

EXPERIMENTAL STUDY AND MODELING OF RHEOLOGICAL AND MECHANICAL PROPERTIES OF NHL GROUTS

Luis G. Baltazar^{1*}, Fernando M.A. Henriques², Maria Teresa Cidade³

¹ PhD in Civil Engineering, Departamento de Engenharia Civil, Faculdade de Ciências e Tecnologia, Universidade Nova de Lisboa, P-2829-516 Caparica, Portugal

² Full Professor, Departamento de Engenharia Civil, Faculdade de Ciências e Tecnologia, Universidade Nova de Lisboa, P-2829-516 Caparica, Portugal

³ Assistant Professor with Habilitation, Departamento de Ciência dos Materiais e Cenimat/I3N, Faculdade de Ciências e Tecnologia, Universidade Nova de Lisboa, P-2829-516 Caparica, Portugal

* Corresponding author. Tel.: +351 21 2948580; fax: +351 21 2948398; E-mail addresses: luis.baltazar@fct.unl.pt

ABSTRACT

This paper aims to model the effect of grout composition on properties of two natural hydraulic lime grouts based on the correlation between grout rheometer results and simple flow tests. Firstly, the effects of water/binder ratio and superplasticizer dosage on its rheological properties and flowability were analyzed. Dosage of superplasticizer and water/binder ratio were varied from 0.6% to 1.2% (by mass of binder) and 0.45 to 0.55, respectively. A good correlation between classical flow tests and the rheological properties was obtained. Then, statistical models were formulated in order to estimating the grout parameters, such as plastic viscosity and yield stress just by performing simple flow tests. The models coefficients were calculated using multiple regression analysis. The statistical modeling results indicated that the properties of the grouts studied are linearly related to water/binder ratio, superplasticizer dosage and specific surface area of natural hydraulic limes. Finally, the accuracy of the models was experimentally confirmed using random grout compositions. The predicted-to-measured ratio ranged from 0.97 to 1.08, indicating a good agreement between the confirmation results and the expected results from the statistical models.

Keywords: Grout, natural hydraulic limes, superplasticizer, Marsh cone test, mini-slump test, rheology

34

35 INTRODUCTION

36 Multi-leaf stone masonry walls are a masonry typology with particular weaknesses, namely
37 weak monolithic behavior under both horizontal and vertical loads. Due to the presence of
38 poor materials in the inner core, voids and scarce of connections between elements this
39 masonry typology is prone to detach from the rest of the building and to fail out-of-the plane
40 by disaggregating into different parts (Binda et al 2006; Mauro and Felice 2012). Grout
41 injection is an interesting consolidation technique to repair and reinforce such walls. This
42 method is defined as the introduction of a fluid material injected under pressure into the
43 masonry inner core. Injection grouts aim to fill cracks and voids so that the load on the wall is
44 distributed evenly over the whole bearing area of the masonry, and bond the delaminated
45 layers together and help them resist lateral forces.

46

47 Grouts are mixtures of a binder with water and admixtures, which must feature adequate
48 fluidity and penetrability in order to be injectable. However, choosing appropriated grout is
49 often a critical phase. In most cases, the selection of grout depends on the desired fresh
50 properties and performance characteristics. It has been pointed out that fluidity and injection
51 capacity are essential fresh properties (Jorne et al. 2014; Baltazar et al. 2014; Baltazar et al.
52 2012). The control of fluidity can be done through rheological measurements. Currently,
53 rheology is often used as a tool in the control and design of cementitious suspensions, such as
54 cement based pastes, mortars, concretes and grouts (Cardoso et al. 2014; Baltazar et al. 2013;
55 Roussel et al. 2010; Roussel 2007). Therefore, rheological measurements using rheometers or
56 viscometers can be expensive and complex to operate, and also give too many information
57 when often just two rheological parameters are needed, such as yield stress and plastic
58 viscosity (Baltazar et al. 2013; Baltazar et al. 2015). In this framework, this paper aims to

59 model the effect of water/binder ratio and dosage of superplasticizer on the rheological
60 properties of natural hydraulic lime (NHL) grouts using simple tests easy to use at
61 construction sites, such as Marsh cone and spread test using a mini-slump flow. For this
62 reason the statistical modeling approach was used in order to establish models for predicting
63 the rheological properties, meaning that these models are able to calculate a physical
64 parameter (e.g. viscosity) instead of an empirical one (e.g. fluidity). The proposed models can
65 be very useful to smooth the grout design methodology, since the rheological properties of the
66 grout can be predicted just by performing simple flow tests traditionally used on field.
67 Additionally, two more models are proposed to help predicting the compressive and flexural
68 strength of those grouts.

69

70 This paper is part of a larger study and aims to help to achieve an adequate balance between
71 the grout rheological properties and its injectability, which can be tested in reduced models
72 simulating old masonries as the type used in (Jorne et al. 2014). Only then, based on the
73 injection tests, can the composition of those grouts be tailored according to the properties of
74 the masonry to which such a grout is to be injected. In practice, the variability of voids within
75 masonries requires an ability to fine tune the rheological properties of the grout in order to get
76 a successful consolidation process. The methodology presented in this research will provide
77 necessary tools for that purpose.

78

79 **METHODOLOGY**

80 The influence and significance of each grout design variable on its rheological behavior was
81 identified in a previous work using the design of experiments method (DOE) together with
82 analysis of variance (ANOVA) (Baltazar et al. 2013). Based on these previous results, the
83 water/binder ratio and dosage of superplasticizer were pointed out as the most significant

84 grout components. The first part of this paper will focus on the effects of these two grout
 85 components on yield stress, plastic viscosity, flow time, spread diameter and mechanical
 86 strength. In the second part, a correlation between rheological properties and the flow time
 87 and spread diameter results is made, in order to formulate statistical models for prediction of
 88 the rheological properties associated to each grout. Finally, in a third part, the accuracy of the
 89 models is experimentally confirmed using random grout compositions.

90

91 STATISTICAL MODELS

92 Statistical models were created for estimating the grout parameters, such as plastic viscosity
 93 and yield stress. This statistical modeling allows to estimate the value of a dependent
 94 parameter, such as yield stress or plastic viscosity, based on a set of independent variables,
 95 such as water/binder ratio, superplasticizer dosage, flow time, specific surface area of NHL
 96 used and spread diameter. The general form of the equation of multiple linear regression can
 97 be written as follows (eq. 1):

$$98 \quad y_i = \beta_0 + \beta_1 x_{i,1} + \dots + \beta_{p-1} x_{i,p-1} + \varepsilon_i, i = 1, 2, \dots, n, \quad (1)$$

99 Where y_i means the dependent variable, x_i is the independent variables, β_p is the regression
 100 coefficients and ε_i is the additive error. The matrix formulation involving predictor variables
 101 x_1, \dots, x_p takes the following structure (eq. 2):

$$102 \quad \begin{pmatrix} y_1 \\ y_2 \\ \dots \\ y_n \end{pmatrix} = \begin{pmatrix} 1 & x_{1,1} & x_{2,1} & \dots & x_{p,1} \\ 1 & x_{1,2} & x_{2,2} & \dots & x_{p,2} \\ \dots & \dots & \dots & \dots & \dots \\ 1 & x_{1,n} & x_{2,n} & \dots & x_{p,n} \end{pmatrix} \times \begin{pmatrix} \beta_1 \\ \beta_2 \\ \dots \\ \beta_p \end{pmatrix} + \begin{pmatrix} \varepsilon_1 \\ \varepsilon_2 \\ \dots \\ \varepsilon_n \end{pmatrix} \quad (2)$$

103 The first step in multiple linear regression analysis is to determine the matrix of coefficients
 104 ($\hat{\beta}$). The least-squares approach was used to obtain the coefficients of the regression
 105 equations. This means, that based on experimental data the regression coefficients $\hat{\beta}$ were
 106 calculated in order to minimize the mean square error between the observed and estimated

107 values of y -parameter. That is, to find the regression coefficient $\hat{\beta}$ that minimizes the
108 following criterion (Anderson 1984):

$$109 \quad D(\beta) = \sum_{i=1}^n (y_i - x_i \beta)^2 \quad (3)$$

110 Taking derivatives with respect to β , and setting these to 0, it is obtained the normal equations:

$$111 \quad \frac{dD}{d\beta} = -2X'(y - X\beta) = 0 \Rightarrow (X'X)\beta = X'y \quad (4)$$

112 where X and y are the matrix of variables x_1, \dots, x_p and y_1, \dots, y_n , respectively. To solve for
113 β the inverse of $X'X$ was applied to both sides of eq. 4 and the eq. 5 was obtained:

$$114 \quad \hat{\beta} = (X'X)^{-1}X'y \quad (5)$$

115 where $\hat{\beta}$ is the matrix of coefficients.

