

1 **Technical and environmental evaluation of an integrated scheme for the co-treatment of wastewater**
2 **and domestic organic waste in small communities**

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12 **Abstract**

13 A technical and environmental evaluation of an innovative scheme for the co-treatment of domestic
14 wastewater and domestic organic waste (DOW) was undertaken by coupling an upflow anaerobic sludge
15 blanket (UASB), a sequencing batch reactor (SBR) and a fermentation reactor. Alternative treatment
16 configurations were evaluated with different waste collection practices as well as various schemes for
17 nitrogen and phosphorus removal. All treatment systems fulfilled the required quality of the treated
18 effluent in terms of chemical oxygen demand (COD) and total suspended solids (TSS) concentrations.
19 However, only the configurations performing the short-cut nitrification/denitrification with biological
20 phosphorus removal met the specifications for water reuse. The environmental assessment included the
21 analysis of impacts on climate change (CC), freshwater eutrophication (FE) and marine eutrophication
22 (ME). A functional unit (FU) of 2,000 people receiving treatment services was considered. The most
23 relevant sources of environmental impacts were associated to the concentration of dissolved methane
24 in the UASB effluent that is emitted to the atmosphere in the SBR process (accounting for 37% of
25 impacts in CC), electricity consumption, mainly for aeration in the SBR (representing 13% of the impacts
26 produced in CC), and the discharge of the treated effluent in receiving waters (contributing 98% and

27 57% of impacts in FE and ME, respectively). The scheme of separate waste collection together with
28 biological nitrogen removal and phosphorus uptake via nitrite was identified as the best configuration,
29 with good treated effluent quality and environmental impacts lower than those of the other examined
30 configurations.

31 **Keywords**

32 Decentralised treatment processes; treated effluent reuse; environmental profile; nutrient removal;
33 resource recovery; wastewater and domestic organic waste

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Abbreviation	Description
AOB	Ammonium oxidising bacteria
BNR	Biological nutrient removal
BOD ₅	Five-day biochemical oxygen demand
CC	Climate change
COD	Chemical oxygen demand
DNBPR	Denitrifying via nitrite biological phosphorus removal
DO	Dissolved oxygen
DOW	Domestic organic waste
DPAO	Denitrifying phosphorus accumulating organism
EBPR	Enhanced biological phosphorus removal
FE	Freshwater eutrophication
FWD	Food waste disposer
GHG	Greenhouse gas
HRT	Hydraulic retention time
LCA	Life cycle assessment
ME	Marine eutrophication
ND	Nitrification/denitrification
NOB	Nitrite oxidising bacteria
OLR	Organic loading rate
PAO	Phosphorus accumulating organism
PE	Population equivalent
SBR	Sequencing batch reactor
scND	Short-cut nitrification/denitrification
sNUR	Specific nitrogen uptake rate
sPUR	Specific phosphorus uptake rate
SRT	Solids retention time
TN	Total nitrogen
TP	Total phosphorus
TS	Total solids
TSS	Total suspended solids
UASB	Upflow anaerobic sludge blanket
VFA	Volatile fatty acid
vNLR	Volumetric nitrogen loading rate
vPLR	Volumetric phosphorus loading rate
VS	Volatile solids
WWTPs	Wastewater treatment plants

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40 1. Introduction

41 Centralised wastewater treatment may not be feasible or the most cost-effective option for all sites. For
42 instance, due to geographical conditions and dispersed settlements, more than 9,000 wastewater
43 treatment plants (WWTPs) in Italy are designed for 2,000 population equivalent (PE) or lower (Libralato
44 et al., 2012). The European legislation on urban wastewater treatment defines discharge limits for
45 biochemical oxygen demand (BOD₅), chemical oxygen demand (COD) and total suspended solids (TSS)
46 for WWTPs serving PE higher than 2,000, while for lower agglomerations, it only states that appropriate
47 treatment must be implemented (EEC, 1991). Moreover, when it comes to nutrient concentrations,
48 limitations for total phosphorus (TP) and nitrogen (TN) are only specified for **treated** effluents from
49 facilities with a treatment capacity larger than 10,000 PE discharging into sensitive recipients. The
50 option of reusing the treated water from small scale WWTPs in agriculture is interesting, provided that
51 the treated effluent is available near the potential points of use, thus, decreasing the costs of reclaimed
52 water distribution systems (Hophmayer-Tokich, 2000). Currently, there is no European Union legislation
53 concerning the use of reclaimed water; therefore, countries **should** apply national or regional
54 regulations (Norton-Brandão et al., 2013).

55 Considering the requirements imposed for the treated effluent, the applied treatment process should
56 accomplish a number of objectives: relatively low capital and operating expenses, reduced energy
57 consumption and **enhanced** reuse potential of water and other valuable by-products, such as biogas. In
58 this context, the application of anaerobic processes, i.e. upflow anaerobic sludge blanket (UASB),
59 appears as a robust and attractive technology, particularly for hot **climates** (Latif et al., 2011). Compared
60 to aerobic treatment, the UASB process has several advantages, such as low operating expenses, high
61 efficiency, simplicity, flexibility, low requirements of space, energy and chemicals as well as reduced
62 sludge production (Latif et al., 2011). **However**, there are still some barriers that limit the use of
63 anaerobic processes, including the process instability at temperatures below 20°C, low pathogen
64 removal, negligible nutrient removal, odours, long start-ups and the need for adequate post-treatment
65 (Latif et al., 2011). In addition, it is important to consider the concentration of dissolved methane in the

66 anaerobic effluent since low temperature raises methane solubility, which promotes its release into the
67 environment (Cookney et al., 2016, 2012; Matsuura et al., 2015).

