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EFFECTIVE PROPERTIES OF RANDOMLY ORIENTED KENAF SHORT FIBER REINFORCED EPOXY COMPOSITE

by

Dayakar Naik L

A dissertation submitted in partial fulfillment of the requirements for the degree

of

DOCTOR OF PHILOSOPHY

in

Mechanical Engineering

Approved:

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UTAH STATE UNIVERSITY Logan, Utah

2015

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Abstract

Effective Properties of Randomly Oriented Kenaf Short Fiber Reinforced Epoxy Composite

by

Dayakar Naik L, Doctor of Philosophy Utah State University, 2015

Major Professor: Dr. Thomas H. Fronk Department: Mechanical and Aerospace Engineering

Natural fibers have drawn attention of researchers as an environmentally-friendly alternative to synthetic fibers. Developing natural fiber reinforced bio-composites are a viable alternative to the problems of non-degrading and energy consuming synthetic composites. This study focuses on (i) the application of kenaf fiber as a potential reinforcement and, (ii) determining the tensile properties of the randomly oriented short kenaf fiber composite both experimentally and numerically. Kenaf fiber micro-structure and its Young's modulus with varying gage length (10, 15, 20, and 25.4 mm) were investigated. The variation in tensile strength of kenaf fibers was analyzed using the Weibull probability distribution function. It was observed that the Young's modulus of kenaf fiber increased with increase in gage length. Fabrication of randomly oriented short kenaf fiber using vacuum bagging techniques and hand-lay-up techniques were discussed and the tensile properties of the specimens were obtained experimentally. The tensile modulus of the composite sample at 22% fiber volume fraction was found to be 6.48 GPa and tensile strength varied from 20 to 38 MPa. Numerical models based on the micro mechanics concepts in conjunction with finite element methods were developed for predicting the composite properties. A two-step homogenization procedure was developed to evaluate the elastic constants at the cell wall level and the meso-scale level respectively. Von-Mises Fisher probability distribution function was applied to model the random orientation distribution of fibers and obtain equivalent modulus of composite. The predicted equivalent modulus through numerical homogenization was in good agreement with the experimental results.

(154 pages)

Public Abstract

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Natural fibers have drawn attention of researchers as an environmentally-friendly alternative to synthetic fibers. Developing natural fiber reinforced bio-composites are a viable alternative to the problems of non-degrading and energy consuming synthetic composites. This study focuses on (i) the application of kenaf fiber as a potential replacement for glass fibers and (ii) determining the mechanical properties of the randomly oriented short kenaf fiber composite both experimentally and numerically. Kenaf fiber micro-structure and its mechanical properties with varying gage length (10, 15, 20, and 25.4 mm) were investigated. The variation in tensile strength of kenaf fibers was analyzed using a statistical method called Weibull probability distribution function. It was observed that the Young's modulus of kenaf fiber increased with increase in gage length. Fabrication of randomly oriented short kenaf fiber using vacuum bagging techniques and hand-layup techniques were discussed and the tensile properties of the specimens were obtained experimentally. The tensile modulus of the composite sample at 22% fiber volume fraction was found to be 6.48 GPa and the tensile strength varied from 20 to 38 MPa. Simultaneously, a computer program (finite element method) was written to predict the tensile properties of composites using a micro mechanics approach. The predicted equivalent modulus through a computer program (finite element method) was in good agreement with the experimental results.

To my parents Laxmi and Balaiah, and all my teachers.

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Dayakar L. Naik

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Chapter 1 Introduction

1.1 Background

The composite material is processed from a mixture of two or more different materials in certain proportions. Generally speaking, composite comprises a fiber reinforcement embedded in a polymeric matrix. The motivation behind the invention of a composite material comes from the demand of low weight and high strength material for the aerospace industry. The major work in this area was carried out during 1960's and to date there are several types of composites being developed for various applications [1]. Examples of some synthetic fibers are glass, carbon, boron, aramid and Kevlar. Commercially available polymer matrices include epoxy, polypropylene and polyethylene. Metals and ceramics are also used as matrix materials in composite processing. A wide range of these composite materials have been successfully used for structural applications in the aircraft, space, automotive, marine and infrastructure industries.

Generally, for structural applications, composite laminates are processed by stacking lamina with varying fiber orientations to achieve the desired structural behavior. Structural functionality includes high tensile load carrying members; low thermal expansion, thermal barriers etc; and sometimes discontinuous fiber composite as shown in Fig 1.1. The mechanical behavior of such laminates is anisotropic in nature, meaning it depends on the fiber and matrix properties, fiber orientation and volume fraction, the interface bond between fiber and matrix, and processing techniques. The choice of a particular composite processing technique depends on the type of matrix to be used for composite, either thermoset or thermoplastic. Techniques used for thermoset kind matrix include resin transfer molding, vacuum assisted resin transfer molding, compression resin transfer molding, and pultrusion process. Thermoplastic kind matrix includes compression molding, filament winding and injection molding. Application of these techniques depends on the type of structure (flat or complex), rate of production and type of application.



(a) Laminated Composite (b) Discontinuous Fiber Composite (c) Particulate Composite

Fig. 1.1: Various Types of Composites

Most of the existing composite materials (both fibers and polymers) are processed from petroleum based products and the previously mentioned processing techniques are power consuming. Consequently, some concerns associated with the commercially available composites are high energy consumption, non-recyclability, non-renewability and cost. There is a need for developing an alternate composite material or processing technique that is economical, low energy consuming and environmental friendly. In recent years, researchers explored the potential natural fibers (derived from plants and animals) as a replacement for synthetic fibers. An experimental investigation of some natural fibers conducted by S. V. Joshi et al.[2] proved to be capable of replacing E-glass fibers. At this point, before proceeding into details, the following questions must be answered:

- What is the morphology of natural fibers?
- Do these fibers have enough benefits to replace existing commercial fibers?
- How does one process a natural fiber reinforced composite?
- What are the major advantages and applications of natural fiber composites?

Natural fibers in this context imply those derived or obtained from plants. These fibers can be obtained from different parts of a plant, including the stem, leaf, root, core and fruit [3]. The fibers obtained from the stem are called bast fibers and those obtained from the leaf, root and fruit are called leaf fibers, root fibers and fruit fibers respectively. Examples of bast fibers are hemp, flax, kenaf, and jute, leaf fibers are abaca, sisal and pineapple, and fruit fibers are cotton, coir and kapok [3]. A summary of worldwide production of various natural fibers was given in [3, 4]. It was observed that the bast fibers are most commonly used, followed by leaf and fruit fibers, proving their abundance in nature. This is why past few years of research have focused on using bast fibers as a replacement for synthetic fibers. The potential of various bast fibers as a composite reinforcement is discussed in the subsequent sections. Once the source for fibers has been chosen, the next step is extracting fibers from the stem (known as retting). Various retting processes currently used in industry include dew-retting, water retting, chemical retting and physical methods [4]. The effect of retting methods on the bast fiber properties was collectively discussed in a review article [4]. Water retting results in good quality fiber but takes 2-3 weeks, whereas chemical retting is done quickly and results in decreased strength of the fibers. The process of retting is followed by decortication, carding and spinning into yarn. The full process of various fiber extractions [5] is shown in Fig 1.2.

Properties	E-glass	Hemp	Jute	Ramie	Coir	Sisal	Flax	Cotton
Density (g/cm3)	2.55	1.48	1.46	1.5	1.25	1.33	1.4	1.51
E-modulus (GPa)	73	70	10-30	44	6	38	60-80	12
Specific modulus	29	47	7-21	29	5	29	26-46	8

Table 1.1: Comparison of Natural and E-glass Fiber Properties

The potential (specific modulus) of the natural fibers as a reinforcement was studied by Wambua et al. [6] and shown in Table 1.1. Five different fibers; sisal, kenaf, hemp, jute and coir were selected in [6] and a polypropylene matrix based composite was processed. The mechanical properties of these composites were compared to that of E-glass fiber reinforced composites. Specific modulus of natural fiber reinforced and E-glass fiber reinforced composites were reported to be comparable except in the case of coir. Earlier research focused on flax, hemp, bamboo and jute fibers due to their abundant availability and extensive use in the textile industry. This study will focus on a similar common fiber source, the kenaf plant. Kenaf fiber secures third place in terms of worldwide production (870.103 ton per year) after jute. In addition to its availability, kenaf belongs to the same family as the jute plant [7] and is likely to share jute's desirable properties. There is a need for research about kenaf fiber properties and its surface characteristics if it is to become a successful reinforcement for composite production.



Fig. 1.2: Processing Steps of Natural Fiber

Unlike artificial fibers, natural fibers show great variation in their mechanical properties due to: growth conditions, age of the plant, which part of the stem they are extracted from, varying constituent's fraction at the microscopic level, etc. Shinji Ochi [8] reported the variation of kenaf fiber modulus as a function of fiber location on the stem, where fibers obtained from the bottom part of a plant exhibited more tensile strength (about 20% more). The tensile strength of a fiber determined experimentally at the macroscopic scale is governed by the structure and the chemical composition present at the microscopic scale. Most of the materials available in nature are composite in nature (i.e. the material is a mixture of different chemical constituents). Similarly, bast fibers also consist of constituents namely: cellulose, hemicellulose, lignin, pectin and waxes at the microscopic scale [3]. Fibers obtained from different plants have variation of these constituents and consequently exhibit different properties. The volume fraction of each constituent in different fibers was reported in [3, 4] and is presented here in Table 1.2.

The major advantages of developing natural fiber based composites include low cost, low density, recyclability, low pollution, no health hazards and effective utilization of resources [2]. To this point, natural fiber composites can be used for secondary structural application due to their lower tensile strength and mechanical properties compared to that of primary structural applications. Some applications were listed in [10, 11] that includes seat backs, dashboards, door panels, and sports goods. In order to expand the use of natural fiber based composites, further detailed investigations are required with a focus on strength improvement.

1.2 Kenaf Fiber as Reinforcement for Composites

Kenaf (Hibiscus Cannabinus) is an annual herbaceous fiber plant from the tropical and subtropical regions of Africa and Asia [12, 13]. It is also found in the parts of Europe and USA. Kenaf plants grow up to 4m in about 3-4 months with a base diameter of 3-5 cm. The cost of kenaf fibers in the year 2000 was \$278-302 per ton and 15MJ of energy was consumed to produce 1 kg of kenaf fiber, where glass fibers consumed 54MJ [14]. After processing, the average length of a kenaf fiber available in the market is around 70 mm with the diameter ranging from 10 μ m to 80 μ m. The potential of kenaf fiber as a reinforcement in composite was reported by various authors [6, 12, 15] in the past by comparing the specific modulus of a composite with that of glass fiber reinforced composite. The tensile modulus of kenaf fiber reinforced polypropylene composite was reported to be the same as that of glass fiber mat reinforced composite [6] at 22% volume fraction. A discussion on manufacturing problems of kenaf fiber reinforced composite, due to the limited available length of fiber, was produced by Zampaloni et al. [12]. They concluded that the short fiber and compression molding technique resulted in a composite with 40% weight fraction of fiber and greater specific strength compared to E-glass fiber composite. Most of the research investigations in the past were conducted on composites processed through thermoplastic techniques that required high temperature and pressure during processing. These higher temperatures (160⁰C), resulted in reduced tensile strength [8] which consequently resulted in reduced composite properties. An alternative to thermoplastic processing is thermoset processing, which does not require higher temperatures. It is also a good technique for fabrication of complex structures.

Fiber	Cellulose (%)	Hemicellulose (%)	Lignin (%)	Reference
	61-71	14-20	12-13	[3]
Jute	51-84	12-20	5-13	[4]
	45-63	21-26	18-21	[9]
Konaf	72	20.3	9	[3]
Rellai	44-57	21	15-19	[4]
Hemn	68	15	10	[3]
memp	70-92	18-22	5-3	[4]
Flow	71	18.6-20.6	2.2	[3]
FIAN	60-81	14-19	2.3	[4]

Table 1.2: Volume Fraction of Basic Constituents in a Bast Fiber

The Young's modulus of kenaf fiber varied from source to source [7]. Many factors, such as growth conditions, location on the stem from where the fiber is obtained, varying composition of basic constituents, and defects such as kink bands, can influence the fiber properties. Along with the above mentioned factors, the cross sectional area calculation of the fiber also plays a vital role in determining the Young's modulus. The effective properties of a composite are a function of fiber volume fraction, Young's modulus and its geometry, orientation of fibers, interfacial bonding between fibers and matrix and defects such as voids. Therefore, the behavior of kenaf fibers should be explored in detail. To the authors knowledge, neither has there been significant effort to determine the interfacial properties of kenaf fiber and epoxy matrix, nor to explore the option of short kenaf fiber as reinforcement.

This study aims at determining Young's modulus of kenaf fiber and its composite (fabricated through hand-lay-up technique) and develop a numerical homogenization model to predict the effective properties of the composite. Objectives are discussed more specifically in the next chapter.

1.3 Review of Literature

The organization of this section is as follows: micro-structure of bast fiber, mechanical properties of kenaf fibers and its composites.

Micro-structure of any bast fiber consists of a bundle of cell walls together with a middle lamella as an interfacing layer. Cell walls are hollow laminated composite tubes (cross-section) consisting of Primary (P) and Secondary (S1, S2 and S3) layers with varying micro fibril or cellulose orientation in each layer. Volume fraction of basic constituents in kenaf fiber is: cellulose 44% - 72%, hemicellulose 20%-22% and lignin 9%-19% based on values presented in Table 1.2. The schematic representation of cell wall structure of a bast fiber is shown in Figure 1.3. The micro-structure of a kenaf fiber obtained from an optical microscope and a scanning electron microscope is presented in Chapter 3.



Fig. 1.3: Schematic Representation of Bundle of Cell Walls in Bast Fiber

It is a well-known fact that the effective properties of a composite are governed by the

Fiber	Tensile Strength (MPa)	Tensile Modulus (GPa)	Reference
	400-800	10-30	[7]
Into	533	20-22	[7]
Jute	393-773	26.5	[7]
	860	60	[13]
	223	14.5	[7]
Kenaf	240-600	14-38	[13]
	930	53	[17]
	550-900	70	[7]
	270	23.5	[7]
Homn	534-900	30-90	[7]
memp	900	34	[7]
	920	70	[13]
	690	70	[17]
	800-1500	60-80	[7]
Flow	1339	54	[7]
I lax	343-1035	27.6	[7]
	1339	58	[13]

Table 1.3: Tensile Strength and Modulus of Various Bast Fibers from Literature

properties of a fiber, matrix and fiber/matrix interface. Experiments were conducted to calculate kenaf fiber modulus, considering fixed gauge length, loading rate, and the effect of moisture content. The results were collectively reported in several review papers, which are compiled and presented in Table 1.3. A significant variation in tensile strength and modulus values were observed due to varying chemical composition in fibers and uncertainties associated with measurements of fiber dimensions. Measuring a cross-sectional area of a fiber provides a major challenge, which is significant in calculating stress and determining the Young's modulus of a kenaf fiber. Any assumption of a circular, elliptical or other crosssectional shape will produce results with more uncertainty. Obtaining specimen dimensions (gage length) of the fiber to be tested is vital in calculating a reliable Young's modulus. Studies conducted on the sisal fibers [18] showed that there is no effect of gage length on the fiber properties, whereas studies conducted on polymeric fibers [19] showed that gage length plays a major role, with defects increasing with an increase in length. There has been no significant effort made to obtain reliable value of kenaf fiber modulus by considering the rate of loading, standard gage length and appropriate techniques for cross-sectional area measurement.

Kenaf fibers were mostly reinforced in a PLA/PP polymeric matrix and tensile (Table 1.4), flexural and impact properties were obtained. Tensile modulus of these kinds of composites were already discussed in section 1.2. Not many studies have been conducted on using thermoset polymers, though they provide more wettability, lower cost and are more effective for manufacturing of complex shapes. To attain composites with increased strength, effects of kenaf fiber surface treatments were investigated. 3% maleated anhydride poly-propylene (MAPP) improved the strength by 30% [19] and 3% alkali improved the strength by 20% [20].

Material	Tensile Modulus (GPa)
Konaf/PLA	6.3 [14]
Reliai/1 LA	20 [8]
	8.3 [9]
Kenaf/PP	4.84 [12]
	1.2 [16]

Table 1.4: Tensile Modulus of Kenaf Fiber Composites

Most of these results were based on an indirect measurement technique (i.e., composites were fabricated from chemically treated fibers and the strength of the composite was measured). This method does not guarantee the optimum volume fraction of fibers, as the fiber aspect ratio is not known. A single fiber pull out test will result in evaluating fiber/matrix interfacial strength and fiber aspect ratio, which governs the composite properties. Detailed interfacial studies [21] were conducted on flax and hemp fibers that showed that 9 mm and 13 mm are the critical lengths for complete stress transfer.

Little attention has been given to numerical modeling of the composites reinforced with natural fibers. Few models were developed in the past for modeling the behavior of wood and fibers using laminate theory. These are discussed in Chapter 4 in detail. From the literature, it can be concluded that there is a need for fundamental investigations on obtaining appropriate tensile modulus of a kenaf fiber, elastic constants of kenaf fiber reinforced epoxy composite and numerical modeling a natural fiber composite. Dissertation objectives were set based on the conclusions from existing literature and are presented in the next chapter.

1.4 Structure of Dissertation

This dissertation is divided into five chapters followed by references at the end. The chapters are as follows:

- 1. Introduction
- 2. Research Objectives
- 3. Experimentation
- 4. Numerical Modeling
- 5. Summary, Conclusion, and Future Work

The first chapter discussion involves the background of natural fiber composite, a literature review and some conclusions drawn from the past research, based on which the dissertation work was established. Chapter 2 lists the research objectives followed by a section explaining the approach used to accomplish the objectives. Chapter 3 explains the experimental work carried out to determine tensile modulus of kenaf fibers, fabrication technique of kenaf fiber reinforced composite, and concludes with an evaluation of Young's modulus and Poisson's ratio through tensile tests. Chapter 4 provides the 3D finite element micromechanical model of natural fiber composite to predict the homogenized or effective properties of natural fiber composite. Chapter 5 summarizes the research study, findings and conclusions from this work, and proposes further future work.

Chapter 2

Research Objectives

2.1 Objectives

- To determine the Young's modulus of a kenaf fiber through tensile test by considering the appropriate fiber cross-sectional area after failure.
- To determine Young's modulus and Poisson's ratio of a kenaf short fiber reinforced composite.
- To develop a RVE based model of a randomly oriented kenaf short fiber composite that predicts the approximate effective properties of a composite as a function of fiber volume fraction and its equivalent properties.

2.2 Research Approach

Based on the research objectives established in the previous section, the following tasks have been identified and proposed. The tasks can be divided into two main categories: experimentation and numerical modeling.

Experimentation

- To explore the micro-structure of kenaf fiber bundles through optical microscopy and scanning electron microscope examination. This task will help in understanding the structural morphology of the fiber.
- To find out a novel technique for evaluating the cross-section of kenaf fiber during a tensile test. This plays a major role in evaluating the tensile modulus of kenaf fiber.
- To investigate the effect of gage length on Young's modulus of kenaf fiber subjected to quasi-static loading. The purpose of this task is to examine the strength-limiting

defect over a certain length of fiber. With the increase in gage length, there is a possibility of included defect that limits the tensile strength of the fiber.

- To fabricate the kenaf short fiber epoxy composite by vacuum bagging technique and hand-lay-up technique. This task involves two processing techniques in preparing the tensile test specimens.
- To perform a tensile test on kenaf fiber reinforced composites and evaluate the Young's modulus and Poisson's ratio.

Numerical Modeling

- To predict the elastic constants of cell wall layers in bast fiber through unit cell modeling of the structure at the microscale. The elastic constants as a function of varying volume fractions of basic constituents in each layer of cell wall will be studied by developing a parametric 3D finite element model.
- To predict the effective properties of a unidirectional natural fiber composite through unit cell modeling of the structure at the meso-scale. This homogenization model incorporates the elastic constants obtained from the previous step as the cell wall layer properties. Also, the effective properties as a function of micro fibril angle in the S2 layer of cell wall will be investigated.
- To generate the RVE geometry of randomly oriented short fibers by applying the Von-Mises Fisher probability distribution function.
- As a final step, applying the orientational averaging technique (Von-Mises Fischer PDF) on the unidirectional composite properties to evaluate the quasi-isotropic properties of a randomly oriented short fiber composite.

Chapter 3

Experimentation

3.1 Micro-Structure of Kenaf Fiber

The micro-structure of a natural fiber consists of a cell wall bundle, as stated in Chapter 1. The shape of a cell wall is polygonal and governs the cross-sectional geometry of a fiber. This section presents the micro-structure of kenaf fibers examined under an optical microscope and scanning electron microscope (SEM). To view the micro-structure under an optical microscope, a polished mounting specimen was prepared with kenaf fibers encased in an epoxy matrix. The cross-section of kenaf fibers obtained at 50X magnification are shown in Figure 3.1. Figure 3.1(a)-3.1(d) reveals the inconsistency in cross-sectional shape and the presence of voids called lumen in the cell wall. The cell walls are seen to be circular or elliptical in the optical microscopic images. A scanning electron microscope (SEM) image of a single kenaf fiber, as shown in Figure 3.2, depicts the delamination of cell walls and kink bands. A single cell wall image shown in Figure 3.2(b) is hollow and rectangular in shape, with dimensions 13 μ m X 7 μ m X 2 μ m.

The optical microscopic images obtained along the axial direction of kenaf fibers (as shown in Figure 3.3) displays the defects present in the fiber. The possible defects along the fiber axial direction include varying diameter, fiber damage and delaminated cell walls. These defects combined with fiber anisotropy at micro scale level play an important role in the fiber properties.

3.2 Influence of Gage Length on Kenaf Fiber's Young's Modulus

The tensile modulus of artificial fibers (glass, carbon, Kevlar) and natural fibers depends on the test speed and their gage length [23]. When compared with artificial fibers, kenaf fibers display more uncertainty towards the consistent properties. The tensile testing



Fig. 3.1: Optical Microscopic Image of Kenaf Fibers at 50X Magnification



Fig. 3.2: Scanning Electron Microscopic Image of Kenaf Fibers

of kenaf fiber with varied gage length decides the critical length of the kenaf fiber. In other words, the influence of defects is less pronounced towards the tensile modulus at the critical length. This section describes the experimental methodology used to determine the tensile modulus of kenaf fiber and evaluate the associated uncertainties.

3.2.1 Materials and Procedure

The carded kenaf fibers, averaging 70mm in length, were obtained from Bast Fiber LLC. The gage lengths of 10, 15, 20 and, 25.4 mm were chosen to study the influence of gage length on fiber properties. At least ten specimens were tested for each gage length as



(a) Fiber Delamination



(b) Reduction in Cross-Sectional Area



(c) Diameter Variation Along Length



(d) Fiber Damage

Fig. 3.3: Optical Microscopic Image of Kenaf Fibers Along the Length

per ASTM D3822 standards, Standard Test Method for Tensile Properties of Single Textile Fibers.

A paperboard of width (25.4mm) suitable for tensile testing was prepared with varying gage lengths as shown in Figure 3.4(a). The fibers were fixed on the paperboard as per ASTM D3822 standards and shown in Figure 3.4(b). A tensile test was performed on the Instron 5848 micro tensile tester machine maintaining an extension rate of 1 mm/min [24]. Before testing each individual fiber, auto calibration was done and load-extension curve was recorded. The detailed procedure is explained below:

Procedure

- A single kenaf fiber (technical fiber) of length 60-70mm was randomly selected from the sample (up to 15 fibers).
- As the fibers are naturally curved, they were straightened with proper care while being fixed on the paperboard. The fiber fixation on paperboard is shown in Figure 3.4(b).
- 3. Using forceps, the prepared paper frame in step 2 was carefully mounted on the tensile testing machine and the grips were tightened, followed by cutting the paper frame as shown in Figure 3.4(c).
- 4. Bluehill software available on the Instron machine was launched and auto calibration was done. Tensile testing speed was set to 1 mm/min to carry out a quasi-static test and the load-extension curve of fibers was recorded.
- 5. Specimens that failed close to grip or slipped during the test were discarded and the data of at least 10 specimens were recorded.
- 6. The tested fibers were carefully stapled to the paper, which was later used for evaluating the cross-sectional area of fibers at the break point.

3.2.2 Cross-Sectional Area Measurement

The cross-sectional image of kenaf fiber was acquired using an optical microscope, based on the assumption that cross-section remains the same after failure. The mounted specimen, for observation under an optical microscope, was prepared by following the procedure described below:

Procedure

1. Two rectangular hollow boxes (top mold and bottom mold), as shown in Figure 3.5, were prepared using MPPA blocks.



(a) Load

Fig. 3.4: Fixation of Kenaf Fiber Specimen for Tensile Test on Paperboard

- 2. Double sided tape (blue) was fixed on the faces of the bottom block to adhere to the fibers.
- 3. Fibers were carefully attached to the tape, so that the break point of the fiber was close to the edge of the block.
- 4. The upper mold was placed on the bottom block to create a full box and sealed on all sides using duct tape.
- 5. The mixed epoxy resin was poured into the mold and left for curing.
- 6. The resulting mounted sample from Step 5 was grinded and polished to prepare the specimen for observation under an optical microscope.