116

117 **EXPERIMENTAL DETAILS**

118 **Materials**

119 The experimental program was conducted using two types of NHL, namely NHL3.5 and
120 NHL5 produced according to the European Standard EN459-1:2010. NHL was chosen as
121 binder, since it is a hydraulic binder that presents chemical and physical properties closer to
122 those of pre-existing materials in old masonries and is able to set both in dry and wet
123 conditions, with or without contact with air (Baglioni et al. 1997; Binda et al. 2006).The
124 physical and chemical properties of both NHLs are listed in table 1. The grain size
125 distribution was determined in dry powder samples using a laser diffraction particle size
126 analyzer (Coulter LS 230), the results are represented in Fig. 1. A comparison of these two
127 NHL types revealed that NHL3.5 had a larger specific surface area than NHL5. A
128 commercially available polycarboxylate ether superplasticizer (i.e. high range water reducer)

129 conforming to ASTM C494-05 Type F was used. It had a specific gravity, pH, chloride
130 content, charge and solid content of 1.05, 8, <0.10%, anionic and 28-32%, respectively.

131

132 **Mixing procedure**

133 All grouts were prepared in laboratory in batches of 4 liters, and mixed using a high shear
134 mixer equipped with helicoidal blade (see Fig. 2). For the preparation of grouts ordinary tap
135 water was used. The mixing procedure adopted was obtained in previous research using the
136 design of experiments method (Baltazar et al. 2012): the whole lime is added to 70% of total
137 mix water (the water was first introduced in the mixer and then the binder) and mixed for 10
138 minutes. The remaining water (with diluted superplasticizer) is added within 30s. After all
139 materials had been added, the mixture was maintained for an additional 3 minutes. The
140 mixing speed during the whole mixing was 800 rpm. The hydraulic lime grouts were prepared
141 at ambient temperature of $20\pm 2^{\circ}\text{C}$ and a relative humidity of $60\pm 5\%$. Following the end of
142 mixing, the grout flow tests as well as the rheological measurements were performed as
143 described below.

144

145 **Experimental procedures**

146 As mentioned, in this study two key grout design variables were chosen to use in formulating
147 models for evaluating fresh and hardened properties. The values of each design variable are
148 summarizes in table 2. In total, eighteen grout combinations were tested (nine for each type of
149 NHL). The ranges of water/binder ratio and superplasticizer dosage were proportioned so that
150 the grouts exhibit acceptable performance to be used in masonry injection. Therefore, the
151 proposed models have a valid domain for grouts made with 100% of NHL, water/binder ratio
152 of 0.45 – 0.55 and superplasticizer dosage range of 0.6 – 1.2%, by mass of lime. The ranges
153 of water/binder ratio and superplasticizer dosage were proportioned so that the grouts exhibit

154 acceptable performance to be used in masonry injection. Water/binder ratio lower than 0.45
155 results in grouts with insufficient workability and therefore impossible to be injected
156 (Miltiadou, 1990). On the other hand, water/binder ratio higher than 0.55 leads to an
157 excessive bleeding of free water that compromises the grout stability and therefore its
158 homogeneity until onset of hardening. Three experimental procedures were performed
159 simultaneously: (i) rheological measurements using a rheometer to evaluate the grouts
160 rheological properties, (ii) Marsh cone test to evaluate the flow time and (iii) mini-slump test
161 to estimate the spread diameter. For each grout composition (see table 2) a total of 3200g
162 were used in each batch of preparation, three samples were collected to be used on the
163 rheological measurements, marsh cone test and mini-slump test. All these experimental tests
164 were performed at 5 minutes after the end of mixing process.

165

166 **Rheological measurements**

167 A rotational rheometer was used to evaluate rheological properties of NHL grouts, including
168 yield stress and plastic viscosity. The geometry used consisted of a plate-plate geometry (with
169 $\varnothing = 40$ mm) and a gap of 2 mm (see Fig. 3). In all measurements the testing protocol
170 consisted on subjecting the grout to a pre-shearing stage of 60s at shear rate of 1s^{-1} followed
171 by 60s at rest. The pre-shearing was applied in order to ensure a similar initial state for all
172 samples, since after mixing and depending on the time elapsed, the sample may not be exactly
173 at the same stage and the pre-shear has the advantage of eliminate those small differences,
174 before starting the rheological measurements. Then, the shear rate was increased from 0 to
175 300s^{-1} (the maximum shear rate used – see Fig. 4). Each shear rate was applied long enough in
176 order to ensure the attendance of the steady state, before measurements were recorded. All
177 grout samples were analyzed with a constant temperature of 20°C , maintained by means of a
178 temperature unit control. A solvent trap was used to prevent drying of the grout samples

179 during testing. The results were interpreted in the frame of rheology suspension knowledge;
180 this means that the Bingham model (eq. 6) was adopted to describe the grout rheological
181 behavior and to determine the yield stress and plastic viscosity of fresh grouts (Barnes 2000):

$$182 \quad \tau = \tau_0 + \eta_p \times \dot{\gamma} \quad (6)$$

183 where τ is the shear stress (Pa), τ_0 is the yield stress (Pa), η_p is the plastic viscosity (Pa.s), and
184 $\dot{\gamma}$ is the shear rate

185

186 **Marsh cone test**

187 The Marsh cone test was performed according to ASTM C939-02. Based on this standard the
188 measurement of flow time is connected to the grout fluidity. In order to improve the physical
189 significance of Marsh cone test (especially for grouts with high water/binder ratio), the grout
190 fluidity was evaluated using a modified Marsh cone having an outlet diameter of 5 mm. A
191 volume of 1,000 ml of grout was placed into the cone and the flow time (expressed in
192 seconds) refers to the time required for 800 ml of grout to flow through the cone.

193

194 **Mini-slump test**

195 Besides the Marsh cone test, the slump flow was also performed. In this work the mini-slump
196 test adopted is similar to the procedure developed by Roussel in (Roussel et al. 2005), in order
197 to try to correlate it with the yield stress measured in the rheometer. The mini-cone used has
198 an height of 50mm and a top and bottom diameter equal to 35mm and 50mm, respectively.

199

200 **Mechanical Strength**

201 In order to determine the mechanical characteristics of the formulated grouts, the flexural and
202 compressive strengths were evaluated with five samples of each grout, which were poured
203 into steel moulds (160x40x40 mm). After 5 days, the specimens were taken out from the

204 moulds and cured in a controlled atmosphere at $20\pm 2^{\circ}\text{C}$ and $60\pm 5\%$ relative humidity until the
205 age of maturity of 28 days. The load was applied at constant rate of 0.5mm/min and
206 0.7mm/min for flexural and compressive strength, respectively using a Z050 Zwick
207 mechanical test machine with 50 kN capacity following standard EN 1015-11:1999. In order
208 to understand the previous results, the evaluation of entrained air in fresh grout was measured
209 based on the standard EN 492-2:2002. Moreover, the effect of superplasticizer on the grout
210 microstructure was carried out using a Zeiss DSM 962 Scanning Electron Microscope (SEM)
211 at an accelerating voltage of 5kV.

212

213 **RESULTS AND DISCUSSION**

214 **Rheological measurements**

215 Typical shear stress vs. shear rate profile determined for the grout with water/binder of 0.50
216 and superplasticizer dosage of 0.8% is shown in Fig. 5 (the viscosity profile is shown in Fig.
217 6). The profiles obtained for the other grouts are qualitatively similar. The relationship
218 between shear stress and shear rate shows that all grouts (both types of NHL) have shear-
219 thinning behavior. However, it should be pointed out that, the high shear-thinning observed
220 may be caused by some slippage at the plate-material interface that can occur at low shear
221 rates, since smooth plates were used (Barnes, 1997). Notwithstanding, it is believed that the
222 shear-thinning behavior also occurred owing to the elongated shape of NHL particles
223 (Toumbakari, 2002; Bras, 2011). Moreover, and according to some authors (Banfill 2006;
224 Saak et al, 2001; Nehdi and Rahman, 2004) the slippage can also contribute to
225 underestimation of yield stress values. Therefore, in order to mitigate the effect of eventual
226 slippage, the yield stress values were determined as the intercept of eq 6 at zero shear rate (as
227 shown in Fig 5).

