



The permeability and elastic moduli of tuff from Campi Flegrei, Italy: implications for ground deformation modelling

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Abstract. The accuracy of ground deformation modelling at active volcanoes is a principal requirement in volcanic hazard mitigation. However, the reliability of such models relies on the accuracy of the rock physical property (permeability and elastic moduli) input parameters. Unfortunately, laboratory-derived values on representative rocks are usually rare. To this end we have performed a systematic laboratory study on the influence of pressure and temperature on the permeability and elastic moduli of samples from the two most widespread lithified pyroclastic deposits at the Campi Flegrei volcanic district, Italy. Our data show that the water permeability of Neapolitan Yellow Tuff and a tuff from the Campanian Ignimbrite differ by about 1.5 orders of magnitude. As pressure (depth) increases beyond the critical point for inelastic pore collapse (at an effective pressure of 10–15 MPa, or a depth of about 750 m), permeability and porosity decrease significantly, and ultrasonic wave velocities and dynamic elastic moduli increase significantly. Increasing the thermal stressing temperature increases the permeability and decreases the ultrasonic wave velocities and dynamic elastic moduli of the Neapolitan Yellow Tuff; whereas the tuff from the Campanian Ignimbrite remains unaffected. This difference is due to the presence of thermally unstable zeolites within the Neapolitan Yellow Tuff. For both rocks we also find, under the same pressure conditions, that the dynamic (calculated from ultrasonic wave velocities) and static (calculated from triaxial stress-strain data) elastic moduli differ significantly. The choice of elastic moduli in ground deforma-

tion modelling is therefore an important consideration. While we urge that these new laboratory data should be considered in routine ground deformation modelling, we highlight the challenges for ground deformation modelling based on the heterogeneous nature (vertically and laterally) of the rocks that comprise the caldera at Campi Flegrei.

1 Introduction

Monitoring ground deformation, the surface expression of deeper magmatic and/or hydrothermal activity, at active volcanoes is an important tool in volcanic hazard forecasting and mitigation. Ground deformation at a volcano (measured by global positioning system (GPS) satellites, interferometric synthetic aperture radar (InSAR), tiltmeters, or electronic distance metres (EDM)) are typically analysed using inverse problem models that consider a source (e.g. a magma chamber, a zone of overpressurized fluids, or a combination of the two) embedded within a homogenous elastic or viscoelastic half-space (e.g. Mogi, 1958; Dzurisin, 2006; Hurwitz et al., 2007). These models yield important information regarding the location, shape, and volume/pressure changes of the source. The accuracy of such modelling relies on the accuracy of the rock physical property input parameters (typically elastic moduli and permeability, depending on the type of model). Even small changes in the values of key controlling parameters can lead to large differences in the rate,

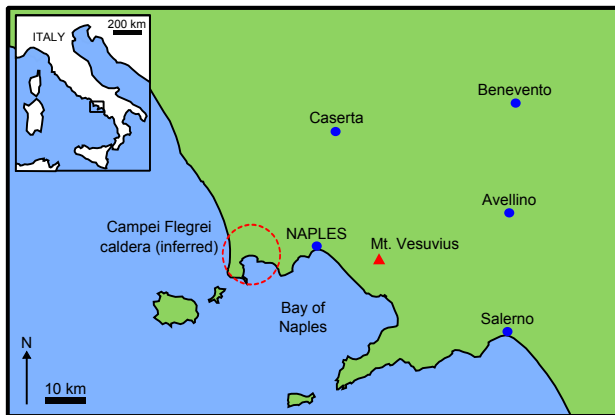


Fig. 1. Map showing the location of the inferred Campi Flegrei caldera and the proximity of Naples to both the Campi Flegrei caldera and Mt. Vesuvius. The Neapolitan Yellow Tuff used in this study was sourced from an open quarry within the inferred CF caldera at Monte San Severino (i.e. within the red circle), while the Grey Campanian Ignimbrite was sourced from an open quarry to the north-west of the town of Caserta (the blocks used in this study are the same as those used in Heap et al., 2012).

magnitude, and geometry of ground surface deformation (e.g. Hurwitz et al., 2007). For instance, a recent contribution using viscoelastic modelling to better understand flank motion and summit subsidence at Kīlauea (Hawai‘i) showed that deformation rates are enhanced when the elastic moduli input parameters are lowered (Plattner et al., 2013). Furthermore, homogenous half-space models, by definition, assume that all the rocks that comprise the volcano have identical physical properties. However, volcanoes are built from successive eruptive episodes and thus the physical properties of the rock strata that form the edifice are likely to span a wide range. For this reason, conventional homogenous half-space modelling at volcanoes has recently been considered an oversimplification that could lead to misinterpretation of the derived source parameters (Manconi et al., 2007, 2010). For instance, Manconi et al. (2010) showed that this “standard” approach can lead to inaccurate values for the source volume changes. Therefore, models that consider mechanical heterogeneities (e.g. Manconi et al., 2007, 2010) require a good knowledge of the breadth of elastic moduli that can be expected for representative rocks, and thermodynamic and magmatic-hydrothermal models (e.g. Hurwitz et al., 2007; Todesco et al., 2010) require accurate values of their permeability and elastic moduli. However, such laboratory data are commonly scarce or absent.

The densely populated (about 3 million) Neapolitan area, southern Italy, is in a state of constant threat provided by the proximity of Mt. Vesuvius and the increasingly restless Campi Flegrei (CF) volcanic district (Ricci et al., 2013; Fig. 1). The eruptive history of the CF volcanic district can be characterized by two major eruptions: (1) the eruption re-

lated to the emplacement of the Campanian Ignimbrite about 39 000 yr ago (De Vivo et al., 2001) and, (2) the eruption of the Neapolitan Yellow Tuff (NYT) about 15 000 yr ago (Deino et al., 2004); although the area has been volcanically active for more than 300 000 yr (Rolandi et al., 2003).

Today, the CF volcanic district is dominated by a resurgent, nested caldera (Fig. 1) that hosts a large, shallow (< 4 km) hydrothermal system (e.g. De Natale et al., 2006). The CF caldera is considered to have formed due to collapse following (1) both of the major eruptions (e.g. Barberi et al., 1991; Orsi et al., 1996) or, (2) the eruption of the NYT only (see Rolandi et al., 2003 and references therein). In the latter hypothesis, the Campanian Ignimbrite is thought to be the result of eruptive events originating from pre-existing neotectonic faults formed during the Apennine uplift (De Vivo et al., 2001; Rolandi et al., 2003). Although there has not been an eruption for almost 500 yr (since the Monte Nuovo eruption of 1538 AD), CF has become increasingly restless and is densely monitored by permanent seismic and ground deformation networks. In recent times, two major episodes of unrest have occurred, between 1969–1972 and 1982–1984 (Bianchi et al., 1987; Bonafede, 1991). Surface uplift, on the order of several metres (bradyseism), and accompanying earthquakes in 1984 led to the evacuation of the town of Pozzuoli. Since then, there has been an overall subsidence trend (e.g. see Fig. 2 in D’Auria et al., 2011), periodically interrupted by small (centimetre-scale) and short-lived (months) uplifts in 1989, 1994, 2000–2001 (e.g. Lanari et al., 2004; Bianco et al., 2004; D’Auria et al., 2011), and 2004–2006 (e.g. Saccorotti et al., 2007; Trasatti et al., 2008; D’Auria et al., 2011). However, the interpretation of long-term and short-term ground deformation patterns at CF is a matter of debate (see De Natale et al., 2001, and De Natale et al., 2006, for reviews on the topic). Models to explain the origin of the uplift can be broadly divided into two camps: those that consider solely the input of magma at depth (e.g. Berrino et al., 1984; Bonafede et al., 1986; Bianchi et al., 1987) and those that invoke an interaction between magma and fluids (magmatic-hydrothermal models and thermodynamic models; e.g. Bonafede, 1991; Gaeta et al., 1998; Bonafede and Mazzanti, 1998; De Natale et al., 2001; Lundgren et al., 2001; Troise et al., 2001; Gaeta et al., 2003; Chiodini et al., 2003; Battaglia et al., 2006; Gottsmann et al., 2006; Troise et al., 2007; Bodnar et al., 2007; Lima et al., 2009; Todesco et al., 2010; D’Auria et al., 2011; Troiano et al., 2011; Chiodini et al., 2012). The latter category can be broken down further into models that require the input of fresh magma from depth (e.g. Gaeta et al., 1998) and those that consider magma body cooling and concomitant crystallization (e.g. Bodnar et al., 2007; Lima et al., 2009). Other models account for the surface deformation by invoking an interaction between the pressure source and caldera boundary fractures (e.g. De Natale and Pingue, 1993; Beauducel et al., 2004) or mechanical heterogeneities (e.g. Manconi et al., 2010).

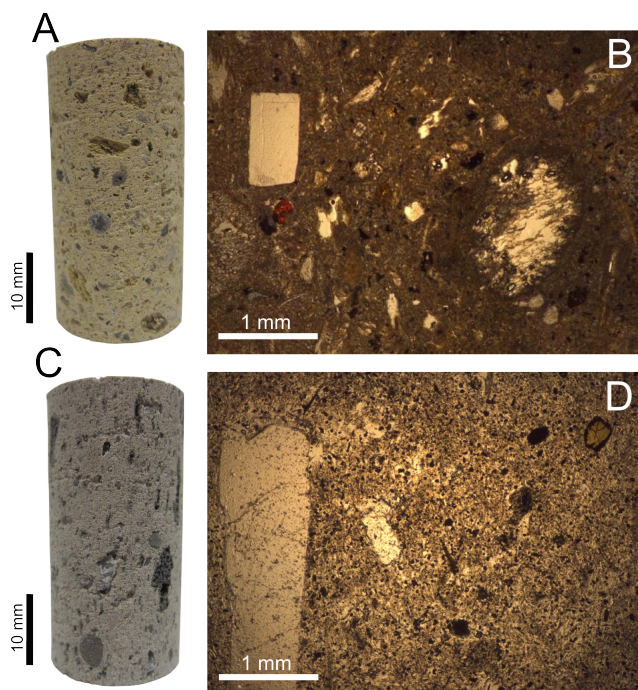


Fig. 2. Photographs and optical microscopy images of the as-collected Neapolitan Yellow Tuff (A and B) and Grey Campanian Ignimbrite (C and D). The photomicrographs are taken from Heap et al. (2012).

While we note that the goal of this contribution is not to critically review the numerous models invoked to explain the ground deformation at CF, we highlight that the accuracy of all these models relies on accuracy of the rock physical property input parameters. Unfortunately, published laboratory investigations on the physical properties of representative materials from the CF caldera are rare. Values of permeability have, thus far, either been inferred from in situ observations (Rosi and Sbrana, 1987) or have been taken from experiments conducted on NYT under ambient pressure conditions (Ascolese et al., 1993a, b; Peluso and Arienzo, 2007). In the most recent study, Peluso and Arienzo (2007) measured the permeability of NYT at ambient pressure to be between 2.0×10^{-15} and 6.3×10^{-17} m² (the range of porosity was between 48 and 52 %). However, not only are the deposits within the CF caldera present at depth (which is likely to severely influence their permeability), but it is known that the permeability of lithified pyroclastic deposits can be highly variable (a variety of representative materials should therefore be measured), depending on their degree of lithification (Vinciguerra et al., 2009). We also highlight that permeability data of borehole samples are presented in an open access report (Giberti et al., 2006). This report shows, for the San Vito 1 borehole, that the permeability can range from 10^{-13} m² at the surface to 10^{-16} m² at a depth of almost 3000 m.