Four such mounted specimens were prepared, each corresponding to a particular gage length, and images were acquired at 50X magnification. The acquired images were then analyzed using an ImageJ software to evaluate the cross-sectional area. The process of measurement requires an image of the calibrated scale (stage micrometer), acquired at the



Fig. 3.5: Specimen Preparation for Measuring Area of Fiber

same magnification as that of the fiber. In this study, a Nikon stage micrometer with 0-1 mm range was used as a calibration scale, shown in Figure 3.6. The optical microscopic images and the evaluated images from ImageJ software are presented in Appendix Table A.1-A.4



Fig. 3.6: Calibration Scale at 50X Magnification

3.2.3 Uncertainty in Area Calculation

The linear dimension associated with the image was evaluated as a product of conversion factor (k) and the number of pixels occupied by the image. Mathematically, it is expressed as [25,26],

$$s = kN \tag{3.1}$$

where k is the conversion factor and N is the number of pixels

Following Taylor Series Method (TSM) approach [27], the uncertainty associated with the

image based measurement is expressed as:

$$\left(\frac{u_s}{s}\right)^2 = \left(\frac{u_k}{k}\right)^2 + \left(\frac{u_N}{N}\right)^2 \tag{3.2}$$

where u_s , u_k and, u_N are the uncertainties associated with the linear dimension, conversion factor and the number of pixels respectively. The uncertainty associated with conversion factor k is given by Equation 3.3 [26]

$$\left(\frac{u_k}{k}\right)^2 = \left(\frac{u_m}{m}\right)^2 + \left(\frac{u_{Nl}}{Nl}\right)^2 \tag{3.3}$$

where m is the dimension on calibration scale and Nl is the number of pixels obtained for the calibrated length.

The uncertainty quantification of each term in Equation 3.3 was done by assuming probability distributions as presented in [26]

$$u_{Nl} = \frac{1}{2\sqrt{12}} \tag{3.4}$$

$$u_m = \frac{0.5 * Least \ dimension \ on \ calibrated \ scale}{\sqrt{3}} \tag{3.5}$$

$$u_N = \frac{1}{2\sqrt{6}} \tag{3.6}$$

The uncertainty of cross-sectional area is then quantified as

$$\left(\frac{u_A}{A}\right)^2 = \left(\frac{u_N}{N}\right)^2 + 4 * \left(\frac{u_k}{k}\right)^2 \tag{3.7}$$

3.2.4 Uncertainty in Young's Modulus

The tensile (initial) modulus of kenaf fiber was evaluated using the expression given in ASTM D638 Standard. The stress-strain curve of kenaf fibers obtained for various gage lengths are shown in Figure 3.7 and 3.7. Compliance correction was neglected during calculations, as the cross-sectional dimensions vary from fiber to fiber. Following the step by step procedure explained in [28], the uncertainty associated with the tensile modulus was quantified.

The slope and standard deviation associated with the load-deformation curve of a fiber was calculated using Equations 3.8 and 3.10,

$$m = \frac{n \sum_{i=1}^{n} x_i y_i - \sum_{i=1}^{n} x_i \sum_{i=1}^{n} y_i}{\sum_{i=1}^{n} x_i^2 - \left(\sum_{i=1}^{n} x_i\right)^2}$$
(3.8)

$$b = \frac{\sum_{i=1}^{n} y_i - m \sum_{i=1}^{n} x_i}{n}$$
(3.9)

$$S_m = \sqrt{\frac{(1-r^2)S_y^2}{(n-2)S_x^2}}$$
(3.10)

where

$$S_{xy} = \frac{1}{n-1} \left[\sum_{i=1}^{n} xy_i - \frac{\sum_{i=1}^{n} x_i \sum_{i=1}^{n} y}{n} \right]$$
(3.11)

$$S_x = \sqrt{\frac{1}{n-1} \left[\sum_{i=1}^n x - \frac{\left(\sum_{i=1}^n x\right)^2}{n} \right]}$$
(3.12)

$$S_{y} = \sqrt{\frac{1}{n-1} \left[\sum_{i=1}^{n} y_{i}^{2} - \frac{\left(\sum_{i=1}^{n} y_{i}\right)^{2}}{n} \right]}$$
(3.13)

$$r = \frac{S_{xy}}{S_x S_y} \tag{3.14}$$

The Young's modulus of a fiber in terms of stiffness is then expressed as Equation 3.15

$$E = m \frac{L}{A} \tag{3.15}$$
and the uncertainty according to TSM approach is given by Equation 3.16. The Young's modulus with associated uncertainty is shown in Figure 3.8

$$\left(\frac{u_E}{E}\right)^2 = \left(\frac{u_m}{m}\right)^2 + \left(\frac{u_L}{L}\right)^2 + \left(\frac{u_A}{A}\right)^2 \tag{3.16}$$



(b) Gage Length of 20 mm

Fig. 3.7: Kenaf Fiber Stress-Strain Curve



(d) Gage Length of 10 mm

Fig. 3.7: Kenaf Fiber Stress-Strain Curve (Contd)

Specimen	Stiffness (N/mm)	Young's Modulus (GPa)	u_E
1	1.37	34.852	0.201
2	1.47	22.949	0.133
3	1.08	30.523	0.176
4	1.70	61.567	0.356
5	2.03	22.264	0.129
6	1.87	33.037	0.191
7	1.91	31.831	0.184
8	2.99	40.203	0.232
9	2.66	34.587	0.2
10	0.74	28.701	0.166
Young's Modulus		$30.994 \pm 4.108 \text{ GPa}$	

Table 3.1: 25.4 mm

Table 3.2: 20 mm

Specimen	Stiffness (N/mm)	Young's Modulus (GPa)	u_E
1	1.79	25.694	0.148
2	2.32	35.35	0.204
3	2.82	29.80	0.172
4	1.21	11.768	0.068
5	1.85	14.753	0.085
6	2.96	23.99	0.139
7	2.72	21.026	0.121
8	2.81	16.355	0.094
9	2.18	25.57	0.148
10	2.98	27.995	0.162
Young's Modulus		$23.23\pm5.225~\mathrm{GPa}$	

Specimen	Stiffness (N/mm)	Young's Modulus (GPa)	u_E
1	6.37	19.475	0.112
2	3.62	26.106	0.151
3	1.20	31.489	0.182
4	0.99	15.03	0.087
5	2.23	20.522	0.119
6	2.42	26.98	0.156
7	1.19	12.72	0.073
8	1.06	12.061	0.07
9	0.97	12.058	0.07
10	2.21	12.004	0.07
Young's Modulus		$18.845\pm 6~\mathrm{GPa}$	

Table 3.3: 15 mm

Table 3.4: 10 mm

Specimen	Stiffness (N/mm)	Young's Modulus (GPa)	u_E
1	1.11	12.29	0.071
2	1.81	9.08	0.052
3	2.51	15.54	0.09
4	0.66	8.58	0.05
5	1.066	10.39	0.06
6	1.68	12.28	0.071
7	1.09	9.86	0.057
8	0.954	11.41	0.066
9	1.51	9.1	0.053
Young's Modulus		10.948 ± 1.694 GPa	



Fig. 3.8: Young's Modulus of Kenaf Fiber with Varying Gage Length

Remarks on Young's Modulus

- 1. The stress-strain curve of kenaf fiber exhibited linear behavior and brittle failure.
- 2. Based on the weakest links theory, the tensile strength of each fiber is influenced by the defects present along the length of the fiber and voids present in the cross sectional area. More detailed statistical analysis is presented in the next section.
- 3. The micro fibril orientation at the microscopic scale also plays a major role in the tensile properties of kenaf fiber.
- 4. The tensile modulus of kenaf fiber as a function of gage length was observed from the experiment. Though the wide variability in Young's modulus was seen in the data presented in Table 3.5-3.8, an overall observation suggests that the Young's modulus decreased with a decrease in gage length. The mean values of Young's modulus

calculated are 10.948, 18.845, 23.23 and 30.994GPa for gage lengths of 10, 15, 20, and 25.4 mm respectively.

3.2.5 Weibull Analysis for Tensile Strength of Kenaf Fiber

Kenaf fiber exhibited the brittle failure mode under tensile loading, and it was observed that the tensile strength varied among fibers. Such brittle behavior of the fiber is governed by the number of flaws present in the volume of material [29]. Strength characterization of brittle materials is mathematically expressed by a probability distribution function known as Weibull Distribution. This mathematical expression used to explain the probability of failure of a chain with n weakest links, is given as [30]

$$\phi(z) = 1 - \exp\left[\left(\frac{z - z_o}{z_s}\right)^{\beta}\right]$$
(3.17)

where z_s and β are scale and shape parameters respectively. Weibull distribution (CDF)



Fig. 3.9: Weibull Probability Density Function with Varying Shape and Scale Parameters[31] applied for strength characterization of brittle materials, based on the assumption that

the strength of a material is directly proportional to volume of flaws V, yields

$$P(\sigma) = 1 - \exp\left[-V\left(\frac{\sigma}{\sigma_o}\right)^m\right]$$
(3.18)

where $P(\sigma)$ is the probability of fiber failure below specified stress σ , V is the volume of flaws, m is the Weibull modulus and σ_o is the characteristic strength. The higher value of the Weibull modulus m signifies less variation in the tensile strength of a material. When the cross-sectional area is constant, Equation 3.18 can be modified and expressed as Equation 3.19. The average strength of the material is then evaluated as the expectation of distribution as shown in Equation 3.20.

$$P(\sigma) = 1 - \exp\left[-L\left(\frac{\sigma}{\sigma_o}\right)^m\right]$$
(3.19)

$$\bar{\sigma} = \sigma_o \Gamma \left(1 + \frac{1}{m} \right) L^{\frac{-1}{m}} \tag{3.20}$$

Application of the equation 3.19 was observed to be inadequate in characterizing the strength of Nicalon ceramic fibers with varying diameters [32]. Therefore, a three parameter model was proposed by Zhu et al. [31, 32], as shown in Equation 3.21, which takes diameter of fiber into account. The three parameters m, h, σ_o were determined from experimental data.

$$P(\sigma) = 1 - \exp\left[-Ld^h \left(\frac{\sigma}{\sigma_o}\right)^m\right]$$
(3.21)

$$\bar{\sigma} = \sigma_o \Gamma \left(1 + \frac{1}{m} \right) L^{\frac{-1}{m}} d^{\frac{-h}{m}}$$
(3.22)

In recent years, significant efforts have been made by researchers to develop the statistical model for strength characterization of natural fiber [33-37]. A Modified Weibull model was proposed by Xia et al. [38], as given in Equation 3.23, where γ accounts for diameter variation within the fiber. This model predicted the average strength of fiber more accurately than the two and three parameter model. In his study, Anderson [37] applied the Weibull of Weibull (WoW) model to characterize the strength of flax fibers. Weibull of Weibull (WoW) model was developed by Curtin [39], which accounted for incorporating the characteristic strength itself as a Weibull distribution.

$$P(\sigma) = 1 - \exp\left[-L^{\gamma} \left(\frac{\sigma}{\sigma_o}\right)^m\right]$$
(3.23)

$$\bar{\sigma_2} = \bar{\sigma_1} \left(L_2 / L_1 \right)^{\frac{-\gamma}{m}} \tag{3.24}$$

In this section, the procedure for evaluating the parameters associated with each Weibull model is explained. The computed values are presented in Table 3.5-3.7. The cumulative distribution function, corresponding to each model, was plotted against the experimental data to observe the parameter fit.

Steps for Two Parameter Model

(a)

- 1. The tensile strength of all the fibers were arranged in ascending order. The $P(\sigma)$ value corresponding to each tensile strength was estimated as $\frac{i}{N+1}$, where i=1, 2, 3, ..., N specimens.
- 2. The plot of $ln(-ln(1 P(\sigma)) ln(V)$ vs $ln(\sigma)$ was obtained for the tensile strength data, and Weibull modulus m, characteristic strength σ_o was estimated for the slope and intercept of the curve respectively as shown in Figure 3.10.
- (b)

$$P(\sigma) = 1 - \exp\left[-d^h \left(\frac{\sigma}{\sigma_o}\right)^m\right]$$
(3.25)

- 1. Assumption of constant gage length results in Equation 3.25, which implies probability of failure is function of diameter.
- 2. The value of $\frac{h}{m}$ was obtained by plotting $ln(\sigma)$ vs ln(d) and computing the slope.
- 3. A trial h value was assumed, and a $ln(-ln(1 P(\sigma))) h ln(d)$ vs $ln(\sigma)$ plot resulted in a m value, which is the slope of a line. An update h value is evaluated as m times the value obtained in Step 2. This process is continued till the h value converges.

4. The parameters h, m, σ_o were obtained for each gage length by following Steps 2 and 3 and the corresponding plots of probability distribution are shown in Figure 3.11. Cumulative distribution plots from the evaluated parameters are shown in Appendix Figure B.1

Parameters	$10 \mathrm{mm}$	$15 \mathrm{~mm}$	$20 \mathrm{~mm}$	$25.4 \mathrm{~mm}$
h/m	0.839	0.5108	1.5318	1.164
h	3.2	2.93	5.271	4.087
m	3.8148	5.7363	3.441	3.511
σ_o	36.83	93.56	5.57	16.288

Table 3.5: Two Parameter Model



Fig. 3.10: Linear Fit for Two Parameter Weibull Model



Fig. 3.11: Linear Fit for Two Parameter Weibull Model with Diameter Dependence



Fig. 3.11: Linear Fit for Two Parameter Weibull Model with Diameter Dependence (Contd)

Steps for Three Parameter Model

Based on the average strength, $\bar{\sigma} = CL^{\alpha}D^{-\beta}$, taking a logarithm on both sides

$$\ln \bar{\sigma} - \alpha \ln L = \ln C - \beta \ln D \tag{3.26}$$

$$\ln \bar{\sigma} + \beta \ln D = \ln C + \alpha \ln L \tag{3.27}$$

- 1. An assumed value of α is substituted in Equation 3.26 and β is obtained from the plot of $\ln \bar{\sigma} \alpha \ln L$ vs $\ln D$.
- 2. The obtained value of β is substituted in Equation 3.27 to obtain new α. This iteration is carried out until α and β converge. The cumulative distribution plot for all the data put together is shown in Figure 3.12. The cumulative distribution plot for consistent data is shown in Appendix Figure B.2

	α	β	h	m	σ_o
Full Data	0.2334	0.82	3.5	4.284	22.207
Consistent Data		5.41	1.95		

Table 3.6: Three Parameter Weibull Distribution Constants



Fig. 3.12: Cumulative Distribution Function of Three Parameter Model for Full Data

- 1. The plots of $ln(-ln(1 P(\sigma)) ln(V) vs ln(\sigma))$ were obtained for each gage length and Weibull modulus *m*, characteristic strengths σ_o were estimated as the slope and intercept of the line respectively.
- 2. The plots of ln (-ln (1-P)) vs ln (σ_o) were obtained for each gage length and the parameter ξ and χ were estimated as the slope and intercept of the curve respectively.
- 3. The parameters γ, α and Σ were evaluated for a batch of fibers using Equations 3.28
 3.30 given by Curtin.

$$\alpha = \frac{\xi}{\sqrt{\xi^2 + m^2}} \tag{3.28}$$

$$\rho = \frac{\xi m}{\sqrt{\xi^2 + m^2}} \tag{3.29}$$

$$\Sigma = \left[1 - \left(m^2 + \xi^2\right)^{-0.75}\right]\chi$$
(3.30)

GL	ξ	σ_o	α	ρ	Σ
25.4	3.3	1903.63	0.54	1.8	609.12
20	2.43	2271.95	0.66	1.6	449.58
15	3.74	954.11	0.49	1.86	801.77
10	2.97	1280.34	0.58	1.74	735.17

Table 3.7: Weibull of Weibull Model



(b) GL : 20 mm

Fig. 3.13: Cumulative Distribution Function of Three Parameter Model for Consistent Data



Fig. 3.13: Cumulative Distribution Function of Three Parameter Model for Consistent Data (Contd)



Fig. 3.14: Average Strength Comparison

3.2.6 Results and Discussion

- 1. The fundamental assumption of the Weibull model, that the flaws are directly proportional to the length, was observed to be inverse in this case. The tensile strength was observed to decrease with increasing volume.
- 2. The Weibull model with diameter dependence corresponds well with individual fiber lengths as observed in Figure B.1
- Parameters determined for the three parameter Weibull model, given by Equation
 3.26 for full tensile strength data of fiber batch, fits well with 10, 15, 20 mm gage lengths, as shown in Figure 3.12.
- 4. Tensile strength of fibers with consistent Young's modulus was selected and a three parameter model was fit as shown in Figure B.2. The difference in the fit is due to the varying diameter from fiber to fiber.

- 5. Parameters for Weibull of Weibull model (WoW), was observed to fit with the tensile strength data of 25.4 and 20mm gage length, whereas for 15 and 10 mm there was a wide range of discrepancy. This suggests that there is less scatter in data for 10 and 15 mm fibers compared to that of 25.4 and 20mm.
- Average tensile strength predicted from WoW models are very similar to the experimental data as observed in Figure 3.14

3.3 Tensile Modulus of Kenaf Fiber Composite

In this section, the preparation of a tensile specimen and evaluation of tensile modulus is discussed. The tensile modulus and Poisson's ratio of epoxy matrix was evaluated through tensile tests and a similar procedure was carried out on a kenaf composite specimen.



3.3.1 Specimen Preparation

Fig. 3.15: Mold for Casting Tensile Specimens

Epoxy

The epoxy resin PT2050 and hardener B1 were obtained from PTMW industries. Their density is 0.9 g/cc. The resin and hardener were mixed in 100:27 proportion according to the manufacturer's specification. A mold, as shown in Figure 3.15, was designed for casting 10 tensile specimens at a time and, the dimensions of the specimen were selected from ASTMD638 type I. Mixed epoxy was pour into mold and left for curing in oven at 80°C for 12 hrs. Meniscus formed on top of the epoxy matrix sample was grinded using 320, 600, 1200 grit sand paper, until the specimen was flat. The epoxy samples before and after grinding is shown in Figure 3.16.



(a) Before Grinding (b) After Grinding Fig. 3.16: Epoxy Samples

Kenaf Composite

The kenaf fibers were chopped to a length of 10-15 mm and soaked in a 3% Na OH solution for 12 hrs to remove any impurities present on the surface of the fiber. The Na OH solution was then drained and the fibers were oven dried at 80°C for 8 hrs. The dried kenaf fibers were shredded using carding brushes and mixed with an epoxy matrix such that a 22% fiber volume fraction was maintained. The mixture was then placed in mold as shown in Fig 3.17. Pressure was applied to the composite mix by tightening the clamps and left for curing at 80°C for 12 hrs. An attempt was made to cast the composite through vacuum bagging as shown in Fig 3.18. This process proved problematic, as there was no way of ensuring a flat top surface in the end product.



Fig. 3.17: Casting Kenaf Fiber Composite Sample



(c) Removal of Resin during Curing (d) Kenaf Composite Plate After Curing

Fig. 3.18: Processing of Kenaf Fiber Composite Plate Using Vacuum Bagging Technique

3.3.2 Experimental Setup

The experimental setup for tensile tests included load cell, strain gage and Vernier calipers as measuring devices to measure force, strain and specimen dimensions respectively.

Tests were performed on the Tinius Olsen tensile tester (Figure 3.19(a)) available in the material science lab at USU. In general, when tensile tests are performed on this machine force and extension readings are obtained through the Navigation software provided by the manufacturer. In this study, a data acquisition system was designed to acquire force and strain readings (axial and transverse) through a NI 9237 module, as shown in Figure 3.19(c). The NI 9237 module reads the Wheatstone bridge output in terms of voltage and converts to the desired unit such as force and strain.





(a) Tinius Olsen Testing Machine Frame (b) Load Cell on Machine Frame



(c) DAQ setup Fig. 3.19: Tensile Test Setup

3.3.3 Load Cell and Calibration

The load cell, shown in Figure 3.19(b) attached to the testing machine is an S-shaped bending load cell constructed on the principle of a Wheatstone full bridge. The output terminal of the load cell is a 15 pin D-sub connector, with only four pins associated with the bridge terminals. The connection details are shown in Figure 3.20. A calibration curve was generated using a bridge (mV/V) module of NI 9237 as explained below. The load cell was built based on the Wheatstone bridge principle. The output is read in millivolts. The change in the voltage of the bridge is proportional to the load applied. A calibration curve was generated for the load cell using the output (millivolts) of bridge for corresponding calibrated loads applied as shown in Figure 3.21(c). This procedure involved obtaining voltage readings for both loading and unloading of calibrated loads (Figure 3.21(d)) using LabVIEW generated code Figure 3.21(a)-3.21(b). A least squares linear regression method was applied to the calibration data and the resulting voltage-force conversion equation was obtained as shown in Figure 3.22. This equation was needed as input for NIDAQ9237 to convert the bridge output (millivolts) to Newton while performing tensile tests.



Fig. 3.20: Load Cell Wiring Diagram



(b) Bridge Module Dialog Box





OK.

(d) Loading and Unloading Diagram during Calibration

Fig. 3.21: Load Cell Calibration



Fig. 3.22: Calibration Curve

3.3.4 Tensile Testing

The epoxy samples and kenaf composite samples were prepared as explained in the previous section and strain gages were fixed on the sample as shown in Figure 3.23. The LabVIEW code is presented in Appendix Figure B.3. The sample was aligned in the loading direction and the grips were fixed tightly enough to prevent slipping. The testing speed was set to 8 mm/min as per ASTM standards and the tensile test was performed until the specimen failed. The specimens that failed during the test are shown in Figure 3.23 and Figure 3.24. The Young's modulus and Poisson's ratio was calculated using the procedure explained in [28] and presented in Table 3.8. A similar procedure was followed for determining kenaf fiber composite properties and the properties of the material were presented in Table 3.10.



Fig. 3.23: Epoxy Tensile Test



Fig. 3.24: Kenaf Fiber Composite Tensile Test

Specimen No.	Youngs Modulus, E (GPa)	u_E	Poisons Ratio, ν	u_{ν}
1	2.804	0.022016	0.388	0.00194
2	2.652	0.020864	0.3704	0.001852
3	2.815	0.022339	0.3793	0.001897
4	2.821	0.02223	0.38	0.0019
5	2.912	0.023392	0.3817	0.001909

 Table 3.8: Epoxy Matrix Properties

Table 3.9: Uncertainties Associated with Kenaf Composite Geometry

Specimen No.	Width (W)	Thickness (T)	u_W	u_T	$Area(mm^2)$	u_A
1	12.938	5.578	0.03195	0.04445	72.168	0.00834
2	12.858	4.956	0.01538	0.02216	63.724	0.00463
3	12.74	6.624	0.02612	0.08002	84.39	0.01225
4	12.804	6.226	0.01315	0.0869	79.718	0.014
5	12.962	5.504	0.01713	0.04947	71.343	0.00908
6	12.82	5.42	0.01508	0.00892	69.484	0.00202
7	12.832	5.354	0.01931	0.01357	68.703	0.00295
8	12.922	4.459	0.01056	0.01282	57.619	0.00299
9	12.824	5.258	0.0084	0.01056	67.429	0.00211
10	12.865	4.976	0.00908	0.01128	64.016	0.00238
11	12.9	5.228	0.02133	0.02257	67.441	0.00462
12	12.855	4.719	0.01359	0.0291	60.663	0.00626
13	12.9	4.712	0.00954	0.0245	60.785	0.00525
14	12.924	4.434	0.005	0.03818	57.305	0.00862

Specimen No.	Young's Modulus, E (GPa)	u_E	Poisons Ratio, ν	u_{ν}
1	6.917	0.07911	0.33	0.001639
2	8.89	0.080839	0.43	0.002138
3	6.213	0.090316	0.43	0.002157
4	6.613	0.106066	0.28	0.001378
5	4.923	0.059014	0.3	0.001503
6	6.795	0.054922	0.35	0.001728
7	5.875	0.049137	0.29	0.00147
8	7.369	0.061737	-	-
9	6.938	0.056237	0.38	0.001935
10	6.154	0.05034	0.29	0.00149
11	6.132	0.055729	0.28	0.001414
12	5.87	0.058828	0.33	0.00165
13	6.995	0.065921	0.32	0.001584
14	5.094	0.059308	0.28	0.00141
Young's Modulus	$6.48\pm0.572~\mathrm{GPa}$			

Table 3.10: Kenaf Fiber Composite Properties

3.3.5 Results and Discussion



Fig. 3.25: Stress-Strain Diagram Kenaf Fiber Composite



Fig. 3.25: Stress-Strain Diagram Kenaf Fiber Composite (Contd)

- 1. Kenaf fiber composite exhibited linear brittle failure with a tensile strength in the range of 20-38 MPa. These values are comparable to that of the tensile strength of other natural fiber composites published in [40, 41].
- The Poisson's ratio varied from specimen to specimen and in the range of 0.28 to 0.43. A possible reason might be the fiber orientation and inconsistent micro-structure at the point where the strains were measured.
- 3. The Young's modulus of the composite was observed to be 6.48 *pm* 0.572 GPa for 22% volume fraction of kenaf fibers and comparable to that of glass fiber composites, as published in [42].
- 4. The lower tensile strength of the composite was attributed to the amount of voids present in the specimen due to insufficient pressure applied while casting specimen.

