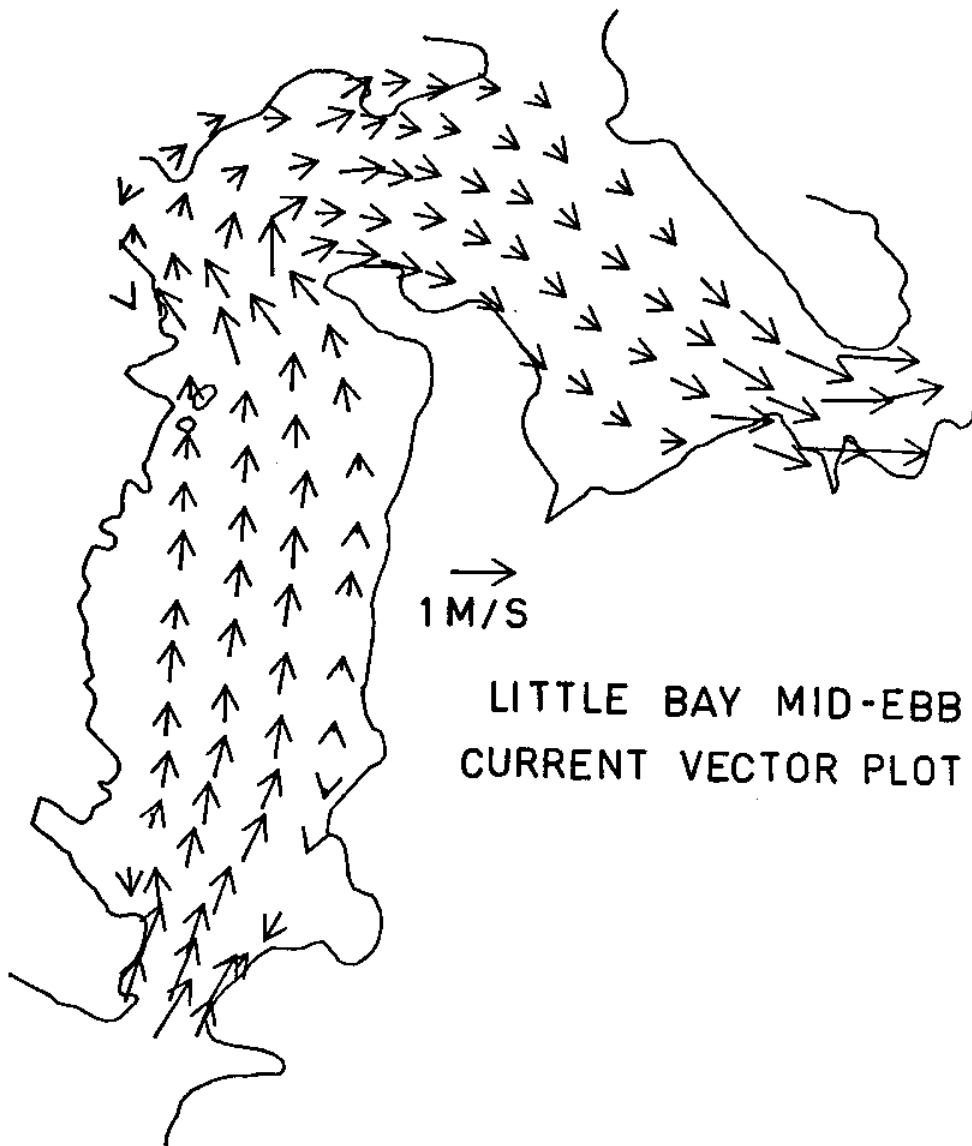
\* ETA = WATER SURFACE DEVIATION FROM MSL

RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE

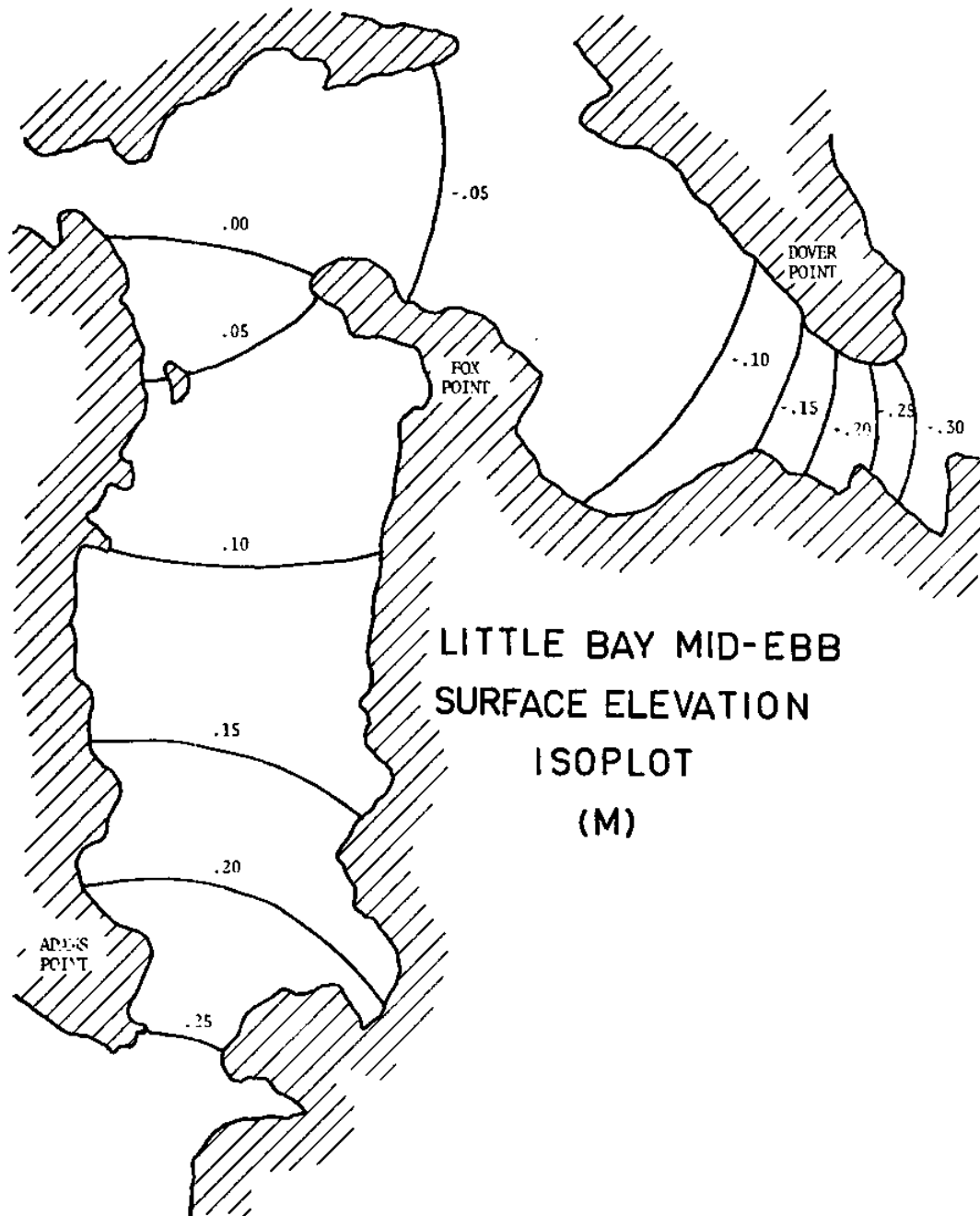
MKS UNITS

NODE	WATER DEPTH	ETA	U VELOCITY	V VELOCITY
41	12.70	0.28	0.14	0.41
42	12.22	0.28	0.08	0.50
43	5.21	0.15	0.04	0.56
44	13.97	0.26	-0.12	0.45
45	14.07	0.29	-0.14	0.39
46	13.04	0.27	0.11	0.39
47	4.61	0.30	0.07	0.51
48	10.66	0.29	0.05	0.27
49	4.61	0.30	-0.23	0.60
50	4.49	0.30	-0.09	0.71
51	4.01	0.30	-0.02	0.44
52	16.42	0.31	-0.12	0.46
53	14.67	0.30	0.00	0.41
54	12.95	0.31	-0.20	0.57
55	4.53	0.31	-0.54	0.54
56	12.56	0.32	-0.24	0.37
57	4.50	0.31	-0.12	0.28
58	6.57	0.42	-0.22	0.27
59	4.04	0.32	-0.30	-0.43
60	4.04	0.33	-0.35	0.10
61	11.04	0.32	-0.27	0.13
62	13.05	0.36	-0.46	-0.14
63	15.29	0.37	-0.63	-0.43
64	4.91	0.33	-0.60	-0.43
65	12.57	0.32	-0.41	-0.07
66	10.82	0.41	-0.25	-0.00
67	4.12	0.41	-0.51	-0.10
68	17.20	0.38	-0.59	-0.24
69	4.64	0.32	-0.44	-0.44
70	4.06	0.35	-0.02	0.01
71	4.14	0.43	-0.29	-0.02
72	4.09	0.39	-0.50	-0.01
73	13.08	0.39	-0.44	0.04
74	4.12	0.41	-0.42	-0.01
75	4.11	0.40	-0.20	0.07
76	13.44	0.44	-0.56	0.05