68 Biological nutrient removal (BNR) from the low strength anaerobic effluent can be applied as a polishing
69 step (Frison et al., 2013b; Malamis et al., 2013). Biological nitrogen removal via nitrite has several
70 benefits compared to conventional nitrification/denitrification such as 25% of oxygen savings during
71 nitrification and 40% less need for organic carbon source during heterotrophic denitrification (Galí et al.,
72 2007). Enhanced biological phosphorus removal (EBPR) can be performed using nitrite as electron
73 acceptors (Katsou et al., 2015). Denitrifying via nitrite biological phosphorus removal (DNBPR) offers the
74 possibility of integrating phosphorus and nitrogen removal in a robust process. In the presence of nitrite
75 and lack of oxygen, nitrite is denitrified to gaseous nitrogen and simultaneously, phosphate is taken by
76 denitrifying phosphorus accumulating organisms (DPAOs) (Peng et al., 2011). DPAOs are able to
77 accumulate significant amounts of polyphosphate under anoxic conditions, similarly to the phosphorus
78 accumulating organisms (PAOs) in the conventional EBPR process.

79 Due to the substantial organic matter removal attained in the anaerobic treatment, the addition of an
80 external carbon source is required in the subsequent aerobic process for effective BNR (Frison et al.,
81 2013a). The latter opens up the possibility of integrating the management of domestic organic waste
82 (DOW) with sewage. The use of organic waste (i.e. fermented liquids) from households as external
83 carbon source achieves satisfactory rates of denitrification and phosphorus accumulation, while
84 decreasing operational costs (Frison et al., 2013a). Food waste disposers (FWDs) are being promoted as
85 an alternative practice for the collection of DOW (Iacovidou et al., 2012). Specifically, the
86 implementation of FWDs entails reduced transport requirements and odours when compared to the
87 conventional collection (Battistoni et al., 2007; Bernstad et al., 2013). However, the environmental
88 assessment of FWD use is required, with specific focus on energy demand, water consumption and
89 increased organic loads in the WWTP (Battistoni et al., 2007; Marashlian and El-Fadel, 2005).

90 **Several works** on the environmental performance of **WWTPs have been published** following the life
91 cycle assessment (LCA) approach. Rodriguez-Garcia et al. (2011) assessed the environmental impact of
92 24 WWTPs, classifying them in six different typologies by the quality requirements according to their

93 final use or discharge point. Besides, LCA has also been applied for the environmental assessment of
94 integrated processes for waste and wastewater management. Weichgrebe et al. (2008) compared, in
95 terms of energy and environmental impact, the conventional, separate treatment of wastewater and
96 organic waste with their combined treatment by psychrophilic anaerobic digestion and aerobic post-
97 treatment, which showed greenhouse gas (GHG) savings compared to conventional wastewater
98 treatment. Nakakubo et al. (2012) compared different technologies for the disposal of sewage sludge
99 and food waste in order to identify the best option regarding the reduction of GHG emissions. Similarly,
100 Righi et al. (2013) analysed the environmental profile of a decentralised scheme for the management of
101 sewage sludge and biodegradable municipal solid waste (MSW). However, no LCA study has been
102 conducted on the assessment of the environmental performance of anaerobic – aerobic processes for
103 domestic sewage and DOW at community level.

104 This work evaluates the feasibility of an integrated system designed for the decentralised co-
105 management of wastewater and DOW in a small community of 2,000 PE. Various scenarios were
106 evaluated including (i) alternatives in the collection of DOW regarding the integration rates of FWDs
107 within the community, (ii) different nitrogen removal processes and (iii) the potential of including
108 phosphorus removal in the treatment scheme.

109 2. Materials and methods

110 2.1. Integrated treatment system: UASB – SBR configuration

111 The selection of the treatment configuration was based on the results of a pilot scale UASB-SBR system
112 operating at the premises of the University of Verona, taking into consideration cost criteria, legislative
113 aspects for the treated effluent and DOW management in Italy, as well as the characteristics of the small
114 community in terms of waste collection and sewage management. The treatment scheme included: (i)
115 an UASB reactor to treat sewage and produce biogas (ii) a fermentation process to produce volatile fatty
116 acids (VFAs) from DOW, a sequencing batch reactor (SBR) to remove nutrients from the UASB effluent
117 and produce reusable water (iii) composting to treat the excess sludge and convert it to compost to be
118 applied as soil conditioner. Nutrient removal in small scale wastewater treatment systems is not

119 required by European Legislation; however, depending on the relevant National and or Regional Law of
120 countries, compliance to specific nitrogen and phosphorus limits of the treated effluent before
121 discharge to specific water recipients. Mass balances were developed for the whole treatment scheme
122 for total solids (TS), volatile solids (VS), COD and nutrients (TN and TP) to model the different streams of
123 the treatment system. The flowchart of the baseline treatment scheme is shown in Figure 1.

124 Figure 1 around here

125 **UASB process**

126 The average **sewage** flow was 400 m³/d, assuming a production of 200 L wastewater/capita d for a
127 community population of 2000 people. The production rates of COD, N and P were taken as 120 g
128 COD/capita d, 12 g N/capita d and 1.8 g P/capita d, respectively. The UASB reactor operated **at ambient**
129 **temperature (22±2°C)**, at an average organic loading rate (OLR) of 1.4-2.1 kgCOD/m³_{reactor} d, a hydraulic
130 retention time (HRT) of 8 h and an upflow velocity of 1 m/s. According to **the** experimental results, the
131 UASB **produced** 7.2-13.2 L/d **of biogas** with a methane content ranging between 60-65%, which
132 corresponded to an average experimental methane yield of 0.26 m³CH₄/kg COD_{removed}. **This** value is
133 lower compared to the theoretical value of methane yield of 0.35 m³ CH₄/kg COD_{removed} since it does not
134 include the dissolved methane present in the UASB effluent which is not recovered. For calculation
135 purposes, global removal efficiencies of 77% and 70% were considered for COD and TSS respectively,
136 **assuming** that 1 kg of COD degraded produced 0.26 m³ of methane. The dissolved methane derived
137 from the operation of the UASB at moderate temperature was also taken into account in terms of its
138 environmental impact, by considering concentrations **of** 20 mg CH₄/L under supersaturation conditions
139 (Souza et al., 2011). **The** biogas produced in the UASB was treated in a biotrickling filter to remove
140 hydrogen sulphide. The biogas was burnt in a boiler for heat production, appropriate for small and
141 decentralised systems. The boiler had a thermal efficiency of 90% and 10% of losses.