Chapter 4 Numerical Modeling

4.1 Introduction

This chapter presents the numerical model of a kenaf short fiber reinforced composite in order to predict the effective properties. Finite element method was applied as a computational tool to accomplish this objective. To predict the effective properties of a unidirectional kenaf fiber composite, a two-step numerical homogenization was carried out on a unit cell at the micro- and meso-scales. The following sections of this chapter explain the finite element formulation for the homogenization of a unit cell, Von-Mises Fisher probability distribution and the orientational averaging technique to determine the effective properties of the short fiber composite.

The unit cell is defined as the smallest repetitive part of the structure, as shown in Figure 4.1. In the field of composites, it is a very common practice to assume that the fibers are periodically arranged as a reinforcement in a matrix at the micro-scale. This leads to two types of basic unit cell models, square and hexagonal, which have been studied by various researchers [43-46] in the past. The volume fraction of constituents in a unit cell is same as that of a composite. The motivation behind selecting the unit cell was to reduce the computational effort involved in analyzing the whole micro-structure. The appropriate boundary conditions [46] were applied to the unit cell and a stress-strain field was predicted, leading to evaluation of macroscopic (homogenized) properties. The influence of homogeneous and periodic boundary conditions on unit cell was described in [47, 48], which proved that the former is an over-constrained boundary condition.

Macroscopic stress (Σ) is defined as the volumetric average of a microscopic stress (σ_{ij}) field in a body subjected to a uniform macroscopic strain (E). The macroscopic properties of a material can be derived from the analysis of microscopic structure once the

properties of constituents at the microscopic scale are known. The average stress and strain is mathematically expressed as [49]

$$\Sigma = \frac{1}{V} \int\limits_{V} \sigma_{ij} dV \tag{4.1}$$

$$E = \frac{1}{V} \int_{V} \epsilon_{ij} dV \tag{4.2}$$



Fig. 4.1: Schematic Representation of Unit Cell

4.2 Finite Element Formulation

The principle of virtual work is applied to derive the general finite element equations, which is defined as [49]

Among all admissible configurations of a conservative system, those that satisfy the equations of equilibrium make the potential energy stationary with respect to small admissible variations of displacement. The minimization of potential energy $\delta \Pi = \delta U - \delta W$ results in

$$\delta U = \delta W \tag{4.3}$$

where δU is internal strain energy and δW is external work done given by Equation 4.4 and 4.5 respectively.

$$\delta U = \iiint_V \delta \varepsilon^T \sigma \, dV \tag{4.4}$$

$$\delta W = \iint_{S} \delta \psi_{s}^{T} T \, dS + \iiint_{V} \delta \psi^{T} X \, dV + \delta d^{T} P \tag{4.5}$$

where $\delta \varepsilon$, δd , $\delta \psi_s$, $\delta \psi$ vector of virtual strains, virtual nodal displacements, virtual displacement function δu , δv , δw and virtual displacement functions acting over surface P, X, T are vectors of applied nodal loads, body forces and surface tractions. $\psi = Nd$ and $\psi_s = N_s d$, $\varepsilon = Bd$ and $\sigma = D\varepsilon$. Substitution of Equation 4.4 and 4.5 in Equation 4.3 results in

$$\delta d^T \iiint_V B^T D B dV d = \delta d^T \iint_S N_S^T T dS + \delta d^T \iiint_V N^T X dV + \delta d^T P$$
(4.6)

Neglecting body forces,

$$[K]d = [P] + [f_s] \tag{4.7}$$

where stiffness matrix $[K] = \iiint_V B^T D B d V$

and [P] is load vector

Equivalent nodal loads due to surface forces $[f_s] = \iint_S N_S^T T dS$

Lagrange Multipliers to Enforce Constraints

The minimization of a potential energy subjected to constraint was solved using the Lagrange multiplier method. Mathematically, the problem was addressed as shown in Equation 4.8, where constraint equation G is added to the potential energy.

$$L = \Pi + \lambda G \tag{4.8}$$

where L is the Lagrangian function, Π is potential, λ is Lagrange multiplier and, G = [C]d - [Q] is constraint equation. Minimization of Lagrangian with respect to 'd', i.e. $\frac{\partial L}{\partial d} = 0$ and λ i.e. $\frac{\partial L}{\partial \lambda} = 0$ results in the system of equations put in broad form [49], as shown in Equation 4.9.

$$\begin{bmatrix} K & C^T \\ C & 0 \end{bmatrix} \begin{cases} d \\ \lambda \end{cases} = \begin{bmatrix} P \\ Q \end{bmatrix}$$
(4.9)

Hexahedral Element

A Hexahedral element, also known as 8-noded brick element, is one of the 3D discretized elements frequently used in the finite element analysis of a structure. Each node in this element is associated with three degrees of freedom u, v, w in x, y, z directions respectively as shown in Figure 4.2.



Fig. 4.2: Hexahedral Element

The associated shape functions for the element, with r_i, s_i, t_i as the values of natural coordinates:

$$N_i = \frac{1}{8}(1 + rr_i)(1 + ss_i)(1 + tt_i)$$
(4.10)

The Jacobian and B matrix were computed as:

$$\begin{bmatrix} x_{,r} & y_{,r} & z_{,r} \\ x_{,s} & y_{,s} & z_{,s} \\ x_{,t} & y_{,t} & z_{,t} \end{bmatrix} = \sum \begin{bmatrix} N_{i,r}x_i & N_{i,r}y_i & N_{i,r}z_i \\ N_{i,s}x_i & N_{i,s}y_i & N_{i,s}z_i \\ N_{i,t}x_i & N_{i,t}y_i & N_{i,t}z_i \end{bmatrix}$$
(4.11)

where $[\Gamma]$ is inverse Jacobian matrix. The numerical computation of the stiffness matrix for a single element is written as

$$[K] = \int_{-1}^{1} \int_{-1}^{1} \int_{-1}^{1} B^{T} DB |J| dr ds dt$$
(4.13)

Average stress in numerical form is written as

$$\Sigma = \frac{\sum_{k=1}^{No \, of \, Elem} \sigma_{ij}^k}{V} \tag{4.14}$$

Average strain in numerical form is written as

$$E = \frac{\sum_{k=1}^{No \, of \, Elem} \epsilon_{ij}^k}{V} \tag{4.15}$$

Algorithm:

- 1. Input: 'Nodal Coordinates', 'Element Connectivity', 'Material Properties', and 'Boundary Conditions' from text file.
- 2. Compute stiffness matrix of each element.
- 3. Assembling stiffness matrix of whole structure.
- 4. Partitioning of stiffness matrix into known and unknown degrees of freedom.
- 5. Solving system of equations using 'UMFPACK' algorithm in SCILAB.
- 6. Recovery of displacements, strains and stress.
- 7. Computing average stress and strain of the unit cell.

4.3 Boundary Conditions

In order to obtain the macroscopic properties of the cell wall layer and unidirectional fiber composite, the unit cell was subjected to four load cases: axial, transverse, longitudinal shear and transverse shear, as the material is transversely isotropic. For axial and transverse loading, a quarter model was selected due to its symmetry, as shown in Figure 4.3(a). Transverse shear was simulated by applying periodic boundary conditions to the 2D model as shown in Figure 4.3(b). The boundary conditions applied to the unit cell as explained in [46] are described in Table 4.1.



Fig. 4.3: Schematic Representation of Boundary Conditions on the Model

Load Case	U_x	U_y	U_z
Arrial	$U_x(0,y,z) = 0$	$U_y(x,0,z) = 0$	$U_z(x,y,0) = 0$
Axiai	$U_x(a, y, z) = 0.0005$	$U_y(x,b,z) = \delta$	$U_z(x, y, c) = \delta$
Troport	$U_x(0,y,z) = 0$	$U_y(x,0,z) = 0$	$U_z(x,y,0) = 0$
Transverse	$U_x(a, y, z) = \delta$	$U_y(x,b,z) = 0.0005$	$U_z(x, y, c) = \delta$
	$U_x(0, y, z) = U_x(2a, y, z)$	$U_y(0, y, z) = U_y(2a, y, z)$	$U_z(0, y, z) = U_z(2a, y, z)$
Longitudinal Shear	$U_x(x,0,z) = 0$	$U_y(x,0,z) = 0$	$U_z(x,0,z) = 0$
	$U_x(x, 2b, z) = 0.0005$		

Table 4.1: Boundary Conditions

For longitudinal shear, the face at y=0 is fixed and the displacement Ux, Uy, Uz on the faces x=0 and x=a are kept same. To obtain shear modulus, a constant displacement is applied on the face y=a in the x-direction (shear loading). [Uy, Uz] on L = [Uy, Uz] on R; [Uy, Uz] on T = [Uy, Uz] on B are the periodic boundary conditions in the case of transverse shear as shown in Figure 4.3(b), where L, R, T and B stand for left, right, top and bottom surfaces respectively and the periodic boundary conditions are applied to all the nodes on these surfaces. To simulate transverse shear, a displacement of delta y is applied at (a, h).

4.4 Effective Properties of Cell Wall Layers in Bast Fiber

At the mesoscopic scale, all the bast fibers possess a bundle of laminated tube-like structures (Figure 3.2(b)) called cell walls. Each cell wall is made of Primary, Secondary S1, S2 and S3 layers and mechanical properties of each constituent in these layers are given in Table 4.2. The thickness of each layer differs from the others with S2 layer occupying 80% of the total thickness of the cell wall [51]. The thicknesses of each layer in a cell wall, obtained from [51], are presented in Table 4.3.

The purpose of this section is to evaluate the effective properties (independent elastic constants) of a secondary cell wall layer with a varying volume fraction of basic constituents (C, HC, L). The volume fraction of basic constituents in different bast fibers are given in [52]. Some volume fraction combinations chosen for the analysis are given in Table 4.4. There are

Material	$E_{11}(MPa)$	$E_{22}(MPa)$	$G_{12}(MPa)$	ν_{12}	ν_{23}
Cellulose	138000	27200	4400	0.235	0.48
Hemi-Cellulose	7000	3500	1800	0.2	0.4
Lignin	2000	2000	770	0.3	0.3

Table 4.2: Elastic Constants of Constituents [11]

 Table 4.3: Structural Dimensions

Layer	Thickness (μm)	$MFA(^{\circ})$
М	0.25	-
Р	0.1	-
S1	0.3	$\pm 50^{\circ}$ -70°
S2	4	0° - 30°
S3	0.04	$\pm 60^{\circ}$ - 80°

n number of combinations possible and it is impractical to determine the effective properties for each combination through numerical experiments. From the structural point of view, the orientation of fibrils, volume fraction and geometry of constituents are all that is required to evaluate the effective properties. A schematic representation of cell wall layers with constituents is shown in Figure 4.4. The shape of cellulose and arrangement of constituents are of significance in the analysis. The shape of cellulose was found to be square with the work carried out by O'Sullivan [53]. Regarding the arrangement of these constituents, the results presented by Salmen and Preston [54] are also of importance.

Table 4.4: Volume Fraction of Constituents

S.No	Vcellulose	Vhemicellulose	Vlignin
1	50	27	23
2	55	24	21
3	60	23	17
4	65	20	15
5	70	17	13



Fig. 4.4: Basic Constituents in Cell Wall Layer

Geometry and Meshing

The full 3-dimensional unit cell geometry of the cell wall layer is shown in Figure 4.5. All the constituents are assumed to be square in shape. This assumption results in transversely isotropic properties of an equivalent material with five independent elastic constants. Therefore, five numerical tests were performed to obtain five independent elastic constants. The unit cell geometry was created in the Gmsh meshing software. Gmsh is a 3D mesh generator software developed by Geuzaine and Remacle[55] with the capability of meshing 2D and 3D geometries using different kinds of elements. The Gmsh code was written to create the geometry of a unit cell with a specified mesh size. Also, Gmsh facilitates the option of selecting the number of elements/division along the line during discretization. The application of periodic boundary conditions requires the nodal points to be exactly on the opposite face. Orderly numbering and transfinite algorithms are built-in functions of Gmsh software, facilitating the use of the structured meshes required to implement periodic boundary conditions. The mesh file .msh was generated from Gmsh, which comprises nodal coordinates and element connectivity. A pseudo code is shown in Table 4.5) and the mesh


file .msh for a simple geometry is presented in Appendix Table ??.

Fig. 4.5: 3D Unit Cell Geometry



Fig. 4.6: Node Numbering of Unit Cell Geometry

The 3D quadrant model created to simulate axial and transverse load case is shown in Figure 4.7. The geometry of the longitudinal shear model is similar to the one shown in Figure 4.5.

Table 4.5: Mesh Format File

\$MeshFormat
version-number file-type data-size
\$EndMeshFormat
\$Nodes
number-of-nodes node-number x-coord y-coord z-coord
...
\$EndNodes
\$Elements
number-of-elements
elm-number elm-type number-of-tags < tag > ... node-number-list
...
\$EndElements



Fig. 4.7: Quadrant Unit Cell Model

FE Analysis and Results

A 3D finite element code was developed in a SCILAB environment, which requires an input file .msh generated from Gmsh, to compute the effective properties. The input files required to run finite element analysis were generated by the program created to read .msh file as shown in Appendix C.1 and boundary conditions (constraints) were generated by the code sortingsurface.sci (Appendix C.3). The full 3D FE code is presented in Appendix C.4. A finite element code was run for each load case, according to the boundary conditions specified in the previous section. Stress and displacement contour obtained for each load case are shown in Figure 4.8-4.10. The computed elastic constants through the 3D finite element unit cell model are compared with those values presented in [51] and semi-empirical relations given in Equations 4.16-4.19. Comparisons of elastic constants with various methods are



presented in Tables 4.6-4.8, corresponding to each secondary layer and Figure 4.11-4.14.

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Fig. 4.9: Transverse Load Case



Fig. 4.10: Longitudinal Shear Loading

Semi-Empirical Equations:

Rule of Mixtures:

$$E_{11} = V_C E_{11}(C) + V_{HC} E_{11}(HC) + V_L E_{11}(L)$$
(4.16)

Tsai-Hahn Empirical Relation:

$$\frac{1}{E_2} = \frac{1}{V_f + \eta V_m} \left(\frac{V_f}{E_f} + \eta \frac{V_m}{E_m} \right); \eta = 0.5$$
(4.17)

$$\frac{1}{G_{12}} = \frac{1}{V_f + \eta V_m} \left(\frac{V_f}{G_f} + \eta \frac{V_m}{G_m} \right); \eta = 0.5 \left(1 + \frac{G_m}{G_f} \right)$$
(4.18)

Halpin-Tsai Empirical Relation:

$$\frac{E_f}{E_m} = \frac{1 + \eta \Psi V_f}{1 - \eta V_f}; \eta = \frac{\gamma - 1}{\gamma + \Psi}; \gamma = \frac{E_f}{E_m};$$
(4.19)

The Young's modulus in the axial direction and Poisson's ratio computed by all the methods are in good agreement and match exactly with the Rule of Mixtures. Transverse modulus compared from the Tsai-Hahn relation is in good agreement with 3D FE results, whereas the multi-pass homogenization procedure gives an error of 17%. The Halpin-Tsai relation involves a parameter that is dependent on the geometry of the fiber and can be derived if the exact results are known. Here the values compared in the Table 4.6-4.8 are computed using $\Psi = 2$ and the error was observed to be around 7%. Based on 3D results and through inverse calculations, Ψ was found to be 1.58 for the transverse modulus and 0.9 for the shear modulus in this particular problem. The Tsai-Hahn equation, in conjunction with the Rule of Mixtures, results in the elastic constants approximate to 3D results. After validating the existing semi-empirical relations with those of the 3D results for a set of combinations, these equations can be directly applied to derive effective elastic constants.

Given any natural fiber, the geometric parameters that play a major role in the cell wall tube properties are: micro fibril orientation, thickness of each layer and the cross-sectional shape. In the next section, the micro fibril orientation in the S2 layer and volume fraction of the constituents were varied to obtain the effective properties of composite.



Fig. 4.11: Young's Modulus in Axial Direction



Fig. 4.12: Young's Modulus in Transverse Direction



Fig. 4.13: Longitudinal Shear Modulus



Fig. 4.14: Poisson's Ratio from 3D Model

Elastic Constant	3D FEM	Leon	Halpin-Tsai	Tsai-Hahn
$E_{11}(GPa)$	51.1	51.1	51.1	51.1
$E_{22}(GPa)$	5.131	4.28(16.6%)	5.441	5.138
$G_{12}(GPa)$	1.695	1.71	-	1.648
ν_{12}	0.245	0.25	0.247	0.247
ν_{23}	0.3384	0.34	-	-

Table 4.6: Comparison of Elastic Constants in S1 Layer

Table 4.7: Comparison of Elastic Constants in S2 Layer

Elastic Constant	3D FEM	Leon	Halpin-Tsai	Tsai-Hahn
$E_{11}(GPa)$	71.35	71.35	71.35	71.35
$E_{22}(GPa)$	7.171	5.86(18.2%)	7.628	7.087
$G_{12}(GPa)$	2.13	2.15	-	2.087
ν_{12}	0.239	0.24	0.24	0.24
ν_{23}	0.3105	0.35	-	-

Table 4.8: Comparison of Elastic Constants in S3 Layer

Elastic Constant	3D FEM	Leon	Halpin-Tsai	Tsai-Hahn
$E_{11}(GPa)$	64.95	64.95	64.95	64.95
$E_{22}(GPa)$	6.704	5.56(17%)	7.164	6.698
$G_{12}(GPa)$	2.118	2.14	-	2.08
ν_{12}	0.235	0.24	0.235	0.235
ν_{23}	0.3198	0.36	-	-

Volume Fraction(C/HC/L)	$E_{11}(GPa)$	$E_{22}(GPa)$	$G_{12}(GPa)$	ν_{12}	ν_{23}
50/27/23	71.35	7.171	2.13	0.2391	0.3018
55/24/21	78	7.914(17%)	2.55	0.2389	0.2932
60/23/17	84.75	8.919	2.446	0.2372	0.2961
65/20/15	91.4	9.974	2.68	0.2369	0.2962
70/17/13	98.1	11.135	2.83	0.2367	0.2979

Table 4.9: Elastic Constants with Varying Volume Fractions

4.5 Effective Properties of Unidirectional Composite

The properties of the cell wall layers obtained in previous section were used in the second step of homogenization which was carried out at mesoscopic scale. The elastic constants of each layer in a cell wall for various volume fractions of basic constituents were used in performing parametric modeling of the unit cell of unidirectional kenaf fiber composite properties. It was assumed that the bond between matrix and fiber was perfect and the fibers were straight (without any flaws) in the finite element analysis of a unit cell. The fiber volume fraction was considered to be 22% in this analysis as the experiments were carried out at the same volume fraction.

Geometry and Meshing

The cell wall geometry was assumed to be hexagonal in shape with the dimensions adopted from [50] and shown in Table 4.3. The basic geometrical parameters required to create hexagon is shown in Figure 4.15. The parameter θ in the Gmsh code is a shape factor, meaning that at $\theta = 30^{\circ}$ the geometry of the cell wall is a regular hexagon and changes to an irregular hexagon at other θ values. A fiber geometry consisting of a bundle of seven cell walls and the periodic arrangement of fibers in the matrix was created in Gmsh is shown in Figure 4.16 and 4.17 respectively. The unit cell geometry meshed with hexahedral elements is shown in Figure 4.18. As the properties obtained were invariant of length (Z-dir), the unit cell thickness and number of elements in thickness direction was kept about (1/10)th of cross-sectional dimension, which reduced the computation effort.



Fig. 4.15: Basic Hexagonal Shaped Cell

$$P = [A + B\sin(\theta), 0]; \quad Q1 = Q + \left[\frac{T}{\tan(\theta_1)}, T\right]$$
$$P1 = P + \left[\frac{T}{\cos(\theta)}, 0\right]; \quad R = [0, B\cos(\theta)];$$
$$Q = [A, B\cos(\theta)]; \quad R1 = R + [0, T];$$



Fig. 4.16: Schematic Representation of Bundle of Cell Walls



Fig. 4.17: Periodic Arrangement of Natural Fiber in a Matrix

FE Analysis and Results

The secondary layers of the cell wall consist of micro fibrils with orientations varying as follows: $S1:50^{\circ}-70^{\circ}$, $S2:0^{\circ}-30^{\circ}$, $S3:60^{\circ}-80^{\circ}$. A finite element analysis by Qing et.al. [50], concluded that the fiber properties in the axial direction were strongly influenced by the S2 layer properties. Therefore, only S2 layer micro fibril orientation was varied from 0° to 30° with an interval of 5° , whereas the S1 and S3 layer orientation was fixed as 70° and 80° respectively throughout the analysis. The effect of S1 and S3 micro fibril orientation on the transverse modulus was observed to be minimal (5%)[50]. In this section, for varying volume fractions of basic constituents (Table 4.4), the MFA in the S2 layer was varied and FE analysis was carried out to obtain the macroscopic properties of the unit cell shown in Figure 4.18. As explained in [50], for layer S1 and S3, the bidirectional reinforcement was considered and the properties of these layers were calculated in a similar way. The properties are listed in Table 4.10.



Fig. 4.18: Unit Cell of Natural Fiber Reinforced Unidirectional Composite

Table 4.10: Materia	l Properties	of Each Layer in	Cell Wall Except S2 (2)	MPa)
---------------------	--------------	------------------	-------------------------	------

Layer	E_{XX}	E_{YY}	E_{ZZ}	G_{YZ}	G_{ZX}	G_{XY}	$ u_{XY} $	ν_{XZ}	ν_{YZ}
M	2820	2820	2820	1084.62	1084.62	1084.62	0.3	0.3	0.3
P	3970	3970	3970	1550.78	1550.78	1550.78	0.28	0.28	0.28
S_1	12845.03	4280	4134.92	1598.67	2213.2	1694.4	0.175	0.157	0.292
S_3	35593.55	5560	5475.36	2046.88	2291.83	2136.98	0.197	0.063	0.34



(b) Displacements U_z

Fig. 4.19: Axial Load Case



(b) Displacements $U_{\boldsymbol{y}}$

Fig. 4.20: Transverse Load Case

Augmented Lagrange Method

The Lagrange multipliers method introduces more equations (equal to constraint equations) to the stiffness matrix and the diagonal terms of the matrix go to zero, implying that the matrix becomes nearly singular. Therefore, an Augmented Lagrange method was applied to obtain a new stiffness matrix, that is a combination of penalty and Lagrange multiplier methods and was solved for displacements. The potential energy functional with perturbed constraint equation is given by

$$\Pi = \frac{1}{2}D^T K D - D^T F + \frac{1}{2}\alpha \left(penalty function - perturbation\right)^2$$
(4.20)

Let g(D) be the penalty function ([C][D]-[Q]=0) and δ is perturbation

$$\Pi = \frac{1}{2}D^{T}KD - D^{T}F + \frac{1}{2}\alpha \left(g(D) - \delta\right)^{2}$$
(4.21)

After expansion of the third term and neglecting delta square term, the final equation takes the following form:

$$\Pi = \frac{1}{2}D^T K D - D^T F + \frac{1}{2}g^T \alpha g - g^T \alpha \delta$$
(4.22)

replacing $\alpha \delta$ by λ^T

$$\Pi = \frac{1}{2}D^T K D - D^T F + \lambda^T g + \frac{1}{2}g^T \alpha g$$
(4.23)

The first three terms represents the Lagrange multiplier formulation and fourth term is the penalty function augmented. Minimization of potential w.r.t D results in

$$\left[K + C^T \alpha C\right] D = F + C^T \alpha Q - C^T \lambda^P \tag{4.24}$$

and considering the last two terms of Equation 4.23 as equivalent to Lagrange formulation

$$\lambda^P = \lambda^i + \alpha g(D) \tag{4.25}$$

Algorithm to obtain λ

- 1. Input : α (penalty factor), tolerance
- 2. While $\lambda \lambda^P \leq$ tolerance { Calculate D using Equation 4.24 Substitute D in Equation 4.25 to obtain updated λ value. }



Fig. 4.21: Longitudinal Shear

Transverse Shear Simulation

The transverse shear properties of a composite were obtained by subjecting the unit cell to the periodic boundary conditions, as explained in [46]. The implementation of periodic boundary conditions involves the nodes on opposite faces (i.e. i-j and k-l) as shown in Figure 4.22. These nodes were subjected to same displacements. In order to prevent rigid body motion during the finite element simulation, the following boundary conditions were imposed on the unit cell.