\* ETA = WATER SURFACE DEVIATION FROM MSL









APPENDIX X  
GREAT BAY MODEL RESULTS

RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE  
MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
1	1.57	0.07	-0.08	-0.05
2	1.67	0.17	-0.02	0.15
3	1.65	0.15	0.01	-0.00
4	1.64	0.14	-0.01	0.01
5	1.55	0.05	0.01	-0.01
6	1.64	0.14	-0.12	0.00
7	1.87	0.07	-0.03	-0.01
8	1.97	0.17	-0.09	0.01
9	1.63	0.13	0.03	-0.03
10	1.55	0.05	-0.18	0.11
11	2.27	0.16	-0.00	-0.00
12	2.52	0.11	-0.05	0.02
13	2.18	0.07	0.02	0.04
14	1.65	0.15	-0.14	0.06
15	1.93	0.13	-0.12	0.07
16	3.10	0.08	-0.06	0.00
17	4.70	0.15	-0.03	0.00
18	1.63	0.13	-0.13	-0.02
19	1.87	0.07	-0.11	0.05
20	3.47	0.14	-0.05	0.05
21	2.53	0.12	-0.05	0.06
22	1.86	0.06	-0.14	-0.03
23	3.15	0.13	-0.07	0.03
24	3.71	0.08	0.01	0.12
25	2.87	0.15	-0.20	0.21
26	1.92	0.12	0.03	0.08
27	1.61	0.11	-0.01	0.31
28	1.55	0.05	-0.19	-0.03
29	4.08	0.14	0.11	-0.00
30	3.73	0.10	-0.16	0.10
31	2.80	0.08	0.04	0.20
32	1.89	0.09	-0.00	0.25
33	1.57	0.07	-0.25	0.16
34	1.66	0.16	0.04	0.01
35	1.62	0.12	0.03	0.11
36	1.87	0.07	0.03	0.09
37	5.28	0.12	-0.03	0.20
38	7.10	0.11	0.09	0.14
39	10.71	0.07	-0.07	0.13
40	13.70	0.01	0.13	0.22

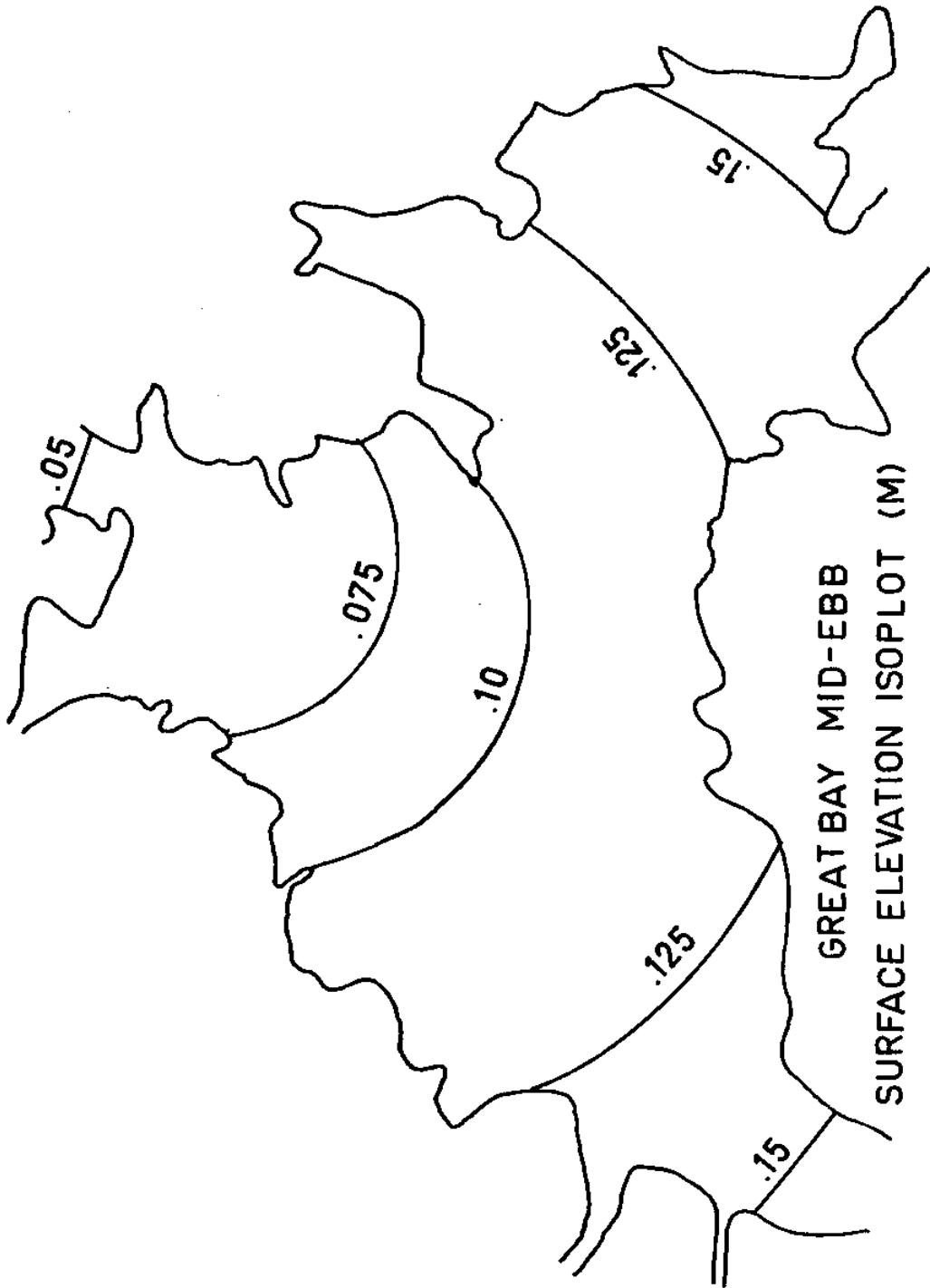
\* ETA = WATER SURFACE DEVIATION FROM MSL

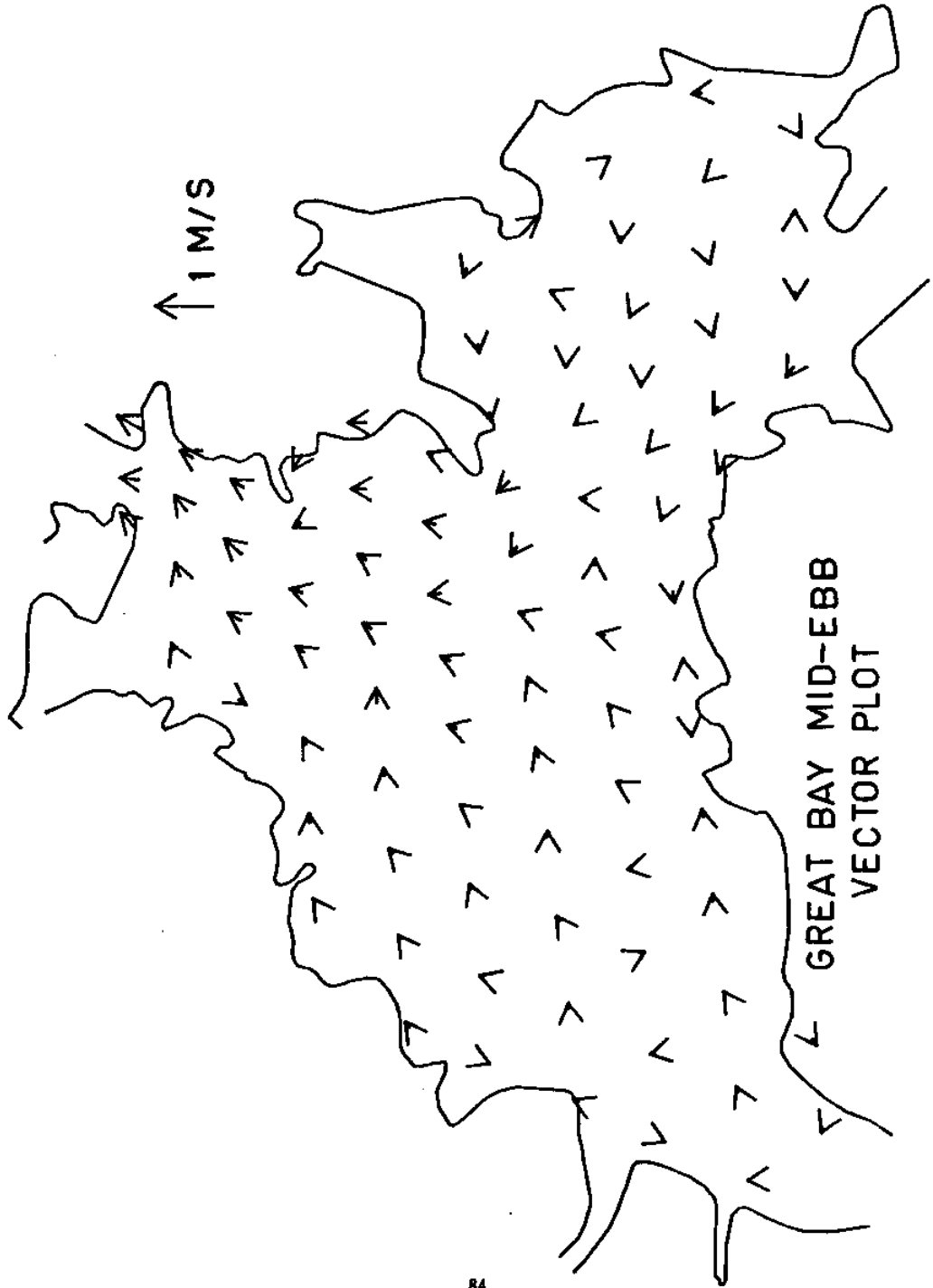
RESULTS OF THE MODEL AT 3.0 HOURS AFTER HIGH TIDE .