142 **Fermentation process**

143 Considering that the UASB effluent had a very low COD/N ratio (2.5 kg COD/kg N), fermentation of DOW
144 was applied to produce VFAs, which would then be fed to the SBR to promote nutrient removal from the

145 UASB effluent. Furthermore, the surplus of fermented DOW was sent to the UASB in order to increase
146 the OLR and, thus, biogas production. A production rate of 0.30 kg DOW/capita d was considered
147 (Bolzonella et al., 2003). Assuming a collection efficiency of 83%, 500 kg of DOW are separately collected
148 at household level and transported to the treatment facility on a daily basis for a community of 2,000
149 inhabitants. Regarding its physicochemical composition, TS of DOW was 25%, including 1,200 mg COD/g
150 TS, 25 mg N/g TS and 3 mg P/g TS. Moreover, its content in carbohydrates was 600 g/kg VS, in proteins
151 200 g/kg VS and in sugars 160 g/kg VS. In the baseline configuration, DOW was firstly ground to produce
152 a homogeneous mixture and then diluted with water up to 6% TS. The fermenter was fed at an average
153 OLR of 11 kg COD/m³ d and operated at 35°C and at a HRT of 5.2 d (Katsou et al., 2015; Lee et al., 2014;
154 Traverso et al., 2000). During the fermentation process, organic matter is converted to acids (e.g. acetic
155 acid, butyric acid, lactic acid etc.) while CO₂ is also released as a result of metabolic processes.
156 Furthermore, hydrolysis of organic nitrogen and subsequent ammonification takes place, pH increases
157 and some ammonia is released into the atmosphere. It was assumed that 8.5% of COD was converted
158 into carbon dioxide and methane, and losses of TN (2%) as ammonia and TS (2%) take place (Battistoni
159 et al, 2002).

160 **Dewatering unit**

161 The fermented DOW and the excess sludge from the UASB and SBR were separated into a liquid and a
162 solid fraction by applying a screw-press. Therefore, a liquid fraction rich in VFAs was produced to be
163 used in the SBR and a solid fraction to be further treated in the composting unit. The separation
164 efficiencies of fermented DOW and sludge are different. More specifically, in the case of fermented
165 DOW 65% of TS, 40% of COD and TN and 50% of TP are transferred to the solid stream and the
166 remaining in the liquid fraction; when sludge was separated 95% of TS, COD, TN and TP ended up in the
167 solid fraction (Albertson et al., 1991; Battistoni et al., 2002). The produced solid fraction was sent to the
168 composting unit, while the liquid fraction was stored in an equalisation tank with a HRT of 10 h before
169 being fed to the SBR.

170

171 **SBR**

172 The SBR was applied as a post-treatment stage of the UASB effluent and of the liquid stream generated
173 from the screw-press. The SBR cycle comprised of filling, the sequential operation under anaerobic,
174 aerobic and anoxic conditions, settling and decanting. The system operated at low dissolved oxygen
175 level (around 1 mg/L) in order to perform short-cut nitrification/denitrification (scND) instead of the
176 conventional nitrification/denitrification (ND) process. Previous work has shown that the combination of
177 a suitable vNLR and low DO can result in effective via nitrite nutrient removal from domestic sewage
178 (Katsou et al., 2015). The calculation of the oxygen demand was based on the organic carbon and
179 ammonia load.

180 **Composting process**

181 Sludge composting took place in an enclosed system equipped with a biofilter (Colón et al., 2009).
182 Wheat straw was used as a bulking agent and was mixed with sludge in order to improve aeration, to
183 provide a C/N ratio in the range of 25:1-35:1 and adjust the moisture content of the mixture in the range
184 of 60-65% (Hernandez et al., 2006; Tremier et al., 2005). The addition of the bulking agent also
185 prevented the compost mixture from excessive compacting. The straw had the following characteristics:
186 90% of TS out of which 90% were VS, 60% of total carbon, 0.9% of TN and 0.1% of TP (Rihani et al.,
187 2010).

188 **2.2. Integrated treatment schemes for wastewater and DOW management**

189 Alternative approaches were examined to identify the best treatment configuration from a technical
190 and environmental point of view. More specifically, three options were analysed considering different
191 integration levels of FWDs in the community, diverse nitrogen removal options in the SBR and the
192 possibility of including phosphorus removal.

193 **DOW collection**

194 The collection of DOW in the community was considered with various FWDs integration rates. These
195 disposal units are equipped with a shredding system, allowing effective collection of DOW, which is
196 pumped together with wastewater to the treatment plant.

197 - Configuration 1 involved the separate collection of wastewater and DOW (0% FWDs
198 integration). Wastewater was pumped from the households to the WWTP, whereas DOW was
199 separately collected at households and transported by trucks to the WWTP.

200 - Configuration 2 included FWD integration in 50% of the households in the community (Evans et
201 al., 2010). The remaining DOW that was not managed through FWDs was transported by trucks to
202 the treatment plant.

203 - Configuration 3 considered complete integration of FWDs (100%) in the community.
204 Wastewater and DOW streams produced were delivered together to the WWTP.