Elastic moduli are generally assumed, or extrapolated from seismic tomography studies (e.g. Chiarabba and Moretti, 2006; Vinciguerra et al., 2006). Typically, Poisson's ratio is taken as 0.3 and shear modulus as 5 GPa (e.g. De Natale et al., 1991). However, dynamically-determined elastic moduli (i.e. those calculated from ultrasonic wave velocities) may not represent the most appropriate values to use in volcano ground deformation modelling. Deformation caused by a volcanic source proceeds quasi-statically rather than dynamically and therefore static elastic moduli are likely to be the most appropriate input parameters (Heap et al., 2009; Manconi et al., 2010). It is well known that dynamic and static moduli differ as a result of the large differences in the frequency at which they are measured (Simmons and Brace, 1965; Cheng and Johnston, 1981; Eissa and Kazi, 1989; Ciccotti and Mulargia, 2004; Ciccotti et al., 2004). Static elastic moduli for representative materials from CF are not yet available (Manconi et al., 2010).

The pyroclastic deposits that comprise the caldera at CF are exposed to elevated temperatures, as evidenced by two-dimensional conductive/convective numerical modelling (Wohletz et al., 1999), seismic attenuation tomography (de Lorenzo et al., 2001), and infrared imaging (Chiodini et al., 2007). Surface geothermal gradients of about 150–200 °C km⁻¹ are estimated (for the first 1.5 km) and, at the edge of the hydrothermal system, a temperature of 420 °C was measured at a depth of 3 km (AGIP borehole San Vito 1, de Lorenzo et al., 2001). It has been shown previously that thermal stresses can increase the permeability (e.g. Homand-Etienne and Troalen, 1984; Jones et al., 1997; David et al., 1999; Nara et al., 2011) and decrease the Young's modulus (e.g. Keshavarz et al., 2010) of rock. This is usually interpreted as being a consequence of the formation of new microcracks due to the build-up of internal thermal stresses. Volcanic rocks are persistently challenged by elevated temperatures due to their proximity to large permanent heat sources, and the fluctuations in temperature caused by the movement of magma, and are therefore especially prone to thermal microcracking. Furthermore, many fine-grained pyroclastic deposits can be further jeopardized by high temperatures due to the presence of thermally unstable zeolites (Heap et al., 2012). Since zeolitization promoted lithification, the loss of zeolites can impose dramatic consequences on rock physical properties. Recent data has shown that NYT becomes structurally unstable upon exposure to high (100–750 °C) temperatures, resulting in a severe decrease in both tensile and compressive strength (Heap et al., 2012). A recent contribution by Manconi et al. (2010) highlighted the need for the evaluation of the temperature-dependence of the material properties of the rocks at CF.

For the reasons outlined above we have conducted a systematic study of the influence of pressure and temperature on the physical properties (permeability, porosity, ultrasonic velocities, and elastic moduli) of two lithified pyroclastic deposits (one zeolitized) from CF. We first present

Table 1. Summary of the ambient pressure, as-collected physical properties of NYT and WGI. “Dry” indicates measurements on samples that were dried in a vacuum oven for at least 24 h; the measurements were then performed under ambient laboratory humidity. “Wet” indicates measurements on samples that were vacuum-saturated with distilled water.

	Neapolitan Yellow Tuff (NYT)	Grey Campanian Ignimbrite (WGI)
connected porosity [%]	43.8	48.5
dry bulk sample density [kg m^{-3}]	1270	1330
dry P wave velocity [kms^{-1}]	2.29	2.31
wet P wave velocity [kms^{-1}]	2.60	2.56
dry S wave velocity [kms^{-1}]	1.25	1.28
wet S wave velocity [kms^{-1}]	1.30	1.33
dry V_P/V_S	1.84	1.80
wet V_P/V_S	2.00	1.93
dry dynamic Young’s modulus [GPa]	5.07	5.58
wet dynamic Young’s modulus [GPa]	7.68	8.42
dry dynamic Poisson’s ratio	0.28	0.28
wet dynamic Poisson’s ratio	0.33	0.31
dry dynamic shear modulus [GPa]	1.97	2.19
wet dynamic shear modulus [GPa]	2.88	3.20
dry unconfined compressive strength [MPa] (from Heap et al., 2012)	3.47	9.23

the investigated materials and methods. We then present our experimental results before discussing our data in terms of ground deformation modelling at CF.

2 Materials investigated

Our experiments were performed on samples of NYT and Grey Campanian Ignimbrite (WGI), sampled from the two most abundant and widespread volcanic deposits in the CF volcanic district. NYT was sourced from an open quarry within the inferred CF caldera at Monte San Severino (i.e. within the red circle in Fig. 1), while the WGI was sourced from an open quarry to the north-west of the town of Caserta (the blocks used in this study are the same as those used in Heap et al., 2012). In this paper we refer to both lithified pyroclastic rocks as “tuffs”.

NYT and WGI contain average connected porosities of 44 and 49 %, respectively (measured using the triple weight water saturation technique; Guéguen and Palciauskas, 1994). We note that, although our samples are small compared to the volume of the natural deposits, a report by Giberti et al. (2006) showed that the porosity of 12 and 125 cm^3 samples were very similar, for a wide range of material from CF. Photographs and optical microscopy photomicrographs of the samples are provided as Fig. 2 and their ambient pressure, “as-collected” (i.e. “natural” samples that have undergone no heating or deformation) physical properties are listed in Table 1. NYT (Fig. 2a, b), a trachytic pyroclastic deposit characterized by both pyrogenic and authigenic phases (de

Gennaro et al., 2000), contains phenocrysts of sanidine, plagioclase, clinopyroxene, biotite, and minor amounts of Ti-magnetite and apatite within a matrix of pumiceous lapilli and glassy ash (glass shards and blocky shaped glass fragments). X-ray diffraction pattern analysis has indicated the presence of phillipsite, chabazite, and analcime (Heap et al., 2012). The mean content of these zeolites in NYT can exceed 50 % (e.g. de Gennaro et al., 1990, 2000). WGI (Fig. 2c and d), feldspathized by authigenic mineralization processes, is made up of reversely graded black scoriae embedded in an ashy matrix with subordinate lithics and crystals (Cappelletti et al., 2003; Langella et al., 2013). WGI contains hypidiomorphic phenocrysts of alkali-feldspars with minor amounts of clinopyroxene, as well as microlites of alkali-feldspar, Ti-magnetite and apatite. The matrix comprises well-sorted glass shards with occasional accretionary ash clots and porous lapilli fragments (Heap et al., 2012, and references therein). Although WGI does not contain any zeolites, we note that portions of the Campanian Ignimbrite are pervasively zeolitized (e.g. see Langella et al., 2013).

3 Experimental methods

As mentioned above, the caldera at CF hosts a large, shallow (< 4 km) hydrothermal system (e.g. De Natale et al., 2006). Indeed, laboratory studies have demonstrated that the water-saturated ultrasonic velocities of tuffs from CF are more representative of the in situ values than “dry” (measurements conducted on oven dried samples at ambient laboratory

Table 2. Summary of the estimated measurement accuracy.

measurement	accuracy
confining pressure [Pa]	$\pm 100\,000$ (UCL) $\pm 10\,000$ (Strasbourg)
pore fluid pressure [Pa]	$\pm 10\,000$
pore fluid volume [m ³]	$\pm 1.0 \times 10^{-12}$
LVDT displacement [m]	± 0.000001
axial stress [Pa]	$\pm 10\,000$
original sample dimensions [m]	± 0.00001

humidity) ultrasonic velocities (Zamora et al., 1994; Vanorio et al., 2005; Vinciguerra et al., 2006). Since the tuffs of CF are present at depth, and are likely to contain a fluid phase (e.g. a mixture of meteoric water and seawater contaminated by rising magmatic gases, see Valentino et al., 1999), we consider experimental values on pressurized, water-saturated samples as the most representative. Our experimental program was twofold. (1) Hydrostatic (i.e. $\sigma_1 = \sigma_2 = \sigma_3$) experiments to measure changes in permeability, porosity, ultrasonic wave velocities, and dynamic elastic moduli with increasing effective pressure (P_{eff} , from 5 to 50 MPa) on samples that had been thermally stressed to a range of temperatures (from as-collected to 1000 °C). (2) Constant strain rate conventional triaxial (i.e. $\sigma_1 > \sigma_2 = \sigma_3$) deformation experiments at a P_{eff} of 5 MPa to measure static elastic moduli. Importantly, we measure both static and dynamic elastic moduli at the same P_{eff} (= 5 MPa) so that the values can be easily compared. All our experiments were performed at room temperature (while this may not accurately represent the natural case, we note that, to explore the influence of temperature on the physical properties of the tuffs, we conducted experiments on samples thermally stressed to a range of temperatures).

Experimental data are subject to error as a result of the accuracy of the various transducers. Estimations of the accuracy of the measurements of this study are listed in Table 2. The errors are extremely small and lead to error bars that are smaller than the data points in the figures provided in this study. However, we note that measurement errors are dwarfed by the natural sample variability of the tuffs (i.e. the natural variability of samples cored from the same block of material). Estimations of the natural sample variability of the tuffs used this study are provided in Table 3. While one of the goals of this contribution is to demonstrate the variability of different tuffs from the CF volcanic district, we strived to minimize the variability between samples cored from the same block by (1) coring many samples and selecting those within a strict porosity range, (2) discarding samples with obvious, large heterogeneities and, (3) discarding samples with anomalous P wave velocities. Using these sample selection guidelines, our experiments under different conditions (dif-

Table 3. Expected natural variability between tuff samples cored from the same block. Note that these are not “errors” in the measurements. Measurement accuracies (Table 2) are insignificant compared to the natural sample variability, despite efforts to reduce the variability between samples cored from the same block of material (see text for details).

	expected natural variability
Young’s modulus [GPa]	± 0.5
Poisson’s ratio	± 0.05
shear modulus [GPa]	± 0.5
water permeability [m ²]	$\pm 1.0 \times 10^{-14}$
P wave velocity [km s ⁻¹]	± 0.1
S wave velocity [km s ⁻¹]	± 0.1

ferent thermal stressing temperatures and pressures) can be compared with the greatest confidence.

3.1 Hydrostatic experiments

Hydrostatic experiments were performed in the Rock & Ice Physics Laboratory (RIPL) at University College London (UCL) using a 300 MPa hydrostatic pressure vessel equipped with two 70 MPa servo-controlled pore fluid intensifiers or volumeters (Fig. 3, see also Kolzenburg et al., 2012). The apparatus is designed to measure permeability, porosity, and ultrasonic wave velocities contemporaneously. In our experiments we chose an experimental pressure range of 5–50 MPa (i.e. up to a depth of about 3.5 km).