MKS UNITS

NODE	WATER DEPTH	* ETA	U VELOCITY	V VELOCITY
41	15.58	0.06	0.12	0.22
42	3.08	0.06	0.24	0.44
43	1.90	0.10	-0.05	-0.01
44	1.88	0.08	0.06	0.05
45	3.45	0.12	0.08	0.04
46	5.27	0.11	0.07	0.16
47	7.66	0.06	0.08	0.14
48	10.75	0.11	0.06	0.20
49	7.67	0.07	0.25	0.30
50	10.72	0.08	0.27	0.21
51	15.28	0.06	0.03	0.32
52	1.88	0.08	0.13	0.02
53	1.95	0.15	0.03	0.05
54	3.45	0.12	0.15	0.06
55	1.87	0.07	0.04	0.14
56	1.92	0.12	0.24	0.01
57	3.43	0.10	0.10	0.15
58	3.40	0.07	0.13	0.21
59	2.21	0.10	0.23	0.08
60	3.08	0.06	0.26	0.25
61	1.64	0.14	0.12	0.01
62	1.92	0.12	-0.02	0.07
63	3.40	0.07	0.09	0.00
64	2.86	0.14	0.02	0.03
65	2.22	0.11	0.11	0.02
66	2.18	0.07	0.10	0.06
67	1.89	0.09	-0.08	-0.12
68	1.59	0.09	-0.10	-0.05
69	1.66	0.16	-0.10	-0.07
70	1.64	0.14	0.04	-0.03
71	2.19	0.08	0.02	-0.05
72	3.17	0.15	0.08	0.05
73	1.63	0.13	0.08	0.03
74	1.57	0.07	0.08	0.07
75	1.64	0.14	0.15	0.01
76	1.63	0.13	-0.07	0.03
77	1.58	0.08	0.08	0.04
78	3.47	0.14	-0.01	0.06
79	2.23	0.12	0.09	0.01
80	1.57	0.07	0.01	0.02

\*  
ETA = WATER SURFACE DEVIATION FROM MSL





APPENDIX XI  
PROGRAM FOR CALCULATING NORMAL  
ANGLES FOR LAND BOUNDARY ANGLES

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C      CAFE (CIRCULATION ANALYSIS BY FINITE ELEMENTS) DATA ASSIST PROGRAM
C      PROGRAM CALCULATES NORMAL ANGLES FOR LAND BOUNDARY NODES
C
C      INPUT: CARD GROUP 1 (1 CARD) NUMBER OF NODE POINTS NMNP (I10)
C             CARD GROUP 2 (NMNP CARDS) NODE DATA CARDS AS USED IN CAFE
C             CARD GROUP 3 LAND BOUNDARY DATA AS USED IN CAFE
C
C      OUTPUT: NODE NUMBER, NORMAL ANGLE, ANGLE FORMED BY NODE AND
C              PRECEDING NODE, ANGLE FORMED BY NODE AND FOLLOWING NODE
C
C
C      DIMENSION N(10),NODES(100),X(200),Y(200)
C      DIMENSION ANGLE(200),DEPTH(200),I1(200),I2(200)
C      OPEN(UNIT=5,DEVICE='DSK',ACCESS='SEGIN',FILE='PORT.DAT')
C      OPEN(UNIT=7,DEVICE='DSK',ACCESS='SEQOUT',FILE='PORTS.DAT')
C      PI=3.14159
C      DEG=360./(2.*PI)
C
C      READ NUMBER OF NODES
C      READ(5,100) NMNP
C      DO 10 I=1,NMNP
10     ANGLE(I)=0.0
C
C      READ X AND Y COORDINATES OF NODES
C      READ(5,150) (I1(I),I2(I),X(I),Y(I),DEPTH(I),I=1,NMNP)
150     FORMAT(2I5,3F10.0)
C
C      READ NUMBER OF LAND BOUNDARY STRINGS (NUM) AND NUMBER OF NODES
C      IN EACH STRING (N(I))
C      READ(5,100) NUM,(N(I),I=1,NUM)
100     FORMAT(8I10)
C
C      DO 99 K=1,NUM
C      IEND=N(K)
C
C      READ NODES IN STRING (K)
C      READ(5,101) (NODES(I),I=1,IEND)
101     FORMAT(20I4)
C
C      JEND=IEND-1
C      WRITE(6,200)
200     FORMAT(1H1,5X,'NORMAL ANGLES FOR LAND BOUNDARY'//11X,'NODE',10X,
1      'ANGLE',9X,'ANGLE1',9X,'ANGLE2'//)
C
C      SET UP LOOP TO CALCULATE NORMAL ANGLES FOR ALL NODES EXCEPT

```

```

C      FIRST AND LAST
      DO 50 J=2,JEND
C
      NP1=NODES(J-1)
      NP0=NODES(J)
      NP2=NODES(J+1)
      Y1=Y(NP1)-Y(NP0)
      Y2=Y(NP2)-Y(NP0)
      X1=X(NP1)-X(NP0)
      X2=X(NP2)-X(NP0)
C
C      CALCULATE PRECEDING ANGLE (ANG1)
      ANG1=ATAN(Y1/X1)*DEG
C
C      CALCULATE FOLLOWING ANGLE (ANG2)
      ANG2=ATAN(Y2/X2)*DEG
C
      IF(X1.LT.0.0) ANG1=180.+ANG1
      IF(X2.LT.0.0) ANG2=180.+ANG2
      IF(ANG1.LT.0.0) ANG1=ANG1+360.
      IF(ANG2.LT.0.0) ANG2=ANG2+360.
C
C      CALCULATE NORMAL ANGLE
      ANG=(ANG2+ANG1)/2.
C
C      CORRECT ANGLE TO OUTWARD NORMAL IF NECESSARY
      IF(ANG1.GT.ANG2) ANG=ANG+180.
C
      IF(ANG.GT.360.) ANG=ANG-360.
      NODEJ=NODES(J)
      ANGLE(NODEJ)=ANG
50  WRITE(6,201) NODES(J),ANG,ANG1,ANG2
201  FORMAT(5X,110,3(5X,F10.4))
99  CONTINUE
      WRITE(7,202) (I1(I),I2(I),X(I),Y(I),DEPTH(I),ANGLE(I),I=1,NMNP)
202  FORMAT(215,3F10.2,10X,F10.2)
      STOP
      END

```



APPENDIX XII

PROGRAM FOR CALCULATING NORMAL ANGLES FOR LAND BOUNDARY ANGLES

```

DIMENSION TITLE(20),TEXT1(2,2),TEXT2(2,2),TEXT3(2,2),
1  ICON(190,3),A(190,3),B(190,3),AREA(190),
2  NEXT(135),NINT(135),XORD(135),YORD(135),DEPTH(135),NBC(135),
3  SYSMH(135,12),SYSMQ(270,24),
4  H(135),Q(270),HPREV(135),QPREV(270),HOLD(135),QOLD(270),
5  H2(135),Q2(270),SYSFH(135),SYSFQ(270),SYSFH8(135),SYSFQ8(270),
6  SYSBMH(12,12),SYSBMQ(112,24),NHN(25),NGN(135),NVN(135),
7  HE(25),ALAG(25),QB(135),QBANG(135),TAUWX(135),TAUWY(135),
8  PSPLUS(135),CF(190),EDXX(190),EDYY(190),
9  EDXY(190),NFLUX(30),FLUX(30)
DIMENSION ETA(135),U(135),V(135),ETAPRV(135),
1  NMLBN(3),ICONL(3,70),NMHNPB(3),ICONB(3,70)
EQUIVALENCE(HPREV(1),ETAPRV(1))
COMMON/SORTNO/K1,K2,K3
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1  MAXBWQ,NMHBN,NMQBN,NMYBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2  MAXQBM,MAXHBM
COMMON/COUPTP/NOUT
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
COMMON/COPT/IDFPIC,IDEPTH,IEDVIS,ICONVEC,IWIND,IVERSN
COMMON/CPROP/GRAVT,CORIO,DENSTY
DATA TEXT1,TEXT2,TEXT3/4HVARY,4HCONS,3HING,4HTANT,4HSET,4HREAD,
1  4HTO,0,3H IN,4HIGNO,4HINCL,4HRED,4HUDED/
C  MAXHBN=NMHBN
C  MAXQBN=NMQBN
C  MAXL=GE,MAX(NMLBN);MAXQ=GE,MAX(NMHNPB)
C  DIMENSION SYSMH(MAXNOD,MAXBWH),SYSMQ(MAXMQ,MAXBWQ),
C  SYSBMH(MAXHBM,MAXBWM),SYSBMQ(MAXQBM,MAXBWQ),ICONL(3,MAXL),ICONB(3,
  OPEN(UNIT=5,ACCESS='SEQIN',DEVICE='DSK',FILE='PORTSH')
  OPEN(UNIT=6,DEVICE='DSK',ACCESS='SEQOUT',DISPOSE='RENAME',
1  FILE='PORTSH.LPT')
  QPFN(UNIT=8,DEVICE='DSK',ACCESS='SEQOUT',FILE='PHOUT')
  OPEN(UNIT=4,DEVICE='DSK',ACCESS='SEQIN',FILE='CONDIN')
MAXQ=70
MAXL=70
MAXNOD=135
MAXEL=190
MAXBWH=12
MAXBWQ=24
MAXHBN=6
MAXQBN=56
MAXMQ=2*MAXNOD
MAXQBM=MAXQBN*2
MAXHBM=MAXHBN*2
320 NBANDH=0