Boundary conditions

$$\begin{aligned} v(x, -b) &= v(x, b); \\ u(-a, y) &= u(a, y); \\ v(-a, y) &= v(a, y) = 0 \\ v(-a, -b) &= v(a, -b) = v(a, b) = v(-a, b) = 0 \\ u(x, b) &= \delta; u(x, -b) = -\delta \end{aligned}$$



Fig. 4.22: Periodic Boundary Conditions for Transverse Shear

The variation of elastic properties as a function of the MFA in the S2 layer are shown in Figure 4.24. The axial modulus of the composite decreased with an increase in MFA of the S2 layer. The transverse modulus and transverse shear modulus were least effected. The transverse modulus decreased by 7%. For the transverse loading case, the maximum stress in y-direction was observed at the point P of the hexagon, as shown in Figure 4.20(a). For the transverse shear load, maximum shear stress was observed on the interface between matrix and cell wall layer P1 and Q1, as shown in Figure 4.23(a).



Fig. 4.23: Transverse Shear

	Volume fraction $50/27/23$												
θ	Ezz	Exx	Eyy	Gxy	Gzy	Gzx	ν_{zx}	$ u_{zy} $	$ u_{yx}$				
0	6.4660	3.1690	3.1690	1.0613	2.7490	2.7490	0.3700	0.3700	0.4930				
5	6.0720	3.1530	3.1530	1.0609	2.7910	2.7910	0.3720	0.3720	0.4860				
10	5.2650	3.1130	3.1130	1.0610	2.8790	2.8790	0.3748	0.3748	0.4670				
15	4.5280	3.0660	3.0660	1.0614	2.9620	2.9620	0.3770	0.3770	0.4443				
20	3.9930	3.0240	3.0240	1.0625	3.0200	3.0200	0.3790	0.3790	0.4230				
25	3.6290	2.9910	2.9910	1.0631	3.0600	3.0600	0.3790	0.3790	0.4067				
30	3.3840	2.9690	2.9690	1.0649	3.1400	3.1400	0.3800	0.3800	0.3940				

Table 4.11: Elastic Constants (GPa) at Volume Fraction 50/27/23

	Volume fraction $55/24/21$												
θ	Ezz	Exx	Eyy	Gxy	Gzy	Gzx	ν_{zx}	$ u_{zy} $	$ u_{yx}$				
0	6.8200	3.2280	3.2280	1.0589	2.7880	2.7880	0.3703	0.3703	0.4964				
5	6.4170	3.2120	3.2120	1.0587	2.8280	2.8280	0.3718	0.3718	0.4891				
10	5.5700	3.1720	3.1720	1.0584	2.9120	2.9120	0.3747	0.3747	0.4706				
15	4.7730	3.1230	3.1230	1.0587	2.9910	2.9910	0.3774	0.3774	0.4480				
20	4.1810	3.0790	3.0790	1.0598	3.0470	3.0470	0.3792	0.3792	0.4267				
25	3.7730	3.0420	3.0420	1.0614	3.0820	3.0820	0.3801	0.3801	0.4090				
30	3.4980	3.0170	3.0170	1.0636	3.0990	3.0990	0.3803	0.3803	0.3957				

Table 4.12: Elastic Constants (GPa) at Volume Fraction 55/24/21

Table 4.13: Elastic Constants (GPa) at Volume Fraction 60/23/17

	Volume fraction $60/23/17$												
θ	Ezz	Exx	Eyy	Gxy	Gzy	Gzx	$ u_{zx}$	$ u_{zy}$	$ u_{yx}$				
0	7.1800	3.2560	3.2560	1.0556	2.7780	2.7780	0.3701	0.3701	0.5011				
5	6.7040	3.2390	3.2390	1.0559	2.8240	2.8240	0.3718	0.3718	0.4931				
10	5.7340	3.1940	3.1940	1.0566	2.9160	2.9160	0.3752	0.3752	0.4731				
15	4.8510	3.1410	3.1410	1.0581	3.0000	3.0000	0.3782	0.3782	0.4488				
20	4.2140	3.0920	3.0920	1.0603	3.0570	3.0570	0.3802	0.3802	0.4261				
25	3.7850	3.0530	3.0530	1.0625	3.0920	3.0920	0.3812	0.3812	0.4075				
30	3.4990	3.0250	3.0250	1.0652	3.1100	3.1100	0.3814	0.3814	0.3936				

	Volume fraction $65/20/15$													
θ	Ezz	Exx	Eyy	Gxy	Gzy	Gzx	ν_{zx}	$ u_{zy} $	$ u_{yx}$					
0	7.5340	3.2820	3.2820	1.0902	2.7990	2.7990	0.3699	0.3699	0.5052					
5	7.0200	3.2650	3.2650	1.0932	2.8440	2.8440	0.3717	0.3717	0.4934					
10	5.9720	3.2200	3.2200	1.0899	2.9360	2.9360	0.3752	0.3752	0.4772					
15	5.0190	3.1660	3.1660	1.0897	3.0170	3.0170	0.3783	0.3783	0.4527					
20	4.3330	3.1160	3.1160	1.0899	3.0730	3.0730	0.3803	0.3803	0.4294					
25	3.8720	3.0750	3.0750	1.0902	3.1070	3.1070	0.3814	0.3814	0.4103					
30	3.5660	3.0450	3.0450	1.0907	3.1240	3.1240	0.3816	0.3816	0.3959					

Table 4.14: Elastic Constants (GPa) at Volume Fraction 65/20/15

Table 4.15: Elastic Constants (GPa) at Volume Fraction 70/17/13

	Volume fraction $70/17/13$												
θ	Ezz	Exx	Eyy	Gxy	Gzy	Gzx	$ u_{zx}$	$ u_{zy}$	$ u_{yx}$				
0	7.8910	3.3080	3.3080	1.0961	2.8120	2.8120	0.3698	0.3698	0.5090				
5	7.3280	3.2890	3.2890	1.0956	2.8580	2.8580	0.3717	0.3717	0.5010				
10	6.1890	3.2440	3.2440	1.0955	2.9500	2.9500	0.3754	0.3754	0.4806				
15	5.1620	3.1880	3.1880	1.0951	3.0320	3.0320	0.3786	0.3786	0.4556				
20	4.4300	3.1360	3.1360	1.0952	3.0860	3.0860	0.3807	0.3807	0.4317				
25	3.9400	3.0930	3.0930	1.0953	3.1190	3.1190	0.3818	0.3818	0.4119				
30	3.6160	3.0610	3.0610	1.0955	3.1360	3.1360	0.3821	0.3821	0.3970				



Fig. 4.24: Effect of MFA on Axial Modulus



Fig. 4.25: Effect of MFA on Macroscopic Elastic Properties

Observation

Increase in MFA

- 1. The axial modulus reduced by 48-54%.
- 2. Transverse and shear modulus were least effected.
- 3. The longitudinal shear modulus increased by 15%.
- 4. The longitudinal Poisson's ratio increased by 3% and transverse Poisson's ratio decreased by 10% due to reduced stiffness in the axial direction.

Increase in Cellulose Content

- 1. The axial modulus increased by 22% at MFA of zero.
- 2. The shear modulus and Poisson's ratio were least effected, with an increase in cellulose content.

4.6 Von-Mises Fisher Probability Distribution

The importance of Von-Mises distributions for directional data is similar to that of normal distribution for linear data [56]. The generalized (p-1) dimensional Von-Mises density function for a vector of observations X can be written as

$$f_{\mu,k,p}(X) = C_P(k)e^{k\mu X}$$
 (4.26)

where μ is the mean vector, k is the concentration parameter and Cp(k) is the normalizing factor with the values, For p=2, circle:

$$C_p(k) = 1/(2\pi I_o(k))^2 \tag{4.27}$$

For p=3, sphere:

$$C_p(k) = k/(4\pi\sinh(k)) \tag{4.28}$$

For low concentration values of k < 1, the distribution is normal on a spherical plot and as the concentration value increased, all the data points were concentrated (green) in one direction, as shown in Figure 4.26.



Fig. 4.26: Von-Mises Fisher Random Variables

The purpose of this probability distribution function is to define the orientational distribution of fibers in a composite. The higher the k value, the more parallel the fibers are to the longitudinal axis, as shown in Figure 4.27(d). The lower the k-value, the more randomly distributed the fibers are in all directions, as shown in Figure 4.27(a). The derivation of random variables from the Von-Mises Fisher probability distribution function is explained below:

Derivation of random variables

Probability density function

$$f(\theta) = \frac{k}{2\sinh(k)} e^{k\cos(\theta)}\sin(\theta) \qquad ; g(\phi) = \frac{1}{2\pi}$$
(4.29)

Cumulative distribution function

$$\xi = \frac{ke^{-k}}{1 - e^{-2k}} \int_{-\pi}^{\theta} e^{k\cos(\theta)} \sin(\theta) \, d\theta \tag{4.30}$$

After evaluating integral in WxMaxima and performing inverse,

$$\theta = 2\sin^{-1}\left[\sqrt{\frac{-\log\left(\xi\left(1-\lambda\right)+\lambda\right)}{2k}}\right]$$
(4.31)

where $\lambda = e^{-2k}$

To generate random θ values for a particular concentration factor k, a uniformly distributed random number vector ξ is given as an input to the Equation 4.31. The PDF obtained for different concentration values is shown in Figure 4.28. A SCILAB program was written to generate the randomly oriented cylindrical fiber in a bounded cube, where the orientation is controlled by a factor k.



(a) Random Fiber Distribution k=1



(b) Random Fiber Distribution k=10



(c) Random Fiber Distribution $k{=}50$





Fig. 4.27: Random Fiber Distribution with Varying \boldsymbol{k}





(c) Von-Mises PDF k=1000

Fig. 4.28: Von-Mises PDF with Varying k

4.7 Equivalent Properties of Randomly Oriented Short Fiber Composite

The orientational averaging technique was explained in the work done by [57] to obtain the properties of random fiber composite. Some of the other works related to orientation averaging can be seen in [58-62]. In this section, the Von-Mises probability distribution is considered for use in obtaining properties of random short fiber composite. The concentration parameter for the distribution is in a selected range of 0.5 to 80, explaining fiber orientation in a particular direction to random. The properties for various volume fractions were considered, and the orientation averaging technique was applied to each of them respectively.

Orientational Averaging

The orientational averaging technique to obtain effective elastic modulus was derived by Christensen and Waals [57]. The various empirical relations for predicting the elastic modulus of a 2D and 3D randomly oriented short fiber composite was presented by [62]. The idea behind orientational averaging technique is to obtain the average of unidirectional fiber composite properties for all possible orientations. Mathematically, the orientation averaging is expressed as:

$$\frac{\overline{\sigma'_{ij}}}{\varepsilon'_{ij}} = \frac{\int\limits_{0}^{\pi} \int\limits_{0}^{\pi} \frac{\sigma'_{ij}}{\varepsilon'_{ij}} (pdf) \sin(\theta) \, d\theta d\phi}{\int\limits_{0}^{\pi} \int\limits_{0}^{\pi} \int\limits_{0}^{\pi} \sin(\theta) \, d\theta d\phi}$$
(4.32)



Fig. 4.29: Coordinate System

Assuming that the fiber is cylindrical and oriented in 3 dimensional space, as shown in Figure 4.29, with 123 as a rotated coordinate system and 1'2'3' as a fixed coordinate system. If the composite properties is transversely isotropic with fiber oriented along 1axis, the constitutive relation in transformed axis is given by $CM = \lambda_{ij} C \lambda_{ij}^T$

$$\lambda = \begin{bmatrix} a^2 & b^2 & c^2 & 2ab & 2ac & 2bc \\ d^2 & e^2 & f^2 & 2de & 2df & 2ef \\ g^2 & h^2 & i^2 & 2gh & 2gi & 2ih \\ ad & be & cf & ae + bd & af + cd & bf + ce \\ ag & bh & ci & ah + bg & ai + cg & bi + ch \\ dg & eh & fi & dh + eg & di + fg & ei + fh \end{bmatrix}$$
(4.33)
$$a & b & c \\ d & e & f \\ g & h & i \end{bmatrix} = \begin{bmatrix} \sin(\theta)\cos(\phi) & -\cos(\theta)\cos(\phi) & \sin(\phi) \\ \sin(\theta)\sin(\phi) & -\cos(\theta)\sin(\phi) & -\cos(\phi) \\ \sin(\theta)\sin(\phi) & 0 & 0 \end{bmatrix}$$
(4.34)
$$C = \begin{bmatrix} C_{11} & C_{12} & C_{13} & 0 & 0 & 0 \\ C_{22} & C_{23} & 0 & 0 & 0 \\ C_{33} & 0 & 0 & 0 \\ C_{44} & 0 & 0 \\ C_{66} & 0 \\ C_{66} & 0 \end{bmatrix}$$
(4.35)

The relation between σ_{33} and ϵ_{33} , σ_{22} and ϵ_{33} for an arbitrarily applied ϵ_{33} were given by

$$\frac{\sigma'_{33}}{\varepsilon'_{33}} = C_{11}\cos^4\theta + (2C_{12} + 4C_{66})\cos^2\theta\sin^2\theta + C_{22}\sin^4\theta \tag{4.36}$$

$$\frac{\sigma_{22}'}{\varepsilon_{33}'} = C_{11}d^2g^2 + C_{12}(d^2h^2 + e^2g^2 + d^2f^2) + C_{22}e^2h^2 + 4C_{66}ghde + C_{23}h^2f^2$$
(4.37)

$$\overline{\frac{\sigma'_{33}}{\varepsilon'_{33}}} = \frac{1}{15} (3C_{11} + 4C_{12} + 8C_{22} + 8C_{66})$$
(4.38)

$$\overline{\frac{\sigma'_{22}}{\varepsilon'_{22}}} = \frac{1}{15}(C_{11} + 8C_{12} + C_{22} - 4C_{66} + 5C_{23})$$
(4.39)

For a normal distribution, pdf=1 and the orientational averaging results in Equations 4.39 and 4.38 in 2 and 3 directions respectively.

Assuming the behavior of a randomly oriented composite to be quasi isotropic, the equivalent relation to the Equations 4.39 and 4.38 were written as

$$\frac{\sigma_{33}'}{\varepsilon_{33}'} = \frac{(\overline{E}(1-\overline{\nu}))}{((1-2\overline{\nu})(1+\overline{\nu}))}$$
(4.40)

$$\frac{\sigma_{22}'}{\varepsilon_{22}'} = \frac{(\overline{E}\overline{\nu})}{((1-2\overline{\nu})(1+\overline{\nu}))} \tag{4.41}$$

Equating 4.39 and 4.41, 4.38 and 4.40 and solving the equations will result in equivalent elastic properties \overline{E} and $\overline{\nu}$. The Von-Mises Fisher probability density function was chosen to calculate the average macroscopic properties of a random oriented composite with varying concentration parameters, as explained in the previous section. For the purpose of analysis, concentration parameters of k=0.5, 2, 5, 8, 10, 20, 40, 60, and 80 were chosen. The transverse isotropic properties evaluated on the unit cell as explained in Section 4.5 for various volume fractions of basic constituents were substituted in the constitutive relation given in the Equation 4.35 and average value of elastic modulus and Poisson's ratio were calculated according to the procedure explained in Equations 4.38 to 4.41. The evaluated elastic constants for the volume fraction of 50/27/23 is shown in Table 4.35 and the values of remaining volume fraction are presented in Appendix A.5-A.8.

				-									
	E(GPa)												
θ	K=0.5	K=2	K=5	K=8	K=10	K=20	K=40	K=60	K=80				
0	5.18406	5.81209	6.82745	7.17109	7.26577	7.38135	7.38273	7.37206	7.36437				
5	5.1534	5.73906	6.65229	6.92508	6.98606	7.01121	6.95236	6.91889	6.89925				
10	5.08847	5.5865	6.28919	6.41635	6.40814	6.24779	6.0655	5.9853	5.94116				
15	5.03039	5.44854	5.95905	5.95311	5.88165	5.55163	5.25638	5.13344	5.06688				
20	4.98729	5.34766	5.7195	5.61773	5.50076	5.04877	4.67237	4.51872	4.43605				
25	4.9573	5.27661	5.55117	5.38311	5.23483	4.69932	4.26754	4.09296	3.99933				
30	4.98993	5.28729	5.49798	5.27767	5.10301	4.49367	4.01106	3.81699	3.71309				
					$\overline{\nu}$								
0	0.29795	0.28298	0.26253	0.25759	0.25681	0.25767	0.2598	0.26083	0.26141				
5	0.2978	0.2835	0.26474	0.26119	0.26112	0.26405	0.26765	0.26926	0.27014				
10	0.296	0.2831	0.26796	0.26752	0.26908	0.27677	0.28379	0.28674	0.28833				
15	0.2938	0.28219	0.27064	0.27338	0.27664	0.28937	0.30009	0.3045	0.30688				
20	0.29237	0.28175	0.27298	0.27824	0.28287	0.2997	0.31348	0.31911	0.32214				
25	0.29091	0.28097	0.2742	0.28132	0.28697	0.30691	0.32304	0.32961	0.33316				
30	0.289	0.2794	0.27403	0.2827	0.28923	0.31191	0.33016	0.3376	0.34161				

Table 4.16: Elastic Constants (GPa) of Random Fiber Composite at Volume Fraction 50/27/23



Fig. 4.30: Equivalent Young's Modulus With Varying Concentration Factor



Fig. 4.31: Equivalent Poisson's Ratio With Varying Concentration Factor

4.7.1 Observations

- 1. The finite element model for unidirectional composite properties was based on the assumption that the bond between fiber and matrix was perfect.
- 2. From the graph Figure 4.30-4.31, it was deduced that the concentration factor of 0.5 and 2 suggests more random fiber orientation and quasi isotropic properties. That is, properties were least affected by the MFA orientation.
- The range of equivalent Young's modulus obtained through orientation averaging was 5.3-6.34 GPa, whereas the mean Young's modulus evaluated from tensile test was 6.48GPa.
- 4. The increase in concentration factor k resulted in direction dependent effective properties. The Poisson's ratio remained almost same for varying cellulose content.

Chapter 5

Summary, Conclusion, and Future Work

5.1 Summary of Work Performed

As a part of this dissertation, the micro-structure of kenaf fiber was explored using an optical microscope and a scanning electron microscope. Defects such as fiber damage, variation in the fiber width along axial direction, and delamination were observed (Figure 3.3). The images of the kenaf fiber obtained from the scanning electron microscope revealed one of the cell wall shapes to be a hollow rectangle Figure 3.2(b), whereas in general the cell wall shapes were an irregular polygon, as observed from the optical microscope images as shown in Figure 3.1. In order to obtain the tensile modulus of kenaf fiber, a tensile test was performed using the Instron 5848 testing machine. To study the influence of fiber gage length on the tensile modulus, four different gage lengths of 10, 15, 20 and 25.4 mm were selected and ten fibers in each lot were tested. The approximate cross-sectional area of fiber after failure was measured using an optical microscope. The procedure used to prepare the sample for microscopy examination was discussed in section 3.2.2. ImageJ software was used to evaluate the fiber cross-sectional area of the images obtained from the optical microscope at 50X magnification.

The kenaf fiber composite sample was processed by mixing chopped kenaf fibers and epoxy matrix using a vacuum bagging technique in the first attempt. Before preparing the composite, the chopped kenaf fibers were rinsed in a 3% sodium hydroxide solution to remove surface impurities and shredded using carding brushes. The vacuum bagging technique resulted in a composite plate with an uneven surface that was not suitable for tensile tests.Therefore, in the second attempt composite samples (dimensions as per ASTM D638) were prepared in HDPE molds by applying pressure through clamps and cured at 80^{0} C. Tensile tests were conducted on the composite samples to obtain Young's modulus and Poisson's ratio. Fourteen samples were tested on the Tinius Olsen tensile testing machine and force and strain data were acquired through NIDAQ 9237, for which a LabVIEW code was written, as shown in Figure B.3.

A 3D finite element code was written in a SCILAB environment to calculate the homogenized or effective properties of the natural fiber composite. A two-step homogenization was carried out: the first step obtained the properties of the cell wall layers at the microscopic scale and the second obtained unidirectional natural fiber composite properties. To avoid an ill-conditioned matrix that appeared due to the number of constraint equations, an augmented Lagrange technique was applied in the finite element code. To create a geometric model, Gmsh 3D mesh generator software was used, which provided an advantage of the structured mesh. In the final step of modeling, an orientational averaging technique was applied to evaluate the random fiber composite properties using the Von-Mises Fisher probability distribution function, as explained in section 4.6.

5.2 Summary of Findings and Conclusion

Micro-structure of Kenaf Fiber

- 1. The optical microscopic images of kenaf fibers obtained along the fiber axial direction exhibited defects such as delamination between cell walls, varying width, damage of fiber and sudden reduction in cross-sectional area. This lead to varying tensile strengths of kenaf fiber due to the amount of defects present based on the weakest links theory.
- 2. The cross-sectional images obtained through the optical microscope and the scanning electron microscope revealed the irregular cell wall shape and the hollow portion of the cell wall. Therefore, an appropriate cross-sectional area was required to evaluate stress in the fiber, taking into account the voids present on the cross-section. The cross-sectional area of the kenaf fiber measured after tensile test had an equivalent circular diameter of 45 μ m on average.

Influence of Gage Length on Kenaf Fiber Modulus

- 1. The tensile behavior of kenaf fibers was observed to be linear and failure to be brittle.
- 2. The tensile modulus of kenaf fiber was seen to increase with an increase in gage length. This might be due to inconsistent micro-structure, accurate cross-sectional area for evaluating stress and effective gage length in strain calculations. It can be concluded that to evaluate the Young's modulus of a kenaf fiber, a minimum gage length of 25.4 mm or more should be adopted.
- 3. The Weibull method was applied to characterize the tensile strength of a kenaf fiber. Two parameter, three parameter and Weibull of Weibull models were used to fit the tensile strength data. The average tensile strength obtained from the Weibull of Weibull model was observed to be in good agreement with the experimental values. In order to obtain appropriate Weibull distribution fit, more samples should be tested.

Tensile Properties of Randomly Oriented Kenaf Fiber Composite

- The kenaf fiber composite exhibited linear behavior and brittle failure with the tensile strength in the range of 20-38 MPa, as presented by other researchers in the past. The tensile strength is very low compared to that of the neat resin due to the amount of tensile strength reducing voids present in the composite.
- 2. The Poisson's ratio varied from specimen to specimen and was found to be in the range of 0.28-0.43. A possible reason for this might be the fiber orientation and inconsistent micro-structure at the point where the strains were measured.
- 3. The mean Young's modulus of the kenaf fiber composite at 22% fiber volume fraction is 6.48 GPa and comparative to the glass fiber composite of 7-8 GPa. This is an evidence that the kenaf fiber composite can replace glass fiber composite in terms of elastic modulus.

Numerical Modeling of Natural Fiber Composite

1. The Youngs modulus in the axial direction and Poisson's ratio computed using 3D finite element and semi-empirical relations presented in Section 4.4 were observed

to be in good agreement with the Rule of Mixtures. The transverse modulus was observed to be in good agreement with Tsai-Hahn empirical relation, whereas results from the multi-pass homogenization procedure gave an error of about 17%. Through inverse calculations, parameter χ of Halpin-Tsai empirical relation was seen to be 1.58 for the transverse modulus and 0.9 for the shear modulus.

- 2. The homogenized properties of an unidirectional kenaf fiber composite were obtained using parametric finite element modeling, with varying micro-fibril orientation in S2 layer. The axial modulus was reduced by 48-54% with an increase in MFA in the S2 layer. The transverse modulus and shear modulus were least affected. The axial modulus increased by 22% with an increase in cellulose content at MFA of 0°. The shear modulus and Poisson's ratio were least affected by an increase in cellulose content. These results indicate that the axial modulus of a composite is a function of fiber anisotropy. The numerical tests shows that the axial modulus increases with an increase in the cellulose content and the composites processed from fibers like cotton and ramie (which constitute 90% cellulose) as reinforcement will have increased Young's modulus as .
- 3. It was deduced that the composite properties remained quasi-isotropic at the concentration parameter of 0.5 and 2 (i.e. equivalent modulus remained almost constant) as shown in Figure 4.30. With an increase in the concentration parameter, the equivalent modulus appeared to be directional dependent (i.e. it decreases with an increase in MFA). The homogenization model developed for randomly oriented short fiber composite was able to predict the equivalent modulus (material is quasi-isotropic) and also explained the direction dependence property with fibers oriented in a particular direction.

5.3 Future Work

1. Understanding fiber and matrix interfacial characteristics through fiber pull out tests/single fiber fragmentation tests will provide an opportunity for enhancing the strength of natural fiber composites.

- 2. The appropriate manufacturing method for reducing the voids will enhance the strength of composites.
- 3. The application of kenaf fiber reinforced composites becomes crucial in the hygroscopic environment due to fiber water absorption property. Therefore, durability studies will be required to understand the behavior of the composite.
- 4. The voids in the unit cell and an imperfect bond (interface model) between fiber and matrix will be required to model the fracture and damage behavior of the composite.