Cylindrical samples, 25 mm in diameter and nominally 40 mm in length, were all cored from the same set of blocks and in the same orientation. Samples were precision ground so that their end faces were flat and parallel. Prior to experimentation, samples were either (1) held at ambient temperature (as-collected) or, (2) thermally stressed to pre-determined temperatures of 100, 200, 300, 500, 750, or 1000 °C (note: NYT could not be tested after exposure to 1000 °C due to a severe volume reduction). Thermal stressing was achieved by heating the sample to the target temperature at a rate of 1 °C/min, holding the temperature constant for 60 min, and then cooling at the same rate. Once at room temperature, all samples were vacuum-saturated in distilled water prior to experimentation. The measured sample was then inserted into a nitrile rubber jacket and fixed between the two endcaps. The sample assembly was then lowered into the pressure vessel. Once inside the setup, the confining pressure (P_c) and the pore fluid (distilled water) pressures (P_p) in both the “upstream” (P_{up}) and “downstream” (P_{down}) pore volumeters were increased to 10 and 5 MPa, respectively. The confining and pore pressures were increased slowly to avoid damaging the sample, and care was taken to ensure the sample was not pressurized beyond the maximum effective pressure targeted for the experiments (5 MPa). For the purpose of this study we apply the simple effective pressure

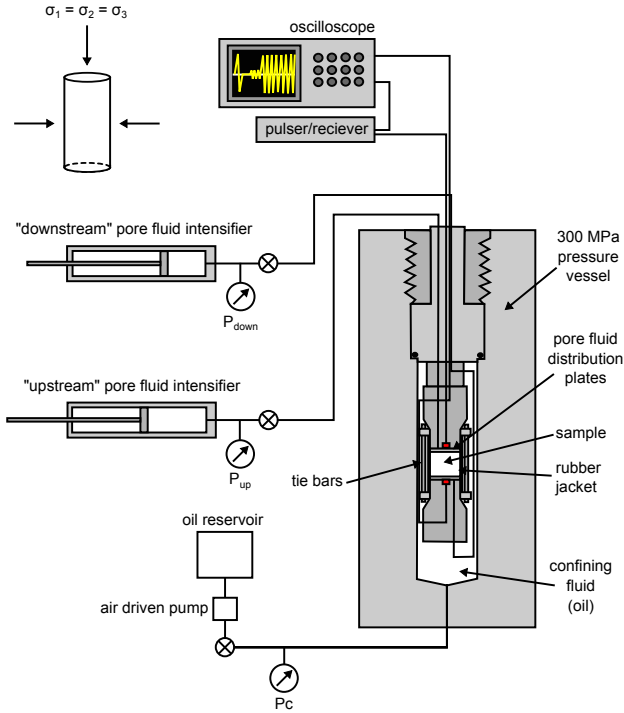


Fig. 3. Schematic diagram of the permeameter at the Rock & Ice Physics Laboratory (RIPL), University College London. Schematic is not to scale.

law of $P_{\text{eff}} = P_c - \alpha P_p$, assuming that poroelastic constant $\alpha = 1$ (Guéguen and Palciauskas, 1994). The sample was left for 30 min at an effective pressure of 5 MPa to ensure microstructural equilibration and complete saturation.

Once equilibration at $P_{\text{eff}} = 5$ MPa was complete, the first ultrasonic measurements were taken. Ultrasonic waves velocities were measured via PZT (lead-zirconate-titanate) piezoelectric P and S wave transducer crystals housed in the sample endcaps (Fig. 3) using an Agilent Technologies 1.5 GHz “Infiniium” digital storage oscilloscope and a JSR DPR300 35 MHz ultrasonic pulser/receiver. All ultrasonic wave arrival times were individually picked as the first deviation from the baseline signal. Dynamic elastic moduli were calculated from the resultant ultrasonic wave velocities using the following formulae (Guéguen and Palciauskas, 1994):

$$E_d = \rho \frac{V_S^2 (3V_P^2 - 4V_S^2)}{V_P^2 - V_S^2}, \quad (1)$$

$$v_d = \frac{V_P^2 - 2V_S^2}{2(V_P^2 - V_S^2)}, \quad (2)$$

$$\mu_d = \frac{E_d}{2(1 + v_d)}. \quad (3)$$

where E_d is Young’s modulus, v_d is Poisson’s ratio, μ_d is the shear modulus, ρ is the bulk sample density and V_P and V_S are the P and S wave velocities, respectively.

Water permeability measurements were made by imposing a 1 MPa pressure difference across the jacketed sample. To achieve this, P_{up} and P_{down} were set at 4.5 and 5.5 MPa, respectively. The volumeters were then allowed to move full stroke (10 cm^3) and steady-state flow was only assumed when the flow rate was constant over a protracted period. Water permeability (κ_{water}) was then calculated directly from Darcy’s law:

$$\frac{Q}{A} = \frac{\kappa_{\text{water}}}{\eta L} (P_{\text{up}} - P_{\text{down}}), \quad (4)$$

where Q is the fluid volume flux, A is the cross-sectional area of the sample, η is the viscosity of the pore fluid (taken as $8.94 \times 10^{-4} \text{ Pa}\cdot\text{s}$), L is the length of the sample, and P_{up} and P_{down} are the pore pressures at the “upstream” and “downstream” ends of the sample, respectively.

Once the permeability measurement was complete, the “downstream” volumeter was isolated and the “upstream” volumeter was set back at 5 MPa. The P_c was then slowly increased to 15 MPa. By monitoring the movement of the “upstream” volumeter the porosity change from $P_{\text{eff}} = 5$ MPa to $P_{\text{eff}} = 10$ MPa could be accurately calculated. The sample was then left for 30 min at the new pressure to ensure microstructural equilibration. Once equilibration was complete, the ultrasonic measurements for $P_{\text{eff}} = 10$ MPa were taken. This procedure was repeated for every 5 MPa P_{eff} increment up to 50 MPa.

During our experiments, the length of the sample L and the cross-sectional area A will change due to the compaction of the sample at elevated pressure. We have corrected for this (in our calculations of permeability and ultrasonic wave velocities) using the volume reduction of our sample (as measured by the water expelled from the sample) at each pressure interval, assuming isotropic compaction. Although one sample was used per thermal-stressing temperature, we reiterate that great care was taken during sample selection to exclude samples that contained large heterogeneities or anomalous connected porosities/ P wave velocities.

3.2 Triaxial deformation experiments

Constant strain rate ($1.0 \times 10^{-5} \text{ s}^{-1}$) conventional (i.e. $\sigma_1 > \sigma_2 = \sigma_3$) triaxial experiments were performed on as-collected cylindrical samples of the two tuffs (20 mm in diameter and nominally 40 mm in length). Samples were cored from the same blocks and in the same direction as for the hydrostatic experiments described in the previous section. The samples were precision ground so that their end faces were flat and parallel. Both experiments were performed in the conventional triaxial deformation apparatus (Fig. 4) at the Laboratoire de Déformation des Roches (Université de Strasbourg) at a P_{eff} of 5 MPa (P_p of 5 MPa and a P_c of 10 MPa). Axial stress and strain were monitored continuously using a load cell and an LVDT displacement transducer, respectively. Pore volume change (used as a proxy for volumetric strain,

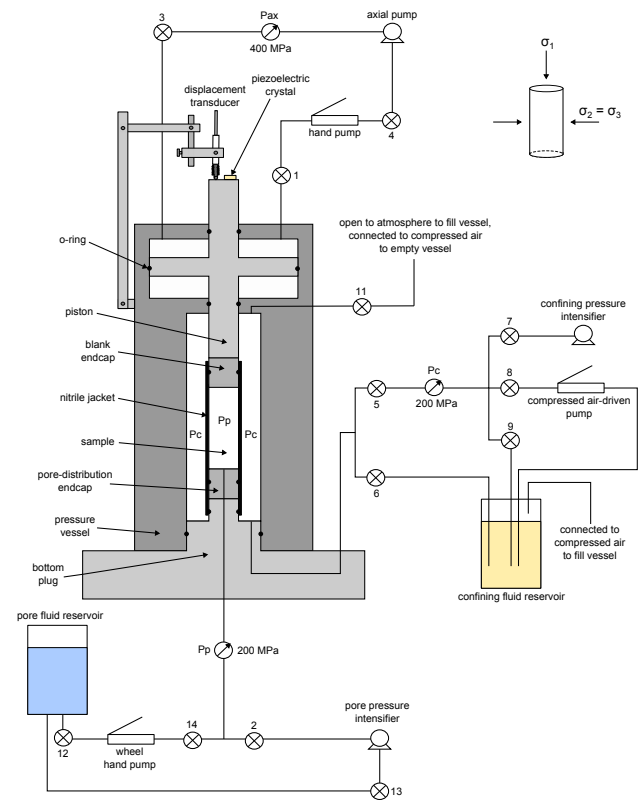


Fig. 4. Schematic diagram of the triaxial deformation apparatus at the Laboratoire de Déformation des Roches, Université de Strasbourg. Schematic is not to scale.

ε_v) was monitored using a pore pressure intensifier, and the output of acoustic emissions (AEs) by a piezoelectric transducer crystal (located on the top of the piston) using a Physical Acoustics USB AE Node. AEs are high frequency elastic wave packets generated by the rapid release of strain energy such as during brittle microfracturing (see Lockner, 1993 for a review). During experimentation, an AE hit was recorded if a signal exceeded the set threshold of 40 dB. The AE “energy” (the area under the received AE waveform envelope) of each received AE signal was provided by the AEwin software. In this study we will adopt the convention that compressive stresses and compactive strains are positive.

