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## Runoff Estimates for Small Rural Watersheds and Development of a Sound Design Method: Volume I. Research Report

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Report No. FHWA-RD-77-158

Bill Grenney peak flow  
797-3186

# **RUNOFF ESTIMATES FOR SMALL RURAL WATERSHEDS AND DEVELOPMENT OF A SOUND DESIGN METHOD**

**Vol. I Research Report**



**October 1977**

**Final Report**

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**Prepared for**  
**FEDERAL HIGHWAY ADMINISTRATION**  
**Offices of Research & Development**  
**Washington, D. C. 20590**

## FOREWORD .

This report is composed of three volumes: Volume I is the Research Report; Volume II consists of recommendations for establishing design manuals and Appendices B, C, D, E, F, G, and H, which are the design aids required for establishing design manuals; Volume III consists of Appendix A, an accumulation of the data base used in the study. FHWA chose to arrange the report as described to facilitate distribution of the results. The methods reported herein and designated as the Federal Highway Administration Methods are designed to be applied to watersheds smaller than 50 square miles but may be used on areas up to 100 square miles in size.

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Charles F. Schefrey

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16. Abstract Potter's method for runoff peak forecasting was examined on its original watersheds and it was found to be soundly conceived. The method was modified to extend it to other watersheds in the same States for which it was originally developed. The results after modifying Potter's C parameter were found to be satisfactory.  The method was simplified and extended to all of the contiguous United States, Alaska, Hawaii, and Puerto Rico. After beginning with a 7-parameter method, it was found that 3-parameters, namely area, rainfall erosivity factor and difference in elevation from top to bottom of the watershed produced peak flow estimates of virtually equal reliability. The standard error of estimates of the ten year peak flow as computed by the different equations for a random sample of 51 watersheds from throughout the United States and Puerto Rico were: 39 percent for the 3-parameter all zone, 62 percent for 7-parameter all zone, 50 percent for 3-parameter zone, and 38 percent for 7-parameter zone equations. Consequently the three parameter equations were selected for design purposes and nomographs for solving the equations were developed for each hydrophysiographic zone of the United States and Puerto Rico. The other volumes of this report are:  <table border="0"> <tr> <td><u>FHWA-RD-</u></td> <td><u>Subtitle</u></td> </tr> <tr> <td>77-159</td> <td>Vol II Recommendations for preparing design manuals and Apps B, C, D, E, F, and H</td> </tr> <tr> <td>77-160</td> <td>Vol III App A "Data and Frequency Curves"</td> </tr> </table>						<u>FHWA-RD-</u>	<u>Subtitle</u>	77-159	Vol II Recommendations for preparing design manuals and Apps B, C, D, E, F, and H	77-160	Vol III App A "Data and Frequency Curves"
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# LIST OF SYMBOLS

- A Area of watershed acres, 1000's of acres or square miles.
- L' The length of the principal stream channel from the proposed crossing to the end of the solid or broken blue line shown on the USGS map defining Potter's principal stream channel. L' is in miles.
- T Topographic factor of Potter. L' is in miles.
- $$T = \frac{(0.7L')^{1.5}}{(E_2 - E_1)^{1/2}} + \frac{(0.3L')^{1.5}}{(E_3 - E_2)^{1/2}}$$
- P<sub>60</sub> Precipitation factor. This is the 10-year 60-minute rainfall at the centroid of the drainage area. Inches.
- DD Drainage density. This is the total length of all channels in blue on the topographic map, or the LL of the watershed, in miles, divided by the area, A, in thousands of acres. Miles/1000 acres.
- C factor Potter's correction factor. It is the number by which a 10-year peak estimated from area, topographic factor, and precipitation factor must be multiplied to obtain the correct estimate of the 10-year peak flow or  $q_{10}/\hat{q}_{10(ATP)}$  as derived from  $T/\hat{T}_{AP}$ .
- q<sub>10</sub> The 10-year flood peak flow, cubic feet per second or thousands of cubic feet per second. Other subscripts indicate other return periods.
- $\bar{q}_{10}$  Average of the measured 10-year peak flow. In general, the bar over a symbol denotes an average or mean value.
- $\hat{q}_{10}$  Estimated 10-year peak flow.
- $\hat{q}_{10(A)}$  Estimated 10-year peak flow based on area A. In general, the symbols enclosed in the subscript parentheses denote the independent variables used in obtaining the estimate.
- r Simple correlation coefficient.
- t "Students" t for testing the level of probability of differences.
- e Potter's error percentage =  $\left( \frac{\hat{T}_{AP} - T}{\hat{T}_{AP}} \right) 100.$
- Mcfs Thousands of cubic feet per second.



a Acres.