205 The introduction of FWDs has been reported to cause an additional load of 60 g TSS/capita d, 95 g
206 COD/capita d, 2.1 g N/capita d and 0.3 g P/capita d in the influent wastewater (Bernstad et al., 2013; De
207 Koning, 2003). The application of FWDs leads to an increase in tap-water consumption (up to 4.5
208 L/capita d) required for pumping wastewater and DOW to the WWTP (Bernstad et al., 2013;
209 Rosenwinkel and Wendler, 2001). A primary settler was implemented before the UASB to receive the
210 mixture of wastewater and shredded DOW for the effective settling of the primary sludge (Figure 2). The
211 removal efficiencies of COD, TSS, TP and TN in the primary settler were assumed to be 30%, 50%, 10%
212 and 5%, respectively (Tchobanoglous et al., 2014). The produced sludge was fed to the fermentation
213 unit to produce VFAs, while the supernatant was fed into the UASB.

214 **Alternative processes for nitrogen removal**

215 Biological nitrogen removal was integrated in the scheme by applying conventional
216 nitrification/denitrification (ND) and short-cut nitrification/denitrification (scND) in the SBR.

217 **Phosphorus removal**

218 Biological phosphorus removal using oxygen and nitrate or nitrite as electron acceptors in the SBR was
219 also evaluated. Nitrogen and phosphorus removal accomplished under anoxic conditions require lower
220 amounts of external carbon source and energy compared to aerobic conditions (Malamis et al., 2013).
221 The tested configurations are summarised in Table 1.

222 Table 1 around here

223 **2.3. Environmental impact of the decentralised schemes**

224 This section includes the quantification of the environmental impact of each configuration for the
225 identification of the most favourable one from an environmental point of view, using the LCA
226 methodology (ISO 14040, 2006). The functional unit (FU) selected was the treatment of the wastewater
227 and DOW produced by a community of 2,000 PE per day.

228 **System boundaries**

229 The processes considered within the system boundaries of the tested configurations are outlined in
230 Figure 2. The generation of waste streams (wastewater and DOW) was excluded from the
231 environmental analysis, since it does not affect the resource valorisation. The sewer network has an
232 important contribution to the total environmental impact of wastewater management (Doka, 2007).
233 However, in this work, the sewer system was excluded for the purpose of comparison because it was
234 considered to be similar for all scenarios.

235 In LCA studies, when waste treatment systems are converted into alternatives for resource recovery,
236 they are usually credited by considering the avoided environmental impacts of producing a different
237 product with the same function (Finnveden et al., 2005). In this manner, the environmental benefits of
238 the production of valuable products can be quantified. The produced heat from biogas was partially
239 used to heat the fermentation reactor, while the surplus heat can be exploited for heating nearby
240 households 8 months per year. Fuel oil was assumed as the fuel used for accounting the environmental
241 credits, since it was considered the most appropriate for a small and decentralised community.

242 Numerous studies have demonstrated the horticultural properties of compost, being able to substitute
243 peat in the production of ornamental plants (Ceglie et al., 2015; Russo et al., 2011), although its fertiliser
244 capacity is lower than that of other organic substrates such as manure or digestate (De Vries et al.,
245 2012). Therefore, it was assumed that the produced compost can be used as soil conditioner avoiding
246 the extraction, transport and use of a similar quantity of peat (Boldrin et al., 2009; Saer et al., 2013).

247 Figure 2 around here

248 **Inventory data**

249 Inventory data regarding all inputs and outputs for each configuration were based on experimental
250 results from the UASB-SBR pilot plant and mass balances. A description of the bibliographic sources used
251 to build the life cycle inventory is given in Table 2. A detailed description of inventory data of the base
252 case can be found in Table S2 of the supplementary material.

253 Table 2 around here

254 The ecoinvent® database (2016) was used to introduce background data for production of electricity,
255 heat from fuel oil and peat (Dones et al., 2007), manufacturing of chemicals (Althaus et al., 2007),
256 transportation (Spielmann et al., 2007) and waste disposal (Doka, 2007). Concerning the production of
257 electricity, the process included in the database has been updated using data for the average electricity
258 generation and import/export data for Italy in 2014 (Terna Rete Italia, 2015).

259 **Impact assessment methodology**

260 This section describes the methodology used to select representative impact categories. Direct and
261 indirect GHG emissions and eutrophication attributed to the discharge of the treated effluent have
262 important environmental impacts in most WWTPs (Rodriguez-Garcia et al., 2011). Hence, among all the
263 available impact categories within the LCA methodology, only three were considered: climate change
264 (CC), freshwater eutrophication (FE) and marine eutrophication (ME). In particular, CC estimates the
265 contribution of the system to the global warming effect and it is influenced by the amount of direct and
266 indirect GHGs. The categories of FE and ME measure the potential enrichment of nutrients in water

267 bodies (freshwater and marine water, respectively). FE is affected by phosphorus-based substances,
268 while ME accounts for nitrogen-based compounds. Finally, the potential impacts regarding CC were
269 determined by considering the characterisation factors provided by IPCC (2013), while the potential
270 damages due to FE and ME were measured through the characterisation factors reported by the ReCiPe
271 Midpoint H methodology (Goedkoop et al., 2009).

272 The relation between the background processes and CC used in the study is shown in Table S3 of the
273 supplementary material.