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Appendices

Appendix A

Fiber Cross-Sectional Area

Specimen	Optical Microscopic Image	ImageJ				
Fiber 1	a second					
Fiber 2						
Fiber 3		and the second s				

Table A.1: Evaluated Fiber Area Using ImageJ (25.4mm)



Fiber 8	
Fiber 9	
Fiber 10	
Fiber 11	
Fiber 12	

Specimen	Optical Microscopic Image	ImageJ
Fiber 1		
Fiber 2		
Fiber 3		

Table A.2: Evaluated Fiber Area Using ImageJ (20mm)

r		
Fiber 4		0000
Fiber 5	Case	
Fiber 6		
Fiber 7		



Table A.3: Evaluated Fiber Area Using ImageJ (15mm)

Specimen Optical Microscopic Image	ImageJ
------------------------------------	--------

Fiber 1		
Fiber 2		500000
Fiber 3	1 200	- Cons
Fiber 4	1	633





Table A.4: Evaluated Fiber Area Using ImageJ (10mm)

Specimen Optical Microscopic Image	ImageJ
------------------------------------	--------

Fiber 1		Jor -
Fiber 2		
Fiber 3		0000
Fiber 4	1.200	
Fiber 5		

Fiber 6	
Fiber 7	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
Fiber 8	Fish and the second sec
Fiber 9	6.00

Fiber 10	
Fiber 11	

	$\overline{E}(GPa)$								
θ	K = 0.5	K=2	K=5	K=8	K=10	K=20	K=40	K=60	K=80
0	5.30745	5.97683	7.07735	7.4696	7.58541	7.75367	7.78794	7.78939	7.788
5	5.27136	5.89663	6.89196	7.21203	7.29365	7.37048	7.34403	7.32251	7.30909
10	5.19512	5.7278	6.50239	6.67103	6.68096	6.56606	6.41227	6.34261	6.30395
15	5.12254	5.56833	6.13584	6.16248	6.10518	5.81051	5.53734	5.42254	5.36022
20	5.06717	5.44764	5.8605	5.78186	5.67486	5.24761	4.88656	4.73855	4.65882
25	5.02547	5.36018	5.66583	5.515	5.37409	4.85666	4.43601	4.26552	4.174
30	4.99362	5.29558	5.52663	5.32717	5.16372	4.58701	4.12754	3.94244	3.84329
					$\overline{\nu}$				
0	0.29781	0.28239	0.26098	0.25532	0.25418	0.25401	0.25543	0.25619	0.25663
5	0.29711	0.28234	0.26256	0.25825	0.2578	0.25963	0.26245	0.26375	0.26447
10	0.29548	0.28214	0.266	0.26475	0.2659	0.27238	0.27852	0.28112	0.28254
15	0.29369	0.28174	0.26932	0.2713	0.27417	0.28571	0.29554	0.2996	0.30179
20	0.29244	0.28158	0.27213	0.27677	0.28105	0.29685	0.30984	0.31516	0.31802
25	0.29159	0.28151	0.27423	0.28082	0.28617	0.30518	0.32058	0.32687	0.33025
30	0.29147	0.28197	0.27624	0.28421	0.29029	0.31154	0.32865	0.33562	0.33937

Table A.5: Elastic Constants (GPa) of Random Fiber Composite at Volume Fraction 55/24/21

Table A.6: Elastic Constants (GPa) of Random Fiber Composite at Volume Fraction 60/23/17

	$\overline{E}(GPa)$								
θ	K=0.5	K=2	K=5	K=8	K=10	K=20	K=40	K=60	K=80
0	5.37147	6.082	7.27544	7.72763	7.87109	8.1118	8.19338	8.2126	8.2205
5	5.32812	5.98629	7.05521	7.4222	7.52535	7.65837	7.66848	7.66068	7.65442
10	5.23638	5.78838	6.60487	6.79922	6.82073	6.73563	6.60098	6.53846	6.50352
15	5.15284	5.60826	6.19531	6.23284	6.18021	5.89707	5.63102	5.51883	5.45785
20	5.08907	5.47429	5.89561	5.82066	5.715	5.29049	4.93082	4.78327	4.70376
25	5.04363	5.38063	5.68927	5.53871	5.39758	4.87886	4.45698	4.28596	4.19416
30	5.0098	5.31311	5.54479	5.34386	5.17935	4.59907	4.13682	3.95062	3.85087
					$\overline{\nu}$				
0	0.29826	0.2823	0.25966	0.25302	0.25138	0.24979	0.25022	0.25059	0.25082
5	0.29753	0.28235	0.26156	0.25644	0.25557	0.2562	0.25818	0.25916	0.25971
10	0.29582	0.28224	0.2655	0.26376	0.26465	0.27036	0.27596	0.27834	0.27964
15	0.29401	0.28196	0.26927	0.27102	0.27376	0.28491	0.29446	0.29841	0.30054
20	0.29264	0.28175	0.27222	0.27678	0.28102	0.29666	0.30954	0.31481	0.31765
25	0.29182	0.28175	0.27449	0.28108	0.28642	0.3054	0.32078	0.32705	0.33043
30	0.2916	0.28213	0.27646	0.28447	0.29058	0.31188	0.32903	0.33602	0.33978

	$\overline{E}(GPa)$								
θ	K=0.5	K=2	K=5	K=8	K=10	K=20	K=40	K=60	K=80
0	5.46197	6.21603	7.50126	8.00763	8.17504	8.47639	8.59637	8.62982	8.64514
5	5.407	6.10471	7.25676	7.67262	7.79735	7.98487	8.02944	8.03441	8.0348
10	5.30552	5.88678	6.76404	6.99349	7.03036	6.98374	6.87325	6.81966	6.78936
15	5.20677	5.68259	6.31164	6.37311	6.33087	6.07362	5.82374	5.71751	5.65962
20	5.13359	5.53303	5.98306	5.92399	5.82512	5.41727	5.06785	4.92405	4.84649
25	5.08118	5.42845	5.75735	5.61756	5.48093	4.97304	4.55768	4.38903	4.29845
30	5.04231	5.35348	5.60026	5.40704	5.24567	4.67276	4.21484	4.03019	3.93124
	$\overline{\nu}$								
0	0.29759	0.28108	0.25734	0.24995	0.24794	0.24533	0.24504	0.24514	0.24523
5	0.29542	0.27969	0.25788	0.25206	0.25084	0.2505	0.25181	0.25252	0.25293
10	0.29544	0.28145	0.26386	0.26148	0.26205	0.26685	0.2718	0.27393	0.2751
15	0.29387	0.28153	0.26817	0.26941	0.27187	0.28224	0.29122	0.29494	0.29695
20	0.29265	0.28155	0.27154	0.27569	0.27972	0.29473	0.30713	0.31222	0.31496
25	0.29187	0.28166	0.27404	0.28032	0.28549	0.30396	0.31897	0.32509	0.32838
30	0.29164	0.28206	0.27611	0.28388	0.28985	0.31075	0.32759	0.33445	0.33815

Table A.7: Elastic Constants (GPa) of Random Fiber Composite at Volume Fraction 65/20/15

Table A.8: Elastic Constants (GPa) of Random Fiber Composite at Volume Fraction 70/17/13

	$\overline{E}(GPa)$								
θ	K = 0.5	K=2	K=5	K=8	K=10	K=20	K = 40	K=60	K=80
0	5.54608	6.34325	7.72093	8.28318	8.4756	8.84088	9.00171	9.05038	9.07362
5	5.48636	6.22089	7.4514	7.91427	8.05994	8.30083	8.37938	8.39698	8.40395
10	5.36526	5.97285	6.90557	7.16771	7.21901	7.20892	7.12155	7.07653	7.05069
15	5.25392	5.74727	6.41223	6.49405	6.46063	6.22527	5.989	5.88778	5.8325
20	5.17037	5.58163	6.05546	6.00949	5.9162	5.52205	5.18103	5.04031	4.96434
25	5.11154	5.46708	5.81216	5.68088	5.54779	5.04834	4.63804	4.47122	4.38159
30	5.06898	5.38633	5.64471	5.45712	5.29798	4.7301	4.27503	4.09138	3.99294
	$\overline{\nu}$								
0	0.29725	0.28021	0.2554	0.24726	0.24487	0.24122	0.24022	0.24005	0.23999
5	0.29662	0.28045	0.25766	0.25112	0.24954	0.24819	0.24879	0.24923	0.2495
10	0.29531	0.28098	0.26263	0.25968	0.25996	0.26393	0.26828	0.27018	0.27123
15	0.29381	0.28124	0.26734	0.26815	0.2704	0.28013	0.28864	0.29217	0.29409
20	0.29269	0.28144	0.27104	0.27487	0.27873	0.29323	0.30527	0.3102	0.31286
25	0.29196	0.28164	0.27375	0.27979	0.28484	0.30292	0.31763	0.32364	0.32687
30	0.2917	0.28204	0.2759	0.28349	0.28937	0.30999	0.3266	0.33338	0.33703

Appendix B

Figures



Fig. B.1: Cumulative Distribution Function with Evaluated Parameters



Fig. B.1: Cumulative Distribution Function with Evaluated Parameters (Contd)



Fig. B.2: Cumulative Distribution Function of Three Parameter Model for Consistent Data



Fig. B.2: Cumulative Distribution Function of Three Parameter Model for Consistent Data (Contd)



Fig. B.3: Block Diagram of Tensile Test

Appendix C

Finite Element Code

C.1 Input File Code

0001 function GenerateInputFile(filename) 0001 0002 function [xl, k]=GenerateNodes(txt) 0001 0002 Nodes=evstr(txt(5)); 0003 k=Nodes+5; 0004 xl=evstr(txt(6:k)); 0005 fnodes=mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\nodes3d.txt','wt'); 0006 0007 mfprintf(fnodes,'%i %20.16f %20.16f %20.16f\n',xl(:,1),xl(:,2),xl(:,3),xl(:,4)); 0008 mclose(fnodes); 0009 0010 endfunction 0011 0014 function [El, kr]=GenerateElements(txt, k) 0001 0002 j=<u>evstr(txt(k+3));</u> 0003 for i=1:j ty = evstr(txt(k+3+i));0004 0005 [m,n]=size(ty); 0006 if n==13 then $El=\underline{evstr}(txt(k+3+i:k+3+j));kr=k+3+i;$ 0007 0008 break; 0009 0010 end 0011 0012 end felem=mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\elements3d.txt','wt'); 0013 0014 mfprintf(felem,'%i %i %i %i %i %i %i %i %i %i \n',El(:,1)-i+1,El(:,5),El(:,6),El(:,7),El(:,8),El(:,9),El(:,10),El(:,11),El(:,12),El(:,13)); mclose(felem); 0015 0016 endfunction 0017 0032 function [Sfnodes]=GenerateSurfaceNodes(txt, k, kr) 0001 j=<u>evstr(txt(k+3));</u> 0002 for i=1:j ty=<u>evstr(txt(k+3+i));</u> 0003 0004 [m,n]=size(ty); 0005 if n==9 then 0006 Sfnodes=evstr(txt(k+3+i:kr-1)); 0007 break; 0008 end 0009 end 0010 fsnod=mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\SurfaceFile.txt','wt'); 0011 0012 mclose(fsnod); 0013 endfunction 0014 0047 ft=mopen(filename,'rt') 0048 txt=mgetl(ft,-1); 0049 mclose(ft) 0050 0051 $[xl, k] = \underline{GenerateNodes}(txt);$ 0052 [El, kr] = $\underline{\text{GenerateElements}}(\text{txt, k});$ 0053 [Sfnodes] = GenerateSurfaceNodes(txt, k, kr); 0054 0055 endfunction

C.2 Sorting Surface Code

0001 function sortingsurface(filename) 0002 0003 fsf = mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\SurfaceFile.txt', 'rt'); 0004 AA=mfscanf(-1,fsf,'%f %f %f %f %f %f %f ');[ma,na]=size(AA); 0005 mclose(fsf); 0006 0007 disp('Enter 1 for Rear Face') 0008 disp('Enter 2 for Front Face') 0009 disp('Enter 3 for Left Face') 0010 disp('Enter 4 for Bottom Face') 0011 disp('Enter 5 for Right Face') 0012 disp('Enter 6 for Top Face') 0013 0014 sno=input('Enter the Number for Corresponding Face') 0015 w=sno; 0016 ft=mopen(filename,'rt') 0017 txt=mfscanf(-1,ft,'%f %f %f %f %f %f %f %f'); 0018 mclose(ft) 0019 0020 SM=txt; 0021 0022 // h=input('Entert how many surfaces')//Number of Surfaces Divided on One Big Surface 0023 [h,hh]=size(SM(:,sno)); 0024 p=1;j=1; fim=mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\sortingsurface.txt', 'wt') 0025 0026 for k=1:h 0027 //i=input('Entert the surface number in increasing order') i=SM(k,sno); 0028 0029 if $i \sim = 0$ then 0030 0031 for j=j:ma//Gathers the Nodal Data of Particular Surface Selected 0032 0033 if i = AA(j,1) then mfprintf(fim,'%i %i %i %i \n',AA(j,2),AA(j,3),AA(j,4),AA(j,5)); 0034 0035 p=p+1; 0036 end 0037 0038 end 0039 j=p; 0040 end 0041 end 0042 0043 mclose(fim) 0044 0045 <u>tic();</u> 0046 fsrt=mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\sortingsurface.txt','rt'); 0047 A=mfscanf(-1,fsrt,'%f %f %f %f %f'); 0048 [m,n]=size(A); 0049 BB=matrix(A,[m*n,1]) 0050 BB=<u>mtlb_sort(BB);</u> 0051 0052 [m,n]=size(BB);p=0;t=1; 0053 for j=1:m 0054 a=BB(j,1);p=0; 0055 0056 for i=1:m-t 0057 k=t+i-p; 0058 c=BB(k,1);0059 if a==c then BB(k,1)=[]; 0060 0061 p=p+1;[m,n]=size(BB); 0062

0063 end 0064 if a~=c then 0065 break 0066 end 0067 end 0068 t=t+1; 0069 if j==m then 0070 break 0071 end 0072 0073 end 0074 0075 time=<u>toc(</u>); 0075 tume=toc(), 0076 0077 fsrtmod=mopen('C:\Users\Dayakar Naik\Documents\FEMProgramFiles\Scilab program files\modnodes.txt','wt'); 0078 mfprintf(fsrtmod,'%i\n',BB(:,1)) 0079 mclose(fsrtmod);mclose(fsrt) 0090 printf(\ntime needed to sort: %.3f\n',toc());

0081

0082 endfunction

C.3 Finite Element Code

0001 //	

0002 //	FUNCTION TO EVALUATE STIFFNESS OF HEXAHEDRAL ELEMENT
******	*****
0004	
0001	function [Elemstiff]=stiffness(x, y, z, D)
0002	
0003	// INTEGRATION OR SAMPLING POINTS
0004	r=[-1/sqrt(3);1/sqrt(3);1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3);-1/sqrt(3);1/sqrt(3);-1/sqrt(3);
0005	s=[-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3)];
0006	t=[-1/sqrt(3);-1/sqrt(3);-1/sqrt(3);
0007	
0008	Elemsun=zeros(24,24);
0009	//shape functions /PafiChandrakant Dassi & Tribikram Kundu)
0010	//snape functions (ker.e.haninakanin Desai & Tholianin Kundu) //***********************************
0012	$//N_1 = 1/8*(1-r)*(1+s)*(1+t):$
0012	//N2=1/8*(1-r)*(1-s)*(1+t);
0014	//N3=1/8*(1-r)*(1-s)*(1-t);
0015	//N4=1/8*(1-r)*(1+s)*(1-t);
0016	//N5=1/8*(1+r)*(1+s)*(1+t);
0017	//N6=1/8*(1+r)*(1-s)*(1+t);
0018	//N7 = 1/8*(1+r)*(1-s)*(1-t);
0019	$//N8=1/8^{*}(1+r)^{*}(1+s)^{*}(1-t);$
0020	for in 1.9
0021	10 I-1.0
0022	//#*#*********************************
0024	N1r=zeros(8,1);
0025	N1r(1,1)=-1/8*(1-s(i))*(1-t(i));
0026	N1r(2,1)=1/8*(1-s(i))*(1-t(i));
0027	N1r(3,1)=1/8*(1+s(i))*(1-t(i));
0028	$\mathbf{N} \mathbf{Ir}(4, 1) = -1/8^{+} (1 + 8(1))^{+} (1 - 1(1));$
0029	$N \ln(3, 1) = -1/5^{-1}(1+5(1))/(1+1(1));$ $N \ln(6(1) - 1/2^{-1}(1+f(1)))$
0031	N(r(t, t) = 1/8) (1+s(t)) (1+t(t))
0032	NIr(8,1)=1/8*(1+s(i))*(1+t(i));
0033	
0034	//Derivatives of shape functions w.r.t s
0035	//*************************************
0036	NIs=zeros(8,1);
0037	$N IS(1, 1) = -1/8 \times (1 - r(1)) \times (1 - f(1));$ $N I = (2 - 1) \times (1 - r(1)) \times (1 - r(1)) \times (1 - r(1));$
0030	$\mathbf{N}\mathbf{I}_{S}(z, 1) = -1/0^{-1}(1+i(1))^{-1}(1-i(1)),$ $\mathbf{N}\mathbf{I}_{S}(z, 1) = 1/(S^{-1}(1+i(1)))^{-1}(1+i(1)).$
0040	$N(s_1, t_1) = 10 \ (t_1(t_1), t_1(t_1))$ $N(s_1(t_1)) = 10 \ (t_1(t_1), t_1(t_1))$
0041	$NI_{3}(5,1)=1/8*(1-r(i))*(1+t(i));$
0042	N1s(6,1)=-1/8*(1+r(i))*(1+t(i));
0043	N1s(7,1)=1/8*(1+r(i))*(1+t(i));
0044	N1s(8,1)=1/8*(1-r(i))*(1+t(i));
0045	
0046	//Derivatives of shape functions w.r.t t
0047	//
0048	N1t(1,1)=-1/(8*(1-r(i))*(1-s(i))):
0050	N1t(2,1)=-1/8*(1+r(i))*(1-s(i));
0051	NIt(3,1)=-1/8*(1+r(i))*(1+s(i));
0052	Nlt(4,1)=-1/8*(1-r(i))*(1+s(i));
0053	N1t(5,1)=1/8*(1-r(i))*(1-s(i));
0054	N1t(6,1)=1/8*(1+r(i))*(1-s(i));
0055	$NIt(/,1)=1/8^{*}(1+r(1))^{*}(1+s(1));$ $NIt(0,1)=1/8^{*}(1+r(1))^{*}(1+s(1));$
0056	$1 \times 1 \cup (\delta, 1) = 1/\delta^{\infty} (1 - f(1))^{\infty} (1 + S(1));$

0057	
0058	//Jacobian Matrix
0059	J=zeros(3,3);
0060	Nt = [N Ir; N Is; N It];
0061	$J=Nt^{*}[x,y,z];$
0062	IJ=inv(J);
0063	IJ=inv(J);
0064	
0065	//Computation of B Matrix
0066	//*************************************
0067	
0068	Ntt=zeros(6,24);
0069	$Ntr = [N1r(1,1) \ 0 \ 0 \ N1r(2,1) \ 0 \ 0 \ N1r(3,1) \ 0 \ 0 \ N1r(4,1) \ 0 \ 0 \ N1r(5,1) \ 0 \ 0 \ N1r(6,1) \ 0 \ 0 \ N1r(7,1) \ 0 \ 0 \ N1r(8,1) \ 0 \ 0;$
0070	N1s(1,1) 0 0 N1s(2,1) 0 0 N1s(3,1) 0 0 N1s(4,1) 0 0 N1s(5,1) 0 0 N1s(6,1) 0 0 N1s(7,1) 0 0 N1s(8,1) 0 0;
0071	$N1t(1,1) \\ 0 \\ 0 \\ N1t(2,1) \\ 0 \\ 0 \\ N1t(3,1) \\ 0 \\ 0 \\ N1t(4,1) \\ 0 \\ 0 \\ N1t(5,1) \\ 0 \\ 0 \\ N1t(6,1) \\ 0 \\ 0 \\ N1t(7,1) \\ 0 \\ 0 \\ N1t(8,1) \\ 0 \\ 0 \\ N1t(8,1) \\ 0 \\ N1$
0072	$0 \mathbf{Nlr}(1,1) 0 0 \mathbf{Nlr}(2,1) 0 0 \mathbf{Nlr}(3,1) 0 0 \mathbf{Nlr}(4,1) 0 0 \mathbf{Nlr}(5,1) 0 0 \mathbf{Nlr}(6,1) 0 0 \mathbf{Nlr}(7,1) 0 0 \mathbf{Nlr}(8,1) 0;$
0073	0 N1s(1,1) 0 0 N1s(2,1) 0 0 N1s(3,1) 0 0 N1s(4,1) 0 0 N1s(5,1) 0 0 N1s(6,1) 0 0 N1s(7,1) 0 0 N1s(8,1) 0 ;
0074	0 N1t(1,1) 0 0 N1t(2,1) 0 0 N1t(3,1) 0 0 N1t(4,1) 0 0 N1t(5,1) 0 0 N1t(6,1) 0 0 N1t(7,1) 0 0 N1t(8,1) 0
0075	$0 0 \mathbf{N}\mathbf{Ir}(1,1) 0 0 \mathbf{N}\mathbf{Ir}(2,1) 0 0 \mathbf{N}\mathbf{Ir}(3,1) 0 0 \mathbf{N}\mathbf{Ir}(4,1) 0 0 \mathbf{N}\mathbf{Ir}(5,1) 0 0 \mathbf{N}\mathbf{Ir}(6,1) 0 0 \mathbf{N}\mathbf{Ir}(7,1) 0 0 \mathbf{N}\mathbf{Ir}(8,1);$
0076	$0 0 \mathbf{NIs}(1,1) 0 0 \mathbf{NIs}(2,1) 0 0 \mathbf{NIs}(3,1) 0 0 \mathbf{NIs}(4,1) 0 0 \mathbf{NIs}(5,1) 0 0 \mathbf{NIs}(6,1) 0 0 \mathbf{NIs}(7,1) 0 0 \mathbf{NIs}(8,1);$
0077	$0 \circ Nlt(1,1) \circ 0 \circ Nlt(2,1) \circ 0 \circ Nlt(3,1) \circ 0 \circ Nlt(4,1) \circ 0 \circ Nlt(5,1) \circ 0 \circ Nlt(6,1) \circ 0 \circ Nlt(7,1) \circ 0 \circ Nlt(8,1)];$
0078	$\mathbf{B}=\mathbf{zeros}(6,24);$
0079	bmult =[10000000;00001000;00000;0000000];
0080	0 0 0 0 0 1 0 1 0 0 0 1 0 0 0 1 0 0 0 1 0 1 0 0 0 0 0 0];
0081	B=bmult*[IJ,zeros(3,6);zeros(3,3),IJ,zeros(3,3);zeros(3,6),IJ]*Ntt;
0082	В;
0083	
0084	//Stiffness Computation
0085	//***********
0086	Elemstiff = B'*D*B*det(J)+Elemstiff;
0087	
0088	end
0089	Elemstiff;
0090	
0091 end	function
0092	
0097 //	
******	***************************************
0098 //	END OF FUNCTION EVALUATING STIFFNESS OF HEXAHEDRAL ELEMENT *
0099 //	
~~~***	^^^^^^ <u>^^</u>
0100	
0101 //	
~*****	***************************************
0102 //	FUNCTION TO EVALUATE CONSTITUTIVE MATRIX *
0103 /////	/
~*****	
0104 //fu	ncuon [1] maura(ex,Ey,Ez,Gyz,Gzx,Gxy,Nuxy,Nuxz,Nuyz,MIxyx,MIxyy,MIxyz,MUxzyz,theta)
0105 //	
0100 //	m=cosq(neta);
0107 //	n=sind(ineta);
0100 //	T1 [
0110 //	11=[III'2 II'2 0 0 0 2*m'n];
0110 //	n*2 m*2 0 0 0 - 2*m*n; 0 0 1 0 0 0:
0112 //	
0112 //	
0115 //	
0114 //	-m ⁻ n m ⁻ n 0 0 0 m ⁻ /-n ⁻ 2];
0115 //	T2 Err/2 = 42.0.0.0 = ***
0115 //	12=[m^2 m^2 0 0 0 m*n;
UII/ //	n*2 m*2 0 0 0 -m*n;
0118 //	0 0 1 0 0 0;

0119 // 000m-n0; 0120 // 0 0 0 n m 0;0121 // -2*m*n 2*m*n 0 0 0 m^2-n^2]; 0122 // 0123 // S=[1/Ex -Nuxy/Ex -Nuxz/Ex 0 0 MIxyx/Gxy; 0124 // -Nuxy/Ex 1/Ey -Nuyz/Ey 0 0 MIxyy/Gxy; 0125 // -Nuxz/Ex -Nuyz/Ey 1/Ez 0 0 MIxyz/Gxy; 0 0 0 1/Gyz MUxzyz/Gzx 0; 0126 // 0127 // 0 0 0 MUxzyz/Gzx 1/Gzx 0; 0128 // MIxyx/Gxy MIxyy/Gxy MIxyz/Gxy 0 0 1/Gxy]; 0129 // 0130 // D=inv(T1)*inv(S)*T2; 0131 // //D=inv(S); 0132 // 0133 //endfunction 0134 function [D]=Dmatrix(Ex, Ey, Ez, Gyz, Gzx, Gxy, Nuxy, Nuxz, Nuyz, MIxyx, MIxyz, MUxzyz, theta) 0135 0136 m=<u>cosd(theta);</u> n=sind(theta); 0137 0138 0001 T1=[m^2 0 n^2 0 2*m*n 0; 0002 010000; n^2 0 m^2 0 -2*m*n 0; 0003 0004 000m0-n; 0005 -m*n 0 m*n 0 m^2-n^2 0; 0006 000**n**0**m];** 0007 T2=[m^2 0 n^2 0 m*n 0; 0008 010000; 0009 0010 n^2 0 m^2 0 -m*n 0; 000m0-n; 0011 -2*m*n 0 2*m*n 0 m^2-n^2 0; 0012 0013 0 0 0 n 0 m]; 0014 0015 S=[1/Ex -Nuxy/Ex -Nuxz/Ez 0 0 MIxyx/Gxy; 0016 -Nuxy/Ex 1/Ey -Nuyz/Ez 0 0 MIxyy/Gxy; 0017 -Nuxz/Ez -Nuyz/Ez 1/Ez 0 0 MIxyz/Gxy; 0018 0 0 0 1/Gyz MUxzyz/Gzx 0; 0019 0 0 0 MUxzyz/Gxy 1/Gzx 0; 0020 MIxyx/Gxy MIxyy/Gxy MIxyz/Gxy 0 0 1/Gxy]; 0021 0022 0023 0024 0025 D=inv(T1)*inv(S)*T2;0026 // D=inv(S); 0027 0028 endfunction 0029 0030 //function [RD]=Rmatrix(D,thetax) // Rotation About X-Axis 0031 // m=cosd(thetax); 0032 // n=sind(thetax); 0033 // Rx = [1 0 0 0 0 0;0 m^2 n^2 2*m*n 0 0; 0172 // 0173 // 0 n^2 m^2 -2*m*n 0 0; 0174 // 0 -m*n m*n m^2-n^2 0 0; 0175 // 0000m-n; 0176 // 0000nm]; 0177 // 0178 // 0179 // Rx1=[1 0 0 0 0 0; 0180 // 0 m^2 n^2 m*n 0 0;

0181 // 0 n^2 m^2 -m*n 0 0; 0 -2*m*n 2*m*n m^2-n^2 0 0; 0182 // 0000m-n; 0183 // 0184 // 0000nm]; 0185 // 0186 // RD=inv(Rx)*D*Rx1; 0187 //endfunction 0188 0189 function [RD]=Rmatrix(D, thetax) // Rotation About Z-Axis 0190 m=cosd(thetax); 0191 n=sind(thetax);  $Rx=[m^2 n^2 0 0 0 2*m*n;$ 0192 0193 n^2 m^2 0 0 0 -2*m*n; 0001 001000: 0002 0 0 0 m - n 0;0003 0 0 0 n m 0;-m*n m*n 0 0 0 m^2-n^2]; 0004 0005  $Rx1 = [m^2 n^2 0 0 0 m^*n;$ 0006 n^2 m^2 0 0 0 -m*n; 0007 0008 001000; 0009 0 0 0 m - n 0;000nm0; 0010 0011 -2*m*n 2*m*n 0 0 0 m^2-n^2]; 0012 RD=inv(Rx)*D*Rx1; 0013 0014 endfunction 0015 0016 //0017 // END OF FUNCTION TO EVALUATING CONSTITUTIVE MATRIX 0018 // 0019 0213 // FUNCTION TO EVALUATE STIFFNESS MATRIX ASSEMBLY 0214 // 0215 // ****** 0216 0217 function [MatProp, Elmat, gdofg, CM, nceq]=GenerateDOF(xl, El, Eprop, kdo, ceq) 0218 0219 [mn,nn]=size(xl); [m,n]=size(El);MatProp=El(:,2);[mk,nk]=size(kdo);[mc,nc]=size(ceq); 0220 xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);kdof=kdo'; 0221 0001 Elmat=zeros(m(1),8); 0002 for i=1:m(1)0003 for j=2:9Elmat(i,j-1)=El(i,j+1);0004 0005 end 0006 end 0007 0008 0009 tdof=mn(1)*3;dof=tdof+mc(1); 0010 ldof=24; //ldof=number of dof associated with element 0011 //e=number of elements 0012 ek=m(1); 0013 npe=8; //npe=number of nodes per element 0014 dofpn=3; //dofnp=dof per node 0015 0016 CM=zeros(ek,ldof); 0017

