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ABSTRACT

In this report a methodology is described for determining the optimal allocation of water supplies in the State of Utah to minimize the cost of meeting an assumed set of water requirements. A linear programming model was formulated to represent the ten interconnected hydrologic study areas of the state. The comprehensive model considers virtually all uses, areas, sources, transfers and costs of water. The model has 204 constraints and 338 variables and was solved by the simplex method.

Included in the results are the following: the optimal water allocation of the groundwater, surface water, and water transfers which minimize the cost; the shadow prices of the resources; sensitivity analyses to identify the critical cost coefficients in the optimal solution; parametric analyses to test the effects of changing constraints; and manipulations of the model to test other factors such as operating rules, legal policies, political and institutional limitations. The tabulated data were carefully condensed so as to be more easily understood. Flow diagrams and graphs summarize the important information. The work is fully documented so others can follow what was done and improve the method or apply the model to other areas.

Keywords: *Water resources planning, *Water management, *Operations research, *Optimal allocation, *Utah, Hydrology, Surface water, Groundwater, Water storage, Water transfer, Water requirements, Water supply, Municipal water use, Industrial water use, Agricultural water use, Wetlands water use, Groundwater recharge, Water reuse, Mathematical model, Linear programming, Objective function, Constraints, Simplex method, Cost minimization, State water plan, Legal aspects, Social aspects, Political factors.

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SUMMARY

Water resources planners have the responsibility of developing plans which will best utilize scarce water resources. It is difficult for planners to see in advance which of the many possible alternatives will most enhance the well-being of people within a planning area. The general objective of the research reported in this study has been to assist the water resource planners of the State of Utah in particular and in other states in general by developing a methodology for optimal allocation of available water such that the broad overview and objectives of the state can be incorporated in planning decisions. The research has extended the capability for mathematical analysis of complex water resource systems to a state-wide area in which the many multiple alternatives and interrelationships can be considered simultaneously. The research was designed to enhance the quality of decisions to be finalized in the State Water Plan of Utah and in other water plans.

A particular problem to be solved is how best to utilize Utah's share of Colorado River waters. There are also some other available surface water and groundwater in the state not now being used. In this research a number of alternative patterns and levels of demand for water are postulated for the ten study areas of the state. The costs of meeting the projected demands were then minimized by solving a linear programming model of the economic-hydrologic-physical system. The cost-minimizing system consists of the various combinations of groundwater, surface water, and interregional transfer activities which minimize the cost.

A mathematical programming model with the appropriate constraints was formulated to represent the ten study areas of Utah. The constraints in the model include the following categories: groundwater and surface water availabilities in the various areas, water requirements of all kinds (including municipal and industrial, wetlands, and agriculture), present and potential reservoir storage, evaporation losses from storage, return flows, free groundwater for wetlands, groundwater recharge limits, inter-basin water transfer limits, required outflows and other physical limits on the system. The model is comprehensive and all inclusive rather than partial, and virtually all uses, areas, sources, and transfers of water have been included in the analysis.

In developing the objective function for the model, the minimum cost to supply an assumed water requirement was selected as the criteria for the optimal allocation of water by the model. It was realized that a least cost allocation is not as meaningful as an allocation done with net benefits as the measure of value. However, the available project resources precluded giving attention to the general question of the value of water. Fortunately, the important question of the value of water and its effect on the optimal allocation is already under study in another project.

A method was developed in the study which allows consideration of the full cost structure, rather than just part of the development costs. In fact, the cost coefficients in the objective function and the appropriate cost constraints can be made just as comprehensive and complete as the resources and time available might allow. Little effort was expended in trying to define the costs precisely. In many cases the best available estimate was used, since the objective was to work out a methodology of water planning rather than to carry out a specific water planning activity.

The final version of the model used in this research contains, besides the objective function, 204 constraints with 338 variables. The linear programming model was solved by an IBM 360/44 digital computer using a form of the simplex method contained in a mathematical programming package supplied by IBM and identified as MPS/360.

Results from the model are of three kinds: 1) The optimum solution to the linear programming problem—including both the optimal allocation of the water resources and the determination of the shadow prices of these resources. 2) The post-optimal analysis—including a sensitivity analysis of the cost coefficients to determine which are most crucial to the solution and a parametric analysis of the right-hand-side values of the constraints to test the effects of changing the constraints, particularly changing the projections of demand for water over time. 3) Manipulation of structural coefficients, right-hand-side values and variable bounds to determine such effects as changes in irrigation efficiency, changes in operating rules, legal policies, political and social limitations, groundwater restrictions, water transfer limitations, alternative growth projections, etc.

The computer program supplies large amounts of data and these were carefully condensed so the data could be examined, interpreted, and understood. Flow diagrams and graphs are the result of this effort to distill the important information from the voluminous computer output.

Under this research effort a methodology has been developed for determining the optimal allocation of water supplies to minimize the cost of meeting given require-

ments for water in a large and complex area. The research was done by an interdisciplinary team so as to utilize various viewpoints and skills. Considerable effort was made to work closely with the appropriate state and federal agencies to make the study represent real world problems in water resource allocation. The method is broad in scope and the suggested model is flexible so it can be applied in planning situations other than in the State of Utah. The work has been documented in this report so that others can follow what was done, improve upon the method, or apply the model to other areas.

INTRODUCTION

Development of the State's Water Resources

National water planning program

The concept of comprehensive water resources planning is not new. By the turn of this century the interdependencies in the development and use of water for various purposes were very evident to some of the nations leaders. President Theodore Roosevelt's instructions to the Inland Waterways Commission in 1907 outlined the idea much the same as we know it today:

The time has come for merging local projects and uses of the inland waters in a comprehensive plan designed for the benefit of the entire country. Such a plan should consider all the uses to which streams may be put and should bring together and coordinate all the points of view of all users of water... (U.S. Congress, Senate, 1908).

Numerous national water commissions and committees have espoused this concept during the ensuing 50 years. During this period, the growth of institutional complexities and the recognition of additional uses of water have broadened the meaning of the term "comprehensive water planning" and made the implementation of this approach imperative though much more difficult.

Although Congress has continued to authorize individual projects, recently it has taken a number of significant and impressive steps toward a comprehensive approach to planning. The Senate Select Committee on National Water Resources report in 1961 contained recommendations for establishing a national water planning program and a research program under which, among other things, planning techniques might be improved (U.S. Congress, Senate, 1961). An important statement of federal policies, standards, and procedures for water planning and development was printed in Senate Document 97, 87th Congress in 1962. These policies and standards were intended to provide a common basis for formulation, evaluation and review of plans, and encouraged a comprehensive, long-range viewpoint in planning with full consideration of all types of water demands and development possibilities. Efforts to establish machinery for coordinating the diverse interests in planning represented by a large number of federal, state, and local agencies culminated in the Water Resources Planning Act of 1965, which established the Water Resources Council and provided financial assistance to states to improve state

potentials for water planning. The Act further provided for the establishment of river-basin planning commissions made up of state and federal regional representatives and the implementation of a planning program to prepare and keep up-to-date plans for comprehensive water development for all major river basins in the United States. Today, under the aegis of the U.S. Water Resources Council, five congressionally authorized river basin commissions, newly established under provisions of the Water Resources Planning Act of 1965, and a number of basin interagency committees are engaged in a nationwide water planning program.

Framework plans, being prepared by these organizations, will cover large multi-state areas and provide basic information on the future requirements for resource development; inventories of available resources; interrelationships of resource uses, problems, and suggested solutions; and a broad-gaged plan to be used as a guide for development. Together these framework plans will cover the entire nation and provide the basis for a total assessment of water resources.

Regional and river basin plans covering river systems or subregions within the areas of the framework plans are also the responsibility of river basin commissions and basin interagency committees. These plans are intended to extend the scope and intensity of the framework plans.

As members of river basin commissions and interagency committees, states are participating with the federal agencies in this planning program. The State of Utah is involved in the development of framework plans for the Great Basin and for the Upper Colorado River Basin. At the same time, many states, including Utah, are developing their own statewide water development plans as well.

Utah's water planning program

Developing and conserving water resources in Utah began with small projects and moved in logical sequence to larger, more complex and costly ones as time passed. One of the first acts of the Mormon pioneers upon entering the Great Salt Lake Valley in 1847 was to divert water to irrigate the parched soil. From that time on people have built dams, canals, ditches, and pipelines to establish an irrigated agriculture and provide water for their towns, cities, and industries.

Recognizing the need for a comprehensive planning approach to achieve the best development of the state's scarce water resources, the Utah Water and Power Board in 1961 undertook a cooperative study with Utah State University. This study, which was preliminary to the preparation of a comprehensive statewide water development plan, was initiated for the purpose of showing why increased water planning was essential. It took a searching look at the problems and needs on a statewide basis, and its report outlined the general water use-water supply picture, what the major problems were, and what challenges would have to be faced in overcoming the problems. This preliminary report was published in 1963, and the Legislature in that same year appropriated special funds for the preparation of a state water plan.

Since 1963 emphasis in the planning program has been given to acquiring basic information and data for appraising available resources and potential requirements. Data have been obtained from all available sources, including local universities and several state and federal agencies. Several papers and statistical reports have been published by the Division of Water Resources (formerly the Utah Water and Power Board), the agency primarily responsible for preparing the plan.

An Interim Report on the State Water Plan, outlining progress to date in the planning program and indicating things that remain to be accomplished to complete the plan, was released in May of 1970. An updated version of the same report is in preparation for publication in 1972. The intent of publication and distribution of this report is to obtain public reaction to and discussion of the planning accomplished to date and problems remaining to be solved. After a period of public meetings and discussions on this interim report, the planning staff will complete separate appendixes for each of the 10 hydrologic study regions of the state. Each appendix will contain specific projections of water demand, inventories of supplies available to meet these demands, and alternative plans of development. Eventually a State Water Plan, culminating from these efforts, will be recommended to the Utah Legislature for approval.

Policy and institutional situation

As water development has proceeded over the years, numerous institutions have been established with concern for various aspects of water administration. At least 25 units in five departments and three major independent agencies of the federal government have significant responsibilities related to water resources. In Utah State government, there are 11 agencies engaged directly or indirectly in water activity, and in addition, there are 13 water conservancy districts, three water improvement districts, six metropolitan water districts, more than 1000 mutual irrigation companies, and unnumbered individual communities involved within the state (Bagley, 1969).

Prior to major reorganization of Utah State government in 1967, the primary functions of water resources administration were assigned to three independent agencies—the Office of the State Engineer, the Utah Water and Power Board, and the Water Pollution Control Board. The Office of State Engineer established in 1903, has been responsible for the general administration and regulation of the waters of the state, including measurement, appropriation, apportionment, and distribution. The Utah Water and Power Board, created in 1947 with the establishment of a statewide water development and conservation program, was responsible for administering a water resources development fund established at that same time and for water planning and development activities of the state. In 1963, it was given specific responsibility for preparing a statewide water development plan. The Water Pollution Control Board, the most recently established of these three water agencies, was organized in 1953 to develop programs for prevention, control, and abatement of water pollution. The board has been responsible for classifying the waters of the state and setting quality standards along with maintaining a surveillance and regulatory program for preserving water quality.

Although a major reorganization of state government in 1967 changed the names and composition of these agencies, it did not alter significantly their major functions. The Utah Water and Power Board became the Board of Water Resources retaining essentially the same powers, and its staff was redesignated as the Division of Water Resources in the newly formed Department of Natural Resources. The statewide water planning is continuing under the board and the division.

Federal agency activities related to water development in Utah are numerous and diverse. All of the federal water agencies are involved to a degree, but some have much larger roles than others. Since the Reclamation Act of 1902, the Bureau of Reclamation has built several major dams and other water projects affecting the state. The Central Utah Project presently under construction will have great impact on Utah State water problems. The bureau has also cooperated with the state in investigations of water availability, requirements, and development possibilities in other areas of the state.

Agencies of the Department of Agriculture, including the Soil Conservation Service, the Forest Service, and the Economic Research Service, are also involved in various water development activities in Utah. Aside from its small watershed projects and other activities, the Soil Conservation Service has cooperated with the Bureau of Reclamation, Utah State University, and the Board of Water Resources in a statewide land-capability survey. Along with the Forest Service and Economic Research Service, it has cooperated with the state in studies of the Sevier River, Beaver River, and Escalante Desert areas.

The Corps of Engineers has a much smaller role in constructing water projects in Utah than the Bureau of Reclamation, but nevertheless, has been studying flood problems in the state for many years. Several projects which have been planned and authorized have not been constructed because of the lack of local cooperation (financing). The state is not authorized under the present laws to participate directly with the Corps in flood control projects. The providing of lands, easements and right-of-way required for such projects, therefore, is left to the counties or local entities, who have been unable or unwilling in many cases to raise the funds needed.

The U.S. Geological Survey has had a substantial role in the collection of basic data useful in the state water planning program. Extensive data gathering networks have been set up, and much information on the quantity and quality of both surface water and groundwater has been acquired.

The colonization of Utah by the Mormon pioneers involved the establishment of many small communities, usually separated by miles of desert or mountain ranges and therefore largely self supporting. The major activities in these communities, including the management of irrigation water supplies, were carried out cooperatively. Out of these early cooperative efforts evolved the typical Utah mutual irrigation company, the dominant form of irrigation organization in the state. At the other end of the spectrum at the local level is the highly organized and powerful conservancy district, which encompasses several smaller entities, such as mutual companies, irrigation districts, partnerships, individuals, etc. Conservancy districts are created under state law and have extensive powers, including limited taxing authority. The Central Utah Conservancy District, formed by several counties to contract with the federal government for construction of the Central Utah Project, is an example of this form of local organization.

The state's primary water development function with the various local water organizations has been that of providing financial and technical assistance in the construction of small water projects (primarily for irrigation). The Board of Water Resources provides financial assistance through its revolving development fund, and the Division of Water Resources provides technical assistance.

In addition to the growth over the years in number of organizations in the state concerned with water, a substantial body of law and regulations has accumulated which sets bounds to the way water may be developed and used. The influence of political boundaries, statutes, decrees, administrative rules and regulations, court decisions, ordinances, etc., greatly affects planning and development.

The appropriation doctrine of water rights is recognized in Utah, and, in general, the water of Utah streams is fully covered by applications to appropriate. Neverthe-

less, large quantities of water continue to flow out of the state or into the Great Salt Lake without being fully utilized. In a similar way, large quantities of groundwater remain in storage, while many groundwater basins are overflowing. Although water planning by the state has not overlooked the significance of water rights, planning studies have not been constrained by such rights. Planning has been directed toward a means of protecting existing uses while satisfying new and increasing demands. Some questions of water rights will have to be resolved when plans are implemented (Utah Division of Water Resources, 1970).

In an act approved on August 19, 1921, by the United States Congress, the States of Arizona, Nevada, New Mexico, Utah, and Wyoming entered into a compact to provide an equitable division and apportionment of the waters of the Colorado River System. The compact, known as the Colorado River Compact, basically divided the waters of the Colorado River between the upper and lower basins. This compact gave each basin the right to the exclusive beneficial consumptive use of 7,500,000 acre feet of water annually and provided that in cases of deficiencies the shortages would be allocated to each basin in equal proportions.

In a later act passed on April 6, 1949, the states of the upper basin (Arizona, Colorado, New Mexico, Utah, and Wyoming) joined together in a compact known as the Upper Colorado River Basin Compact to further divide and apportion their share of Colorado River water. This compact divided the 7,500,000 acre feet given to the upper basin as follows: 50,000 acre feet to Arizona, and of the remaining quantity 51.75 percent to the State of Colorado, 11.25 percent to the State of New Mexico, 23.00 percent to the State of Utah, and 14.00 percent to the State of Wyoming. Although this allocation would give Utah 1,714,000 acre feet per year, flow in the river generally has been less than the 15 million acre feet that was divided by the compact and Utah's potential share has been estimated at between 1,277,000 and 1,714,000 acre feet per year (Tipton and Kalmbach, 1965). An amount of 1,438,000 acre feet per year will be used in these present studies as Utah's share of the Colorado River water.

Systems approach

Fundamentally, water resources development entails the modification of a natural hydrologic system to meet man's needs. Regardless of the modifications made to certain parts of the system, the equilibrium of the system is changed and other components or elements are affected. Consequently, one of the main questions raised in connection with any water development scheme is: What will be the effect on existing uses? The interrelationships among elements of the hydrologic system, though varied and complex, are relatively simple in comparison with the social, legal, economic, and institutional interdependencies involved. These relationships, economic and

social as well as physical, are so close and so strong as to require that planning of water development be accomplished on a systems basis. In fact, the general move toward comprehensive water resources planning is founded on a recognition that these close system relationships require unified treatment.

The major program now underway to formulate comprehensive water development plans for the entire nation comes at a time when fundamental changes are taking place in the pattern and composition of water uses and in water technology. Although methodology has not been devised which can consider all of the variables and parameters involved, describe their interaction in space and time, and arrive at a simultaneous solution to the whole matrix, advances in the social and physical sciences and in technology have made available a number of new and improved decision-making techniques for application to water resources planning. Operations research and systems analysis intimately associated with advances in computer technology are particularly useful.

In the application of systems analysis to water resources planning, the first step is to define the system to be analyzed. In water planning this means the identification of objectives along with associated boundary conditions or constraints. These are then transformed into optimal plans for development. In general, water resources planning is a technique of public investment decision-making. The decisions relate to the allocation of scarce resources among competing claims. To choose among alternative courses of action, a set of objectives must be specified and a decision rule developed for use as a guide to optimal design—the design or set of alternatives that best meets the objectives. Expressed more formally, the decision problem is to maximize the value of an objective function, subject to limitations imposed from outside the system and further limited by the production function imposed by nature and the state of technology (Hufschmidt, 1965).

Objectives of the Research

The development and allocation of water for the state calls for a long sequence of crucial decisions. Water planners are faced with the problem of identifying optimal development plans in order to best utilize scarce water resources. Research is needed on the interrelationships and impacts of water projects. There is no easy way to foresee which of the many possible alternatives will do most to enhance the well-being of the people of the state. Piecemeal and uncoordinated planning for water resources development is inappropriate. Long-term plans are needed which incorporate a broad overview of the state.

The general objective of this research has been to extend the capability for mathematical analysis of complex water resource systems to a statewide system. The research was designed to develop a method to enhance the quality of decisions to be finalized in the State Water Plan. A particular problem is the optimal allocation of Utah's share of Colorado River water from an engineering economic standpoint. A considerable amount of Colorado River water and other available water is not presently being used. While the research began with an emphasis on undeveloped Colorado River water, it was soon recognized that all the available water resources must be included in the analysis. In this research a number of alternative patterns and levels of demand for water are postulated for the ten study areas. The costs of meeting the projected demands were then minimized by solving a linear programming model of the economic-hydrologic-physical system. The optimal cost-minimizing system consists of the various combinations of groundwater, surface water, and interregional transfer activities which minimize the cost.

Elements of specific projects were evaluated in this research as well as other general relationships for which no project feasibility or authorizing documents are available.

Specifically, the objectives of the research were:

1. To formulate a mathematical programming model consisting of an objective function with the appropriate constraints for allocation of water within the ten hydrologic subdivisions of the state, including transfer of water between hydrologic subdivisions.

The objective function, expressed as $c_1x_1 + c_2x_2 \dots + c_nx_n$, describes the economic inputs (costs) associated with each of the alternative allocations.

In the mathematical programming model the various allocation alternatives and the hydrologic characteristics of the system can be expressed as system "constraints" which in matrix form are as follows:

$$\begin{aligned}
 a_{11}x_1 + a_{12}x_2 + \dots + a_{1n}x_n &\leq b_1 \\
 a_{21}x_1 + a_{22}x_2 + \dots + a_{2n}x_n &\leq b_2 \\
 \dots & \\
 \dots & \\
 a_{m1}x_1 + a_{m2}x_2 + \dots + a_{mn}x_n &\leq b_m
 \end{aligned}$$

In this matrix the b_i values are quantities of various resources which place limits on the system. The coefficients $a_{11}, a_{12} \dots a_{mn}$ are the input requirements of the alternative allocations of the scarce resources. The columns contain all coefficients for each alternative allocation, and the rows contain all of the coefficients for each resource. The inequalities indicate that no more of a resource may be used than is available, but some of it may go unused.

Physical features and quantities as well as the limitations imposed by the Colorado and other river compacts have been included in the system constraints.

Cooperative effort from the State of Utah Division of Water Resources assisted in deriving the appropriate objective function, constraints, and the alternative demand projections which enter into the model.

2. To solve the mathematical model using an appropriate optimizing algorithm to determine the optimal allocation of Colorado River water and other surface and ground waters in the State of Utah with least cost as the measure of effectiveness in the objective function.

The problem is to minimize the objective function subject to the constraints. While there is no general algorithm, or systematic method of solution, for solving the general mathematical programming problem, the basic relations in this model lend themselves to a linear formulation, so the simplex algorithm was employed as the optimum seeking method. The quality of the data presently available did not justify more than linear approximations of the nonlinear relationships.

3. To optimize the allocation of Colorado River entitlement and other waters in Utah under various arbitrary operating rules and water use policies to determine the economic effects of some of the common legal, political, and social limitations.

Social policies and political limitations are often not readily formulated analytically as system constraints. In order to evaluate the effects on an economic objective function and optimal allocations which are caused by operating rules imposed on the system by social and political policies, various operating rules which reflected these effects were imposed on the model. The operational results of the imposed operating rules were examined and in this way imputed costs of such decisions were defined.

4. To evaluate the usefulness of the analytical approach for state water planning and determine its usefulness for future application to other planning areas.

The results of the first three objectives were evaluated to determine the usefulness of such an analytical approach in large scale water resources planning and to point out the advantages and disadvantages of the methodology.

Review of the State-of-the-Art: The Systems Approach to Water Resource Planning

In recent years systems analysis has become increasingly useful as a tool in water resources planning, design and development, operating procedures, and management.

According to Drobney (1968, p. 534) systems analysis is:

... A strategy for problem solving which relies heavily on mathematical modeling to assess the technical and economic optimality of alternative systems designs, policies, operating procedures, etc., for performing various functions and meeting various needs with limited resources. It is important to keep in mind that systems analysis *per se* does not provide these assessments which also must incorporate professional, legal, political, and social consideration. Rather systems analysis may be employed as a decision aid in assessing the technical and economic consequence of alternative courses of action.

A mathematical model is defined as a set of equations which describe some physical, biological, or chemical process and can be classified by three methods; (1) performance versus optimization models; (2) deterministic versus stochastic models, and (3) analytical versus simulation models. Drobney (1968) further distinguishes between the usefulness of the various models and states the type of problems which might be solved by each model. The optimization model using analytical definitions of the function to be optimized and based on deterministic technology has proven to be most useful for water resource planning (James and Lee, 1971, Maass et al., 1962).

A mathematical programming problem occurs when one seeks to maximize or minimize an analytical function (called an objective function) of one or more variables subject to certain relationships involving the variables (called constraints). (See Intriligator, 1971.) Under certain limited conditions, a solution to this problem can be found using classical differential calculus, including Lagrangian multipliers and the calculus of variations. The complex engineering and economic aspects of today's water resource problems are far beyond the computational adequacy of the classical methods and have motivated a keen interest in programming models (Drobney, 1968). Several programming models have been developed and computational algorithms exist for some of their solutions. These are linear programming (Hadley, 1962), non-linear programming (Hadley, 1964) including quadratic programming and geometric programming (Duffin, Peterson, and Zener, 1967), and dynamic programming (Hadley, 1964).

Linear programming is one of the most widely used of all systems analysis techniques. A statement of this problem might be:

Given a set of m linear inequalities or equations in r variables ($r \geq m$), non-negative values of these variables are sought which will satisfy the constraints and maximize or minimize some linear function of the variables. (Hadley, 1962)

Many applications have been made of the linear programming model to solve problems in water resources. Some of these are:

- (1) Least costly plan for waste treatment (Loucks, Revelle, and Lynn, 1967; Johnson, 1967; Rogers and Gemmel, 1966; Sobel, 1965; Thomann, 1965)
- (2) Optimum operation of large dams considering benefits from hydropower and irrigation (Thomas and Nevelle, 1966)
- (3) Sewage treatment plant design (Lynn, Logan, and Charnes, 1962)
- (4) Conjunctive use of surface water and groundwater (Milligan, 1969)

Non-linear programming is similar to linear programming except the objective function and constraints are not required to be linear functions of the decision variables (Hadley, 1964). One form of this non-linear problem for which numerical computation techniques have been developed is known as quadratic programming in which the objective function has quadratic terms subject to linear constraints. Quadratic programming was used by Lynn (1966) to determine a least-cost pumping schedule for wells. A more general, and consequently harder to solve, form of non-linearity occurs with an objective function that is non-linear to a higher degree than quadratic. This form is known as geometric programming (Duffin, Peterson, and Zener, 1967). Geometric programming is just in its infancy in water resources use but has been used successfully in other applications (Beightler, Crisp, and Meier, 1968; and Wilde and Beightler, 1967).

A tool that has been used quite successfully to solve sequential decision problems is dynamic programming. According to Drobney (1968, p. 543):

A sequential decision problem is a problem in which a sequence of decisions (termed a policy) must be made and in which each decision affects future decisions. . . . unlike linear programming, there exists no standard mathematical model format according to which a problem may be structured for solution by dynamic programming. Rather dynamic programming is an approach oriented technique, and the particular equations to be used must be developed to fit the problems at hand.

Examples of its use are: (1) design and operation of multi-reservoir systems (Amir, 1967; Buras, 1965; Meier and Beightler, 1967; and Schweig and Cole, 1968), (2) optimization of individual multi-purpose reservoirs (Hall, 1964; and Hall, Butcher, and Esogbue, 1968), (3) minimization of overall cost of waste treatment among

discharges (Liebman and Lynn, 1966), (4) optimal use of groundwater over time (Burt, 1964), and (5) optimization of conjunctive use of groundwater and surface water (Aron, 1969). A combination of dynamic programming with linear programming has been used to study the problem of optimal future operation of a water resource system with random streamflows (Shailendra and Shepard, 1967).

Systems analysis approach in other states

Several studies have been done in other states utilizing operations research techniques to attack regional and statewide water planning problems.

Susquehanna River Basin - New York and Pennsylvania

Howes (1966) used linear programming to develop an interregional model which specifies economically feasible water resource investments. The model enabled simultaneous estimates of the benefits resulting from a project and market prices. The model generated a spatial economic equilibrium solution. Optimal solutions were generated for ranges of production costs and resource rents and values of agricultural commodities. The dual of the linear programming problem was developed to determine marginal values of water in agriculture. Demand functions for water were then generated. These data allowed a determination of the impacts of water development upon resource owners.

River basin - Iowa

Baldwin (1970) used linear programming to construct a model of a river basin and determine optimum water use pattern and value of water. Iowa's water permit system was a major constraint. Benefits were estimated for several major water users and combined with costs to give a net benefit objective function.

Trans-Texas Division, Texas Water System - Texas

Orlob (1970) discussed the approach taken by planners for the Texas Water System. The Trans-Texas Division of the Texas Water System would be comprised of 18 reservoirs, more than 500 miles of canals, and pumping facilities to raise the water from near sea level to over 3000 feet elevation. The planning problem is:

Given:

1. Location of all reservoirs
2. Routes of connecting canals
3. Schedules of in-basin demand for each reservoir or major junction in the system
4. Hydrology of supply for each major storage element
5. Cost of imported water, and
6. Costs of construction and O & M for all elements

Find:

The least costly alternative system and schedule for its construction to meet specified demands to the year 2020 within the prescribed legal, financial, contractual, and political constraints.

The approach was to seek “near optimum” solutions rather than exact optima to overcome limits on time and computer capability. The procedure was carried out in four phases:

1. Preliminary sizes of elements and operating rules for reservoirs were determined by a formal optimization procedure.
2. Initial screening was performed by simulation of the given hydrology, element sizes, and operating rules for each of a large number of alternative stage development schedules selected by random sampling of the cost “response surface.” The most attractive schedules were improved by a method of successive perturbations.
3. Element sizes were refined by a second simulation procedure which constrained flows in some expensive canals.
4. Final screening was performed by a formal optimization of the most attractive systems and development schedules.

Entire state - Texas

McKee (1966) developed a linear programming model for determining least cost of agricultural production for the entire State of Texas. Account was made of soil classification, acreage required per unit of production, and cost of production per unit in each soil class. Constraints were the acreage in each soil class and the demand for each crop. Cost data included the cost of supplying water for each soil class and each crop. Cost of drainage was also included. On-farm production costs were estimated. Requirements for crop production were projected to year 1975 and the production allocation determined by the linear programming algorithm. Marginal costs were derived for each of the crops.

Pecos River Basin - New Mexico

Gisser (1970) applied the method of parametric linear programming to forecast the demand for imported irrigation water in the future. The objective function was net return to land and management. Acreage and salinity constraints were incorporated with a water application at unit increments from 0 to 4 acre-feet per acre.

Sacramento Basin - California

Hall et al. (1967) discuss the development of analytical techniques for optimization of water resource systems. The study area included four major streams, ten reservoirs, and the associated pumping plants, aqueducts,

and power generation facilities. The objective maximized is financial feasibility based on deliveries of firm energy, firm water, off-peak energy, and off season water. The procedure decomposes the complete system by a “master wholesaler”–“individual producer” relationship. The individual reservoir operators used dynamic programming to optimize their returns based on a schedule of prices provided by the master and report the corresponding outputs over the study period. The master, using these outputs as “available resources” maximizes the actual returns he could obtain from water and power contracts using linear programming. A new set of prices is generated which reflect the value of a modified output schedule for the operators. The cycle of calculations is repeated until the improvement is negligible.

Santa Clara Valley - California

Aron (1969) developed a conceptual model of a regional water conservation and distribution system under conjunctive use of surface water and groundwater, and a set of procedures for establishing water allocation and import policies of maximum economic efficiency. Dynamic programming was chosen as the primary optimizing technique because of its flexibility of application. In particular the sequence of operations necessary to arrive at an optimal operating policy made dynamic programming the best choice of mathematical tools. Limitations on the number of state variables are noted with the suggestion that simulation may be the only practical tool for developing an efficient water allocation policy in a complex, multisource, multipurpose system.

San Joaquin Valley - California

Moore (1962) estimated a demand schedule for irrigation water in a highly commercialized farm area by constructing linear programming models to represent five farms of different size with maximization of farm income as the objective function. Cost of irrigation water was varied with the result that new combinations of crops became optimum making it possible to trace quantity used versus price. In addition, the temporal distribution of water was studied by shifting the run-off pattern to successively later times and determining the net increase in farm income, thus estimating value of storage.

Statewide - California

Lofting and McGauhey (1968) used input-output programming analysis in continuing study on the economic evaluation of water on a statewide basis. Earlier Lofting and McGauhey (1963) had presented an input-output table as a first step in establishing the procedure for developing guidelines for a statewide water resources policy. In their later work these authors up-dated the model from 1947 economic data to 1958 data. Linear programming was used as an optimizing technique to identify the time path of shadow prices of water for 24 productive water dependent sectors of the California

economy. A time series Gross State Product was developed for 1940 to 1966 in 1958 constant dollars and growth projections were made to the year 1990. Ranges of final demands for the model were set and solutions obtained so as to maximize value added given different levels of fresh water availability.

Previous studies for Utah

Bradley and Gander (1968) performed an input-output analysis on the economics of water allocation in the state. A 40 sector model was employed. Direct and indirect water coefficients were estimated and water use was projected to 1975. This study is discussed by Bradley, Short, and Kolb (1970) and shows how the model is used in projecting relevant economic parameters to 1975 and 1985.

Gold, Milligan, and Clyde (1969) published an interim report for the project reported herein. The report presented the development of the mathematical model and essentially met the first objective of the study. The model and the procedures and resultant data which were generated to meet the remaining objectives are the subjects of this report.

This study advances the state-of-the-art in several ways: A *statewide* water resources planning model structured in the linear programming format is developed and applied to the State of Utah; methodology is suggested for using the model to determine the optimal allocation of water resources of the state to meet projected demands at minimum cost; methodology is suggested for bringing political and social factors, operating rules and policies and other peripheral considerations into the decision process through the model.

GENERAL BACKGROUND FOR THE PHYSICAL SYSTEM AND THE ALLOCATION MODEL

In this section a description is given of the detailed physical system to be represented by the allocation model. Such features as the general area covered and its breakdown into convenient hydrologic study units, the population and economic growth, the land areas and associated water uses and water requirements, the available water resources, the major water resource problems, the present status of water resource development, the estimated storage requirements and storage potential, and the groundwater recharge potential are each discussed as the necessary basis of the mathematical model which is described later.

Water Resources Requirements, Availability, and Problems in Utah

The area

Located in the arid southwest, Utah is one of the driest states in the nation, and in general is considered an area of chronic water shortage. A closer look at the pattern of valleys and high mountain ranges, however, reveals sharply contrasting differences of climatic conditions within the state. Although some of the valleys receive a scant 4 to 5 inches of precipitation annually, nearby mountains may receive 60 inches or more. Wide cyclic and geographical variations of precipitation added to erratic seasonal distribution makes the development and efficient utilization of the total water resources very difficult.

The state lies in three major drainage basins. Most of the 84,916 square mile area of the state is divided between the Colorado River and Great Basins with only a very small portion in the Columbia River basin.

In terms of physiography, the state may again be divided three ways. The Great Basin, lying in the western half, is an interior drainage basin with no outlets. Streams emanating from the high Wasatch Mountains on its eastern perimeter discharge into valley fills and lakes. The Great Salt Lake is located in the northern part of the basin, and much of the remaining area is desert. In the south and east, the land—most of it part of the Upper Colorado River Basin—is in the form of high plateaus. The area is characterized by a highly dissected land surface with deep, steep-walled canyons. The Rocky Mountains constitute the third physiographic region of the state. The Wasatch Range, in a line generally running north and south through the central portion of the state, divides the Great

Basin portion of the state to the west from the Colorado River drainage in the east. This Wasatch Range together with the Uinta Range, running generally east and west in the northeastern part of the state, are areas of high precipitation and, consequently, the primary sources of runoff.

Hydrologic study units

The appropriate geographic unit for water resource planning and development is the river basin or a closely related group of basins which drain to a common point. Within such a hydrologic complex the visible and invisible water supplies are connected and continuous.

Within each of the two major drainage basins, many streams and stream systems make up smaller hydrologic areas which lend themselves to analysis as individual units. As a practical matter, determination of available water supplies and their quality; extent and nature of uses and requirements; estimated future needs; considerations of water management administration, and adjudication; assembly and analysis of planning data, as well as the planning itself, must be done according to such river basins or hydrologic entities.

A geographic division of the state that is acceptable to most state and federal agencies involved in water resources activities is presented in Figure 1. The proposed division consists of 11 hydrologic basins which can be grouped in various ways to correspond to the larger divisions and numbering established by various Federal Inter-Agency Groups or to the three major river basins. Referring to Figure 1 the numbers are assigned as follows:

<u>Hydrologic Study Unit (HSU)</u>	<u>Area Explanation</u>
0	Columbia River
1	Great Salt Lake Desert
2	Bear River
3	Weber River
4	Jordan River
5	Sevier River
6	Cedar-Beaver
7	Uintah Basin
8	West Colorado
9	South and East Colorado
10	Lower Colorado

The Columbia River Basin portion of the state is not included in this study.

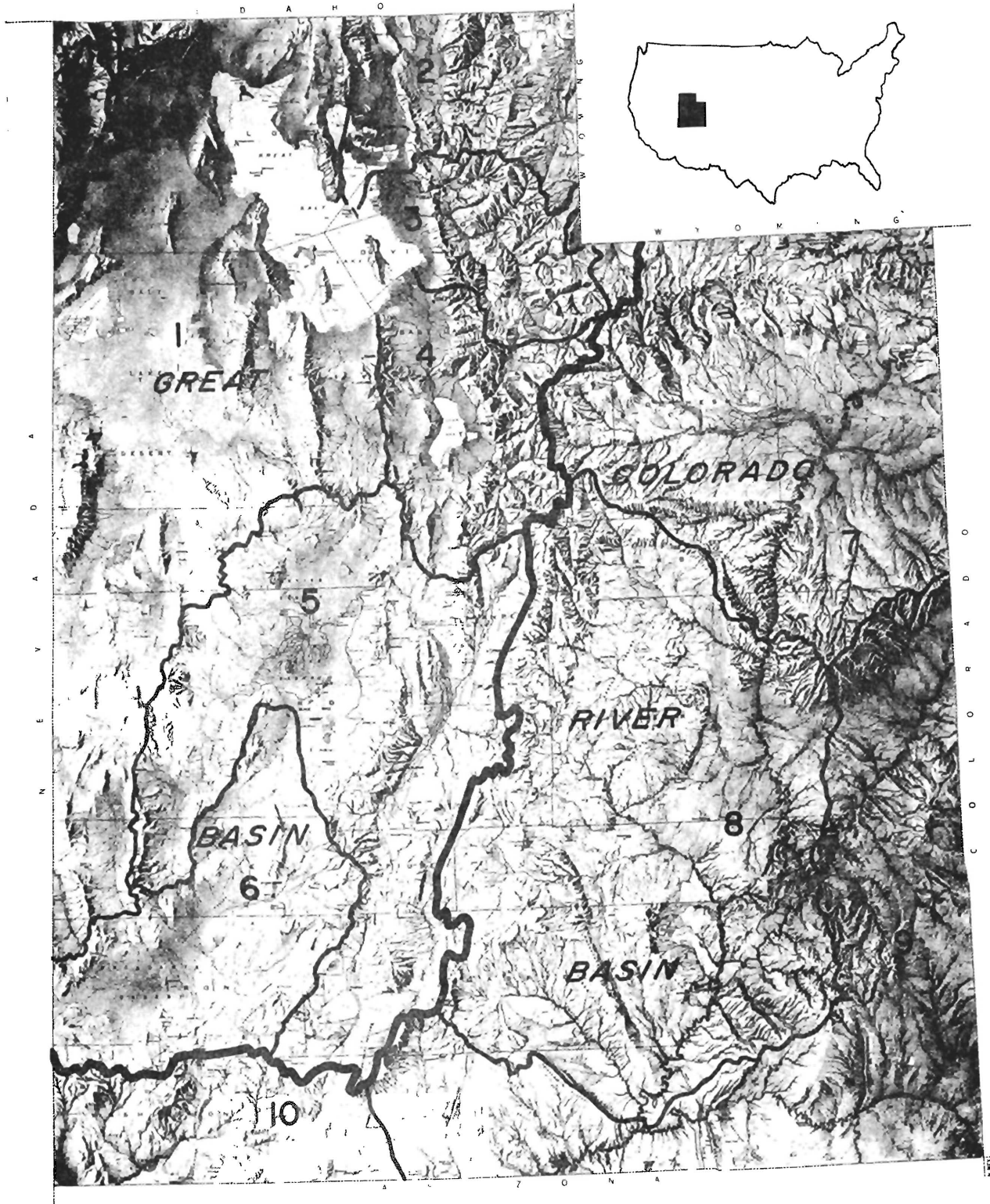


Figure 1. Hydrologic study units of Utah.

Population and economic growth

Utah's economy has a rather diverse base including as major segments agriculture, mining, manufacturing, construction, utilities, trades and services, and government (Nelson and Harline, 1964). Percentages of total personal income from these sources for Utah and the nation are compared in Table 1.

There have been some significant shifts in employment between segments of Utah's economy, and increases in population, labor force, and employment have been greater in recent years than national averages. From 1940 to 1964, Utah's increases were 81 percent for population, 100 percent for labor force, and 130 percent for employment as compared with national increases of 45 percent, 50 percent, and 60 percent respectively. For particulars about economic growth and shifts in employment patterns during this period see Cluff (1964).

The population of Utah, estimated by the U.S. Bureau of Census to be 997,000 in 1965 (U.S. Bureau of Census, 1966), is expected to continue to grow at a relatively high rate. In the future, average growth in the Great Basin region, encompassing western Utah and most of Nevada, will probably be at 2.5 percent annually according to one estimate (U.S. Water Resources Council, 1968). The eastern areas of the state are expected to show growth at a somewhat lower rate.

The greatest economic development and concentration of population in the state occurs in the Provo-Salt Lake City-Ogden-Logan area, a relatively small area on the eastern edge of the Great Basin. Increased concentration of population and economic growth projected for this Wasatch Front area in the future indicates a continuing shift of development toward urban, commercial, and industrial activities.

Water uses and projected requirements

As shown by Table 2 approximately 92.3 percent of the total precipitation over the state is used in grazing lands and watersheds, wastelands, national parks and monuments, water area (primarily Great Salt Lake), and outflow in interstate streams. It is the remaining 7.7 percent that this research project is concerned with since it is within the immediate control capability of man and can be considered as an available resource. This water appears in two forms: 1) surface runoff in rivers and streams originating in the watershed areas, and 2) groundwater in alluvial reservoirs which originated from percolation of precipitation and water bodies on the ground surface and from groundwater interflow from the watershed areas.

Man's use of his available water resource falls into three primary categories: 1) agriculture, 2) municipal and industrial, and 3) recreation and maintenance of natural

Table 1. Percentage of total income from various sources.^a

Basic Physical Production	Percentage of total income ^b	
	Utah	Continental U.S.
Agriculture	3.0	4.4
Mining	4.8	1.2
Manufacturing	19.7	29.2
Utilities and transportation	8.3	7.4
Contract construction	8.8	6.4
Subtotal production	44.6	48.6
Wholesale and retail trade	19.7	19.1
Finance and insurance	4.3	5.2
Service	10.2	13.5
Government	21.1	13.2
Other Miscellaneous	0.1	0.4
Subtotal service	55.4	51.4
TOTAL	100.0	100.0

^aSource: (Nelson and Harline, 1964).

^bTotal personal income (millions of dollars): Utah \$2,083; the nation \$461,610. This does not include transfer payments, unemployment insurance, welfare, etc.

Table 2. Land use and water consumed in Utah (McGuiness, 1963).

Type of Land	Percent Total Area	Percent Water Consumed
Grazing land and watersheds	81.7	72.1
Arable but uncropped land used for grazing	2.6	1.9
Dry-farmed land	1.1	1.0
Irrigated land	2.1	4.6
Cities and towns, industrial sites	.5	.2
Wasteland, national parks, and monuments	9.0	6.4
Water area	<u>3.0</u>	<u>9.5</u>
	100.0	95.7
Outflow to interstate streams		<u>4.3</u>
		100.0

vegetation and wildlife. Water appearing in rivers and streams is diverted by man through canals and other irrigation works to flood croplands during the dry months of the year. In those areas where local surface water is not available in sufficient supply, pumps are installed to utilize the groundwater. Excess water not used by the crops either runs off as surface water back to the streams or percolates into the groundwater reservoir for use again. Likewise surface and groundwater resources are diverted by man through municipal and industrial systems. The sewage and other excess water can be treated before being returned to the sources. Water for recreation and maintenance of natural vegetation and wildlife primarily appears as part of the water storage and conveyance systems. Some water used by non-beneficial phreatophytes could be made available for other use by proper management of wetlands.

Beginning with the settlement of the Mormon pioneers in the middle 1800's, irrigation has been one of the major uses of water in Utah. In fact, the practice of irrigation by pioneers in the Great Basin is held to be the first on an extensive scale by Anglo-Saxons in the United States.

Because of water scarcity and the development of needs other than irrigation, the annual amount diverted for irrigation has not increased greatly in recent years. This has occurred in spite of the fact that a considerable acreage of arable land remains undeveloped. The withdrawal uses estimated by the U.S. Geological Survey between 1950 and 1965 reflect only a 14 percent overall increase for this 15-year period (U.S. Geological Survey, 1951, 1968). Total arable land in the state has been estimated at approximately 5 million acres of which only about 1½ million are irrigated (Utah State University, 1968). The breakdown of arable and irrigated lands by hydrologic study unit is presented in Table 3.

In the foreseeable future, irrigation will undoubtedly maintain its position as the largest water user in the state despite a trend for rural areas in general not to keep pace economically with urban areas. While additional water alone will not reverse present trends, more water for supplemental irrigation and new irrigation in established agricultural communities will assist in establishing a more viable economy in rural areas. Water will be needed to eliminate present irrigation shortages and to bring new lands into cultivation as demands for agricultural products increase in the future.

Some other major water uses will probably increase faster than irrigation. In the Provo-Salt Lake City-Ogden-Logan area of relatively high population growth, demands for industrial and municipal water supplies will increase rapidly. Other areas of the state showing little urban growth in the past could experience such growth in the future as government policies designed to alleviate pressing problems of the cities may encourage development of sparsely populated regions. Water supplies will be needed to enable and facilitate this growth. Population and municipal-industrial water use by hydrologic region in 1965 are shown in Table 4.

With greater emphasis being placed on environmental and recreational goals by society, demands for water related to these goals will increase throughout the state. Managed water fowl areas, for example, will require supplemental water supplies and additional supplies for expansion.