Mi Mile.

ft Foot or feet.

in Inch.

AH or DH Difference in elevation between the top of a watershed and the gaging site--feet.

L Length in miles up the principal channel from the proposed crossing to the upper watershed boundary.

LL The total length in miles of all stream channels within a watershed. On a USGS map all channels show in solid or dashed blue lines.

s Slope gradient, feet/100 feet.

S Storage, percent of the area of a watershed occupied by storages such as swamps, lakes, and playas.

ARS Agricultural Research Service.

SCS Soil Conservation Service.

USGS U.S. Geological Survey.

B.P.R. Bureau of Public Roads--Federal Highway Administration.

FHWA Federal Highway Administration.

$E_1$  Elevation at culvert site. Feet above reference datum.

$E_2$  Elevation at a point 0.7L upstream from culvert site. Feet above reference datum.

$E_3$  Elevation at the most remote point on the watershed opposite the upper end of the main channel. Feet above reference datum.

$\hat{q}_{10}(AP)$   $\hat{q}_{10}$  estimated utilizing A and P.

$\hat{q}_{10}(ATP)$   $\hat{q}_{10}$  estimated utilizing A, T, and P.

$\hat{T}_{(AP)}$   $\hat{T}$  estimated from A and P.

$\hat{q}_{10}(ATPC)$   $\hat{q}_{10}$  estimated utilizing A, T, P, and C.

$\hat{q}_{10}(ATPK)$   $\hat{q}_{10}$  estimated utilizing A, T, P, and K.

- $s_1$  Slope of channel segment from culvert site upstream a distance of 0.7L. Feet/mile.
- $s_2$  Slope of channel segment from the watershed boundary downstream a distance of 0.3L. Feet/mile.
- R The mean annual rainfall kinetic energy times the maximum respective 30-minute annual maximum rainfall.
- K USU correction factor. It is the number by which a 10-year peak estimated from area, topographic factor, and precipitation factor must be multiplied to obtain the correct estimate of the 10-year peak flow or  $q_{10}/\hat{q}_{10(ATP)}$ .

x Independent variables in regression or graphical correlation.

y Dependent variable in regression or graphical correlation.

$PS_e$  The standard error of a regression equation in its linear form as a percent of the mean value of the measured dependent variable in its linear form. For most of the regression equations derived in this report, the  $\log_{10}$  transformation was used, therefore

$$PS_e = \frac{100}{\log_{10} \bar{y}} \sqrt{\frac{\sum (\log_{10} y - \log_{10} \hat{y})^2}{df}}$$

$PS_{EE}$  The standard error of a point estimate made from any estimating equation as a percentage of the mean value of the measured dependent variable in its original untransformed state. For the equations derived in this report

$$S_{EE} = \frac{100}{\bar{y}} \sqrt{\frac{\sum (y - \hat{y})^2}{n - 2}}$$

n Number of observations.

df Degrees of freedom for hypothesis testing, in general,  $df = n - k - 1$ .

k Is the number of independent variables used in an estimating equation.

DY or  $\Delta Y$  Residual difference between the observed value of a dependent variable and the corresponding computed or estimated value.

$L'_2$  Potter's length of lower reach of channel from culvert site elevation  $E_1$  to upstream elevation  $E_2$ ; a distance of 0.7  $L'$  in miles.

$L'_3$  Potter's length of upper reach of channel from point elevation  $E_2$  to the end of the solid or broken blue line at elevation  $E_3$ ; a distance of 0.3  $L'$  in miles.

## CONVERSION FACTORS

### English to International System

1 mile	= 1.609 kilometers (km)
1,000 cubic feet per second (cfs)	= 28.32 cubic meters/second ( $\text{m}^3/\text{s}$ )
1 foot	= 0.3048 meters (m)
1 acre (a)	= 0.4047 hectares (ha) or 4047 square meters ( $\text{m}^2$ )
1 square mile ( $\text{mi}^2$ )	= 640 acres = 2,590 square kilometers ( $\text{km}^2$ ) = 259 ha
1 cubic foot	= 0.02832 cubic meters ( $\text{m}^3$ )
1 acre-foot	= 1,233 cubic meters = 0.01028 hectare meters (ha-m)
1 ton/acre/year	= 2.24 tonnes/hectare/year (tonnes (t)/ha/yr)
inches (in)	= 25.4 millimeters (mm) = 2.54 centimeters (cm)