```
0018 // GENERATE GLOBAL DOF'S SUCH THAT KNOWN DISP ARE SUBSTRUCTRED TO BOTTOM
0019 //--
0020
              gdof=zeros(tdof,1);p=1;jr=1;
0021
                 q=dof-mk+1;
                    for i=1:tdof
0022
0023
                       for j=jr:mk
0024
                            if i==kdo(j,1) then
0025
                              gdof(i,1)=q;
0026
                              jr=jr+1;
0027
                              \mathbf{q}{=}\mathbf{q}{+}1\mathbf{;}
0028
                           end
0029
                       end
                           if gdof(i,1)==0 then
0030
0031
                            gdof(i,1)=p;
0032
                            p=p+1;
0033
                          end
0034
                   end
0035
               gdofg=zeros(mn(1),3);o=1;
0036
0037
                   for i=1:mn(1)
0038
                      gdofg(i,1)=gdof(o);
0039
                      gdofg(i,2)=gdof(o+1);
0040
                      gdofg(i,3)=gdof(o+2);
0041
                     o=o+3;
0042
                   end
0043
0044 //GENERATE GLOBAL DOF'S CONNECTIVITY MATRIX FOR ELEMENTS
0045 //-
0046
0047
                      for i=1:ek
0048
                        p=1;k=0;
0049
                         for j=1:npe
0050
                             for l=p:p+2
0051
0052
                                CM(i,l)=gdofg(Elmat(i,j),l-k);
0053
0054
                              end
0055
                              k=1;p=l+1;
0056
                        end
0057
                     end
0058
0059
          nceq=zeros(mc,5); //Incorporate Constraint Equations
0060
         for i=1:mc
0061
             lp1=gdofg(ceq(i,2),ceq(i,4));
0062
             lp2=gdofg(ceq(i,3),ceq(i,5));
0063
             lp3=ceq(i,6);
0064
             lp4=ceq(i,7);
0065
             nceq(i,1)=i;nceq(i,2)=lp1;nceq(i,3)=lp2;nceq(i,4)=lp3;nceq(i,5)=lp4;
0066
          end
0067
0068
0069
       endfunction
0070 // GENERATE STIFFNESS FOR EACH ELEMENT AND STORE IN kl MATRIX
0071 //-
0072
0073 function [KG, re, yo]=Assemble(xl, Elmat, Eprop, CM, nceq, MatProp, ek, tdof, dof, mk, mc)
0074
        kl=zeros(576,ek);xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);
0296
        E11=Eprop(:,2);E22=Eprop(:,3);E33=Eprop(:,4);
0297
           G12=Eprop(:,5);G23=Eprop(:,6);G13=Eprop(:,7);
0298
           nu12=Eprop(:,8);nu23=Eprop(:,9);nu13=Eprop(:,10);
0001
           mix=Eprop(:,11);miy=Eprop(:,12);miz=Eprop(:,13);mu=Eprop(:,14);
```

0002 theta=Eprop(:,15);thetax=Eprop(:,16);

0003 for w = 1:ek0004 xx=zeros(8,1);yy=zeros(8,1);zz=zeros(8,1); 0005 **for i**=1:8 0006 j=Elmat(w,i); 0007 xx(i,1)=xc(i): 0008 yy(i,1)=yc(j); 0009 zz(i,1)=zc(j);0010 end 0011 0012 mp=MatProp(w,1); 0013  $[D] = \underline{Dmatrix}(E11(mp,1), E22(mp,1), E33(mp,1), G12(mp,1), G13(mp,1), nu12(mp,1), nu23(mp,1), nu13(mp,1), mix(mp,1), mix(mp,1),$ 0014 [Cij]=Rmatrix(D,thetax(mp,1)); [Elemstiff]=<u>stiffness</u>(xx,yy,zz,Cij); 0015 0016 kl(:,w)=matrix([Elemstiff],576,1); 0017 0018 end 0019 [m,n]=size(kl); nkl=matrix(kl,m*n,1);clear kl; 0020 0021 0022 //GENERATE ROW AND COLUMN INDEXES (i,j) BASED ON CM MATRIX FOR CORRESPONDING kl 0023 //-0024 0025 rindx=zeros(576,ek); //Row Index 0026 cindx=zeros(576,ek); //Column Index 0027 findex=zeros(24,ek); //Force Index 0028 0029 for w = 1:ek0030 rindx(:,w) = matrix(repmat(CM(w,:)',1,24),576,1); 0031 cindx(:,w) = matrix(repmat(CM(w,:),24,1),576,1); 0032 end 0033 for w = 1:ekfindex(:,w) = CM(w,:)';0034 0035 end 0036 0037 nrindx=matrix(rindx,m*n,1); 0038 clear rindx; 0039 ncindx=matrix(cindx,m*n,1); 0040 clear cindx: 0041 0042 findex=matrix(findex,24*ek,1); 0043 R = sparse([findex,ones(24*ek,1)],zeros(24*ek,1)); 0044 0045 ps=tdof-mk; 0046 ixee=zeros(2*mc,1); 0047 kle=zeros(2*mc,1); 0048 klee=zeros(2*mc,1); 0049 jxee=zeros(2*mc,1); 0050 fii=zeros(4*mc,1);fjj=zeros(4*mc,1); 0051 nceeq=nceq'; 0052 ixee=matrix(nceeq(2:3,:),2*mc,1); 0053 kle=matrix(nceeq(4:5,:),2*mc,1); 0054 klee=matrix(repmat(kle,1,2),4*mc,1);clear kle; 0055 jxet=repmat(ps+nceeq(1,:),2,1); 0056 jxee=matrix(jxet,2*mc,1); 0057 fii=[jxee;ixee];fjj=[ixee;jxee]; 0058 clear ixee; clear jxee; 0059 KG = sparse([[nrindx;fii],[ncindx;fjj]],[nkl;klee]); 0060 0061 **R(dof-mk**,1)=0.0005; 0062 re=R;yo=dof-mk;clear nrindx;clear ncindx;clear fii;clear fjj;clear nkl;clear klee;clear R; 0063 KG(yo+1:dof,:)=[]; KG(:,yo+1:dof)=[];re(yo+1:dof,:)=[];

0064 endfunction 0065 0066 0067 // END OF FUNCTION EVALUATING ASSEMBLY MATRIX 0068 // 0069 // ****** 0368 0369 // 0370 // FUNCTION TO EVALUATE DISPLACEMENTS 0371 // 0372 function [u]=<u>FEMsol(KG</u>, re, yo, mc, pf) 0373 0374 **u** = umfpack(KG(1:yo,1:yo),\',full(re(1:yo)));//Direct Inverse Method 0375 // u=full(KG(1:yo,1:yo))\re(1:yo); 0376 //Iteration Method Based on Lagrange and Penalty Method Combined 0377 // KK=KG(1:yo-mc,1:yo-mc);AA=KG(yo-mc+1:yo-mc,1:yo-mc);w=sparse(pf*max(KG)*eye(mc,mc));lp=zeros(mc,1); 0378 // f=re(1:yo-mc);b=re(yo-mc+1:yo-mc+mc); 0001 // NK=KK+AA'*w*AA; 0002 // R=f-AA'*lp+AA'*w*b; 0003 // u = umfpack(NK, '', R);0004 // lo=lp+w*(AA*u-b); 0005 // lp=lo;a=1; 0006 // 0007 //while a>0 R=f-AA'*lp+AA'*w*b; 0008 // 0009 //  $u = umfpack(NK, \'\,R);$ 0010 // ln=lp+w*(AA*u-b); 0011 // if abs(ln-lp)<=1d-5 then a=1-a; break 0012 // 0013 // 0014 // else 0015 // lp=ln; 0016 // end 0017 // a=a+1 0018 // end 0019 0020 endfunction 0021 0022 // ***** 0023 // FUNCTION TO EVALUATE B-MATRIX * 0024 // ***** 0025 function [Bm]=<u>Bmatrix(x, y, z, r, s, t)</u> 0026 0027 //shape functions (Ref:Chandrakant Desai & Tribikram Kundu) 0406 //**** ***** 0407 N1=1/8*(1-r)*(1+s)*(1+t);0408 N2=1/8*(1-r)*(1-s)*(1+t);N3=1/8*(1-r)*(1-s)*(1-t); 0409 0001 N4=1/8*(1-r)*(1+s)*(1-t);0002 N5=1/8*(1+r)*(1+s)*(1+t);N6=1/8*(1+r)*(1-s)*(1+t);0003 0004 N7=1/8*(1+r)*(1-s)*(1-t); 0005 N8=1/8*(1+r)*(1+s)*(1-t);0006 0007 N1r=zeros(8,1); N1r(1,1)=-1/8*(1-s)*(1-t);0008

0009	N1r(2,1)=1/8*(1-s)*(1-t);
0010	N1r(3,1)=1/8*(1+s)*(1-t);
0011	N1r(4,1)=-1/8*(1+s)*(1-t);
0012	N1r(5,1)=-1/8*(1-s)*(1+t);
0013	N1r(6,1)=1/8*(1-s)*(1+t);
0014	N1r(7,1)=1/8*(1+s)*(1+t);
0015	N1r(8,1)=-1/8*(1+s)*(1+t);
0016	
0017	//Derivatives of shape functions w.r.t s
0018	N1s=zeros(8,1);
0019	N1s(1,1)=-1/8*(1-r)*(1-t);
0020	N1s(2,1)=-1/8*(1+r)*(1-t);
0021	N1s(3,1)=1/8*(1+r)*(1-t);
0022	N1s(4,1)=1/8*(1-r)*(1-t);
0023	Nls(5,1)=-1/8*(1-r)*(1+t);
0024	N1s(6,1)=-1/8*(1+r)*(1+t);
0025	N1s(7,1)=1/8*(1+r)*(1+t);
0026	N1s(8,1)=1/8*(1-r)*(1+t);
0027	
0028	//Derivatives of shape functions w.r.t t
0029	N1t=zeros(8,1);
0030	N1t(1,1)=-1/8*(1-r)*(1-s);
0031	N1t(2,1)=-1/8*(1+r)*(1-s);
0032	N1t(3,1)=-1/8*(1+r)*(1+s);
0033	N1t(4,1)=-1/8*(1-r)*(1+s);
0034	N1t(5,1)=1/8*(1-r)*(1-s);
0035	N1t(6,1)=1/8*(1+r)*(1-s);
0036	N1t(7,1)=1/8*(1+r)*(1+s);
0037	N1t(8,1)=1/8*(1-r)*(1+s);
0038	
0039	//Jacobian Matrix
0040	J=zeros(3,3);
0041	Nt=[N1r;N1s;N1t];
0042	$J=Nt^{*}[X,Y,Z];$
0043	$\Pi = \Pi V(J);$
0044	Nite-range (6.24)
0045	Nu=zeros(0,24), Nu=Nu=(1,1,0,0,Nu=(1,1,0,0,Nu=(2,1,0,0,Nu=(4,1,0,0,Nu=(5,1,0,0,Nu=(4,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0,Nu=(2,1,0,0)))
0040	$NI_{2}(1,1) = 0$ $NI_{2}(1,1$
0047	$NI_{2}(1,1) = 0 NI_{2}(2,1) = 0 NI_{3}(2,1) = 0$
0040	0 NIr(1) 0 0 NIr(2) 0 0 NIr(3) 0 0 NIr(4) 0 0 NIr(5) 0 0 NIr(5) 0 0 NIr(7) 0 0 NIr(5) 0 0,
0050	0 NIs(1) 0 0 NIs(2) 0 0 NIs(3) 0 0 NIs(4) 0 0 NIs(5) 0 0 NIs(6) 0 0 NIs(7) 0 0 NIs(6) 0
0051	0 Nit(1) $0.0$ Nit(2) $0.0$ Nit(3) $0.0$ Nit(4) $0.0$ Nit(6) $0.0$ Nit(6) $0.0$ Nit(7) $0.0$
0052	0.0 NIr(1) $0.0$ NIr(2) $0.0$ NIr(3) $0.0$ NIr(4) $0.0$ NIr(5) $0.0$ NIr(6) $0.0$ NIr(7) $0.0$ NIr(8)
0053	0 0 NIS(1) 0 0 NIS(2) 0 0 NIS(3) 0 0 NIS(4) 0 0 NIS(5) 0 0 NIS(6) 0 0 NIS(7) 0 0 NIS(8):
0054	0.0 N1t(1,1) $0.0$ N1t(2,1) $0.0$ N1t(3,1) $0.0$ N1t(4,1) $0.0$ N1t(5,1) $0.0$ N1t(6,1) $0.0$ N1t(7,1) $0.0$ N1t(8,1):
0055	B=zeros(6.24):
0056	$\mathbf{bmult} = \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 &$
0057	0 0 0 0 0 1 0 1 0;0 0 1 0 0 0 1 0 0;0 1 0 1
0058	B=bmult*[IJ,zeros(3,6);zeros(3,3),IJ,zeros(3,3);zeros(3,6),IJ]*Ntt;
0059	В;
0060	Bm=B;
0061	
0062	endfunction
0063	
0064	function [avrg]= <u>aveg</u> (astrain, x, y, z)
0065	r=[-1/sqrt(3);1/sqrt(3);-1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3);-1/sqrt(3)];
0066	s=[-1/sqrt(3);-1/sqrt(3);1/sqrt(3);-1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3)];
0067	t=[-1/sqrt(3);-1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3)];
0068	voi=u;asigina=u;
0470	101 I=1.0
04/9	// Derivatives of shape functions w.r.t i

0001         NI=recond(8,1);           0002         NIr(1,1)=1/8*(1-40))*(1-10);           0003         NIr(3,1)=1/8*(1-40))*(1-10);           0005         NIr(4,1)=1/8*(1-40))*(1-10);           0006         NIr(5,1)=1/8*(1-40))*(1-10);           0007         NIr(5,1)=1/8*(1-40))*(1-10);           0008         NIr(5,1)=1/8*(1-40))*(1-10);           0009         NIr(5,1)=1/8*(1-40))*(1-10);           0001         //Derivatives of shape functions w.r.t s           0013         NIs=zeros(8,1);           0014         NIs(5,1)=1/8*(1-40))*(1-10);           0015         NIs(2,1)=1/8*(1-40))*(1-10);           0016         NIs(5,1)=1/8*(1-40))*(1-40);           0017         NIs(4,1)=1/8*(1-40))*(1-40);           0018         NIs(5,1)=1/8*(1-40))*(1-40);           0019         NIs(5,1)=1/8*(1-40))*(1-40);           0020         NIs(5,1)=1/8*(1-40)*(1-40);           0021         //website           0022         //website           0023         /Nis(1,1)=1/8*(1-40)*(1-40);           0024         //website           0025         NIs(1,1)=1/8*(1-40)*(1-40);           0026         NIs(1,1)=1/8*(1-40)*(1-40);           0027         NIs(2,1)=1/8*(1-40)*(1-40);           0028	0480	//**********
0002         NIr(1,1)=-1/8'(1-sii)'(1-ii);           0003         NIr(2,1)=1/8'(1-sii)'(1-ii);           0004         NIr(3,1)=1/8'(1-sii)'(1-ii);           0005         NIr(4,1)=-1/8'(1-sii)'(1-iii);           0006         NIr(3,1)=1/8'(1-sii)'(1-iii);           0007         NIr(4,1)=-1/8'(1-sii)'(1-iii);           0008         NIr(3,1)=1/8'(1-sii)'(1-iii);           0010         //britali'(1-iii)'(1-iii);           0011         //britali'(1-iii)'(1-iii);           0012         //***********************************	0001	N1r=zeros(8,1);
0003         NIr(2,)=1/8'(-160);           0004         NIr(3,)=1/8'(-160);           0005         NIr(4,)=-1/8'(-160);           0006         NIr(5,)=1/8'(-160);           0007         NIr(5,)=1/8'(-160);           0008         NIr(5,)=1/8'(-160);           0009         NIr(5,)=1/8'(-160);           0000         NIr(5,)=-1/8'(-160);           0001         //Derivatives of shape functions w.r.1s           0012         //###################################	0002	N1r(1,1)=-1/8*(1-s(i))*(1-t(i));
0004         NIr(3,1)=1/8*(1+60))*(1+60);           0005         NIr(5,1)=-1/8*(1+60))*(1+60);           0007         NIr(5,1)=-1/8*(1+60))*(1+60);           0008         NIr(7,1)=1/8*(1+60))*(1+60);           0009         NIr(7,1)=1/8*(1+60))*(1+60);           0011         //bervarives of shape functions w.r.ts           0012         //bervarives of shape functions w.r.ts           0013         NIs*(2,1)=-1/8*(1+60))*(1+60);           0014         NIs*(2,1)=-1/8*(1+60))*(1+60);           0015         NIs*(2,1)=-1/8*(1+60))*(1+60);           0016         NIs*(2,1)=-1/8*(1+60))*(1+60);           0017         NIs*(2,1)=-1/8*(1+60))*(1+60);           0018         NIs*(2,1)=-1/8*(1+60))*(1+60);           0019         NIs*(2,1)=-1/8*(1+60))*(1+60);           0020         NIs*(2,1)=-1/8*(1+60))*(1+60);           0021         NIs*(2,1)=-1/8*(1+60))*(1+60);           0022         //bervarives of shape functions w.r.t1           0023         /NIs*(2,1)=-1/8*(1+60))*(1+60);           0024         //bervarives of shape functions w.r.t1           0025         NIIt=2ens(4,1);           0026         NIIt*(2,1)=1/8*(1+60))*(1+60);           0027         NIIt*(2,1)=1/8*(1+60))*(1+60);           0028         NIIt*(2,1)=1/8*(1+60))*(1+60); <td>0003</td> <td>N1r(2,1)=1/8*(1-s(i))*(1-t(i));</td>	0003	N1r(2,1)=1/8*(1-s(i))*(1-t(i));
0005         NIr4.1)=-18*(1-si0)*(1-ti0);           0006         NIr5.1)=18*(1-si0)*(1-ti0);           0007         NIr6.1)=18*(1-si0)*(1-ti0);           0008         NIr7.1)=18*(1-si0)*(1-ti0);           0009         NIr6.1)=18*(1-si0)*(1-ti0);           0011         //Derivatives of shape functions w.r.t s           0012         ////////////////////////////////////	0004	N1r(3,1)=1/8*(1+s(i))*(1-t(i));
0006         NHr5.1)=1/8*(1-si0)*(1-si0);           0007         NHr5.1)=1/8*(1-si0)*(1-si0);           0008         NHr7.1)=1/8*(1-si0)*(1-si0);           0011         //Derivatives of shape functions w.r.t s           0012         //***********************************	0005	N1r(4,1)=-1/8*(1+s(i))*(1-t(i));
0007       NIr6.10=1/8*(1-si)*(1+i0);         0008       NIr6.10=1/8*(1-si)*(1+i0);         0000       //Derivatives of shape functions w.r.ts         0011       //Derivatives of shape functions w.r.ts         0012       //###################################	0006	N1r(5,1)=-1/8*(1-s(i))*(1+t(i));
0008         NIr(7,1)=1/8*(1+s(i))*(1+s(i));           0009         NIr(8,1)=1/8*(1+s(i))*(1+s(i));           0011         //Derivatives of shape functions w.r.t s           0012         //***********************************	0007	N1r(6,1)=1/8*(1-s(i))*(1+t(i));
0009       NIr(s,1)=1/8*(1+fu)*(1+fu);         0011       //Derivatives of shape functions w.r.t s         0013       N1s=zeros(8,1);         0014       N1s(1)=1/8*(1+fi)*(1+fi);         0015       N1s(2,1)=1/8*(1+fi)*(1+fi);         0016       N1s(3,1)=1/8*(1+fi)*(1+fi);         0017       N1s(4,1)=1/8*(1+fi)*(1+fi);         0018       N1s(5,1)=1/8*(1+fi)*(1+fi);         0019       N1s(5,1)=1/8*(1+fi)*(1+fi);         0021       N1s(5,1)=1/8*(1+fi)*(1+fi);         0023       //Derivatives of shape functions w.r.t t         0024       //Merivatives of shape functions w.r.t t         0025       N1s(3,1)=1/8*(1+fi)*(1+fi);         0026       N1s(1,1)=1/8*(1+fi)*(1+fi);         0027       N1s(2,1)=1/8*(1+fi)*(1+fi);         0028       N1s(2,1)=1/8*(1+fi)*(1+fi);         0029       N1s(1,1)=1/8*(1+fi)*(1+fi);         0031       N1s(2,1)=1/8*(1+fi)*(1+fi);         0032       N1s(2,1)=1/8*(1+fi)*(1+fi);         0033       N1s(2,1)=1/8*(1+fi)*(1+fi);         0034       N1s(1,1)=1/8*(1+fi)*(1+fi);         0035       N1s(2,1)=1/8*(1+fi)*(1+fi);         0036       N1s(2,1)=1/8*(1+fi)*(1+fi);         0037       N1s(2,1)=1/8*(1+fi)*(1+fi);         0038       N1	0008	NIr(7,1)=1/8*(1+s(1))*(1+t(1));
0010       //Derivatives of shape functions w.r.t s         0011       //merceros(8,1);         0014       N1s=zcros(8,1);         0015       N1s(1,1)=1/8*(1-fi))*(1-fi);         0016       N1s(3,1)=1/8*(1-fi))*(1-fi);         0017       N1s(4,1)=1/8*(1-fi))*(1-fi);         0018       N1s(5,1)=1/8*(1-fi))*(1-fi);         0019       N1s(5,1)=1/8*(1-fi))*(1-fi);         0011       N1s(5,1)=1/8*(1-fi))*(1-fi);         0012       //Derivatives of shape functions w.r.t t         0013       N1s(5,1)=1/8*(1-fi))*(1-s(i));         0014       //merrors(8,1);         0015       N1s(1,1)=1/8*(1-fi))*(1-s(i));         0016       N1s(1,1)=1/8*(1-fi))*(1-s(i));         0017       N1s(1,1)=1/8*(1-fi))*(1-s(i));         0018       N1s(2,1)=1/8*(1-fi))*(1-s(i));         0019       N1s(1,1)=1/8*(1-fi))*(1-s(i));         0011       N1s(2,1)=1/8*(1-fi))*(1-s(i));         0012       N1s(1,1)=1/8*(1-fi))*(1-s(i));         0013       N1s(2,1)=1/8*(1-fi))*(1-s(i));         0014       sigma=astrain(i,1)*(1-fi))*(1-s(i));         0015       //Jacobian Matrix         0016       sequences         0017       N1s(2,1)=1/8*(1-fi))*(1-s(i));         0018       N1s(2,1)=1/8*(1-fi	0009	NIr(8,1)=-1/8*(1+s(1))*(1+t(1));
0011       //Derivatives of shape functions w.r.f.s         0013       N18=zeros(8,1);         0014       N1s(1)=-1/8*(1+fi))*(1-fi);         0015       N1s(2,1)=-1/8*(1+fi))*(1-fi);         0016       N1s(3,1)=-1/8*(1+fi))*(1-fi);         0017       N1s(4,1)=1/8*(1+fi))*(1+fi);         0018       N1s(5,1)=-1/8*(1+fi))*(1+fi);         0019       N1s(5,1)=-1/8*(1+fi))*(1+fi);         0010       N1s(5,1)=-1/8*(1+fi))*(1+fi);         0011       N1s(5,1)=-1/8*(1+fi))*(1+fi);         0012       //Derivatives of shape functions w.r.ft         0013       N1s(2,1)=-1/8*(1+fi))*(1+fi);         0014       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0015       N11=zeros(8,1);         0016       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0017       N1s(2,1)=-1/8*(1+fi))*(1+fi);         0018       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0019       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0019       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0011       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0012       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0013       N1s(1,1)=-1/8*(1+fi))*(1+fi);         0014       v=det(0;         0015       end         0016       v=get(3,3);	0010	10 - righting of the sting of the
0013       N1s=zeros(8,1);         0014       N1s(1,1)=1/3*(1-ti))*(1-ti);         0015       N1s(1,1)=1/3*(1-ti))*(1-ti);         0016       N1s(1,1)=1/3*(1-ti))*(1-ti);         0017       N1s(1,1)=1/3*(1-ti))*(1-ti);         0018       N1s(5,1)=-1/3*(1-ti))*(1-ti);         0019       N1s(5,1)=-1/3*(1-ti))*(1-ti);         0011       N1s(1,1)=1/3*(1-ti))*(1-ti);         0022       //Derivatives of shape functions w.r.t t         0023       //Derivatives of shape functions w.r.t t         0024       //Metricular (1-ti))*(1-si);         0025       N1t=zeros(8,1);         0026       N1t(1,1)=-1/3*(1-ti))*(1-si);         0027       N1t(2,1)=-1/3*(1-ti))*(1-si);         0028       N1t(1,1)=-1/3*(1-ti))*(1-si);         0029       N1t(1,1)=-1/3*(1-ti))*(1-si);         0031       N1t(5,1)=1/3*(1-ti))*(1-si);         0032       /Jacobian Matrix         0033       N1t(8,1)=1/3*(1-ti))*(1-si);         0034       end         0035       /Jacobian Matrix         0036       end         0037       red         0038       J=N*(5,2);         0039       J=inv(1);         0031       red         0032	0011	//Derivatives of snape functions w.r.t.s
0014       N1s(1,1)=1/8*(1+r(1)*(1+t(1));         0015       N1s(2,1)=1/8*(1+r(1))*(1+t(1));         0016       N1s(3,1)=1/8*(1+r(1))*(1+t(1));         0017       N1s(4,1)=1/8*(1+r(1))*(1+t(1));         0018       N1s(5,1)=1/8*(1+r(1))*(1+t(1));         0019       N1s(4,1)=1/8*(1+r(1))*(1+t(1));         0020       N1s(7,1)=1/8*(1+r(1))*(1+t(1));         0021       N1s(8,1)=1/8*(1+r(1))*(1+t(1));         0022       //Derivatives of shape functions w.r.t1         0024       //It=zeros(8,1);         0025       N1tr=zeros(8,1);         0026       N1tr(1,1)=-1/8*(1+r(1))*(1+s(0));         0027       N1t(3,1)=-1/8*(1+r(1))*(1+s(0));         0028       N1tr(3,1)=-1/8*(1+r(1))*(1+s(0));         0031       N1tr(5,1)=1/8*(1+r(1))*(1+s(0));         0032       N1tr(1,1)=-1/8*(1+r(1))*(1+s(0));         0033       N1tr(5,1)=1/8*(1+r(1))*(1+s(0));         0034       N1tr(1,1)=-1/8*(1+r(1))*(1+s(0));         0035       //Jacobian Matrix         1=zeros(3,3);       N1tr(N1s*(N1t');         0034       vol=v-vol;         0035       Jacobian Matrix         0036       avg=asigma;         0037       U=N1tr(N1s*(N1t');         0038       N1tr(1,1)=-1/8*(1+r(1))*(1+s(0));	0012	$\mathbf{N}_{1}$
0015       N1s(2,1)=-1/8*(1+r(1))*(1+(1));         0016       N1s(3,1)=-1/8*(1+r(1))*(1+(1));         0017       N1s(4,1)=-1/8*(1+r(1))*(1+(1));         0018       N1s(5,1)=-1/8*(1+r(1))*(1+(1));         0019       N1s(6,1)=-1/8*(1+r(1))*(1+(1));         0020       N1s(7,1)=-1/8*(1+r(1))*(1+(1));         0021       N1s(8,1)=-1/8*(1+r(1))*(1+(1));         0022       //Derivatives of shape functions w.r.t1         0024       ///derivatives of shape functions w.r.t1         0025       N1s(2,1)=-1/8*(1+r(1))*(1+s(1));         0026       N1s(1,1)=-1/8*(1+r(1))*(1+s(1));         0027       N1s(2,1)=-1/8*(1+r(1))*(1+s(1));         0028       N1s(3,1)=-1/8*(1+r(1))*(1+s(1));         0030       N1s(3,1)=-1/8*(1+r(1))*(1+s(1));         0031       N1s(3,1)=-1/8*(1+r(1))*(1+s(1));         0032       N1s(3,1)=-1/8*(1+r(1))*(1+s(1));         0033       N1s(3,1)=-1/8*(1+r(1))*(1+s(1));         0034       Jacobian Marix         0035       J-N*(1*s,y.z);         0036       J=zeros(3,3);         0037       J=N*(1*s,y.z);         0038       J=nvtJ;         0040       vel=v+vol;         0041       asigma=astrain(1,1)*det(J)+asigma;         0052       function	0013	$N_{1} \leq 1 > 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 + 1/2 $
0016       N1s(2,1)=1/8*(1+r(i))*(1-t(i));         0017       N1s(4,1)=1/8*(1+r(i))*(1+t(i));         0018       N1s(5,1)=1/8*(1+r(i))*(1+t(i));         0019       N1s(6,1)=1/8*(1+r(i))*(1+t(i));         0020       N1s(7,1)=1/8*(1+r(i))*(1+t(i));         0021       N1s(8,1)=1/8*(1+r(i))*(1+t(i));         0022       //Derivatives of shape functions w.r.t t         0023       //Derivatives of shape functions w.r.t t         0024       //N1s(7,1)=-1/8*(1+r(i))*(1+s(i));         0025       N1st=zeros(8,1);         0026       N1st(1,1)=-1/8*(1+r(i))*(1+s(i));         0027       N1st(2,1)=-1/8*(1+r(i))*(1+s(i));         0028       N1st(3,1)=-1/8*(1+r(i))*(1+s(i));         0029       N1st(3,1)=-1/8*(1+r(i))*(1+s(i));         0031       N1st(5,1)=-1/8*(1+r(i))*(1+s(i));         0032       N1st(3,1)=-1/8*(1+r(i))*(1+s(i));         0033       N1st(3,1)=-1/8*(1+r(i))*(1+s(i));         0034       //Jacobian Matrix         0035       //Jacobian Matrix         0036       J=N*(1s,v_2);         0037       N1st(3,1)=-1/8*(1+r(i))*(1+s(i));         0038       J=N*(1s,v_2);         0039       J=Inv(J);         0041       v=det(J);         0042       vol=v=vol;	0015	$N_{15}(2) = -1/(*(1+r(i))*(1+r(i)))$
0017       N1s(4,1)=1/8*(1-r(0))*(1+(0));         0018       N1s(5,1)=-1/8*(1+r(0))*(1+(1));         0019       N1s(5,1)=-1/8*(1+r(0))*(1+(1));         0020       N1s(7,1)=-1/8*(1+r(0))*(1+(1));         0021       N1s(2,1)=-1/8*(1+r(0))*(1+(1));         0023       //Derivatives of shape functions w.r.t t         0024       ///**********************************	0016	NIs(3,1)=1/8*(1+r(i))*(1-t(i));
0019       N1s(5,1)=-1/8*(1+ri))*(1+rii);         0019       N1s(7,1)=1/8*(1+rii))*(1+rii);         0020       N1s(7,1)=1/8*(1+rii))*(1+rii);         0021       N1s(8,1)=1/8*(1+rii))*(1+rii);         0023       //Derivatives of shape functions w.r.t t         0024       //Ntr(1,1)=1/8*(1+rii))*(1-sii);         0025       N1t=zeros(8,1);         0026       N1r(1,1)=-1/8*(1+rii))*(1-sii);         0027       N1r(2,1)=-1/8*(1+rii))*(1-sii);         0028       N1r(3,1)=-1/8*(1+rii))*(1-sii);         0029       N1r(1,1)=-1/8*(1+rii))*(1-sii);         0030       N1r(5,1)=1/8*(1+rii))*(1-sii);         0031       N1r(5,1)=1/8*(1+rii))*(1-sii);         0032       N1r(1,1)=1/8*(1+rii))*(1-sii);         0033       N1r(6,1)=1/8*(1+rii))*(1-sii);         0034       //Jacobian Matrix         0035       //Jacobian Matrix         0036       J=zeros(3,3);         0037       NE[N1r/N1s'N1r[];         0038       J=N*[*xyz];         0039       J=ainv(1);         0041       v=der(J);         0043       endfunction         0044       asigma=astrain(i, 1)*der(J)+asigma;         0055       endfunction         0056       avrg=asigma;	0017	N1s(4,1)=1/8*(1-r(i))*(1-t(i));
0019       N1s(c,1)=1/8*(1+r(i))*(1+t(i));         0020       N1s(c,1)=1/8*(1+r(i))*(1+t(i));         0021       N1s(c,1)=1/8*(1+r(i))*(1+t(i));         0023       //Derivatives of shape functions w.r.t t         0024       //***********************************	0018	N1s(5,1)=-1/8*(1-r(i))*(1+t(i));
0020         N1s(7,1)=1/8*(1+rii))*(1+rii);           0021         N1s(8,1)=1/8*(1+rii))*(1+rii);           0022         //Derivatives of shape functions w.r.t t           0025         N1t=zeros(8,1);           0026         N1t(1,1)=-1/8*(1+rii))*(1-s(i);           0027         N1t(2,1)=-1/8*(1+rii))*(1-s(i);           0028         N1t(2,1)=-1/8*(1+rii))*(1-s(i);           0029         N1t(4,1)=-1/8*(1+rii))*(1-s(i);           0030         N1t(5,1)=1/8*(1+rii))*(1-s(i);           0031         N1t(5,1)=1/8*(1+rii))*(1-s(i);           0032         N1t(7,1)=1/8*(1+rii))*(1+s(i);           0033         N1t(6,1)=1/8*(1+rii))*(1+s(i);           0034         N1t(8,1)=1/8*(1+rii))*(1+s(i);           0035         //Jacobian Matrix           0036         J=zeros(3,3);           0037         N=[N1riN1s'N1t'];           0038         J=Nt*[X,y,z];           0041         v=det(J);           0043         asigma=astrain(i,1)*det(J)+asigma;           0044         asigma=astrain(i,1)*det(J)+asigma;           0045         end           0046         avg=asigma;           0047         FUNCTION DISPLACEMENT RECOVERY           0050 //         FUNCTION DISPLACEMENT RECOVERY           0051	0019	N1s(6,1)=-1/8*(1+r(i))*(1+t(i));
001       NIs(8,1)=1/8*(1-r(i))*(1+t(i));         0023       //Derivatives of shape functions w.r.t t         0024       //***********************************	0020	N1s(7,1)=1/8*(1+r(i))*(1+t(i));
<pre>0022 0023 0024 0025 0025 0025 0026 0027 0027 0027 0027 0027 0027 0027</pre>	0021	N1s(8,1)=1/8*(1-r(i))*(1+t(i));
0023       //Derivatives of shape functions w.r.t1         0024       //***********************************	0022	
0024 ///////////////////////////////////	0023	//Derivatives of shape functions w.r.t t
0025 NIEzeros(8,1); 0026 NIt(1,1)=-1/8*(1-f(i))*(1-s(i)); 0027 NIt(2,1)=-1/8*(1+f(i))*(1-s(i)); 0028 NIt(3,1)=-1/8*(1-f(i))*(1-s(i)); 0030 NIt(5,1)=1/8*(1-f(i))*(1-s(i)); 0031 NIt(5,1)=1/8*(1+f(i))*(1-s(i)); 0033 NIt(5,1)=1/8*(1+f(i))*(1-s(i)); 0033 NIt(5,1)=1/8*(1-f(i))*(1+s(i)); 0034 //Jacobian Matrix 0035 //Jacobian Matrix 0035 J=zeros(3,3); 0037 Nt=[NIr';NIs';NIt']; 0038 J=Nt*[x,y,z]; 0039 U=inv(J); 0041 v=det(J); 0041 v=det(J); 0041 v=det(J); 0042 vol=v+vol; 0043 end 0044 asigma=astrain(i,1)*det(J)+asigma; 0045 end 0046 end 0047 endfunction 0049 // 0049 // 0049 // 0051 // FUNCTION DISPLACEMENT RECOVERY * 0050 // FUNCTION DISPLACEMENT RECOVERY * 0051 // 0051 u(yo+1:dof)=0; 0053 (u(yo+1:dof)=0; 0053 (t=1:mn(1)) 053 for j=1:3	0024	// "我是我说是我的学校的是是我的学校的是是我们的问题。"
0026 NI(1,1)=-1/8*(1-r(1))*(1-s(1)); 0027 NI(1,1)=-1/8*(1-r(1))*(1-s(1)); 0028 NI(1,1)=-1/8*(1-r(1))*(1-s(1)); 0030 NI(1,1)=-1/8*(1-r(1))*(1-s(1)); 0031 NI(1,1)=-1/8*(1-r(1))*(1-s(1)); 0032 NI(1,1)=-1/8*(1-r(1))*(1-s(1)); 0033 NI(1,1)=-1/8*(1-r(1))*(1+s(1)); 0034 //Jacobian Matrix 0036 J=zeros(3,3); 0037 N=[N1r:N1s;N1r']; 0038 J=Nt*[x,y,z]; 0039 J]=inv(J); 0041 v=det(J); 0042 vol=v+vol; 0043 asigma=astrain(1,1)*det(J)+asigma; 0044 asigma=astrain(1,1)*det(J)+asigma; 0045 end 0046 avrg=asigma; 0047 endfinuction 0048 0049 // 0051 // 0051 function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat) 0054 xc=xl(:,2);yc=xl(:,3);zc=xl(s,4); 0055 u(yo=1:dof)=(; 0056 for j=1:3)	0025	Nit=zeros(8,1);
$\begin{array}{llllllllllllllllllllllllllllllllllll$	0026	$N \Pi(1, 1) = -1/3^{-n} (1 - \Gamma(1))^{-1} (1 - S(1));$ $N \Pi(2, 1) = -1/3^{-n} (1 - \Gamma(1))^{-1} (1 - S(1));$
0029       N1R(4,1)=-1/8*(1-r(i))*(1-s(i));         0030       N1r(5,1)=1/8*(1-r(i))*(1-s(i));         0031       N1r(5,1)=1/8*(1-r(i))*(1-s(i));         0032       N1r(7,1)=1/8*(1-r(i))*(1-s(i));         0033       N1r(7,1)=1/8*(1-r(i))*(1-s(i));         0034       N1r(7,1)=1/8*(1-r(i))*(1-s(i));         0035       //Jacobian Matrix         0036       J=zeros(3,3);         0037       Nt=[N1r';N1s';N1r'];         0038       J=N*f(x,y,z];         0039       IJ=inv(0);         0041       v=det(J);         0042       vol=++vol;         0043       asigma=astrain(i,1)*det(J)+asigma;         0044       asigma=astrain(i,1)*det(J)+asigma;         0045       end         0046       avrg=asigma;         0047       end         0048       0049         0049       //         0050       //         0051       FUNCTION DISPLACEMENT RECOVERY         *       0051         0051       vc=xl(,2);yc=xl(,3);zc=xl((,4);         0052       function [DU, Elemendisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)         0053       gdof1=zeros(mn(1),3);         054       for j=1:3 <td>0027</td> <td>$N11(2,1) = 1/8^{-1}(1+5(1))^{-1}(1+5(1)),$</td>	0027	$N11(2,1) = 1/8^{-1}(1+5(1))^{-1}(1+5(1)),$
$\begin{array}{llllllllllllllllllllllllllllllllllll$	0028	$N_1(d_3) = 1/6 (1+t_1)/(1+s_1)/2$ $N_1(d_3) = 1/8 (1-t_1)/2 (1+s_1)/2$
0031       NIt(x,)=1/8*(1+r(i))*(1-s(i));         0032       NIt(7,)=1/8*(1+r(i))*(1+s(i));         0033       NIt(8,1)=1/8*(1-r(i))*(1+s(i));         0034       //Jacobian Matrix         0035       //Jacobian Matrix         0036       J=zeros(3,3);         0037       Nt=[N1r';N1s';N1r'];         0038       J=nt*[x,y,z];         0039       J=inv(J);         0040       vol=v+vol;         0041       v=det(J);         0042       vol=v+vol;         0043       asigma=astrain(i,1)*det(J)+asigma;         0044       asigma=astrain(i,1)*det(J)+asigma;         0045       end         0046       avg=asigma;         0047       edfunction         0048       0049 //         0050 //       FUNCTION DISPLACEMENT RECOVERY       *         0051 //       function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)       *         0052       function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)       *         0053       u(y+1:dof)=0;       *       *         0054       gdf1=zeros(mn(1),3);       *       *         0053       for j=1:3       *       *	0020	$NI(5, 1) = 1/8 \cdot (1-r(1)) \cdot (1-s(1)),$ NII(5, 1) = 1/8 \cdot (1-r(1)) \cdot (1-s(1)):
$\begin{array}{llllllllllllllllllllllllllllllllllll$	0031	NIt(6,1)= $ /\langle *( +r(i))*( -s(i))\rangle$
$\begin{array}{ccccc} 0033 & Nlt(\$,1)=1/\$^{\ast}(1-r(i))^{\ast}(1+s(i));\\ 0034 \\ 0035 & //Jacobian Matrix \\ 0036 & J=zeros(3,3);\\ 0037 & Nt=[Nlr;Nls';Nlr];\\ 0038 & J=Nt^{\ast}[x,y,z];\\ 0039 & J=inv(J);\\ 0040 \\ 0041 & v=det(J);\\ 0042 & vol=v+vol;\\ 0043 \\ 0044 & asigma=astrain(i,1)^{\ast}det(J)+asigma;\\ 0045 & end \\ 0046 & end \\ 0046 & end \\ 0047 & endfunction \\ 0048 \\ 0049 & //\\ \hline \\ \hline \\ 0050 & // & FUNCTION DISPLACEMENT RECOVERY & * \\ 0051 & // & FUNCTION DISPLACEMENT RECOVERY & * \\ 0052 & function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat) \\ 0055 & xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);\\ 0055 & u(xp+1:dof)=0;\\ 0056 & gdofl=zeros(mn(1),3);\\ 0538 & for i=1:mn(1) \\ 0539 & for j=1:3 \\ \hline \end{array}$	0032	$N_{1}(7, 1) = 1/8*(1+r(i))*(1+s(i)):$
0034 0035 //Jacobian Matrix 0036 J=zeros(3,3); 0037 N=[N1r;N1s;N1r']; 0038 J=Nt*[x,y,z]; 0039 JJ=inv(J); 0040 0041 v=det(J); 0042 vol=v+vol; 0044 asigma=astrain(i,1)*det(J)+asigma; 0045 end 0046 avrg=asigma; 0047 endfunction 0048 0049 // * 0050 // FUNCTION DISPLACEMENT RECOVERY * 0051 // 0052 function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat) 0052 function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat) 0052 function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat) 0053 dy (yo+1:dof)=0; 0053 gdof1=zeros(mn(1),3); 0054 tr=1; 0053 for j=1:3	0033	N1t(8,1)=1/8*(1-r(i))*(1+s(i));
0035       //Jacobian Matrix         0036       J=zeros(3,3);         0037       Nt=[N1r;N1s';N1t'];         0038       J=Nt*[x,y,z];         0039       IJ=inv(J);         0041       v=det(J);         0042       vol=v+vol;         0043       asigma=astrain(i,1)*det(J)+asigma;         0044       asigma=astrain(i,1)*det(J)+asigma;         0045       end         0046       avrg=asigma;         0047       endfunction         0048	0034	
0036 J=zeros(3,3); 0037 Nt=[N1r';N1s';N1t']; 0038 J=Nt*[x,y,2]; 0040 0041 v=det(J); 0042 vol=v+vol; 0043 0044 asigma=astrain(i,1)*det(J)+asigma; 0045 end 0046 avrg=asigma; 0047 endfunction 0048 0049 // ***********************************	0035	//Jacobian Matrix
0037       Nt=[N1r;N1s';N1r'];         0038       J=Nt*[x,y,z];         0039       U=inv(J);         0040       v=det(J);         0042       vol=v+vol;         0043       out=v=vol;         0044       asigma=astrain(i,1)*det(J)+asigma;         0045       end         0046       avrg=asigma;         0047       endfunction         0048       out=v=vol;         0049 //       *         ************************************	0036	J=zeros(3,3);
0038 J=Nt*[x,y,z]; 0039 IJ=inv(J); 0040 0041 v=det(J); 0042 vol=v+vol; 0043 0044 asigma=astrain(i,1)*det(J)+asigma; 0045 end 0046 avrg=asigma; 0047 endfunction 0048 0049 // ***********************************	0037	Nt=[N1r';N1s';N1t'];
0039       J=inv(J);         0040       v=det(J);         0041       v=det(J);         0043       ou=v+vol;         0044       asigma=astrain(i,1)*det(J)+asigma;         0045       end         0046       avrg=asigma;         0047       endfunction         0048       ou49         0049       //         *       function IDU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)         0052       function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)         0053       u(yo+1:dof=0;         0054       xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);         0055       u(yo+1:dof=0;         056       gdof1=zeros(mn(1),3);         057       tt=1;         058       for i=1:mn(1)         0539       for j=1:3	0038	J=Nt*[x,y,z];
0040 0041 v=det(J); 0042 vol=v+vol; 0043 0044 asigma=astrain(i,1)*det(J)+asigma; 0045 end 0046 avrg=asigma; 0047 endfunction 0048 0049 // ***********************************	0039	IJ=mv(J);
$\begin{array}{cccccccc} 0041 & v=(de(J); \\ 0042 & vol=v+vol; \\ 0043 & asigma=astrain(i,1)*det(J)+asigma; \\ 0045 & end \\ 0046 & avrg=asigma; \\ 0047 & endfunction \\ 0048 & 0049 // \\ 0049 // \\ \hline \\ 0050 // & FUNCTION DISPLACEMENT RECOVERY & * \\ 0051 // \\ \hline \\ 0052 & function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat) \\ 0053 & u(yo+1:dof)=0; \\ 0054 & xc=xl(:,2);yc=xl(:,3);zc=xl(:,4); \\ 0055 & u(yo+1:dof)=0; \\ 0536 & gdofl=zeros(mn(1),3); \\ 0537 & tt=1; \\ 0538 & for i=1:mn(1) \\ 0539 & for j=1:3 \end{array}$	0040	
$\begin{array}{llllllllllllllllllllllllllllllllllll$	0041	
0043       asigma=astrain(i,1)*det(J)+asigma;         0045       end         0046       avrg=asigma;         0047       endfunction         0048       0049 //         0050       //         FUNCTION DISPLACEMENT RECOVERY       *         0051       //         0052       function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)         0053       0054       xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);         0055       u(yo+1:dof)=0;         0536       gdofl=zeros(mn(1),3);         0537       tt=1;         0538       for i=1:mn(1)         0539       for j=1:3	0042	VOL-V+VOI,
0045       end         0046       avrg=asigma;         0047       endfunction         0048       0049 //         0050       //         FUNCTION DISPLACEMENT RECOVERY       *         0051       //         0052       function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)         0053       0054       xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);         0055       u(yo+1:dof)=0;         0536       gdofl=zeros(mn(1),3);         0537       tt=1;         0538       for i=1:mn(1)         0539       for j=1:3	0043	asigma=astrain(i_1)*det(1)+asigma.
0046 avrg=asigma; 0047 endfunction 0048 0049 // ***********************************	0045	end
0047 endfunction 0048 0049 // ***********************************	0046	avrg=asigma:
0048 0049 // ***********************************	0047	endfunction
0049 // ***********************************	0048	
<pre>************************************</pre>	0049	//
0050 //       FUNCTION DISPLACEMENT RECOVERY       *         0051 //       *         0052 function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)       *         0053       0054 xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);         0055 u(yo+1:dof)=0;       *         0536 gdofl=zeros(mn(1),3);       *         0538 for i=1:mn(1)       *         0539 for j=1:3       *	****	***************************************
0051 // 0052 function [DU, Elementdisp]= <u>DispRecovery</u> (u, yo, mn, dof, ek, gdofg, xl, Elmat) 0053 0054 xc=xl(:,2);yc=xl(:,3);zc=xl(:,4); 0055 u(yo+1:dof)=0; 0536 gdofl=zeros(mn(1),3); 0537 tt=1; 0538 for i=1:mn(1) 0539 for j=1:3	0050	// FUNCTION DISPLACEMENT RECOVERY *
0052       function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)         0053       xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);         0055       u(yo+1:dof)=0;         0536       gdofl=zeros(mn(1),3);         0537       tt=1;         0538       for i=1:mn(1)         0539       for j=1:3	0051	
0052       function [D0, Elementdisp]=DispRecovery(u, yo, mn, dot, ek, gdofg, xl, Elmat) $0053$ $0054$ $xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);$ $0055$ $u(yo+1:dof)=0;$ $0536$ $gdofl=zeros(mn(1),3);$ $0537$ $tt=1;$ $0538$ for i=1:mn(1) $0539$ for j=1:3	*****	
$\begin{array}{rl} 0053 \\ 0054 & \textbf{xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);} \\ 0055 & \textbf{u(yo+1:dof)=0;} \\ 0536 & \textbf{gdofl=zeros(mn(1),3);} \\ 0537 & \textbf{tt=l;} \\ 0538 & \textbf{for i=1:mn(1)} \\ 0539 & \textbf{for j=1:3} \end{array}$	0052	function [DU, Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat)
$\begin{array}{rcl} 00.54 & \mathbf{x} \in \mathbf{x}(\cdot, z), \mathbf{y} \in \mathbf{x}(\cdot, z); \mathbf{z} \in \mathbf{z}(\cdot, z); \mathbf{z}(\cdot, z); \mathbf{z} \in \mathbf{z}(\cdot, z); \mathbf{z}(\cdot, $	0053	n-n/(, ))ua-n/(, ))n-n/(, 4).
$\begin{array}{llllllllllllllllllllllllllllllllllll$	0054	$A \cup -A(,,z), y \cup -A(,,z), z \in X(,,+),$ $u(y_{A+1}, z_{A}) \in D^{-1}$
$\begin{array}{llllllllllllllllllllllllllllllllllll$	0536	adof_acros(m(1)3);
0538 for i=1:mn(1) 0539 for j=1:3	0537	5007-500(m(x),5), tt=:
0539 for j=1:3	0538	for i=1:mn(1)
	0539	for j=1:3
0540 gdof1(i,j)=tt; 0001 tt=tt+1; 0002 end 0003 end 0004 0005 DU=zeros(dof,1); 0006 for i=1:mn 0007 for j=1:30008 DU(gdof1(i,j))=u(gdofg(i,j)); 0009 end 0010 end 0011 CM=zeros(ek,24); 0012 for i=1:ek 0013 p=1;k=0; 0014 for j=1:8 0015 for l=p:p+2 0016 CM(i,l)=gdof1(Elmat(i,j),l-k); 0017 end 0018 k=l;p=l+1;0019 end 0020 end 0021 Elementdisp=zeros(24,ek); 0022 0023 for i=1:ek 0024 x = zeros(8,1);0025 **y**=**zeros**(8,1); 0026 z=zeros(8,1);for h=1:8 0027 0028 j=Elmat(i,h); 0029 x(h,1)=xc(j);0030 y(h,1)=yc(j); 0031 z(h,1)=zc(j); 0032 end Eldisp=zeros(24,1); 0033 0034 for j=1:24 pp=CM(i,j); 0035 Eldisp(j,1)=DU(pp,1); 0036 0037 end 0038 Elementdisp(:,i)=Eldisp; 0039 end 0040 0041 0042 endfunction 0043 // 0044 // END OF FUNCTION DISPLACEMENT RECOVERY 0045 // ****** 0046 0047 // 0048 // FUNCTION STRAIN & STRESS RECOVERY * 0049 // ****** 0050 function [Epsilon, avstrainz, avstrainx, avstrainz, avstrainz, avstrainz, avstrainz, avstrainz)=<u>StrainRecovery</u>(Elementdisp, MatProp, ek, Elmat, xl, Eprop) 0051 0052 avstrainz=0;avstrainy=0;avstrainyz=0;avstrainzx=0;avstrainxy=0; 0593 rr=[-1/sqrt(3);1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3);-1/sqrt(3)]; 0594 ss=[-1/sqrt(3);-1/sqrt(3);1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3)]; 0595 tt=[-1/sqrt(3);-1/sqrt(3);-1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3);1/sqrt(3)

0596