228

229 The effect of superplasticizer dosage and water/binder ratio on grout rheological properties is
230 presented in Fig. 7 and 8. It is known that superplasticizers enable to impose repulsive forces
231 that prevent the particle flocculation. Comparing the two types of NHL it is clear that (at the
232 same composition) the NHL3.5 has higher yield stress and plastic viscosity values than
233 NHL5, which is due to higher specific surface of NHL3.5 that makes less superplasticizer and
234 water available to promote fluidity. Consequently, the internal friction among solid particles
235 increases, leading to higher yield stress and plastic viscosity. Moreover, regardless the type of
236 NHL it can be seen that the yield stress is more affected with increasing superplasticizer
237 dosage than the plastic viscosity. As expected, the water/binder ratio has a similar behavior to
238 the superplasticizer dosage, however, simple addition of water to make the grout more fluid is
239 not an efficient solution because more water also increases porosity and consequently reduces
240 mechanical properties.

241

242 **Marsh test and mini-slump test**

243 The Marsh cone test is a simple test which can be used to characterize the workability of fresh
244 grout in the laboratory and in the field. The flow time seems to be a relevant parameter for
245 estimating the grout plastic viscosity. The Marsh cone value as function of superplasticizer
246 dosage and water/binder ratio is shown in Fig. 9, from which it can be observed that the flow
247 time is influenced by both superplasticizer dosage and water/binder ratio and it exhibits a
248 trend to decrease with their increase. The range of change observed in flow time between the
249 two types of NHL depends on the NHL specific surface area, which corroborates the
250 rheological parameters previous determined.

251

252 From the results of mini-slump test presented in Fig. 10 it can be concluded that grout spread
253 diameter increases with increasing superplasticizer dosage, which is in agreement with the
254 obtained results of yield stress. However, care must be taken when using high dosages of

255 superplasticizer. One can see in Fig. 10 that for NHL5 grout at the superplasticizer dosage of
256 1.2% the spread diameter is lower for water/binder ratio of 0.55 than for 0.50. It is believed
257 that this behavior is due to the joint effect of high water/binder ratio with excessive
258 unabsorbed superplasticizer present in the suspension. According to some authors (Banfill
259 2011; Flatt 1999) a higher superplasticizer concentration lead to a reverse effect. Regarding
260 the NHL3.5 this behavior was not observed as result of its high surface area.

261

262 **Correlation between flow tests and rheological properties**

263 The relationship between the flow time and plastic viscosity is presented in Fig. 11 showing a
264 good correlation (coefficient of correlation R^2 of 0.82 and 0.79 for NHL5 and NHL3.5,
265 respectively). The figure shows that when the flow time increases the same apply to the
266 plastic viscosity. Similarly, the coefficient of correlation between spread diameter and yield
267 stress was also good (R^2 of 0.71 and 0.92 for NHL5 and NHL3.5, respectively), as shown in
268 Fig. 12. It can be noted that the increase in spread diameter leads to reduced yield stress
269 values, as expected. The relationship between these properties shows that these simple test
270 methods (such as Marsh cone and mini-slump) can be used to predict the plastic viscosity and
271 yield stress value for grout optimization or fresh quality control in situ.

272

273 **Mechanical strength**

274 The grout mechanical strength is of great importance since the behavior of hardened grout has
275 a determinant effect on the mechanical properties of the grouted masonry. The average
276 measurements of flexural and compressive strength of NHL5 and NHL3.5 grouts at the
277 maturity age of 28 days with different water/binder ratios and superplasticizer dosages are
278 shown in Fig. 13 and 14. The obtained results and their analysis indicate that the general
279 tendency is that mechanical strength increases up to a superplasticizer dosage of 0.8 wt%,
280 which means that superplasticizer can be useful for improvement of mechanical properties.

281 The NHL hydration process and mechanism of the superplasticizer effects are out of the scope
 282 of this study. However, representative samples of grouts with and without superplasticizer
 283 were examined using a scanning electron microscope (SEM). Grout sample, containing
 284 superplasticizer, observation shows that instead of larger and well-defined crystals, smaller
 285 crystals and denser microstructure are formed (see Fig.15). Notwithstanding, a slight
 286 downward trend in the mechanical strength values between 0.8 and 1.2 wt% has been
 287 observed. According with the results of air entrained presented in Fig 16 it can be concluded
 288 that the presence of high concentration of superplasticizer leads to a slight increase of air
 289 incorporated in the fresh grouts, which may also explain the reduction in mechanical strength.

290 **Proposed models**

291 Based on the experimental results the least-square approach was used to found the coefficients
 292 of each modeled parameter. In the following steps the least-square approach is illustrated. The
 293 first step was the product of X' and X as follows:

294 $A = X' \cdot X$

295 where X is the matrix of independent variables

$$A = \begin{bmatrix} 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 & 1 \\ .55 & .55 & .55 & .50 & .50 & .50 & .45 & .45 & .45 & .55 & .55 & .55 & .50 & .50 & .50 & .45 & .45 & .45 \\ .6 & .8 & 1.2 & .6 & .8 & 1.2 & .6 & .8 & 1.2 & .6 & .8 & 1.2 & .6 & .8 & 1.2 & .6 & .8 & 1.2 \\ 598 & 598 & 598 & 598 & 598 & 598 & 598 & 598 & 598 & 480 & 480 & 480 & 480 & 480 & 480 & 480 & 480 & 480 & 480 \end{bmatrix} \times \begin{bmatrix} 1 & .55 & .6 & 598 \\ 1 & .55 & .8 & 598 \\ 1 & .55 & 1.2 & 598 \\ 1 & .50 & .6 & 598 \\ 1 & .50 & .8 & 598 \\ 1 & .50 & 1.2 & 598 \\ 1 & .45 & .6 & 598 \\ 1 & .45 & .8 & 598 \\ 1 & .45 & 1.2 & 598 \\ 1 & .55 & .6 & 480 \\ 1 & .55 & .8 & 480 \\ 1 & .55 & 1.2 & 480 \\ 1 & .50 & .6 & 480 \\ 1 & .50 & .8 & 480 \\ 1 & .50 & 1.2 & 480 \\ 1 & .45 & .6 & 480 \\ 1 & .45 & .8 & 480 \\ 1 & .45 & 1.2 & 480 \end{bmatrix}$$

296

$$= \begin{bmatrix} 18 & 9 & 15.6 & 9702 \\ 9 & 4.53 & 7.8 & 4851 \\ 15.6 & 7.8 & 14.64 & 8408.4 \\ 9702 & 4851 & 8408.4 & 5292036 \end{bmatrix}$$

297

298 In the next step the inverse matrix was determined:

$$K = (A)^{-1} = \begin{bmatrix} 13.696 & -16.667 & -0.774 & -0.01 \\ -16.667 & 33.333 & -7.9E-14 & -9.58E-16 \\ -0.774 & -6.5E-14 & 0.893 & 5.227E-18 \\ -0.001 & -1E-15 & 2.42E-30 & 1.596E-5 \end{bmatrix}$$

299

300 Using this matrix K , it can now be determined the coefficients for each model:

$$301 \hat{\beta} = K \cdot X' \cdot y$$

302 where y is the matrix of the outputs from each experimental test, such as the spread diameter,
303 flow time, etc. in order to determine the coefficient of the corresponding regression model.

304 The variables coefficients and their contribution for the modeled parameters are present in
305 table 3. The presentation in table 3 gives an indication of the relative significance of various
306 variables on each estimated parameter. The majority of the statistical models developed in this
307 study have high coefficients of determination (R^2) of more than 0.86 with the results from
308 experimental work. The models for rheological properties (plastic viscosity, yield stress) and
309 mechanical strength (flexural and compressive strength) of the NHL grouts are given in the
310 eqs (7), (8) and eqs. (9), (10), respectively:

$$311 \text{ Plastic viscosity (Pa.s) = } 0.119w/b + 0.035 SP + 0.015 \text{ flow time} + 0.001 SS - 0.390 \quad (7)$$

312

$$313 \text{ Yield stress (Pa) = } -113.362w/b + 12.586 SP - 3.324 \text{ spread diameter} - 0.041 SS + 189.386 \quad (8)$$

314

$$315 \text{ Flexural strength (MPa) = } -4.592w/b - 0.419 SP - 0.001 SS + 5.167 \quad (9)$$

316

$$317 \text{ Compressive strength (MPa) = } -17.560w/b - 1.152 SP - 0.01 SS + 20.765 \quad (10)$$

318 where w/b is the water/binder ratio, SP is the superplasticizer dosage (%), SS is the specific
319 surface area of NHL (m^2/kg) and flow time (sec.) and spread diameter (cm) are the results
320 obtained from experimental work i.e. Marsh cone and mini-cone test, respectively.