## VOLUME I: RESEARCH REPORT

### RUNOFF ESTIMATES FOR SMALL RURAL WATERSHEDS AND DEVELOPMENT OF A SOUND DESIGN METHOD

by

Joel E. Fletcher, A. Leon Huber,  
Frank W. Haws, and Calvin G. Clyde

#### Introduction

##### Problem statement

A basic consideration in the design of bridges and culverts is the estimation of the rate of runoff expected during peak flow periods. Watershed area alone does not account for the wide variation in peak rates of runoff found among watersheds. Area alone is insufficient even within homogeneous physiographic areas. The addition of other indices such as precipitation, topography, etc., is needed to reduce the unexplained variation in the magnitude of peak rates of runoff to workable limits. However, this technique does not always explain the large discrepancies between the estimated and measured peaks that sometimes occur.

Potter (ref. 13) realized that a design engineer cannot always wait for streamflow measurements. He used watershed data at hand and developed the method commonly known as the "Bureau of Public Roads Method" or simply Potter's method. This method is the subject of the present investigation.

##### Objective

The objective of this study is to improve the accuracy of Potter's method using more recent data while retaining its basic simplicity and independence from complicated computational aides.

##### Statement of work

Work conducted in the present investigation proceeded as follows:

1. Verify, update, and extend Potter's method.
2. Refine Potter's method and other methods.
3. Field visits and evaluation.
4. Evaluation and analysis.

## Summary of Potter's Method

### Introduction

The method proposed by Potter (ref. 13) depends on a graphical correlation between the logs of the 10-year peak flow and the logs of the area, a topographic factor, a precipitation factor, and a correction factor largely representing the drainage density. The procedure begins with an estimate of the 10-year peak,  $q_{10(ATP)}$ , based on the area, a topographic factor,  $T$ , and a precipitation factor,  $P$ . A built-in check using the topographic factor,  $T$ , estimates an approximate error at this point. If the error is greater than 30 percent, the correction factor,  $C$ , is utilized. Prior to any computations the unknown watershed is classified from a physiographic map into a zone depending on and reflecting its climatic and geologic properties.

### Procedure

1. Zone. Locate the desired crossing on a United States Geological Survey (USGS)  $7\frac{1}{2}$  minute quadrangle topographic map. Carefully outline the watershed boundaries leading to the location of the desired culvert. By eye locate the center of mass of the enclosed area and record its latitude and longitude. With this location enter Potter's zone map and read the proper zone.

2. Area. Carefully measure the watershed area outlined in No. 1. This may be done by planimetering, digitizing, or counting the intersections or dots on a dense grid. The area is expressed in thousands of acres.

3. Topographic factor,  $T$ . On the topographic map, carefully measure the distance up the main stream channel to its most remote point shown as either a solid or broken blue line. This distance is expressed in miles and designated as  $L'$ . Record the elevation,  $E_1$ , at the culvert site. At a point  $0.7 L$  upstream from the culvert site again read the elevation. This is elevation  $E_2$ . Lastly determine the elevation of the most remote point on  $L'$ . This elevation is  $E_3$ . From these elevations  $E_1$ ,  $E_2$ , and  $E_3$  and distances  $L'_2$  and  $L'_3$  the topographic factor,  $T$  is calculated as follows:

$$T = 0.7L'/(s_1)^{\frac{1}{2}} + 0.3L'/(s_2)^{\frac{1}{2}} \quad . . . . . (1)$$

where  $0.7L' = L'_2$  and  $0.3L' = L'_3$

$$\text{or} \quad T = \frac{(0.7L')^{1.5}}{(E_2-E_1)^{\frac{1}{2}}} + \frac{(0.3L')^{1.5}}{(E_3-E_2)^{\frac{1}{2}}} \quad . . . . . (2)$$

4. Precipitation factor. Utilizing the location of the centroid of the watershed to the nearest second, locate this centroid on the 1-hour 10-year rainfall map and read the precipitation to the nearest 0.01 inch. This value is recorded as the precipitation factor,  $P_{60}$ .

5. Drainage density. Measure the total length of all drainage channels shown as blue lines on the watershed map. This length in miles is called  $LL$ . Drainage density,  $DD$ , is computed by dividing this length by the watershed area in thousands of acres. The units of  $DD$  are miles per thousand acres.

6. "C" factor. Potter (ref. 13) constructed correction factor curves by plotting the logs of the ratio,  $q_{10}/\hat{q}_{10(ATP)}$ , against the logs of the ratio,  $T/\hat{T}_{AP}$ . After he had obtained the curves, he called the values of the ratio  $q_{10}/\hat{q}_{10(ATP)}$ , read from the curves "C." Thus "C" is the value the estimated  $\hat{q}_{10(ATP)}$  must be multiplied by to obtain  $\hat{q}_{10(ATPC)}$ .