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      READ(S,1001) IVERSN, NMEL, NMNP, IBFRIC, IDEPTH, IEDVIS, IWIND, INPUTH,
1     INPUTQ, ICNVEC, IPLOT
1001 FORMAT(16I5)
      IF( IVERSN .NE. 2) STOP
      LENGTH=MAXNOD*MAXBWH
      CALL AMATZR(SYSMH,LENGTH)
      LENGTH=MAXMQ*MAXBWQ
      CALL AMATZR(SYSMQ,LENGTH)
      LENGTH=MAXHBM*MAXBWH
      CALL AMATZR(SYSBMH,LENGTH)
      LENGTH=MAXOBM*MAXBWQ
      CALL AMATZR(SYSBMQ,LENGTH)
      CALL AMATZR(PSPLUS, NMNP)
      CALL AMATZR(ETA, MAXNOD)
      CALL AMATZR(Q, MAXMQ)
      CALL AMATZR(QPREV, MAXMQ)
      TUP1=6.28318
      NMHBN=0
      NMVBN=0
      NMOBN=0
      IFLUX=0
      READ(S,1003) TITLE
1003 FORMAT(20A4)
      WRITE(6,1002) TITLE
1002 FORMAT(1H1//1H-.25X,20A4)
      CALL SLINE(36)
      WRITE(6,1004) NMEL, NMNP, IVERSN, (TEXT1( IBFRIC, I), I=1,2),
1     (TEXT1( IDEPTH, I), I=1,2), (TEXT1( IEDVIS, I), I=1,2),
2     (TEXT1( IWIND, I), I=1,2), (TEXT2( INPUTH, I), I=1,2),
3     (TEXT2( INPUTQ, I), I=1,2), (TEXT3( ICNVEC, I), I=1,2)
1004 FORMAT(1H0,5X,'THIS PROBLEM HAS THE FOLLOWING CHARACTERISTICS:/'
1     1H0,10X,'NUMBER OF ELEMENTS, NMEL = ',I5/1H ,
2     10X,'NUMBER OF NODES, NMNP = ',I5/1H ,
3     10X,'THE MODEL APPLIED IS VERSION ',I1/1H ,
4     10X,'IT IS ASSUMED THAT SPATIALLY, '/1H ,
5     40X,'BOTTOM FRICTION IS ',2A4/1H ,
6     40X,'MEAN LOW WATER DEPTH IS ',2A4/1H ,
7     40X,'EDDY VISCOSITY IS ',2A4/1H ,
8     40X,'WIND STRESS IS ',2A4/1H ,
9     10X,'INITIAL VALUES OF H ARE ',2A4/1H ,
A     10X,'INITIAL VALUES OF Q ARE ',2A4/1H ,
B     10X,'CONVECTIVE ACCELERATIONS ARE ',2A4/1H )
      CALL SLINE(36)
      NMNP2=NMNP*2
      CALL AMATZR(ETA, NMNP)
      CALL AMATZR(Q, NMNP2)
      WRITE(6,1010)
1010 FORMAT(1H0,6X,'NODE',8X,'X-',11X,'Y-',8X,'DEPTH',4X,'NODE',5X,
1     ' TIDAL',5X,'TIME',5X,'LOCAL X',6X,'FLUX',5X,' LOCAL Y',4X,
3     ' SOURCE'/1H ,5X,'NUMBER',3X,'COORDINATE',3X,'COORDINATE',4X,
4     '(MWL)',4X,'CODE',3X,'AMPLITUDE',4X,'LAG',6X,'FLUX',7X,'ANGLE',
5     7X,'FLUX',7X,'FLUX'/1H ,18X,'(M)',10X,'(M)',8X,'(M)',15X,
6     '(M)',6X,'(SEC)',3X,'(M2/SEC)',3X,'(DEGREES)',3X,'(M2/SEC)',
7     4X,'(M/SEC)'/1H0)
      DO 10 I=1, NMNP
      READ(S,1005) NEXT(I), NBC(I), XORD(I), YORD(I), DEPTH(I), DUM1, DUM2,
1     DUM3, DUM4
1005 FORMAT(2I5,7F10.0)
      NINT(NEXT(I))=I
      I1=NBC(I)

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N=NEXT(I)
IF(IDEPTH .EQ. 1) GO TO 60
DEPTH(I)=DEPTH(I)
60 IF(I1 .EQ. 0) GO TO 70
IF(I1 .EQ. 1) GO TO 80
IF(I1 .EQ. 2) GO TO 90
IF(I1 .EQ. 3) GO TO 100
IF(I1 .EQ. 6) GO TO 50
IF(I1 .EQ. 4) GO TO 110
NMHBN=NMHBN+1
NMQBEN=NMQBEN+1
NMVBN=NMVBN+1
NHN(NMHBN)=I
NQN(NMQBN)=I
NVN(NMVBN)=I
HB(NMHBN)=DUM1
ALAG(NMHBN)=DUM2
QB(NMQBN)=DUM3
QBANG(NMQBN)=DUM4
DUM4=0.
WRITE(6,1006) N,XORD(I),YORD(I),DEPTH(I),NBC(I),HB(NMHBN),
I ALAG(NMHBN),QB(NMQBN),QBANG(NMQBN),DUM4
1006 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I6,4X,F5.2,4X,
I F6.0,4X,F8.4,5X,F6.1,6X,F3.1)
QBANG(NMQBN)=QBANG(NMQBN)*3.14159/180.
GO TO 10
110 NMQBEN=NMQBEN+1
NMVBN=NMVBN+1
NQN(NMQBN)=I
NVN(NMVBN)=I
QB(NMQBN)=DUM1
QBANG(NMQBN)=DUM2
WRITE(6,1008)N,XORD(I),YORD(I),DEPTH(I),NBC(I),QB(NMQBN),
I QBANG(NMQBN),DUM3
1008 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I5,24X,F8.4,
I 5X,F6.1,6X,F3.1)
GO TO 10
100 NMHBN=NMHBN+1
NMQBEN=NMQBEN+1
NHN(NMHBN)=I
NQN(NMQBN)=I
HB(NMHBN)=DUM1
ALAG(NMHBN)=DUM2
QB(NMQBN)=DUM3
QBANG(NMQBN)=DUM4
WRITE(6,1012)N,XORD(I),YORD(I),DEPTH(I),NBC(I),HB(NMHBN),
I ALAG(NMHBN),QB(NMQBN),QBANG(NMQBN)
1012 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I4,6X,F5.2,4X,
I F6.0,4X,F8.4,5X,F6.1)
QBANG(NMQBN)=QBANG(NMQBN)*3.14159/180.
GO TO 10
90 NMHBN=NMHBN+1
NHN(NMHBN)=I
HB(NMHBN)=DUM1
ALAG(NMHBN)=DUM2
WRITE(6,1014)N,XORD(I),YORD(I),DEPTH(I),NBC(I),HB(NMHBN),
I ALAG(NMHBN)
1014 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I3,7X,F5.2,4X,F6.0)
GO TO 10
80 NMQBEN=NMQBEN+1

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      NQN(NMQBN)=I
      QB(NMQBN)=DUM1
      QBANG(NMQBN)=DUM2
      WRITE(6,1016)N,XORD(I),YORD(I),DEPTH(I),NBC(I),QB(NMQBN),
1    QBANG(NMQBN)
1016 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I2,27X,F8.4,
1    5X,F6.1)
      QBANG(NMQBN)=QBANG(NMQBN)*3.14159/180.
      GO TO 10
70    WRITE(6,1018) N,XORD(I),YORD(I),DEPTH(I),NBC(I)
1018 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I1)
      GO TO 10
50    IFLUX=IFLUX+1
      NFLUX(IFLUX)=I
      FLUX(IFLUX)=DUM1
      WRITE(6,1020)N,XCRD(I),YORD(I),DEPTH(I),NBC(I),FLUX(IFLUX)
1020 FORMAT(1H ,5X,I4,5X,F10.2,3X,F10.2,3X,F6.2,3X,I6,56X,F8.4)
10    CONTINUE
      CALL SLINE(15)
      WRITE(6,1030)NMHBN,NMQBN,NMVBN,IFLUX
1030 FORMAT(1H0,5X,'NUMBER OF PRESCRIBED BOUNDARY AND INTERNAL FLUX NOD
1    IES'/1H0,10X,'PRESCRIBED HEIGHTS, NMHBN =',I5/1H ,10X,
2    'PRESCRIBED LOCAL X FLUX, NMQBN =',I5/1H ,10X,
3    'PRESCRIBED X AND Y FLUX, NMVBN =',I5/1H ,10X,
4    'INTERNAL FLUX NODES, IFLUX =',I5)
      CALL SLINE(36)
      READ(5,1007)(N,(ICON(N,J),J=1,3),CF(N),EDXX(N),EDYY(N),EDXY(N),
1    L=1,NMEL)
1007 FORMAT(4I10,4F10.0)
      WRITE(6,1022)
1022 FORMAT(1H0,5X,'ELEMENT CONNECTIVITIES.'/1H0,10X,'ELEMENT NUMBER',
1    3X,'NODE 1',3X,'NODE 2',3X,'NODE 3',3X,'FRICTION COEFFICIENT',
2    3X,'EDDY XX',3X,'EDDY YY',3X,'EDDY XY'/1H0)
      IF(1BFRICT.EQ.1) GO TO 130
      DO 120 I=2,NMEL
      CF(I)=CF(1)
120    CONTINUE
130    IF(1VERSN.EQ.1.OR.1EDVIS.EQ.1) GO TO 200
      DO 210 I=2,NMEL
      EDXX(I)=EDXX(1)
      EDYY(I)=EDYY(1)
      EDXY(I)=EDXY(1)
210    CONTINUE
200    DO 220 I=1,NMEL
      WRITE(6,1024)I,(ICON(I,J),J=1,3),CF(I),EDXX(I),EDYY(I),EDXY(I)
1024 FORMAT(1H ,16X,I3,10X,I3,6X,I3,6X,I3,12X,F10.6,3X,F9.2,
1    1X,F9.2,1X,F9.2)
220    CONTINUE
      CALL SLINE(36)
      CALL GEOM(NINT,ICON,A,B,AREA,XORD,YORD,SYSMH)
      NBANDQ=2*NBANDH
      CALL QMAT(SYSM0,SYSMH)
      IF(NMQBN.GT.0) CALL ROTM0(SYSM0,NQN,QBANG)
      READ(5,1009) ALATT,OMEGA,GRAVT,PERIOD,DENSTY
1009 FORMAT(F9.1,E11.5,F7.2,F11.1,F9.1)
      CORIO=2.*OMEGA*57.3/(ALATT*3.14159/180.)
      PHASE=TUPI/PERIOD
      WRITE(6,1026)ALATT,CORIO,GRAVT,OMEGA,PERIOD,DENSTY
1026 FORMAT(1H0,5X,'SYSTEM PROPERTIES.'/1H0,10X,
1    'AVERAGE LATITUDE, ALATT =',F7.2,2X,'(DEGREES N)'/1H ,