Available resources

There are four basic sources of water that may be more fully developed to provide for future requirements in Utah (Haycock, 1968):

1. Water resources along the Wasatch Front including Bear River.

2. The Virgin River and minor streams draining into the lower Colorado River.
3. Groundwater basins within the state.
4. Upper Colorado River water belonging to Utah.

Streams within the state have been measured or gaged extensively, and surface-water availability is well defined.

Although there already has been considerable groundwater development in Utah, extensive groundwater supplies remain available. Water availability by hydrologic area is presented in Table 5.

One of the state's greatest sources of undeveloped water is in the Upper Colorado River Basin separated from the most significant population growth areas by the

Wasatch Mountains. Because of this separation of present growth areas from potential supply, much of Utah's share of the Colorado River water currently flows out of the state unused. Even with the transfer of a sizeable amount of Upper Colorado River Basin water to the Great Basin by the Central Utah Project, a large scale project of the U.S. Bureau of Reclamation, some of Utah's share of this water may still be unused (Haycock, 1968). Other projects or expansion of current projects will be required to fully utilize this supply.

Several other means by which available supplies can probably be increased include: control of phreatophytes and evaporation, saline water conversion, waste water reclamation and reuse, and better watershed management. Weather modification and importation schemes also may eventually provide additional supplies.

Table 3. Land use and water use in the hydrologic study units.

Hydrologic Study Unit	Arable Land Acres	Irrigated Land Acres	Water Consumed ac-ft/yr
1	1,483,200	52,000	59,000
2	445,400	246,000	354,000
3	194,100	166,700	236,000
4	448,400	207,200	310,000
5	1,022,200	293,000	436,000*
6	838,300	71,800	137,000
7	340,700	195,000	293,000
8	206,200	98,100	114,000
9	531,300	16,000	30,000
10	89,000	17,500	34,000
Total	5,598,800	1,363,300	2,003,000

* Includes 105,000 ac-ft direct groundwater use.

Source: Utah State University, 1968.

Table 4. Population and municipal and industrial demand.

Hydrologic Study Unit	Population	Municipal and Industrial Water Use ac-ft/yr
1	23,000	3,000
2	70,000	15,000
3	215,000	28,000
4	567,000	94,000
5	33,000	9,000
6	16,000	4,000
7	20,000	4,000
8	26,000	5,000
9	16,000	5,000
10	12,000	1,000
Total	997,000	168,000

Source: Utah Division of Water Resources, 1970.

Table 5. Available water resources in Utah.

Hydrologic Study Unit	Groundwater ac-ft/yr	Water Availability	
		Local Surface Water ac-ft/yr	Local Surface Water Plus Groundwater ac-ft/yr
1	187,000	613,000	800,000
2	138,000	917,000	1,055,000
3	65,000	660,000	725,000
4	394,000	560,000	954,000
5	356,000	417,000	773,000
6	130,000	80,000	210,000
7	40,000	1,319,000	1,359,000*
8	---	650,000	650,000*
9	---	430,000	430,000*
10	10,000	250,000	260,000*
Total	1,320,000	5,896,000	7,216,000

* Much of this water considered as available for transfer.
 Source: Utah Division of Water Resources, 1970, and the U.S. Geological Survey.

Major water and related land resources problems

Utah, generally considered an area of chronic water shortage, has access to only partial supplies for nearly two-thirds of its irrigated land. Yet, it has over 2 million acres of swamp land, marshes, mud flats, and valley bottoms suffering from an excess of water. In addition, water evaporation from reservoirs and lakes, as well as transpiration by phreatophytes, amounts to far more than is withdrawn for public supplies. Herein lies the challenge for water planning and management in Utah (Utah Water and Power Board-Utah State University, 1963).

In spite of the fact that there are more than 3 million acres of land in Utah that could be added to agricultural production if water were available, and industrial and urban areas in the state need water to sustain growth, a substantial share of Utah's portion of Colorado River water continues to flow out of the state unused.

Maximum development of Utah's vast groundwater reservoirs will require changes or at least more realistic interpretations of present state statutes in harmony with natural hydrologic laws. In the past, well owners have commonly held the view that their rights involve a guarantee by the state to maintain given water pressures or water table levels in wells. Such control, though not physically possible, would limit the use of groundwater to a fraction of the amount available in storage. Recent court decisions indicate that some improvement in this condition is imminent.

Despite the large sums of money invested in municipal and agricultural waterworks in Utah, much

remains to be done. Worn out and obsolete control and conveyance works must be replaced, new water projects must be constructed to meet growing demands, and some legal and institutional changes must be implemented. Problems of water quality are intimately interwoven with other development problems, and will require careful consideration. In general, in spite of aridity, Utah's major concern in water development is not in deficiency of total supply, but in the maldistribution of water resources seasonally and geographically. The challenge is to store, transport, treat, and distribute the available water in an optimal manner.

Present Status of Water Resource Development

A summary of the status of water resource development in the State of Utah is shown in Table 6. Explanation and reference information are given in the following paragraphs.

- a. Basin Yield—These data are the same as shown previously in Table 5.
- b. Net Evaporation Loss—Large Lakes—These data show the loss of water as a result of evaporation from Bear Lake in Hydrologic Study Unit (HSU) 2 and from Utah Lake in HSU 4. Account was taken of the precipitation on the lake surface to calculate the net loss. Since about one-half of the surface area of Bear Lake is in Idaho, only one-half the net evaporation loss was charged to Utah. Water budget studies were used to determine the loss which was assumed to be equally divided between surface and groundwater.
- c. Net Evaporation Loss—Other Major Reservoirs—These data were determined as discussed in b except that in HSU 5 the loss was

- distributed 75 percent to surface water and 25 percent to groundwater and in HSU 7 and 8 where no groundwater is available.
- d. Storage Capacity—The storage capacity data were taken from several sources:
 1. An early report on the state water plan published March 1963, PR-EC4Bg-20,
 2. Investigations by the Utah Division of Water Resources (DWR), and
 3. Investigations by the Pacific South-West Inter-agency Committee, Water Resources Council.
 - e. Direct Use of Groundwater by Croplands—These data were only calculated in the water budget for the Sevier Basin. It was included there as a reduction in the available groundwater to make the data compatible in all study units.
 - f. Excess Precipitation on Irrigated Croplands, October-April—These data were determined from the water budget for HSU 2, 3, 4, 5, and 7. The values represent the amount of precipitation which is in excess of the amount consumptively used by the crops. This represents an addition to the water supply since it would appear as runoff into the streams or an addition to groundwater.
 - g. Transbasin Diversions—These data were obtained from two sources:
 1. Water budgets for HSU 2, 3, 4, 5, and 7, and
 2. Utah Division of Water Resources Interim Report published March 1970.
 - h. Gross Supply—These data are the summation of: Basin Yield; Net Evaporation Loss Large Lakes; Net Evaporation Loss Other Major Reservoirs; Direct Use of Groundwater by Croplands; Excess Precipitation on Irrigated Croplands, October-April; and Net Imported Water from Transbasin Diversions.
 - i. In-Basin Water Availability—These data are the summation of: Basin Yield; Net Evaporation Loss Large Lakes; Direct Use of Groundwater by Croplands; and Excess Precipitation on Irrigated Croplands, October-April.
 - j. Diversions—The total diversions to agriculture and to municipal and industrial for HSU 2, 3, 4, 5, and 7 were taken from the water budget studies. Total diversions to the other five units were based primarily on data from Utah DWR except where modified to account for Utah Water Research Laboratory (UWRL) studies on the return flow coefficient for agriculture and to approximate the return flow coefficient indicated for the year 2020. This latter modification was made since the LP model must hold the coefficient constant over time. Groundwater pumpage was determined by using the average figure from 1964-1968 given by DWR-USGS in yearly reports on “Ground Water Conditions in Utah.” Surface water diversions were obtained by subtraction.
 - k. Return Flows—The return flows for HSU 2, 3, 4, 5, and 7 were obtained from the water budget studies. Agriculture return flow for HSU 1, 6, 8, and 10 were based on Utah DWR data while for HSU 9 was based on UWRL studies. Municipal and industrial return flows for HSU 1 and 6 were based on Utah DWR data whereas for HSU 8, 9, and 10 were based on approximations to the expected return flow coefficients projected by Utah DWR for the year 2020.
 - l. Depletions Other Than Reservoir Evaporation—Depletions for HSU 2, 3, 4, 5, and 7 were based on the UWRL water budget studies while for HSU 1, 6, 8, 9, and 10 were based on Utah DWR data. The division between surface water and groundwater was determined using individual budgets for each knowing the groundwater outflow. It is recognized that much of the water in the upper areas of the river basins which is below ground may rise to the surface in the lower areas and be consumed by wetlands, etc. This fact is reflected by the large depletions of groundwater by wetlands.
 - m. Outflow from HSU—The groundwater outflow to Great Salt Lake from HSU 1, 2, 3, and 4 was estimated using the results of several studies conducted on this subject by UWRL and others. HSU 5 and 6 have groundwater mining which is shown by negative outflow. Groundwater outflow for HSU 7 was obtained from the water budget study. Surface water outflow was determined by balancing water availability, depletions, and groundwater outflow.

Storage Requirements

Storage requirements, including amounts needed to regulate seasonal fluctuations in stream flow as well as to provide the long-term carryover needed to meet extended series of dry years, were estimated for each of the 10 hydrologic study areas. The required storage for a given water requirement depends on the magnitude and frequency distribution of the streamflow.

Estimates of long-term carryover storage requirements are based upon the results of frequency mass-curve analyses completed for 76 streams located throughout the state and published in the Hydrologic Atlas of Utah (Utah State University—Utah Department of Natural Resources, 1968). A frequency mass-curve is obtained by plotting, for any selected probability of occurrence, the expected values of accumulated volumes of runoff during each of many sequences of consecutive months (through several

years) against the carryover period in months. Separate frequency mass-curves are obtained for each probability of occurrence selected.

Since the volume of required storage can be considered a function of probability, carryover period, and the water demand level, the frequency mass-curve analysis provides information necessary for plotting demand vs. storage curves. A computer program developed to carry out the large amount of computation involved (Jeppson, 1967) was used to analyze monthly runoff data and provide the information necessary to compute draft vs. storage curves for the 76 streams considered in the Hydrologic Atlas. Draft was expressed in percent of mean annual flow for values of 50, 65, 80, 95, and 110 percent. Storage was given in inches over the watershed. Probability values of 75, 90, and 95 percent were used.

The long-term storage requirement corresponded to the maximum values of storage as a function of the carryover period. These values were determined for each of the streams at each of the five draft values and three probability levels. The total long-term storage for each of the hydrologic study areas was then determined by weighting each stream's watershed area to the total watershed area.

The seasonal storage was determined for each hydrologic study area by calculating the difference between the annual supply curve on a monthly basis and the draft requirement for each of the five draft values. Where water budgets were available (areas 2, 3, 4, 5, and 7) the draft curves were based on these data. Where water budgets were not available, the draft curves were based on calculations using Munson's Index (Munson, 1966). The supply curve was based on monthly stream flow data from the Hydrologic Atlas weighted for the watershed area as before.

The seasonal storage was added to the long-term storage to determine the total storage required for HSU 2 through 10. Insufficient stream flow data were available for HSU 1. Figures 3 through 11 show the storage required vs. draft at probability levels of 75, 80, 85, 90, and 95 percent where the intermediate values were obtained by cross plots. The curves for HSU 1 shown on Figure 2 were obtained from Figures 12 and 13 which are a summary of HSU 2 through 10 in non-dimensional form. An average value for HSU 2, 3, and 5 was used to determine storage requirements for HSU 1 at a probability of 75 percent while an average value for HSU 2 through 6 was used to determine storage requirements at a probability of 95 percent.

The use of these storage-draft curves can be illustrated by the following example using Figure 5.

Assume it is desired to know how much storage would be required in the Jordan River study unit (HSU 4) to meet a total draft in the area equal to 80 percent of the

mean annual flow or 450,000 ac-ft/yr. From Figure 5 the required storage is seen to be 460,000 ac-ft at the 95 percent probability level. The interpretation of the probability level is that approximately 95 percent of the time one would expect to be able to provide the draft of 450,000 ac-ft/yr by building 460,000 ac-ft of storage. Both long-term holdover storage and annual storage requirements would be provided.

Groundwater Recharge Potential

The groundwater recharge potential or opportunity was assessed in each study unit in order to define the recharge constraint. The problem was to designate the areas where artificial recharge to the groundwater basin is practicable, provided the water table is low enough to permit recharge, and to estimate for each area the amount of water that could be put underground in basins and/or through wells.

In HSU 2, 3, and 4 the reservoirs are essentially alluvial fans intercalated with and overlapped by lake-bottom sediments of Pleistocene Lake Bonneville. The aquifers in these fans are sheets or trains of stream gravel that spread outward from the canyon mouths and thin and decrease in particle size toward the valley bottom. Recharge to these reservoirs is largely at the apex of the alluvial fans, where the stream gravel is coarse, and where lake bottom sediments, deposited over the fan during high stages of the lake, have been stripped away by the stream after the lake lowered. These recharge areas are surrounded, valleyward, by the most productive parts of the artesian basins, where pressures, yields and water quality are best. The areas near the apexes of the fans, where a recharge basin would not be perched on lake-bottom sediments, are small, and their position can be judged only partly by the present surface layer of coarse stream alluvium. In any case, it is a limited area very near the mouth of the canyon from which the fan material came.

In practically all cases the fans are at present full or nearly full of water, and a program of artificial recharge would depend upon lowering of the water table in the fans so that additional recharge could be accommodated.

Based on results of the few artificial recharge experiments that have been conducted in Utah, and experience elsewhere, a possible recharge rate of 2 feet per day for 300 days of the year was selected.

The favorable position for recharge wells would also have to be high on the alluvial fan where the aquifers are relatively thick and coarse-grained. Based on experience in Utah and elsewhere, a value of 2500 gallons per minute per well was selected as a reasonable estimate, with the wells spaced one to a quarter section.

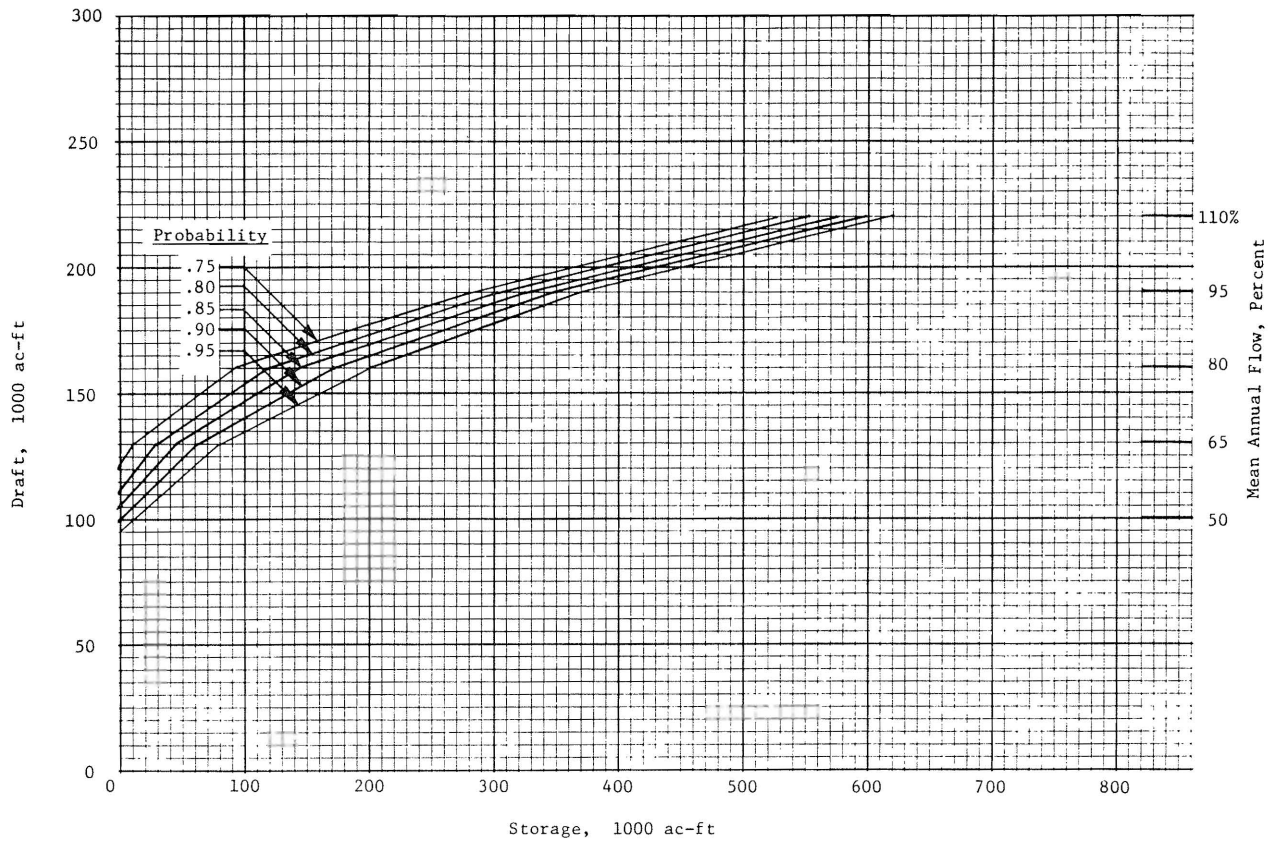


Figure 2. Reservoir storage requirement for the Great Salt Lake Desert hydrologic study unit.

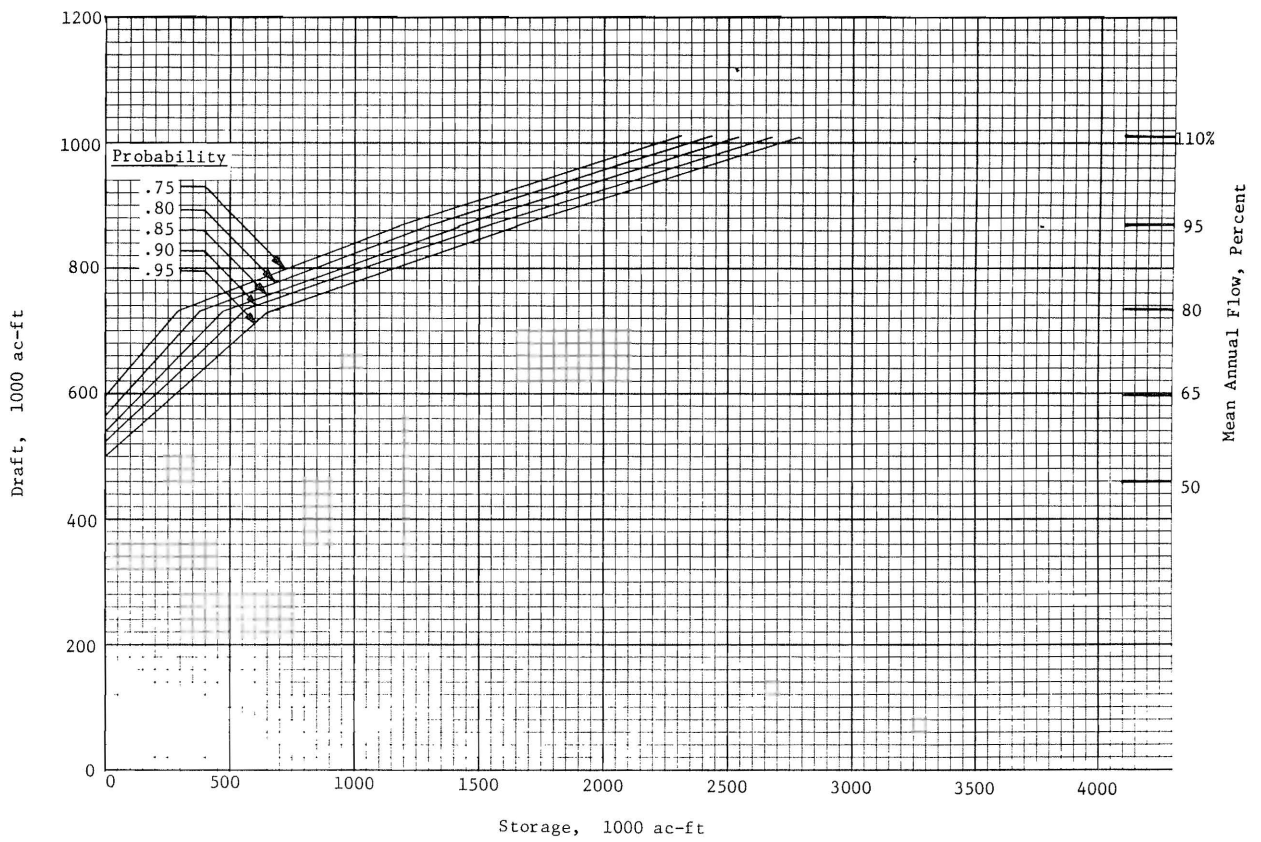


Figure 3. Reservoir storage requirement for the Bear River hydrologic study unit.

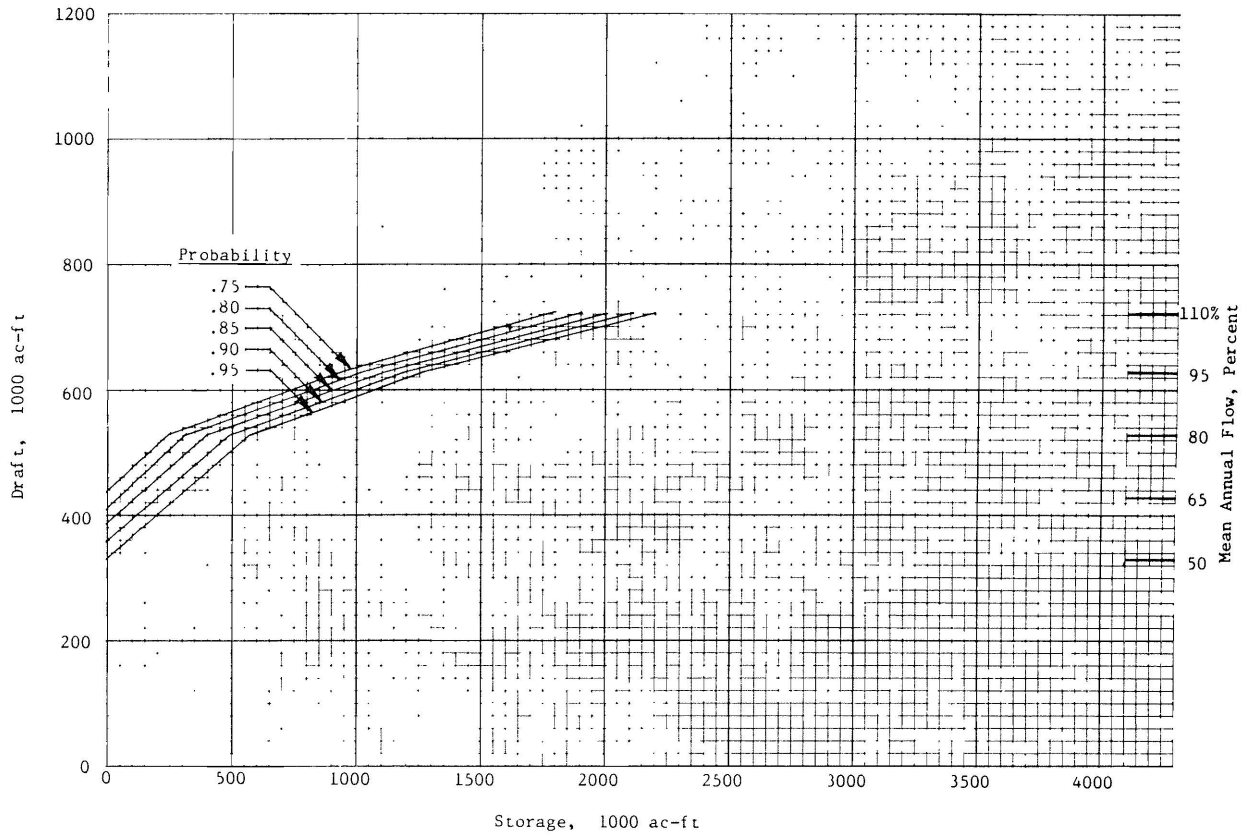


Figure 4. Reservoir storage requirement for the Weber River hydrologic study unit.

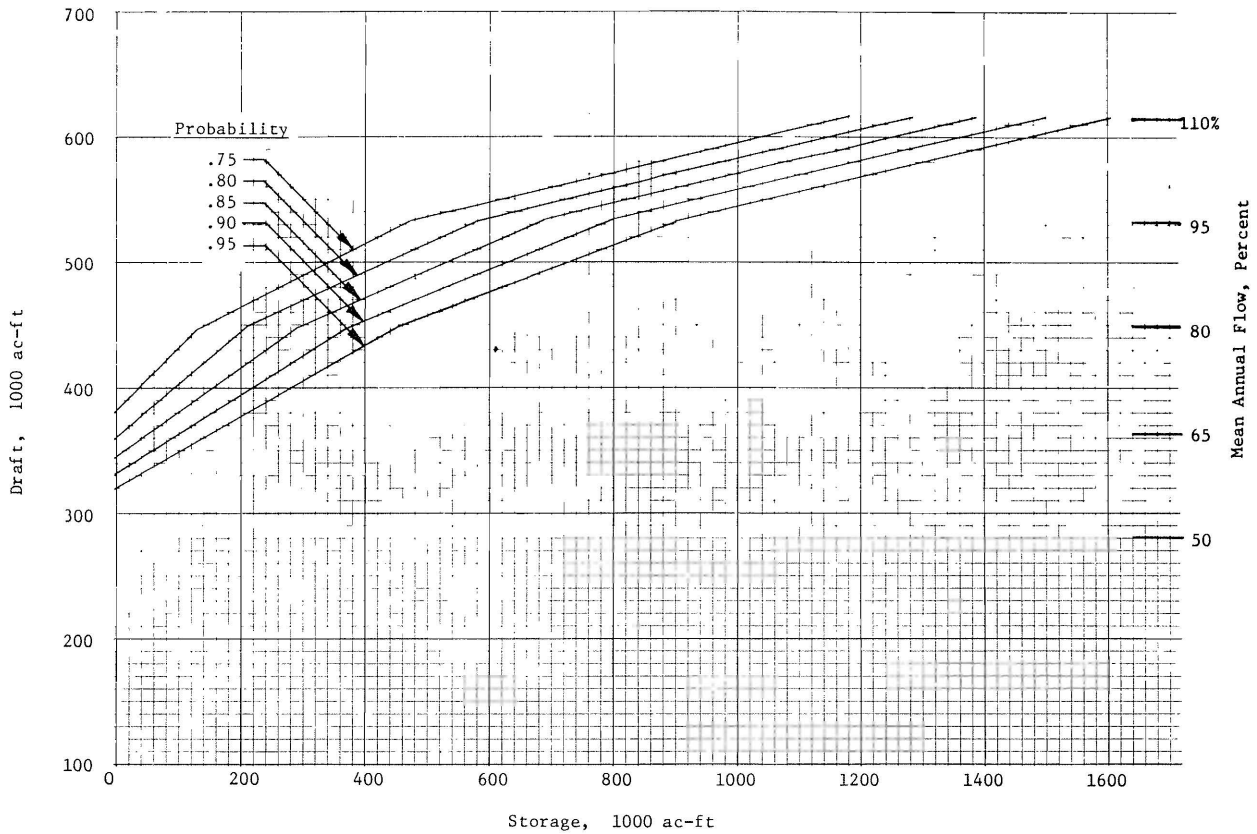


Figure 5. Reservoir storage requirement for the Jordan River hydrologic study unit.

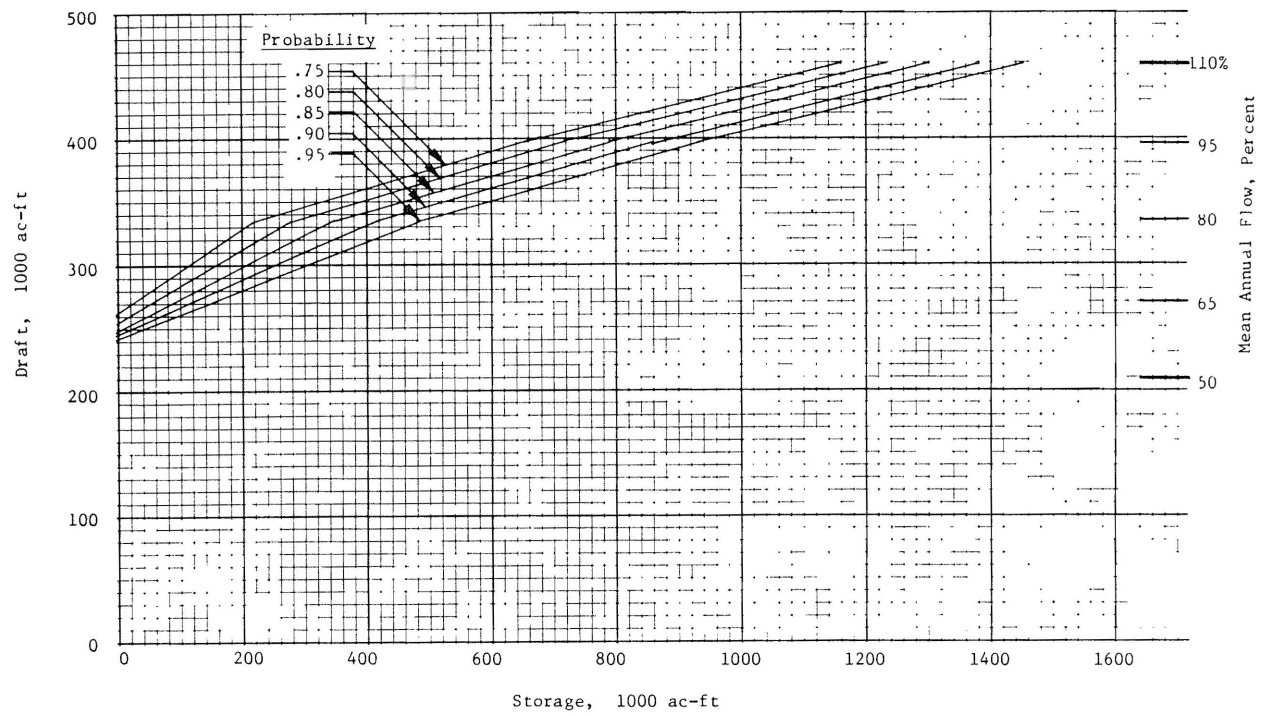


Figure 6. Reservoir storage requirement for the Sevier River hydrologic study unit.

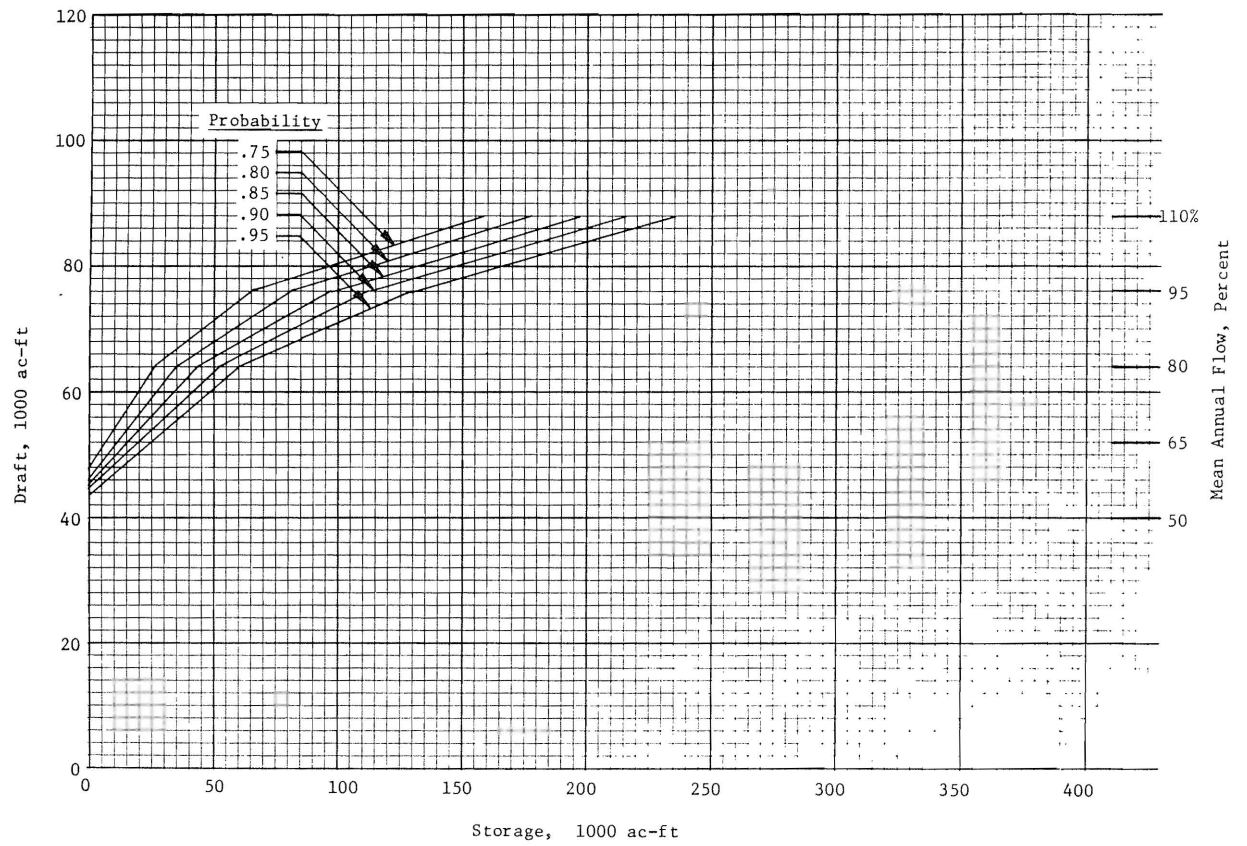


Figure 7. Reservoir storage requirement for the Cedar-Beaver hydrologic study unit.

The limit on Colorado River outflow was established as follows:

HSU	Present (1965) Depletions (ac-ft/yr)			Total Basin Yield
	Man Caused	Total	Difference	(ac-ft/yr)
7	468,300	692,100	223,800	1,359,000
8	156,000	174,500	18,500	650,000
9	39,400	43,100	3,700	430,000
			<u>246,000</u>	<u>2,439,000</u>
Total Yield Upper Basin				2,439,000
Realistic Allocation to Utah			1,438,000	
Net Mainstem Evaporation			152,000	
Net Allocation to Meet Demands			1,286,000	
Additional Water Allocation Due to Definition that Only Man Caused Depletions are Chargeable Against the Allocation			246,000	
Total Allocation				<u>1,532,000</u>
Water that Must be Released as Colorado River Outflow				<u>907,000</u>

A complete matrix of the model is shown on Figure 31. As indicated, the letters of the alphabet show the order of magnitude of the coefficients. There is one objective function, 204 constraints, and 338 variables in the model.

Variable bounds

Bounds have been established on several groups of variables in the model. These groups are: 1) inter-basin

Table 9. Variable bounds on new inter-basin transfers.

Variable	Bound (ac-ft/yr)	Type of Bound
QBULSW5	29,000	Upper
QBUMP5	136,600	Upper
QUILSW3	20,000	Upper
QUILSW5	57,000	Upper
QUIMPT	420,000	Upper
QSALSW4	15,000	Upper
QSAMPT	22,400	Upper
QLSW2SW1	90,000	Upper
QLSW2SW3	130,000	Upper
QLSW3SW4	146,000	Upper
QLSW4SW5	69,000	Upper
QLSW5SW6	60,000	Upper
QLSW0SW6	47,000	Upper

transfer, 2) additional surface water storage, and 3) surface water and groundwater outflow from each of the hydrologic study units. In addition, an upper bound of unity was placed on each of the dummy variables as part of the separable programming algorithm.

Inter-basin transfer

Bounds on presently existing inter-basin transfers were established primarily from the water budget studies. Average values to represent approximate 1965 conditions were used in the model. Bounds on new development were taken from the DWR Interim Report of 1970 and from consultation with Bureau of Reclamation personnel associated with the Central Utah Project. New development bounds are shown on Table 9.

Additional surface water storage

These bounds were established from data of the USGS, the Utah DWR, the Pacific Southwest Inter-Agency Committee, and from studies conducted at UWRL. These results are shown on Table 10.

Surface and groundwater outflow

These bounds were established from a consideration of minimum river flow to achieve a salt balance, studies conducted by the USGS on groundwater outflow, and on studies made at UWRL. The bounds are shown on Table 11.

<u>Constraint Name</u>	<u>Constraint</u>		<u>Explanations and Comments</u>
GWRC1	1.0 QLSW1R1 + 1.0 QWW1R1	≤ 0.0	These inequalities show the constraint on groundwater recharge. The RHS was estimated from geologic and hydrologic considerations discussed earlier in this report.
GWRC2	1.0 QLSW2R2 + 1.0 QWW2R2	≤ 60.0	
GWRC3	1.0 QLSW3R3 + 1.0 QWW3R3	≤ 366.0	
GWRC4	1.0 QLSW4R4 + 1.0 QWW4R4	≤ 434.0	
GWRCU4	1.0 QLSW4RU4 + 1.0 QWW4RU4	≤ 100.0	
GWRC5	1.0 QLSW5R5 + 1.0 QWW5R5	≤ 52.0	
GWRCU5	1.0 QLSW5RU5 + 1.0 QWW5RU5	≤ 52.0	
GWRC6	1.0 QLSW6R6 + 1.0 QWW6R6	≤ 65.0	
GWRC7	1.0 QLSW7R7 + 1.0 QWW7R7	≤ 0.0	
GWRC0	1.0 QLSWOR0 + 1.0 QWWOR0	≤ 0.0	

Figure 28. Constraints for groundwater artificial recharge limits.

<u>Constraint Name</u>	<u>Constraint</u>		<u>Explanation and Comments</u>
BUMPT	1.0 QBULSW4 + 1.25 QBULSW5 - 1.0 QBUMPT	= 0	These equations calculate the total water imported to the Great Basin from each of the three sources in the CUP. The 1.25 coefficient accounts for transport losses.
UIMPT	1.0 QUILSW3 + 1.0 QUILSW4 + 1.25 QUILSW5 - 1.0 QUIMPT	= 0	
SAMPT	1.0 QSALSW4 + 1.0 QSALSW5 - 1.0 QSAMPT	= 0	
TLSW3SW4	1.0 PLSW3SW4 + 1.0 QLSW3SW4 - 1.0 RLSW3SW4	= 0	These equations show the constraint on inter-basin transfer in those basins presently having some transfer.
TLSWOSW6	1.0 PLSWOSW6 + 1.0 QLSWOSW6 - 1.0 RLSWOSW6	= 0	

Figure 29. Constraints for inter-basin transfer limits.

<u>Constraint Name</u>	<u>Constraint</u>		<u>Explanation and Comments</u>
INFLOGSL	1.0 QLSW1OF1 + 1.0 QLSW2OF2 + 1.0 QLSW3OF3 + 1.0 QLSW4OF4 + 1.0 QGW1OF1 + 1.0 QGW2OF2 + 1.0 QGW3OF3 + 1.0 QGW4OF4	≥ 201.0	This inequality shows the constraint on total inflow to the Great Salt Lake. The RHS will change depending upon the ground rules for the particular run being made. The number 201.0 is simply the sum of the individual minimum inflows.
CROUT	1.0 QLSW7OF7 + 1.0 QLSW8OF8 + 1.0 QLSW9OF9 + 1.0 QGW7OF7	≥ 907.0	

Figure 30. Constraints for inflow and outflow limits.

<u>Constraint Name</u>	<u>Constraint</u>	<u>Explanation and Comments</u>
AGRFSW1	.4000 RLSW1AG1 + .4000 RGW1AG1 - 1.0 QAR1LSW1 = 0	These equations calculate the amount of agriculture return flow that goes to local surface water. The non-unity coefficient is called the return flow coefficient to surface water.
AGRFSW2	.5077 RLSW2AG2 + .5077 RGW2AG2 + .5077 PLSW3AG2 - 1.0 QAR2LSW2 = 0	
AGRFSW3	.4833 RLSW3AG3 + .4833 RGW3AG3 - 1.0 QAR3LSW3 = 0	
AGRFSW4	.4609 RLSW4AG4 + .4609 RGW4AG4 - 1.0 QAR4LSW4 = 0	
AGRFSW5	.5250 RLSW5AG5 + .5250 RGS5AG5 + .5250 PLSW8AG5 - 1.0 QAR5LSW5 = 0	
AGRFSW6	.4000 RLSW6AG6 + .4000 RGW6AG6 - 1.0 QAR6LSW6 = 0	
AGRFSW7	.4788 RLSW7AG7 + .4788 RGW7AG7 - 1.0 QAR7LSW7 = 0	
AGRFSW8	.6250 RLSW8AG8 - 1.0 QAR8LSW8 = 0	
AGRFSW9	.8000 RLSW9AG9 + .8000 PLSW5AG9 - 1.0 QAR9LSW9 = 0	
AGRFSW0	.5000 RLSW0AG0 + .5000 RGW0AG0 - 1.0 QAR0LSW0 = 0	
AGRFGW1	.1242 RLSW1AG1 + .1242 RGW1AG1 - 1.0 QAR1GW1 = 0	These equations calculate the amount of agriculture return flow that goes to groundwater. The non-unity coefficient is called the return flow coefficient to groundwater.
AGRFGW2	.1500 RLSW2AG2 + .1500 RGW2AG2 + .1500 PLSW3AG2 - 1.0 QAR2GW2 = 0	
AGRFGW3	.1500 RLSW3AG3 + .1500 RGW3AG3 - .10 QAR3GW3 = 0	
AGRFGW4	.1500 RLSW4AG4 + .1500 RGW4AG4 - 1.0 QAR4GW4 = 0	
AGRFGW5	.1500 RLSW5AG5 + .1500 RGW5AG5 + .1500 PLSW8AG5 - 1.0 QAR5GW5 = 0	
AGRFGW6	.1447 RLSW6AG6 + .1447 RGW6AG6 - 1.0 QAR6GW6 = 0	
AGRFGW7	.1500 RLSW7AG7 + .1500 RGW7AG7 - 1.0 QAR7GW7 = 0	
AGRFGW0	0.0 RLSW0AG0 + 0.0 RGW0AG0 - 1.0 QAR0GW0 = 0	

Figure 26. Constraints for return flow from agricultural use.

<u>Constraint Name</u>	<u>Constraint</u>	<u>Explanations and Comments</u>
FGWAVWL1	1.0 QFGW1WL1 - 0.50 QAR1GW1 = 166.8	These equations calculate the amount of groundwater that is used from natural sources by wetlands. These sources are; 1) the groundwater that returns to the surface in the wetlands by natural conditions and 2) the groundwater which is available for wetland consumption which had as its source the agriculture return flow to the groundwater.
FGWAVWL2	1.0 QFGW2WL2 - 0.50 QAR2GW2 = 147.5	
FGWAVWL3	1.0 QFGW3WL3 - 0.50 QAR3GW3 = 51.8	
FGWAVWL4	1.0 QFGW4WL4 - 0.50 QAR4GW4 = 96.0	
FGWAVWL5	1.0 QFGW5WL5 - 0.50 QAR5GW5 = 209.1	
FGWAVWL6	1.0 QFGW6WL6 - 0.50 QAR6GW6 = 42.5	
FGWAVWL7	1.0 QFGW7WL7 - 0.50 QAR7GW7 = 59.2	
FGWAVWL0	1.0 QFGW0WL0 = 10.0	The coefficient of 0.50 for the return flow and the RHS were estimated using present conditions based on water budgets and accounting for groundwater outflow.

Figure 27. Constraints for free groundwater for wetlands.