7. Potter's flood frequency. Potter (ref. 12) developed what can be termed an upper and lower frequency curve method for extrapolating short records to determine the less frequent flood peaks. In Potter's own words,

Frequency studies of the maximum annual peak rates of runoff were made for each watershed. In order to minimize the error that might result from the many short periods of runoff record, relations between peak rates of high and low frequency were used to obtain values of peaks that could be expected to be equalled or exceeded on an average of once in 10 years ( $q_{10}$ ) and once in 50 years ( $q_{50}$ ). It has been found that when the maximum annual peaks for these watersheds were plotted on extremal probability paper, they defined two straight line frequency curves. (Potter, ref. 12)

A high degree of correlation ( $r^2 > 0.9$ ) was found to exist between the 10-year peak as defined by the lower curve and the 10- and 50-year peaks as defined by the upper curve. Thus, it is only necessary to determine the  $q_{10}$  on the lower curve to estimate the true  $q_{10}$  or  $q_{50}$ . The lower curve is usually well defined by even short records.

8. Peak flow estimate. Enter the appropriate zone curves with the area, A, the topographic factor, T, and the precipitation factor, P, and read peak flow  $\hat{q}_{10}$ . This yields a value for  $\hat{q}_{10(ATP)}$ . Now, enter the corresponding curves for the relation between T, A, and P and read a value for  $\hat{T}_{AP}$ . Express the difference between this value and the measured value of T as a percentage of error or

$$\text{error} = \frac{\hat{T}_{AP} - T}{\hat{T}_{AP}} \times 100$$

If this error is smaller than 30 percent, the estimate of  $q_{10(ATP)}$  is considered to be an adequate estimate of the 10-year peak flow. If the error is greater than 30 percent, the estimate  $q_{10(ATP)}$  must be multiplied by a "C" value obtained by entering the correction curve with  $T/\hat{T}_{AP}$ .

9. Errors and assumptions described by Potter.

a. Map scale. Potter (ref. 13) did not specify the scale of the USGS maps from which he determined the watershed parameters. He presumably preferred the 1/24,000 maps. From Potter's notes, however, it appears that four or five of his watersheds were located in areas where only 1/62,500 scale maps were available. He made no mention of any adjustments for map scale.

b. Area. Potter used the published USGS areas which are in square miles and converted them to thousands of acres by multiplying by the factor 0.640. He recognized that these areas are subject to periodic revision.

## PHASE I

### Verify, Update, and Extend Potter's Method

#### Parameters and measurement

1. Map scales. Only two map scales were encountered on the Potter watersheds, 1/24,000 and 1/62,500. No corrections were made for map scale until the section on areal extension of the Potter method. These effects will be discussed at that time.

2. Area. Potter's areas and those determined by Utah State University (USU) differ by only about 1 percent (1.08). Errors in area appear insignificant so no correction for map scale need be made. The major differences found were due to new and more accurate maps which actually indicated some differing watershed boundaries.

3. Operator differences. The differences between operators were small and non-significant so long as the same external controls were applied. In the most complex operation, that of determining the topographic factor, T, the differences, though larger, were still not significant as indicated by Students t test (ref. 16). Both sets of topographic factors are given in Table 1 (p. 5-9) along with other data on the Potter watersheds. These data are basic to all additional studies in this paper.

4. Interrelationships among parameters. Potter (ref. 13) stated that he found a close correlation between drainage density, DD, and the topographic factor, T, and thus decided to discard DD in favor of T. The Utah State University data show a significant correlation between DD and T but there is sufficient residue to justify retention of DD. Furthermore, there are better interrelationships among all of the other parameters that Potter did retain than between DD and T.

5. Comparison between Potter's original curves and new curves using Potter's data. The values of the 10-year peak flow,  $q_{10}$ , were determined by USU as outlined by Potter (ref. 12). For example, Figure 1 (p. 10) shows the extraction of the  $q_{10}$  upper value for Potter Zone I Watershed 19. In this particular 16 years of record the values read from the upper and lower frequency curves are not materially different. The second example is for a shorter (10-year) record station, Figure 2 (p. 11). In this example, there is a sharp break upward because of the outlier at 0.99 thousand cubic feet per second (Mcfs)

The regression line for the relationship between Potter's 10-year peak flows,  $q_{10}$ , and the USU 10-year peak flows,  $q_{10}$ , has a one to one slope and an  $r^2$  value of 0.94 using 96 watersheds. The two series of peak flows are not significantly different as indicated by the t test.