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2 10X,'CORIOLIS PARAMETER, CORIO = 2*OMEGA*SIN(ALATT) = ',E10.3,
3 2X,'(SEC-1)'/1H ,10X,'GRAVITATIONAL ACCELERATION, GRAVT = ',F6.3
5 , 2X,'(M/SEC2)'/1H ,10X,'ANGULAR VELOCITY OF EARTH ROTATION, OME
6GA = ',E10.3,2X,'(SEC-1)'/1H ,10X,
7 'PERIOD OF HARMONIC TIDAL EXCITATION, PERIOD = ',F6.0,2X,
8 '(SEC)'/1H 10X,'DENSITY OF WATER, DENSTY = ',F7.2,' (KG/M3)')
CALL SLINE(36)
READ(5,1011)STRTIM,ENDTIM,TINC,NO,BOUND,IDT,NOU
1011 FORMAT(2F9.1,F8.1,4X,I10,F6.1,4X,2I10)
IF(IDT,EQ.1) GOTO 450
450 IF(NMHBN,EQ.0) GOTO 400
CALL STORNO(MAXNOD,MAXBWH,MAXHBM,MAXBWH,NBANDH,NMHBN,1,
1 SYSMH,NHN,SYSBMH,0)
400 IF(NMVBN ,EQ. 0) GO TO 410
CALL STORNO(MAXMQ,MAXBWQ,MAXQBM,MAXBWQ,NBANDQ,NMVBN,2,SYSMQ,NVN,
1 SYSBMQ,1)
LENGTH=MAXQBN*MAXBWQ
CALL AMATZP(SYSBMQ,LENGTH)
410 IF(NMQBN ,EQ. 0) GO TO 170
CALL STORNO(MAXMQ,MAXBWQ,MAXQBM,MAXBWQ,NBANDQ,NMQBN,2,SYSMQ,
1 NQN,SYSBMQ,0)
170 CALL DECOMP(NMNP,MAXNOD,MAXBWH,NBANDH,SYSMH)
CALL DECOMP(NMNP2,MAXMQ,MAXBWQ,NBANDQ,SYSMQ)
TINC2=TINC/2
IF(INPUTQ,EQ.2) READ(4,9940) STRTIM
9940 FORMAT(F10.0)
TIME=STRTIM
WRITE(6,1028)STRTIM,ENDTIM,TINC,NO,BOUND,(TEXT1(IDT,J),J=1,2),
1 NOU,IPLOT
1028 FORMAT(1H0,5X,'INTEGRATION PARAMETERS, '/1H0,10X,
1 'START TIME OF INTEGRATION, STRTIM = ',F9.1,2X,'SEC'/
2 1H ,10X,'END TIME OF INTEGRATION, ENDTIM = ',F9.1,2X,'SEC'/
3 1H ,10X,'CONSTANT TIME INCREMENT, TINC = ',F7.1,2X,'SEC'/1H ,10X
4 , 'EXTERNAL NODE AT WHICH VARIATION IS BOUNDED BY BOUND, NO = ',
5 14/1H ,10X,'CRUDE STABILITY CONTROL,BOUND = ',F6.2/1H ,10X,
6 'THE TIME INCREMENT IS ASSUMED ',2A4/1H ,10X,
7 'OUTPUT WILL BE PRINTED FOR EVERY ',I3,' TIMESTEPS',
8 'OUTPUT WILL BE STORED FOR EVERY ',I3,' TIMESTEPS')
NO=NINT(NO)
CALL SLINE(36)
READ(5,1015) NMLB,(NMLBN(L), L=1,NMLB)
DO 350 I=1,NMLB
JEND=NMLBN(I)
READ(5,1013)(ICONL(I,J), J=1,JEND)
1015 FORMAT(8(10))
350 CONTINUE
DO 360 I=1,NMLB
WRITE(6,1052) I,NMLBN(I),(ICONL(I,J), J=1,JEND)
1052 FORMAT(1H0,5X,'LAND SEGMENT ',I2,5X,'# NODES, NMLBN = ',I2/1H ,5X,
1 'EXTERNAL NODE NUMBERS: ',25(I3,'-')/1H ,20X,25(I3,'-'))
DO 370 J=1,JEND
ICONL(I,J)=NINT(ICONL(I,J))
370 CONTINUE
WRITE(6,1054) (ICONL(I,J), J=1,JEND)
1054 FORMAT(1H ,5X,'INTERNAL NODE NUMBERS: ',25(I3,'-')/1H ,
120X,25(I3,'-'))
360 CONTINUE
CALL SLINE(36)
IF(IVERSN ,EQ. 1) GO TO 20
READ(5,1015) NSEGMT,(NMHNPB(I), I=1,NSEGMT)

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1013 FORMAT(20I4)
      WRITE(6,1046) NSEGMT
1046 FORMAT(1H0,10X,'MODEL VERSION 2 CHOSEN. THE ADDITIONAL',
1 ' BOUNDARY INFORMATION IS: '/1H0,15X,
2 ' NUMBER OF BOUNDARY SEGMENTS, NSEGMT = ',15/1H0)
      IF(NSEGMT .EQ. 0) GO TO 40
      DO 30 I=1,NSEGMT
        J1=NMHNPB(I)
        READ(5,1013)(ICONB(I,J), J=1,J1)
        WRITE(6,1048)I,NMHNPB(I),(ICONB(I,J), J=1,J1)
1048 FORMAT(1H ,5X,'SEGMENT ',I3,', NUMBER OF NODES, NMHNPB = ',
1 I3,', EXTERNAL NODE NUMBERS: ',15('-',I3))
        DO 140 J=1,J1
          ICONB(I,J)=NINT(ICONB(I,J))
140 CONTINUE
        WRITE(6,1050) (ICONB(I,J), J=1,J1)
1050 FORMAT(1H ,51X,'INTERNAL NODE NUMBERS: ',15('-',I3)/1H0)
30 CONTINUE
40 CALL SLINE(36)
20 ITIME=0
      IF(INPUTH .EQ. 1) GO TO 230
      CALL READX(ETA,NMNP)
      WRITE(6,1042)
1042 FORMAT(1H0,'INITIAL VALUES OF SURFACE ELEVATIONS ARE: '/1H )
      WRITE(6,1040)(ETA(I), I=1,NMNP)
1040 FORMAT(1H ,10(2X,E10.3))
      CALL SLINE(40)
230 IF(INPUTQ .EQ. 1) GO TO 270
      CALL READX(QPREV,NMNP2)
      DO 190 I=1,NMNP2
        Q(I)=QPREV(I)
190 CONTINUE
      IF(NMQBN .GT. 0) CALL LCCGLO(QBANG,NQN,QPREV,1.)
      WRITE(6,1044)
1044 FORMAT(1H0,10X,'INITIAL VALUES OF THE FLUXES ARE: '/1H )
      WRITE(6,1040)(Q(I), I=1,NMNP2)
      CALL SLINE(40)
270 TIME=TIME+TINC2
240 CALL STETAB(ETA,PB,NHN,ALAG)
      CALL CETA(H,DEPTH,ETA)
      CALL INTIME(ETA,ETAPRV,NMNP,SYSFQ,NMNP2)
      ITIME =ITIME+1
      TIME=TIME+TINC2
      CALL FORCEQ(H,Q,TAUWX,TAUWY,DEPTH,AREA,CF,EDXX,EDYY,EDXY,ICON,
1 A,B,PSPLUS,SYSFQ,ETA)
      IF(IVERSN .EQ. 1) GO TO 250
      CALL BCUNDF(SYSFQ,XORD,YORD,NMLB,NMLBN,ICONL,ETA,DEPTH,MAXL)
      IF(NSEGMT .EQ. 0) GO TO 250
      CALL BOUNDF(SYSFQ,XORD,YORD,NSEGMT,NMHNPB,ICONB,ETA,DEPTH,MAXD)
250 IF(NMQBN .GT. 0) CALL LCCGLO(QBANG,NQN,SYSFQ,1.)
      CALL SOLVX(Q,SYSFQ,QPREV,SYSMQ,NMNP2,NBANDQ,MAXMQ,MAXBWQ)
      IF(NMQBN .EQ. 0) GO TO 180
      CALL STOB(Q,QB,NQN,NVN)
180 DO 150 I=1,NMNP2
        QPREV(I)=Q(I)
150 CONTINUE
      IF(NMQBN .GT. 0) CALL LCCGLO(QBANG,NQN,Q,-1.)
      CALL OUTPUT(H,Q,ETA,U,V,DEPTH,ISTAT)
      GO TO (330,300),ISTAT
260 IF(IDT.NE.1) CALL VARTIM(TIME)

```

```

TINC2=TINC/2.
TIME=TIME+TINC2
CALL AMATZR(SYSFH,NMNP)
CALL FORCEH(Q,SYSFH,A,B,ICON)
IF(NMHBN .EQ. 0) GO TO 160
CALL STETAB(ETA,HB,NHN,ALAG)
CALL SUBOON(NMHBN,NHN,NBANDH,SYSFH,SYSBMH,ETA,ETAPRV,NMNP,
1 MAXHBM,MAXBWH,1)
C CALL NOFLOW(Q,QB,QBANG,XORD,YORD,SYSFH,NMLBN,ICONL,NMLB,
C 1 NMNP,NMNP2,NMQBN,MAXL)
160 CALL SOLVX(ETA,SYSFH,ETAPRV,SYSMH,NMNP,NBANDH,MAXNOD,MAXBWH)
GO TO 240
300 CALL STETAB(ETA,HB,NHN,ALAG)
CALL CETA(H,DEPTH,ETA)
CALL VOLUME(ETA,AREA,ICCN,VOL)
WRITE(6,1034) TIME,TINC,ITIME,VOL
1034 FORMAT(1H0,10X,'TIME = ',F12.2,' SEC',5X,'DELTA T WAS, TINC = ',
1 F8.2,1X,'SEC.',5X,'TIME STEP, ITIME = ',I5/1H0,10X,
2 'NET VOLUME ABOVE MLW, VOL = ',E13.6)
CALL SLINE(15)
WRITE(6,1021)
1021 FORMAT(1H0,'INTERNAL',2X,'EXTERNAL',6X,'H',11X,'QX',10X,'QY',10X,
1 'ETA',10X,'U',11X,'V',9X,'SYSFH',8X,'SYSFQ',8X,'SYSFQ'/1H ,2X,'N
2 E',6X,'NODE'/1H ,1X,'NUMBER',4X,'NUMBER'/1H )
DO 310 I=1,NMNP
WRITE(6,1032)I,NEXT(I),H(I),Q(2*I-1),Q(2*I),ETA(I),U(I),V(I),
1 SYSFH(I),SYSFQ(2*I-1),SYSFQ(2*I)
1032 FORMAT(1H ,2X,I4,6X,I4,4X,6(F10.5,2X),2(E11.4,2X),E11.4)
310 CONTINUE
CALL SLINE(36)
CALL GRAPH(XORD,YORD,ETA,NMNP)
330 CALL CHECKS(H(N0),DEPTH(N0),BOUND,ICHECK)
IF(ICHECK.EQ.1) GOTO 320
IF(TIME+TINC .GT. ENDTIM+0.001) GO TO 320
IF(IPLDT.EQ.0) GOTO 260
IF(ITIME/IPLDT*IPLDT.NE.ITIME) GOTO 260
CALL PLOTOT(ETA,U,V)
CALL INITCD(ETA,Q)
GO TO 260
END