<u>Constraint Name</u>	<u>Constraint</u>	<u>Explanations and Comments</u>	
EVLSW1	0.070 RLSW1ST1 - 1.0 QLSW1EV1 = 0	These equations calculate the amount of evaporation loss from the major reservoirs (except Bear and Utah lakes) as function of the reservoir storage. In HSU 2 and 4 the evaporation loss-storage relationship is highly non-linear and is calculated using the separable programming algorithm of MPS 360.	
EVLSW2	0.50 QEV2 - 1.0 QLSW2EV2 = 0		
EVLSW3	0.023 RLSW3ST3 - 1.0 QLSW3EV3 = 0		
EVLSW4	0.50 QEV4 - 1.0 QLSW4EV4 = 0		
EVLSW5	0.093 RLSW5ST5 - 1.0 QLSW5EV5 = 0		
EVLSW6	0.0525 RLSW6ST6 - 1.0 QLSW6EV6 = 0		
EVLSW7	0.028 RLSW7ST7 - 1.0 QLSW7EV7 = 0		
EVLSW8	0.045 RLSW8ST8 - 1.0 QLSW8EV8 = 0		
EVLSW9	0.070 RLSW9ST9 - 1.0 QLSW9EV9 = 0		
EVLSW0	0.070 RLSW0ST0 - 1.0 QLSW0EV0 = 0		
EVGW1	0.0 RLSW1ST1 - 1.0 QGW1EV1 = 0		
EVGW2	0.5 QEV2 - 1.0 QGW2EV2 = 0		
EVGW3	0.023 RLSW3ST3 - 1.0 QGW3EV3 = 0		
EVGW4	0.5 QEV4 - 1.0 QGW4EV4 = 0		
EVGW5	0.031 RLSW5ST5 - 1.0 QGW5EV5 = 0		
EVGW6	0.0175 RLSW6ST6 - 1.0 QGW6EV6 = 0		
EVGW7	0.0 RLSW7ST7 - 1.0 QGW7EV7 = 0		
EVGW0	0.0 RLSW0ST0 - 1.0 QGW0EV0 = 0		
EV2ST2	208.0 E21 + 103.0 E22 + 1500.0 E23 - 1.0 RLSW2ST2 = 0		These equations calculate the amount of evaporation loss as function of storage in HSU 2 and 4.
EV2	0.0 E21 + 3.0 E22 + 105.0 E23 - 1.0 QEV2 = 0		
EV4ST4	220.0 E41 + 196.0 E42 + 1500.0 E43 - 1.0 RLSW4ST4 = 0		
EV4	0.0 E41 + 25.5 E42 + 105.0 E43 - 1.0 QEV4 = 0		

Figure 24. Constraints for net evaporation loss from Reservoirs (other than Bear and Utah Lakes).

<u>Constraint Name</u>	<u>Constraint</u>	<u>Explanations and Comments</u>
WWRF1	.7000 RLSW1MI1 + .7000 RGW1MI1 - 1.0 QWW1LSW1 - 1.0 QWW1R1 = 0	These equations calculate the amount of waste water return flow from municipal and industrial uses. The return flow can go either to local surface water or ground water depending upon economics and need. The non-unity coefficients are called the return flow coefficients.
WWRF2	.6600 RLSW2MI2 + .6600 RGW2MI2 - 1.0 QWW2LSW2 - 1.0 QWW2R2 = 0	
WWRF3	.4366 RLSW3MI3 + .4366 RGW3MI3 - 1.0 QWW3LSW3 - 1.0 QWW3R3 = 0	
WWRF4	.6889 RLSW4MI4 + .6889 RGW4MI4 + .6889 RLSW1MI4 - 1.0 QWW4LSW4 - 1.0 QWW4RU4 = 0	
WWRF5	.4588 RLSW5MI5 + .4588 RGW5MI5 - 1.0 QWW5LSW5 - 1.0 QWW5R5 = 0	
WWRF6	.6970 RLSW6MI6 + .6970 RGW6MI6 - 1.0 QWW6LSW6 - 1.0 QWW6R6 = 0	
WWRF7	.6500 RLSW7MI7 + .6500 RGW7MI7 - 1.0 QWW7LSW7 - 1.0 QWW7R7 = 0	
WWRF8	.3000 RLSW8MI8 - 1.0 QWW8LSW8 = 0	
WWRF9	.2500 RLSW9MI9 - 1.0 QWW9LSW9 = 0	
WWRF0	.3000 RLSW0MI0 + .3000 RGW0MI0 - 1.0 QWW0LSW0 - 1.0 QWW0R0 = 0	

Figure 25. Constraints for waste water return flow from municipal and industrial use.

Constraint Name	Constraint	Explanation and Comments
GRID1	123. D11 + 7. D12 + 30. D13 + 30. D14 - 1.0 QDREQ1 = 0	These equations calculate the amount of storage required as function of the draft required. The draft-storage relationship is highly non-linear and these equations represent the approximation for the separable programming algorithm in the MPS 360.
LSW1ST1	0. D11 + 10. D12 + 80. D13 + 190. D14 - 1.0 RLSW1ST1 = 0	
GRID2	596. D21 + 138. D22 + 137. D23 + 138. D24 - 1.0 QDREQ2 = 0	
LSW2ST2	0. D21 + 300. D22 + 880. D23 + 1140. D24 - 1.0 RLSW2ST2 = 0	
GRID3	435. D31 + 93. D32 + 99. D33 + 99. D34 - 1.0 QDREQ3 = 0	
LSW3ST3	0. D31 + 240. D32 + 690. D33 + 870. D34 - 1.0 RLSW3ST3 = 0	
GRID4	382. D41 + 66. D42 + 84. D43 + 84. D44 - 1.0 QDREQ4 = 0	
LSW4ST4	0. D41 + 130. D42 + 340. D43 + 710. D44 - 1.0 RLSW4ST4 = 0	
GRID5	262. D51 + 71. D52 + 63. D53 + 62. D54 - 1.0 QDREQ5 = 0	
LSW5ST5	0. D51 + 220. D52 + 430. D53 + 510. D54 - 1.0 RLSW5ST5 = 0	
GRID6	48. D61 + 16. D62 + 12. D63 + 12. D64 - 1.0 QDREQ6 = 0	
LSW6ST6	0. D61 + 26. D62 + 38. D63 + 94. D64 - 1.0 RLSW6ST6 = 0	
GRID7	870. D71 + 185. D72 + 198. D73 + 198. D74 - 1.0 QDREQ7 = 0	
LSW7ST7	0. D71 + 320. D72 + 600. D73 + 1280. D74 - 1.0 RLSW7ST7 = 0	
GRID8	394. D81 + 126. D82 + 98. D83 + 97. D84 - 1.0 QDREQ8 = 0	
LSW8ST8	0. D81 + 200. D82 + 300. D83 + 710. D84 - 1.0 RLSW8ST8 = 0	
GRID9	272. D91 + 72. D92 + 65. D93 + 64. D94 - 1.0 QDREQ9 = 0	
LSW9ST9	0. D91 + 120. D92 + 150. D93 + 280. D94 - 1.0 RLSW9ST9 = 0	
GRID0	160. D01 + 40. D02 + 38. D03 + 37. D04 - 1.0 QDREQ0 = 0	
LSW0ST0	0. D01 + 75. D02 + 100. D03 + 285. D04 - 1.0 RLSW0ST0 = 0	
TST1	1.0 PLSW1ST1 + 1.0 QLSW1ST1 - 1.0 RLSW1ST1 = 0	These equations sum the present developed storage and new development of storage to get the total storage.
TST2	1.0 PLSW2ST2 + 1.0 QLSW2ST2 - 1.0 RLSW2ST2 = 0	
TST3	1.0 PLSW3ST3 + 1.0 QLSW3ST3 - 1.0 RLSW3ST3 = 0	
TST4	1.0 PLSW4ST4 + 1.0 QLSW4ST4 - 1.0 RLSW4ST4 = 0	
TST5	1.0 PLSW5ST5 + 1.0 QLSW5ST5 - 1.0 RLSW5ST5 = 0	
TST6	1.0 PLSW6ST6 + 1.0 QLSW6ST6 - 1.0 RLSW6ST6 = 0	
TST7	1.0 PLSW7ST7 + 1.0 QLSW7ST7 - 1.0 RLSW7ST7 = 0	
TST8	1.0 PLSW8ST8 + 1.0 QLSW8ST8 - 1.0 RLSW8ST8 = 0	
TST9	1.0 PLSW9ST9 + 1.0 QLSW9ST9 - 1.0 RLSW9ST9 = 0	
TST0	1.0 PLSW0ST0 + 1.0 QLSW0ST0 - 1.0 RLSW0ST0 = 0	

(a) Probability of 0.75

Constraint Name	Constraint	Explanation and Comments
GRID1	96. D11 + 34. D12 + 30. D13 + 30. D14 - 1.0 QDREQ1 = 0	
LSW1ST1	0. D11 + 80. D12 + 120. D13 + 170. D14 - 1.0 RLSW1ST1 = 0	
GRID2	500. D21 + 234. D22 + 137. D23 + 138. D24 - 1.0 QDREQ2 = 0	
LSW2ST2	0. D21 + 660. D22 + 1000. D23 + 1140. D24 - 1.0 RLSW2ST2 = 0	
GRID3	330. D31 + 198. D32 + 99. D33 + 99. D34 - 1.0 QDREQ3 = 0	
LSW3ST3	0. D31 + 570. D32 + 690. D33 + 940. D34 - 1.0 RLSW3ST3 = 0	
GRID4	320. D41 + 128. D42 + 84. D43 + 84. D44 - 1.0 QDREQ4 = 0	
LSW4ST4	0. D41 + 450. D42 + 450. D43 + 710. D44 - 1.0 RLSW4ST4 = 0	
GRID5	242. D51 + 91. D52 + 63. D53 + 62. D54 - 1.0 QDREQ5 = 0	
LSW5ST5	0. D51 + 480. D52 + 450. D53 + 520. D54 - 1.0 RLSW5ST5 = 0	
GRID6	44. D61 + 20. D62 + 12. D63 + 12. D64 - 1.0 QDREQ6 = 0	
LSW6ST6	0. D61 + 60. D62 + 68. D63 + 107. D64 - 1.0 RLSW6ST6 = 0	
GRID7	730. D71 + 325. D72 + 198. D73 + 198. D74 - 1.0 QDREQ7 = 0	
LSW7ST7	0. D71 + 650. D72 + 820. D73 + 1350. D74 - 1.0 RLSW7ST7 = 0	
GRID8	340. D81 + 180. D82 + 98. D83 + 97. D84 - 1.0 QDREQ8 = 0	
LSW8ST8	0. D81 + 430. D82 + 450. D83 + 750. D84 - 1.0 RLSW8ST8 = 0	
GRID9	228. D91 + 116. D92 + 65. D93 + 64. D94 - 1.0 QDREQ9 = 0	
LSW9ST9	0. D91 + 220. D92 + 185. D93 + 345. D94 - 1.0 RLSW9ST9 = 0	
GRID0	127. D01 + 73. D02 + 38. D03 + 37. D04 - 1.0 QDREQ0 = 0	
LSW0ST0	0. D01 + 130. D02 + 150. D03 + 295. D04 - 1.0 RLSW0ST0 = 0	

(b) Probability of 0.95.

Figure 23. Constraints for water storage requirements.

<u>Constraint Name</u>	<u>Constraint</u>	<u>Explanation and Comments</u>
WLREQ1	1.0 QLSW1WL1 + 1.0 QCGW1WL1 + 1.0 QFGW1WL1 = 715.0	These equations show the constraint on water to meet the depletion requirement for wetland use. The RHS is the 1965 wetland demand shown earlier.
WLREQ2	1.0 QLSW2WL2 + 1.0 QCGW2WL2 + 1.0 QFGW2WL2 = 240.0	
WLREQ3	1.0 QLSW3WL3 + 1.0 QCGW3WL3 + 1.0 QFGW3WL3 = 143.1	
WLREQ4	1.0 QLSW4WL4 + 1.0 QCGW4WL4 + 1.0 QFGW4WL4 = 276.4	
WLREQ5	1.0 QLSW5WL5 + 1.0 QCGW5WL5 + 1.0 QFGW5WL5 = 332.6	
WLREQ6	1.0 QLSW6WL6 + 1.0 QCGW6WL6 + 1.0 QFGW6WL6 = 130.0	
WLREQ7	1.0 QLSW7WL7 + 1.0 QCGW7WL7 + 1.0 QFGW7WL7 = 315.0	
WLREQ8	1.0 QLSW8WL8 = 36.0	
WLREQ9	1.0 QLSW9WL9 = 8.0	
WLREQ0	1.0 QLSW0WL0 + 1.0 QCGW0WL0 + 1.0 QFGW0WL0 = 19.0	

Figure 21. Constraints for water depletion requirements for wetland use.

<u>Constraint Name</u>	<u>Constraint</u>	<u>Explanation and Comments</u>
DREQ1	1.0 RLSW1AG1 + 1.0 RLSW1MI1 + 1.0 PLSW1MI4 - 1.0 QLSW2SW1 - 0.0 QWW1LSW1 - 0.2 QAR1LSW1 - 1.0 QDREQ1 = 0	These equations calculate the amount of draft required from water in storage reservoirs. Provision is made to include a portion of the M&I waste water return flow and agriculture return flow in the equation. This portion of the return flow is that which is available for re-use downstream. The coefficient for M&I return flow was estimated to be zero since the geographic location of the major cities and towns indicated negligible re-use of waste water downstream. The coefficient for agriculture return flow was estimated from an examination of the present relationship between draft and storage.
DREQ2	1.0 RLSW2AG2 + 1.0 RLSW2MI2 + 1.0 QLSW2SW1 + 1.0 QLSW2SW3 - 0.0 QWW2LSW2 - 0.8 QAR2LSW2 - 1.0 QDREQ2 = 0	
DREQ3	1.0 RLSW3AG3 + 1.0 RLSW3MI3 + 1.0 PLSW3AG2 + 1.0 RLSW3SW4 - 1.0 QLSW2SW3 - 1.0 QUILSW3 - 0.0 QWW3LSW3 - 0.5 QAR3LSW3 - 1.0 QDREQ3 = 0	
DREQ4	1.0 RLSW4AG4 + 1.0 RLSW4MI4 + 1.2 5QLSW4SW5 - 1.0 RLSW3SW4 - 1.0 QBULSW4 - 1.0 QUILSW4 - 1.0 QSALSW4 - 1.0 PLSW7SW4 - 0.0 QWW4LSW4 - 0.7 QAR4LSW4 - 1.0 QDREQ4 = 0	
DREQ5	1.0 RLSW5AG5 + 1.0 RLSW5MI5 + 1.0 QLSW5SW6 + 1.0 PLSW5AG9 - 1.0 QLSW4SW5 - 1.0 QBULSW5 - 1.0 QUILSW5 - 1.0 QSALSW5 - 0.0 QWW5LSW5 - 0.8 QAR5LSW5 - 1.0 QDREQ5 = 0	
DREQ6	1.0 RLSW6AG6 + 1.0 RLSW6MI6 - 1.0 QLSW5SW6 - 1.0 RLSW0SW6 - 0.0 QWW6LSW6 - 0.6 QAR6LSW6 - 1.0 QDREQ6 = 0	
DREQ7	1.0 RLSW7AG7 + 1.0 RLSW7MI7 + 1.0 PLSW7SW4 + 1.0 QBUMPT + 1.0 QUIMPT - 0.0 QWW7LSW7 - 0.3 QAR7LSW7 - 1.0 QDREQ7 = 0	
DREQ8	1.0 RLSW8AG8 + 1.0 RLSW8MI8 + 1.0 PLSW8AG5 + 1.0 QSAMPT - 0.0 QWW8LSW8 - 0.2 QAR8LSW8 - 1.0 QDREQ8 = 0	
DREQ9	1.0 RLSW9AG9 + 1.0 RLSW9MI9 - 0.0 QWW9LSW9 - 0.2 QAR9LSW9 - 1.0 QDREQ9 = 0	
DREQ0	1.0 RLSW0AGO + 1.0 RLSW0MI0 + 1.0 RLSW0SW6 - 0.0 QWW0LSW0 - 0.3 QAR0LSW0 - 1.0 QDREQ0 = 0	

Figure 22. Constraints for reservoir draft requirements.

Figure 22. Constraints for reservoir draft requirements.

Constraint Name	Constraint		Explanations and Comments
AGREQ1	1.0 RLSW1AG1 + 1.0 RGW1AG1	≧ 124.0	These inequalities show the constraints on water to meet the diversion requirements for agricultural use. The RHS is the 1965 agriculture demand shown earlier.
AGREQ2	1.0 RLSW2AG2 + 1.0 RGW2AG2 + 1.0 PLSW3AG2	≧ 1034.0	
AGREQ3	1.0 RLSW3AG3 + 1.0 RGW3AG3	≧ 643.4	
AGREQ4	1.0 RLSW4AG4 + 1.0 RGW4AG4	≧ 796.7	
AGREQ5	1.0 RLSW5AG5 + 1.0 RGW5AG5 + 1.0 PLSW8AG5	≧ 1017.9	
AGREQ6	1.0 RLSW6AG6 + 1.0 RGW6AG6	≧ 300.0	
AGREQ7	1.0 RLSW7AG7 + 1.0 RGW7AG7	≧ 789.1	
AGREQ8	1.0 RLSW8AG8	≧ 303.0	
AGREQ9	1.0 RLSW9AG9 + 1.0 PLSW5AG9	≧ 150.0	
AGREQ0	1.0 RLSW0AG0 + 1.0 RGW0AG0	≧ 68.0	
TLSW1AG1	1.0 PLSW1AG1 + 1.0 QLSW1AG1 - 1.0 RLSW1AG1	= 0	These equations sum the diversion from present developments to agriculture from local surface water with the new development diversions to get the total diversions to agriculture from local surface water.
TLSW2AG2	1.0 PLSW2AG2 + 1.0 QLSW2AG2 - 1.0 RLSW2AG2	= 0	
TLSW3AG3	1.0 PLSW3AG3 + 1.0 QLSW3AG3 - 1.0 RLSW3AG3	= 0	
TLSW4AG4	1.0 PLSW4AG4 + 1.0 QLSW4AG4 - 1.0 RLSW4AG4	= 0	
TLSW5AG5	1.0 PLSW5AG5 + 1.0 QLSW5AG5 - 1.0 RLSW5AG5	= 0	
TLSW6AG6	1.0 PLSW6AG6 + 1.0 QLSW6AG6 - 1.0 RLSW6AG6	= 0	
TLSW7AG7	1.0 PLSW7AG7 + 1.0 QLSW7AG7 - 1.0 RLSW7AG7	= 0	
TLSW8AG8	1.0 PLSW8AG8 + 1.0 QLSW8AG8 - 1.0 RLSW8AG8	= 0	
TLSW9AG9	1.0 PLSW9AG9 + 1.0 QLSW9AG9 - 1.0 RLSW9AG9	= 0	
TLSW0AG0	1.0 PLSW0AG0 + 1.0 QLSW0AG0 - 1.0 RLSW0AG0	= 0	
TGW1AG1	1.0 PGW1AG1 + 1.0 QGW1AG1 - 1.0 RGW1AG1	= 0	These equations sum the diversion from present developments to agriculture from groundwater with the new development diversions to get the total diversions to agriculture from groundwater.
TGW2AG2	1.0 PGW2AG2 + 1.0 QGW2AG2 - 1.0 RGW2AG2	= 0	
TGW3AG3	1.0 PGW3AG3 + 1.0 QGW3AG3 - 1.0 RGW3AG3	= 0	
TGW4AG4	1.0 PGW4AG4 + 1.0 QGW4AG4 - 1.0 RGW4AG4	= 0	
TGW5AG5	1.0 PGW5AG5 + 1.0 QGW5AG5 - 1.0 RGW5AG5	= 0	
TGW6AG6	1.0 PGW6AG6 + 1.0 QGW6AG6 - 1.0 RGW6AG6	= 0	
TGW7AG7	1.0 PGW7AG7 + 1.0 QGW7AG7 - 1.0 RGW7AG7	= 0	
TGW0AG0	1.0 PGW0AG0 + 1.0 QGW0AG0 - 1.0 RGW0AG0	= 0	
AGEXC3	1.0 QAG3LSW3 + 1.0 QAG3GW3	= 0	These equations are for use in transferring excess water from agriculture where these depletions reduce with time.
AGEXC4	1.0 QAG4LSW4 + 1.0 QAG4GW4	= 0	
AGEXC8	1.0 QAG8LSW8	= 0	

Figure 20. Constraints for water diversion requirements for agricultural use.

<u>Constraint Name</u>	<u>Constraint</u>		<u>Explanation and Comments</u>
MIREQ1	1.0 RLSW1MI1 + 1.0 RGW1MI1	≥ 10.0	These inequalities show the constraint on water to meet the diversion requirements for municipal and industrial use. The RHS is the 1965 M&I demand shown earlier.
MIREQ2	1.0 RLSW2MI2 + 1.0 RGW2MI2	≥ 44.0	
MIREQ3	1.0 RLSW3MI3 + 1.0 RGW3MI3	≥ 49.7	
MIREQ4	1.0 RLSW4MI4 + 1.0 RGW4MI4 + 1.0 PLSW1MI4	≥ 302.5	
MIREQ5	1.0 RLSW5MI5 + 1.0 RGW5MI5	≥ 17.0	
MIREQ6	1.0 RLSW6MI6 + 1.0 RGW6MI6	≥ 13.0	
MIREQ7	1.0 RLSW7MI7 + 1.0 RGW7MI7	≥ 10.0	
MIREQ8	1.0 RLSW8MI8	≥ 7.0	
MIREQ9	1.0 RLSW9MI9	≥ 6.8	
MIREQ0	1.0 RLSW0MI0 + 1.0 RGW0MI0	≥ 1.5	
TLW1MI1	1.0 PLSW1MI1 + 1.0 QLSW1MI1 - 1.0 RLSW1MI1	= 0	These equations sum the diversion from present development to M&I from local surface water with the new development diversions to get the total diversion to M&I from local surface water.
TLW2MI2	1.0 PLSW2MI2 + 1.0 QLSW2MI2 - 1.0 RLSW2MI2	= 0	
TLW3MI3	1.0 PLSW3MI3 + 1.0 QLSW3MI3 - 1.0 RLSW3MI3	= 0	
TLW4MI4	1.0 PLSW4MI4 + 1.0 QLSW4MI4 - 1.0 RLSW4MI4	= 0	
TLW5MI5	1.0 PLSW5MI5 + 1.0 QLSW5MI5 - 1.0 RLSW5MI5	= 0	
TLW6MI6	1.0 PLSW6MI6 + 1.0 QLSW6MI6 - 1.0 RLSW6MI6	= 0	
TLW7MI7	1.0 PLSW7MI7 + 1.0 QLSW7MI7 - 1.0 RLSW7MI7	= 0	
TLW8MI8	1.0 PLSW8MI8 + 1.0 QLSW8MI8 - 1.0 RLSW8MI8	= 0	
TLW9MI9	1.0 PLSW9MI9 + 1.0 QLSW9MI9 - 1.0 RLSW9MI9	= 0	
TLW0MI0	1.0 PLSW0MI0 + 1.0 QLSW0MI0 - 1.0 RLSW0MI0	= 0	
TGW1MI1	1.0 PGW1MI1 + 1.0 QGW1MI1 - 1.0 RGW1MI1	= 0	These equations sum the diversion from present developments to M&I from groundwater with the new development diversion to get the total diversion to M&I from groundwater.
TGW2MI2	1.0 PGW2MI2 + 1.0 QGW2MI2 - 1.0 RGW2MI2	= 0	
TGW3MI3	1.0 PGW3MI3 + 1.0 QGW3MI3 - 1.0 RGW3MI3	= 0	
TGW4MI4	1.0 PGW4MI4 + 1.0 QGW4MI4 - 1.0 RGW4MI4	= 0	
TGW5MI5	1.0 PGW5MI5 + 1.0 QGW5MI5 - 1.0 RGW5MI5	= 0	
TGW6MI6	1.0 PGW6MI6 + 1.0 QGW6MI6 - 1.0 RGW6MI6	= 0	
TGW7MI7	1.0 PGW7MI7 + 1.0 QGW7MI7 - 1.0 RGW7MI7	= 0	
TGW0MI0	1.0 PGW0MI0 + 1.0 QGW0MI0 - 1.0 RGW0MI0	= 0	

Figure 19. Constraints for water diversion requirements for municipal and industrial use.

Constraint Name	Constraint	Explanations and Comments
AVAILSW1	1.0 RLSW1AG1 + 1.0 QLSW1R1 + 1.0 RLSW1MI1 + 1.0 QLSW1WL1 + 1.0 PLSW1MI4 + 1.0 QLSW1EV1 - 1.0 QWW1LSW1 - 1.0 QAR1LSW1 - 1.0 QLSW2SW1 + 1.0 QLSW1OF1 = 613.0	The equations calculate the maximum surface water outflow in each of the HSU. The RHS is the local surface water availability.
AVAILSW2	1.0 RLSW2AG2 + 1.0 QLSW2R2 + 1.0 RLSW2MI2 + 1.0 QLSW2WL2 + 1.0 QLSW2SW1 + 1.0 QLSW2SW3 + 1.0 QLSW2EV2 - 1.0 QWW2LSW2 - 1.0 QAR2LSW2 + 1.0 QLSW2OF2 = 941.5	
AVAILSW3	1.0 RLSW3AG3 + 1.0 QLSW3R3 + 1.0 RLSW3MI3 + 1.0 QLSW3WL3 + 1.0 PLSW3AG2 + 1.0 RLSW3SW4 + 1.0 QLSW3EV3 - 1.0 QAG3LSW3 - 1.0 QWW3LSW3 - 1.0 QAR3LSW3 - 1.0 QUILSW3 - 1.0 QLSW2SW3 + 1.0 QLSW3OF3 = 789.2	
AVAILSW4	1.0 RLSW4AG4 + 1.0 QLSW4R4 + 1.0 QLSW4RU4 + 1.0 RLSW4MI4 + 1.0 QLSW4WL4 + 1.25QLSW4SW5 + 1.0 QLSW4EV4 - 1.0 QAG4LSW4 - 1.0 QWW4LSW4 - 1.0 QAR4LSW4 - 1.0 QBULSW4 - 1.0 QUILSW4 - 1.0 QSALSW4 - 1.0 RLSW3SW4 - 1.0 PLSW7SW4 + 1.0 QLSW4OF4 = 513.6	
AVAILSW5	1.0 RLSW5AG5 + 1.0 QLSW5R5 + 1.0 QLSW5RU5 + 1.0 RLSW5MI5 + 1.0 QLSW5WL5 + 1.0 QLSW5SW6 + 1.0 PLSW5AG9 + 1.0 QLSW5EV5 - 1.0 QLSW4SW5 - 1.0 QBULSW5 - 1.0 QUILSW5 - 1.0 QSALSW5 - 1.0 QWW5LSW5 - 1.0 QAR5LSW5 + 1.0 QLSW5OF5 = 453.2	
AVAILSW6	1.0 RLSW6AG6 + 1.0 QLSW6R6 + 1.0 RLSW6MI6 + 1.0 QLSW6WL6 + 1.0 QLSW6EV6 - 1.0 QWW6LSW6 - 1.0 QAR6LSW6 - 1.0 RLSWOSW6 - 1.0 QLSW5SW6 + 1.0 QLSW6OF6 = 80.0	
AVAILSW7	1.0 QBUMPT + 1.0 QUIMPT + 1.0 Q7LSW7 + 1.0 PLSW7SW4 + 1.0 QLSW7EV7 + 1.0 QLSW7OF7 = 1351.6	
AVAILSW8	1.0 QSAMPT + 1.0 Q8LSW8 + 1.0 PLSW8AG5 + 1.0 QLSW8EV8 - 1.0 QAG8LSW8 + 1.0 QLSW8OF8 = 650.0	
AVAILSW9	1.0 RLSW9AG9 + 1.0 RLSW9MI9 + 1.0 QLSW9WL9 + 1.0 QLSW9EV9 - 1.0 QWW9LSW9 - 1.0 QAR9LSW9 + 1.0 QLSW9OF9 = 430.0	
AVAILSW0	1.0 RLSW0AG0 + 1.0 QLSW0R0 + 1.0 RLSW0MI0 + 1.0 QLSW0WL0 + 1.0 QLSW0EV0 + 1.0 RLSW0SW6 - 1.0 QWW0LSW0 - 1.0 QAR0LSW0 + 1.0 QLSW0OF0 = 250.0	
LSWU7	1.0 RLSW7AG7 + 1.0 QLSW7R7 + 1.0 RLSW7MI7 + 1.0 QLSW7WL7 - 1.0 QWW7LSW7 - 1.0 QAR7LSW7 - 1.0 Q7LSW7 = 0	These equations calculate the surface water use in HSU 7 and 8 and are for convenience in writing other constraints.
LSWU8	1.0 RLSW8AG8 + 1.0 RLSW8MI8 + 1.0 QLSW8WL8 - 1.0 QWW8LSW8 - 1.0 QAR8LSW8 - 1.0 Q8LSW8 = 0	

Figure 17.. Constraints for availability of local surface water.

Constraints Name	Constraint	Explanation and Comments
AVAILGW1	1.0 RGW1AG1 + 1.0 RGW1MI1 + 1.0 QCGW1WL1 + 1.0 QFGW1WL1 + 1.0 QGW1EV1 - 1.0 QWW1R1 - 1.0 QAR1GW1 - 1.0 QLSW1R1 + 1.0 QGW1OF1 = 187.0	These equations calculate the maximum groundwater outflow in each of the HSU except 8 and 9 where groundwater is negligible. The RHS is the groundwater availability.
AVAILGW2	1.0 RGW2AG2 + 1.0 RGW2MI2 + 1.0 QCGW2WL2 + 1.0 QFGW2WL2 + 1.0 QGW2EV2 - 1.0 QWW2R2 - 1.0 QAR2GW2 - 1.0 QLSW2R2 + 1.0 QGW2OF2 = 103.5	
AVAILGW3	1.0 RGW3AG3 + 1.0 RGW3MI3 + 1.0 QCGW3WL3 + 1.0 QFGW3WL3 + 1.0 QGW3EV3 - 1.0 QWW3R3 - 1.0 QAR3GW3 - 1.0 QAG3GW3 - 1.0 QLSW3R3 + 1.0 QGW3OF3 = 94.9	
AVAILGW4	1.0 RGW4AG4 + 1.0 RGW4MI4 + 1.0 QCGW4WL4 + 1.0 QFGW4WL4 + 1.0 QGW4EV4 - 1.0 QWW4R4 - 1.0 QWW4RU4 - 1.0 QAR4GW4 - 1.0 QAG4GW4 - 1.0 QLSW4R4 - 1.0 QLSW4RU4 + 1.0 QGW4OF4 = 272.1	
AVAILGW5	1.0 RGW5AG5 + 1.0 RGW5MI5 + 1.0 QCGW5WL5 + 1.0 QFGW5WL5 + 1.0 QGW5EV5 - 1.0 QWW5R5 - 1.0 QWW5RU5 - 1.0 AR5GW5 - 1.0 QLSW5R5 - 1.0 QLSW5RU5 + 1.0 QGW5OF5 = 254.6	
AVAILGW6	1.0 RGW6AG6 + 1.0 RGW6MI6 + 1.0 QCGW6WL6 + 1.0 QFGW6WL6 + 1.0 QGW6EV6 - 1.0 QWW6R6 - 1.0 QAR6GW6 - 1.0 QLSW6R6 + 1.0 QGW6OF6 = 130.0	
AVAILGW7	1.0 RGW7AG7 + 1.0 RGW7MI7 + 1.0 QCGW7WL7 + 1.0 QFGW7WL7 + 1.0 QGW7EV7 - 1.0 QWW7R7 - 1.0 QLSW7R7 - 1.0 QAR7GW7 + 1.0 QGW7OF7 = 40.0	
AVAILGW0	1.0 RGW0AG0 + 1.0 RGW0MI0 + 1.0 QCGW0WL0 + 1.0 QFGW0WL0 + 1.0 QGW0EV0 - 1.0 QWW0R0 - 1.0 QAR0GW0 - 1.0 QLSW0R0 + 1.0 QGW0OF0 = 10.0	

Figure 18. Constraints for availability of groundwater.

Diversion to agriculture. The general forms of the cost coefficients for diverting local surface water and ground water to agriculture are:

<u>Variable</u>	<u>Component of Cost Coefficient</u>
QLSWXAGX	CLSWXDAG
QGWXAGX	CGWXDAG + CPXAG

Diversion to municipal and industrial. The general forms of the cost coefficient for diverting local surface water and groundwater to municipal and industrial use includes the cost of treatment. These forms are:

<u>Variable</u>	<u>Component of Cost Coefficient</u>
QLSWXMIX	CLSWDMI + CTCXSW
QGWXMIX	CGWDMI + CPXMI + CBMI + CTGWX

Diversion of groundwater to wetlands. The cost coefficient has only a single component which is the cost to pump water for agriculture. The general form is:

<u>Variable</u>	<u>Component of Cost Coefficient</u>
QCGWXWLX	CPXAG

Groundwater recharge. The general forms for these cost coefficients are shown below. The municipal and industrial waste water must be treated before it can be used for recharge.

<u>Variable</u>	<u>Component of Cost Coefficient</u>
QLSWXRX	CRC + CC
QLSWXRUX	CRC + CC + CTRC
QWWXRX	CRC + CTWWRC
QWWXRUX	CRC + CTWWRC + CTRC

Reclaiming municipal and industrial waste water. These variables represent the reclamation of waste water when it is returned to local surface water. The general form of the cost coefficient is:

<u>Variable</u>	<u>Component of Cost Coefficient</u>
QWWXLSWX	CTWWLSW

Storage of local surface water. The general form of the cost coefficient is:

<u>Variable</u>	<u>Component of Cost Coefficient</u>
QLSWXSTX	CSTX

Constraints

The model constraints consist of both equations and inequalities and are described in the following paragraphs. Each equation is given a name for the computer solution. The equations are grouped according to the classifications discussed earlier.

Water availability

The constraints related to water availability are divided into two groups: (1) those related to available local surface water shown in Figure 17 and (2) those related to available groundwater shown in Figure 18.

Water requirements

The constraints related to water requirements are divided into three groups: (1) those related to diversion requirements for municipal and industrial shown in Figure 19, (2) those related to diversion requirements for agriculture shown in Figure 20, and (3) those related to depletion requirements for wetlands shown in Figure 21.

Reservoir storage and evaporation loss

These constraints are divided into three groups: (1) those related to the storage draft requirements shown in Figure 22, (2) those related to the determination of the storage required shown in Figure 23, and (3) those related to determination of the net loss by reservoir evaporation shown in Figure 24.

Return flows

The constraints related to the return flows are divided into two groups: (1) those related to waste water return flow from municipal and industrial use shown in Figure 25 and (2) those related to return flow from agriculture shown in Figure 26.

Free groundwater for wetlands

The constraints related to the groundwater that can be used freely by wetlands are shown in Figure 27.

Limits

The constraints defining additional limits other than water availability and demands are divided into three groups: (1) those limiting the amount of groundwater recharge shown in Figure 28, (2) those limiting the amount of the interbasin transfers shown in Figure 29, and (3) those limiting the outflow from the various study units shown in Figure 30.

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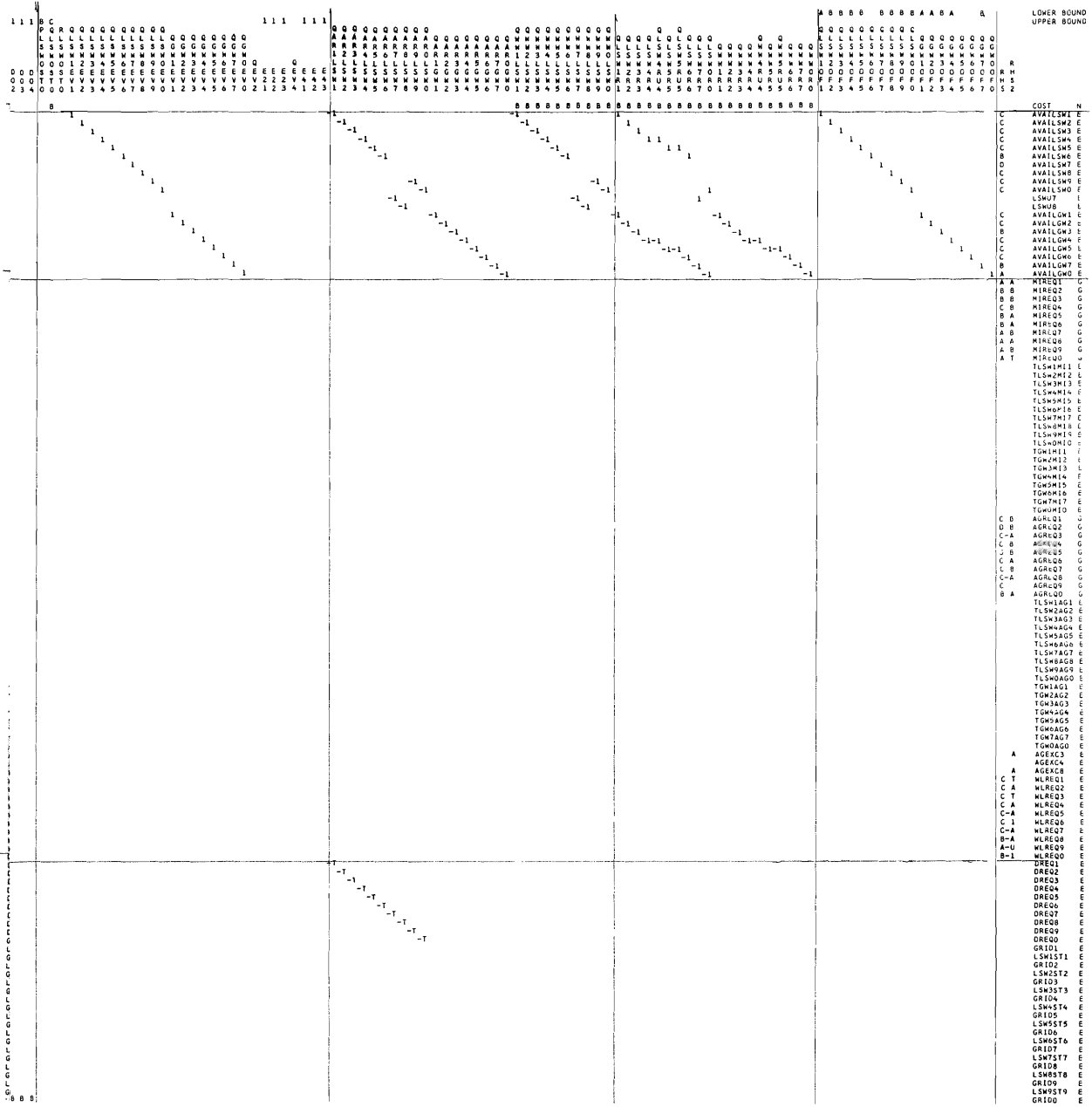


Figure 31. Complete matrix of the allocation model.

Table 10. Variable bounds on additional surface water storage.

Variable	Bound (ac-ft/yr)	Type of Bound
QLSW1ST1	25,000	Upper
QLSW2ST2	1,200,000	Upper
QLSW3ST3	125,000	Upper
QLSW4ST4	1,050,000	Upper
QLSW5ST5	125,000	Upper
QLSW6ST6	100,000	Upper
QLSW7ST7	1,500,000	Upper
QLSW8ST8	285,000	Upper
QLSW9ST9	140,000	Upper
QLSW0ST0	280,000	Upper

Table 11. Variable bounds on surface and groundwater outflow.

Variable	Bound (ac-ft/yr)	Type of Bound
QLSW1OF1	7,000	Lower
QLSW2OF2	50,000	Lower
QLSW3OF3	50,000	Lower
QLSW4OF4	50,000	Lower
QLSW5OF5	13,700	Lower
QLSW6OF6	0,000	Lower
QLSW7OF7	100,000	Lower
QLSW8OF8	100,000	Lower
QLSW9OF9	100,000	Lower
QLSW0OF0	100,000	Lower
QGW1OF1	6,000	Lower
QGW2OF2	5,000	Lower
QGW3OF3	25,000	Lower
QGW4OF4	8,000	Lower
QGW5OF5	0,000	Lower
QGW6OF6	0,000	Lower
QGW7OF7	40,000	Lower
QGW0OF0	0,000	Lower

RESULTS FROM THE MODEL

Results from the model can be classified in three general categories: 1) those which are available as part of the optimum solution to the linear programming problem, 2) those available in a post-optimal analysis, and 3) those which can be obtained only through a manipulation of the structural coefficients, right-hand-side values, and variable bounds. Included in the first category are the optimal solution (the optimum value of the objective function and the minimum cost allocation of water) and the determination of the shadow prices of the various resources. In the second are the sensitivity analysis of the cost coefficients and the parametric analysis of the right-hand-side. In the third category are included the effect of changing irrigation efficiency, and effect of various policies such as groundwater restrictions, inter-basin transfer limitations, changing growth projections with time, etc.

Computer print-outs of the control cards and data cards are shown in Tables B-1 and B-2 of Appendix B. The example includes the necessary control cards and data cards to systematically vary (or parameterize) the right-hand-side. The parameterized RHS values are the estimated values as time passes from the year 1965 to the year 2020. This 55 year time interval was divided into 5.5 year increments. The symbol θ (Theta) is the time parameter and takes values between 0 and 10. Thus the optimum allocation can be found for the year 1965 ($\theta = 0$) and at each 5.5 year time interval thereafter to the year 2020 ($\theta = 10$). A computer print-out of the optimum allocation for 1965 is also shown in Table B-3 of Appendix B.

Results from the Optimal Solution

Solution to the linear programming problem consists of several parts including the optimum value of the objective function, the optimal activity levels or values of the real and slack variables, and the solution of the dual to the linear programming problem.

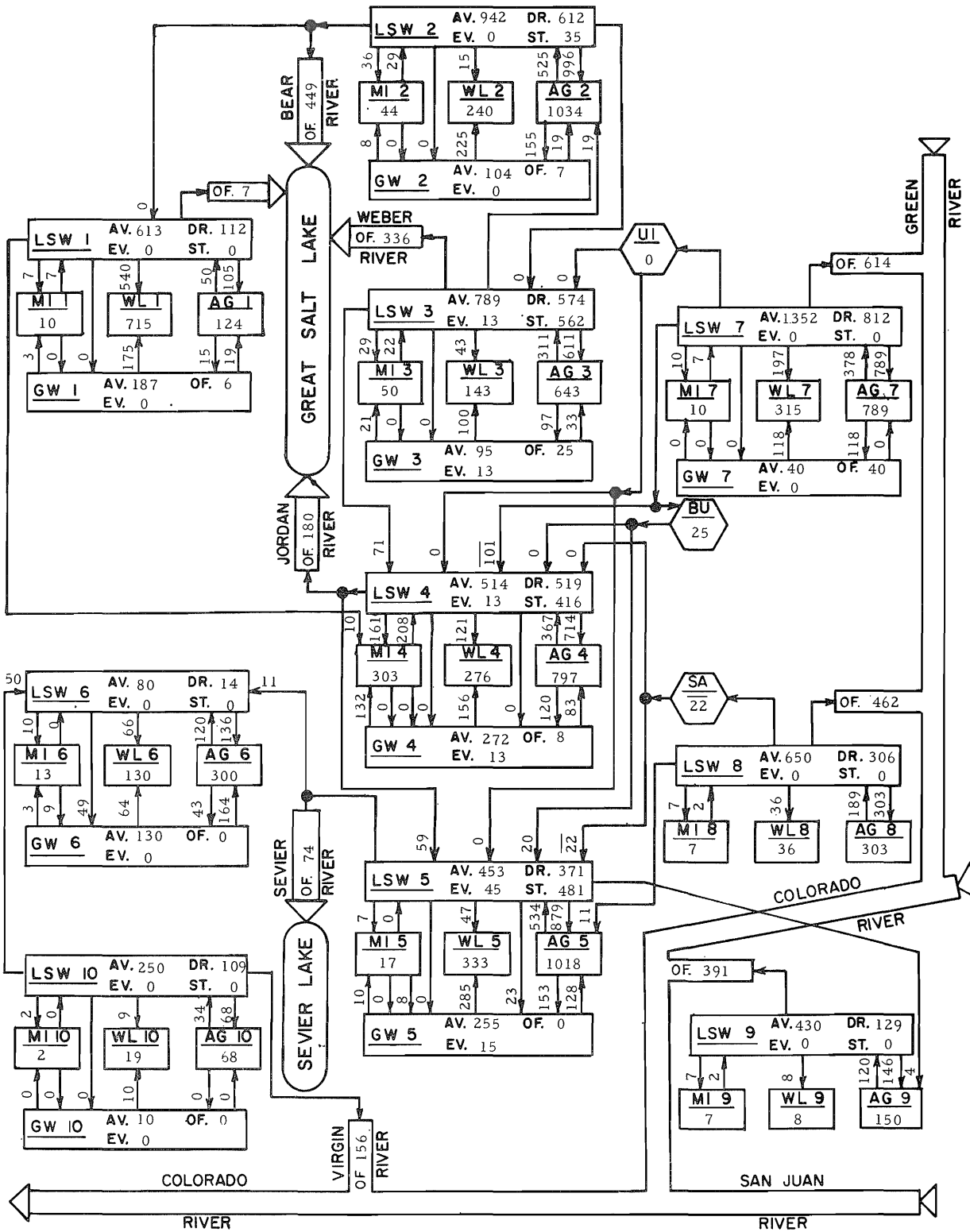
Optimum value of the objective function

The optimum value of the objective function is used primarily to compare one optimum solution with another. In this project, the optimum value (scaled in thousands of dollars) represents the minimum annual cost of development of new facilities to meet the specified demands for water under a particular set of assumptions. For example, the computer print-out shown in Appendix B lists the optimum value of the objective function as \$9722.44726

thousands. This solution is based on the water demands for the year 1965 and the assumption is made in the model that groundwater mining is not permitted. Since facilities existing in 1965 are in the model at zero cost, the value of the objective function in this case represents the yearly cost of developing new facilities to eliminate groundwater mining in HSU 5 and 6. Cost projections over time are made by examining the changes in the value of the objective function as the right-hand-side values of the demand constraints are changed as shown in a later paragraph.