321

322 **Effects of water/binder ratio and superplasticizer dosage on plastic viscosity and yield**
323 **stress**

324 The proposed models can therefore be used to create contour diagrams showing the influence
325 of components on the properties that affect the fresh performance of NHL grouts. As shown
326 in eq. 7, the water/binder ratio has a higher effect than superplasticizer dosage over plastic
327 viscosity. Fig. 17 shows the effect of varying the superplasticizer dosage and flow time on the
328 grout plastic viscosity with water/binder ratio of 0.50. Graphically the contour lines show the
329 isoresponse of plastic viscosity when the superplasticizer dosage and flow time (obtained
330 from Marsh cone test) are known. For instance, as can be seen in Fig. 17, a grout made with
331 0.50 of water/binder ratio and 1% of superplasticizer can exhibit an increase in plastic
332 viscosity from 0.34 Pa.s to 0.48 Pa.s when the flow time increased from 10s to 20s. Note,
333 however, that this isoresponse plot is only valid for water/binder ratio of 0.50 and NHL5. As
334 far as yield stress is concerned, it is more significantly affected by the water/binder ratio than
335 the superplasticizer dosage as shown in eq. 8. The effect of varying the superplasticizer
336 dosage and spread diameter on yield stress values at water/binder ratio of 0.50 is shown in
337 Fig. 18. Assuming, for instance, a grout mix with a superplasticizer dosage of 0.8 % and a
338 spread diameter of 25 cm it is expected that the yield stress will have a value around 40.0 Pa.

339

340 **Effect of water/binder ratio and superplasticizer dosage on flexural and compressive**
341 **strength**

342 The effects of water/binder ratio and superplasticizer dosage on the grout mechanical strength
343 at 28 days are shown in eqs. 9 and 10. The water/binder ratio affects the flexural and
344 compressive strengths more significantly than the superplasticizer dosage. The effect of grout
345 components on the compressive and flexural strength are presented in Fig 19 and 20,
346 respectively. However, a trade-off between water/binder ratio and superplasticizer dosage

347 must be done. Since, higher dosages of water ($w/b > 0.50$) have a harmful effect on the
348 hardened grout properties as result of excessive mixing water. Moreover, it can be noticed
349 that higher superplasticizer dosages also reduce the grout mechanical strength, confirming the
350 negative effect of high superplasticizer dosage observed by other authors (Agull et al. 1999;
351 Banfill 2011; Hallal et al. 2010). Notwithstanding, the authors believe that a grout should
352 have high injectability and adequate mechanical properties, the latter are less determinant than
353 the former to get a successful grouting since the typical compressive stresses in old masonry
354 in the range of 1 MPa and most of the analyzed grout compositions are able to conform with
355 these values.

356

357 **Accuracy of the proposed models**

358 The accuracy of the developed models was determined by comparing predicted-to-measured
359 values obtained from ten random grout compositions (see table 4). The results of yield stress,
360 plastic viscosity, compressive and flexural strengths were then used to verify the ability of the
361 proposed models to predict these parameters. All tests were carried out under the same
362 conditions and procedures described earlier.

363

364 The maximum, minimum, average, standard deviation (SD) and coefficient of variation
365 (COV) of the predicted-to-measured values for yield stress, plastic viscosity, compressive and
366 flexural strengths for all the grout compositions tested are presented in table 5. The results
367 reveal that the predicted-to-measured ratio ranged from 0.97 to 1.08, indicating a good
368 agreement between the confirmation results and the expected results from the models. Thus, it
369 can be stated that all the proposed models together with simple test methods can be used in
370 the prediction as well as in the optimization of fresh and hardened performance of NHL
371 grouts.

372

373 **CONCLUSIONS**

374 This paper provides an approach that simplifies the whole methodology involved in NHL
375 grout design by reducing time and resources involved in rheological measurements, especially
376 when complex equipment is needed, such as rheometer or viscometer, which is not readily
377 available in every field. Concerning the results of this research, the following conclusions can
378 be warranted:

- 379 • The statistical modeling results indicate that the plastic viscosity, yield stress,
380 compression and flexural strength of the grouts studied are linearly related to
381 water/binder ratio, superplasticizer dosage and specific surface area of NHLs.
- 382 • The predicted-to-measured ratio ranged from 0.97 to 1.08, indicating a good
383 agreement between the confirmation results and the expected results from the models.
- 384 • The proposed statistical models together with classical flow tests can provide an
385 efficient mean to determine the grout composition that best fits the requirements of the
386 masonry to which such grouts is to be injected.
- 387 • It should be noted that the modeling was done based on a given set of materials and
388 assumptions which means that the generalization of the results should be considered
389 carefully. Therefore, further experimental research is needed to validate these
390 equations using different ranges of test parameters.

391

392

393 **Acknowledgements**

394 This paper is part of the research project PTDC/ECM/104376/2008, funded by FCT/MCTES,
395 Portugal. The authors of this paper wish to acknowledge the support from the strategic project Pest-
396 C/CTM/LA0025/2011, and the support of Dr João Pedro Veiga and Dr João Canejo for helping with
397 materials characterization.

398

399 **References**

- 400 Agull L., Toralles-Carbonari B., Gettu R. and Aguado A. 1999. "Fluidity of cement pastes with mineral
401 admixtures and superplasticizer- A study based on the Marsh cone test." *Mater. Struct.* 32:479–85.
- 402 Anderson T.W. 1984. "*An Introduction to Multivariate Statistical Methods*". 2^{sd} ed. John Wiley & Sons, New
403 York.
- 404 ASTM C494. 2005. "*Standard specification for chemical admixtures for concrete*". ASTM International, West
405 Conshohocken, PA, USA.
- 406 ASTM C939. 2002. "*Standard test method of flow of grout for preplaced-aggregate concrete (flow cone
407 method)*". ASTM International, West Conshohocken, PA, USA.
- 408 Baglioni P., Dei L., Pique F., Sarti G. and Ferroni E. 1997. "New autogenous lime-based grouts used in the
409 conservation of lime-based wall paintings." *Stud. Conserv.* 42:43–54.
- 410 Baltazar L.G., Henriques F.M.A. and Jorne F. 2012. "Optimisation of flow behaviour and stability of
411 superplasticized fresh hydraulic lime grouts through design of experiments." *Constr. Build. Mater.* 35:838–
412 45.
- 413 Baltazar L.G., Henriques F.M.A., Jorne F. and Cidade M.T. 2013. "The use of rheology in the study of the
414 composition effects on the fresh behaviour of hydraulic lime grouts for injection of masonry walls." *Rheol.
415 Acta* 52:127–38.
- 416 Baltazar L.G., Henriques F.M.A., Jorne F. and Cidade M.T. 2014. "Combined effect of superplasticizer, silica
417 fume and temperature in the performance of natural hydraulic lime grouts." *Constr. Build. Mater.* 50:584–
418 97.
- 419 Baltazar L.G., Henriques F.M.A. and Cidade M.T. 2015. "Contribution to the design of hydraulic lime-based
420 grouts for masonry consolidation." *J. Civ. Eng. Manag.* DOI:10.3846/13923730.2014.893918.
- 421 Banfill, P.F.G. 2006. "Rheology of fresh cement and concrete", *Rheol. Rev.* 61-130 (British Society of
422 Rheology).
- 423 Banfill P.F.G. 2011. "Additivity effects in the rheology of fresh concrete containing water-reducing admixtures."
424 *Constr. Build. Mater.* 25:2955–60.
- 425 Barnes H.A. 1997. "Thixotropy a review." *J. Non-Newtonian Fluid Mech.* 70:1-33.
- 426 Barnes H.A. 2000. "*A handbook of elementary rheology*." Institute of Non-Newtonian Fluid Mechanics,
427 University of Wales.

428 Binda L., Saisi A. and Tedeschi C. 2006. “*Compatibility of materials used for repair of masonry buildings:*
429 *research and applications.*” S.K. Kourkoulis ed. *Fracture and Failure of Natural Building Stone*: 167–82.

430 Bras A. 2011. “*Grout optimization for masonry consolidation.*” PhD Thesis. Universidade Nova de Lisboa,
431 Portugal.

432 Cardoso F.A., John V.M, Pileggi R.G. and Banfill P.F.G. 2014. “Characterisation of rendering mortars by
433 squeeze-flow and rotational rheometry.” *Cem. Concr. Res.* 57:79–87.

434 EN 1015-11. 1999. “*Methods of test for mortar for masonry. Determination of flexural and compressive strength*
435 *of hardened mortar*”. European Committee for Standardization.

436 EN 459-2. 2002. “*Building lime - Part 2: Test methods*”. European Committee for Standardization.

437 Fernàndez-Altàble V. and Casanova I. 2006. “Influence of mixing sequence and superplasticiser dosage on the
438 rheological response of cement pastes at different temperatures.” *Cem. Concr. Res.* 36:1222–30.