274 3. Results and discussion

275 3.1. Effluent quality, bioenergy generation and compost production

276 This SBR had a HRT of 10 days, a solids retention time (SRT) of 18 days, a volumetric nitrogen
277 loading rate (vNLR) of 0.15 kg N/m³ d and a volumetric phosphorus loading rate (vPLR) of 0.022 kg
278 P/m³ d. These parameters were considered to be invariable among all the configuration schemes
279 Regarding dissolved oxygen (DO) concentration in the aerobic reactor, the SBR performing BNR via
280 nitrate operated at DO concentrations of 2 mg/L; whereas the DO level was kept close (and even
281 below) 1 mg/L in the process via nitrite. It was observed that under these conditions the ratio of
282 NO₂-N/NO_x-N gradually increased and was steadily maintained above 99% during the operation of
283 the SBR. In the ND configurations, sNUR was on average 2.02 g N/kg VSS h, while in the scND
284 configurations, the sNUR was on average 4.93 g N/kg VSS h, as supported by experimental results.
285 The lower needs of external carbon source in the BNR process via nitrite can maintain higher
286 average sNUR in the reactor. When enhanced biological phosphorus removal was performed, the
287 pathway schemes integrating processes via nitrite resulted in slightly higher specific phosphorus
288 uptake rates (sPUR) compared to the processes via nitrate: 3.85 g P/kg VSS h and 3.19 g P/kg VSS h,
289 respectively. Table 3 shows the characteristics of the treated effluent as this is calculated for each
290 scenario in terms of COD, TSS, TN and TP.

291 Table 3 around here

292 All the scenarios achieved COD levels between 36.4 and 69.5 mg/L and TSS concentrations from 14.1 -
293 25.9 mg/L in the treated effluent; therefore the treated effluent met the EU limits of COD and TSS for
294 discharge into water bodies. However, the quality of the treated effluent regarding nutrients and TSS
295 was not appropriate for reuse. Regardless of the waste collection strategy, only the systems which
296 performed the BNR through the short-cut nitrification/denitrification together with biological
297 phosphorus uptake via nitrite (scND-P configurations) is able to **reduce the nutrients to the levels**
298 **required by existing National standards in Europe in. In any case, to comply with the reuse criteria**
299 **tertiary treatment (coagulation and sand filtration) followed by appropriate disinfection is required.**
300 The configurations applying scND achieved 85-86% nitrogen removal, while the nitrogen removal
301 efficiency was 67-85% for ND configurations. Phosphorus removal, when applied, was higher than 80%
302 for BNR via nitrite and around 43-73% via nitrate. The relation between the carbon source supplied and
303 the one required for the BNR process is the reason behind these results, since in some configurations
304 such as Configuration 1-ND, the carbon source required for the denitrification process is significantly
305 higher than the one that is available and is thus supplied by the system. In this case, high levels of
306 external carbon source were required for BNR in the conventional treatment system. The COD
307 consumed for denitrification ranged from 49.6-57.2 kg COD/day, while the COD required for conventional
308 denitrification via nitrate varied from 61.2-99.8 kg COD/day. The latter was not enough to remove
309 nitrogen and this is the reason why the nutrient concentrations of the treated effluent are **higher** in the
310 conventional nitrification/denitrification processes. Diverting fermented DOW liquid from the UASB to
311 the SBR resulted in lower biogas production in the UASB (Table 4). More specifically, the application of
312 conventional ND allowed recirculation rates of fermented liquid to the UASB from 0%-11% of the
313 amount of fermented liquid produced, while the respective recirculation rates were up to 45% for the
314 scND scheme. **As a result, when scND was performed, the average biogas production was usually higher.**
315 Regarding the food waste collection options, the use of FWDs (Configurations 2 and 3) increased the
316 COD levels at the head of the plant. After primary settling, the settled sludge was fed to the
317 fermentation unit to produce VFAs, while the clarified effluent was sent to the UASB. Part of the
318 fermented liquid was sent to cover the BNR needs of the SBR and the remaining part to the UASB to

319 increase biogas recovery. More specifically, 59% and 52% of the inlet COD was fed to the UASB in
320 configurations 2 and 3, respectively.

321 Table 4 around here

322 The treatment and disposal of sludge is an important issue in WWTPs (Wei et al., 2003). The most
323 commonly applied methods for sludge disposal at EU level include landfills, land application and
324 incineration. In the examined systems, sludge was valorised through composting. The compost
325 properties must be in line with the quality assurance protocol. As seen in Table 4, sludge production was
326 directly affected by the food waste collection system. The partial or total application of FWDs
327 (Configurations 2 and 3) resulted in higher sludge production compared to the separate collection
328 schemes (Configuration 1) (around 18-19% increase). The larger sludge production is attributed to the
329 operation of the primary settler required when FWDs are used. The implementation of the primary
330 settler implies the separation of primary sludge that is further sent to the fermentation reactor.
331 Furthermore, the amount of sludge produced was 2-4% lower in the scND configurations. Finally, the
332 schemes with EBPR produced more sludge than the respective ones without EBPR (around 3-7%
333 increase).

334 **3.2. Environmental profile of the UASB – scSBR configuration**

335 Table 5 summarises the LCA characterisation results for the scND configuration per functional unit, split
336 up by the processes involved. Positive values indicate environmental burdens, whereas negative values
337 are indicative of environmental credits.

338 Table 5 around here

339 Despite the differences in environmental results among the examined impact categories, it is important
340 to highlight the general positive effect of avoided processes. Avoided peat use had a modest
341 contribution, while avoided heat production from fuel oil played an important role in offsetting the GHG
342 emissions. In LCA studies, the choice of avoided products has a strong influence in the results. For this
343 reason, a detailed analysis of the influence of these methodological assumptions was performed in
344 section 3.5. In addition, the treatment scheme under assessment was evaluated excluding

345 environmental credits; therefore, each impact category was examined in detail considering only
346 negative loads in order to identify the system components with greater environmental impacts. Figure 3
347 summarizes the relative contributions of each process to CC for the baseline scenario.