Optimal allocation

For a given set of water requirements and constraints, the minimum cost allocation of water in the state is given by the activity levels or values of the variables in the optimal solution. As an aid in the analysis of the allocation pattern, these activity levels are transferred to flow diagrams as shown on Figure 15. For example data from the computer print-out in Appendix B were transferred to the flow diagram shown on Figure 32. As discussed in the previous paragraph, the allocations represent those values of the variables which bring about the minimum cost to develop new facilities to eliminate groundwater mining in HSU 5 and 6 and to meet water demands for the year 1965. The actual water allocations existing in 1965 are shown on the flow diagram of Figure 16. A comparison of these two flow diagrams shows that the water which is being mined can be replaced by importing additional water from HSU 4, 7, 8, and 10. This imported water together with M & I waste water is used to recharge the groundwater aquifers at an annual rate equal to the present mining rate so that presently existing pumping facilities can be continued. The additional imported water totals about 148,000 ac-ft/yr whereas only about 89,000 ac-ft/yr is presently being mined. An examination of the flow diagram shows that this extra water is dumped into the Sevier River. The reason for this apparent discrepancy or waste lies in the storage probability. One of the assumptions made in generating the data for the no groundwater mining case was that the probability of having sufficient surface water storage was 75 percent. Since the runoff in the Sevier Basin is highly variable from year to year, some of the runoff in high flow years will be lost down the river to the Sevier Dry Lake. The difference between the average outflow of about 14,000 ac-ft/yr under 1965 conditions and the calculated outflow of 74,000 ac-ft/yr under conditions that would eliminate groundwater mining must then represent a difference in



FLOW DIAGRAM FOR ALLOCATION MODEL

Figure 32. Flow diagram for the basic model (1965).

the probability of having sufficient storage. Computations were made at lower probability levels and a value of about 65 percent reduces the outflow to 14,000 ac-ft/yr. As shown the storage on the two diagrams is the same, and the presently existing storage facilities must be providing sufficient storage at a probability of about 65 percent. Thus the existing storage provides the needed water about two thirds of the years.

Resource shadow prices

Resource shadow prices are determined from the solution of the dual of the linear programming problem. The economic interpretation of the dualism property of linear programming lies in the concept that resource allocation and pricing are two aspects of the same problem. The dual problem is formulated as follows:

- a) transpose rows and columns of the constraint matrix.
- b) transpose the right-hand-side of constraints with the objective function coefficients,
- c) change the sense of the inequality signs in the constraints,
- d) change the sense of the objective function (e.g. maximize instead of minimize).

The optimal solution to this dual problem gives the values of the dual variables which are referred to as shadow prices and indicate the rate at which costs increase or decrease for a corresponding increase or decrease in the amount of resource given by the right-hand-side value of the resource constraint. These values are listed under the heading "dual activity" of the rows section of the computer print-out as shown in Appendix B. For example, the shadow price or value of the resource "available surface water in HSU 6, AVAILSW6" (shown on line 7), is \$14.00 per ac-ft/yr. This says that the value of the objective function (which is new development cost) would change by \$14.00 per year if the available surface water in HSU 6 were changed one ac-ft/yr, thus the value of this resource is defined.

Post-Optimal Analysis

Analysis of the linear programming problem after an optimal solution has been achieved is referred to as post-optimal analysis and consists primarily of two possible phases of analysis; sensitivity analysis and parametric analysis.

Sensitivity analysis

Practical problems formulated in the linear programming framework are seldom completely "solved" by the optimal solution. The coefficients of the model (objective function coefficients, structural coefficients of the constraint matrix, and constraint right-hand-side values) are seldom known with the desired degree of certainty. Also, the linear relationships assumed for a given problem formulation may not hold in the range indicated by the model solution. Therefore it is usually desirable to carry

out some sort of sensitivity analysis to determine the effect on the optimal solution of changing certain coefficients or constants to other possible values. If such an analysis indicates the optimal solution is very sensitive to small changes in the coefficients or constants, then special care should be taken in checking the values of these coefficients or constants. Thus one of the greatest helps which can come from a sensitivity analysis is the identification of those coefficients or constants which are critical to the solution, thereby reducing the number which must be reexamined. For example, an examination of the sensitivity analysis shown in sections 2 and 4 in Table B-4 of Appendix B reveals three variables for which a change in their related cost coefficients of less than 10 percent would change the allocation pattern. These variables are:

- a) QLSW3SW4 (new imported water from HSU 3 to HSU 4)
- b) QBULSW5 (water imported to HSU 5 via Bonneville Unit of CUP)
- c) QLSW4SW5 (new imported water from HSU 4 to HSU 5)

Further examination of the activity range over which the solution is valid for each of these three variables reveals narrow ranges for each, thus leading to the conclusion that these three variables have critical cost coefficients which should be determined as accurately as possible. Similar analyses can be made for the constraint right-hand-side values using data from sections 1 and 3 of the sensitivity analysis. Thus the constraint RHS values describing surface water availability, groundwater availability, M & I diversion requirements, wetland requirements, reservoir draft requirements, evaporation loss, return flow, artificial recharge, inter-basin transfer limits, inflow or outflow limits, etc., can be investigated to see which RHS values are critical limitations on the optimal solution. The critical RHS values would deserve careful review and checking.

Parametric analysis

Parametric analysis is a procedure for generating new optimal solutions from an original optimal solution while allowing one or more parameters (constraints or coefficients) to vary systematically over a specified range of values. Either the objective function coefficients or the constraint right-hand-side values or both can be varied over a desired range either singularly or in any combination. Use is made of this procedure to vary the right-hand-side values of some of the constraint equations, in particular those showing the demand for water. Thus projections of demand over time can be inserted in the model and new optimal solutions generated quite easily.

The Division of Water Resources Alternate 1 projections of growing demand in the future were put into the model as increasing values with time and the resulting optimal allocations are shown on Figures C-1 (a) through C-1 (d) of Appendix C. Some of the more significant allocation changes are plotted versus time (or the para-

meter θ) on Figure 33. These data show, for the assumptions of no groundwater mining and a minimum inflow to the Great Salt Lake of 500,000 ac-ft/yr, that these activities generally increase as time passes except for QBUMPT (Bonneville Unit import), QUIMPT (Ute Indian Unit import), and QLSW4SW5 (HSU 4 import to HSU 5). Examination of the data from the computer print-out indicated the reason the computation stopped about the year 1996 ($\theta = 5.57$) instead of continuing to the year 2020 was that the maximum achievable surface water storage was reached in HSU 2. Other significant data from this example are shown in Figure 34. This plot shows the

excess water above the minimum required for outflow of the Upper Colorado River drainage and inflow to the Great Salt Lake. As indicated the excess inflow to the Great Salt Lake goes to zero about the year 1996 ($\theta = 5.57$). Almost 400,000 ac-ft/yr of water is still available at that time for use from the Upper Colorado River allocation. This indicates further development can take place provided the problem of surface water storage in HSU 2 can be resolved. Thus the first place to look for improving the model would be to determine more accurately just what can be done about storage in HSU 2. Since storage in HSU 2 is critical in the solution, the cost

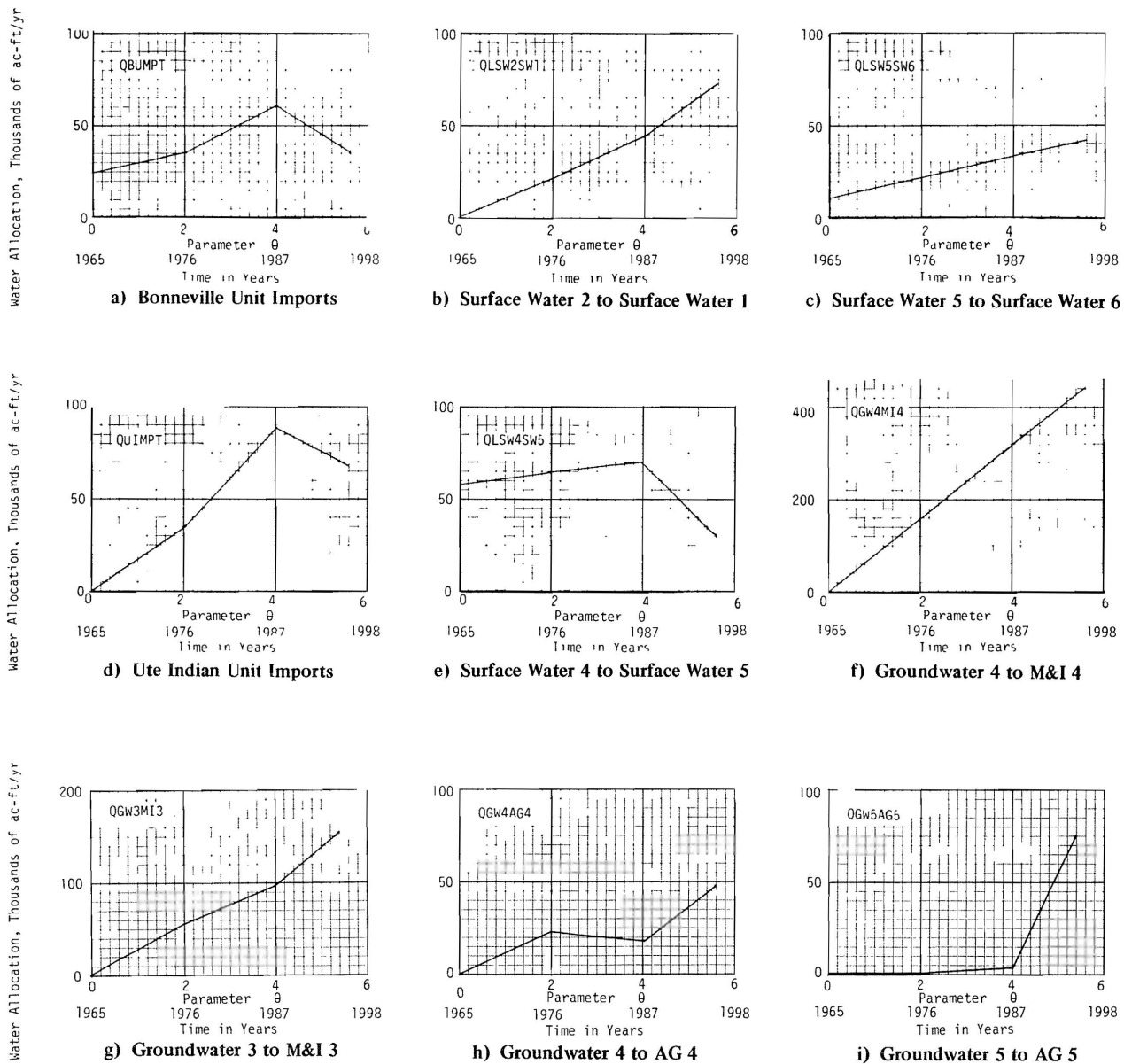


Figure 33. Allocations for the basic model as function of time.

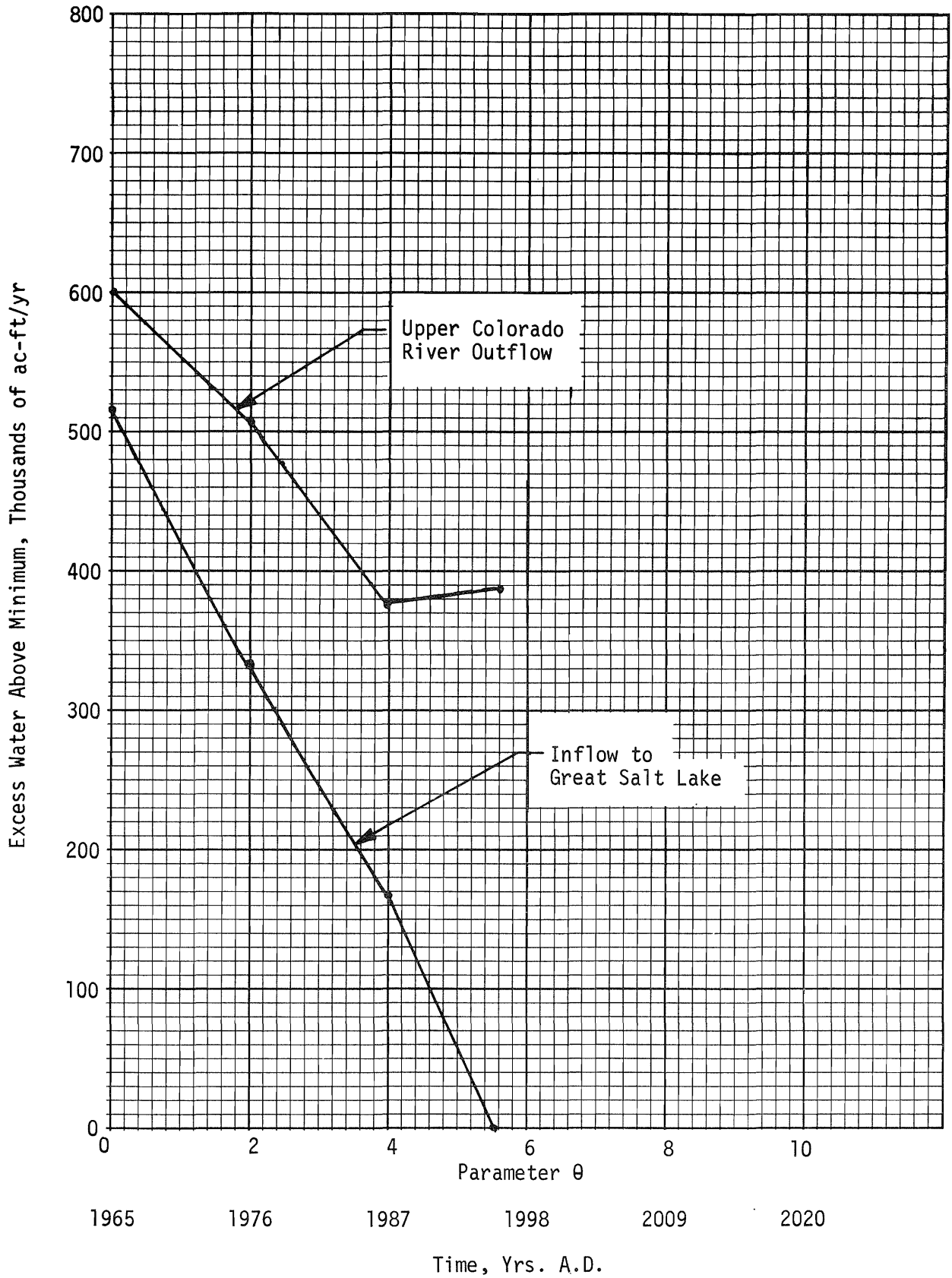


Figure 34. Excess water for the basic model as function of time.

should be reexamined to be sure it is accurate. Possibly an acceptable non-linear cost relationship could be developed which would be more accurate than the linear approximation.

Other Results

The effect of such things as changing irrigation efficiency, groundwater policy, inter-basin transfer limits, and changing growth projections can be determined by a manipulation of the model structural coefficients, right-hand-side values, and variable bounds.

Effect of changing irrigation efficiency

The effect of changing irrigation efficiency can be determined by changing the agriculture return flow coefficients in the constraints shown on Figure 26 and the right-hand-side values of the constraints shown on Figure 20. Return flow coefficients to local surface water and to groundwater must be redetermined by considering the possible changes in irrigation efficiency due to such practices as land leveling, canal and ditch lining, pipeline installations, sprinkler irrigation, and trickler irrigation. Areas affected by each improved practice must also be known and then the new return flow coefficients can be estimated and applied to the model to test the effects of improved irrigation efficiency.

Effect of changing groundwater policy

There are two rather obvious groundwater policy changes which might be investigated; 1) no groundwater recharge allowed, and 2) no further development of the groundwater allowed. Both policies should include the condition of not allowing groundwater mining as presently occurs in HSU 5 and 6. The effect of a policy of no groundwater recharge can be determined by simply setting to zero the right-hand-side values of the recharge constraints shown on Figure 28. The results of this condition are plotted on Figures C-2 (a) through C-2 (c) of Appendix C. A comparison with data from the basic model (Figures C-1 (a) through C-1 (d)) shows that the effect of the policy change is primarily greater water import to HSU 5 and 6. This import increase resulted in a halt in the computation about 1978 ($\theta = 2.36$) due to reaching maximum levels on the four import possibilities to HSU 5. The effect of a policy of no additional groundwater development (i.e. no increased pumpage but allowing recharge) can be determined by setting zero bounds on the variables representing future groundwater diversions. The results of this condition are plotted on Figures C-3 (a) through C-3 (c) of Appendix C. A comparison with data from the basic model shows the Bonneville and Ute Indian Units of the Central Utah Project (CUP) to be required at greater levels earlier in time. The model stopped about 1986 ($\theta = 3.91$) due to upper limits on new storage development in HSU 2 and 3 and minimum limit on outflow from HSU 7.

Effect of limitation on inter-basin transfer

There are many limitations on inter-basin transfer which could be examined. One of the more interesting is the condition that no further transfer be allowed from the Upper Colorado River Basin to the Great Basin other than the Bonneville Unit of CUP. The effect of this limitation can be determined by setting zero bounds on the two variables representing the other transfers. The results of this condition are plotted on Figures C-4 (a) through C-4 (c). A comparison with data from the basic model shows that the Bonneville Unit does not reach maximum size before the computation stops about 1986 ($\theta = 3.91$). The additional water demands were supplied by reducing the inflow to the Great Salt Lake. The model computation stopped due to reaching an upper limit on imports to HSU 5.

Effect of changing growth projections

The projected growth as shown by the Division of Water Resources as alternate 1 in the Interim Report of 1970 is higher than earlier projections made about June 1969. Likewise the alternate 2, 3, and 4 projections in the Interim Report are significantly different from alternate 1 projections and reflect different possibilities of growth and different means to meet the water demands of the growth. The effect of changing the growth projections to those of the earlier estimate can be determined by changing the increments used in parameterizing the right-hand-side of the water demand constraints shown on Figures 19, 20, and 21. The results of this condition are plotted on Figures C-5 (a) through C-5 (f). A comparison with the data from the basic model shows that the lower growth projection allows the computation to run to the year 2020 ($\theta = 10$). Neither the Bonneville Unit nor Ute Indian Unit of CUP were developed completely, and the additional water requirements were supplied by reducing the inflow to the Great Salt Lake.

Effect of giving up some present diversions

It may be more efficient to give up some of the presently developed facilities and replace them with larger or different facilities in later years. The effect of this policy can be determined by changing the bounds on the variables representing present development from fixed bounds (which forces the model to keep all present developments) to upper bounds (which allows the model to choose how much of the present development should be kept for minimum cost). The results of this condition are plotted on Figures C-6 (a) through C-6 (d). A comparison with the data from the basic model shows the only significant difference between the two models is that this new model does not recharge the groundwater in HSU 6 but chooses to give up some of the present pumpage.

Effect of changing the probability on storage

It may be desired to determine the effect on the allocation pattern of changing the probability of having sufficient storage to supply the required draft. This effect can be determined by changing the draft-storage relationship coefficients as given in Figure 23. The basic model assumed a probability of 0.75 and used the coefficients from Figure 23 (a). Coefficients for other probability levels can be determined using the non-linear curves shown on Figures 2 through 11. These coefficients have been determined for a probability of 0.95 and are shown on Figure 23 (b). The results of assuming a probability of 0.95 are plotted on Figures C-7 (a) through C-7 (d). A comparison with the data from the basic model shows greatly increased storage is required earlier in HSU 2, 3, and 7. As a result the model could only go to about the year 1988 ($\theta = 4.15$) before reaching a limit on new storage in HSU 2.

Effect of changing policy of maintaining Great Salt Lake level

Requirements for mineral rights, recreation, and ecological demands may require maintaining the level of Great Salt Lake at some particular elevation. The average inflow to Great Salt Lake from Utah drainage over recent years, has been about 1,088,000 ac-ft/yr. The effect of having some particular inflow requirement can be determined by simply changing the right-hand-side value of the inflow constraint as given on Figure 30. The results of this policy are plotted on Figures C-8 (a) through C-8 (d) for an inflow $\geq 800,000$ ac-ft/yr and on Figures C-9 (a) through C-9 (d) for an inflow $\geq 1,088,000$ ac-ft/yr. A comparison with data from the basic model (which assumes an inflow $\geq 500,000$ ac-ft/yr) shows no change from the basic model in early years for the 800,000 ac-ft/yr model. Later this model required more import from HSU 7 and greater storage in HSU 5, however, this model stopped at the same time and for the same reason as the basic model. Results from the 1,088,000 ac-ft/yr case showed the requirement for greater import from HSU 7 started even earlier than the 800,000 ac-ft/yr inflow model. This computation stopped in about the year 1988 ($\theta = 4.10$) due to a limit on minimum outflow from HSU 7. A comparison of some of the more significant allocations is shown on Figure 35.

Effect of assuming no development has taken place

This model shows what would be the optimum allocation had no previous developments been made and

all new facilities must be constructed to meet the projected demands. This gives an opportunity to see how far from the optimum the past policies and constraints have pushed the present development in the state. This effect can be determined by changing to zero the bounds on those variables representing present development. The results of this consideration are plotted on Figures C-10 (a) through C-10 (d). A comparison with the data from the basic model shows in general a significant reduction in the amount of surface water storage facilities that would be constructed and a substantial increase in groundwater utilization. The only exception to this is in HSU 2 where eventually the maximum storage limit was reached and the model stopped about the year 1997 ($\theta = 5.73$).

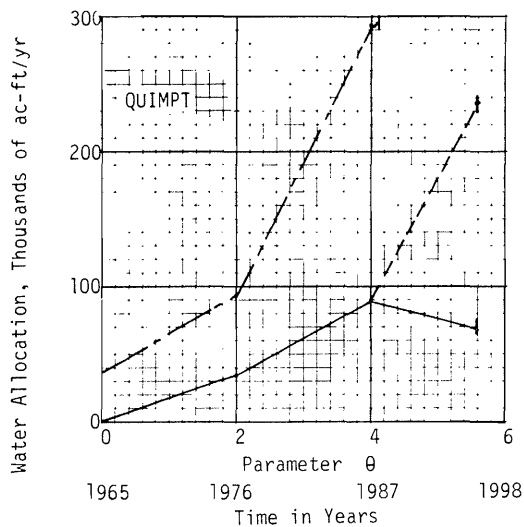
Effect of continually relieving constraints and bounds

One of the more enlightening manipulations which can be done is to run the model until it stops due to some constraint or bound, then relieve the limiting constraint and continue the parametric solution until the model stops again, and continue the process until the model cannot go further in increasing time no matter what is done. This allows a sequence of events to be generated which indicates the order that studies should be made on various development practices or policies. The results of such an investigation are shown on Figures C-11 (a) through C-11 (i). The model started with the basic model which ran to about the year 1996 ($\theta = 5.57$) where it stopped due to the limit on surface water storage in HSU 2. With this bound relieved the model ran to about the year 2000 ($\theta = 6.36$) where it stopped due to minimum limit on outflow from HSU 2. The only way to relieve this bound is to stop further development in HSU 1 and 2. With this done the model ran to about the year 2010 ($\theta = 8.14$) where it stopped due to the limit on new storage in HSU 7. With this bound relieved the model ran to about the year 2011 ($\theta = 8.37$) where it stopped due to import limits to HSU 5. Attempts to run the model further resulted in infeasible solutions.

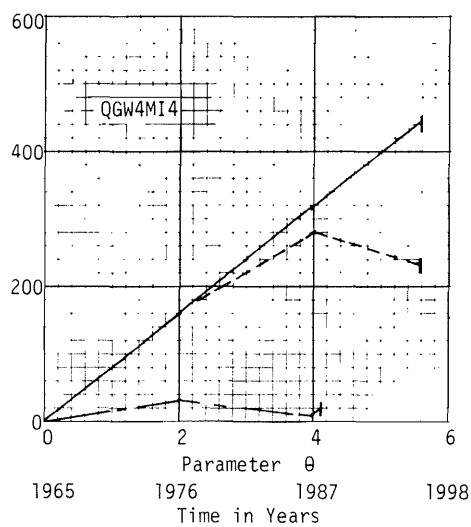
The data discussed in the preceding paragraphs should be considered as examples only and not final results. Many more investigations can be made for other policies or for any combination of policies. The additional investigations to be made using the model should include a variety of political and institutional factors since these are often just as important (or more so) than the economic factors. Such studies are needed before a thorough picture of future development for the State of Utah can be determined.

Min. Inflow GSL

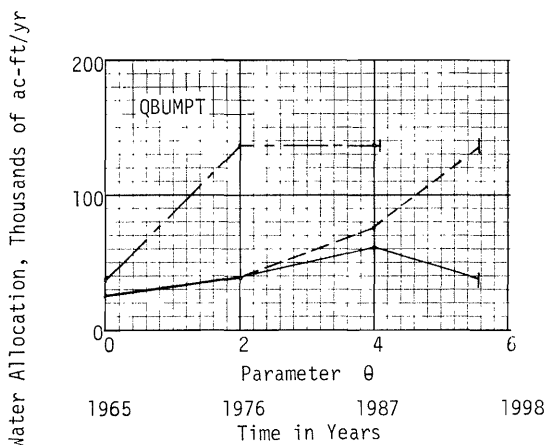
_____ 500,000 ac-ft/yr
 - - - - - 800,000 ac-ft/yr
 - . - . - 1,088,000 ac-ft/yr



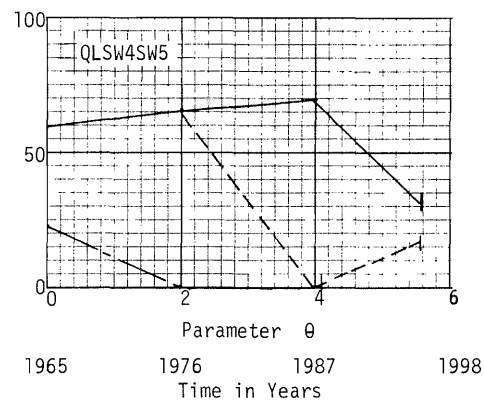
a) Ute Indian Unit Imports



b) Groundwater 4 to M&I 4



c) Bonneville Unit Imports



d) Surface Water 4 to Surface Water 5

Figure 35. Allocations as affected by time and inflow to the Great Salt Lake.

EVALUATION OF THE METHOD

Now that the results of the study have been reported, an evaluation will be made of the advantages and disadvantages, the strengths and weaknesses, the compliments and the cautions related to this method for reaching a water resources planning goal.

In general it is clear that the first objective of the study has indeed been realized. That is, a mathematical programming model with the appropriate constraints has been formulated for a least cost allocation of water within the State of Utah. The model is comprehensive and all inclusive rather than partial and all uses, all areas, all sources, and all transfers of water have been included in the analysis. On the other hand one might argue that the application of the method was too gross with the state divided into only 10 regions. Some of the study areas should have been subdivided further from hydrologic considerations alone. Certainly the model could be greatly improved by dividing the state into a network of smaller areas. Breaking the study areas into smaller geographical subunits would make it easier to utilize functional economic areas so that a determination of economic activity and water use projections would be easier to accomplish.

Similarly, the second objective was realized in the study. The linear programming model was solved with an appropriate algorithm in order to determine the optimal water allocation in the state for various sets of assumptions. While this part of the study was a good beginning, it certainly was not all inclusive and the investigation should be continued to determine the optimal allocation under many other sets of conditions.

The third objective of the study was also accomplished. The model was able to demonstrate how various operating rules, legal policies, and political and social limitations might affect the water allocation. Other such investigations would be needed before such a planning effort might be viewed as complete. Actually a planning effort should probably never be viewed as finished, since as conditions change in the future, the study should be updated to determine the effect of the new conditions.

The computer program used in this study supplies large amounts of data which must be interpreted. There is almost a danger of being "buried" in information. Much effort went into ways of condensing the important data so it could be examined and interpreted and understood. The flow diagrams and graphs are the result of this effort to

distill the important data out of voluminous computer output. This effort in data presentation could well be extended and improved.

A method was developed in this study which allows one to consider the full cost structure, rather than just part of the development costs. In fact the cost coefficients in the objective function and the appropriate cost constraints can be made just as comprehensive and complete as the resources and time available might allow. In this study, not a great deal of effort was expended in trying to precisely define the costs. In many cases the best available estimate was used, since the objective of this study was to work out a methodology of water planning rather than to carry out a specific water planning activity. As has been pointed out, one of the most useful contributions of this kind of a method is that the sensitivity analysis pinpoints those cost coefficients which are crucial and important to the solution and thus identifies those aspects of the problem that should be given more detailed and intensive attention.

One of the important considerations in favor of this kind of a water planning method is that it enables one to look forward into time with respect to the decision making process, rather than to just be concerned with present or past decisions. The method allows one to continuously change various parameters as a function of time and thereby take a look at the changing problem of the optimal water allocation as time passes.

The particular type of mathematical model used, that is the linear programming format, may or may not fit some real world situations exactly. For some situations, the linearization of the problem may so greatly distort reality as to make the results of questionable use. Thus the method must be used with judgment and caution. In this study of the State of Utah, for a rather gross examination of the water allocation problem, the linear programming format worked quite well. Some linear approximations of non-linear relationships were utilized in the method and perhaps more of these should be incorporated into any improvements made in the model. Some kinds of economic, political and social objectives have not been considered in the model and attention should be given to these in future improvements. For example, how could one work into the model the objective of stimulating the economy in lagging areas, or the objective of causing a more equitable distribution of the income in the region. Some of these objectives might

be quite important and consideration should be given to making it possible to assess the effect of such objectives on water allocation.

Considerable effort was made to work closely with the appropriate state and federal agencies who are interested in this problem, so as to make the study represent real world problems in water resource allocation. For example the hydrologic inventories and water demand projections of the Utah Division of Water Resources were used throughout the study. Only two sets of assumptions as to the growth of water demand throughout the state were used in the study. More consideration should be given to alternative growth patterns that might make certain other areas build up with respect to the demand of water at a particular time. Thus the different regions of the state might behave quite differently with respect to growth in water demand. This possibility was not adequately considered in the current application of this model.

In the development of the methodology, not enough attention was given to the effects of water quality on the cost and use of the water. Water treatment costs were included for those supplies such as municipal waste water, which are known to be of such poor quality as to require treatment. Otherwise in the model, adequate consideration was not given to the water quality problem. In future improvements of the methodology, water quality should be given more attention.

The question of water availability should be investigated more thoroughly. This might be done by changing the water available from various sources by various increments in the model. These changes could occur together in various areas or in varying amounts in different regions and the effects of such changes on the allocation of water should be tested.

It is realized, of course, that a fixed requirement for water such as used in this study, which is not dependent on water price, is unrealistic. In fact it was recognized from the beginning that a least cost allocation of the state's water supplies is not as meaningful as an allocation with net benefits as the measure of value in the objective function. The amount of water used should be dependent on the price charged for the water. However, the resources available for this project precluded giving attention to the general question of the value of water. It is realized that the methodology developed herein addresses itself to a lesser type of objective; that is, to determine the optimal allocation of water in the state so as to minimize the cost

of the water. Fortunately resources have been found to continue this research effort and the important question of the value of the water and its effect on the optimal allocation is already under study in another project.

The inadequacy of available information and a limitation on project resources have required that estimates be made of some hydrologic values and relationships used in the model as well as some cost information. These should be more accurately determined in future work by extending and completing the necessary hydrologic studies. Some of the values and relationships which were estimated in the model because better information was not available are as follows:

1. The relationship between a change in groundwater storage and the corresponding change in wetland consumptive use.
2. The portions of available water yield in a basin which are available as surface water and as groundwater in some study areas.
3. The percents of return flows which enter the surface water system and the groundwater system.
4. The percent of consumptive-use requirements met by direct use of groundwater for both wetland consumptive use and cropland consumptive use.
5. The relationship between a change in groundwater storage and the corresponding change in groundwater outflow from the study area.
6. The amount of groundwater outflow from each study area flowing into sink areas such as Great Salt Lake, Utah Lake, and Sevier Lake.
7. Perennial yield of groundwater for some study areas.
8. Cost data for some proposed storage, recharge, and water transfer projects.

Under this research effort a methodology has been developed for determining the optimal allocation of water supplies to minimize the cost of meeting given demands for water in a large and complex area. The research was done by an interdisciplinary team so as to utilize various viewpoints and skills. The writers have tried to make the method as broad in scope as possible and the suggested model was made flexible so it can be applied in planning situations other than in the State of Utah. The work done has been thoroughly documented in this report so others can follow what was done, improve upon the method, or apply the model to other areas.

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APPENDIX A

GENERAL THEORY OF THE ALLOCATION MODEL

by

James H. Milligan

The basic problem of allocating water resources from alternative sources to competing points of use is closely related to mathematical programming problems which deal with determining optimal allocations of resources to meet certain objectives. In general, allocation problems are characterized by the large number of alternatives which could satisfy the system constraints, but the problem becomes more complex when the effect of each alternative on some stated or implied objective must be evaluated in order that some best or optimal alternative can be chosen.

In this study the resources to be allocated in an optimal manner are the quantities of water available in each of the 10 study areas of the state from groundwater supplies, from local surface water supplies, from inter-basin transfers, or from transfers from the Colorado Basin to the Great Basin. The total supply system is to be managed optimally to satisfy existing and projected demands in the various study areas.

Mathematical Form of the Model

The allocation problem has been formulated mathematically as a linear programming model. Linear programming is the systems analysis tool which has come to be most closely associated with resource allocation problems and is a mathematical technique for solving the class of problems in optimization which deal with the interactions of large numbers of variables or alternative activities subject to given constraint conditions. A linear programming problem differs from the general mathematical programming problem in that the mathematical relationship used to describe the objectives and constraints must be linear or "straight-line" relationships. Mathematically the linear programming problem can be stated as follows:

Find the values of x_1, x_2, \dots, x_n which minimize (maximize) the linear objective function

$$Z = c_1x_1 + c_2x_2 + \dots + c_nx_n \quad \dots \quad (1)$$

subject to the constraints,

$$\begin{aligned} a_{11}x_1 + a_{12}x_2 + \dots + a_{1n}x_n & (\leq, =, \geq) b_1 \\ a_{21}x_1 + a_{22}x_2 + \dots + a_{2n}x_n & (\leq, =, \geq) b_2 \\ & \vdots \\ & \vdots \\ a_{m1}x_1 + a_{m2}x_2 + \dots + a_{mn}x_n & (\leq, =, \geq) b_n \end{aligned} \quad \dots \quad (2)$$

and

$$x_1 \geq 0, x_2 \geq 0, \dots, x_n \geq 0$$

where the a_{ij} , b_i , and c_j are given constants. The a_{ij} 's are coefficients which relate a unit of activity to the amount of resource use by that activity. The b_i 's represent the resource demands and availabilities, and the c_j 's represent the unit costs associated with each alternative activity. The x_j 's are referred to as decision variables. Equation 1 is referred to as the objective function and Equations 2 are referred to as the system of constraints. The sign associated with each individual constraint may be less than or equal to (\leq), equality ($=$), or greater than or equal to (\geq) as the individual case may be.

Physical interpretation of the objective function and of the system of constraints in the context of water resources allocation is already suggested by the interpretation of the coefficients and right-hand-side elements above. The objective function describes the economic relationships of the area being modeled. The value of the objective function might be the total cost of all of the alternative water activities considered in the solution, or it might represent the total net benefits, depending upon whether the problem is formulated as a cost minimization problem or a net benefit maximization problem. The system of constraints defines the technical relationships

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and

$$x_1 \geq 0, x_2 \geq 0, \dots, x_n \geq 0$$

where the a_{ij} , b_i , and c_j are given constants. The a_{ij} 's are coefficients which relate a unit of activity to the amount of resource use by that activity. The b_i 's represent the resource demands and availabilities, and the c_j 's represent the unit costs associated with each alternative activity. The x_j 's are referred to as decision variables. Equation 1 is referred to as the objective function and Equations 2 are referred to as the system of constraints. The sign associated with each individual constraint may be less than or equal to (\leq), equality ($=$), or greater than or equal to (\geq) as the individual case may be.

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the area being modeled. For example, a group of constraints may define the condition of hydrologic continuity within the model, whereas another group of constraints might define the relationships between sources of water supply and areas of demand, including return flows and wastes that might occur due to the allocation from supply to demand. Still other constraints might describe the legal limitations on availability of a certain water supply, for example. Thus, the constraint system is the part of the model wherein technical or structural relationships of the model are represented and the objective function is the part of the model wherein the economic relationships, or measure of accomplishment of objectives, are spelled out.

Obtaining Optimal Solutions

In the terminology of linear programming any set of x_j 's which satisfy the constraints and the non-negativity conditions is a *feasible solution*. A feasible solution which also minimizes or maximizes the value of the objective function is called an *optimal feasible solution* (Loomba, 1964). The intersection of the linear constraints forms a convex set, and only points within this set can satisfy the constraint conditions and become feasible solutions to the linear programming problem. The extreme points of the convex set of feasible solutions are defined as *basic feasible solutions*. The theorems of linear programming state that if an optimal solution exists, at least one of the extreme point solutions, or basic feasible solutions, will be the optimal solution. In some cases where the optimal solution is not unique, points between two extreme point solutions are also optimal.

Techniques used for solving the linear programming problem to obtain optimal solutions are iterative, and the most efficient method of solution is called the simplex algorithm. This algorithm is an algebraic iterative procedure which will solve, exactly, any properly formulated linear programming problem in a finite number of steps. Computer routines for solving linear programming problems use the simplex algorithm or some modification of it. The simplex procedure assures that each iteration yields a better (or at least not a worse) solution than the preceding iteration. Therefore, the number of iterations required to obtain the optimal solution is generally small compared to the number of existing basic feasible solutions. In simple terms the solution process can be described as a method of moving along the edge of the region of feasible solutions from one corner to the adjacent corner which will give the most improvement in the value of the objective function. At each corner the method indicates whether or not that corner is optimal, and if not, which corner will be the next one examined.

If at any stage in the solution process a point is examined which has an edge leading to infinity (an unbounded convex set) and if the objective function can be improved by moving along this edge, then an unbounded solution is indicated.

When the linear programming problem is formulated using inequality constraints the method of solution requires the addition of slack variables to the constraints to convert the inequalities to equations so the problem is treated as a system of linear equations in the solution process. These slack variables take on a physical interpretation in applied problems. Their values represent the amount of resource being allocated which is surplus or redundant to the optimal quantities indicated by the final solution.

For a more detailed discussion of the theory of linear programming and of the modified simplex procedures used in computer solutions, see Hadley (1962), Gass (1964), or Hillier and Lieberman (1967).

Interpretation of Solutions

Solutions to linear programming problems consist of several parts including the optimal value of the objective function, the optimal activity levels of the real and slack variables, and the solution of the dual to the linear programming problem. The optimal value of the objective function is useful primarily for comparing one solution with another. The optimal activity levels of the real and slack variables show which activities are included in the optimal solution as well as the optimal quantities associated with those activities. For example, the solution would indicate which inter-basin transfers of water are in the optimal solution, or which groundwater reservoirs should be pumped. The solution would also give the optimal number of acre-feet for each of these activities.

The essence of the economic interpretation of the dualism property of linear programming lies in the concept that resource allocation and pricing are two aspects of the same problem (Dorfman, Samuelson, and Solow, 1958).

The formulation of a typical linear programming problem was given as Equations 1 and 2. The linear programming problem formulated in this manner is known as the *primal* problem. The *dual* problem corresponding to the primal problem is formulated from the primal problem in the following manner:

1. Constraints in the primal problem are re-structured to contain only inequalities in the same sense.
2. Rows and columns of the constraint coefficients are transposed.
3. The right-hand-side values of the constraints are transposed with the objective coefficients.
4. The sense of the inequality signs in the constraints is changed.
5. The objective function is maximized instead of minimized (or minimized instead of maximized, as the case may be).

According to this procedure the dual problem may be stated mathematically as: Find $w_i \geq 0$ ($i = 1, 2, \dots, m$)

in order to maximize (minimize)

$$Z' = b_1 w_1 + b_2 w_2 + \dots + b_m w_m \dots \dots (3)$$

subject to the constraints,

$$\begin{aligned} a_{11} w_1 + a_{21} w_2 + \dots + a_{m1} w_m & (\leq, \geq) c_1 \\ a_{12} w_1 + a_{22} w_2 + \dots + a_{m2} w_m & (\leq, \geq) c_2 \\ & \vdots \\ & \vdots \\ a_{1n} w_1 + a_{2n} w_2 + \dots + a_{mn} w_m & (\leq, \geq) c_n \end{aligned} \dots \dots (4)$$

In this formulation there is one dual constraint for each primal variable, and one dual variable for each primal constraint. Dorfman, Samuelson, and Solow (1958) summarize the relationship between the primal problem and the corresponding dual as follows:

1. The dual has one variable for each constraint in the original problem.
2. The dual has as many constraints as there are variables in the primal.
3. The dual of the minimizing problem is a maximizing problem, and vice versa.
4. The coefficients of the objective function of the primal problem appear as the constant terms of the dual constraints, and the constant terms of the primal constraints are the objective coefficients in the dual.
5. The coefficients of a single variable in the primal constraints become the coefficients of a single constraint in the dual. Thus each column of coefficients in the primal becomes a row of coefficients in the dual.
6. The sense of the inequalities in the dual is the reverse of the sense of the inequalities in the primal, except that non-negativity conditions apply to the dual variables in the same sense that they apply to the primal variables.

The optimal solution to the dual problem is obtained as a by-product of the optimal solution of the primal problem and provides an interesting and useful economic interpretation of the primal problem. The optimal values of the dual variables (w_i 's) are often referred to as *shadow prices* or the marginal costs of introducing marginal amounts of the non-optimal slack variables from the primal problem into the optimal solution. Thus the optimal values of the dual variables (or shadow prices) indicate the rate at which costs increase or decrease for a corresponding increase or decrease in the amount of resource given by the right-hand-side value of the primal constraint. The range of values over which the right-hand-side value can vary for a given shadow price to be valid is given in the sensitivity analysis and is the range of values of that right-hand-side value for which the

original optimal basis is not changed. Thus the optimal value of a dual variable w_i may be interpreted as the marginal cost of resource i . However, shadow prices merely reflect the marginal cost of the resource within the context of the model and in no way should they be considered as the *actual* costs of the resource.

Post-Optimal Analysis

Analysis of the linear programming problem after an optimal solution has been achieved is referred to as post-optimal analysis and consists primarily of two possible phases of analysis: sensitivity analysis and parametric analysis. Practical problems formulated as linear programming problems are seldom completely "solved" by the optimal solution produced by the simplex procedure. The coefficients of the model (c_j 's, a_{ij} 's, and b_i 's) are seldom known with the desired degree of certainty. Also, the linear relationships assumed for a given problem formulation may not hold in the range indicated by the model solution. Therefore, it is usually desirable to carry out some sort of sensitivity analysis to determine the effect on the optimal solution of changing certain coefficients or constants to other possible values without having to re-solve the problem. This provision is provided for on most computer routines for solving linear programming problems. If the sensitivity analysis indicates the optimal solution may change for a small change in the constants or coefficients, then special care should be taken in checking the values of these coefficients or constants. It is not always necessary to re-solve the problem from the beginning each time a minor change is made in the model coefficients or constants. Given the previous optimal solution, it is usually possible through the use of sensitivity analysis to determine whether the same basis is optimal.

Parametric analysis is a procedure for generating new optimal solutions from an original optimal solution while allowing one or more parameters (constants or coefficients) to vary. The parametric procedure allows an evaluation of the optimal solution as one or more parameters are allowed to vary over a specified range of values. When using parametric analysis a "change vector" is specified which indicates the parameters which are to be varied and the increment by which each will be varied. Then the parameters are continuously changed over a specified range. Thus, if coefficients in the objective function are to be changed the cost coefficients are changed according to the relationship

$$COST_2 = COST_1 + \alpha\theta$$

in which

- $COST_2$ is the new value of the cost vector
- $COST_1$ is the original value of the cost vector
- α is the change vector
- θ is the parameter interval at which new

solutions are to be obtained up to some value of θ_{\max}

The function of the parametric procedure is to maintain optimality and feasibility as the problem continues to change. Solutions to this continuously changing problem can usually be obtained at intervals of the parameter values specified by θ , at required basis changes, or at both. Parametric analysis greatly facilitates examination of changes in water demands, for example.