439 Flatt R.J. 1999. “*Interparticle forces and superplasticizers in cement suspensions.*” PhD Thesis École
440 Polytechnique Fédérale de Lausanne.

441 Hallal A., Kadri E.H., Ezziane K., Kadri A. and Khelafi H. 2010. “Combined effect of mineral admixtures with
442 superplasticizers on the fluidity of the blended cement paste.” *Constr. Build. Mater.* 24:1418–23.

443 Jorne F., Henriques F.M.A. and Baltazar L.G. 2014. “Injection capacity of hydraulic lime grouts in different
444 porous media” *Mater. Struct.* DOI 10.1617/s11527-014-0304-9.

445 Mauro A. and Felice G. 2012. “Seismic assessment multi-leaf masonry strengthen with injections or transversal
446 ties.” *Proc. of the 8th International Conference on Structural Analysis of historical Constructions, 15th-*
447 *17th October 2*:1873–79.

448 Miltiadiou A.E., 1990. “*Contribution a l’etude des coulis hydrauliques pour la reparation et le renforcement des*
449 *structures et des monuments historiques en maçonnerie*”. Doctoral de l’Ecole National de Ponts et
450 Chaussees, France.

451 Nehdi N. and Rahman M.A. 2004. “Estimating rheological properties of cement pastes using various rheological
452 models for different test geometry, gap and surface friction”. *Cem. Concr. Res.* 34: 1993-2007.

453 Roussel N., Stefani C. and Leroy R. 2005. “From mini-cone test to Abrams cone test: measurement of cement-
454 based materials yield stress using slump tests.” *Cem. Concr. Res.* 35:817–22.

455 Roussel N. 2007. “Rheology of fresh concrete: from measurements to predictions of casting processes.” *Mater.*
456 *Struct.* 40:1001–12.

- 457 Roussel N., Lemaître A., Flatt J.R. and Coussot P. 2010. "Steady state flow of cement suspensions: A
458 micromechanical state of the art." *Cem. Concr. Res.* 40:77–84.
- 459 Saak A.W., Jennings H.M. and Shah S.P. 2001. "The influence of wall slip on yield stress and viscoelastic
460 measurements of cement paste". *Cem. Concr. Res.* 31:205-12
- 461 Toumbakari, 2002. "*Lime-pozzolan-cement grouts and their structural effects on composite masonry walls.*"
462 PhD Thesis, Katholieke Universiteit Leuven, Belgium.

Table 1

[Click here to download Table: Table 1.doc](#)

	NHL3.5		NHL5	
Compression resistance at 28 days (MPa)	3.5		5.5	
Setting time	Start	1h	Start	2 h
	End	6h	End	6 h
Specific gravity	2.68 g/cm ³		2.73 g/cm ³	
Specific surface area B.E.T.	598 m ² /kg		480 m ² /kg	
Al ₂ O ₃	2.90		2.00	
CaO	82.00		85.00	
Fe ₂ O ₃	2.04		2.00	
MgO	1.10		1.00	
MnO	0.02		0.03	
SiO ₂	8.90		8.00	
SiC	0.03		0.01	
SO ₃	1.80		1.00	
SrO	0.06		0.05	
K ₂ O	0.83		0.70	

Table 2

[Click here to download Table: Table 2.doc](#)

Designation	water/binder (g/batch)	superplasticizer (g/batch)
G1	0.45 (1440)	0.6wt% (19.2)
G2	0.45 (1440)	0.8wt% (25.6)
G3	0.45 (1440)	1.2wt% (38.4)
G4	0.50 (1600)	0.6wt% (19.2)
G5	0.50 (1600)	0.8wt% (25.6)
G6	0.50 (1600)	1.2wt% (38.4)
G7	0.55 (1760)	0.6wt% (19.2)
G8	0.55 (1760)	0.8wt% (25.6)
G9	0.55 (1760)	1.2wt% (38.4)

Table 3

[Click here to download Table: Table 3.doc](#)

	Plastic viscosity	Yield stress	Flexural strength	Compressive strength
Intercept	-0.390	189.386	5.167	20.765
water/binder	0.119	-113.362	-4.592	-17.560
Superplasticizer	0.035	12.586	-0.419	-1.152
Specific surface area	0.001	-0.041	-0.001	-0.01
Flow time	0.015	-	-	-
Spread diameter	-	-3.324	-	-
R ²	0.96	0.90	0.89	0.86

Table 4

[Click here to download Table: Table 4.doc](#)

Designation	water/binder	Superplasticizer (wt%)	NHL type
G10	0.45	0.9	NHL3.5
G11	0.46	1.2	NHL3.5
G12	0.48	0.8	NHL3.5
G13	0.48	0.8	NHL5
G14	0.46	1.2	NHL5
G15	0.48	0.9	NHL5
G16	0.52	0.7	NHL3.5
G17	0.52	0.7	NHL5
G18	0.54	0.6	NHL3.5
G19	0.54	0.6	NHL5

Table 5

[Click here to download Table: Table 5.doc](#)

Property	Predicted-to-measured ratio				
	Maximum	Minimum	Average	SD	COV (%)
Yield stress	1.08	0.92	1.00	0.06	5.60
Plastic viscosity	1.12	0.82	0.97	0.12	12.30
Compressive strength	1.20	0.93	1.08	0.11	9.76
Flexural strength	1.18	0.96	1.06	0.06	6.12
COV=SD/average					

Table 1. Natural hydraulic limes characteristics

Table 2. Values of grout design variables for each NHL type

Table 3. Parameter estimated of the statistical models

Table 4 Random grout composition tested

Table 5 Performance of models in predicting the fresh and hardened properties of grouts

Fig 1

[Click here to download Figure: Fig. 1.pdf](#)

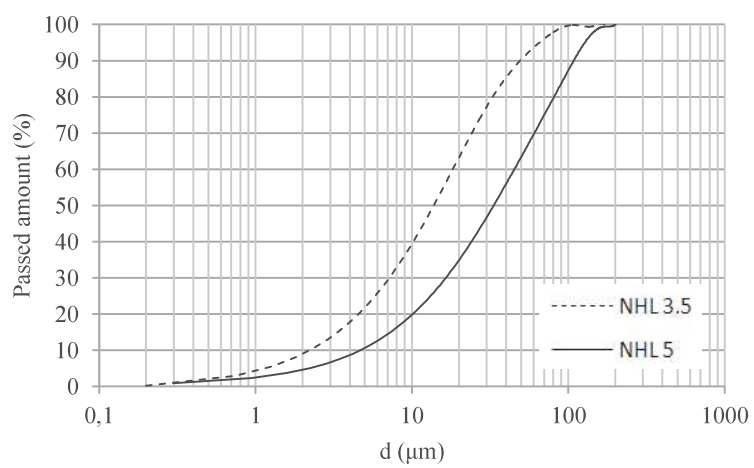


Fig 2
[Click here to download Figure: Fig. 2.pdf](#)



Fig 3(a)

[Click here to download Figure: Fig. 3\(a\).pdf](#)



Fig 3(b)

[Click here to download Figure: Fig. 3\(b\).pdf](#)

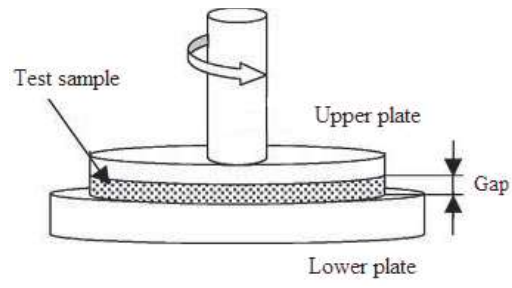


Fig 4

[Click here to download Figure: Fig. 4.pdf](#)

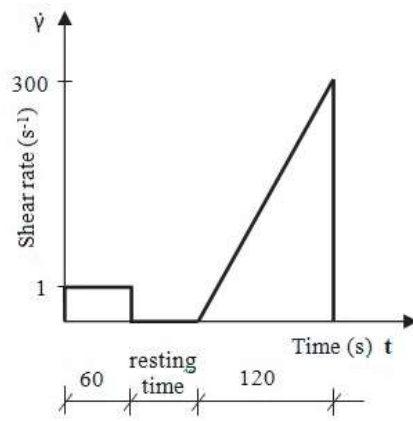


Fig 5
[Click here to download Figure: Fig. 5.pdf](#)