348 Figure 3 around here

349 Regarding the environmental impact in CC (467 kg CO₂ eq/FU), the electricity requirements contributed
350 up to 13% of the global impact produced in CC. From the total electricity consumed in the treatment
351 plant, aeration in SBR accounted for 60%. The environmental impact of the consumption of electricity is
352 directly linked to the electricity mix of the specific country under study. In this case, the Italian electricity
353 profile produces 0.46 kg CO₂ eq/kWh_{produced}. If the treatment plant would have a solar system installed,
354 the emissions would be only 0.08 kg CO₂ eq/kWh_{produced} (Dones et al., 2007). Direct emissions in the SBR
355 unit (76 kg CO₂ eq/FU) were significant contributors to the environmental profile of the treatment
356 scheme, representing 37% of the total impact produced in CC. Dissolved methane from the UASB
357 effluent is by far the most influential compound (201 kg CO₂ eq/FU), followed by nitrous oxide emitted
358 from the SBR (11 kg CO₂ eq/FU). Emissions derived from the composting unit also contributed with 13%
359 of the impacts in CC; these environmental impacts were related to direct emissions of methane and
360 nitrous oxide that were generated during biomass decomposition. Despite the fact that composting is an
361 aerobic process, methane emissions may occur, especially for enclosed systems, in anaerobic pockets of
362 the substrate/mixture that is composted (Boldrin et al., 2009). Methane and nitrous oxide emissions
363 derived from the application of compost on land were accounted according to Bruun et al. (2006);
364 representing 13% of the total impact produced in CC. In addition, these processes were also a source of
365 carbon dioxide; particularly, the composting and the SBR process. However, due to the natural carbon
366 cycle, carbon from biogenic sources can be considered as climate-neutral, since the equivalent amount
367 of carbon dioxide emitted from an organic source is previously uptaken in the photosynthesis. Other
368 minor sources of GHG emissions were waste disposal in landfill (10%), infrastructure (4%),
369 transportation (3%) and biogas losses (2%).

370 The relative contributions of each process to the eutrophication related categories are outlined in Figure
371 4. The discharge of treated effluent was the main source for eutrophication emissions, contributing up

372 to 98% in FE and 57% in ME. Emissions of phosphorus from the treated effluent were responsible for FE,
373 while nitrogen emissions from the treated effluent were related to ME. In addition, leachates of nitrate
374 derived from the application of compost on land had an important contribution in ME (37%).

375 Figure 4 around here

376 **3.3. Towards increased environmental sustainability**

377 The different treatment configurations were analysed in terms of their environmental profile to identify
378 the most sustainable scheme. Characterisation results for each configuration are given in Table 6. Figure
379 5a, b and c summarizes the comparative results for each impact category.

380 Table 6 around here

381 Figure 5 around here

382 In terms of CC, the profile of the system depended on the collection scheme (Figure 5a). The partial use
383 of FWDs (Configuration 2) resulted in GHG emissions between 1.80 and 2.17 times higher than in the
384 baseline scenario. As shown in Table 5, the use of FWDs resulted in less methane generation and
385 subsequently, in lower environmental credits due to avoided heat production that highly affected this
386 category (Table 4). Additionally, this collection system increased the production of sludge, increasing the
387 environmental impacts from direct emissions of nitrous oxide and methane from composting and land
388 application processes. Finally, the implementation of FWDs was associated with additional energy
389 consumption compared to the separate collection of DOW (Table 2). Concerning the removal of
390 nutrients, the denitrification process entailed different emissions and aeration requirements, resulting
391 in different electricity consumption (Table 2). In particular, energy consumed for air supply was 14%
392 higher in the Configurations performing nitrogen removal via nitrate than via nitrite. As a consequence,
393 ND configurations exhibited 15% more environmental impact on average regarding CC than scND
394 configurations.

395 As shown in Figure 4, the discharge of the treated effluent was the most important contributor to FE
396 and ME impact categories. Therefore, the treated effluent quality can explain the differences observed

397 in Figures 5b and 6c among the examined configurations. A reduction of 28%-82% in the FE category
398 was observed in the systems that perform EBRP, since this impact category is only influenced by
399 phosphate-based emissions. Finally, the configurations that perform **nitrogen removal** via nitrate
400 together with EBPR resulted in high nitrogen levels in the final effluent since the carbon source was not
401 enough for complete denitrification. **This** adversely affecting ME, **which was** 44-90% higher compared to
402 the baseline scenario.

403 **3.4. Sensitivity analysis**

404 A sensitivity analysis was performed to analyse three selected, key parameters: (i) the COD removal
405 efficiency in the UASB process, (ii) the efficiency in the separate collection of DOW at household level
406 and (iii) the bulking agent used for composting. The outcomes from the sensitivity analysis are
407 presented in Tables **S4**, **S5** and **S6** of the supplementary material, respectively.

408 *COD removal efficiency in UASB.* **COD removal of 77% was** considered in the UASB for calculations as
409 the base case. In sensitivity analysis 1 (SA1), it has been considered that only around 50-55% of the COD
410 input was **removed**, which implied lower biogas and sludge production in the UASB. In addition, in spite
411 of the higher COD concentration in the UASB effluent compared to the base case, the quality of the
412 **treated effluent has low nutrient concentrations** in Configurations 2 and 3-scND-P **allowing the potential**
413 **reuse**. However, this resulted in higher energy consumption for aeration and sludge production in the
414 SBR process. Therefore, the decrease in biogas production (29-35% lower compared to the base case)
415 was not only attributed to lower COD removal but also to lower flow of fermented liquid recirculated to
416 the UASB reactor. Moreover, total sludge production in the treatment scheme was higher, which was
417 attributed to the sludge production in SBR (48-77% more), which ended up in a larger production of
418 compost (10-19%). From an environmental perspective, all these changes entailed an increase in GHG
419 emissions by 65 kg CO₂ eq/d with respect to base case.