```
0597
            xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);
0598
            E11=Eprop(:,2);E22=Eprop(:,3);E33=Eprop(:,4);
0599
            G12=Eprop(:,5);G23=Eprop(:,6);G13=Eprop(:,7);
0600
            nu12=Eprop(:,8);nu23=Eprop(:,9);nu13=Eprop(:,10);
0601
            mix=Eprop(:,11);miy=Eprop(:,12);miz=Eprop(:,13);mu=Eprop(:,14);
0001
            theta=Eprop(:,15);thetax=Eprop(:,16);
0002
            Epsilon=zeros(48,ek);
0003
          for w=1:ek
0004
            mp=MatProp(w,1);
0005
            for i=1:8
0006
               j=Elmat(w,i);
0007
               x(i,1)=xc(j);
0008
               y(i,1)=yc(j);
0009
               z(i,1)=zc(j);
0010
            end
0011
0012
            Strain=zeros(6,8);
0013
            for i=1:8
0014
0015
              r=rr(i);
0016
              s=ss(i);
0017
              t=tt(i);
0018
              [Bm]=<u>Bmatrix(x,y,z,r,s,t);</u>
0019
              Strain(:,i)=Bm*Elementdisp(:,w);
0020
0021
           end
0022
           Epsilon(:,w) = matrix(Strain,48,1);
           for i=1:8
0023
0024
             ostrain(i,1)=Strain(1,i);
0025
             ostrain1(i,1)=Strain(2,i);
0026
             ostrain2(i,1)=Strain(3,i);
0027
             ostrain3(i,1)=Strain(4,i);
0028
             ostrain4(i,1)=Strain(5,i);
0029
             ostrain5(i,1)=Strain(6,i);
0030
           end
0031
             avstrainx=aveg(ostrain,x,y,z)+avstrainx;
0032
             avstrainy=aveg(ostrain1,x,y,z)+avstrainy;
0033
              avstrainz=aveg(ostrain2,x,y,z)+avstrainz;
0034
             avstrainxy=aveg(ostrain5,x,y,z)+avstrainxy;
0035
             avstrainyz=aveg(ostrain3,x,y,z)+avstrainyz;
0036
              avstrainzx=aveg(ostrain4,x,y,z)+avstrainzx;
0037 end
0038
0039
0040 endfunction
0041
0042
0043
0044
 function [Sigma, avstressz, avstressy, avstressy, avstressy, avstressy]=StressRecovery(Epsilon, MatProp, ek, Eprop, xl, Elmat)
0045
0046
            avstressz=0;avstressy=0;avstressxy=0;avstressyz=0; avstressyz=0;
0047
            xc=xl(:,2);yc=xl(:,3);zc=xl(:,4);
            E11=Eprop(:,2);E22=Eprop(:,3);E33=Eprop(:,4);
0048
0049
            G12=Eprop(:,5);G23=Eprop(:,6);G13=Eprop(:,7);
0050
            nu12=Eprop(:,8);nu23=Eprop(:,9);nu13=Eprop(:,10);
0051
            mix=Eprop(:,11);miy=Eprop(:,12);miz=Eprop(:,13);mu=Eprop(:,14);
0052
            theta=Eprop(:,15);thetax=Eprop(:,16);
0654
            Sigma=zeros(48,ek);
0655
          for w=1:ek
0656
           mp=MatProp(w,1);
```

0001 for i=1:8

0002 j=Elmat(w,i); 0003 x(i,1)=xc(j);0004 y(i,1)=yc(j);0005 z(i,1)=zc(j); 0006 end 0007 Stress=zeros(6,8); 0008 modEpsilon=matrix(Epsilon(:,w),6,8); 0009 0010 [D]=Dmatrix(E11(mp,1),E22(mp,1),E33(mp,1),G12(mp,1),G13(mp,1),nu12(mp,1),nu13(mp,1),nu13(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mn,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),mix(mp,1),m 0011 [Cij]=<u>Rmatrix</u>(D,thetax(mp,1)); 0012 Stress=[Cij]*modEpsilon; 0013 0014 0015 Sigma(:,w) = matrix(Stress,48,1); 0016 for i=1:8 0017 ostress(i,1)=Stress(3,i);ostress1(i,1)=Stress(2,i);ostress2(i,1)=Stress(1,i); 0018 ostress3(i,1)=Stress(6,i);ostress4(i,1)=Stress(5,i);ostress5(i,1)=Stress(4,i); 0019 end 0020 avstressz=aveg(ostress,x,y,z)+avstressz; 0021 avstressy=aveg(ostress1,x,y,z)+avstressy; 0022 avstressx=aveg(ostress2,x,y,z)+avstressx; 0023 avstressyz=aveg(ostress5,x,y,z)+avstressyz; 0024 avstressxy=aveg(ostress3,x,y,z)+avstressxy; 0025 avstressxz=aveg(ostress4,x,y,z)+avstressxz; 0026 0027 end 0028 0029 0030 endfunction 0031 0032 function [PStress]=PStressRecovery(Sigma, ek) 0033 0034 for w=1.ek 0035 PSigma=matrix(Sigma(:,w),6,8); 0036 for i=1:8 0037 0038 A=[PSigma(1,i) PSigma(6,i) PSigma(5,i); 0039 PSigma(6,i) PSigma(2,i) PSigma(4,i); 0040 PSigma(5,i) PSigma(4,i) PSigma(3,i)]; 0041 dig=spec(A); 0042 [m,n] =max(real(dig)); 0699 PStress(i,w)=m; 0001 end0002 end 0003 0004 endfunction 0005 0006 // END OF FUNCTION STRESS RECOVERY 0007 // 0008 // 0009 0010 // * 0011 // FUNCTION TO WRITE OUTPUT FILES 0012 // 0013 0014 function [fim]=postfile(Elementdisp, ek, Epsilon, Sigma, xl, PStress) 0015

0016 0716	<b>xc=xl</b> (:,2); <b>yc=xl</b> (:,3); <b>zc=xl</b> (:,4);
0717	fister-monon('C')Users/Develor Neik/Downloads/stressouty pos' 'wt')
0719	fistry-monen ('C.\Users\Dayakar Naik\Downloads\stressouty.pos', wt'),
0710	fistrz-monen('C:\Users\Dayakar Naik\Downloads\stressoutz.pos', wt'),
0719	fistre-monen('C.)Users/Dayakar Naik/Downloads/Silessoutz.pos, wt ),
0720	fimemonon('C.\Users\Dayakar Naik\Downloads\rstressout.pos, wt),
0721	fim=mopen(C:\Users\Dayakar Naik\Downloads\dispgmZ.pos, wt)
0722	<b>imi=mopen</b> (C:\Users\Dayakar Naik\Downloads\dispgm Y.pos, wt)
0723	Tim2=mopen(C:\Users\Dayakar Naik\Downloads\dispgmX.pos, wt)
0724	
0725	miprinti(fim, view);
0001	mipini((min, ),
0002	miprinu(nim, Displacement in Z);
0003	mprint(min, );
0004	$\operatorname{III}_{\mathcal{D}}(\operatorname{IIII}, \{ \langle \operatorname{II} \rangle \})$
0005	
0000	mforintf(fim1 'View')
0007	mfprintf(fim1, view),
0000	mfprintf(fim1 'Displacement in V'):
0000	mfprintf(fim1 " "" ").
0010	miprint(imi), ),
0011	mpmu(mm, (),
0012	
0013	mfprintf(fim2 'View')
0015	mfprintf(fim2, """).
0015	mfprintf(fim2 'Displacement in X'):
0017	mfprintf(fim2 " "" ")·
0018	mfprintf(fim2;'/')
0019	······································
0020	mfprintf(fistrx 'View ')
0021	mfprintf(fistrx," """):
0022	mfprintf(fistrx 'Stress in X')
0023	mfprintf(fistrx," """):
0024	mfprintf(fistrx.'{'):
0025	······································
0026	mfprintf(fistry,'View ');
0027	mfprintf(fistry," "" ");
0028	mfprintf(fistry,'Stress in Y');
0029	mfprintf(fistry," "" ");
0030	mfprintf(fistry,'{');
0031	
0032	mfprintf(fistrz,'View ');
0033	mfprintf(fistrz," "" ");
0034	mfprintf(fistrz,'Stress in Z');
0035	<pre>mfprintf(fistrz," "" ");</pre>
0036	mfprintf(fistrz,'{');
0037	
0038	mfprintf(fistrp,'View ');
0039	mfprintf(fistrp," "" ");
0040	mfprintf(fistrp,'Max Principal Stress');
0041	mfprintf(fistrp," "" ");
0042	<pre>mfprintf(fistrp,'{');</pre>
0043	
0044	for w=1:ek
0045	
0046	tor I=1:8
0047	J=Elmat(w,i);
0048	x(1,1) = xc(1);
0049	y(1,1) = yc(1);
0050	$Z(I, \bot) = ZC(J);$
0051	ena

0052		
0053	Eldisp=Elementdisp(:,w);	
0054	uzdisp=zeros(8,1);	
0055	uydisp=zeros(8,1);	
0056	uxdisp=zeros(8,1);	
0057	kk=1;	
0058	for jj=1:8	
0059	uzdisp(jj,1)=Eldisp(kk+2,1);	
0060	uydisp(ij,1) = Eldisp(kk+1,1);	
0061	uxdisp(jj,1)=Eldisp(kk,1);	
0062	kk=kk+3;	
0063	end	
0064		
0065		
0066	Epsi=Epsilon(:,w);	
0067	Epsix=zeros(8,1);	
0068	Epsiy=zeros(8,1);	
0069	Epsiz=zeros(8,1);	
0070	kk=1;	
0071	for jj=1:8	
0072	Epsiz(jj,1) = Epsi(kk+2,1);	
0073	Epsiv(jj,1) = Epsi(kk+1,1);	
0074	Epsix(jj,1) = Epsi(kk,1);	
0075	kk=kk+6;	
0076	end	
0077		
0078	PStressp=PStress(:,w);	
0079	Sigm=Sigma(:,w);	
0080	Sigmx=zeros(8,1);	
0081	Sigmy=zeros(8,1);	
0082	Sigmz = zeros(8,1);	
0083	kk=1;	
0084	for jj=1:8	
0085	$\widetilde{Sigmz(jj,1)}=Sigm(kk+2,1);$	
0086	Sigmy(jj,1)=Sigm(kk+1,1);	
0087	Sigmx(jj,1)=Sigm(kk,1);	
0088	kk=kk+6;	
0089	end	
0090		
0091	mfprintf(fim,'\nSH(');	
0092	mfprintf(fim,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));	
0093	mfprintf(fim,'%f,%f,%f',x(8),y(8),z(8));	
0094	mfprintf(fim,'){')	
0095	<pre>mfprintf(fim,'%f,',uzdisp(1:7));</pre>	
0096	mfprintf(fim,'%f',uzdisp(8));	
0097	mfprintf(fim,'};\n')	
0098		
0099	mfprintf(fim1,'\nSH(');	
0100	mfprintf(fim1,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));	
0101	mfprintf(fim1,'%f,%f',%f',x(8),y(8),z(8));	
0102	mfprintf(fim1,'){')	
0103	<pre>mfprintf(fim1,'%f,',uydisp(1:7));</pre>	
0104	mfprintf(fim1,'%f',uydisp(8));	
0105	mfprintf(fim1,'};')	
0106		
0107	mfprintf(fim2,'\nSH(');	
0108	mfprintf(fim2,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));	
0109	mfprintf(fim2,'%f,%f,%f',x(8),y(8),z(8));	
0110	mfprintf(fim2,'){')	
0111	<pre>mfprintf(fim2,'%f,',uxdisp(1:7));</pre>	
0112	mfprintf(fim2,'%f',uxdisp(8));	
0113	mfprintf(fim2,'};')	