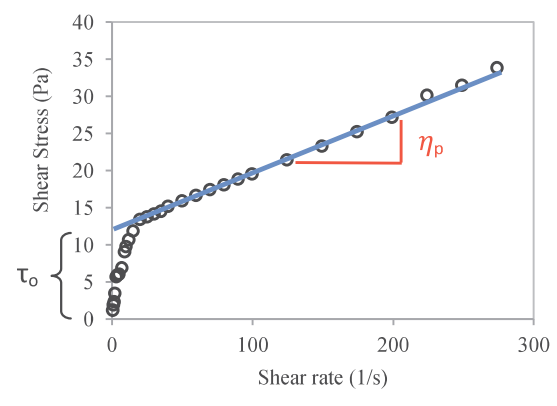


Fig 6
[Click here to download Figure: Fig. 6.pdf](#)

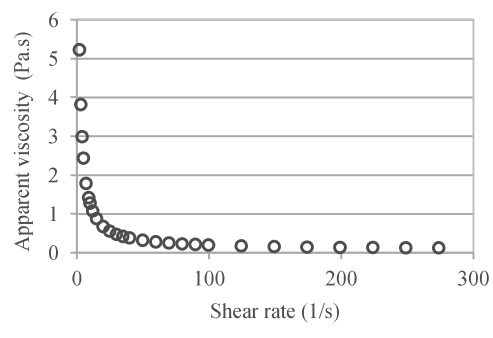


Fig 7

[Click here to download Figure: Fig. 7.pdf](#)

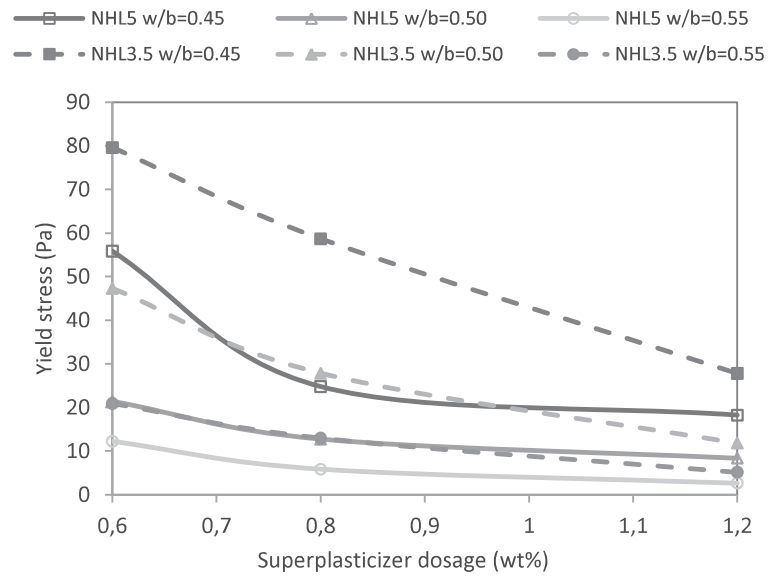


Fig 8

[Click here to download Figure: Fig. 8.pdf](#)

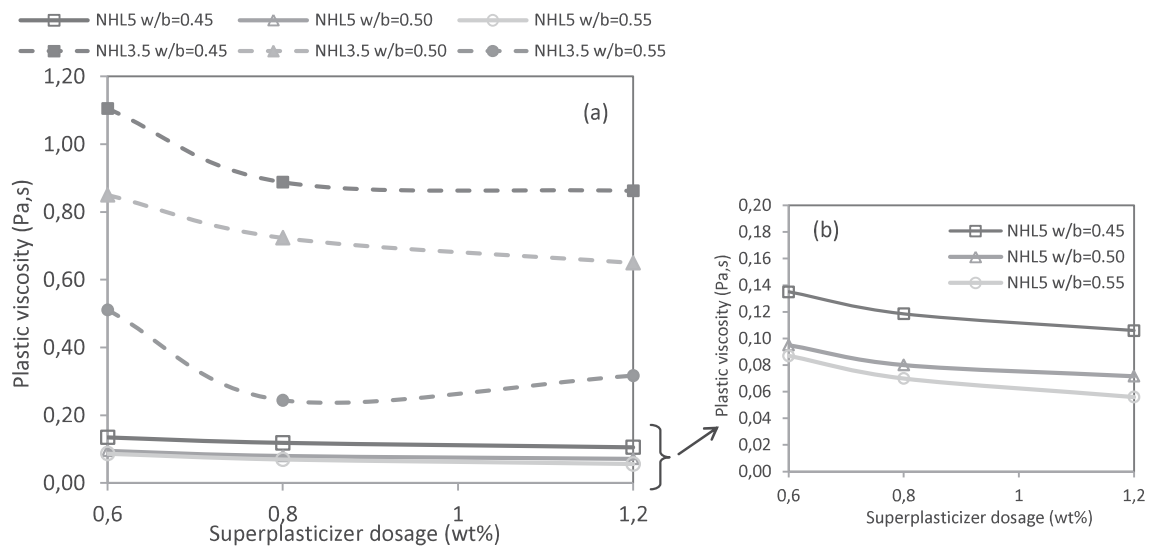


Fig 9

[Click here to download Figure: Fig. 9.pdf](#)

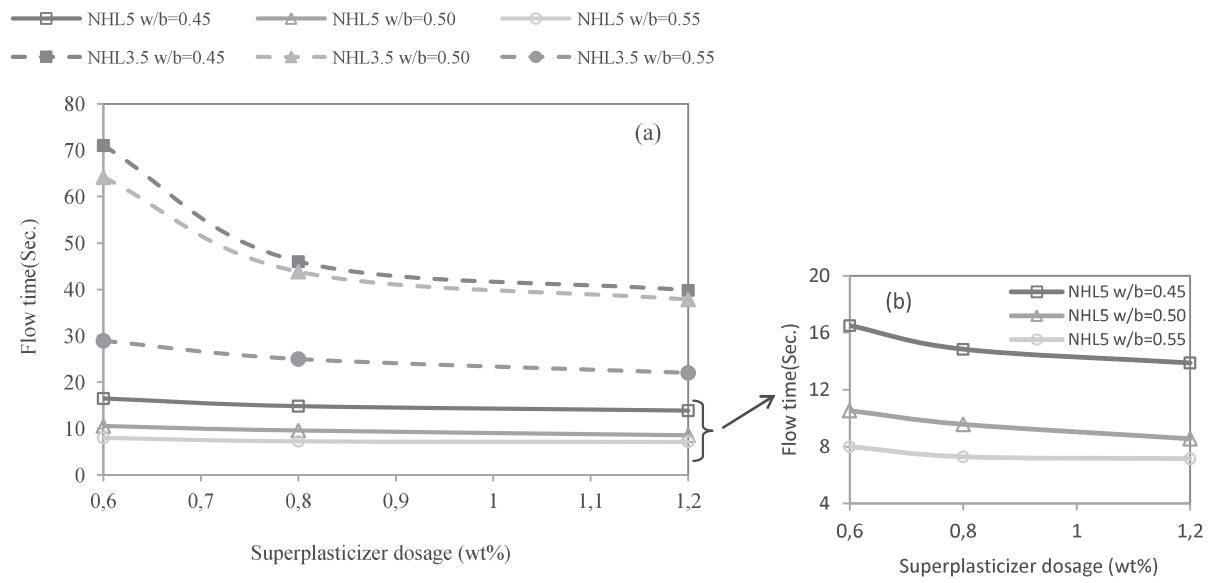


Fig. 10

[Click here to download Figure: Fig.10.pdf](#)

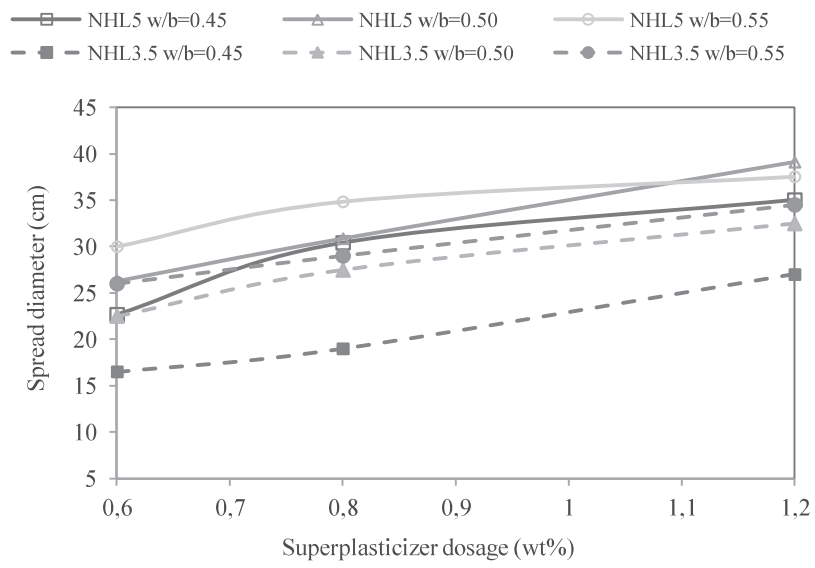


Fig 11

[Click here to download Figure: Fig. 11.pdf](#)