C
C
SUBROUTINE AMATZR(AMAT,N)
DIMENSION AMAT(N)
DO 10 I=1,N
AMAT(I)=0.
10 CONTINUE
RETURN
END

C
C
SUBROUTINE BAKSUB(NE,INDX1,INDX2,NBAND,B,X)
DIMENSION B(INDX1,INDX2), X(INDX1)
X(NE)=X(NE)/B(NE,1)
NDIF=NBAND-1
DO 10 N=1,NDIF
J=NE-N
J1=J+1
A=0.
DO 20 K=J1,NE

```

```

      KJR=K-J+1
      A=A+B(J,KJR)*X(K)
20  CONTINUE
      X(J)=(X(J)-A)/B(J,1)
10  CONTINUE
      NE1=NE-1
      DO 30 N=NBAND,NE1
      J=NE-N
      J1=J+1
      A=0.
      KT=J+NDIF
      DO 40 K=J1,KT
      KJR=K-J+1
      A=A+B(J,KJR)*X(K)
40  CONTINUE
      X(J)=(X(J)-A)/B(J,1)
30  CONTINUE
      RETURN
      END

```

C  
C

```

      SUBROUTINE BOUNDF(SYSFG,XORD,YORD,NMB,NMN,ICONB,ETA,DEPTH,MAX)
      COMMON/CPROP/GRAVT,CORIO,DENSTY
      COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMO,MAXBWH,
1  MAXBWQ,NMHBN,NMCPN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2  MAXQBM,MAXHBM
      DIMENSION SYSFG(NMNP2),XORD(NMNP),YORD(NMNP),NMN(NMB),
1  ICONB(3,MAX),ETA(NMNP),DEPTH(NMNP)
      DO 10 I=1,NMB
      JEND=NMN(I)
      DO 20 J=2,JEND
      K1=ICONB(I,J-1)
      K2=ICONB(I,J)
      ANX=(YORD(K2)-YORD(K1))*GRAVT/12.
      ANY=(XORD(K1)-XORD(K2))*GRAVT/12.
      DSEGMT=DEPTH(K1)+DEPTH(K2)
      ESEGMT=ETA(K1)+ETA(K2)
      VAR1=DSEGMT*ESEGMT+ESEGMT**2/2.
      VAR2=2.*DFPTH(K1)*ETA(K1)+ETA(K1)**2
      VAR3=2.*DEPTH(K2)*ETA(K2)+ETA(K2)**2
      SYSFG(2*K1-1)=SYSFG(2*K1-1)-ANX*(VAR1+VAR2)
      SYSFG(2*K1)=SYSFG(2*K1)-ANY*(VAR1+VAR2)
      SYSFG(2*K2-1)=SYSFG(2*K2-1)-ANX*(VAR1+VAR3)
      SYSFG(2*K2)=SYSFG(2*K2)-ANY*(VAR1+VAR3)
20  CONTINUE
10  CONTINUE
      RETURN
      END

```

C  
C

```

      SUBROUTINE CETA(H,DEPTH,ETA)
      COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMO,MAXBWH,
1  MAXBWQ,NMHBN,NMCPN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2  MAXQBM,MAXHBM
      DIMENSION H(NMNP),DEPTH(NMNP),ETA(NMNP)
      DO 10 I=1,NMNP
      H(I)=DEPTH(I)+ETA(I)
10  CONTINUE
      RETURN
      END

```



C  
C

```
      SUBROUTINE CHECKS(H,D,BOUND,ICHECK)
      ICHECK=0
      IF(ABS(H-D) .LT. BOUND) RETURN
      ICHECK=1
      WRITE(6,1002) H,D,BOUND
1002  FORMAT(1H0,15X,'STABILITY CHECK: BOUND EXCEEDED AT NODE ',/
1     1H ,5X,'HEIGHT WAS ',E11.4,5X,'DEPTH ',F6.2,5X,'BOUND = ',F6.2)
      RETURN
      END
```

C  
C

```
      SUBROUTINE DECOMP(NEQT,INDX1,INDX2,NBAND,A)
      DIMENSION A(INDX1,INDX2)
      I=1
70     DIAG=A(I,1)
      IF(DIAG .LT. 1.E-30) GO TO 10
      DIAG =SQRT(DIAG)
      DO 20 K=1,NBAND
      A(I,K)=A(I,K)/DIAG
20     CONTINUE
      I=I+1
      IF(I .GT. NEQT) RETURN
      LIM=NBAND-1
      I4=I-LIM
      I3=I-1
50     DO 30 J=1,LIM
      IF(I+J .GT. NEQT+1) GO TO 70
      I2=I4+J-1
      IF(I .LT. NBAND) I2=1
      DO 40 I1=I2,I3
      IF(I-I1+J .GT. NBAND) GO TO 40
      K=I-I1
      A(I,J)= A(I,J)-A(I1,K+1)*A(I1,K+J)
40     CONTINUE
30     CONTINUE
      GO TO 70
10     WRITE(6,1002) I
1002  FORMAT(1H-,5X,'SINGULAR ELEMENT IN ROW',I3)
      STOP
      END
```

C  
C

```
      SUBROUTINE FORCEH(Q,SYSFH,A,B,ICON)
      COMMON/CGRID/NMNP,NMNP2,NEANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1     MAXBWQ,NMHBN,NMQBN,NMVBN,MAXHBN,MAXOBN,MAXEL,NMEL,
2     MAXQBM,MAXHBM
      COMMON/SORTNO/K(3)
      DIMENSION Q(NMNP2),SYSFH(NMNP),ICON(MAXEL,3),A(MAXEL,3),
1     B(MAXEL,3)
      DO 10 I=1,NMEL
      VAR=0.
      DO 20 J=1,3
      K(J)=ICON(I,J)
      VAR=VAR+B(I,J)*Q(2*K(J)-1)+A(I,J)*Q(2*K(J))
20     CONTINUE
      VAR=-VAR/6.
      DO 30 J=1,3
      SYSFH(K(J))=VAR+SYSFH(K(J))
```

```

30 CONTINUE
10 CONTINUE
RETURN
END

```

C  
C

```

SUBROUTINE FORCEQ(H,Q,TAUWX,TAUWY,DEPTH,AREA,CF,EDXX,EDYY,
1 EDXY,ICON,A,B,PSPLUS,SYFQ,ETA)
COMMON/CPROP/GRAVT,CORIO,DENSTY
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWM,
1 MAXBWO,NMHBN,NMQBN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
COMMON/COPT/IBFRIC,IDEPTH,IEDVIS,ICNVEC,IWIND,IVERSN
DIMENSION H(NMNP),Q(NMNP2),TAUWX(NMNP),TAUWY(NMNP),DEPTH(NMNP),
1 AREA(NMEL),CF(NMEL),EDXX(NMEL),EDYY(NMEL),EDXY(NMEL),ICON(MAXEL,
2 3),A(MAXEL,3),B(MAXEL,3),PSPLUS(NMNP),SYFQ(NMNP2),ETA(NMNP),
3 SX(200),SY(200),CXX(200),CYY(200),CXY(200),K(3),
4 SBX(200),SBY(200)
CALL WINDS(TAUWX,TAUWY)
DO 10 I=1,NMNP
SX(I)=TAUWX(I)+CORIO*Q(2*I)
SY(I)=TAUWY(I)-CORIO*Q(2*I-1)
VAR=SQRT(Q(2*I-1)**2+Q(2*I)**2)/H(I)**2
SBX(I)=Q(2*I-1)*VAR
SBY(I)=VAR*Q(2*I)
10 CONTINUE
IF(ICNVEC.EQ.1) GO TO 40
DO 90 I=1,NMNP
CXX(I)=Q(2*I-1)**2/H(I)
CYY(I)=Q(2*I)**2/H(I)
CXY(I)=Q(2*I)*Q(2*I-1)/H(I)
90 CONTINUE
40 DO 20 I=1,NMEL
A12=AREA(I)/12.
ETAEMT=0.
DEMT=0.
BDEMT=0.
EDEMT=0.
E2EMT=0.
FXX=0.
FXY=0.
FYY=0.
DO 30 J=1,3
K(J)=ICON(I,J)
ETAEMT=ETAEMT+ETA(K(J))
DEMT=DEMT+DEPTH(K(J))
BDEMT=BDEMT+B(I,J)*DEPTH(K(J))
EDEMT=EDEMT+ETA(K(J))*DEPTH(K(J))
E2EMT=E2EMT+ETA(K(J))**2
FXX=FXX+Q(2*K(J)-1)*B(I,J)
FXY=FXY+Q(2*K(J)-1)*A(I,J)+Q(2*K(J))*B(I,J)
FYY=FYY+Q(2*K(J))*A(I,J)
30 CONTINUE
PRESS=((DEMT*ETAEMT+EDEMT)**2+ETAEMT**2+E2EMT)*GRAVT/48.
VAR=(B(I,1)*DEPTH(K(1))+B(I,2)*DEPTH(K(2))+B(I,3)*DEPTH(K(3)))
1 *GRAVT/24.
SXX=(SX(K(1))+SX(K(2))+SX(K(3)))*A12
SBXX=-((SBX(K(1))+SBX(K(2))+SBX(K(3)))*A12*CF(I)
CONVEC=0.
DO 50 J=1,3

```