Computer Facilities and Routine

The linear programming problems formulated for this study were solved using an IBM 360/44 digital

computer and an advanced, large scale linear programming routine provided by IBM for the 360 machines. The linear programming routine is contained in a mathematical programming package identified as MPS/360. The linear programming procedures of MPS/360 use the bounded variable/product form of the inverse/revised simplex method. In the product form, the inverse matrix, from which basic feasible solutions are obtained, is represented by the product of a sequence of $m \times n$ matrices in which only one column of each matrix differs from a column of the unit matrix. This particular form simplifies the iterative procedure in obtaining an optimal solution. Details of using the MPS/360 package for obtaining linear programming solutions can be found in the MPS/360 Users Manual.

APPENDIX B
COMPUTER PRINT-OUT OF THE SOLUTION OF THE BASIC ALLOCATION MODEL

Table B-1. Print-out of the program control cards.

```

CONTROL PROGRAM COMPILER

0001          PROGRAM
0002          INITIALZ
0061          MOVE(XDATA,'MODEL')
0062          MOVE(XPBNAM,'PBFIL')
0063          CONVEP('CHECK','SUMMARY')
0064          BCROUT
0065          SET('BOUND','ROUNDX')
0066          MOVE(XORJ,'COST')
0067          MOVE(XPHS,'RHS')
0068          PRIMAL
0069          SOLUTION
0070          RANGE
0071          XPARAM=0.0
0072          XPARAMAX=10.0
0073          XPARDEL=2.0
0074          MOVE(XHCOL,'RHS2')
0075          PARARHS
0076          SOLUTION
0077          RANGE
0078          EXIT
0079          PEND
    
```

Table B-2. Print-out of the program data cards.

```

EXECUTOR .      MPS/360 V2-M5

NAME            MODEL
ROWS
N COST
E AVAILSW1      G AGREQ4          E GRID4           E WHRF9
F AVAILSW2      G AGREQ5          E LSW4ST4         E WHRF0
E AVAILSW3      G AGREQ6          E GRID5           E AGRFSW1
E AVAILSW4      G AGREQ7          E LSW5ST5         E AGRFSW2
E AVAILSW5      G AGREQ8          E GRID6           E AGRFSW3
E AVAILSW6      G AGREQ9          E LSW6ST6         E AGRFSW4
E AVAILSW7      G AGREQ0          E GRID7           E AGRFSW5
E AVAILSW8      E TLSW1AG1        E LSW7ST7         E AGRFSW6
E AVAILSW9      E TLSW2AG2        E GRID8           E AGRFSW7
E AVAILSW0      E TLSW3AG3        E LSW8ST8         E AGRFSW8
E LSWU7          E TLSW4AG4        E GRID9           E AGRFSW9
E LSWU8          E TLSW5AG5        E LSW9ST9         E AGRFSW0
E AVAILGW1      E TLSW6AG6        E GRID0           E AGRFGW1
E AVAILGN2      E TLSW7AG7        E LSW0ST0         E AGRFGW2
E AVAILGW3      E TLSW8AG8        E TST1            E AGRFGW3
E AVAILGW4      E TLSW9AG9        E TST2            E AGRFGW4
E AVAILGW5      E TLSW0AG0        E TST3            E AGRFGW5
E AVAILGW6      E TGW1AG1         E TST4            E AGRFGW6
E AVAILGW7      E TGW2AG2         E TST5            E AGRFGW7
E AVAILGW0      E TGW3AG3         E TST6            E AGRFGW0
G MIREQ1        E TGW4AG4         E TST7            E FGWAVWL1
G MIREQ2        E TGW5AG5         E TST8            E FGWAVWL2
G MIREQ3        E TGW6AG6         E TST9            E FGWAVWL3
G MIREQ4        E TGW7AG7         E TST0            E FGWAVWL4
G MIREQ5        E TGW0AG0         E EVLSW1          E FGWAVWL5
G MIREQ6        E AGEXC3          E EVLSW2          E FGWAVWL6
G MIREQ7        E AGEXC4          E EVLSW3          E FGWAVWL7
G MIREQ8        E AGEXC8          E EVLSW4          E FGWAVWL0
G MIREQ9        E WLREQ1          E EVLSW5          L GWRC1
G MIREQ0        E WLREQ2          E EVLSW6          L GWRC2
E TLSW1M11      E WLREQ3          E EVLSW7          L GWRC3
E TLSW2M12      E WLREQ4          E EVLSW8          L GWRC4
F TLSW3M13      E WLREQ5          E EVLSW9          L GWRCU4
E TLSW4M14      E WLREQ6          E EVLSW0          L GWRC5
E TLSW5M15      E WLREQ7          E EVGW1           L GWRC6
E TLSW6M16      E WLREQ8          E EVGW2           L GWRC7
E TLSW7M17      E WLREQ9          E EVGW3           L GWRC0
E TLSW8M18      E WLREQ0          E EVGW4           L GWRC0
E TLSW9M19      E DREQ1           E EVGW5           E BUMPT
E TLSW0M10      E DREQ2           E EVGW6           E UIMPT
E TGW1M11       E DREQ3           E EVGW7           E SAMPT
E TGW2M12       E DREQ4           E EVGW0           E TLSW3SW4
E TGW3M13       E DREQ5           E EV2ST2          E TLSW0SW6
E TGW4M14       E DREQ6           E EV2              G INFLOGSL
E TGW5M15       E DREQ7           E EV4ST4          G CROUT
E TGW6M16       E DREQ8           E EV4              G CROUT
E TGW7M17       E DREQ9           E WHRF1
E TGW0M10       E DREQ0           E WHRF2
G AGREQ1        E GRID1           E WHRF3
G AGREQ2        E LSW1ST1         E WHRF4
G AGREQ3        E GRID2           E WHRF5
E LSW2ST2       E WHRF6
E GRID3         E WHRF7
E LSW3ST3       E WHRF8
    
```

Table B-2. Continued.

COLUMNS									
QBULSW4	COST	7.00000	RUMPT	1.00000	PLSW6AG6	TLW6AG6	1.00000		
QBULSW4	AVAILSW4	1.00000	DREQ4	1.00000	QLSW6AG6	COST	5.00000	TLW6AG6	1.00000
QBULSW5	COST	10.00000	RUMPT	1.25000	FLSW6AG6	AVAILSW6	1.00000	AGREQ6	1.00000
QBULSW5	AVAILSW5	1.00000	DREQ5	1.00000	PLSW6AG6	TLW6AG6	1.00000	AGRFW6	.40000
QBUMPT	BUMPT	1.00000	AVAILSW7	1.00000	RLSW6AG6	AGRFW6	.14470	DREQ6	1.00000
QBUMPT	DREQ7	1.00000			PLSW6M16	TLW6M16	1.00000		
QUILSW3	COST	10.00000	UIMPT	1.00000	QLSW6M16	COST	36.00000	TLW6M16	1.00000
QUILSW3	AVAILSW3	1.00000	DREQ3	1.00000	FLSW6M16	AVAILSW6	1.00000	MIREQ6	1.00000
QUILSW4	COST	10.00000	UIMPT	1.00000	RLSW6M16	TLW6M16	1.00000	WRF6	.69700
QUILSW4	AVAILSW4	1.00000	DREQ4	1.00000	PLSW6M16	DREQ6	1.00000		
QUILSW5	COST	13.00000	UIMPT	1.25000	QLSW6M16	AVAILSW6	1.00000	WLPEQ6	1.00000
QUILSW5	AVAILSW5	1.00000	DREQ5	1.00000	PLSW7AG7	TLW7AG7	1.00000		
QUIMPT	UIMPT	1.00000	AVAILSW7	1.00000	QLSW7AG7	COST	5.00000	TLW7AG7	1.00000
QUIMPT	DREQ7	1.00000			FLSW7AG7	LSWU7	1.00000	AGRFQ7	1.00000
QSALSW4	COST	8.00000	SAMPT	1.00000	RLSW7AG7	TLW7AG7	1.00000	AGRFW7	.47990
QSALSW4	AVAILSW4	1.00000	DREQ4	1.00000	PLSW7AG7	AGRFW7	.15000	DREQ7	1.00000
QSALSW5	COST	4.00000	SAMPT	1.00000	QLSW7M17	TLW7M17	1.00000		
QSALSW5	AVAILSW5	1.00000	DREQ5	1.00000	PLSW7M17	COST	36.00000	TLW7M17	1.00000
QSAMPT	SAMPT	1.00000	AVAILSW8	1.00000	RLSW7M17	LSWU7	1.00000	MIREQ7	1.00000
QSAMPT	DREQ8	1.00000			PLSW7M17	TLW7M17	1.00000	WRF7	.65000
PLSW1AG1	TLW1AG1	1.00000			QLSW7M17	DREQ7	1.00000		
QLSW1AG1	COST	5.00000	TLW1AG1	1.00000	RLSW7L7	LSWU7	1.00000	MIREQ7	1.00000
FLSW1AG1	AVAILSW1	1.00000	AGREQ1	1.00000	Q7LSW7	LSWU7	1.00000	AVAILSW7	1.00000
FLSW1AG1	TLW1AG1	1.00000	AGRFW1	.40000	PLSW7S4	AVAILSW4	1.00000	AVAILSW7	1.00000
FLSW1AG1	AGRFW1	.12420	DREQ1	1.00000	PLSW7S4	DREQ7	1.00000	DREQ4	1.00000
PLSW1M11	TLW1M11	1.00000			PLSW8AG8	COST	.00010	TLW8AG8	1.00000
QLSW1M11	COST	31.00000	TLW1M11	1.00000	QLSW8AG8	COST	5.00000	TLW8AG8	1.00000
RLSW1M11	AVAILSW1	1.00000	MIREQ1	1.00000	RLSW8AG8	LSWU8	1.00000	AGREQ8	1.00000
FLSW1M11	TLW1M11	1.00000	WRF1	.70000	RLSW8AG8	TLW8AG8	1.00000	AGRFW8	.52500
FLSW1M11	DREQ1	1.00000			RLSW8AG8	DREQ8	1.00000		
QLSW1M11	AVAILSW1	1.00000	WLREQ1	1.00000	PLSW8M18	TLW8M18	1.00000		
PLSW1M14	AVAILSW1	1.00000	MIREQ4	1.00000	QLSW8M18	COST	51.00000	TLW8M18	1.00000
PLSW1M14	WRF4	.68890	DREQ1	1.00000	PLSW8M18	LSWU8	1.00000	MIREQ8	1.00000
PLSW2AG2	TLW2AG2	1.00000			PLSW8M18	TLW8M18	1.00000	WRF8	.30000
QLSW2AG2	COST	5.00000	TLW2AG2	1.00000	QLSW8M18	DREQ8	1.00000		
FLSW2AG2	AVAILSW2	1.00000	AGREQ2	1.00000	QLSW8M18	LSWU8	1.00000	WLREQ8	1.00000
FLSW2AG2	TLW2AG2	1.00000	AGRFW2	.50770	Q8LSW8	LSWU8	1.00000	AVAILSW8	1.00000
FLSW2AG2	AGRFW2	.15000	DREQ2	1.00000	PLSW8AG5	AVAILSW8	1.00000	AGREQ5	1.00000
PLSW2M12	TLW2M12	1.00000			PLSW8AG5	AGRFW5	.52500	AGRFW5	.15000
QLSW2M12	COST	32.00000	TLW2M12	1.00000	PLSW8AG5	DREQ8	1.00000		
RLSW2M12	AVAILSW2	1.00000	MIREQ2	1.00000	Q8RLSW8	AVAILSW8	1.00000	AGEXC8	1.00000
FLSW2M12	TLW2M12	1.00000	WRF2	.66000	PLSW9AG9	TLW9AG9	1.00000		
FLSW2M12	DREQ2	1.00000			QLSW9AG9	COST	5.00000	TLW9AG9	1.00000
QLSW2M12	AVAILSW2	1.00000	WLREQ2	1.00000	PLSW9AG9	AVAILSW9	1.00000	AGREQ9	1.00000
QLSW2S11	COST	4.00000	AVAILSW1	1.00000	RLSW9AG9	TLW9AG9	1.00000	AGRFW9	.80000
QLSW2S11	AVAILSW2	1.00000	DREQ2	1.00000	PLSW9AG9	DREQ9	1.00000		
PLSW2S11	DREQ1	1.00000			PLSW9M19	TLW9M19	1.00000		
QLSW2S13	COST	4.00000	AVAILSW2	1.00000	QLSW9M19	COST	43.00000	TLW9M19	1.00000
QLSW2S13	AVAILSW3	1.00000	DREQ2	1.00000	PLSW9M19	AVAILSW9	1.00000	MIREQ9	1.00000
QLSW2S13	DREQ3	1.00000			RLSW9M19	TLW9M19	1.00000	WRF9	.25000
PLSW3AG3	TLW3AG3	1.00000			RLSW9M19	DREQ9	1.00000		
QLSW3AG3	COST	6.00000	TLW3AG3	1.00000	QLSW9M19	AVAILSW9	1.00000	WLREQ9	1.00000
RLSW3AG3	AVAILSW3	1.00000	AGREQ3	1.00000	PLSW0AG0	TLW0AG0	1.00000		
RLSW3AG3	TLW3AG3	1.00000	AGRFW3	.48330	QLSW0AG0	COST	5.00000	TLW0AG0	1.00000
RLSW3AG3	AGRFW3	.15000	DREQ3	1.00000	RLSW0AG0	AVAILSW0	1.00000	AGREQ0	1.00000
PLSW3M13	TLW3M13	1.00000			RLSW0AG0	TLW0AG0	1.00000	AGRFW0	.50000
QLSW3M13	COST	43.00000	TLW3M13	1.00000	PLSW0M10	TLW0M10	1.00000		
RLSW3M13	AVAILSW3	1.00000	MIREQ3	1.00000	QLSW0M10	COST	43.00000	TLW0M10	1.00000
RLSW3M13	TLW3M13	1.00000	WRF3	.43660	RLSW0M10	AVAILSW0	1.00000	MIREQ0	1.00000
QLSW3M13	DREQ3	1.00000			PLSW0M10	TLW0M10	1.00000	WRF0	.30000
PLSW3AG2	AGRFW2	.50770	AGRFW2	.15000	QLSW0M10	DREQ0	1.00000		
PLSW3AG2	DREQ3	1.00000			QLSW0M10	AVAILSW0	1.00000	WLREQ0	1.00000
PLSW3S14	TLW3S14	1.00000			PLSW0S16	TLW0S16	1.00000		
QLSW3S14	COST	4.00000	TLW3S14	1.00000	RLSW0S16	COST	4.00000	TLW0S16	1.00000
PLSW3S14	DREQ3	1.00000	TLW3S14	1.00000	PLSW0S16	AVAILSW6	1.00000	AVAILSW6	1.00000
RLSW3S14	AVAILSW3	1.00000	AVAILSW4	1.00000	RLSW0S16	DREQ0	1.00000	TLW0S16	1.00000
PLSW3S14	DREQ4	1.00000			PLSW0S16	DREQ6	1.00000		
QAG3LSW4	AVAILSW3	1.00000	AGEXC3	1.00000	PGW1AG1	TGW1AG1	1.00000		
PLSW4AG4	TLW4AG4	1.00000			QGW1AG1	COST	4.90000	TGW1AG1	1.00000
QLSW4AG4	COST	6.00000	TLW4AG4	1.00000	RGW1AG1	AVAILGW1	1.00000	AGREQ1	1.00000
PLSW4AG4	AVAILSW4	1.00000	AGREQ4	1.00000	RGW1AG1	TGW1AG1	1.00000	AGRFW1	.40000
PLSW4AG4	TLW4AG4	1.00000	AGRFW4	.46090	PGW1M11	TGW1M11	1.00000		
PLSW4AG4	AGRFW4	.15000	DREQ4	1.00000	QGW1M11	COST	34.25000	TGW1M11	1.00000
PLSW4M14	TLW4M14	1.00000			RGW1M11	AVAILGW1	1.00000	MIREQ1	1.00000
QLSW4M14	COST	43.00000	TLW4M14	1.00000	RGW1M11	TGW1M11	1.00000	WRF1	.70000
RLSW4M14	AVAILSW4	1.00000	MIREQ4	1.00000	QFGW1W11	AVAILGW1	1.00000	WLREQ1	1.00000
RLSW4M14	TLW4M14	1.00000	WRF4	.68890	QFGW1W11	FGWAVW1	1.00000		
FLSW4M14	DREQ4	1.00000			CCGW1W11	COST	2.40000	AVAILGW1	1.00000
QLSW4M14	AVAILSW4	1.00000	WLREQ4	1.00000	CCGW1W11	WLRFQ1	1.00000		
QLSW4S15	COST	5.00000	AVAILSW4	1.25000	PGW2AG2	TGW2AG2	1.00000		
QLSW4S15	AVAILSW5	1.00000	DREQ4	1.25000	QGW2AG2	COST	5.60000	TGW2AG2	1.00000
QLSW4S15	DREQ5	1.00000			RGW2AG2	AVAILGW2	1.00000	AGREQ2	1.00000
QAG4LSW4	AVAILSW4	1.00000	AGEXC4	1.00000	RGW2AG2	TGW2AG2	1.00000	AGRFW2	.50770
PLSW5AG5	TLW5AG5	1.00000			PGW2AG2	AGRFW2	.15000		
QLSW5AG5	COST	5.00000	TLW5AG5	1.00000	RGW2M12	TGW2M12	1.00000		
RLSW5AG5	AVAILSW5	1.00000	AGREQ5	1.00000	QGW2M12	COST	34.25000	TGW2M12	1.00000
RLSW5AG5	TLW5AG5	1.00000	AGRFW5	.52500	RGW2M12	AVAILGW2	1.00000	MIREQ2	1.00000
RLSW5AG5	AGRFW5	.15000	DREQ5	1.00000	RGW2M12	TGW2M12	1.00000	WRF2	.66000
PLSW5M15	TLW5M15	1.00000			QFGW2W2	FGWAVW2	1.00000	AVAILGW2	1.00000
QLSW5M15	COST	36.00000	TLW5M15	1.00000	QFGW2W2	WLREQ2	1.00000		
PLSW5M15	AVAILSW5	1.00000	MIREQ5	1.00000	CCGW2W2	COST	3.10000	AVAILGW2	1.00000
FLSW5M15	TLW5M15	1.00000	WRF5	.45880	CCGW2W2	WLREQ2	1.00000		
RLSW5M15	DREQ5	1.00000			PGW3AG3	TGW3AG3	1.00000		
QLSW5M15	AVAILSW5	1.00000	WLREQ5	1.00000	QGW3AG3	COST	6.70000	TGW3AG3	1.00000
QLSW5S16	COST	4.00000	AVAILSW5	1.00000	RGW3AG3	AVAILGW3	1.00000	AGREQ3	1.00000
QLSW5S16	AVAILSW6	1.00000	DREQ5	1.00000	PGW3AG3	TGW3AG3	1.00000	AGRFW3	.49130
QLSW5S16	DREQ6	1.00000			RGW3AG3	AGRFW3	.15000		
PLSW5AG9	AVAILSW5	1.00000	AGREQ9	1.00000	PGW3M13	TGW3M13	1.00000		
PLSW5AG9	AGRFW5	.80000	DREQ5	1.00000	QGW3M13	COST	41.65000	TGW3M13	1.00000

Table B-2. Continued.

RGW3M13	AVAILGW3	1.00000	MIREQ3	1.00000	QLSW3ST3	COST	16.30000	TST3	1.00000
RGW3M13	TGW3M13	1.00000	WRRF3	.43660	RLSW3ST3	LSW3ST3	1.00000	EVLWS3	.02300
QFGW3M13	AVAILGW3	1.00000	WLREQ3	1.00000	RLSW3ST3	EVGW3	.02300	TST3	1.00000
QFGW3M13	FGHAWL3	1.00000			QDREQ4	DREQ4	1.00000	GRID4	1.00000
QCGW3M13	COST	3.70000	AVAILGW3	1.00000	SET4	*MARKER*		*SEPORG*	
QCGW3M13	WRRF3	1.00000			D41	GRID4	382.00000		
QAG3M13	AVAILGW3	1.00000	AGEXC3	1.00000	D42	GRID4	66.00000	LSW4ST4	130.00000
PGW4AG4	TGW4AG4	1.00000			D43	GRID4	84.00000	LSW4ST4	340.00000
QGW4AG4	COST	7.70000	TGW4AG4	1.00000	D44	GRID4	84.00000	LSW4ST4	710.00000
RGW4AG4	AVAILGW4	1.00000	AGREQ4	1.00000	ENDSET4	*MARKER*		*SEPEND*	
RGW4AG4	TGW4AG4	1.00000	AGRFSW4	.46090	PLSW4ST4	TST4	1.00000		
RGW4AG4	AGPFGW4	.15000			QLSW4ST4	COST	13.00000	TST4	1.00000
PGW4M14	TGW4M14	1.00000			RLSW4ST4	LSW4ST4	1.00000	EV4ST4	1.00000
QGW4M14	COST	61.65000	TGW4M14	1.00000	RLSW4ST4	TST4	1.00000		
RGW4M14	AVAILGW4	1.00000	MIREQ4	1.00000	QDREQ5	DREQ5	1.00000	GRID5	1.00000
FGW4M14	TGW4M14	1.00000	WRRF4	.58890	SET5	*MARKER*		*SEPORG*	
QFGW4M14	AVAILGW4	1.00000	WLREQ4	1.00000	D51	GRID5	262.00000		
QFGW4M14	FGHAWL4	1.00000			D52	GRID5	71.00000	LSW5ST5	220.00000
QCGW4M14	COST	4.70000	AVAILGW4	1.00000	D53	GRID5	63.00000	LSW5ST5	430.00000
QCGW4M14	WRRF4	1.00000			D54	GRID5	62.00000	LSW5ST5	510.00000
QAG4M14	AVAILGW4	1.00000	AGEXC4	1.00000	ENDSET5	*MARKER*		*SEPEND*	
PGW5AG5	TGW5AG5	1.00000			PLSW5ST5	TST5	1.00000		
QGW5AG5	COST	5.80000	TGW5AG5	1.00000	QLSW5ST5	COST	8.60000	TST5	1.00000
RGW5AG5	AVAILGW5	1.00000	ACREQ5	1.00000	LSW5ST5	LSW5ST5	1.00000	EVLWS5	.09300
RGW5AG5	TGW5AG5	1.00000	AGRFSW5	.52500	RLSW5ST5	EVGW5	.03100	TST5	1.00000
RGW5AG5	AGPFGW5	.15000			QDREQ6	DREQ6	1.00000	GRID6	1.00000
PGW5M15	TGW5M15	1.00000			SET6	*MARKER*		*SEPORG*	
QGW5M15	COST	34.25000	TGW5M15	1.00000	D61	GRID6	48.00000		
RGW5M15	AVAILGW5	1.00000	MIREQ5	1.00000	D62	GRID6	16.00000	LSW6ST6	26.00000
RGW5M15	TGW5M15	1.00000	WRRF5	.45880	D63	GRID6	12.00000	LSW6ST6	38.00000
QFGW5M15	AVAILGW5	1.00000	WLREQ5	1.00000	D64	GRID6	12.00000	LSW6ST6	94.00000
QFGW5M15	FGHAWL5	1.00000			ENDSET6	*MARKER*		*SEPEND*	
QCGW5M15	COST	3.30000	AVAILGW5	1.00000	PLSW6ST6	TST6	1.00000		
QCGW5M15	WRRF5	1.00000			QLSW6ST6	COST	14.00000	TST6	1.00000
PGW6AG6	TGW6AG6	1.00000			RLSW6ST6	LSW6ST6	1.00000	EVLWS6	.25250
QGW6AG6	COST	6.40000	TGW6AG6	1.00000	RLSW6ST6	EVGW6	.01750	TST6	1.00000
RGW6AG6	AVAILGW6	1.00000	AGREQ6	1.00000	QDREQ7	DREQ7	1.00000	GRID7	1.00000
RGW6AG6	TGW6AG6	1.00000	AGRFSW6	.40000	SET7	*MARKER*		*SEPORG*	
RGW6AG6	AGPFGW6	.14470			D71	GRID7	870.00000		
PGW6M16	TGW6M16	1.00000			D72	GRID7	185.00000	LSW7ST7	320.00000
QGW6M16	COST	34.25000	TGW6M16	1.00000	D73	GRID7	198.00000	LSW7ST7	600.00000
RGW6M16	AVAILGW6	1.00000	MIREQ6	1.00000	D74	GRID7	198.00000	LSW7ST7	1280.00000
RGW6M16	TGW6M16	1.00000	WRRF6	.69700	ENDSET7	*MARKER*		*SEPEND*	
QFGW6M16	AVAILGW6	1.00000	WLREQ6	1.00000	PLSW7ST7	TST7	1.00000		
QFGW6M16	FGHAWL6	1.00000			QLSW7ST7	COST	10.80000	TST7	1.00000
QCGW6M16	COST	3.00000	AVAILGW6	1.00000	RLSW7ST7	LSW7ST7	1.00000	EVLWS7	.02800
QCGW6M16	WRRF6	1.00000			RLSW7ST7	TST7	1.00000		
PGW7AG7	TGW7AG7	1.00000			QDREQ8	DREQ8	1.00000	GRID8	1.00000
QGW7AG7	COST	4.60000	TGW7AG7	1.00000	SET8	*MARKER*		*SEPORG*	
RGW7AG7	AVAILGW7	1.00000	AGREQ7	1.00000	D81	GRID8	394.00000		
RGW7AG7	TGW7AG7	1.00000	AGRFSW7	.47880	D82	GRID8	126.00000	LSW8ST8	200.00000
RGW7AG7	AGPFGW7	.15000			D83	GRID8	98.00000	LSW8ST8	300.00000
PGW7M17	TGW7M17	1.00000			D84	GRID8	97.00000	LSW8ST8	710.00000
QGW7M17	COST	34.25000	TGW7M17	1.00000	ENDSET8	*MARKER*		*SEPEND*	
RGW7M17	AVAILGW7	1.00000	MIREQ7	1.00000	PLSW8ST8	TST8	1.00000		
RGW7M17	TGW7M17	1.00000	WRRF7	.65000	QLSW8ST8	COST	7.20000	TST8	1.00000
QFGW7M17	AVAILGW7	1.00000	WLREQ7	1.00000	RLSW8ST8	LSW8ST8	1.00000	EVLWS8	.04500
QFGW7M17	FGHAWL7	1.00000			RLSW8ST8	TST8	1.00000		
QCGW7M17	COST	2.10000	AVAILGW7	1.00000	QDREQ9	DREQ9	1.00000	GRID9	1.00000
QCGW7M17	WRRF7	1.00000			SET9	*MARKER*		*SEPORG*	
PGW8AG8	TGW8AG8	1.00000			D91	GRID9	272.00000		
CGW8AG8	COST	4.80000	TGW8AG8	1.00000	D92	GRID9	72.00000	LSW9ST9	120.00000
RGW8AG8	AVAILGW8	1.00000	AGREQ8	1.00000	D93	GRID9	65.00000	LSW9ST9	150.00000
RGW8AG8	TGW8AG8	1.00000	AGRFSW8	.50000	D94	GRID9	64.00000	LSW9ST9	290.00000
PGW8M18	TGW8M18	1.00000			ENDSET9	*MARKER*		*SEPEND*	
QGW8M18	COST	34.10000	TGW8M18	1.00000	PLSW9ST9	TST9	1.00000		
RGW8M18	AVAILGW8	1.00000	MIREQ8	1.00000	QLSW9ST9	COST	13.50000	TST9	1.00000
RGW8M18	TGW8M18	1.00000	WRRF8	.30000	RLSW9ST9	LSW9ST9	1.00000	EVLWS9	.07000
QFGW8M18	AVAILGW8	1.00000	WLREQ8	1.00000	RLSW9ST9	TST9	1.00000		
QFGW8M18	FGHAWL8	1.00000			QDREQ0	DREQ0	1.00000	GRID0	1.00000
QCGW8M18	COST	2.30000	AVAILGW8	1.00000	SET0	*MARKER*		*SEPORG*	
QCGW8M18	WRRF8	1.00000			D01	GRID0	160.00000		
QDREQ1	DREQ1	1.00000	GRID1	1.00000	D02	GRID0	40.00000	LSW0ST0	75.00000
SET1	*MARKER*		*SEPORG*		D03	GRID0	38.00000	LSW0ST0	100.00000
D11	GRID1	123.00000			D04	GRID0	37.00000	LSW0ST0	285.00000
D12	GRID1	7.00000	LSW1ST1	10.00000	ENDSET0	*MARKER*		*SEPEND*	
D13	GRID1	30.00000	LSW1ST1	80.00000	PLSW0ST0	TST0	1.00000		
D14	GRID1	30.00000	LSW1ST1	190.00000	QLSW0ST0	COST	14.30000	TST0	1.00000
ENDSET1	*MARKER*		*SEPEND*		PLSW0ST0	LSW0ST0	1.00000	FVLWS0	.07000
PLSW1ST1	TST1	1.00000			PLSW0ST0	TST0	1.00000		
QLSW1ST1	COST	11.00000	TST1	1.00000	QLSW1EV1	AVAILSW1	1.00000	EVLWS1	1.00000
RLSW1ST1	LSW1ST1	1.00000	EVLWS1	.07000	QLSW2EV2	AVAILSW2	1.00000	AVILSW2	1.00000
RLSW1ST1	TST1	1.00000			QLSW3EV3	AVAILSW3	1.00000	FVLWS3	1.00000
QDREQ2	DREQ2	1.00000	GRID2	1.00000	QLSW4EV4	AVAILSW4	1.00000	AVILSW4	1.00000
SET2	*MARKER*		*SEPORG*		QLSW5EV5	AVAILSW5	1.00000	EVLWS5	1.00000
D21	GRID2	596.00000			QLSW6EV6	AVAILSW6	1.00000	EVLWS6	1.00000
D22	GRID2	138.00000	LSW2ST2	300.00000	QLSW7EV7	AVAILSW7	1.00000	EVLWS7	1.00000
D23	GRID2	137.00000	LSW2ST2	800.00000	QLSW8EV8	AVAILSW8	1.00000	EVLWS8	1.00000
D24	GRID2	138.00000	LSW2ST2	1140.00000	QLSW9EV9	AVAILSW9	1.00000	EVLWS9	1.00000
ENDSET2	*MARKER*		*SEPEND*		QLSW0EV0	AVAILSW0	1.00000	EVLWS0	1.00000
PLSW2ST2	TST2	1.00000			QGW1EV1	AVAILGW1	1.00000	EVGW1	1.00000
QLSW2ST2	COST	4.70000	TST2	1.00000	QGW2EV2	AVAILGW2	1.00000	EVGW2	1.00000
RLSW2ST2	LSW2ST2	1.00000	FV2ST2	1.00000	QGW3EV3	AVAILGW3	1.00000	EVGW3	1.00000
RLSW2ST2	TST2	1.00000			QGW4EV4	AVAILGW4	1.00000	EVGW4	1.00000
QDREQ3	DREQ3	1.00000	GRID3	1.00000	QGW5EV5	AVAILGW5	1.00000	EVGW5	1.00000
SET3	*MARKER*		*SEPORG*		QGW6EV6	AVAILGW6	1.00000	EVGW6	1.00000
D31	GRID3	435.00000			QGW7EV7	AVAILGW7	1.00000	EVGW7	1.00000
D32	GRID3	93.00000	LSW3ST3	240.00000	QGW0EV0	AVAILGW0	1.00000	EVGW0	1.00000
D33	GRID3	99.00000	LSW3ST3	690.00000	QEV2	EVLWS2	.50000	EVGW2	.50000
D34	GRID3	99.00000	LSW3ST3	870.00000	EV2		1.00000		
ENDSET3	*MARKER*		*SEPEND*		SET21	*MARKER*		*SEPORG*	
PLSW3ST3	TST3	1.00000			E21	EV2ST2	208.00000		

Table B-2. Continued.

ROUNDS							
UP	BOUNDX	QBULSW5	29.00000	UP	BOUNDX	D43	1.00000
UP	BOUNDX	QBUMPT	136.60000	UP	BOUNDX	D44	1.00000
UP	BOUNDX	QUILSW3	20.00000	UP	BOUNDX	PLSW4ST4	416.00000
UP	BOUNDX	QUILSW5	57.00000	UP	BOUNDX	QLSW4ST4	1050.00000
UP	BOUNDX	QUI4PT	420.00000	UP	BOUNDX	D51	1.00000
UP	BOUNDX	QSALSW4	15.00000	UP	BOUNDX	D52	1.00000
UP	BOUNDX	QSAMPT	22.40000	UP	BOUNDX	D53	1.00000
FX	BOUNDX	PLSW1AG1	104.90000	UP	BOUNDX	D54	1.00000
FX	BOUNDX	PLSW14I1	7.20000	UP	BOUNDX	PLSW5ST5	481.00000
FX	BOUNDX	PLSW14I4	10.00000	UP	BOUNDX	QLSW5ST5	125.00000
FX	BOUNDX	PLSW2AG2	996.00000	UP	BOUNDX	D61	1.00000
FX	BOUNDX	PLSW2M12	36.00000	UP	BOUNDX	D62	1.00000
UP	BOUNDX	QLSW2SW1	90.00000	UP	BOUNDX	D63	1.00000
UP	BOUNDX	QLSW2SW3	130.00000	UP	BOUNDX	D64	1.00000
UP	BOUNDX	PLSW3AG3	610.50000	UP	BOUNDX	PLSW6ST6	56.00000
FX	BOUNDX	PLSW3M13	29.20000	UP	BOUNDX	QLSW6ST6	100.00000
FX	BOUNDX	PLSW3AG2	19.00000	UP	BOUNDX	D71	1.00000
FX	BOUNDX	PLSW3SW4	71.00000	UP	BOUNDX	D72	1.00000
UP	BOUNDX	QLSW3SW4	146.00000	UP	BOUNDX	D73	1.00000
FX	BOUNDX	PLSW4AG4	713.50000	UP	BOUNDX	D74	1.00000
FX	BOUNDX	PLSW4M14	160.50000	UP	BOUNDX	PLSW7ST7	428.00000
UP	BOUNDX	QLSW4SW5	69.00000	UP	BOUNDX	QLSW7ST7	1500.00000
FX	BOUNDX	PLSW5AG5	879.30000	UP	BOUNDX	D81	1.00000
FX	BOUNDX	PLSW5M15	6.60000	UP	BOUNDX	D82	1.00000
UP	BOUNDX	QLSW5SW6	60.00000	UP	BOUNDX	D83	1.00000
FX	BOUNDX	PLSW5AG9	3.60000	UP	BOUNDX	D84	1.00000
FX	BOUNDX	PLSW6AG6	136.10000	UP	BOUNDX	PLSW8ST8	199.00000
FX	BOUNDX	PLSW6M16	10.10000	UP	BOUNDX	QLSW8ST8	285.00000
FX	BOUNDX	PLSW7AG7	789.10000	UP	BOUNDX	D91	1.00000
FX	BOUNDX	PLSW7M17	10.00000	UP	BOUNDX	D92	1.00000
FX	BOUNDX	PLSW7SW4	101.30000	UP	BOUNDX	D93	1.00000
UP	BOUNDX	PLSW8AG8	303.00000	UP	BOUNDX	D94	1.00000
FX	BOUNDX	PLSW8M18	7.00000	UP	BOUNDX	PLSW9ST9	1.00000
FX	BOUNDX	PLSW8AG5	11.00000	UP	BOUNDX	QLSW9ST9	140.00000
FX	BOUNDX	PLSW9AG9	146.40000	UP	BOUNDX	D01	1.00000
FX	BOUNDX	PLSW9M19	6.90000	UP	BOUNDX	D02	1.00000
FX	BOUNDX	PLSW0AG0	68.00000	UP	BOUNDX	D03	1.00000
FX	BOUNDX	PLSW0M10	1.50000	UP	BOUNDX	D04	1.00000
FX	BOUNDX	PLSW0SW6	3.00000	UP	BOUNDX	PLSW0ST0	14.00000
UP	BOUNDX	QLSW0SW6	47.00000	UP	BOUNDX	QLSW0ST0	280.00000
FX	BOUNDX	PGW1AG1	19.10000	UP	BOUNDX	E21	1.00000
FX	BOUNDX	PGW1M11	2.80000	UP	BOUNDX	E22	1.00000
FX	BOUNDX	PGW2AG2	19.00000	UP	BOUNDX	E23	1.00000
FX	BOUNDX	PGW2M12	8.00000	UP	BOUNDX	E41	1.00000
UP	BOUNDX	PGW3AG3	32.90000	UP	BOUNDX	E42	1.00000
FX	BOUNDX	PGW3M13	20.50000	UP	BOUNDX	E43	1.00000
FX	BOUNDX	PGW4AG4	83.20000	LO	BOUNDX	QLSW10F1	7.00000
FX	BOUNDX	PGW4M14	132.00000	LO	BOUNDX	QLSW20F2	50.00000
FX	BOUNDX	PGW5AG5	127.60000	LO	BOUNDX	QLSW30F3	50.00000
FX	BOUNDX	PGW5M15	10.40000	LO	BOUNDX	QLSW40F4	50.00000
FX	BOUNDX	PGW6AG6	163.90000	LO	BOUNDX	QLSW50F5	13.70000
FX	BOUNDX	PGW6M16	2.90000	LO	BOUNDX	QLSW60F6	.
FX	BOUNDX	PGW7AG7	.	LO	BOUNDX	QLSW70F7	100.00000
FX	BOUNDX	PGW7M17	.	LO	BOUNDX	QLSW80F8	100.00000
FX	BOUNDX	PGW0AG0	.	LO	BOUNDX	QLSW90F9	100.00000
FX	BOUNDX	PGW0M10	.	LO	BOUNDX	QLSW00F0	100.00000
UP	BOUNDX	D11	1.00000	LO	BOUNDX	QGW10F1	6.00000
UP	BOUNDX	D12	1.00000	LO	BOUNDX	QGW20F2	5.00000
UP	BOUNDX	D13	1.00000	LO	BOUNDX	QGW30F3	25.00000
UP	BOUNDX	D14	1.00000	LO	BOUNDX	QGW40F4	8.00000
UP	BOUNDX	PLSW1ST1	17.00000	LO	BOUNDX	QGW50F5	.
UP	BOUNDX	QLSW1ST1	25.00000	LO	BOUNDX	QGW60F6	.
UP	BOUNDX	D21	1.00000	LO	BOUNDX	QGW70F7	40.00000
UP	BOUNDX	D22	1.00000	LO	BOUNDX	QGW00F0	.
UP	BOUNDX	D23	1.00000	ENDATA			
UP	BOUNDX	D24	1.00000				
UP	BOUNDX	PLSW2ST2	311.00000				
UP	BOUNDX	QLSW2ST2	1200.00000				
UP	BOUNDX	D31	1.00000				
UP	BOUNDX	D32	1.00000				
UP	BOUNDX	D33	1.00000				
UP	BOUNDX	D34	1.00000				
UP	BOUNDX	PLSW3ST3	578.00000				
UP	BOUNDX	QLSW3ST3	125.00000				
UP	BOUNDX	D41	1.00000				
UP	BOUNDX	D42	1.00000				

Table B-3. Print-out of the optimal solution.

EXECUTOR. MPS/360 V2-M5							
SOLUTION (OPTIMAL)							
TIME = 4.62 MINS. ITERATION NUMBER = 310							
...	NAME...	...	ACTIVITY...	...	DEFINED AS
	FUNCTIONAL		9722.44726		COST		
	RESTRAINTS				RHS		
	BOUNDS....				BCUNDX		
SECTION 1 - ROWS							
NUMBER	...ROW..	AT	...ACTIVITY...	SLACK ACTIVITY	..LOWER LIMIT.	..UPPER LIMIT.	..DUAL ACTIVITY
	1	COST	BS 9722.44726	9722.44726-	NONE	NONE	1.00000
A	2	AVAILSW1	EQ 613.00000	.	613.00000	613.00000	.
A	3	AVAILSW2	EQ 941.50000	.	941.50000	941.50000	.
A	4	AVAILSW3	EQ 789.20000	.	789.20000	789.20000	.
A	5	AVAILSW4	EQ 513.60000	.	513.60000	513.60000	.
A	6	AVAILSW5	EQ 453.20000	.	453.20000	453.20000	.
A	7	AVAILSW6	EQ 80.00000	.	80.00000	80.00000	14.00000
A	8	AVAILSW7	EQ 1351.60000	.	1351.60000	1351.60000	.
A	9	AVAILSW8	EQ 650.00000	.	650.00000	650.00000	.
A	10	AVAILSW9	EQ 430.00000	.	430.00000	430.00000	.
A	11	AVAILSW0	EQ 250.00000	.	250.00000	250.00000	.
A	12	LSWU7	EQ
A	13	LSWU8	EQ
A	14	AVAILGW1	EQ 187.00000	.	187.00000	187.00000	.
A	15	AVAILGW2	EQ 103.50000	.	103.50000	103.50000	.
A	16	AVAILGW3	EQ 94.90000	.	94.90000	94.90000	.
	17	AVAILGW4	EQ 272.10000	.	272.10000	272.10000	3.00000
	18	AVAILGW5	EQ 254.60000	.	254.60000	254.60000	17.00000
	19	AVAILGW6	EQ 130.00000	.	130.00000	130.00000	31.00000
	20	AVAILGW7	EQ 40.00000	.	40.00000	40.00000	66.66667
	21	AVAILGW0	EQ 10.00000	.	10.00000	10.00000	3.00000
	22	MIREQ1	BS 10.00000	.	10.00000	NONE	.
	23	MIREQ2	BS 44.00000	.	44.00000	NONE	.
	24	MIREQ3	BS 49.70000	.	49.70000	NONE	.
	25	MIREQ4	BS 302.50000	.	302.50000	NONE	.
	26	MIREQ5	BS 17.00000	.	17.00000	NONE	.
	27	MIREQ6	BS 13.00000	.	13.00000	NONE	.
	28	MIREQ7	LL 10.00000	.	10.00000	NONE	52.90000-
	29	MIREQ8	BS 7.00000	.	7.00000	NONE	.
	30	MIREQ9	BS 6.80000	.	6.80000	NONE	.
	31	MIREQ0	BS 1.50000	.	1.50000	NONE	.
	32	TLW1M11	EQ	18.20000
	33	TLW2M12	EQ	17.16000
	34	TLW3M13	EQ	11.35160
	35	TLW4M14	EQ	21.91140
	36	TLW5M15	EQ	15.50560
	37	TLW6M16	EQ	12.60600
	38	TLW7M17	EQ	36.00000-
	39	TLW8M18	EQ	7.80000
	40	TLW9M19	EQ	6.50000
	41	TLW0M20	EQ	7.80000
	42	TGW1M11	EQ	18.20000
	43	TGW2M12	EQ	17.16000
	44	TGW3M13	EQ	11.35160
	45	TGW4M14	EQ	20.91140
	46	TGW5M15	EQ	22.50560
	47	TGW6M16	EQ	29.60600
	48	TGW7M17	EQ	30.66667
	49	TGW0M10	EQ	34.10000-
	50	AGREQ1	BS 124.00000	.	124.00000	NONE	.
	51	AGREQ2	BS 1034.00000	.	1034.00000	NONE	.
	52	AGREQ3	LL 643.40000	.	643.40000	NONE	.
	53	AGREQ4	BS 796.70000	.	796.70000	NONE	.
	54	AGREQ5	BS 1017.90000	.	1017.90000	NONE	.
	55	AGREQ6	BS 300.00000	.	300.00000	NONE	.
	56	AGREQ7	BS 789.33333	.23333-	789.10000	NONE	.
	57	AGREQ8	LL 303.00000	.	303.00000	NONE	5.00000-
	58	AGREQ9	BS 150.00000	.	150.00000	NONE	.
	59	AGREQ0	BS 68.00000	.	68.00000	NONE	.
	60	TLW1AG1	EQ
	61	TLW2AG2	EQ
	62	TLW3AG3	EQ
	63	TLW4AG4	EQ	2.48448
	64	TLW5AG5	EQ	4.52500
	65	TLW6AG6	EQ	5.14425
	66	TLW7AG7	EQ	5.00000-
	67	TLW8AG8	EQ	5.00000-
	68	TLW9AG9	EQ
	69	TLW0AG0	EQ

Table B-3. Continued.