Static Young’s moduli (E_s) and Poisson’s ratio (ν_s) were then calculated from the resultant stress-strain data, following the method of Heap and Faulkner (2008). We take both from the quasi-linear elastic regions of our tangent modulus curves (i.e. those regions where the moduli did not change). Static Poisson’s ratio is given by

$$\nu_s = -\frac{\varepsilon_r}{\varepsilon_a}, \quad (5)$$

where

$$\varepsilon_r = \frac{\varepsilon_v - \varepsilon_a}{2}. \quad (6)$$

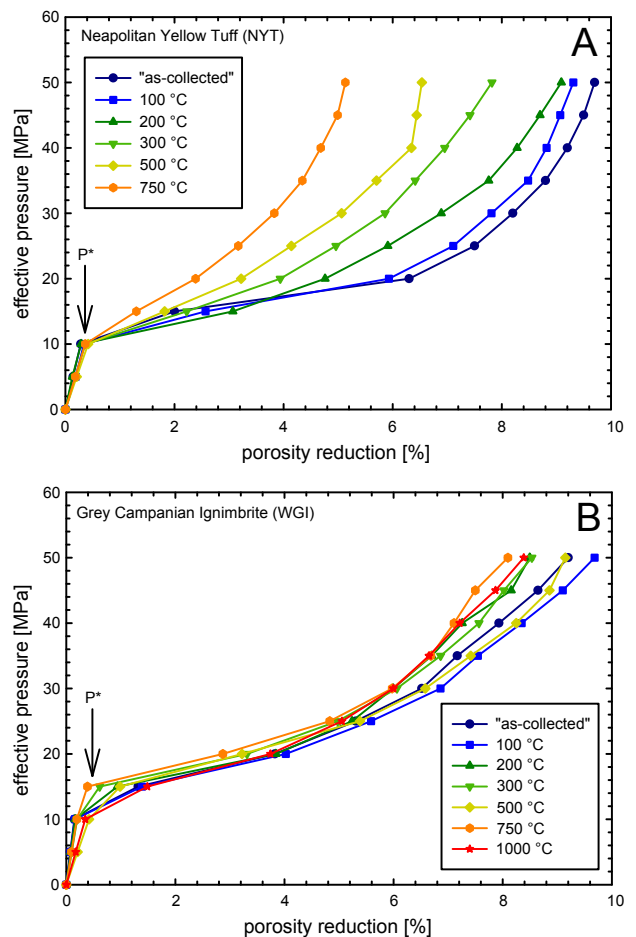


Fig. 5. The evolution of porosity change with increasing effective pressure for Neapolitan Yellow Tuff (A) and Grey Campanian Ignimbrite (B). The temperatures in the legend refer to the thermal stressing temperature (see text for details). Note that the porosity reduction on the x axis is not a relative change.

where ε_r and ε_a are the radial and axial strain, respectively. Static shear modulus (μ_s) was then calculated using the following formula (Guéguen and Palciauskas, 1994):

$$\mu_s = \frac{E_s}{2(1 + \nu_s)}. \quad (7)$$

3.3 Microstructural analyses

Microstructural analyses were performed using (1) the Hitachi S-3600N Environmental Scanning Electron Microscope (E-SEM) at the University of Leicester using a working distance of 14.3 mm and an accelerating voltage of 15 kV and, (2) a Leica DM2500 (equipped for both transmitted and reflected light) microscope with a mounted 5 megapixel Leica DFC425 digital camera (at the Laboratoire de Déformation des Roches, Université de Strasbourg). The E-SEM was used to look for evidence of pore collapse in samples of NYT

taken beyond P^* (the critical pressure for the onset of inelastic compaction). Optical microscopy was used to investigate the influence of high temperatures (1000 °C) on the microstructure of NYT and WGI.

4 Results

4.1 The evolution of porosity with increasing pressure and temperature

Plots of the evolution of porosity with increasing P_{eff} (commonly called “hydrostats”) for both NYT and WGI at each thermal stressing temperature are displayed in Fig. 5. For porous rock, an increase in hydrostatic pressure results in a volume and porosity decrease. Initially, this compaction is elastic (i.e. recoverable) but, at some critical pressure (assuming the rock is porous enough), pore collapse and grain crushing (now non-recoverable damage) ensues and the rate of compaction accelerates. This critical pressure is denoted P^* (Wong and Baud, 2012). The P_{eff} required to reach P^* varies from rock to rock, but depends largely on the initial rock porosity and grain size (generally, the higher the porosity, the lower the P_{eff} for P^*). The stress at which P^* occurs can therefore provide important information on the physical and microstructural state of rock at depth. In our experiments, the position of P^* is about 10 MPa for NYT (Fig. 5a) and about 10–15 MPa for WGI (Fig. 5b). Prior to P^* , during elastic compaction, we note that the porosity change is linear (i.e. there is no concave portion that is usually attributed to the closure of microcracks; however this may be a result of the large steps in P_{eff} between measurements). We note that there is no microstructural evidence for microcracks in the as-collected materials (see Fig. 2). Immediately following P^* , during inelastic compaction, there is a dramatic increase in the rate of porosity reduction, as inelastic compaction proceeds. However, the porosity reduction rate then gradually decreases (especially above about 20 MPa). This represents the hardening of the rock due to compaction. Over the entire pressure range (up to 50 MPa) the porosity change for the as-collected sample is about the same for NYT and WGI (between 9 and 10 %). Figure 5b also shows that the porosity evolution for WGI with increasing P_{eff} is unaffected by thermal stressing. By contrast, in the case of NYT, the porosity change decreases significantly as thermal stressing temperature increases (Fig. 5a). It can also be seen that, for both tuffs, thermal stressing does not appear to influence the position of P^* (Fig. 5).

4.2 The evolution of permeability with increasing pressure and temperature

The evolution of permeability with increasing P_{eff} for NYT and WGI at each thermal stressing temperature is displayed in Fig. 6 (the values are reported in Tables 4 and 5). Firstly, we notice that the as-collected permeabilities of the two sam-

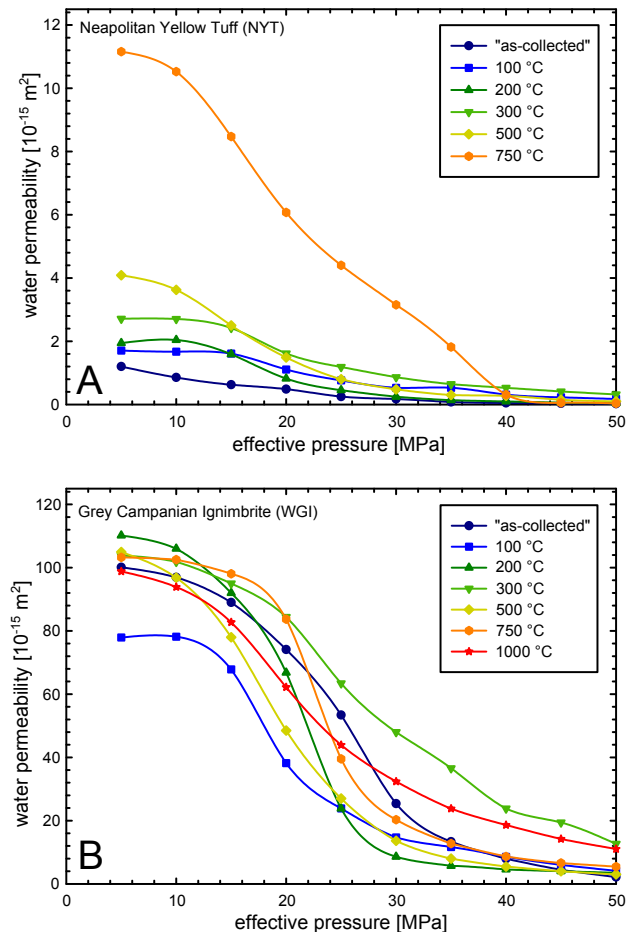


Fig. 6. The evolution of water permeability with increasing effective pressure for Neapolitan Yellow Tuff (A) and Grey Campanian Ignimbrite (B). The temperatures in the legend refer to the thermal stressing temperature (see text for details).

ples are very different. For instance, at a P_{eff} of 5 MPa, the permeabilities are about 1.0×10^{-15} and $1.0 \times 10^{-13} \text{ m}^2$ for NYT (Fig. 6a) and WGI (Fig. 6b), respectively.

For WGI, the permeability curves show little change between 5 and 15 MPa (Fig. 6b). However, above 15 MPa, the permeability starts to decrease rapidly before reaching an apparent plateau above about 30 MPa. We note that this rapid decrease starts at the same pressure as the onset of inelastic compaction (P^*). Overall, the permeability is reduced by about an order of magnitude from $1.0 \times 10^{-13} \text{ m}^2$ at 5 MPa to $1.0 \times 10^{-14} \text{ m}^2$ at 50 MPa. The permeability curves for WGI show no clear trend with increasing thermal stressing temperature (Fig. 6b). The different values obtained for the different thermal stressing temperatures are within the expected range of natural variability between different samples cored from the same block (see Table 3).

However, there is a clear influence of the thermal stressing temperature on the permeability of NYT (Fig. 6a). At a P_{eff}

Table 4. Water permeability of NYT as a function of effective pressure and thermal stressing temperature.

Neapolitan Yellow Tuff (NYT)						
effective pressure [MPa]	“as-collected” permeability [m ²]	100 °C permeability [m ²]	200 °C permeability [m ²]	300 °C permeability [m ²]	500 °C permeability [m ²]	750 °C permeability [m ²]
5	1.2×10^{-15}	1.7×10^{-15}	1.9×10^{-15}	2.7×10^{-15}	4.1×10^{-15}	1.1×10^{-14}
10	8.5×10^{-16}	1.7×10^{-15}	2.0×10^{-15}	2.7×10^{-15}	3.6×10^{-15}	1.1×10^{-14}
15	6.3×10^{-16}	1.6×10^{-15}	1.6×10^{-15}	2.4×10^{-15}	2.5×10^{-15}	8.5×10^{-15}
20	4.9×10^{-16}	1.1×10^{-15}	8.2×10^{-16}	1.6×10^{-15}	1.5×10^{-15}	6.1×10^{-15}
25	2.5×10^{-16}	7.6×10^{-16}	4.5×10^{-16}	1.2×10^{-15}	8.0×10^{-16}	4.4×10^{-15}
30	1.7×10^{-16}	5.3×10^{-16}	2.5×10^{-16}	8.6×10^{-16}	4.8×10^{-16}	3.2×10^{-15}
35	8.0×10^{-17}	5.3×10^{-16}	1.4×10^{-16}	6.4×10^{-16}	3.0×10^{-16}	1.8×10^{-15}
40	4.7×10^{-17}	3.1×10^{-16}	9.6×10^{-17}	5.3×10^{-16}	2.7×10^{-16}	3.1×10^{-16}
45	3.5×10^{-17}	2.3×10^{-16}	6.0×10^{-17}	4.1×10^{-16}	1.5×10^{-16}	5.4×10^{-17}
50	2.4×10^{-17}	1.7×10^{-16}	4.0×10^{-17}	3.2×10^{-16}	1.0×10^{-16}	3.3×10^{-17}

Table 5. Water permeability of WGI as a function of effective pressure and thermal stressing temperature.