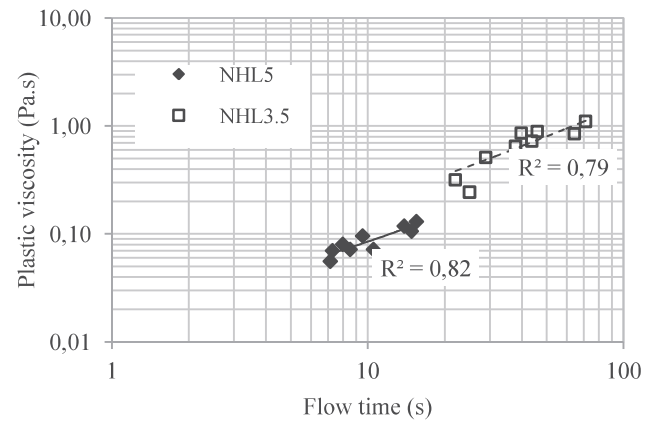


Fig 12

[Click here to download Figure: Fig. 12.pdf](#)

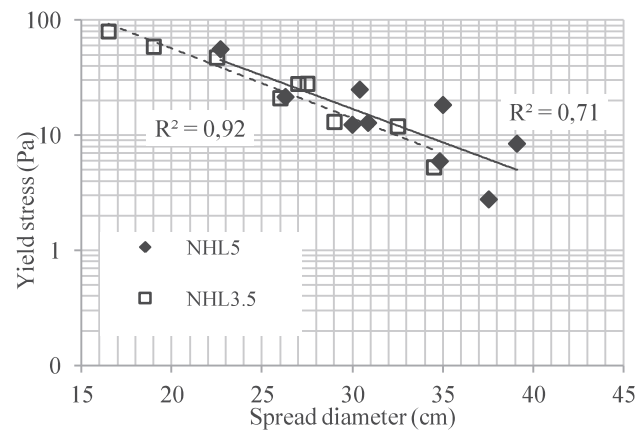


Fig 13

[Click here to download Figure: Fig. 13.pdf](#)

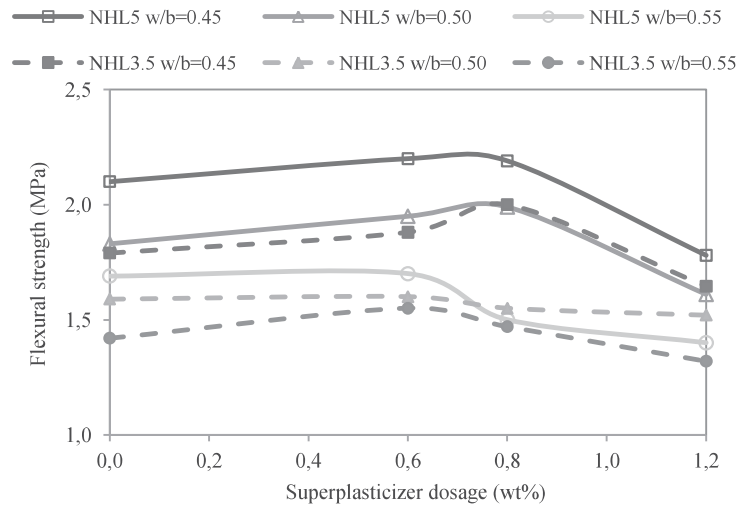


Fig 14

[Click here to download Figure: Fig. 14.pdf](#)

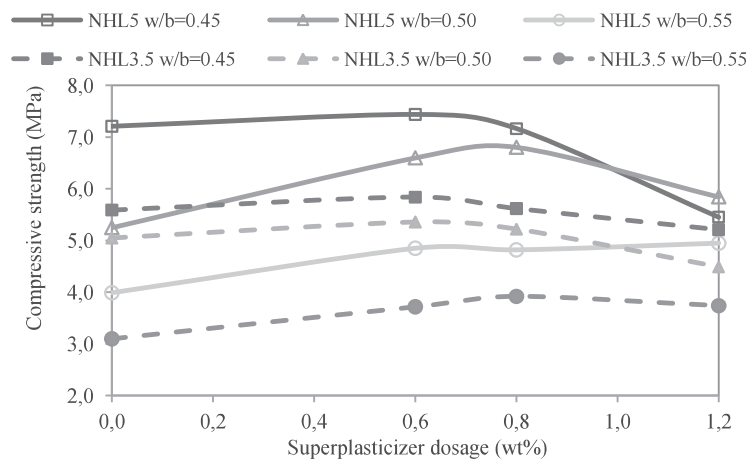


Fig 15(a)

[Click here to download Figure: Fig. 15 \(a\).pdf](#)

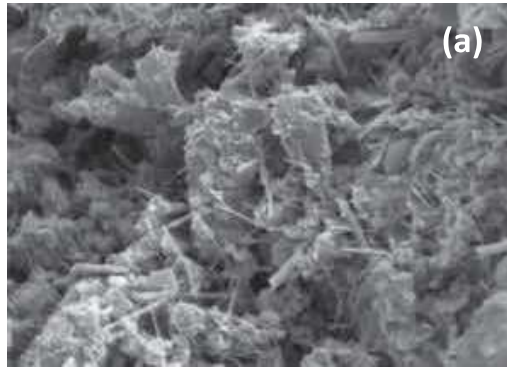


Fig 15(b)

[Click here to download Figure: Fig. 15 \(b\).pdf](#)

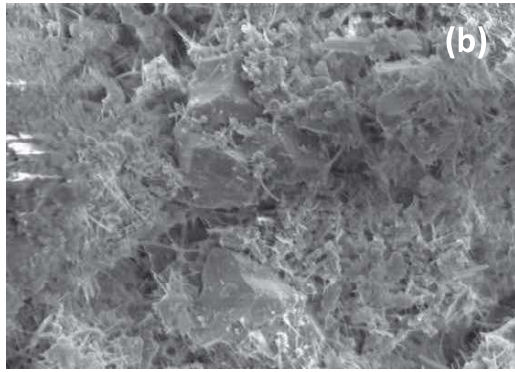


Fig 16

[Click here to download Figure: Fig. 16.pdf](#)