```

    COLL=PRESS*B(I,J)+SXX+SBXX+A12*SX(K(J))-A12*SBX(K(J))*CF(I)
1  +VAR*ETAEMT+VAR*ETA(K(J))-(EDXX(I)*FXX*B(I,J)
2  +EDYY(I)*FYY*A(I,J))/AREA(I)/4.
    SYSFQ(2*K(J)-1)=SYSFQ(2*K(J)-1)+COLL
    IF(ICNVEC .EQ. 1) GO TO 50
    CONVEC=CONVEC+B(I,J)*CXX(K(J))+A(I,J)*CXY(K(J))
50  CONTINUE
    IF(ICNVEC .EQ. 1) GO TO 60
    CONVEC=-CONVEC/6.
    DO 70 J=1,3
    SYSFQ(2*K(J)-1)=SYSFQ(2*K(J)-1)+CONVEC
70  CONTINUE
60  CONVEC=0.
    VAR=(A(I,1)*DEPTH(K(1))+A(I,2)*DEPTH(K(2))+A(I,3)*DEPTH(K(3)))
1  *GRAVT/24.
    SXX=(SY(K(1))+SY(K(2))+SY(K(3)))*A12
    SBXX=-(SBY(K(1))+SBY(K(2))+SBY(K(3)))*A12*CF(I)
    DO 80 J=1,3
    COLL=PRESS*A(I,J)+SXX+SBXX+A12*SY(K(J))-A12*SBY(K(J))*CF(I)
1  +VAR*ETAEMT+VAR*ETA(K(J))-(EDXY(I)*FXY*B(I,J)
2  +EDYY(I)*FYY*A(I,J))/AREA(I)/4.
    SYSFQ(2*K(J))=SYSFQ(2*K(J))+COLL
    IF(ICNVEC .EQ. 1) GO TO 80
    CONVEC=CONVEC+B(I,J)*CXY(K(J))+A(I,J)*CYY(K(J))
80  CONTINUE
    IF(ICNVEC .EQ. 1) GO TO 20
    CONVEC=-CONVEC/6.
    DO 100 J=1,3
    SYSFQ(2*K(J))=SYSFQ(2*K(J))+CONVEC
100 CONTINUE
20  CONTINUE
    RETURN
    END

```

C  
C

```

SUBROUTINE FORSUB(NE,INDX1,INDX2,NBAND,B,C)
DIMENSION B(INDX1,INDX2),C(INDX1)
C(1)=C(1)/B(1,1)
DO 10 J=2,NBAND
A=0.
J1=J-1
DO 20 L=1,J1
LJR=J-L+1
A=A+B(L,LJR)*C(L)
20 CONTINUE
C(J)=(C(J)-A)/B(J,1)
10 CONTINUE
NDIF=NBAND-1
NN=NBAND+1
DO 30 J=NN,NE
A=0.
J1=J-1
LT=J-NDIF
DO 40 L=LT,J1
LJR=J-L+1
A=A+B(L,LJR)*C(L)
40 CONTINUE
C(J)=(C(J)-A)/B(J,1)
30 CONTINUE
RETURN

```

```

END
C
C
SUBROUTINE GEOM(NINT, ICON, A, B, AREA, XORD, YORD, SYSMH)
COMMON/CGRID/NMNP, NMNP2, NBAND1, NBAND0, MAXNOD, MAXMQ, MAXBWH,
1 MAXBWQ, NMHBN, NMQB, NMVBN, MAXHBN, MAXQB, MAXEL, NMEL,
2 MAXQBM, MAXHBM
COMMON/SORTNO/K(3)
DIMENSION NINT(NMNP), ICON(MAXEL,3), A(MAXEL,3), B(MAXEL,3),
1 AREA(NMEL), SYSMH(MAXNOD,MAXBWH), XORD(NMNP), YORD(NMNP)
DIMENSION IPERM(3,2)
DATA IPERM/3,1,2,2,3,1/
WRITE(6,1002)
1002 FORMAT(1H0,5X,'GEOMETRICAL RELATIONS'/1H ,10X,
2 'ELEMENT'/1H ,10X,'NUMBER',4X,'A1',8X,'B1',
3 8X,'A2',8X,'B2',8X,'A3',8X,'B3',8X,'AREA'/1H )
DO 10 I=1,NMEL
DO 20 J=1,3
K(J)=NINT(ICON(I,J))
ICON(I,J)=K(J)
20 CONTINUE
DO 40 J=1,3
A(I,J)=XORD(K(IPERM(J,1)))-XORD(K(IPERM(J,2)))
B(I,J)=YORD(K(IPERM(J,2)))-YORD(K(IPERM(J,1)))
40 CONTINUE
AREA(I)=0.5*(B(I,1)*A(I,2)-B(I,2)*A(I,1))
IF(AREA(I) .GT. 0.) GO TO 30
WRITE(6,1004) I
1004 FORMAT(1H0,5X,'NEGATIVE AREA IN ELEMENT:',I4)
STOP
30 VAR=APEA(I)/12.
CALL SCRTN
K1=K(1)
K2=K(2)
K3=K(3)
SYSMH(K1,1)=SYSMH(K1,1)+2.*VAR
K21=K2-K1+1
SYSMH(K1,K21)=SYSMH(K1,K21)+VAR
K31=K3-K1+1
SYSMH(K1,K31)=SYSMH(K1,K31)+VAR
SYSMH(K2,1)=SYSMH(K2,1)+2.*VAR
K32=K3-K2+1
SYSMH(K2,K32)=SYSMH(K2,K32)+VAR
SYSMH(K3,1)=SYSMH(K3,1)+2.*VAR
WRITE(6,1006)I,A(I,1),B(I,1),A(I,2),B(I,2),A(I,3),B(I,3),AREA(I)
1006 FORMAT(1H ,5X,I6,2X,6(F9.1,1X),F12.1)
IF(K31 .LT. NBANDH) GO TO 10
NBANDH=K31
10 CONTINUE
WRITE(6,1010) NBANDH
1010 FORMAT(1H0,10X,'BANDWIDTH OF THIS GRID IS, NBANDH = ',I4)
CALL SLINE(36)
IF(NBANDH .LE. MAXBWH) RETURN
WRITE(6,1008) NBANDH
1008 FORMAT(1H0,5X,'BANDWIDTH IS TOO LARGE, NBANDH = ',I5)
STOP
END
C
C
SUBROUTINE GRAPH(XORD,YORD,ETA,NMNP)

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```

DIMENSION XORD(1),YORD(1),ETA(1)
RETURN
END
C
C
SUBROUTINE INITCD(ETA,Q)
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMNB,NMQBN,NMVB,NMAXHBN,MAXQBN,MAXEL,NMEL,MAXQBM
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
DIMENSION ETA(NMNP),Q(NMNP2)
OPEN(UNIT=9,DEVICE='DSK',ACCESS='SEQOUT',FILE='INCONO')
WRITE(9,200) TIME
WRITE(9,200) (ETA(I),I=1,NMNP)
WRITE(9,200) (Q(I),I=1,NMNP2)
200 FORMAT(8F10.4)
RETURN
END
C
C
SUBROUTINE INTIME(ETA,ETAPRV,NMNP,SYSFQ,NMNP2)
DIMENSION ETA(NMNP),ETAPRV(NMNP),SYSFQ(NMNP2)
DO 10 I=1,NMNP
ETAPRV(I)=ETA(I)
SYSFQ(I)=0.
10 CONTINUE
I1=NMNP+1
DO 20 I=I1,NMNP2
SYSFQ(I)=0.
20 CONTINUE
RETURN
END
C
C
SUBROUTINE LCCGLO(QBANG,NGN,Q,GLTOLC)
COMMON/ANGLE/S,C
COMMON/CGRID/NMNP,NMNP2,NBANDF,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMHBN,NMQBN,NMVB,NMAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
DIMENSION QBANG(MAXNOD),Q(NMNP2),NGN(MAXNOD)
DO 10 I=1,NMQBN
I1=NGN(I)
ANG=GLTOLC*QBANG(I)
CALL TRIGO(ANG)
CALL ROTV(Q(2*I1-1),Q(2*I1))
10 CONTINUE
RETURN
END
C
C
SUBROUTINE OUTPUT(H,Q,ETA,U,V,DEPTH,ISTAT)
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
COMMON/COUPT/NDOUT
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMHBN,NMQBN,NMVB,NMAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
DIMENSION H(NMNP),Q(NMNP2),ETA(NMNP),U(NMNP),V(NMNP),DEPTH(NMNP)
ISTAT=1
IF((ITIME/NDOUT*NCUT .EQ. ITIME) GO TO 10
IF(ITIME .EQ. 2) GO TO 10
IF(ITIME .EQ. 5) GO TO 10

```

```

RETURN
10 CALL CETA(H,DEPTH,ETA)
CALL VEL(H,Q,U,V)
ISTAT=2
RETURN
END

C
C
SUBROUTINE PLOTOT(ETA,U,V)
DIMENSION ETA(NMNP),U(NMNP),V(NMNP)
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMNB,NMQBN,NMVB,NMAXHBN,MAXQBN,MAXEL,NMEL,MAXQBM,MAXHBM
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
WRITE(8,200) TIME
WRITE(8,200) (ETA(I),I=1,NMNP)
WRITE(8,200) (U(I),V(I),I=1,NMNP)
200 FORMAT(8F10.4)
RETURN
END

C
C
SUBROUTINE POSTTT(A,B,C,D)
COMMON/ANGLE/SIN,COS
A1=A*COS+B*SIN
B=-A*SIN+B*COS
C1=C*COS+D*SIN
D=-C*SIN+D*COS
A=A1
C=C1
RETURN
END

C
C
SUBROUTINE PRET(A,B,C,D)
COMMON/ANGLE/SIN,COS
A1=A*COS+C*SIN
B1=B*COS+D*SIN
C=-A*SIN+C*COS
D=-B*SIN+D*COS
A=A1
B=B1
RETURN
END