420 *DOW collection efficiency.* In the base case assessment, it was assumed that 83% of DOW produced at
421 household level was separately collected and delivered to the treatment facility. In Configurations 1, this
422 meant that from the total 600 kg DOW produced each day, 500 kg were delivered to the treatment

423 facility; in Configurations 2 this meant that 250 kg DOW/d were delivered to the facility because the
424 remaining 300 kg were collected through FWDs. Configurations 3 were not influenced since all DOW
425 produced was collected in FWDs. However, the values of collection efficiency can vary from one
426 community to another. Therefore in sensitivity analysis 2 (SA2), it was assumed that only 40% of the
427 produced DOW was successfully separated in the households. The lower collection efficiency resulted in
428 lower amount of DOW sent to the fermentation reactor, implying less available carbon source and/or
429 biogas production, but also greater amount of DOW sent to landfill. Under this assumption, although
430 100% of the fermented liquid is sent to the SBR in Configurations 1, it is not enough for **efficient nutrient**
431 **removal** and the treated effluent is characterized by elevated **nutrient** concentration. Conversely, the
432 collection efficiency had a lower influence in Configurations 2, where the supply of DOW is guaranteed
433 by the implementation of FWDs in 50% of the households. In these cases, the quality of the effluent was
434 similar in comparison to the base case due to the effective collection of DOW in the FWDs, which
435 allowed the proper supply of carbon source in the SBR. Moreover, the total amount of sludge produced
436 and compost are lower especially in Configurations 1 due to the **lower** amount of DOW handled;
437 however, due to the higher amount of organic waste sent to landfill and the lower amount of biogas
438 produced, the environmental impacts produced in Configurations 1 were on average around 40 kg CO₂
439 eq/FU higher.

440 *Bulking agent used for composting.* In the base case, wheat straw was used as bulking agent in the
441 composting process. In sensitivity analysis 3 (SA3), wheat straw was substituted by sawdust. The change
442 in the bulking agent meant different compost mixture composition, resulting in different emissions from
443 composting and from compost application. Wheat straw had a composition in terms of 10% moisture,
444 TC, TN and TP of 60%, 0.9% and 0.1%, respectively as percent of dry solids; the composition of sawdust
445 was 20% moisture, 60% TC, 0.2% TN and 0.03% TP. The lower content in nutrients resulted in (i) lower
446 amount of sawdust required to achieve the appropriate C/N ratio and (ii) lower emissions of **nutrient-**
447 **based** compounds derived from the composting process and the application of compost on land
448 (including emissions of nitrous oxide and ammonia and leachates of nitrate and phosphate). In terms of
449 CC, this change meant a reduction in GHG emissions of 6% in average, while in ME the global impacts
450 produced by the treatment schemes proposed were reduced up to 10%. However, no significant

451 changes occurred in terms of FE (<0.1%), since almost all of the effects produced in this impact category
452 (>98%) were allocated to the discharge of the treated effluent.

453 **3.5. Assessment on the reliability of the environmental results**

454 The influence of the selection of important parameters in the environmental balance was assessed. A
455 comparison between the baseline case and alternative scenarios was performed to identify sensible
456 variations in the results.

457 *Biogas losses.* Fugitive biogas emissions from anaerobic processes are usually included in the
458 environmental analysis. These emissions directly affect CC, not only due to direct methane emissions,
459 but also by decreasing the potential heat production from biogas. In the baseline scenario of the current
460 study, 1.5% of biogas produced was taken into account as biogas losses in accordance to De Vries et al.
461 (2012). Poeschl et al. (2012) considered that these losses can vary from 1 to 1.8%. A sensitivity analysis
462 was performed to assess the influence of different rates of biogas losses in CC (i.e. 1% and 1.8%). The
463 decrease of the emissions to 1% of the biogas produced can save from 3-4 kg CO₂ eq/FU; whereas when
464 the biogas losses were 1.8%, the environmental profile can increase by 5-7 kg CO₂ eq/FU. Therefore, this
465 assumption had a slight effect on the definition of the environmental profile (±1%).

466 *Avoided products.* As described in Section 3.2., credits from the avoided products played an important
467 role in offsetting the environmental impacts of the applied treatment scheme, especially regarding CC.
468 Alternative avoided products were analysed to identify **their impact**. **The** baseline case where the
469 avoided heat was produced from fuel oil at small-scale in Europe, was compared with the substitution of
470 heat produced from different fuels, such as natural gas and hard coal (Dones et al., 2007). **The**
471 substitution of peat for compost is usually done on a 1:1 volume basis (Boldrin et al., 2009). In this
472 study, identical density was assumed for compost and peat. However, Boldrin et al. (2009) stated that
473 compost and peat densities are very variable and can be different; it is possible that 1 tonne of compost
474 can replace the use of 0.2-1 tonne of peat. Accordingly, an equivalence of 0.2, 0.6 and 0.8 t peat/t
475 compost was considered. Concerning avoided heat, the production of heat from fuel oil generates 0.32
476 kg CO₂ eq/kWh (base case), while the environmental impact of heat production from natural gas and

477 hard coal is 0.26 and 0.57 kg CO₂ eq/kWh, respectively. Considering natural gas as the substitute fuel,
478 the environmental impacts can increase around 13-24 kg CO₂ eq/FU; whereas, when considering hard
479 coal, it can be improved by 35-63 kg CO₂ eq/FU; meaning an environmental profile 5-13% lower
480 compared with the base case. Therefore, the substitution of heat from fuel oil to heat from hard coal
481 has a considerable effect. With regard to avoided peat, the lowest replacement ratio (0.2 t peat/t
482 compost) means an increase of the environmental profile up to 3% (~15 kg CO₂ eq/FU).

483 *Treated effluent reuse.* The quality of the treated effluent in the scND-P configurations met the
484 specifications for water reuse in Italy **provided that effective tertiary filtration and appropriate**
485 **disinfection take place** (Section 3.1.). Therefore, the treated water can be reused for irrigation instead of
486 being discharged in water bodies. This practice reduces the impact of direct discharge of nutrients;
487 however, it entails other potential environmental burdens from **the filtration and disinfection as well as**
488 **the use of agricultural machinery and emissions derived from the treated effluent discharge on land.** In
489 **the sensitivity analysis, it has been considered that the effluent is further treated in a sand filter using**
490 **aluminium sulphate as coagulant, followed by UV disinfection, as described in (Meneses et al., 2010).** In
491 addition, derived emissions were computed using the methodology described in IPCC (2006).