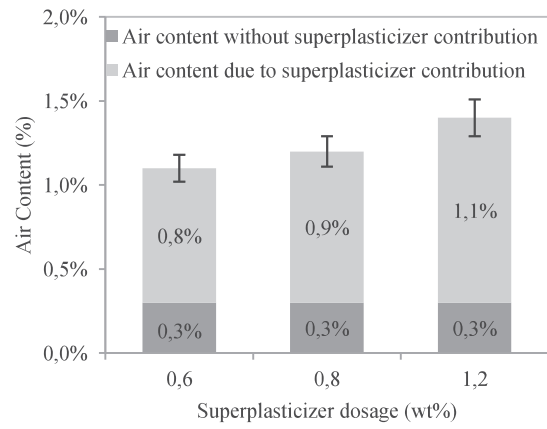


Fig 17
Click here to download Figure: Fig. 17.pdf

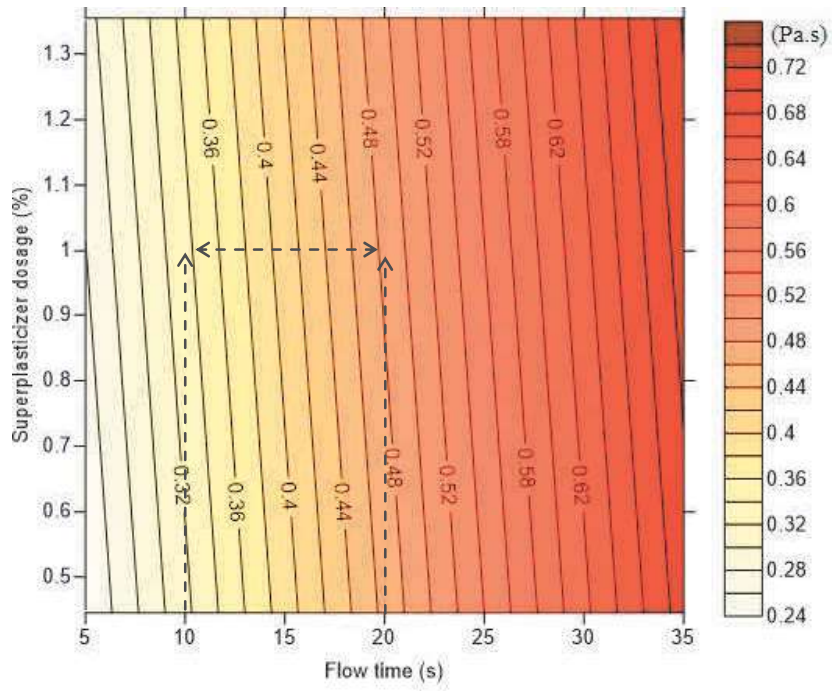


Fig 18
Click here to download Figure: Fig. 18.pdf

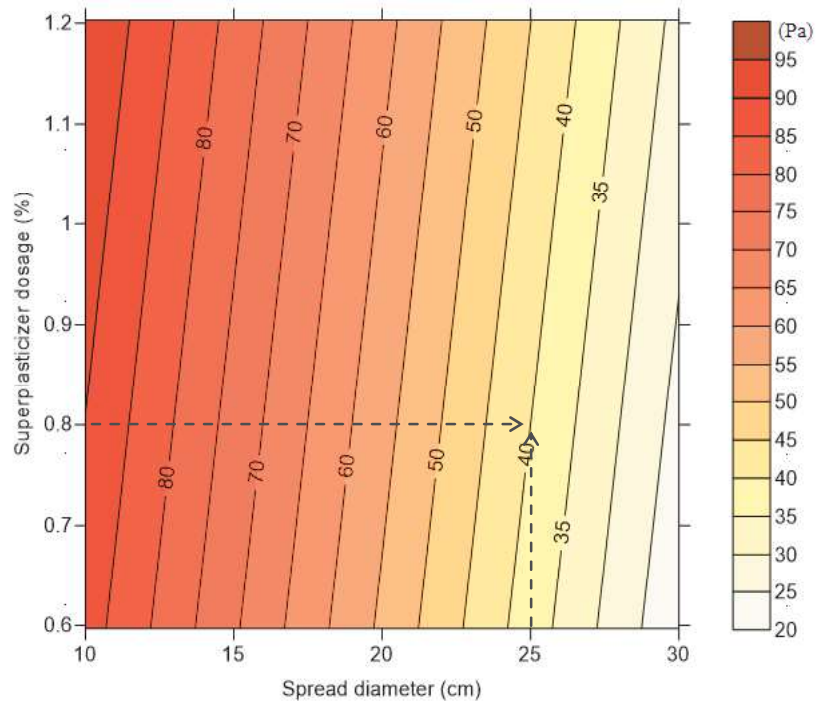


Fig 19

[Click here to download Figure: Fig. 19.pdf](#)

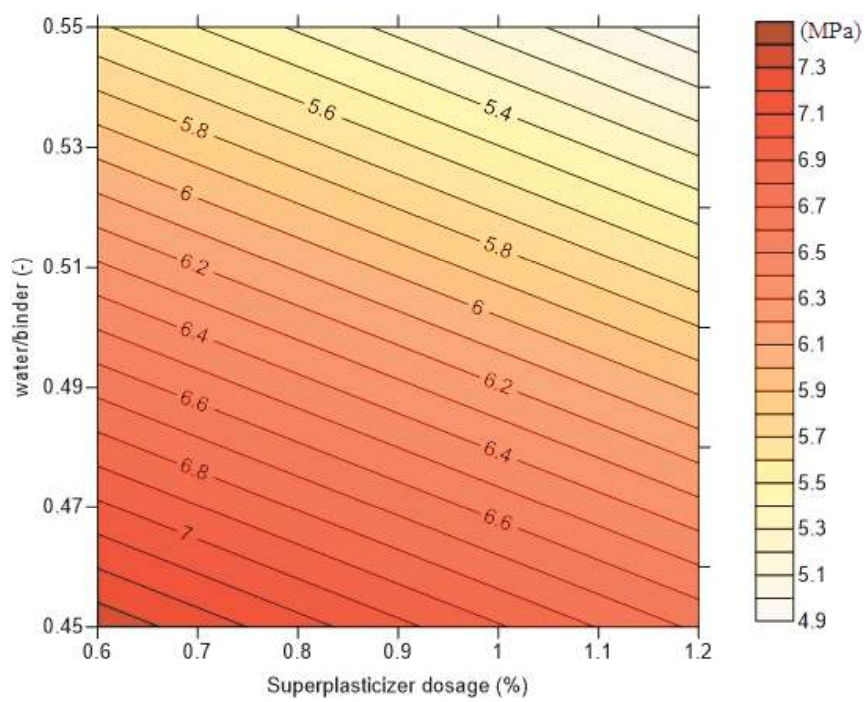


Fig 20
Click here to download Figure: Fig. 20.pdf

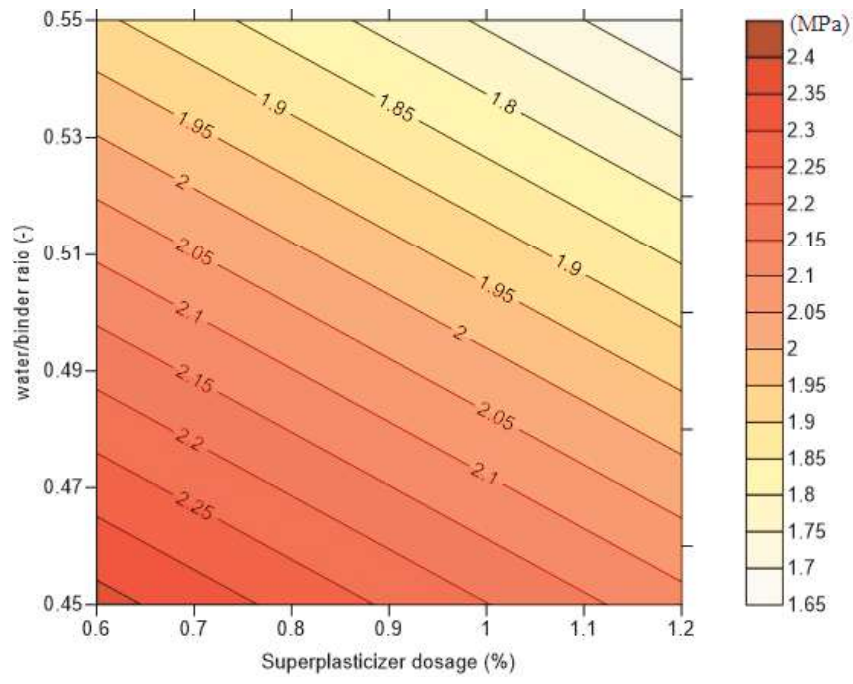


Figure 1 Grain size distribution of both NHL used

Figure 2. Mixer apparatus used in the experimental work

Figure 3. (a) geometry being loaded and (b) plate-plate geometry

Figure 4. Schematic representation of shear rate history adopted for rheological measurements

Figure 5. Shear stress versus shear rate for the NHL 5 grout with $w/b=0.50$ and $SP=0.8\%$

Figure 6. Apparent viscosity versus shear rate for the NHL 5 grout with $w/b=0.50$ and $SP=0.8\%$

Figure 7. Influence of water/binder ratio and superplasticizer dosage on yield stress

Figure 8. (a) Influence of water/binder ratio and superplasticizer dosage on plastic viscosity; (b) focus on NHL5 grouts

Figure 9. (a) Influence of water/binder ratio and superplasticizer dosage on flow time. (b) focus on NHL5 grouts

Figure 10. Influence of water/binder ratio and superplasticizer dosage on spread diameter

Figure 11. Correlation between plastic viscosity and flow time

Figure 12. Correlation between yield stress and spread diameter

Figure 13. Influence of water/binder ratio and superplasticizer dosage on grout flexural strength results at 28 days

Figure 14. Influence of water/binder ratio and superplasticizer dosage on grout compressive strength results at 28 days

Figure 15. SEM image of 7 days hardened grout (NHL5 + w/b=0.50) at 5000x: (a) without superplasticizer and (b) with superplasticizer

Figure 16. Effect of superplasticizer dosage in air content of fresh grout (NHL5+w/b=0.50)

Figure 17. Isoresponse lines for plastic viscosity (Pa.s) of NHL5 grouts with for water/binder ratio of 0.50

Figure 18. Isoresponse lines for yield stress (Pa) of NHL5 grouts with for water/binder ratio of 0.50

Figure 19. Isoresponse lines for compressive strength (MPa) of NHL5 grouts at 28 days

Figure 20. Isoresponse lines for flexural strength (MPa) of NHL5 grout at 28 days