NUMBR	..ROW..	AT	...ACTIVITY...	SLACK	ACTIVITY	..LOWER LIMIT..	..UPPER LIMIT..	..DUAL ACTIVITY
A	70	TGW1AG1	EQ
A	71	TGW2AG2	EQ
A	72	TGW3AG3	EQ
	73	TGW4AG4	EQ	1.48448
	74	TGW5AG5	EQ	11.52500
	75	TGW6AG6	EQ	22.14425
	76	TGW7AG7	BS
	77	TGW0AG0	BS
	78	AGEXC3	BS
	79	AGEXC4	EQ	3.00000
	80	AGEXC8	BS
A	81	WLREQ1	EQ	715.00000	.	715.00000	715.00000	.
A	82	WLREQ2	EQ	240.00000	.	240.00000	240.00000	.
A	83	WLREQ3	EQ	143.10000	.	143.10000	143.10000	.
A	84	WLREQ4	EQ	276.40000	.	276.40000	276.40000	.
A	85	WLREQ5	EQ	332.60000	.	332.60000	332.60000	.
	86	WLREQ6	EQ	130.00000	.	130.00000	130.00000	14.00000-
A	87	WLREQ7	EQ	315.00000	.	315.00000	315.00000	.
A	88	WLREQ8	EQ	36.00000	.	36.00000	36.00000	.
A	89	WLREQ9	EQ	8.00000	.	8.00000	8.00000	.
A	90	WLREQ0	EQ	19.00000	.	19.00000	19.00000	.
A	91	DREQ1	EQ
A	92	DREQ2	EQ
A	93	DREQ3	EQ
	94	DREQ4	EQ	4.00000
	95	DREQ5	EQ	10.00000
A	96	DREQ6	EQ
A	97	DREQ7	EQ
A	98	DREQ8	EQ
A	99	DREQ9	EQ
A	100	DREQ0	EQ
A	101	GRID1	EQ
A	102	LSW1ST1	EQ
A	103	GRID2	EQ
A	104	LSW2ST2	EQ
A	105	GRID3	EQ
A	106	LSW3ST3	EQ
	107	GRID4	EQ	4.00000-
	108	LSW4ST4	EQ93924
	109	GRID5	EQ	10.00000-
	110	LSW5ST5	EQ	1.46512
A	111	GRID6	EQ
	112	LSW6ST6	BS
A	113	GRID7	EQ
	114	LSW7ST7	BS
A	115	GRID8	EQ
	116	LSW8ST8	BS
A	117	GRID9	EQ
	118	LSW9ST9	BS
A	119	GRID0	EQ
A	120	LSW0ST0	EQ
A	121	TST1	EQ
A	122	TST2	EQ
A	123	TST3	EQ
	124	TST4	EQ93324-
	125	TST5	EQ93812-
A	126	TST6	EQ
A	127	TST7	EQ
A	128	TST8	EQ
A	129	TST9	EQ
A	130	TST0	EQ
A	131	EVLSW1	EQ
A	132	EVLSW2	EQ
A	133	EVLSW3	EQ
A	134	EVLSW4	EQ
A	135	EVLSW5	EQ
	136	EVLSW6	EQ	10.33333-
	137	EVLSW7	BS
	138	EVLSW8	BS
A	139	EVLSW9	EQ
A	140	EVLSW0	EQ
	141	EVGW1	BS
	142	EVGW2	BS
A	143	EVGW3	EQ
	144	EVGW4	EQ	3.00000
	145	EVGW5	EQ	17.00000
	146	EVGW6	EQ	31.00000
	147	EVGW7	BS
	148	EVGW0	BS
A	149	EV2ST2	EQ
A	150	EV2	EQ
	151	EV4ST4	EQ10500-
	152	EV4	EQ	1.50000
	153	WRRF1	EQ	26.00000
	154	WRRF2	EQ	26.00000
	155	WRRF3	EQ	26.00000
	156	WRRF4	EQ	26.00000
	157	WRRF5	EQ	12.00000
	158	WRRF6	EQ	2.00000-
	159	WRRF7	EQ	26.00000
	160	WRRF8	EQ	26.00000
	161	WRRF9	EQ	26.00000
	162	WRRF0	EQ	26.00000
A	163	AGRFSW1	EQ
A	164	AGRFSW2	EQ
A	165	AGRFSW3	EQ
	166	AGRFSW4	EQ	2.80000-
	167	AGRFSW5	EQ	8.00000-
	168	AGRFSW6	EQ	14.00000-
A	169	AGRFSW7	EQ
A	170	AGRFSW8	EQ
A	171	AGRFSW9	EQ

Table B-3. Continued.

NUMBER	...ROW...	AT	...ACTIVITY...	SLACK	ACTIVITY	..LOWER LIMIT.	..UPPER LIMIT.	..DUAL ACTIVITY
A	172	AGRFSW0	EQ
A	173	AGRFGW1	EQ
A	174	AGRFGW2	EQ
A	175	AGRFGW3	EQ
	176	AGRFGW4	EQ	1.50000-
	177	AGRFGW5	EQ	8.50000-
	178	AGRFGW6	EQ	22.50000-
	179	AGRFGW7	EQ	33.33333-
	180	AGRFGW0	EQ	3.00000-
A	181	FGWAVWL1	EQ	166.80000	.	166.80000	166.80000	.
A	182	FGWAVWL2	EQ	147.50000	.	147.50000	147.50000	.
A	183	FGWAVWL3	EQ	51.80000	.	51.80000	51.80000	.
	184	FGWAVWL4	EQ	96.00000	.	96.00000	96.00000	3.00000-
	185	FGWAVWL5	EQ	209.10000	.	209.10000	209.10000	17.00000-
	186	FGWAVWL6	EQ	42.50000	.	42.50000	42.50000	17.00000-
	187	FGWAVWL7	EQ	59.20000	.	59.20000	59.20000	66.66667-
	189	FGWAVWL0	EQ	10.00000	.	10.00000	10.00000	3.00000-
	189	GWRC1	BS	.	.	NONE	.	.
	190	GWRC2	BS	.	60.00000	NONE	60.00000	.
	191	GWRC3	BS	.	366.00000	NONE	366.00000	.
	192	GWRC4	BS	.09750	433.90250	NONE	434.00000	.
	193	GWRCU4	BS	.	100.00000	NONE	100.00000	.
	194	GWRC5	BS	31.06850	20.93150	NONE	22.00000	.
	195	GWRCU5	BS	.	52.00000	NONE	52.00000	.
	196	GWRC6	BS	57.59500	7.40500	NONE	65.00000	.
	197	GWRC7	UL	.	.	NONE	.	63.66667
A	198	GWRC0	BS	.	.	NONE	.	.
A	199	BUIMPT	EQ
A	200	UIIMPT	EQ
	201	SIIMPT	EQ	6.00000
	202	TLSW3SW4	EQ	4.00000-
	203	TLSW3SW6	EQ	14.00000-
	204	INFLOGSL	BS	1017.30580	517.30580-	500.00000	NONE	.
	205	CRQUT	BS	1506.78329	599.78328-	907.00000	NONE	.

SECTION 2 - COLUMNS

NUMBER	.COLUMN.	AT	...ACTIVITY...	..INPUT COST..	..LOWER LIMIT.	..UPPER LIMIT.	..REDUCED COST.	
	206	QBULSW4	LL	.	7.00000	.	NONE	3.00000
	207	QBULSW5	BS	19.67295	10.00000	.	29.00000	.
	208	QRUMPT	BS	24.59119	.	.	136.60000	.
	209	QUILSW3	LL	.	10.00000	.	20.00000	10.00000
	210	QUILSW4	LL	.	10.00000	.	NONE	6.00000
	211	QUILSW5	LL	.	13.00000	.	57.00000	3.00000
	212	QUIMPT	BS	.	.	.	420.00000	.
	213	QSALSW4	LL	.	8.00000	.	15.00000	10.00000
	214	QSALSW5	BS	22.40000	4.00000	.	NONE	.
	215	QSAMP	UL	22.40000	.	.	22.40000	6.00000-
	216	PLSW1AG1	EQ	104.90000	.	104.90000	104.90000	.
	217	QLSW1AG1	LL	.	5.00000	.	NONE	5.00000
	218	RLSW1AG1	BS	104.90000	.	.	NONE	.
	219	PLSW1M11	EQ	7.20000	.	7.20000	7.20000	18.20000
	220	QLSW1M11	LL	.	31.00000	.	NONE	49.20000
	221	RLSW1M11	BS	7.20000	.	.	NONE	.
	222	QLSW1WL1	BS	540.49960	.	.	NONE	.
	223	PLSW1M14	EQ	10.00000	.	10.00000	10.00000	17.91140
A	224	PLSW2AG2	EQ	996.00000	.	996.00000	996.00000	.
	225	QLSW2AG2	LL	.	5.00000	.	NONE	5.00000
	226	RLSW2AG2	BS	996.00000	.	.	NONE	.
	227	PLSW2M12	EQ	36.00000	.	36.00000	36.00000	17.10000
	228	QLSW2M12	LL	.	32.00000	.	NONE	49.10000
	229	RLSW2M12	BS	36.00000	.	.	NONE	.
	230	QLSW2HL2	BS	14.95000	.	.	NONE	.
	231	QLSW2SW1	LL	.	4.00000	.	90.00000	4.00000
	232	QLSW2SW3	LL	.	4.00000	.	130.00000	4.00000
	233	PLSW3AG3	BS	610.50000	.	.	610.50000	.
	234	QLSW3AG3	LL	.	6.00000	.	NONE	6.00000
	235	RLSW3AG3	BS	610.50000	.	.	NONE	.
	236	PLSW3M13	EQ	29.20000	.	29.20000	29.20000	11.35160
	237	QLSW3M13	LL	.	43.00000	.	NONE	54.35160
	238	RLSW3M13	BS	29.20000	.	.	NONE	.
	239	QLSW3WL3	BS	43.04500	.	.	NONE	.
A	240	PLSW3AG2	EQ	19.00000	.	19.00000	19.00000	.
A	241	PLSW3SW4	EQ	71.00000	.	71.00000	71.00000	4.00000-
	242	QLSW3SW4	LL	.	4.00000	.	146.00000	.
A	243	PLSW3SW4	BS	71.00000	.	.	NONE	.
	244	QAG3L SW3	LL	.	.	.	NONE	.
	245	PLSW4AG4	EQ	713.50000	.	713.50000	713.50000	2.48448
	246	QLSW4AG4	LL	.	6.00000	.	NONE	8.48448
	247	RLSW4AG4	BS	713.50000	.	.	NONE	.
	248	PLSW4M14	EQ	160.50000	.	160.50000	160.50000	21.91140
	249	QLSW4M14	LL	.	43.00000	.	NONE	64.91140
	250	RLSW4M14	BS	160.50000	.	.	NONE	.
	251	QLSW4WL4	BS	120.64750	.	.	NONE	.
	252	QLSW4SW5	BS	59.19852	.	.	NONE	.
	253	QAG4L SW4	LL	.	5.00000	.	69.00000	.
	254	PLSW5AG5	EQ	879.30000	.	879.30000	879.30000	3.00000
	255	QLSW5AG5	LL	.	5.00000	.	NONE	4.52500
	256	RLSW5AG5	BS	879.30000	.	.	NONE	9.52500
	257	PLSW5M15	EQ	6.60000	.	6.60000	6.60000	15.50560
	258	QLSW5M15	LL	.	36.00000	.	NONE	51.50560
	259	RLSW5M15	BS	6.60000	.	.	NONE	.
	260	QLSW5WL5	BS	47.15750	.	.	NONE	.
	261	QLSW5SW6	BS	10.52000	.	4.00000	60.00000	.
	262	PLSW5AG9	EQ	3.60000	.	3.60000	3.60000	10.00000
	263	PLSW6AG6	EQ	136.10000	.	136.10000	136.10000	5.14425
	264	QLSW6AG6	LL	.	5.00000	.	NONE	10.14425
	265	RLSW6AG6	BS	136.10000	.	.	NONE	.
	266	PLSW6M16	EQ	10.10000	.	10.10000	10.10000	12.60600
	267	QLSW6M16	LL	.	36.00000	.	NONE	48.60600
	268	RLSW6M16	BS	10.10000	.	.	NONE	.
	269	QLSW6WL6	BS	65.79500	.	.	NONE	.
	270	PLSW7AG7	EQ	789.10000	.	789.10000	789.10000	5.00000-
	271	QLSW7AG7	BS	.23333	5.00000	.	NONE	.

Table B-3. Continued.

NUMBER	COLUMN	AT	...ACTIVITY...	..INPUT COST..	..LOWER LIMIT.	..UPPER LIMIT.	..REDUCED COST.
272	RLSW7AG7	BS	789.33333	.	.	NONE	.
273	PLSW7M17	EQ	10.00000	.	10.00000	10.00000	36.00000-
274	QLSW7M17	BS	.	36.00000	.	NONE	.
275	RLSW7M17	BS	10.00000	.	.	NONE	.
276	QLSW7M17	BS	196.60000	.	.	NONE	.
277	Q7LSW7	BS	611.50053	.	.	NONE	.
278	PLSW7S44	EQ	101.30000	.	101.30000	101.30000	4.00000-
279	PLSW8AG8	UL	303.00000	.00010	.	303.00000	4.99990-
280	QLSW8AG8	BS	.	5.00000	.	NONE	.
281	RLSW8AG8	BS	303.00000	.	.	NONE	.
282	PLSW8M18	EQ	7.00000	.	7.00000	7.00000	7.80000
283	QLSW8M18	LL	.	51.00000	.	NONE	58.80000
284	RLSW8M18	BS	7.00000	.	.	NONE	.
285	QLSW8M18	BS	36.00000	.	.	NONE	.
286	Q8LSW8	BS	154.52500	.	.	NONE	.
287	PLSW8AG5	EQ	11.00000	.	11.00000	11.00000	5.47500-
A 288	QAGPLSW8	LL	.	.	.	NONE	.
A 289	PLSW9AG9	EQ	146.40000	.	146.40000	146.40000	.
290	QLSW9AG9	LL	.	5.00000	.	NONE	5.00000
291	RLSW9AG9	BS	146.40000	.	.	NONE	.
292	PLSW9M19	EQ	6.80000	.	6.80000	6.80000	6.50000
293	QLSW9M19	LL	.	43.00000	.	NONE	49.50000
294	RLSW9M19	BS	6.80000	.	.	NONE	.
295	QLSW9M19	BS	8.00000	.	.	NONE	.
A 296	PLSW0AG0	EQ	68.00000	.	68.00000	68.00000	.
297	QLSW0AG0	LL	.	5.00000	.	NONE	5.00000
298	RLSW0AG0	BS	68.00000	.	.	NONE	.
299	PLSW0M10	EQ	1.50000	.	1.50000	1.50000	7.80000
300	QLSW0M10	LL	.	43.00000	.	NONE	50.80000
301	RLSW0M10	BS	1.50000	.	.	NONE	.
302	QLSW0M10	BS	9.00000	.	.	NONE	.
303	PLSW0S44	EQ	3.00000	.	3.00000	3.00000	14.00000-
304	QLSW0S44	UL	47.00000	4.00000	.	47.00000	10.00000-
305	RLSW0S44	BS	50.00000	.	.	NONE	.
A 306	PGW1AG1	EQ	19.10000	.	19.10000	19.10000	.
307	QGW1AG1	LL	.	4.90000	.	NONE	4.90000
308	RGW1AG1	BS	19.10000	.	.	NONE	.
309	PGW1M11	EQ	2.80000	.	2.80000	2.80000	18.20000
310	QGW1M11	LL	.	34.25000	.	NONE	52.45000
311	RGW1M11	BS	2.80000	.	.	NONE	.
312	QGW1M11	BS	174.50040	.	.	NONE	.
313	QCGW1M11	LL	.	2.40000	.	NONE	2.40000
A 314	PGW2AG2	EQ	19.00000	.	19.00000	19.00000	.
315	QGW2AG2	LL	.	5.60000	.	NONE	5.60000
316	RGW2AG2	BS	19.00000	.	.	NONE	.
317	PGW2M12	EQ	8.00000	.	9.00000	8.00000	17.16000
318	QGW2M12	LL	.	34.25000	.	NONE	51.41000
319	RGW2M12	BS	8.00000	.	.	NONE	.
320	QGW2M12	BS	225.05000	.	.	NONE	.
321	QCGW2M12	LL	.	3.10000	.	NONE	3.10000
A 322	PGW3AG3	UL	32.90000	.	.	32.90000	.
323	QGW3AG3	LL	.	6.70000	.	NONE	6.70000
324	RGW3AG3	BS	32.90000	.	.	NONE	.
325	PGW3M13	EQ	20.50000	.	20.50000	20.50000	11.35160
326	QGW3M13	LL	.	41.65000	.	NONE	53.02160
327	RGW3M13	BS	20.50000	.	.	NONE	.
328	QGW3M13	BS	100.05500	.	.	NONE	.
329	QCGW3M13	LL	.	3.70000	.	NONE	3.70000
A 330	QAG3GW3	LL	.	.	.	NONE	.
331	PGW4AG4	EQ	83.20000	.	83.20000	83.20000	1.43448
332	QGW4AG4	LL	.	7.70000	.	NONE	9.13448
333	RGW4AG4	BS	83.20000	.	.	NONE	.
334	PGW4M14	EQ	132.00000	.	132.00000	132.00000	20.91140
335	QGW4M14	LL	.	41.65000	.	NONE	62.55140
336	RGW4M14	BS	132.00000	.	.	NONE	.
337	QGW4M14	BS	155.75250	.	.	NONE	.
338	QCGW4M14	LL	.	4.70000	.	NONE	7.70000
339	QAG4GW4	BS	.	.	.	NONE	.
340	PGW5AG5	EQ	127.60000	.	127.60000	127.60000	11.52500
341	QGW5AG5	LL	.	5.80000	.	NONE	17.32500
342	RGW5AG5	BS	127.60000	.	.	NONE	.
343	PGW5M15	EQ	10.40000	.	10.40000	10.40000	22.50560
344	QGW5M15	LL	.	34.25000	.	NONE	56.75560
345	RGW5M15	BS	10.40000	.	.	NONE	.
346	QGW5M15	BS	285.44250	.	.	NONE	.
347	QCGW5M15	LL	.	3.30000	.	NONE	20.30000
348	PGW6AG6	EQ	163.90000	.	163.90000	163.90000	22.14425
349	QGW6AG6	LL	.	6.40000	.	NONE	28.54425
350	RGW6AG6	BS	163.90000	.	.	NONE	.
351	PGW6M16	EQ	7.90000	.	2.90000	2.90000	29.60600
352	QGW6M16	LL	.	34.25000	.	NONE	63.85600
353	RGW6M16	BS	2.90000	.	.	NONE	.
354	QGW6M16	BS	64.20500	.	.	NONE	.
355	QCGW6M16	LL	.	3.90000	.	NONE	20.90000
A 356	PGW7AG7	EQ
357	QGW7AG7	LL	.	4.60000	.	NONE	4.60000
358	RGW7AG7	LL	.	.	.	NONE	61.65667
359	PGW7M17	EQ	30.65667
360	QGW7M17	LL	.	34.25000	.	NONE	64.91667
361	RGW7M17	BS	.	.	.	NONE	.
362	QGW7M17	BS	118.40000	.	.	NONE	.
363	QCGW7M17	LL	.	2.10000	.	NONE	68.75667
A 364	PGW0AG0	EQ
365	QGW0AG0	LL	.	4.80000	.	NONE	4.80000
366	RGW0AG0	LL	.	.	.	NONE	3.00000
367	PGW0M10	EQ	34.10000-
368	QGW0M10	BS	.	34.10000	.	NONE	.
369	RGW0M10	LL	.	.	.	NONE	44.90000
370	QCGW0M10	BS	10.00000	.	.	NONE	.
371	QCGW0M10	LL	.	2.30000	.	NONE	5.30000
372	QDREQ1	BS	112.18000	.	.	NONE	.
373	D11	BS	.91203	.	.	1.00000	.
A 374	D12	LL	.	.	.	1.00000	.
A 375	D13	LL	.	.	.	1.00000	.
A 376	D14	LL	.	.	.	1.00000	.

Table B-3. Continued.

NUMBER	COLUMN	AT	ACTIVITY	INPUT COST	LOWER LIMIT	UPPER LIMIT	REDUCED COST
377	PLSW1ST1	BS	.	.	.	17.00000	.
378	QLSW1ST1	LL	.	11.00000	.	25.00000	11.00000
379	RLSW1ST1	BS	.	.	.	NONE	.
380	QDREQ2	BS	612.03056	.	.	NONE	.
A 381	D21	UL	1.00000	.	.	1.00000	.
382	D22	BS	.11616	.	.	1.00000	.
A 383	D23	LL	.	.	.	1.00000	.
A 384	D24	LL	.	.	.	1.00000	.
385	PLSW2ST2	BS	34.84904	.	.	311.00000	.
386	QLSW2ST2	LL	.	4.70000	.	1200.00000	4.70000
387	RLSW2ST2	BS	34.84904	.	.	NONE	.
388	QDREQ3	BS	574.22239	.	.	NONE	.
A 389	D31	UL	1.00000	.	.	1.00000	.
A 390	D32	UL	1.00000	.	.	1.00000	.
391	D33	BS	.46680	.	.	1.00000	.
A 392	D34	LL	.	.	.	1.00000	.
393	PLSW3ST3	BS	562.15605	.	.	578.00000	.
394	QLSW3ST3	LL	.	16.30000	.	125.00000	16.30000
395	RLSW3ST3	BS	562.15605	.	.	NONE	.
396	QDREQ4	BS	518.65882	.	.	NONE	.
397	D41	UL	1.00000	.	.	1.00000	1528.00000-
398	D42	UL	1.00000	.	.	1.00000	135.52941-
399	D43	BS	.84118	.	.	1.00000	.
400	D44	LL	.	.	.	1.00000	365.64706
401	PLSW4ST4	UL	416.00000	.	.	416.00000	.89374-
402	QLSW4ST4	LL	.	13.00000	.	1050.00000	12.11676
403	RLSW4ST4	BS	416.00000	.	.	NONE	.
404	QDREQ5	BS	371.23553	.	.	NONE	.
405	D51	UL	1.00000	.	.	1.00000	2620.00000-
406	D52	UL	1.00000	.	.	1.00000	387.67442-
407	D53	BS	.60698	.	.	1.00000	.
408	D54	LL	.	.	.	1.00000	127.20930
409	PLSW5ST5	UL	481.00000	.	.	481.00000	.93812-
410	QLSW5ST5	LL	.	8.60000	.	125.00000	7.66188
411	RLSW5ST5	BS	481.00000	.	.	NONE	.
412	QDREQ6	BS	13.67100	.	.	NONE	.
413	D61	BS	.28481	.	.	1.00000	.
A 414	D62	LL	.	.	.	1.00000	.
A 415	D63	LL	.	.	.	1.00000	.
A 416	D64	LL	.	.	.	1.00000	.
417	PLSW6ST6	BS	.	14.00000	.	56.00000	.
418	QLSW6ST6	LL	.	.	.	100.00000	14.00000
419	RLSW6ST6	BS	.	.	.	NONE	.
420	QDREQ7	BS	811.84468	.	.	NONE	.
421	D71	BS	.93315	.	.	1.00000	.
A 422	D72	LL	.	.	.	1.00000	.
A 423	D73	LL	.	.	.	1.00000	.
A 424	D74	LL	.	.	.	1.00000	.
425	PLSW7ST7	BS	.	.	.	428.00000	.
426	QLSW7ST7	LL	.	10.80000	.	1500.00000	10.80000
A 427	RLSW7ST7	LL	.	.	.	NONE	.
428	QDREQ8	BS	305.52500	.	.	NONE	.
429	D81	BS	.77544	.	.	1.00000	.
A 430	D82	LL	.	.	.	1.00000	.
A 431	D83	LL	.	.	.	1.00000	.
A 432	D84	LL	.	.	.	1.00000	.
433	PLSW8ST8	BS	.	.	.	199.00000	.
434	QLSW8ST8	LL	.	7.20000	.	285.00000	7.20000
A 435	RLSW8ST8	LL	.	.	.	NONE	.
436	QDREQ9	BS	129.20000	.	.	NONE	.
437	D91	BS	.47500	.	.	1.00000	.
A 438	D92	LL	.	.	.	1.00000	.
A 439	D93	LL	.	.	.	1.00000	.
A 440	D94	LL	.	.	.	1.00000	.
441	PLSW9ST9	BS	.	.	.	1.00000	.
442	QLSW9ST9	LL	.	13.50000	.	140.00000	13.50000
443	PLSW9ST9	BS	.	.	.	NONE	.
444	QDREQ0	BS	109.30000	.	.	NONE	.
445	D01	BS	.68313	.	.	1.00000	.
A 446	D02	LL	.	.	.	1.00000	.
A 447	D03	LL	.	.	.	1.00000	.
A 448	D04	LL	.	.	.	1.00000	.
449	PLSW0ST0	BS	.	.	.	14.00000	.
450	QLSW0ST0	LL	.	14.30000	.	280.00000	14.30000
451	RLSW0ST0	BS	.	.	.	NONE	.
452	QLSW1EV1	BS	.	.	.	NONE	.
453	QLSW2EV2	BS	.	.	.	NONE	.
454	QLSW3EV3	BS	12.92959	.	.	NONE	.
455	QLSW4EV4	BS	12.75000	.	.	NONE	.
456	QLSW5EV5	BS	44.73300	.	.	NONE	.
457	QLSW6EV6	LL	.	.	.	NONE	24.33333
A 458	QLSW7EV7	LL	.	.	.	NONE	.
A 459	QLSW8EV8	LL	.	.	.	NONE	.
A 460	QLSW9EV9	LL	.	.	.	NONE	.
461	QLSW0EVO	BS	.	.	.	NONE	.
A 462	QGW1EV1	LL	.	.	.	NONE	.
A 463	QGW2EV2	LL	.	.	.	NONE	.
464	QGW3EV3	BS	12.92959	.	.	NONE	.
465	QGW4EV4	BS	12.75000	.	.	NONE	.
466	QGW5EV5	BS	14.91100	.	.	NONE	.
467	QGW6EV6	BS	.	.	.	NONE	.
468	QGW7EV7	LL	.	.	.	NONE	66.66667
469	QGW0EVO	LL	.	.	.	NONE	3.00000
470	QEV2	BS	.	.	.	NONE	.
A 471	E21	BS	.16754	.	.	1.00000	.
A 472	E22	LL	.	.	.	1.00000	.
A 473	E23	LL	.	.	.	1.00000	.
474	QEV4	BS	25.50000	.	.	NONE	.
475	E41	UL	1.00000	.	.	1.00000	23.10000-
476	E42	UL	1.00000	.	.	1.00000	17.67000-
477	E43	BS	.	.	.	1.00000	.
478	QAR1LSW1	BS	40.60000	.	.	NONE	.
479	QAR2LSW2	BS	524.96180	.	.	NONE	.
480	QAR3LSW3	BS	310.95522	.	.	NONE	.
481	QAR4LSW4	BS	367.19903	.	.	NONE	.

Table B-3. Continued.

NUMBER	.COLUMN.	AT	...ACTIVITY...	..INPUT COST..	..LOWER LIMIT.	..UPPER LIMIT.	..REDUCED COST.
482	QAR5LSW5	BS	534.39750	.	.	NONE	.
483	QAR6LSW6	BS	120.00000	.	.	NONE	.
484	QAR7LSW7	BS	377.93280	.	.	NONE	.
485	QAR8LSW8	BS	189.37500	.	.	NONE	.
486	QAR9LSW9	BS	120.00000	.	.	NONE	.
487	QAR0LSW0	RS	34.00000	.	.	NONE	.
488	QAR1GW1	BS	15.40080	.	.	NONE	.
489	QAR2GW2	BS	155.10000	.	.	NONE	.
490	QAR3GW3	RS	96.51000	.	.	NONE	.
491	QAR4GW4	BS	119.50500	.	.	NONE	.
492	QAR5GW5	RS	152.68500	.	.	NONE	.
493	QAR6GW6	RS	43.41000	.	.	NONE	.
494	QAR7GW7	RS	118.40000	.	.	NONE	.
495	QAR0GW0	BS	.	.	.	NONE	.
496	QWH1LSW1	BS	7.00000	26.00000	.	NONE	.
497	QWH2LSW2	RS	29.04000	26.00000	.	NONE	.
498	QWH3LSW3	BS	21.59900	26.00000	.	NONE	.
499	QWH4LSW4	RS	208.29475	26.00000	.	NONE	.
500	QWH5LSW5	LL	.	26.00000	.	NONE	14.00000
501	QWH6LSW6	LL	.	26.00000	.	NONE	14.00000
502	QWH7LSW7	RS	6.50000	26.00000	.	NONE	.
503	QWH8LSW8	BS	2.10000	26.00000	.	NONE	.
504	QWH9LSW9	BS	1.70000	26.00000	.	NONE	.
505	QWH0LSW0	BS	45000	26.00000	.	NONE	.
506	QLSW1R1	LL	.	17.00000	.	NONE	17.00000
507	QLSW2R2	LL	.	17.00000	.	NONE	17.00000
508	QLSW3R3	LL	.	17.00000	.	NONE	17.00000
509	QLSW4R4	LL	.	17.00000	.	NONE	14.00000
510	QLSW4RU4	LL	.	23.00000	.	NONE	20.00000
511	QLSW5R5	RS	23.26890	17.00000	.	NONE	.
512	QLSW5RU5	LL	.	23.00000	.	NONE	6.00000
513	QLSW6R6	BS	48.53400	17.00000	.	NONE	.
514	QLSW7R7	LL	.	17.00000	.	NONE	14.00000
515	QLSW0R0	LL	.	17.00000	.	NONE	14.00000
516	QWH1R1	LL	.	29.00000	.	NONE	3.00000
517	QWH2R2	LL	.	29.00000	.	NONE	3.00000
518	QWH3R3	LL	.	29.00000	.	NONE	3.00000
519	QWH4R4	RS	09750	29.00000	.	NONE	.
520	QWH4RU4	LL	.	35.00000	.	NONE	6.00000
521	QWH5R5	RS	7.79960	29.00000	.	NONE	.
522	QWH5RU5	LL	.	35.00000	.	NONE	6.00000
523	QWH6R6	BS	9.06100	25.00000	.	NONE	.
524	QWH7R7	RS	.	29.00000	.	NONE	.
525	QWH0R0	RS	.	29.00000	.	NONE	.
526	QLSW1OF1	RS	7.00040	.	7.00000	NONE	.
527	QLSW2OF2	BS	448.55180	.	50.00000	NONE	.
528	QLSW3OF3	RS	336.17965	.	50.00000	NONE	.
529	QLSW4OF4	RS	179.99814	.	50.00000	NONE	.
530	QLSW5OF5	BS	73.68057	.	13.70000	NONE	.
531	QLSW6OF6	LL	.	.	.	NONE	14.00000
532	QLSW7OF7	BS	614.20828	.	100.00000	NONE	.
533	QLSW8OF8	RS	462.07500	.	100.00000	NONE	.
534	QLSW9OF9	RS	390.50000	.	100.00000	NONE	.
535	QLSW0OF0	RS	155.95000	.	100.00000	NONE	.
536	QGW1OF1	BS	6.00040	.	6.00000	NONE	.
537	QGW2OF2	BS	6.55000	.	5.00000	NONE	.
538	QGW3OF3	BS	25.02541	.	25.00000	NONE	.
539	QGW4OF4	LL	8.00000	.	8.00000	NONE	3.00000
540	QGW5OF5	LL	.	.	.	NONE	17.00000
541	QGW6OF6	LL	.	.	.	NONE	31.00000
542	QGW7OF7	LL	40.00000	.	40.00000	NONE	66.66667
543	QGW0OF0	LL	.	.	.	NONE	3.00000

Table B-4. Print-out of the sensitivity analysis. (First page of each section only.)

SECTION 1 - ROWS AT LIMIT LEVEL

NUMBER	...ROW..	AT	...ACTIVITY...	SLACK	ACTIVITY	..LOWER LIMIT.. ..UPPER LIMIT.	LOWER ACTIVITY UPPER ACTIVITY	...UNIT COST.. ...UNIT COST..	..UPPER COST.. ..LOWER COST..	LIMITING PROCESS.	AT
2	AVAILSW1	EQ	613.00000	.	.	613.00000 613.00000	612.99960 INFINITY	.	.	QLSW1OF1 NONE	LL
3	AVAILSW2	EQ	941.50000	.	.	941.50000 941.50000	542.94824 INFINITY	.	.	QLSW2OF2 NONE	LL
4	AVAILSW3	EQ	769.19995	.	.	769.19995 769.19995	503.02051 INFINITY	.	.	QLSW3OF3 NONE	LL
5	AVAILSW4	EQ	513.59985	.	.	513.59985 513.59985	383.60173 INFINITY	.	.	QLSW4OF4 NONE	LL
6	AVAILSW5	EQ	453.19995	.	.	453.19995 453.19995	393.21939 INFINITY	.	.	QLSW5OF5 NONE	LL
7	AVAILSW6	EQ	80.00000	.	.	80.00000 80.00000	70.67295 90.52900	14.00000 14.00000-	.	QBULSW5 QLSW5SW6	UL LL
8	AVAILSW7	EQ	1351.59985	.	.	1351.59985 1351.59985	837.39160 INFINITY	.	.	QLSW7OF7 NONE	LL
9	AVAILSW8	EQ	650.00000	.	.	650.00000 650.00000	287.92505 INFINITY	.	.	QLSW8CF8 NONE	LL
10	AVAILSW9	EQ	430.00000	.	.	430.00000 430.00000	139.50024 INFINITY	.	.	QLSW9OF9 NONE	LL
11	AVAILSW0	EQ	250.00000	.	.	250.00000 250.00000	194.05000 INFINITY	.	.	QLSW0OF0 NONE	LL
12	LSWU7	EQ	514.20825- 611.50049	.	.	QLSW7CF7 Q7LSW7	LL LL
13	LSWU8	EQ	362.07495- 154.52499	.	.	QLSW8CF8 QPLSW8	LL LL
14	AVAILGW1	EQ	187.00000	.	.	187.00000 187.00000	186.99960 INFINITY	.	.	QGW1OF1 NONE	LL
15	AVAILGW2	EQ	103.50000	.	.	103.50000 103.50000	101.95000 INFINITY	.	.	QGW2OF2 NONE	LL
16	AVAILGW3	EQ	94.89999	.	.	94.89999 94.89999	94.87458 INFINITY	.	.	QGW3OF3 NONE	LL
17	AVAILGW4	EQ	272.09985	.	.	272.09985 272.09985	142.10173 272.19735	3.00000 3.00000-	.	QLSW4OF4 QHW4R4	LL LL
18	AVAILGW5	EQ	254.59999	.	.	254.59999 254.59999	233.66850 277.86888	17.00000 17.00000-	.	GWR5 QLSW5R5	UL LL
19	AVAILGW6	EQ	130.00000	.	.	130.00000 130.00000	122.59500 140.52900	31.00000 31.00000-	.	GWR6 QLSW5SW6	UL LL
20	AVAILGW7	EQ	40.00000	.	.	40.00000 40.00000	34.90676 40.01750	66.66666 66.66666-	.	D71 AGREQ7	UL LL
21	AVAILGW0	EQ	10.00000	.	.	10.00000 10.00000	10.00000 10.00000	3.00000 3.00000-	.	GWR0 QWWR0	UL LL
28	MIREQ7	LL	10.00000	.	.	10.00000 NONE	10.00000 68.15532	52.89999- 52.89999	.	QLSW7M17 D71	LL UL
32	TLSW1M11	EQ00133- .	18.20000 18.20000-	.	QLSW1OF1 MIREQ1	LL LL
33	TLSW2M12	EQ	79.64943- .	17.15999 17.15999-	.	E21 MIREQ2	UL LL
34	TLSW3M13	EQ15852- .	11.35160 11.35160-	.	QGW3OF3 MIREQ3	LL LL
35	TLSW4M14	EQ	11.65881- .	21.91139 21.91139-	.	QBULSW5 MIREQ4	UL LL
36	TLSW5M15	EQ	9.32705- .	15.50560 15.50560-	.	QBULSW5 MIREQ5	UL LL
37	TLSW6M16	EQ	30.78233- .	12.60600 12.60600-	.	QBULSW5 MIREQ6	UL LL
38	TLSW7M17	EQ	INFINITY	36.00000- 36.00000	.	QLSW7M17 NONE	LL
39	TLSW8M18	EQ	88.47501- .	7.80000 7.80000-	.	D81 MIREQ8	UL LL
40	TLSW9M19	EQ	142.80000- .	6.50000 6.50000-	.	D91 MIREQ9	UL LL
41	TLSW0M10	EQ	50.69998- .	7.80000 7.80000-	.	D01 MIREQ0	UL LL
42	TGW1M11	EQ00040- .	18.20000 18.20000-	.	QGW1OF1 MIREQ1	LL LL
43	TGW2M12	EQ	1.55000- .	17.15999 17.15999-	.	QGW2OF2 MIREQ2	LL LL

Table B-4. Continued.

SECTION: 2 - COLUMNS AT LIMIT LEVEL

NUMBER	COLUMN	AT	ACTIVITY	INPUT COST	LOWER LIMIT UPPER LIMIT	LOWER ACTIVITY UPPER ACTIVITY	UNIT COST UNIT COST	UPPER COST LOWER COST	LIMITING PROCESS	AT
206	QBULSW4	LL	.	7.00000	. NONE	11.65881- 12.25186	3.07000- 3.03000	INFINITY 4.00000	QBULSW5 QLSW4SW5	UL UL
209	QUILSW3	LL	.	10.00000	20.00000	46.22236	10.00000- 10.00000	INFINITY .	QUIMPT 033	LL LL
210	QUILSW4	LL	.	10.00000	. NONE	12.25186	6.00000- 6.00000	INFINITY 4.00000	QUIMPT QLSW4SW5	LL UL
211	QUILSW5	LL	.	13.00000	56.99999	19.67294	3.00000- 3.00000	INFINITY 10.00000	QUIMPT QBULSW5	LL LL
213	QSALSW4	LL	.	8.00000	. 15.00000	73.95813- 12.25186	10.00000- 10.00000	INFINITY 2.00000	QLSW4SW5 QLSW4SW5	LL UL
215	QSAMPT	UL	22.40000	.	22.40000	13.07295 42.07294	6.00000 6.00000	6.00000 INFINITY	QBULSW5 QBULSW5	UL UL
216	PLSW1AG1	EQ	104.89999	.	104.89999 104.89999	104.89999 104.90074	.	INFINITY INFINITY	AGREQ1 QLSW1CF1	LL LL
217	QLSW1AG1	LL	.	5.00000	. NONE	.00074	5.00000- 5.00000	INFINITY .	AGREQ1 QLSW1CF1	LL LL
219	PLSW1MI1	EQ	7.20000	.	7.20000 7.20000	7.20000 7.20133	18.20000- 18.20000	INFINITY 13.20000	MIREQ1 QLSW1CF1	LL LL
220	QLSW1MI1	LL	.	31.00000	. NONE	.00133	49.20000- 49.20000	INFINITY 19.20000	MIREQ1 QLSW1CF1	LL LL
223	PLSW1MI4	EQ	10.00000	.	10.00000 10.00000	10.00000 10.00040	17.91139- 17.91139	INFINITY 17.91139	MIREQ4 QLSW1DF1	LL LL
224	PLSW2AG2	EQ	996.00000	.	996.00000 996.00000	996.00000 1130.12608	.	INFINITY INFINITY	AGREQ2 E21	LL UL
225	QLSW2AG2	LL	.	5.00000	. NONE	134.12608	5.00000- 5.00000	INFINITY .	AGREQ2 E21	LL UL
227	PLSW2MI2	EQ	36.00000	.	36.00000 36.00000	36.00000 115.64943	17.15999- 17.15999	INFINITY 17.15999	MIREQ2 E21	LL UL
228	QLSW2MI2	LL	.	32.00000	. NONE	79.64943	49.15999- 49.15999	INFINITY 17.15999	MIREQ2 E21	LL UL
231	QLSW2SW1	LL	.	4.00000	. 50.00000	.00040- 79.64943	4.00000- 4.00000	INFINITY .	QLSW1CF1 E21	LL UL
232	QLSW2SW3	LL	.	4.00000	. 129.99989	.15852- 46.22233	4.00000- 4.00000	INFINITY .	QGW3CF3 D33	LL LL
234	QLSW3AG3	LL	.	6.00000	. NONE	INFINITY- 610.50000	6.00000- 6.00000	INFINITY .	NCME PLSW3AG3	LL LL
236	PLSW3MI3	EQ	29.20000	.	29.20000 29.20000	29.20000 29.35851	11.35160- 11.35160	INFINITY 11.35160	MIREQ3 QGW3DF3	LL LL
237	QLSW3MI3	LL	.	43.00000	. NONE	.15852	54.35159- 54.35159	INFINITY 11.35159	MIREQ3 QGW3CF3	LL LL
240	PLSW3AG2	EQ	19.00000	.	19.00000 19.00000	19.00000 19.15852	.	INFINITY INFINITY	AGREQ2 QGW3DF3	LL LL
241	PLSW3SW4	EQ	71.00000	.	71.00000 71.00000	59.34118 71.15852	4.00000 4.00000	4.00000 INFINITY	QBULSW5 QGW3DF3	UL LL
242	QLSW3SW4	LL	.	4.00000	145.99992	11.65881- .15852	.	INFINITY 4.00000	QBULSW5 QGW3DF3	UL LL
244	QAG3LSW3	LL	.	.	. NONE	.	.	INFINITY .	AGEXC3 AGEXC3	UL UL
245	PLSW4AG4	EQ	713.50000	.	713.50000 713.50000	713.50000 714.80000	2.48448- 2.48448	INFINITY 2.48448	AGREQ4 QW4R4	LL LL
246	QLSW4AG4	LL	.	6.00000	. NONE	1.30000	8.48448- 8.48448	INFINITY 2.48448	AGREQ4 QW4R4	LL LL
248	PLSW4MI4	EQ	160.50000	.	160.50000 160.50000	160.50000 172.15882	21.91139- 21.91139	INFINITY 21.91139	MIREQ4 QBULSW5	LL UL
249	QLSW4MI4	LL	.	43.00000	. NONE	.09750- 11.65882	3.00000- 3.00000	INFINITY 3.00000	QW4R4 QAG4SW4	LL LL
253	QAG4LSW4	LL	.	.	. NONE	.	.	INFINITY .	AGEXC4 AGEXC4	UL UL
254	PLSW5AG5	EQ	879.29980	.	879.29980 879.29980	879.29980 895.38092	4.52500- 4.52500	INFINITY 4.52500	AGREQ5 QBULSW5	LL UL
255	QLSW5AG5	LL	.	5.00000	. NONE	16.08112	9.52500- 4.52500	INFINITY 4.52500	AGREQ5 QBULSW5	LL UL
257	PLSW5MI5	EQ	6.60000	.	6.60000 6.60000	6.60000 15.92705	15.50560- 15.50560	INFINITY 15.50560	MIREQ5 QBULSW5	LL UL
258	QLSW5MI5	LL	.	36.00000	. NONE	.932705	51.50560- 51.50560	INFINITY 15.50560	MIREQ5 QBULSW5	LL UL

Table B-4. Continued.