Grey Campanian Ignimbrite (WGI)							
effective pressure [MPa]	“as-collected” permeability [m ²]	100 °C permeability [m ²]	200 °C permeability [m ²]	300 °C permeability [m ²]	500 °C permeability [m ²]	750 °C permeability [m ²]	1000 °C permeability [m ²]
5	1.0×10^{-13}	7.8×10^{-14}	1.1×10^{-13}	1.0×10^{-13}	1.0×10^{-13}	1.0×10^{-13}	9.9×10^{-14}
10	9.7×10^{-14}	7.8×10^{-14}	1.1×10^{-13}	1.0×10^{-13}	9.7×10^{-14}	1.0×10^{-13}	9.4×10^{-14}
15	8.9×10^{-14}	6.8×10^{-14}	9.2×10^{-14}	9.5×10^{-14}	7.8×10^{-14}	9.8×10^{-14}	8.2×10^{-14}
20	7.4×10^{-14}	3.8×10^{-14}	6.7×10^{-14}	8.4×10^{-14}	4.9×10^{-14}	8.4×10^{-14}	6.2×10^{-14}
25	5.3×10^{-14}	2.4×10^{-14}	2.4×10^{-14}	6.3×10^{-14}	2.7×10^{-14}	4.0×10^{-14}	4.4×10^{-14}
30	2.5×10^{-14}	1.5×10^{-14}	8.6×10^{-15}	4.8×10^{-14}	1.4×10^{-14}	2.0×10^{-14}	3.2×10^{-14}
35	1.3×10^{-14}	1.2×10^{-14}	5.7×10^{-15}	3.7×10^{-14}	8.0×10^{-15}	1.2×10^{-14}	2.4×10^{-14}
40	7.9×10^{-15}	8.6×10^{-15}	4.6×10^{-15}	2.4×10^{-14}	5.5×10^{-15}	8.7×10^{-15}	1.9×10^{-14}
45	4.5×10^{-15}	6.0×10^{-15}	4.0×10^{-15}	1.9×10^{-14}	4.0×10^{-15}	6.6×10^{-15}	1.4×10^{-14}
50	2.2×10^{-15}	4.2×10^{-15}	3.5×10^{-15}	1.3×10^{-14}	3.0×10^{-15}	5.5×10^{-15}	1.1×10^{-14}

of 5 MPa, the permeability increases from $1.0 \times 10^{-15} \text{ m}^2$ for the as-collected sample to $1.1 \times 10^{-14} \text{ m}^2$ for the sample thermally stressed to 750 °C, an increase of an order of magnitude. As for the WGI, the permeability curves show little change between 5 and 10 MPa, after which permeability decreases more rapidly. We again note that this rapid decrease starts at the same pressure as the onset of inelastic compaction (P^*). Over the entire pressure range, the permeability is reduced by about an order of magnitude for the as-collected sample and by about three orders of magnitude for the sample thermally stressed to 750 °C (the total decrease in permeability increases with increasing thermal stressing temperature, Fig. 6a). The permeability curves all converge at about 40 MPa (at a permeability of about $4.0 \times 10^{-17} \text{ m}^2$). Therefore, at P_{eff} s of 40 MPa and above, there is no longer any influence of thermal stressing on the permeability of NYT.

4.3 The evolution of ultrasonic velocities and dynamic elastic moduli with increasing pressure and temperature

The evolution of the physical properties (ultrasonic wave velocities, dynamic elastic moduli, and V_P/V_S ratio) for NYT and WGI are shown in Figs. 7 and 8, respectively. Firstly, it can be remarked that the as-collected physical properties of the two tuffs are similar (see also Table 1). For both tuffs, P and S wave velocity (Figs. 7a, b; 8a, b), dynamic Young’s modulus (Figs. 7c, 8c), dynamic Poisson’s ratio (Figs. 7d, 8d), dynamic shear modulus (Figs. 7e, 8e), and V_P/V_S ratio (Figs. 7f, 8f) all increase with increasing P_{eff} , and in a similar manner. For example, for the as-collected NYT sample, P wave velocity increases by 40 % (Fig. 7a), S wave velocity by 21 % (Fig. 7b), Young’s modulus by 53 % (Fig. 7c), Poisson’s ratio by 19 % (Fig. 7d), shear modulus by 47 % (Fig. 7e), and V_P/V_S ratio by 15 % (Fig. 7f) over the entire pressure range (5–50 MPa). The relative increases are

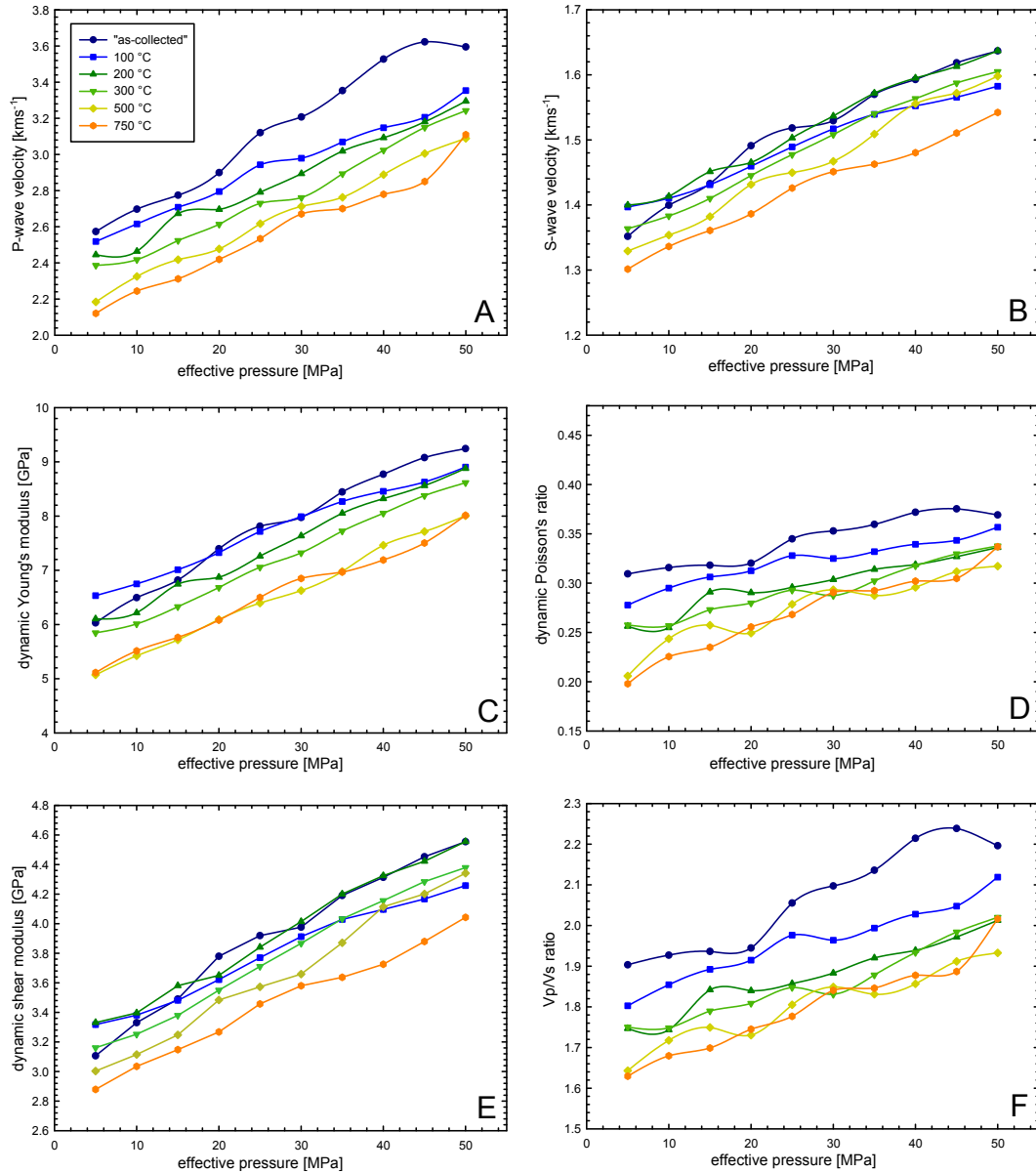


Fig. 7. The evolution of P wave velocity (A), S wave velocity (B), dynamic Young's modulus (C), dynamic Poisson's ratio (D), dynamic shear modulus (E), and V_P/V_S ratio (F) with increasing effective pressure for Neapolitan Yellow Tuff. The temperatures in the legend refer to the thermal stressing temperature (see text for details).

similar for both tuffs. However, whereas the results for NYT (Fig. 7) show a systematic decrease in all the physical properties with increasing thermal stressing temperature, no systematic pattern can be discerned in the WGI results (Fig. 8). At a constant P_{eff} , thermal stressing decreases P and S wave velocity, dynamic Young's modulus, dynamic Poisson's ratio, and V_P/V_S ratio in NYT. For example, for NYT at a P_{eff} of 5 MPa, P wave velocity decreases by 21 % (Fig. 7a), S wave velocity by 4 % (Fig. 7b), Young's modulus by 18 % (Fig. 7c), Poisson's ratio by 56 % (Fig. 7d), shear modulus

by 8 % (Fig. 7e), and V_P/V_S ratio by 17 % (Fig. 7f) over the entire temperature range (as-collected to 750 °C).

4.4 Static elastic moduli under triaxial conditions

The differential stress–axial strain curves and associated AE energy output curves for the triaxial experiments are shown in Fig. 9, and the differential stress–porosity reduction curves are shown in Fig. 10. Even at a P_{eff} as low as 5 MPa, the deformation behaviour of the two tuffs can be described as macroscopically ductile (i.e. their ability to resist load did not decrease, see Rutter, 1986). For both rocks, a critical