492 As shown in Figure 6, the use of treated effluent for irrigation had **an** adverse impact in the
493 environmental profile regarding CC **(by 18-20%) due to the tertiary treatment as well as** the use of
494 agricultural machinery for irrigation. On the contrary, the performance was improved **by 69-83%% and**
495 **37-41%% for FE and ME,** respectively, due to the reduction of direct P and N emissions into water
496 recipients.

497 Figure 6 around here

498 LCA works dealing with wastewater treatment have also identified environmental benefits (i.e.
499 replacement of mineral fertilisers) from the use of reclaimed water for agricultural purposes (Meneses
500 et al., 2010). The application of scND-P configurations can result in savings of 3.85-4.10 kg N/FU and
501 0.95-1.59 kg P₂O₅/FU of nitrogen and phosphorus based fertilisers. This results in a reduction of

502 approximately 44.7 kg CO₂ eq/FU, 8.24 g P eq/FU and 10.5 g N eq/FU for CC, FE and ME, respectively,
503 which enhances the environmental profile of the systems by 11-17%.

504 **3.6. Wastewater treatment alternatives in small communities**

505 Several systems have been reported in literature for wastewater treatment in small and decentralised
506 communities. The most common treatment scheme for wastewater treatment in small communities is
507 constructed wetlands (Barros et al., 2008; Chan et al., 2008; Wu et al., 2011; Ye and Li, 2009). Other
508 configurations have also been proposed, such as trickling filter, activated sludge, membrane bioreactor
509 or extended aeration (Molinos-Senante et al. (2012) and an integrated step-feed biofilm process (Liang
510 et al., 2010). A review of different schemes designed for the treatment of domestic wastewater at
511 decentralised level can be found in Table 7. In more detail, Nogueira et al. (2009) compared the
512 economic and environmental profile of energy-saving and intensive wastewater treatment systems.
513 Energy-saving technologies such as slow rate infiltration plants and constructed wetlands exhibited
514 better results compared to the activated sludge processes. Yildirin and Topkaya (2012) evaluated the
515 environmental behaviour of constructed wetlands, vegetated land and activated sludge (with and
516 without phosphorus removal), which reported similar results in CC impact but also in terms of the
517 eutrophication-related categories.

518 Table 7 around here

519 Regarding more advanced treatment technologies, Zeeman et al. (2008) analysed the operational
520 performance of UASB for the separate treatment of both grey and black water. Grey water was treated
521 in a UASB-SBR system, while a struvite precipitation process was applied after the UASB process for
522 black water. The comparison of the proposed treatment configurations with conventional sanitation
523 showed energy savings of 200 MJ/PE year and phosphorus recovery via struvite of 0.14 kg P/PE year. In
524 comparison with our study, important water reductions related with the use of vacuum toilets are
525 shown. The production of grey and black water was in the range of 60-90 L/PE year and 6.8-7.5 L/PE
526 year, respectively, while a production of 73 m³/PE year was considered in the current work. Energy
527 consumption was estimated as 151 MJ/PE year. Despite the differences among the treatment systems
528 examined in the current work, similar results were obtained in configurations 1 (160 MJ/PE year), while

529 in configurations 2 and 3, energy consumption was higher (300 MJ/PE year). Alternatives of the
530 conventional SBR were also analysed in the literature, including the performance of a sequencing batch
531 membrane bioreactor (SBMBR) (Krampe, 2013). One of the advantages of coupling a membrane to a
532 SBR is the reduced cycle time as a result of the elimination of the settling phase and complete
533 elimination of suspended solids in the treated effluent. However, they are associated with higher
534 operating costs due to membrane fouling.

535

536 **4. Conclusions**

537 The technical evaluation of the systems revealed:

538 - The co-management of wastewater and DOW is feasible for a small community (i.e. up to 2,000
539 PE), regardless of the applied collection scheme; the treated effluent met the discharge
540 requirements.

541 - The removal of nitrogen via nitrite with EBPR in the SBR upgraded the treated effluent quality
542 allowing its reuse for agricultural purposes. Nitrogen and phosphorus uptake rates were higher in
543 the processes removing nutrients via nitrite

544 - Configurations performing denitrification via nitrite allowed higher levels of fermented liquid
545 recirculation in the UASB, resulting in higher biogas generation.

546 The environmental assessment of the alternative processes in the integrated systems showed:

547 - Climate change achieved the lowest results in Configuration 1-scNSD (467 kg CO₂ eq/FU) and
548 the highest GHG emissions were produced in Configuration 2-ND-P (622 kg CO₂ eq/FU). The
549 environmental impacts were mainly attributed to the energy requirements for FWD operation and
550 SBR aeration. The use of FWDs increased GHG emissions by 57% - 67% compared to the separate
551 collection, while denitrification via nitrate entailed 15% higher impacts in CC compared to nitrogen
552 removal via nitrite.

553 - Impacts in eutrophication related categories derived from the discharge of the treated effluent.
554 Thus, the collection scheme does not affect the environmental performance. The systems which
555 perform nitrogen removal via nitrite and EBPR via nitrite resulted in better environmental profile
556 concerning FE and ME.

557 Considering technical and environmental aspects, it can be concluded that the separate collection of
558 waste combined with nitrogen removal and phosphorus uptake via nitrite is the best configuration for
559 the combined treatment of wastewater and DOW in a small community of 2,000 PE. However, the
560 collection efficiency highly influences the performance of the treatment scheme.

561

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