```
0114
0115
           mfprintf(fistrx,'\nSH(');
           mfprintf(fistrx,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));
0116
            mfprintf(fistrx,'%f,%f,%f',x(8),y(8),z(8));
0117
            mfprintf(fistrx,'){');
0118
            mfprintf(fistrx,'%f,',Sigmx(1:7));
0119
0120
            mfprintf(fistrx,'%f',Sigmx(8));
0121
            mfprintf(fistrx,'};');
0122
0123
            mfprintf(fistry,'\nSH(');
0124
            mfprintf(fistry,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));
0125
            mfprintf(fistry,'%f,%f,%f',x(8),y(8),z(8));
            mfprintf(fistry,'){');
0126
            mfprintf(fistry,'%f,',Sigmy(1:7));
0127
0128
            mfprintf(fistry,'%f',Sigmy(8));
            mfprintf(fistry,'};');
0129
0130
0131
            mfprintf(fistrz,'\nSH(');
            mfprintf(fistrz,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));
0132
            mfprintf(fistrz,'%f,%f,%f',x(8),y(8),z(8));
0133
0134
            mfprintf(fistrz,'){');
            mfprintf(fistrz,'%f,',Sigmz(1:7));
0135
0136
            mfprintf(fistrz,'%f',Sigmz(8));
0137
            mfprintf(fistrz,'};');
0138
            mfprintf(fistrp,'\nSH(');
0139
0140
            mfprintf(fistrp,'%f,%f,%f,',x(1:7),y(1:7),z(1:7));
            mfprintf(fistrp,'%f,%f,%f',x(8),y(8),z(8));
0141
0142
            mfprintf(fistrp,'){');
0143
            mfprintf(fistrp,'%f,',PStressp(1:7));
            mfprintf(fistrp,'%f',PStressp(8));
0144
0145
            mfprintf(fistrp,'};');
0146
0147
0148 end
0149
0150 mfprintf(fistrx,'};')
0151 mfprintf(fistry,'};')
0152 mfprintf(fistrz,'};')
0153 mfprintf(fim,'};')
0154 mfprintf(fim1,'};')
0155 mfprintf(fim2,'};')
0156 mfprintf(fistrp,'};')
0157
0158 mclose(fim);
0159 mclose(fim1);
0160 mclose(fim2);
0161 mclose(fistrx);
0162 mclose(fistry);
0163 mclose(fistrz);
0164 mclose(fistrp);
0165
0166 endfunction
0167
0168 //
0169 //
                                                                                                      *
                      END OF FUNCTION WRITE OUTPUT FILES
0170 //
0171
0172 //
```

0173	// MAIN PROGRAM	*
****	· // ***********************************	******
0175	stacksize('max');clear all;clc;	
0177	cpi = getdate(); // initialising time	
0177	cpt(3) = cpt(5)+cpt(7)/1000;	
0179	$(p_1(0) - c_1(10) + c_1(1/1000), (p_1(1/1000)))$	
0180	(iso) (Iterative Method Solution ')	
0181	fnd = mopen'(C:\Users\Dayakar Naik\Downloads\nodes3d.txt', 'rt');	
0182	fed = mopen('C:\Users\Dayakar Naik\Downloads\elements3d.txt', 'rt');	
0183	fmd = mopen('C:\Users\Dayakar Naik\Downloads\materials3d.txt', 'rt');	
0909	fkd=mopen('C:\Users\Dayakar Naik\Downloads\restraint1.txt','rt');//kdof	
0910	fcst=mopen('C:\Users\Dayakar Naik\Downloads\constr1.txt','rt')	
0911		
0912	xl=mfscanf(-1,fnd,'%f %f %f %f %f');	
0913	$E = mfscanf(-1, fed, \%_1, \%_1, \%_1, \%_1, \%_1, \%_1, \%_1, \%_1$	
0914	Eprop=miscan((-1, imd, %i	
0915	kuo = miscan(-1, iku, % 1); aca = miscan(-1, iku, % 1); aca = miscan(-1, iku, % 1);	
0917	ccq-iniscan(-1,icsi, /01 /01 /01 /01 /01 /01 /01 /01 /01 /01	
0918	$[mk nk] = size(kdo) \cdot [mn nn] = size(x]) \cdot [m n] = size(E]) \cdot [mc nc] = size(cea) \cdot cea)$	
0919	MatProp = E[(2):tdof=m(1)*3:dof=tdof=mc(1):ek=m:	
0920		
0921		
0922	mclose(fnd);	
0923	mclose(fed);	
0924	mclose(fmd);	
0925	mclose(tkd);	
0926	mciose(rest);	
0928	ticO	
0929	MatProp. Elmat. gdofg. CM. nceg = GenerateDOF(x]. El. Eprop. kdo. ceg):	
0930	[KG,re, yo]= <u>Assemble</u> (xl, Elmat, Eprop, CM, nceq, MatProp, ek, tdof, dof, mk, mc);	
0931	save('val.dat','KG','re');	
0932	printf('\n Time needed to Assemble Stiffness Matrix: %.3f sec\n', <u>toc()</u> );	
0933	<u>tic();pf</u> =500000;	
0934	//[u,a]=FEMsol(KG,re,yo,mc,pf);//clear KG;clear re; // For Iterative Solution	
0935	stacksize(max)	
0930	(u)= <u>PEMSO(KG,re,yo,mc,pr);</u> //clear KG;clear re; // For Direct Solution	
0937	//u_=un(KO(1,y,r.i,y))/un((c(1,y)))	
0939	tic():	
0940	[DU,Elementdisp]=DispRecovery(u, yo, mn, dof, ek, gdofg, xl, Elmat);clear u;	
0941		
[Eps 0942	silon, avstrainz, avstrainy, avstrainyz, avstrainzx, avstrainzy]= <u>StrainRecovery</u> (Elementdisp, MatProp, ek, Elr	nat, xl, Eprop);
Sig	gma, avstressz, avstressy, avstressx, avstressy, avstressyz, avstressxz]=StressRecovery(Epsilon, MatProp, ek, Eprop, xl	, Elmat);
0943	[PStress]= <u>PStressRecovery</u> (Sigma, ek);	
0944	[fim]= <u>postfile</u> (Elementdisp,ek, Epsilon, Sigma, xl, PStress);	
0945	<pre>printf(\n Time needed to Recover Stresses: %.3f sec\n',toc());</pre>	
0946		
0947		
0948	cpt = getuate();	
0950	cp(0,0) = cpt(6) + cpt(7)/1000;	
0951	cpt3 = cpt(1:6):	
0952	printf(\n Time needed to Run Full Program: %.3f sec\n', <u>etime(cpt3,cpt1));</u>	
0953		
0954	stacksize('min');	
0955		
0956	$\kappa z = [avstressz/avstrainz avstrainx/avstrainz avstrainy/avstrainz avstrainy/avstrainz avstrainz/avstrainz avstrainz/avstrainz avstrainz/avstrainz avstrainz/avstrainz avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrainz/avstrai$	/avstrainz];

0957RY=[avstressy/avstrainy avstrainx/avstrainy avstrainy/avstrainy avstrainy/avstrainy avstrainy/avstrainy avstrainy/avstrainy];0958RX=[avstressx/avstrainx avstrainx/avstrainx avstrainy/avstrainx avstrainy/avstrainx avstrainy/avstrainx];

## Vita

## Dayakar L. Naik

## **Published Journal Articles**

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