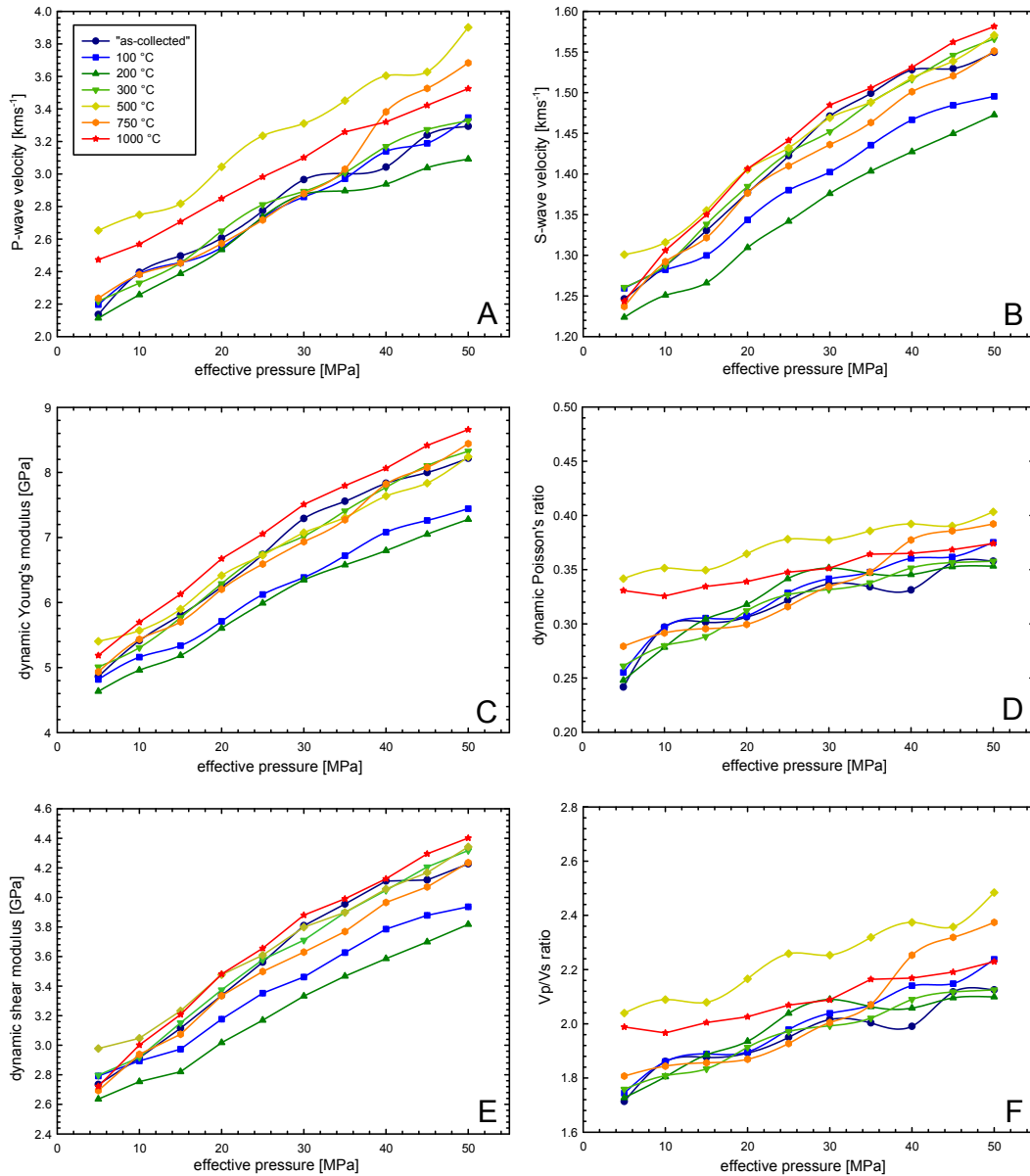


Fig. 8. The evolution of P wave velocity (A), S wave velocity (B), dynamic Young’s modulus (C), dynamic Poisson’s ratio (D), dynamic shear modulus (E), and V_P/V_S ratio (F) with increasing effective pressure for Grey Campanian Ignimbrite. The temperatures in the legend refer to the thermal stressing temperature (see text for details).

pressure, termed C^* (Wong and Baud, 2012), is reached and marks the point where there is an acceleration in axial strain (Fig. 9) and porosity reduction (or volumetric strain, Fig. 10) for a given stress increment. This phenomenon is called “shear-enhanced compaction”, and beyond C^* the rocks are deforming in the compactive, cataclastic flow regime which, in this case, is associated with strain hardening. In our experiments, C^* occurs at differential stresses of about 4 and 7 MPa for NYT (Figs. 9a, 10a) and WGI (Figs. 9b, 10b), respectively. This contrasts with the values for P^* of 10 and 15 MPa, respectively, and demonstrates how the appli-

cation of shear stresses enhances compactive deformation. Although this mode of failure differs greatly from the brittle failure seen in the uniaxial experiments of Heap et al. (2012) on the same rocks, both deformation mechanisms involve the same micromechanical process: microcracking (as evidenced by the output of AE energy; a proxy for microcracking). However, whereas strain localization is seen in the brittle field, cataclastic flow involves distributed microcracking (i.e. localization does not occur). Indeed, we see no evidence for strain localization in the post-experimental samples. The output of AE energy is seen to increase in a somewhat stepwise

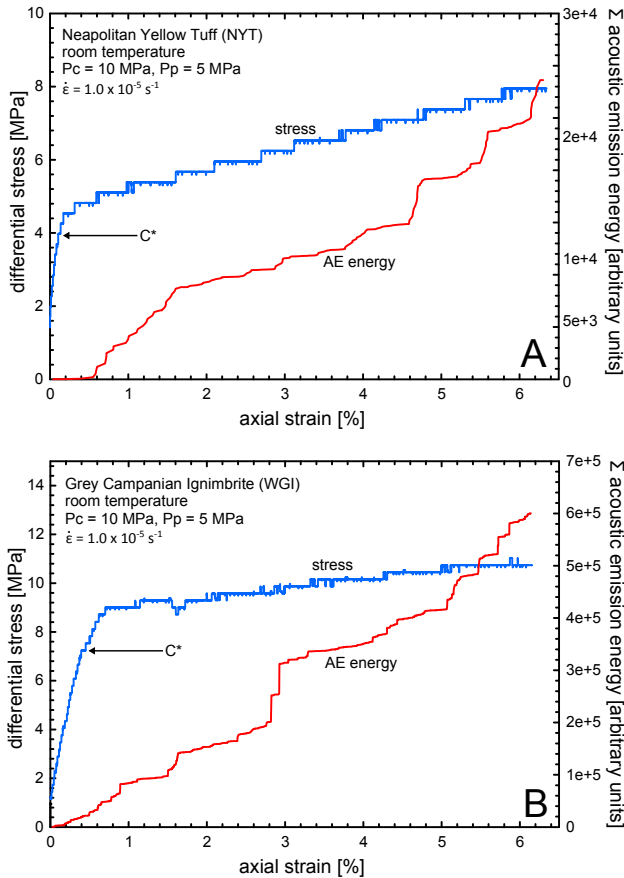


Fig. 9. Constant strain rate stress–strain curves, together with the cumulative output of AE “energy” (the area under the received AE waveform envelope) for as-collected Neapolitan Yellow Tuff (A) and Grey Campanian Ignimbrite (B). The experimental conditions are provided on each panel and the positions of C^* are indicated by the arrows. The steps in the data are due to the stepwise nature of the pumps.

manner for both rocks (Fig. 9), reflecting bursts of microcracking events during deformation, we note that the average rate of AE energy output for WGI is some 20 times higher than for NYT. The difference in AE energy output during deformation is likely to be the result of the compositional differences between the two tuffs.

Values for the static Young’s modulus, static Poisson’s ratio, and static shear modulus were calculated from the elastic portions of the stress–strain curves and are given in Table 6, together with dynamic values determined at the same pressure ($P_{eff} = 5$ MPa) for comparison. We note that both the static Young’s modulus and the static shear modulus are significantly lower than the corresponding dynamic values.

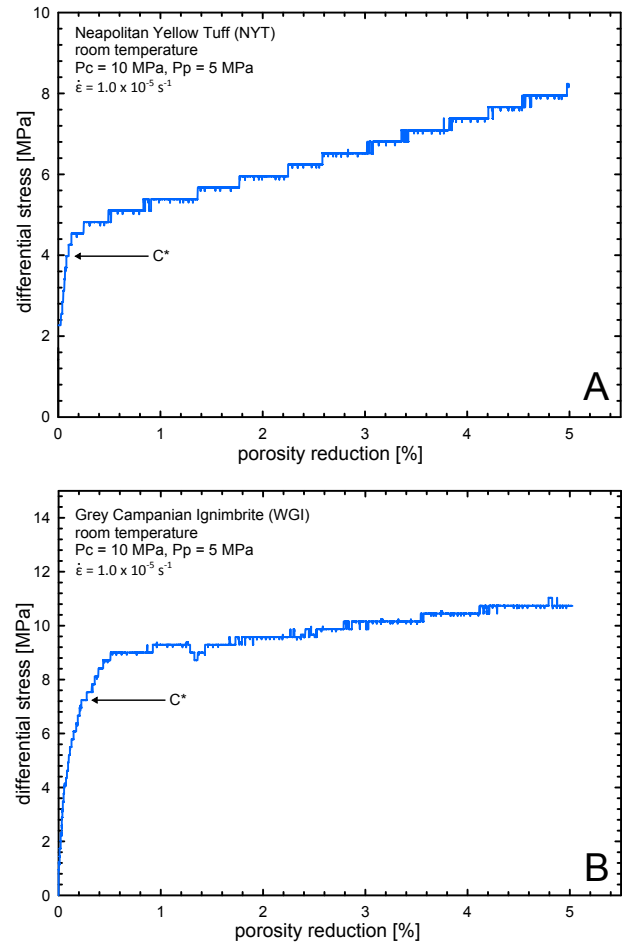


Fig. 10. Constant strain rate stress–porosity reduction curves for as-collected Neapolitan Yellow Tuff (A) and Grey Campanian Ignimbrite (B). The experimental conditions are provided on each panel and the positions of C^* are indicated by the arrows. The steps in the data are due to the stepwise nature of the pumps. Note that the porosity reduction on the x axis is not a relative change.

5 Discussion

5.1 Fluid flow and physical property evolution with depth

Our experimental data show that the water permeability of different as-collected tuff samples from Campi Flegrei can vary by multiple orders of magnitude (at a P_{eff} of 5 MPa, permeabilities are 1.0×10^{-15} and 1.0×10^{-13} m² for NYT and WGI, respectively). This difference in permeability could be considered surprising if one were to solely consider their connected porosities (44 and 49 % for NYT and WGI, respectively). The difference in permeability is likely due to differences in pore space connectivity, perhaps related to the extent of zeolitization and lithification. A similar conclusion was drawn by Vinciguerra et al. (2009). Vinciguerra et

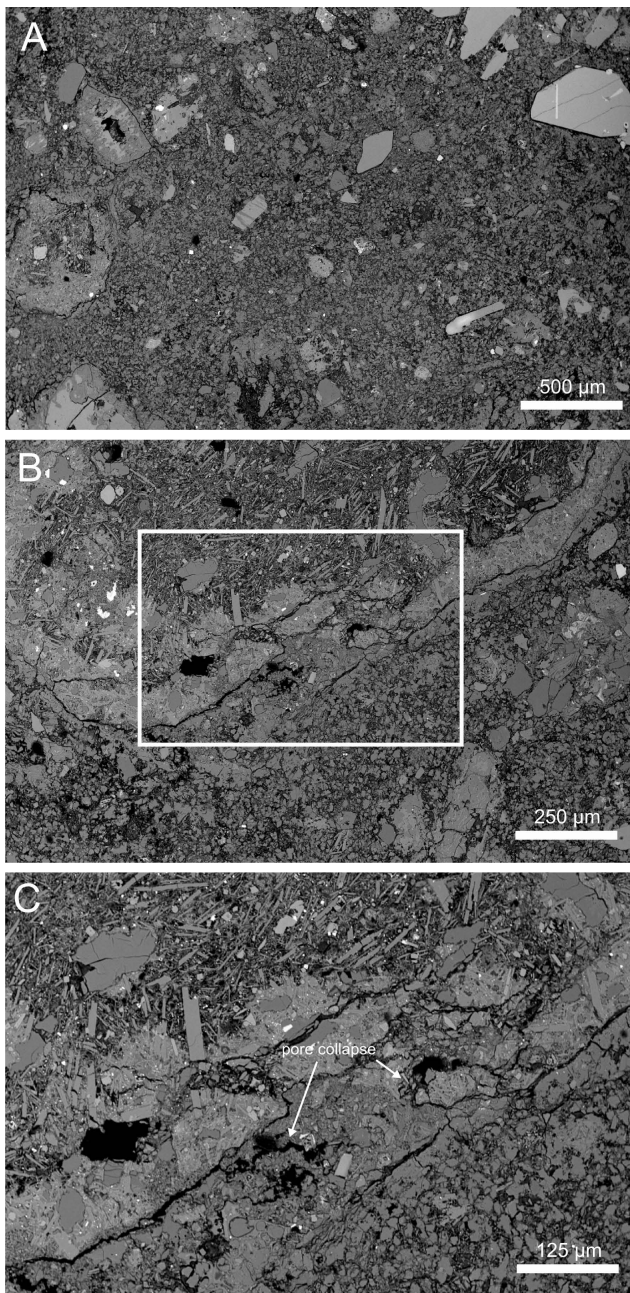


Fig. 11. Scanning electron microscope images of an as-collected sample of Neapolitan Yellow Tuff taken beyond P^* . (A) shows an overview of the post- P^* microstructure at a low magnification. (B) and (C) show detailed evidence of pore collapse (indicated by the white arrows). (C) is a zoom of the white box shown in (B).