C
C
SUBROUTINE QMAT(SYSMQ,YSMH)
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMHBN,NMQBN,NMVB,NMAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
DIMENSION SYSMQ(MAXMQ,MAXBWQ),YSMH(MAXNOD,MAXBWH)
NDIF=NMNP-NBANDH+1
DO 10 IR=1,NDIF
DO 20 IC=1,NBANDH
SYSMQ(2*IR-1,2*IC-1)=YSMH(IR,IC)
SYSMQ(2*IR,2*IC-1)=YSMH(IR,IC)
20 CONTINUE
10 CONTINUE
NI=NDIF+1
J1=0
DO 30 IR=NI,NMNP

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      J1=J1+1
      LIM=NBANDH-J1
      DO 40 IC=1,LIM
      SYSMQ(2*IR-1,2*IC-1)=SYSMH(IR,IC)
      SYSMQ(2*IR,2*IC-1)=SYSMH(IR,IC)
40    CONTINUE
30    CONTINUE
      RETURN
      END

C
C
      SUBROUTINE READX(X,NMN)
      DIMENSION X(NMN)
      READ(4,1001)X
1001  FORMAT(8F10.0)
      RETURN
      END

C
C
      SUBROUTINE ROTMQ(SYSMQ,NQN,QBANG)
      COMMON/ANGLE/S,C
      COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1     MAXBWQ,NMHN,NMQBN,NMVB,NMAXHN,MAXQBN,MAXEL,NMEL,
2     MAXQBM,MAXHBM
      DIMENSION SYSMQ(MAXMQ,MAXBWO),NQN(NMQBN),QBANG(NMQBN)
      LIM1=NBANDQ/2-1
      DO 10 I=1,NMQBN
      IR=NQN(I)
      LIM=LIM1
      IF(IR.LE. LIM) GO TO 20
      CALL TRIGO(QBANG(I))
40    DO 30 IC=1,LIM
      IR1=IR-IC
      CALL POSTTT(SYSMQ(2*IR1-1,2*IC+1),SYSMQ(2*IR1-1,2*IC+2),
1     SYSMQ(2*IR1,2*IC),SYSMQ(2*IR1,2*IC+1))
30    CONTINUE
      GO TO 10
20    IF(IR.EQ. 1) GO TO 10
      CALL TRIGO(QBANG(I))
      LIM=IR-1
      GO TO 40
10    CONTINUE
      NDIF=(NMNP2-NBANDQ+2)/2
      LIM1=NBANDQ/2
      DO 50 I=1,NMQBN
      IR=NQN(I)
      LIM=LIM1
      IF(IR.GT. NDIF) GO TO 60
      CALL TRIGO(QBANG(I))
80    DO 70 IC=2,LIM
      CALL PPET(SYSMQ(2*IR-1,2*IC-1),SYSMQ(2*IR-1,2*IC),
1     SYSMQ(2*IR,2*IC-2),SYSMQ(2*IR,2*IC-1))
70    CONTINUE
      GO TO 50
60    IF(IR.EQ. NMNP) GO TO 50
      CALL TRIGO(QBANG(I))
      LIM=LIM-(IR-NDIF)
      GO TO 80
50    CONTINUE
      RETURN

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      END
C
C
      SUBROUTINE ROTV(A,B)
      COMMON/ANGLE/ S,C
      X=A*C+B*S
      B=-A*S+B*C
      A=X
      RETURN
      END
C
C
      SUBROUTINE SLINE(N)
      DATA STAR/3H* */
      WRITE(6,1002)(STAR,I=1,N)
1002  FORMAT(1H0,5X,42A3)
      RETURN
      END
C
C
      SUBROUTINE SOLVX(X,SYSFX,XPREV,SYSMX,NMN,NBAND,INDEX1,INDEX2)
      COMMON/CGRID/NMNP,NMNP2,NBANDF,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1    MAXBWQ,NMHBN,NMQBN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2    MAXQBM,MAXHBM
      COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,[STEP,PHASE,ITIME
      DIMENSION X(NMN),SYSFX(NMN),SYSMX(INDEX1,INDEX2),
1    XPREV(NMN)
      DO 10 I=1,NMN
      X(I)=TINC*SYSFX(I)
10    CONTINUE
      CALL FCRSUB(NMN,INDEX1,INDEX2,NBAND,SYSMX,X)
      CALL BAKSUB(NMN,INDEX1,INDEX2,NBAND,SYSMX,X)
      DO 20 I=1,NMN
      X(I)=X(I)+XPREV(I)
20    CONTINUE
      RETURN
      END
C
C
      SUBROUTINE SORTN
      COMMON/SORTNO/K1,K2,K3
      IF(K1 .LT. K3) GO TO 10
      K=K3
      K3=K1
      K1=K
10    IF(K2 .LT. K3) GO TO 20
      K=K3
      K3=K2
      K2=K
20    IF(K1 .LT. K2) RETURN
      K=K2
      K2=K1
      K1=K
      RETURN
      END
C
C
      SUBROUTINE STETAB(ETA,HB,NHN,ALAG)
      COMMON/CGRID/NMNP,NMNP2,NBANDF,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1    MAXBWQ,NMHBN,NMQBN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,

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2  MAXQBM,MAXHBM
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
DIMENSION ETA(NMNP),HB(NMHBN),NHN(NMHBN),ALAG(NMHBN)
DO 10 I=1,NMHBN
  II=NHN(I)
  ETA(I)=HB(I)*SIN(PHASE*(TIME-ALAG(I)))
10 CONTINUE
RETURN
END

C
C
SUBROUTINE STORNO(INDX1,INDX2,INDX3,INDX4,NBAND,NMBN,NN,A,NB,C,
1  NV)
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
DIMENSION A(INDX1,INDX2),C(INDX3,INDX4),NB(NMBN)
NDIF=NBAND-1
DO 10 I=1,NMBN
  IR=NB(I)
  IR1=NN*(IR-1)+1+NV
  IF(IR1.EQ.1) GO TO 20
  IF(IR1.LE.NDIF) NDIF=NN*(IR-1)+NV
  DO 30 IC=1,NDIF
    IR2=IR1-IC
    C(2*I-1,IC)=-A(IR2,IC+1)/TINC
    A(IR2,IC+1)=0.
30 CONTINUE
  NDIF=NBAND-1
20 DO 40 IC=1,NDIF
  C(2*I,IC)=-A(IR1,IC+1)/TINC
  A(IR1,IC+1)=0.
40 CONTINUE
  A(IR1,1)=1.
10 CONTINUE
RETURN
END

C
C
SUBROUTINE STQB(Q,QB,NQN,NVN)
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXRWQ,NMHBN,NMGEN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
DIMENSION Q(NMNP2),QB(MAXNOD),NQN(MAXNOD),NVN(MAXNOD)
DO 10 I=1,NMQBN
  II=2*NQN(I)-1
  IF(II.LE.0) II=1
  Q(II)=QB(I)
10 CONTINUE
  IF(NMVBN.EQ.0) GO TO 30
  DO 20 I=1,NMVBN
  II=2*NVN(I)
  Q(II)=0.
20 CONTINUE
30 CONTINUE
RETURN
END

C
C
SUBROUTINE SUBOUN(NMBN,NN,NBAND,SYSFXB,SYSBMX,X,XPREV,
1  NEQT,INDX1,INDX2,NV)

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DIMENSION SYSBMX(INCX1,INDX2),NN(NMBN),SYSFXB(NEQT),X(NEQT),
1 XPREV(NEQT)
DO 10 I=1,NMBN
II=NV*AN(I)-1
IF(II.LT.1)II=1
NDIF=NBAND-1
IF(II.LE.1)GO TO 20
IF(II.LT.NBAND) NDIF=II-1
DO 30 J=1,NDIF
SYSFXB(II-J)=SYSFXB(II-J)+SYSBMX(2*I-1,J)*(X(II)-XPREV(II))
30 CONTINUE
NDIF=NBAND-1
20 IF(II+NDIF.GT.NEQT) NDIF=NEQT-II
DO 40 J=1,NDIF
SYSFXB(II+J)=SYSFXB(II+J)+SYSBMX(2*I,J)*(X(II)-XPREV(II))
40 CONTINUE
10 CONTINUE
RETURN
END

C
C
SUBROUTINE TIMEN
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
TIME=TIME+TINC
RETURN
END

C
C
SUBROUTINE TRIGO(A)
COMMON/ANGLE/S,C
S=SIN(A)
C=COS(A)
RETURN
END

C
C
SUBROUTINE VART(M(TIME))
RETURN
END

C
C
SUBROUTINE VEL(H,Q,U,V)
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMHBN,NMQBN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
DIMENSION H(NMNP),Q(NMNP2),U(NMNP),V(NMNP)
DO 10 I=1,NMNP
U(I)=Q(2*I-1)/H(I)
V(I)=Q(2*I)/H(I)
10 CONTINUE
RETURN
END

C
C
SUBROUTINE VOLUME(ETA,AREA,ICON,VOL)
COMMON/CGRID/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXBWQ,NMHBN,NMQBN,NMVBN,MAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
DIMENSION ETA(NMNP),AREA(NMEL),ICON(MAXEL,3)
VOL=0.

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DO 10 I=1,NMEL
SUM=0.
DO 20 J=1,3
SUM=SUM+ETA((ICON(I,J))
20 CONTINUE
VOL=AREA(I)*SUM/3.+VOL
10 CONTINUE
RETURN
END
C
C
SUBROUTINE WINDS(TAUWX,TAUWY)
COMMON/CINTEG/TIME,TINC,RKFACT,RKFAC,ISTEP,PHASE,ITIME
COMMON/COPT/IBFRIC,IDEPTH,IEDVIS,ICNVEC,IWIND,IVERSN
COMMON/CGR[D/NMNP,NMNP2,NBANDH,NBANDQ,MAXNOD,MAXMQ,MAXBWH,
1 MAXRWQ,NMHBN,NMQBN,NMVBH,MAXHBN,MAXQBN,MAXEL,NMEL,
2 MAXQBM,MAXHBM
DIMENSION TAUWX(NMNP),TAUWY(NMNP)
IF(IWIND.EQ.4) RETURN
DO 10 I=1,NMNP
TAUWX(I)=0.
TAUWY(I)=0.
10 CONTINUE
IWIND=4
RETURN
END
C
C

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