EXECUTOR. MPS/360 V2-M5

SECTION 3 - ROWS AT INTERMEDIATE LEVEL

NUMBER	...ROW..	AT	...ACTIVITY...	SLACK	ACTIVITY	..LOWER LIMIT. ..UPPER LIMIT.	LOWER ACTIVITY UPPER ACTIVITY	...UNIT COST.. ...UNIT COST..	..UPPER COST.. ..LOWER COST..	LIMITING PROCESS.	AT AT
22	MIREQ1	BS	10.00000	.	.	10.00000 NONE	10.00000 10.00133	INFINITY 49.20000		NONE QLSW1M11	LL
23	MIREQ2	BS	44.00000	.	.	44.00000 NONE	44.00000 173.64941	INFINITY 49.15999		NONE QLSW2M12	LL
24	MIREQ3	BS	49.70000	.	.	49.70000 NONE	49.70000 49.72541	INFINITY 53.00159		NONE QGW3M13	LL
25	MIREQ4	BS	302.50000	.	.	302.50000 NONE	302.50000 720.36572	INFINITY 62.56139		NONE QGW4M14	LL
26	MIREQ5	BS	17.00000	.	.	17.00000 NONE	17.00000 26.32705	INFINITY 51.50560		NONE QLSW5M15	LL
27	MIREQ6	BS	13.00000	.	.	13.00000 NONE	13.00000 43.78233	INFINITY 49.60599		NONE QLSW6M16	LL
29	MIREQ8	BS	7.00000	.	.	7.00000 NONE	7.00000 65.47501	INFINITY 58.79999		NONE QLSW8M18	LL
30	MIREQ9	BS	6.80000	.	.	6.80000 NONE	6.80000 149.59999	INFINITY 49.49999		NONE QLSW9M19	LL
31	MIREQ0	BS	1.50000	.	.	1.50000 NONE	1.50000 1.50000	INFINITY 44.89999		NONE RGW0M10	LL
50	AGREQ1	BS	124.00000	.	.	124.00000 NONE	124.00000 124.00043	INFINITY 4.90000		NONE QGW1AG1	LL
51	AGREQ2	BS	1034.00000	.	.	1034.00000 NONE	1034.00000 1168.12607	INFINITY 5.00000		NONE QLSW2AG2	LL
53	AGREQ4	BS	796.69995	.	.	796.69995 NONE	796.69995 797.99995	INFINITY 8.48448		NONE QLSW4AG4	LL
54	AGREQ5	BS	1017.89990	.	.	1017.89990 NONE	1017.89990 1033.98100	INFINITY 9.52500		NONE QLSW5AG5	LL
55	AGREQ6	BS	300.00000	.	.	300.00000 NONE	300.00000 320.48549	INFINITY 10.13425		NONE QLSW6AG6	LL
56	AGREQ7	BS	789.33319	.23333-	.	789.09985 NONE	789.33319 789.33319	INFINITY 4.62500		NONE RGW7AG7	LL
58	AGREQ9	BS	150.00000	.	.	150.00000 NONE	150.00000 319.99998	INFINITY 5.00000		NONE QLSW9AG9	LL
59	AGREQ0	BS	68.00000	.	.	68.00000 NONE	68.00000 69.00000	INFINITY 3.00000		NONE RGW0AG0	LL
76	TGW7AG7	BS	5.58213- INFINITY	61.66666 4.60000-		RGW7AG7 QGW7AG7	LL
77	TGW0AG0	BS INFINITY	3.00000 4.80000		RGW0AG0 QGW0AG0	LL
78	AGEXC3	BS INFINITY	INFINITY .		NONE QAG3LSW3	LL
80	AGEXC8	BS INFINITY	INFINITY .		NONE QAG8LSW8	LL
112	LSW6ST6	BS	55.99995- 107.08948	1.27750 .		QLSW6EV6 D64	LL
114	LSW7ST7	BS 5248.28516	. .		RLSW7ST7 D74	LL
116	LSW8ST8	BS 2236.31567	. .		RLSW8ST8 D84	LL
118	LSW9ST9	BS	1.00000- 545.24951	. .		QLSW9EV9 D94	LL
137	EVL5W7	BS	514.20825- .	. .		QLSW7EV7 RLSW7ST7	LL
138	EVL5W8	BS	362.07495- .	. .		QLSW8EV8 RLSW8ST8	LL
141	EVGW1	BS00040- .	. INFINITY		QGW1EV1 NONE	LL
142	EVGW2	BS	1.55000- 1.21972	. .		QGW2EV2 E23	LL
147	EVGW7	BS	5.09325- .	66.66666 INFINITY		QGW7EV7 NONE	LL
148	EVGH0	BS	3.00000 INFINITY		QGW0EV0 NONE	LL
189	GWRC1	BS	.	.	.	NONE	. .00040	INFINITY 3.00000		NONE QGW1B1	LL
190	GWRC2	BS	.	60.00000	.	NONE 60.00000	. 29.03999	INFINITY 3.00000		NONE QGW2R2	LL

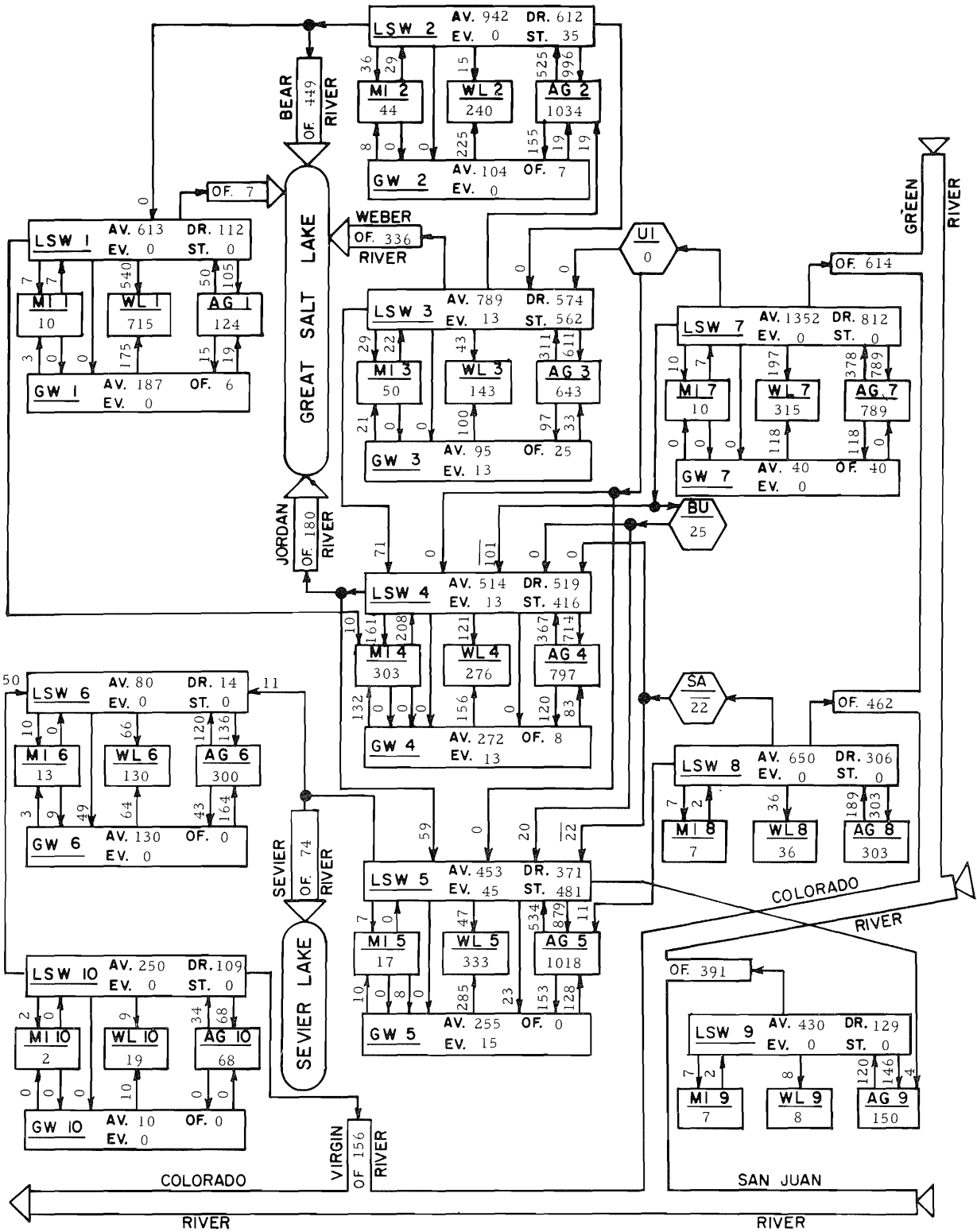
Table B-4. Continued.

EXECUTOR: MPS/360 V2-M5

SECTION 4 - COLUMNS AT INTERMEDIATE LEVEL

NUMBER	COLUMN	AT	ACTIVITY	INPUT COST	LOWER LIMIT UPPER LIMIT	LOWER ACTIVITY UPPER ACTIVITY	UNIT COST UNIT COST	UPPER COST LOWER COST	LIMITING PROCESS	AT
207	QHSLSW5	BS	19.67254	10.00000	28.99999	19.54613 19.67294	4.46875	10.00000 5.53125	QLSW3SW4 PLSW4ST4	LL UL
208	QRUMPT	BS	24.59118	.	136.59997	24.43266 24.59118	3.57500	3.57500-	QLSW3SW4 PLSW4ST4	LL UL
212	QUIMPT	BS	.	.	419.99995	24.59117	INFINITY 2.40000	INFINITY 2.40000-	NCNE QUILSW5	LL
214	QSALSW5	BS	22.39999	4.00000	.	13.07294 22.39999	6.00000 INFINITY	10.00000 INFINITY-	QSAMPT NONE	UL
218	RLSW1AG1	BS	104.89999	.	.	104.89999 104.90074	INFINITY 5.00000	INFINITY 5.00000-	NONE QLSW1AG1	LL
221	RLSW1M11	BS	7.20000	.	.	7.20000 7.20133	INFINITY 49.20000	INFINITY 49.20000-	NCNE QLSW1M11	LL
222	QLSW1WL1	BS	540.49951	.	.	540.49911 540.49951	2.40000 INFINITY	2.40000 INFINITY-	QCGW1WL1 NONE	LL
226	RLSW2AG2	BS	995.99976	.	.	995.99976 1130.12582	INFINITY 5.00000	INFINITY 5.00000-	NONE QLSW2AG2	LL
229	RLSW2M12	BS	35.99998	.	.	35.99998 115.64940	INFINITY 49.15999	INFINITY 49.15999-	NONE QLSW2M12	LL
230	QLSW2WL2	BS	14.95000	.	.	13.40000 14.95000	3.10000 INFINITY	3.10000 INFINITY-	QCGW2WL2 NONE	LL
233	PLSW3AG3	BS	610.49997	.	610.50000	INFINITY- 611.04567	6.00000	6.00000	QLSW3AG3 AGREQ3	LL LL
235	RLSW3AG3	BS	610.49976	.	.	610.46949 611.04545	6.70000	6.70000	QGW3AG3 AGREQ3	LL LL
238	RLSW3M13	BS	29.20000	.	.	29.20000 29.35851	INFINITY 54.35159	INFINITY 54.35159-	NCNE QLSW3M13	LL
239	QLSW3WL3	BS	43.04500	.	.	43.00407 43.04500	INFINITY	INFINITY-	AGREQ3 NONE	LL
243	RLSW3SW4	BS	70.99998	.	.	70.99998 71.15850	INFINITY	INFINITY	NONE QLSW3SW4	LL
247	RLSW4AG4	BS	713.49976	.	.	713.49976 714.79975	INFINITY 8.49448	INFINITY 8.49448-	NCNE QLSW4AG4	LL
250	RLSW4M14	BS	160.49998	.	.	160.49998 172.15880	INFINITY 64.91139	INFINITY 64.91139-	NCNE QLSW4M14	LL
251	QLSW4WL4	BS	120.64749	.	.	87.64723- 120.64749	7.70000 INFINITY	7.70000 INFINITY-	QCGW4WL4 NCNE	LL
252	QLSW4SW5	BS	59.19850	5.00000	.	59.19850 59.32532	4.46875	9.46875 5.00000	PLSW4ST4 QLSW3SW4	UL LL
256	RLSW5AG5	BS	879.29980	.	.	879.29980 895.38091	INFINITY 9.52500	INFINITY 9.52500-	NCNE QLSW5AG5	LL
259	RLSW5M15	BS	6.60000	.	.	6.60000 15.92705	INFINITY 51.50560	INFINITY 51.50560-	NCNE QLSW5M15	LL
260	QLSW5WL5	BS	47.15749	.	.	76.22601 47.15749	20.29999 INFINITY	20.29999 INFINITY-	QCGW5WL5 NONE	LL
261	QLSW5SW6	BS	10.52900	4.00000	.	10.52900 19.85604	INFINITY 10.00000	INFINITY 6.00000-	NONE QLSW5SW6	UL
265	RLSW6AG6	BS	136.09999	.	.	136.09999 156.58548	INFINITY 10.14425	INFINITY 10.14425-	NCNE QLSW6AG6	LL
268	PLSW6M16	BS	10.10000	.	.	10.10000 40.88233	INFINITY 48.60599	INFINITY 48.60599-	NCNE QLSW6M16	LL
269	QLSW6WL6	BS	65.79500	.	.	58.39000 65.79500	20.89999 INFINITY	20.89999 INFINITY-	QCGW6WL6 NONE	LL
271	QLSW7AG7	BS	.23333	5.00000	.	.23333 .23333	INFINITY 4.77500	INFINITY 4.22500	NCNE GWRC7	UL
272	RLSW7AG7	BS	789.33325	.	.	789.33325 789.33325	INFINITY 4.77500	INFINITY 4.77500-	NCNE GWRC7	UL
274	QLSW7M17	BS	.	36.00000	.	5.58213- 58.15530	64.91666 52.89999	100.91666 16.89999-	QGW7M17 MIRE07	LL LL
275	RLSW7M17	BS	10.00000	.	.	10.00000 68.15530	64.91666 52.89999	64.91666 52.89999-	QGW7M17 MIREJ7	LL LL
276	QLSW7WL7	BS	196.59999	.	.	186.41350 196.59999	34.39335 INFINITY	34.39335 INFINITY-	QCGW7WL7 NONE	LL
277	Q7LSW7	BS	611.50049	.	.	611.50049 611.50049	INFINITY 11.20574	INFINITY 11.20574-	NCNE QGW7EV7	LL
280	QLSW8AG8	BS	.	5.00000	.	.	INFINITY 4.99990	INFINITY .00010	NONE PLSW8AG8	UL

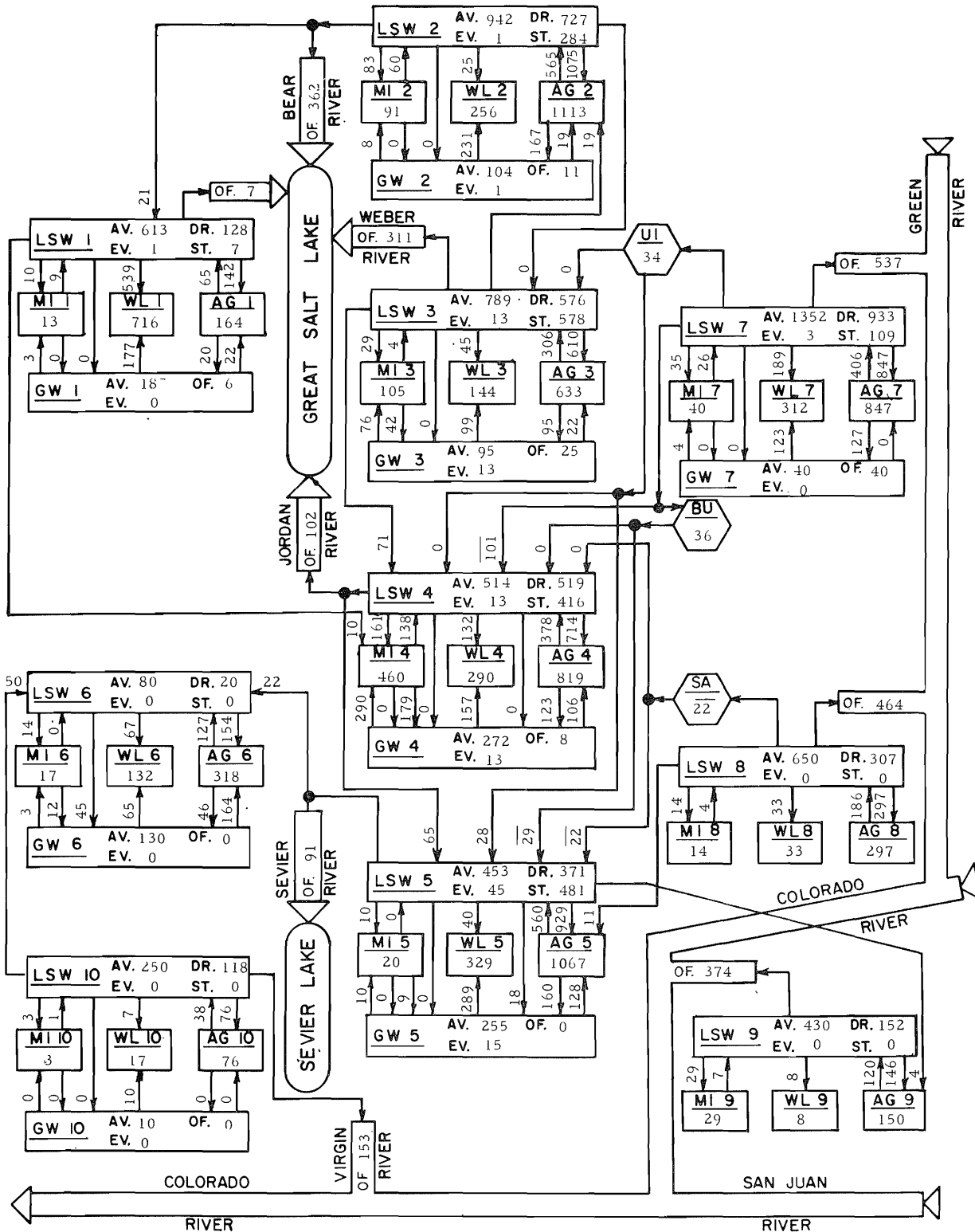
APPENDIX C
FLOW DIAGRAMS FOR ALLOCATION MODELS



FLOW DIAGRAM FOR ALLOCATION MODEL

(a) Theta = 0 (Time = 1965)

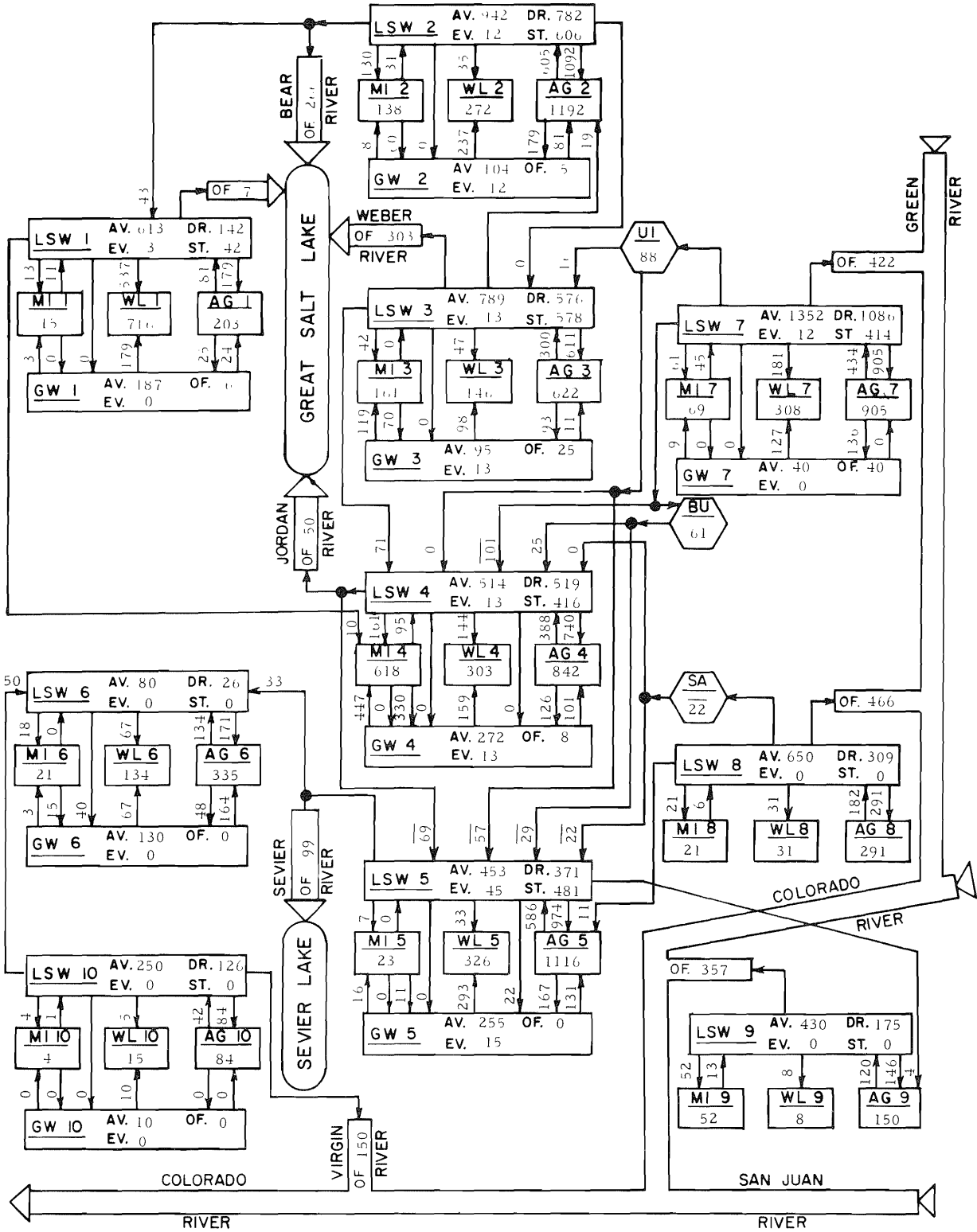
Figure C-1. Basic model.



FLOW DIAGRAM FOR ALLOCATION MODEL

(b) Theta = 2 (Time = 1976)

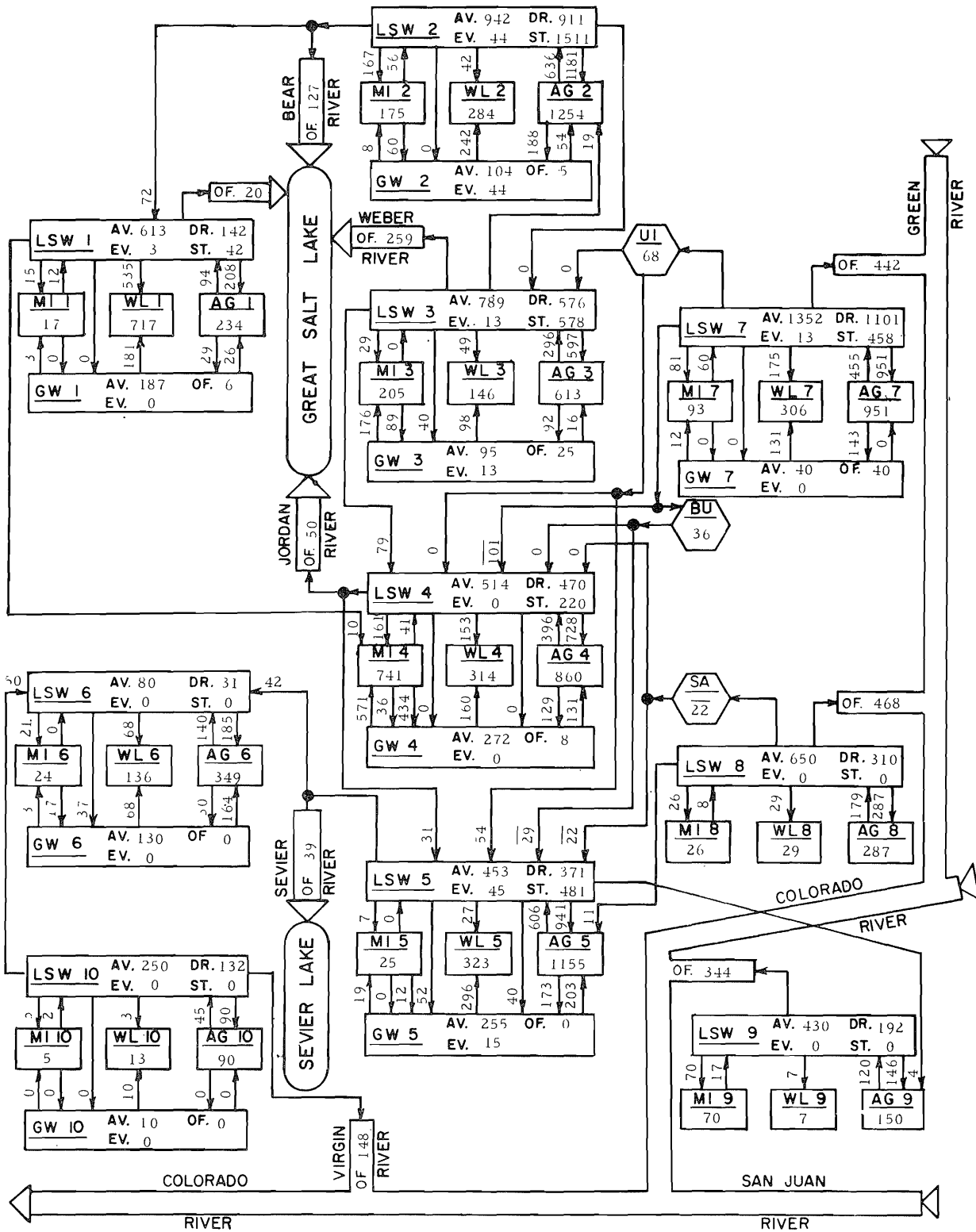
Figure C-1. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(c) Theta = 4 (Time = 1987)

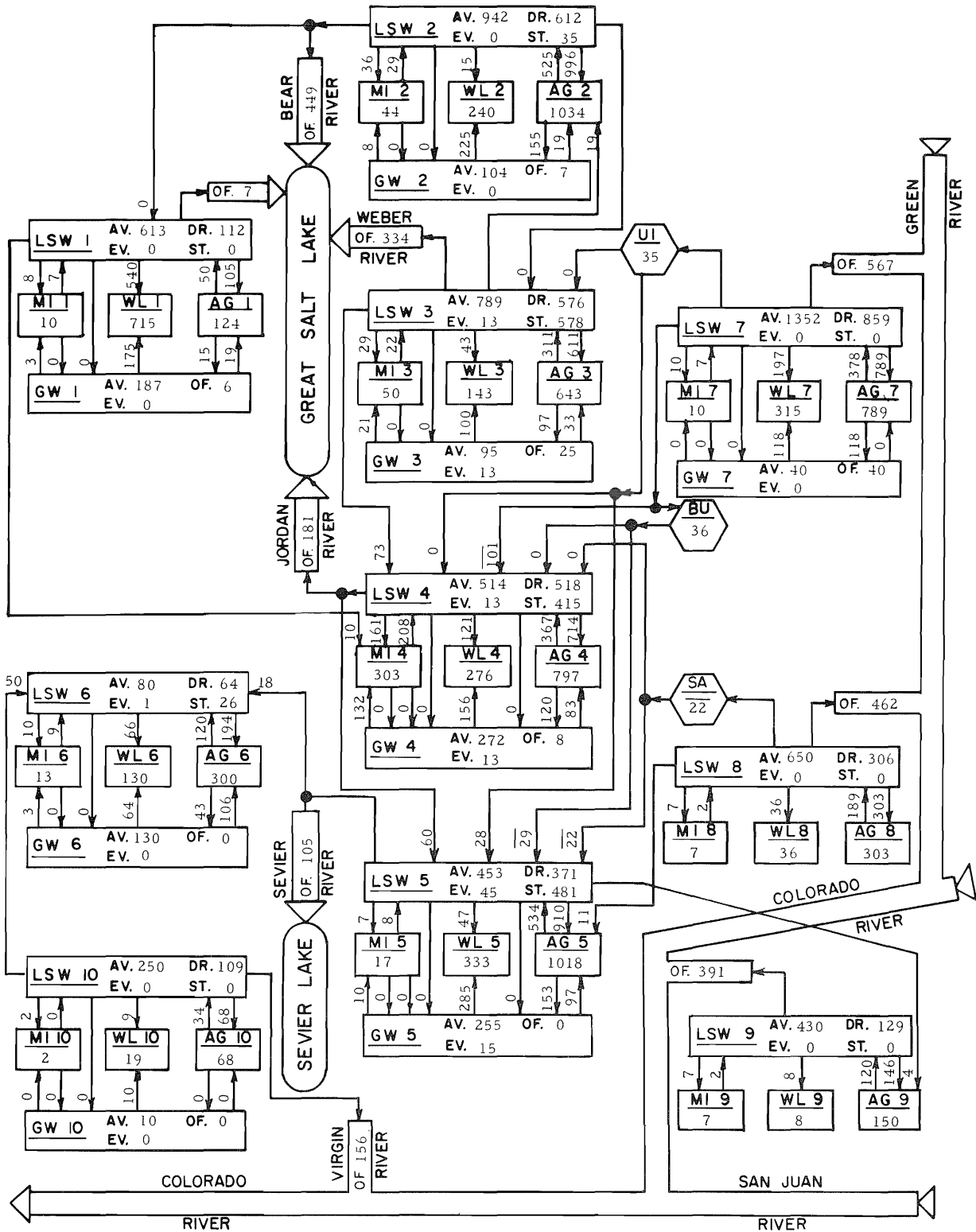
Figure C-1. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(d) Theta = 5.57104 (Time = 1996)

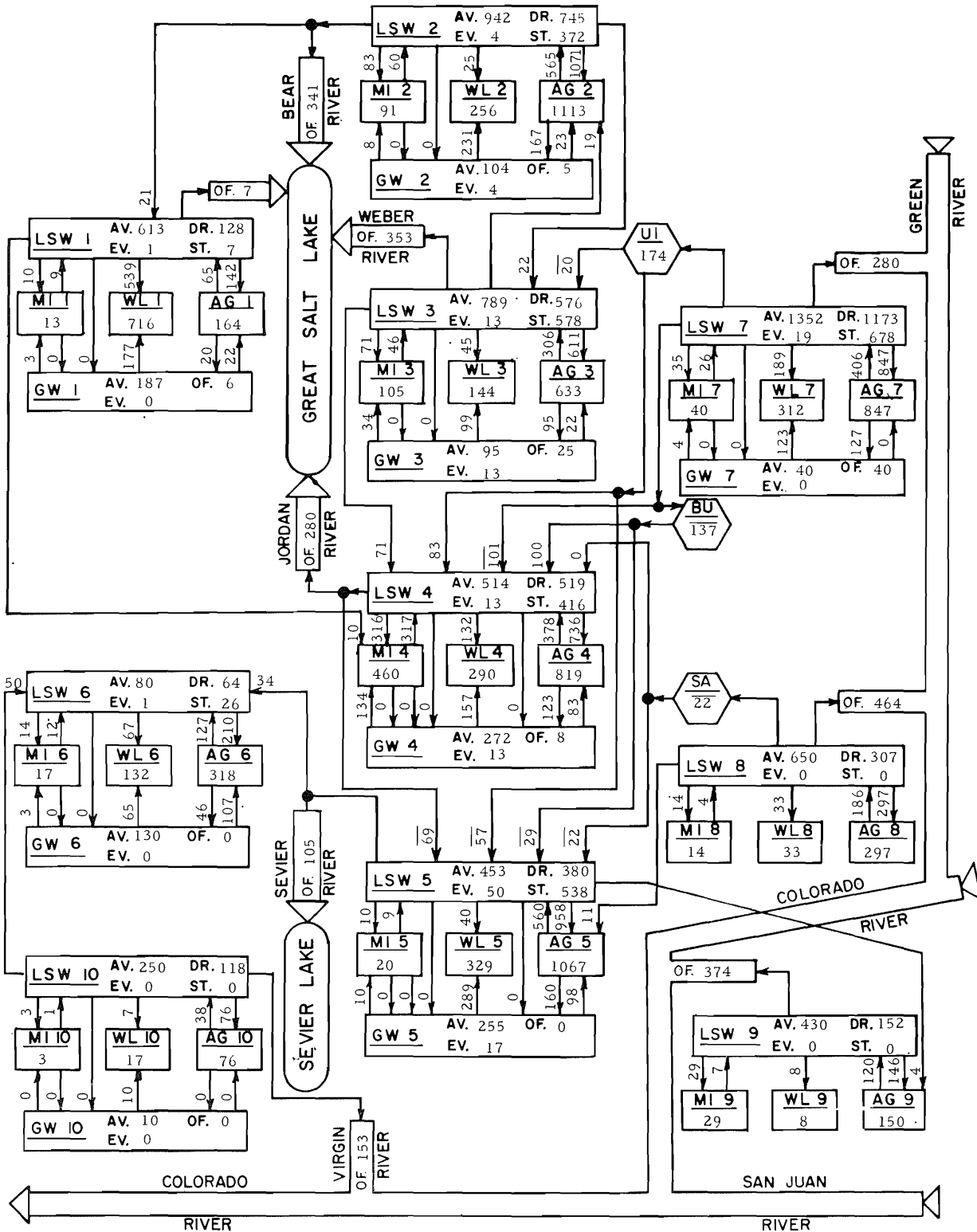
Figure C-1. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

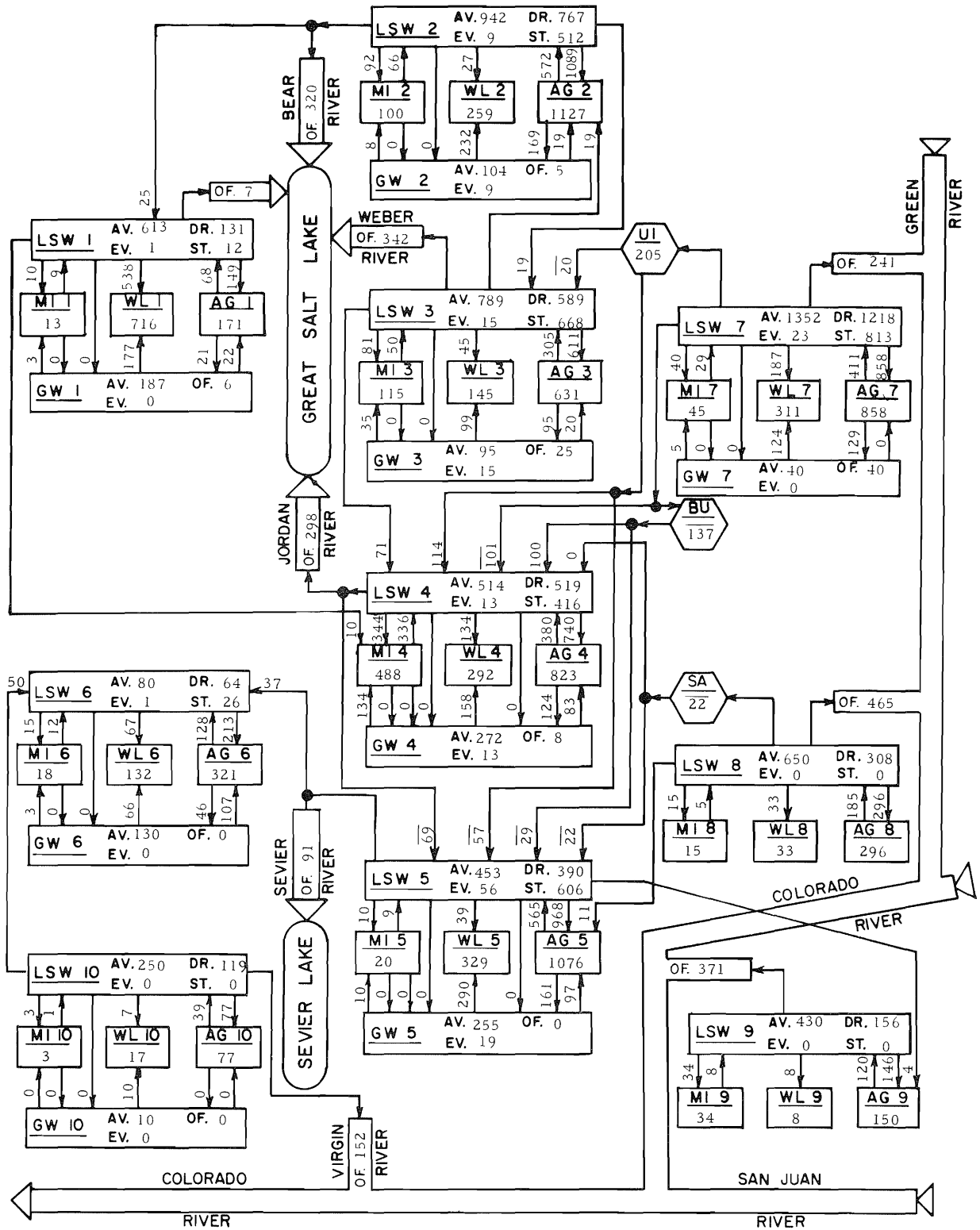
(a) Theta = 0 (Time = 1965)

Figure C-2. No groundwater recharge model.



FLOW DIAGRAM FOR ALLOCATION MODEL
 (b) Theta = 2 (Time = 1976)

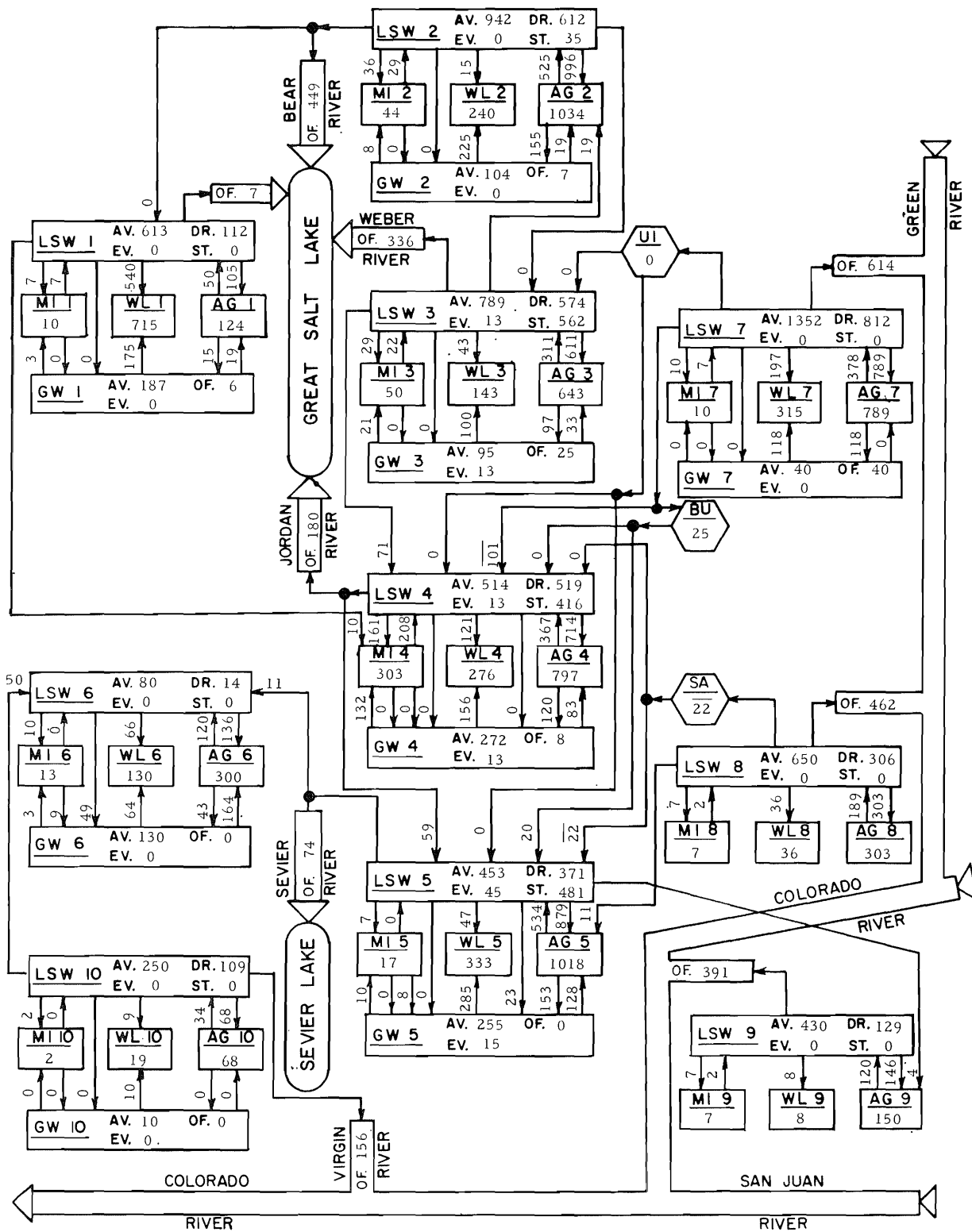
Figure C-2. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

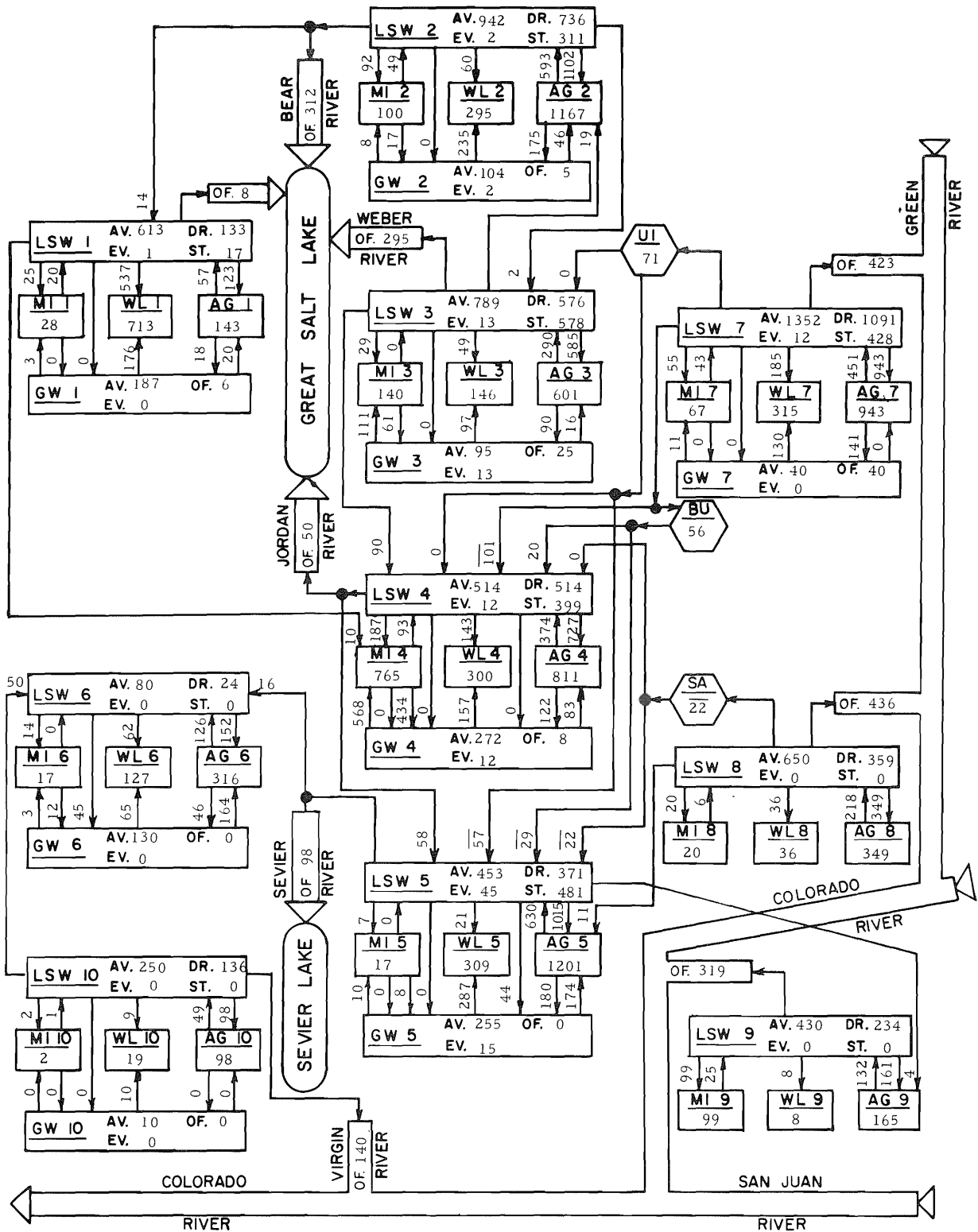
(c) Theta = 2.36025 (Time = 1978)

Figure C-2. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL
(a) Theta = 0 (Time = 1965)

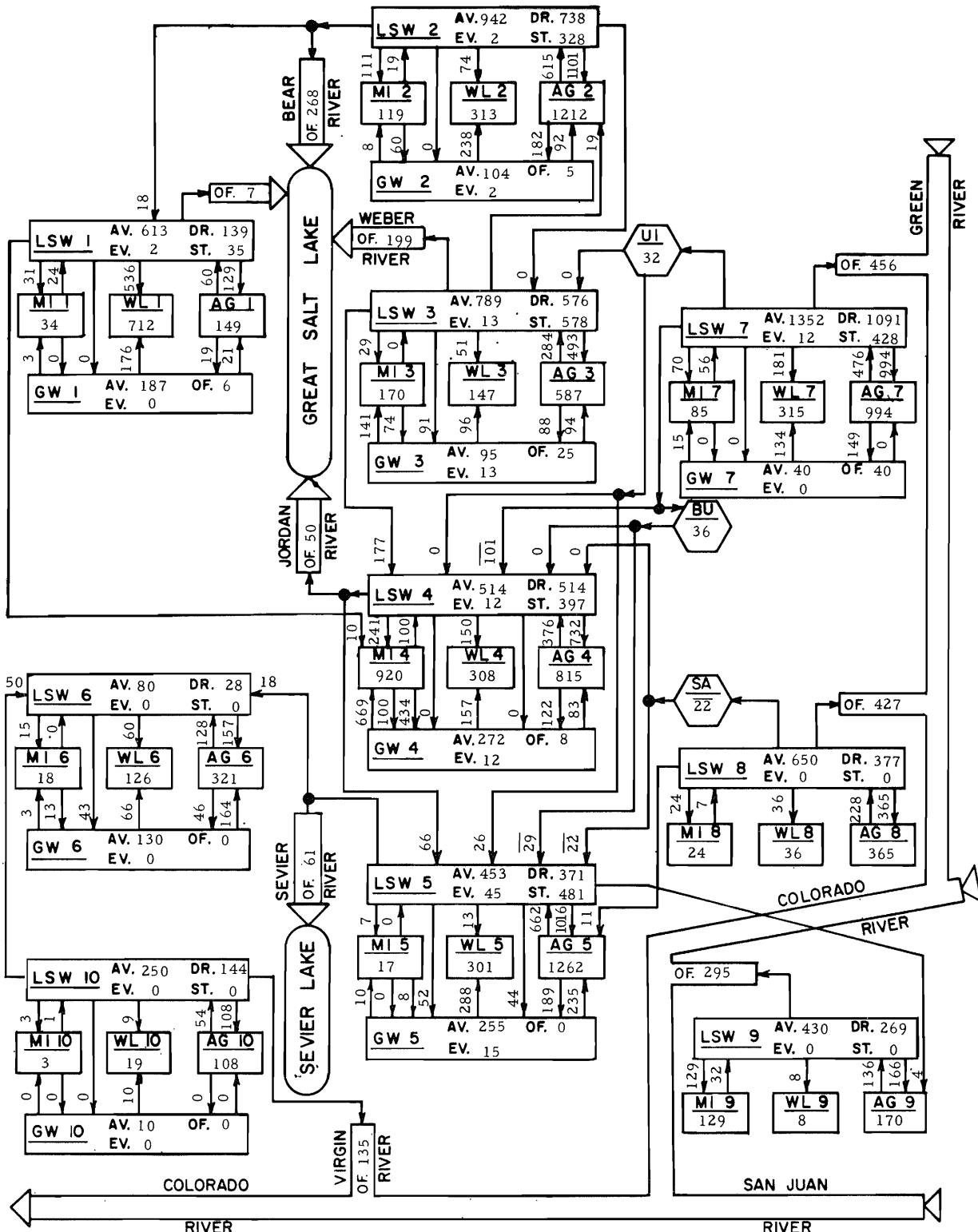
Figure C-3. No further groundwater development model.