al. (2009) measured the permeability of two different tuffs from the Alban Hills (Italy) and found that, at a P_{eff} of 5 MPa, the permeabilities of the two tuffs were significantly different. While the first (well-lithified, zeolitized facies with an average porosity of 14 %) was found to have a permeability of about 10^{-18} m^2 , which decreased by about an order of magnitude upon the application of a P_{eff} of 70 MPa, the

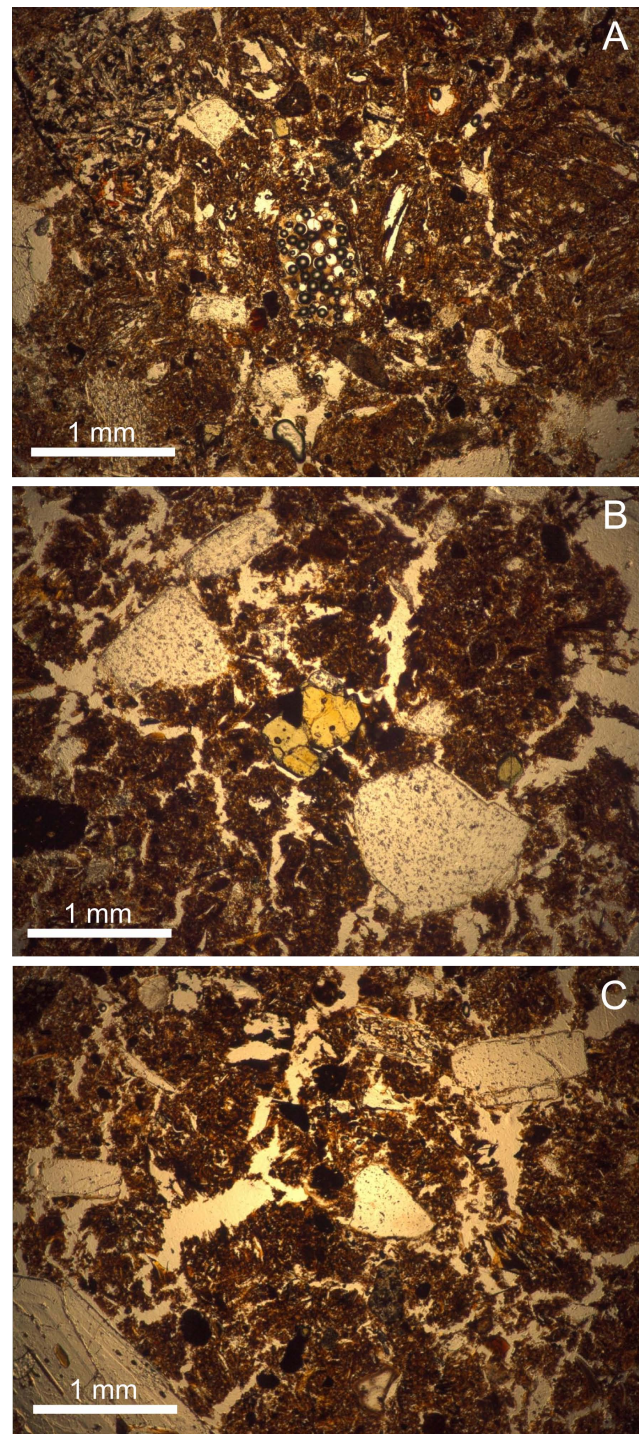


Fig. 12. Optical microscope images of Neapolitan Yellow Tuff thermally stressed to a temperature of 1000 °C showing macrocracks. The photomicrograph in panel A, showing foaming, is taken from Heap et al. (2012).

second (fine-grained, matrix-supported facies with frequent centimetre-sized accretionary lapilli and an average porosity of 18 %) had a much higher permeability (about 10^{-15} m^2)

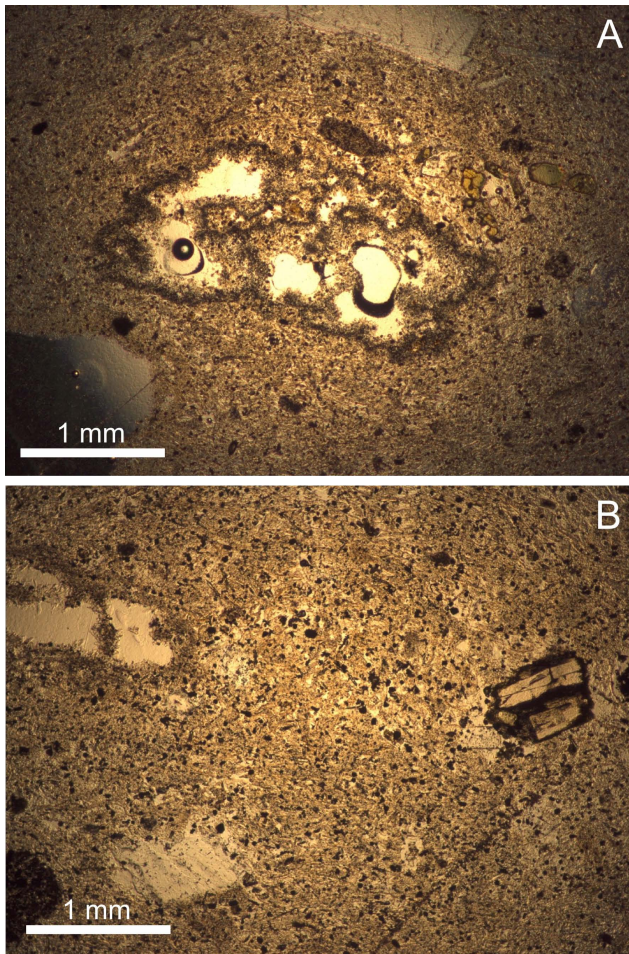


Fig. 13. Optical microscope images of Grey Campanian Ignimbrite thermally stressed to a temperature of 1000 °C. Both photographs are taken from Heap et al. (2012).

that decreased by about two orders of magnitude over the same pressure range. Further, considering the high porosities of NYT and WGI, their permeabilities are actually surprisingly low; considered to be a consequence of their complex pore structure. By contrast, Boise sandstone (porosity of 35 %), a rock with a much simpler microstructure, has a permeability of $1.8 \times 10^{-12} \text{ m}^2$ at a P_{eff} of 5 MPa (Zhu and Wong, 1997).

Our experimental data also show that the permeability of the two tuffs is reduced by about an order of magnitude over the pressure range from 5 to 50 MPa. In detail, the reduction in permeability with increasing P_{eff} is modest up to a P_{eff} of about 10–15 MPa, and accelerates at pressures above 10–15 MPa. This can be explained by the position of P^* (Fig. 5), the onset of inelastic pore collapse and grain crushing. As pores collapse and grains are crushed, the pathways for fluid flow are obstructed. This inelastic compaction also has a significant influence on other physical properties of the tuffs (ultrasonic wave velocities, dynamic elastic moduli, and the

V_P/V_S ratio all increase), in agreement with similar studies on NYT (Vanorio et al., 2002; Vinciguerra et al., 2006). Evidence of pore collapse is illustrated in the E-SEM image of a sample of NYT taken beyond P^* provided as Fig. 11. Pore collapse above P^* has previously been observed in a tuff from the Alban Hills, Italy (Zhu et al., 2011). A pressure of about 10–15 MPa roughly equates to a depth of about 750 m. Geological cross sections of CF (e.g. Orsi et al., 1996) suggest therefore that a large volume of the NYT and WGI tuffs are located at depths where the pressure will be above P^* . This conjecture is confirmed by the reduced porosity of samples taken from borehole samples (see the report by Giberti et al., 2006). Measurements on borehole samples from San Vito 1 (at the periphery of the inferred caldera) showed that the porosity decreases from 40.5 % at the surface to 32.9, 21.9, 21.9, and 15.1 % at depths of 810, 1420, 2130, and 2860 m, respectively. Our data show that the porosity loss for NYT at 2860 m will be about 9 %. A starting porosity of 44 % yields a porosity, purely due to mechanical compaction, of 35 % at a depth of 2860 m. This would imply a porosity loss due to chemical alteration of about 20 % and suggests that the impact of hot, circulating fluids plays the dominant role in the porosity loss of these pyroclastic deposits at depth. Indeed, the report by Giberti et al. (2006) suggests that it is the presence of clay minerals, rather than compaction, that is responsible for the major changes in porosity with depth.

We are certainly aware that our permeability data were collected on rocks from open quarries and, although their properties were measured at the relevant pressures (and under a range of thermal stressing temperatures), may not therefore accurately represent the material at depth (which have had time to compact, lithify, undergo chemical alteration; e.g. see de Gennaro et al., 2000). However, the open access report of Giberti et al. (2006) offers some permeability data on borehole samples. Data from borehole samples taken from San Vito 1 (at the periphery of the inferred caldera) show that, as the porosity is reduced to 32.9, 21.9, 21.9, and 15.1 % at depths of 810, 1420, 2130, and 2860 m, respectively, the permeability (Klinkenberg corrected gas permeabilities) of the samples are 1.1×10^{-13} , 2.5×10^{-16} , 7.9×10^{-16} , and $4.9 \times 10^{-16} \text{ m}^2$, respectively. The permeability of the quarry samples of this study are $8.0 \times 10^{-17} \text{ m}^2$ at a depth of about 2860 m. From these data it is clear that there is no simple relationship between the mechanical compaction and chemical alteration that afflicted the samples at depth (causing a substantial porosity loss) and their permeability.