FLOW DIAGRAM FOR ALLOCATION MODEL

(d) Theta = 6 (Time = 1998)

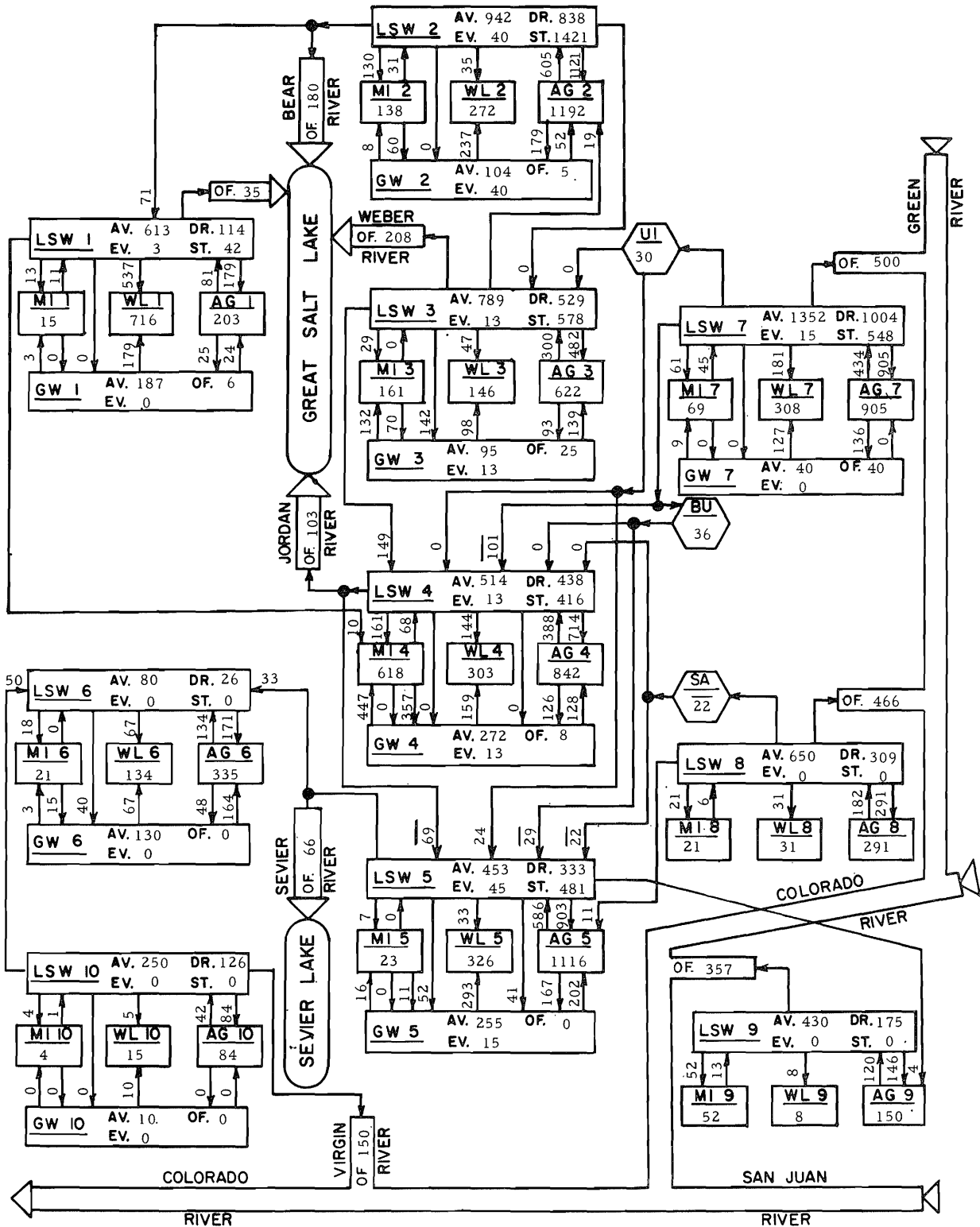
Figure C-5. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

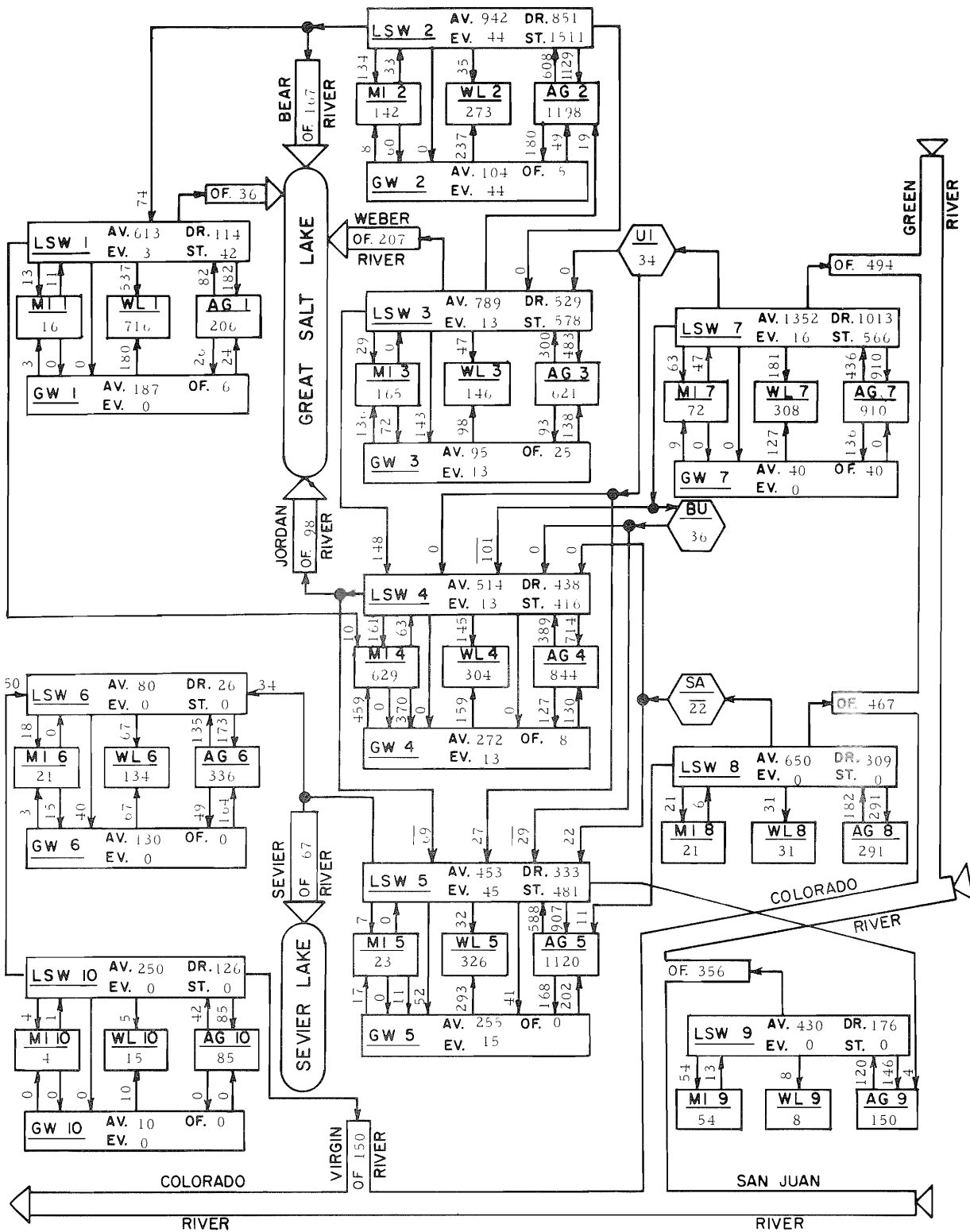
(e) Theta = 8 (Time = 2009)

Figure C-5. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL
(c) Theta = 4 (Time = 1987)

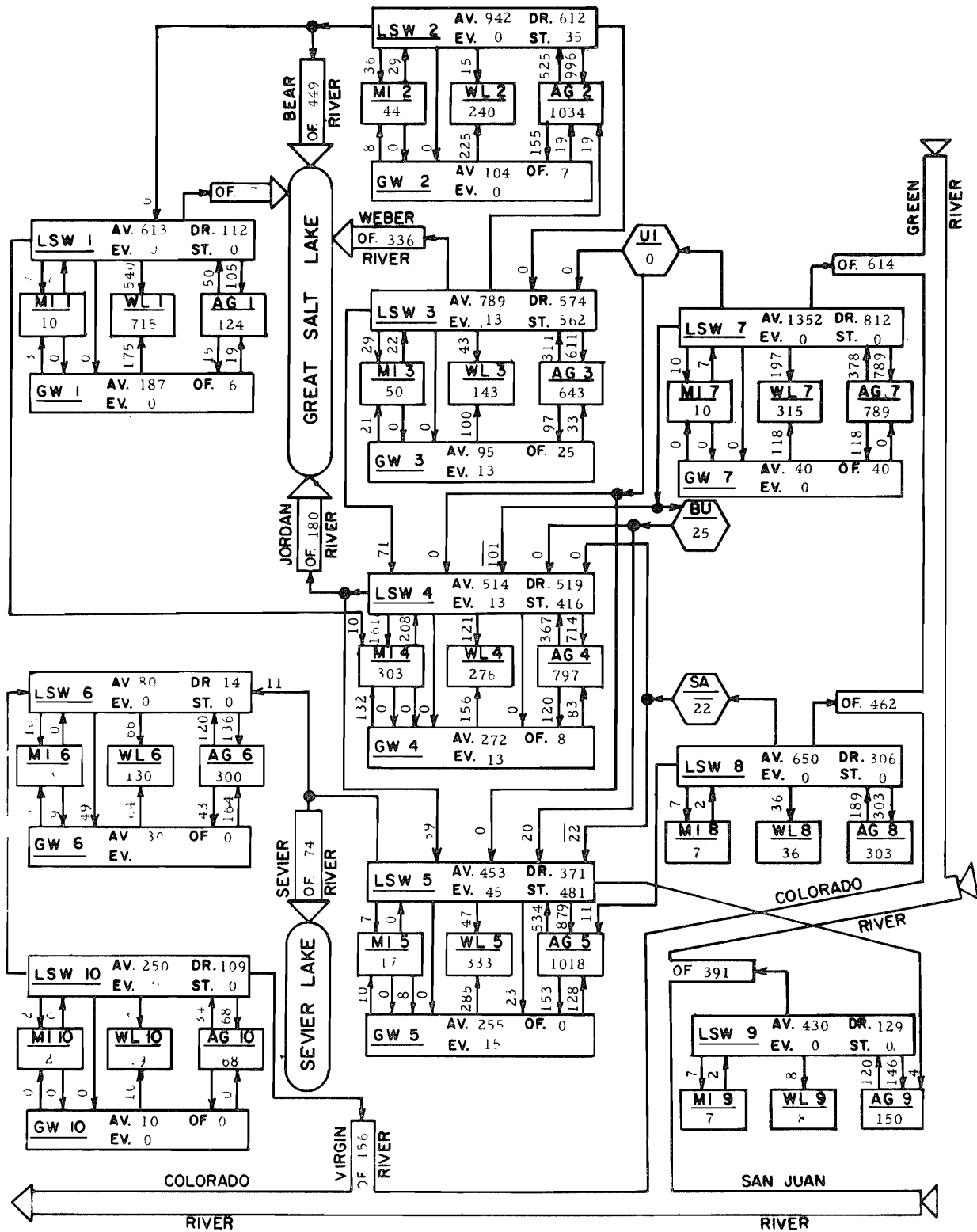
Figure C-7. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

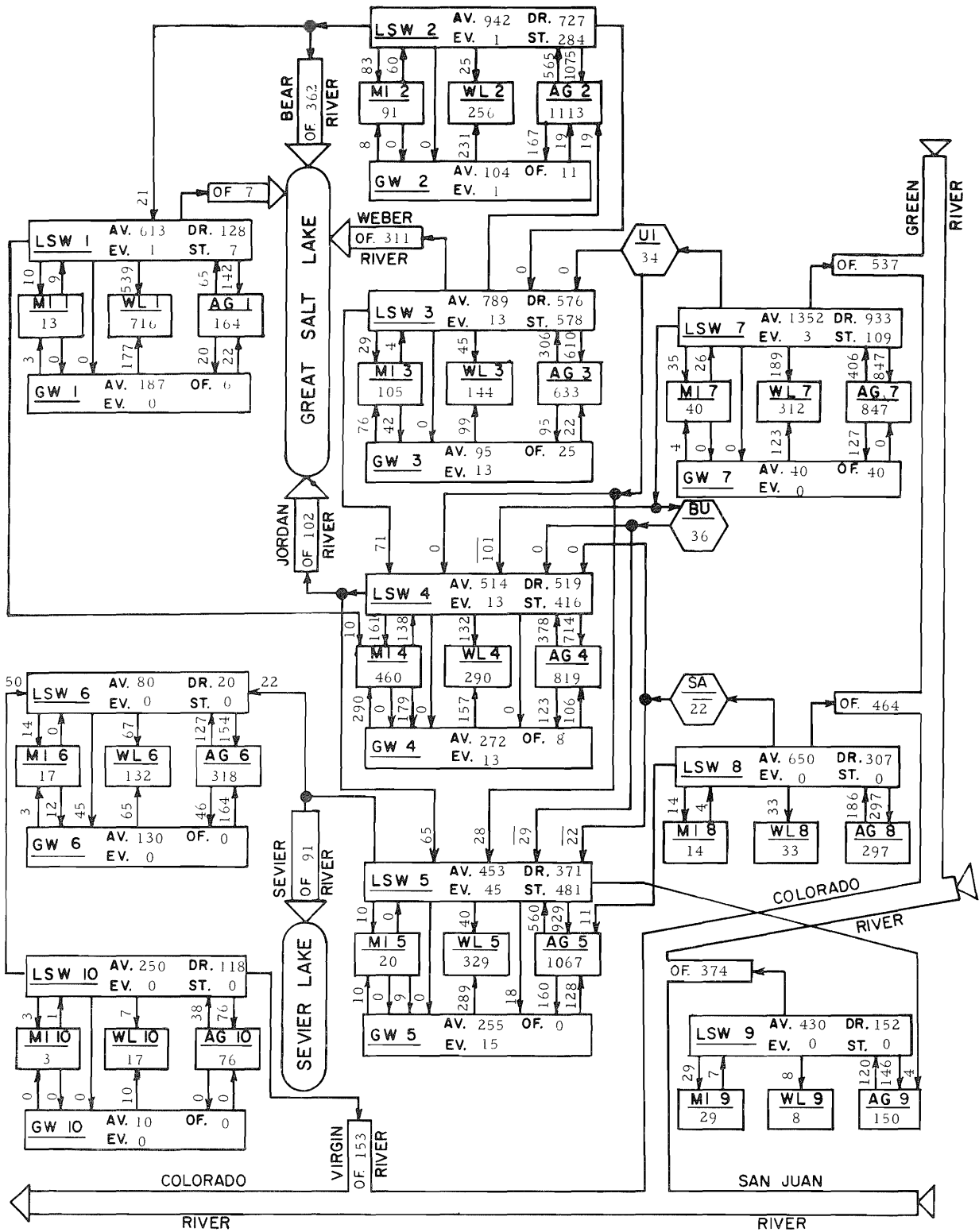
(d) Theta = 4.14770 (Time = 1988)

Figure C-7. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL
 (a) Theta = 0 (Time = 1965)

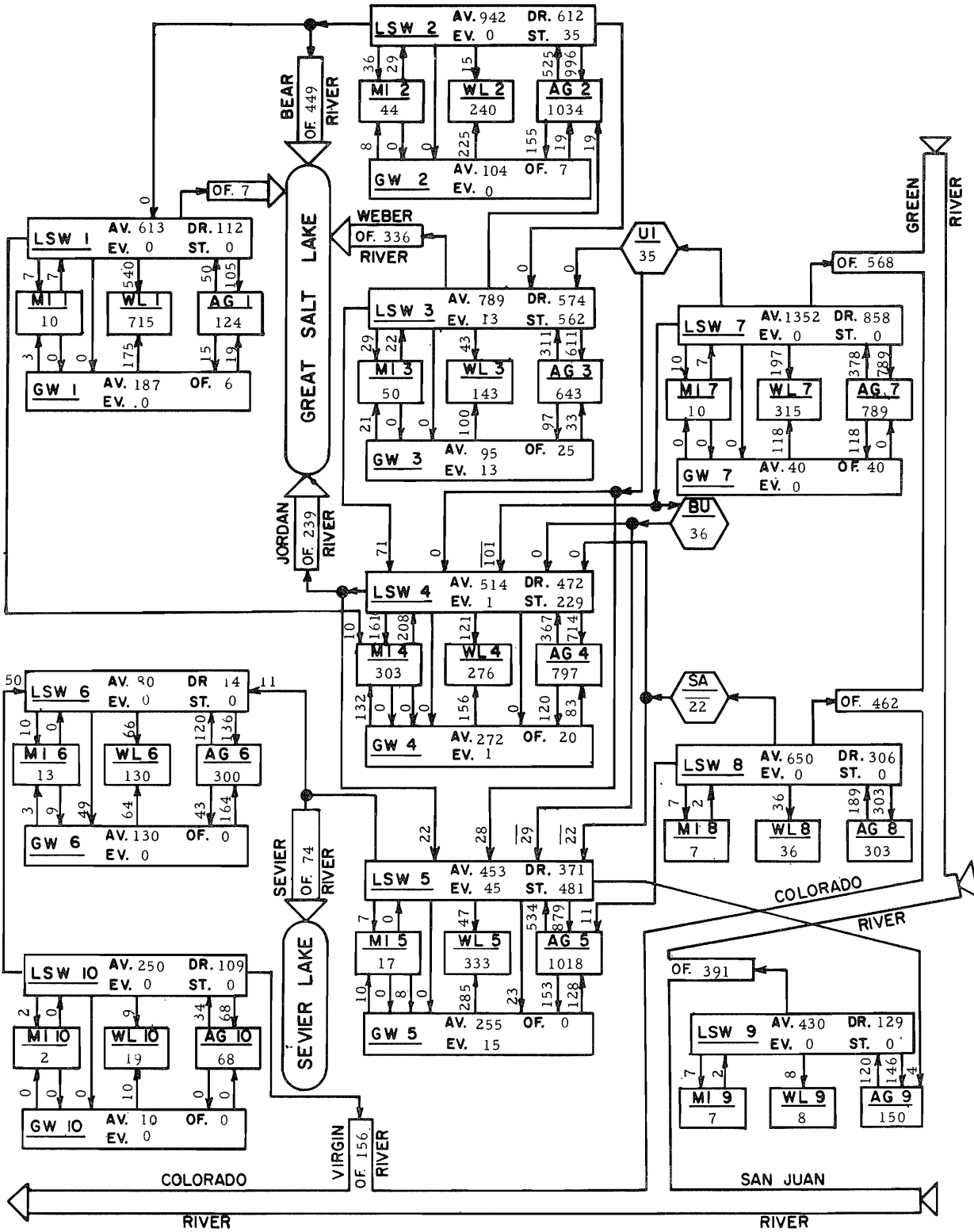
Figure C-8. Inflow to Great Salt Lake $\geq 800,000$ ac-ft/yr model.



FLOW DIAGRAM FOR ALLOCATION MODEL

(b) Theta = 2 (Time = 1976)

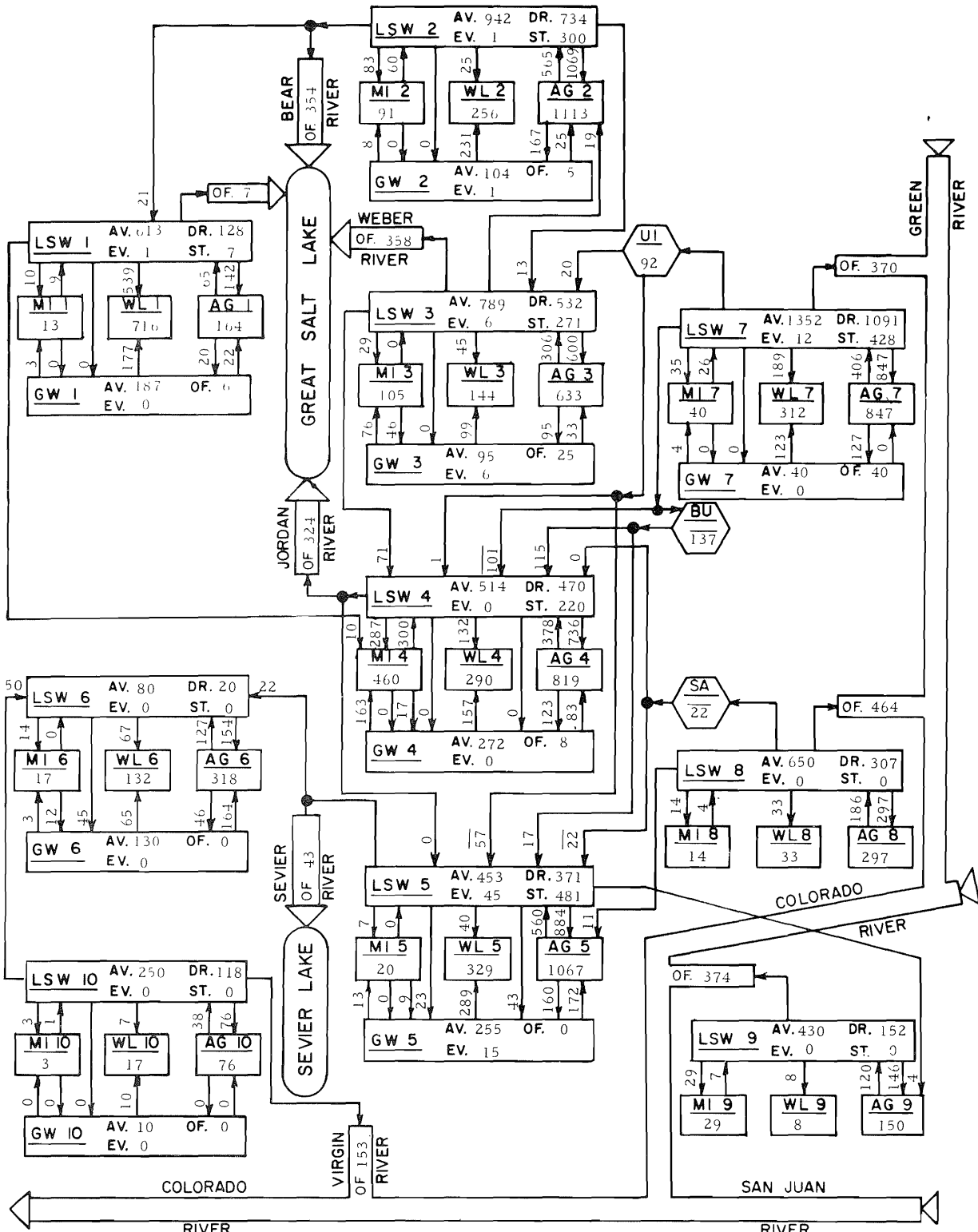
Figure C-8. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(a) Theta = 0 (Time = 1965)

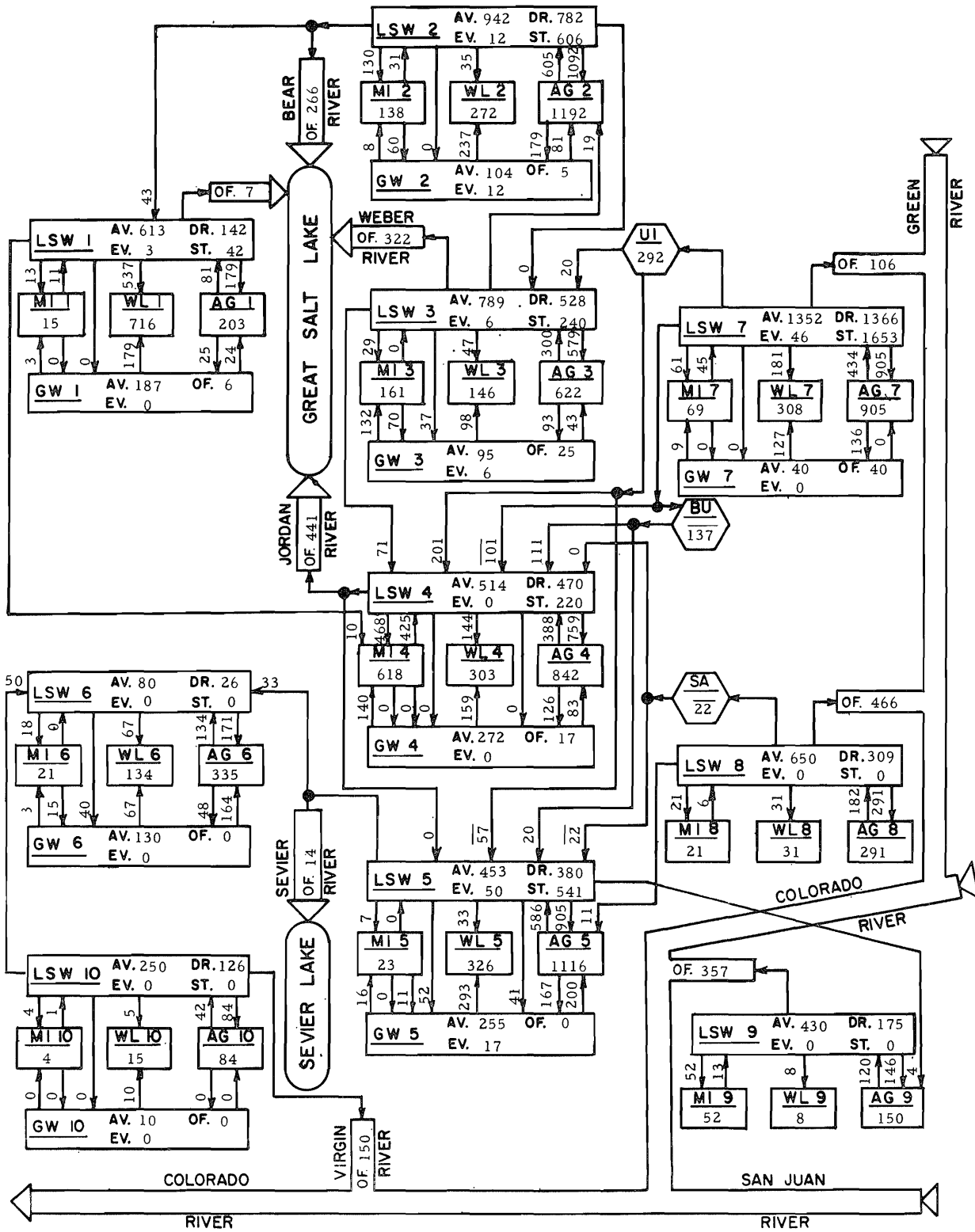
Figure C-9. Inflow to Great Salt Lake $\geq 1,088,000$ ac-ft/yr model.



FLOW DIAGRAM FOR ALLOCATION MODEL

(b) Theta = 2 (Time = 1976)

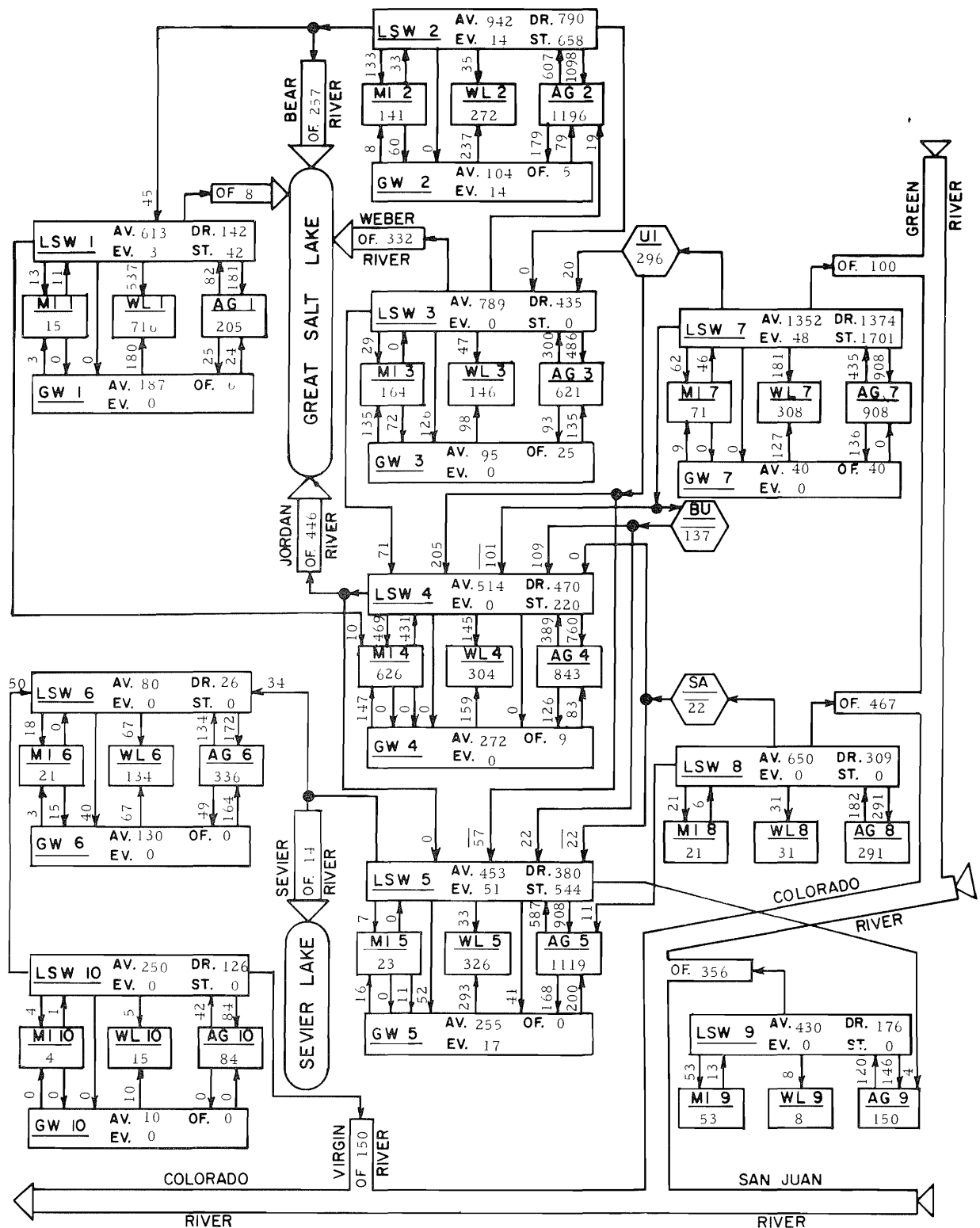
Figure C-9. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(c) Theta = 4 (Time = 1987)

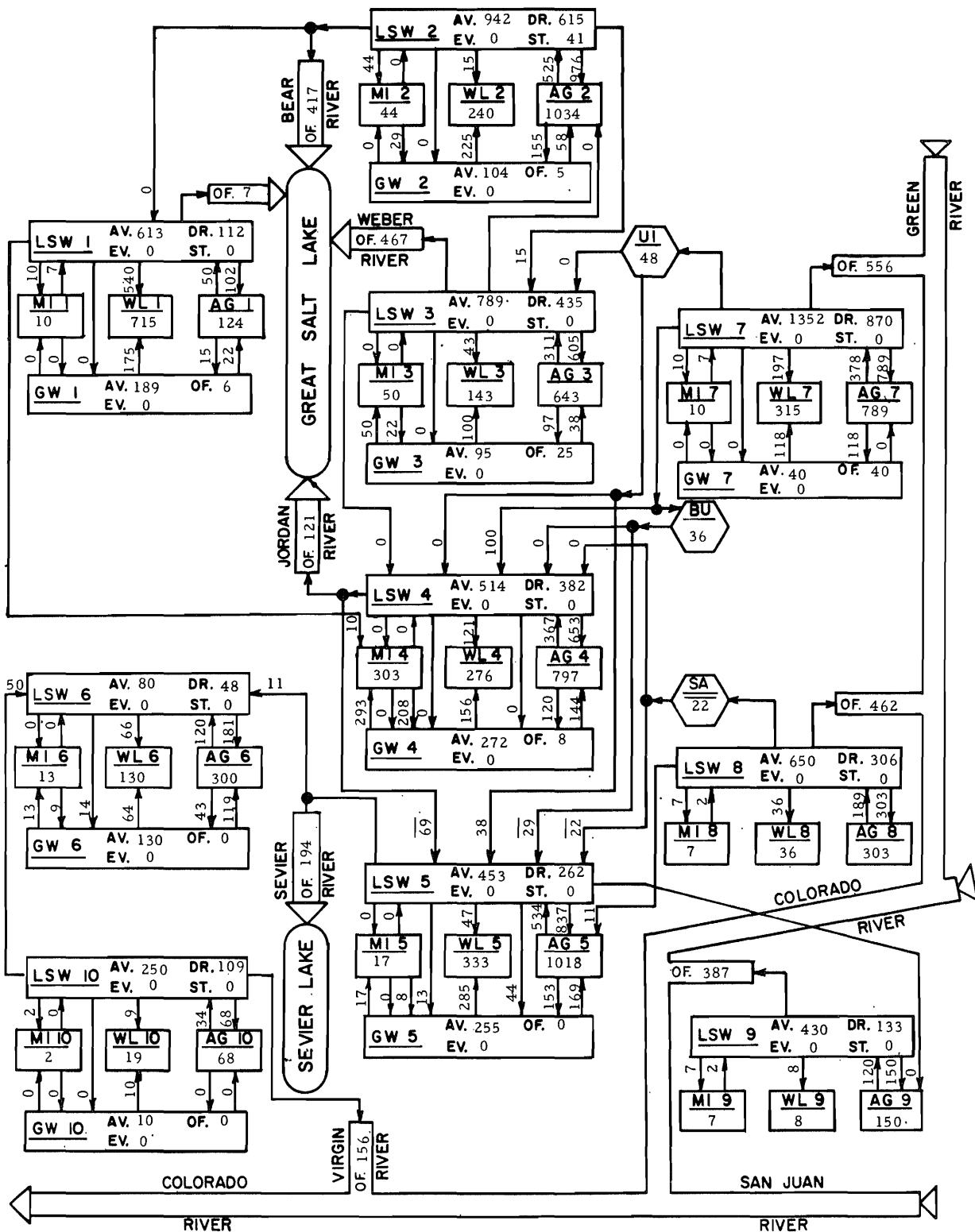
Figure C-9. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(d) Theta = 4.10232 (Time = 1988)

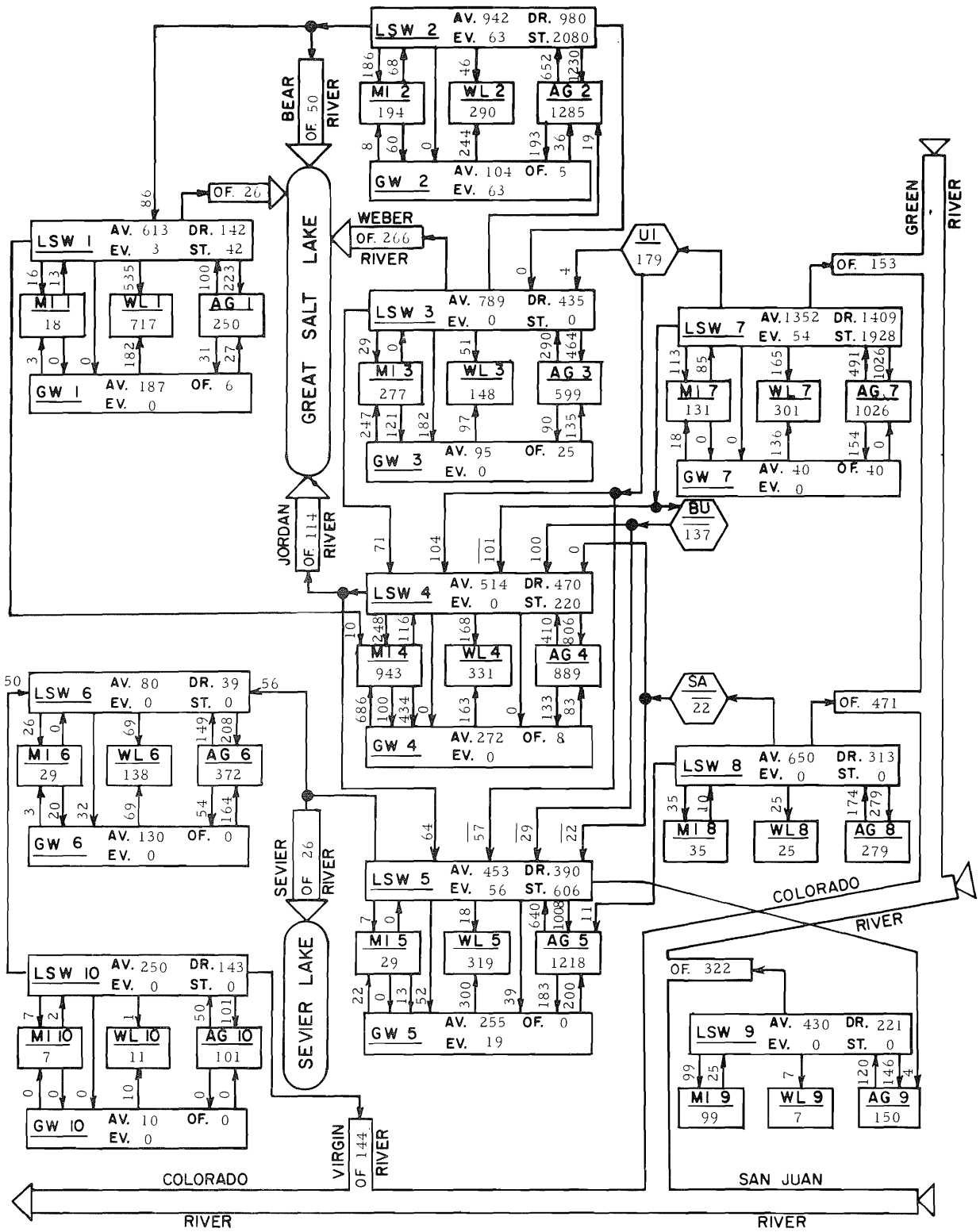
Figure C-9. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(a) Theta = 0 (Time = 1965)

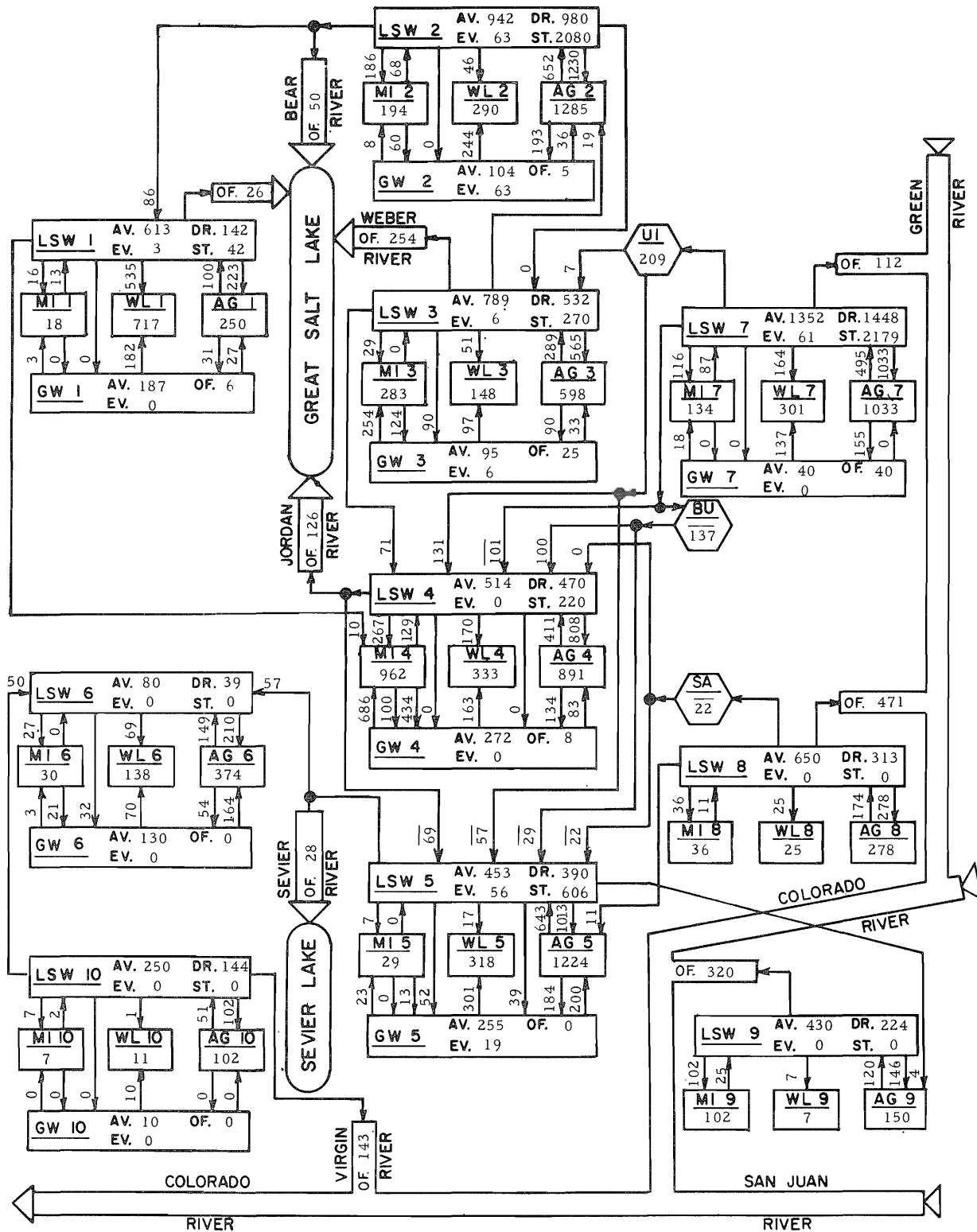
Figure C-10. No previous development model.



FLOW DIAGRAM FOR ALLOCATION MODEL

(h) Theta = 8.13826 (Time = 2010)

Figure C-11. Continued.



FLOW DIAGRAM FOR ALLOCATION MODEL

(i) Theta = 8.37806 (Time = 2011)

Figure C-11. Continued.