5.2 The influence of temperature on fluid flow and physical properties

Our experimental data show that thermal stressing has a strong influence on the physical properties of NYT, whereas those for WGI are unaffected. The fluid flow properties of NYT are enhanced (especially at shallow depths) upon exposure to high temperatures, and the ultrasonic wave velocities,

dynamic elastic moduli, and the V_P/V_S ratio decrease. Thermal stressing has previously shown to decrease ultrasonic wave velocities in a zeolitized tuff from CF (Vinciguerra et al., 2006). The marked difference in the temperature dependence of the physical properties between the two tuffs is likely due to the presence of significant quantities of thermally unstable zeolites in NYT, namely phillipsite and chabazite, which are not present in WGI (Heap et al., 2012). Heap et al. (2012) showed, using a combination of thermogravimetric analysis, optical microscopy, and X-ray diffraction, that NYT lost 18 % of its initial mass, contained large numbers of macrocracks, and no longer contained any zeolites after exposure to 1000 °C. By contrast, no changes in mass, microstructure, or chemistry were seen in WGI heated to the same temperature (Heap et al., 2012). Optical microscope photomicrographs of NYT and WGI thermally stressed to a temperature of 1000 °C are provided as Figs. 12 and 13, respectively. Figure 12 shows that the microstructure of NYT is very different to that depicted in Fig. 2b for the as-collected material. Many cracks are present (Fig. 12a–c) and some areas contain 1 mm wide foamed glass (Fig. 12a). By contrast, the microstructure of WGI, upon exposure to 1000 °C (Fig. 13), is indistinguishable from the as-collected microstructure shown in Fig. 2d. These observations have been previously reported in Heap et al. (2012). Since, phillipsite and chabazite represent the “cement” that promoted the lithification of the originally incoherent pozzolanic material constituting NYT (de Gennaro et al., 2000), the structural integrity of NYT deteriorates significantly upon their loss (Heap et al., 2012). Detailed studies (de Gennaro and Colella, 1989, and references therein) on the thermal decomposition of the zeolites in NYT have highlighted that analcime loses water irreversibly, and that chabazite and phillipsite undergo a partial reversible dehydration at 240 °C. Phillipsite breaks down during dehydration and chabazite undergoes reversible hydration at 350 °C, and, by 900 °C, the structure of the zeolites will be so damaged that no further water molecules can be stored (see de Gennaro and Colella, 1989, and references therein). Therefore, the reported changes in NYT physical properties are due to a combination of thermal cracking and the cracks formed as a result of the disintegration of the material through the loss of zeolites.

If we consider NYT at a depth of 1 km, the geothermal gradients provided by the AGIP (1987) exploration boreholes show that temperatures of 200–250 °C are not unreasonable (Wohletz et al., 1999; de Lorenzo et al., 2001). The data of this study reveal that the zeolitized NYT are prone to undesirable thermal alteration at these temperatures. At temperatures of 200–250 °C, permeability increases by a factor of 2.5, ultrasonic wave velocities, dynamic elastic moduli, and V_P/V_S ratio decrease by roughly 10 %; and uniaxial compressive strength and indirect tensile strength are reduced by more than a factor of 2 (Heap et al., 2012). A reduction in tensile strength may further promote physical property changes by encouraging fluid pressure driven fracturing. An

internal pore fluid pressure of 22–23 MPa (under a confining pressure of 6–7 MPa) was sufficient to fracture a sandstone of 13 % porosity (Vinciguerra et al., 2004). It is therefore likely that the estimated overpressures needed to explain the ground deformation at CF (e.g. 10 MPa, Gaeta et al., 1998) are sufficient to fracture the tuffs and cause further changes in rock physical properties. Although it has been shown that the porosity of borehole samples can be much less than those collected from the surface (see the report of Giberti et al., 2006), perhaps, given their complex microstructure, it is unwise to assume that these rocks are stronger. To understand whether fluid driven fracturing is prevalent at CF, measurements of the tensile strength of samples taken from boreholes is required.

5.3 Application of these data to ground deformation modelling at CF

Our data highlight that the elastic moduli of two different tuffs from CF are significantly depth-dependent (Figs. 7, 8). The implication of these data is that the assumption of a homogenous half-space may be an oversimplification, and is exacerbated further when one considers the extent of the variability of the tuffs within the caldera (which are variably lithified, altered, and zeolitized, see the report of Giberti et al., 2006). These data highlight the need for the development of more complex, multi-layer ground deformation models. In order to assess the extent of the variability in elastic moduli of the rocks within the caldera at CF, a systematic experimental approach involving borehole samples from different depths and locations within the caldera is now required (discussed further at the end of the section).

We also find that static and dynamic moduli for the same tuff differ substantially. Although it is not uncommon for static and dynamic elastic moduli to be different, due to their frequency-dependence (Simmons and Brace, 1965; Cheng and Johnston, 1981; Eissa and Kazi, 1989; Ciccotti and Mulargia, 2004; Ciccotti et al., 2004), it raises an important question regarding which values are more appropriate in modelling. Manconi et al. (2010) highlighted that, while dynamic elastic constants (those derived from seismic velocities) are representative for rock subject to a dynamic stress, perhaps static values are more appropriate in the analysis of deformation caused by volcanic sources. A similar conclusion was drawn by Heap et al. (2009). However, static elastic moduli for representative rocks at CF have not been available until now. Thus far, elastic moduli have been generally assumed, or extrapolated from seismic tomography studies (e.g. Chiarabba and Moretti, 2006). Typically, Poisson's ratio is taken as 0.3 and shear modulus as 5 GPa (e.g. De Natale et al., 1991). However, while our data show that static and dynamic Poisson's ratio is similar for the measured tuffs (and equal to about 0.3; measurements on borehole samples are also consistently about 0.3, see the report by Giberti et al., 2006), we also observe that the static shear modulus is about

Table 6. The static and dynamic elastic moduli of NYT and WGI measured under an effective pressure of 5 MPa.

	Neapolitan Yellow Tuff (NYT)		Grey Campanian Ignimbrite (WGI)	
	static	dynamic	static	dynamic
Young's modulus [GPa]	2.1	6.0	1.7	4.9
Poisson's ratio	0.30	0.31	0.29	0.24
shear modulus [GPa]	0.81	3.1	0.66	2.7

a factor of four lower than the dynamic value (Table 6). If one were to assume that our static values are representative, then a more suitable shear modulus would be 0.5 GPa, an order of magnitude lower than the values typically used in ground deformation modelling at CF. We note that, while values of the shear modulus of borehole samples provided in the report of Giberti et al. (2006) show that the dynamic shear modulus can reach values of 10.9 GPa at a depth of 2860 m, no complementary static values exist. Future research should focus on the determination of the static elastic moduli of borehole samples.

To date, the values of permeability used in the numerous thermodynamical and magmatic-hydrothermal models have spanned many orders of magnitude. For example, Gaeta et al. (1998) use a value of 10^{-11} m^2 , inferred from the measurements of Ascolese et al. (1993a, b) and De Natale et al. (2001) use the same value, but inferred from the in situ observations of Rosi and Sbrana (1987). By contrast, Gaeta et al. (2003) use a much lower value of 10^{-15} m^2 , taken from the ambient pressure measurements of Peluso and Arienzo (2007). The experimental data of this study has shown that (1) the permeability of tuffs at CF can differ by about 1.5 orders of magnitude (from 2.0×10^{-15} to $6.3 \times 10^{-17} \text{ m}^2$, due to the extent of zeolitization and lithification, see Tables 4 and 5), (2) effective pressure (depth) can significantly alter the permeability of tuff (by up to 2 orders of magnitude, see Fig. 6) and, (3) if the tuff is zeolitized, permeability can be increased by thermal stressing episodes (Fig. 6a). While we note that the permeability of different tuffs at CF can differ greatly (we expect the extent of the variation to greatly exceed the 1.5 orders of magnitude quoted here), the same will also be true for tuff from the same eruptive episode. The NYT and the Campanian Ignimbrite – both thick and widespread pyroclastic deposits – are well known to be variably lithified and zeolitized (de Gennaro et al., 2000; Langella et al., 2013). The highly variable nature of tuffs at CF (both laterally and vertically, see the report by Giberti et al., 2006), coupled with the depth-dependence of permeability, is likely to produce highly variable permeabilities within the caldera. Unfortunately, the implication of this conclusion is that, to accurately model ground deformation using a model that requires an estimation of the permeability of the materials within the caldera, we now require (1) permeability measurements on borehole samples (from different depths and different locations within the caldera) to assess the extent of

the variability in permeability within the caldera and, (2) the development of more complex models that can account for such variations in permeability.

To conclude, while we advise that our laboratory-derived values should be considered for routine ground deformation modelling at CF, we also urge caution. Firstly, our measurements on laboratory-sized samples do not account for large faults or fractures, which, for example, would serve to lower Young's modulus. Secondly, an important question arises: what constitutes “representative” materials for the caldera at CF? Although our experiments were conducted (1) on samples from the two most widespread tuff lithologies that comprise CF, (2) under the relevant pressures or depths, (3) on water-saturated samples and, (4) over a range of thermal stressing temperatures, our samples were collected from an open quarry and may therefore not represent the material at depth (which have had time to compact, lithify, undergo chemical alteration; e.g. see de Gennaro et al., 2000; see also the report by Giberti et al., 2006). However, we highlight that the permeability measurements on borehole samples presented in the report of Giberti et al. (2006) suggest that (1) the permeability measurements of this study are not dissimilar to those measured on borehole samples and, (2) there is clearly no simple relationship between porosity and permeability. Further, the tuffs of CF are likely to be extremely variable (due to variable lithification, zeolitization, interaction with fluids and temperatures) laterally (i.e. within the same lithological unit) and therefore their physical properties at a constant depth are also likely to span a wide range. It is clear that systematic measurements on deep scientific borehole samples are now needed from multiple locations and depths within the caldera to assess the extent of the variability in static elastic moduli and permeability of the rocks that form the caldera. To conclude, we anticipate that no unique values of permeability or elastic moduli exist for the materials within CF, highlighting the need for the development of more complex ground deformation models.

6 Conclusions

1. Our experimental data show that the permeabilities of tuffs from Campi Flegrei (the Neapolitan Yellow Tuff and a tuff from the Campanian Ignimbrite) can vary by multiple orders of magnitude. Despite this, our data also show that their elastic moduli are similar;

however, we note that dynamic and static moduli differ greatly. These data emphasize the heterogeneous nature of the tuffs that comprise the caldera at Campi Flegrei.

- Increasing the effective pressure from 5 to 50 MPa results in a permeability reduction of about an order of magnitude and a porosity reduction between 5 and 10% for both tuffs. As effective pressure increases we also observe an increase in ultrasonic wave velocities, dynamic elastic moduli, and V_P/V_S ratio. These changes all accelerate after the onset inelastic pore collapse (P^*), which exists between effective pressures of 10–15 MPa.
- Thermal stressing increases the permeability and decreases the ultrasonic wave velocities, dynamic elastic moduli, and V_P/V_S ratio of the Neapolitan Yellow Tuff. However, the tuff from the Campanian Ignimbrite is unaffected by thermal stressing. This is the result of the loss of thermally unstable zeolites, namely phillipsite and chabazite, in Neapolitan Yellow Tuff. For example, for the sample thermally stressed to 750 °C, the permeability at an effective pressure of 5 MPa increases by an order of magnitude relative to the as-collected material.
- While we urge that these new laboratory data should be considered in routine ground deformation modelling, our study highlights that the physical properties of just two rocks that comprise the caldera at Campi Flegrei can be extremely heterogeneous (we also anticipate that future measurements will further expand our knowledge of such heterogeneity). These data underline the challenges for accurate ground deformation modelling at Campi Flegrei. We anticipate that no unique values of permeability or elastic moduli exist for the materials within Campi Flegrei, highlighting the need for the development of more complex ground deformation models.

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