The derivation of the curves for the relationships between the individual parameters and the 10-year peak flow can also be influenced by operator differences. The curves for the relationships between  $q_{10}$  and area were fitted graphically as Potter described plus a least squares fit in the ordinary

Table 1. Data for Potter watersheds.

1	2	3	4	5	6	7	8	9	10	11	12	13		
SEQ #	POTTER #	ARS #	USGS #	STATION NAME	LAT	LONG	USU AREA	PT AREA	PTR ZN	PR AREA	PTR	USU P60	PTR T	USU T
A 1	1-1	31.3		ARS PENNINORE WISCONSIN WATERSHED W-111	48-42	93-34	.853	8.851	1	C10	2.2	2.88	0.827	0.831
A 2	1-2	31.4		ARS PENNINORE WISCONSIN WATERSHED W-112	48-42	93-34	0.171	8.171	1	C10	2.2	2.88	0.827	0.831
A 3	1-3	17.4		ARS EDWARDSVILLE ILLINOIS WATERSHED W-IV	38-51	98-34	8.298	8.298	1	C6	2.5	2.27	0.121	0.121
A 4	1-4	31.1		ARS PENNINORE WISCONSIN WATERSHED W-1	42-42	93-34	8.338	8.338	1	C10	2.2	2.88	0.827	0.831
A 5	1-5	44.1		ARS HASTINGS NEBRASKA WATERSHED W-3	48-40	99-49	8.481	8.481	1	B14	2.4	2.48	0.288	0.279
A 6	1-6		8545688	RALSTON CR AT IOWA CITY IOWA	41-40	91-29	1.928	1.938	1	C9	2.2	2.23	0.478	0.647
A 7	1-8		83247188	PATTERSON RUN NR OHENSVILLE OHIO	39-57	84-88	2.138	2.148	1	B18	2.0	1.97	0.772	1.084
A 8	1-9		84198188	NORWALK CR NR NORWALK OHIO	41-12	82-31	3.140	2.988	1	B15	2.0	1.79	0.949	1.062
A 9	1-10		94286188	PLUM CR AT OBERLIN OHIO	41-18	82-19	3.891	3.128	1	B15	1.9	1.76	1.048	1.285
A 10	1-10		9595888	HICKORY CR ABOVE LAKE BLOOMINGTON ILL	48-37	98-07	6.464	6.468	1	C86	2.0	1.78	1.798	1.989
A 11	1-20		8138888	BOND CR AT OUNHAM BASIN NEW YORK	43-20	73-34	9.408	9.410	1	B9	1.8	1.66	0.868	1.444
A 12	1-22		85418588	EP GALENA R COUNCIL HILL ILLINOIS	42-29	90-17	12.864	12.868	1	C18	2.2	2.88	1.228	1.619
A 13	1-12A		81581888	SAGE BROOK NR SOUTH NEW BERLIN NEW YORK	42-32	75-28	.448	8.448	1	B8	1.7	1.78	0.848	0.877
A 14	1-13A		83182888	CLEAR CR AT DILKORTH OHIO	41-27	88-41	.582	8.582	1	B15	1.9	1.78	0.131	0.280
A 15	1-18B		81485888	COLD SPRING BROOK AT CHINA NEW YORK	42-18	78-34	.988	8.988	1	B8	1.7	1.76	0.138	1.098
A 16	1-38		94218188	HOBKINS CR AT HARTSGROVE OHIO	41-36	91-88	3.278	4.448	1	B8	1.8	1.69	1.398	1.338
A 17	1-39		81598888	ALBRIGHT CT AT EAST HOMER NEW YORK	42-42	78-07	4.388	4.83	1	B8	1.8	1.82	0.488	0.570
A 18	1-41		83884888	WALNUT CR AT COURTLAND OHIO	41-21	88-42	5.488	5.83	1	B8	1.8	1.72	1.318	1.367
A 19	1-42		93181888	SUGAR RUN AT PYMATUNG OAK PENNA	41-29	88-38	5.978	5.98	1	B15	1.9	1.66	0.938	1.184
A 20	1-44		81388888	QUAKER CR AT FLORIDA NEW YORK	41-19	74-31	6.234	6.23	1	B2	1.9	2.83	0.737	0.717
A 21	1-47		83892188	WINKLEY CR NR CHARLESTON OHIO	41-12	81-88	6.784	6.92	1	B8	1.6	1.73	1.818	2.119
A 22	1-49		93118188	LITTLE CHIPPENVA CR NR SMITHVILLE OHIO	48-52	91-47	18.498	8.98	1	B8	1.8	1.76	1.798	2.781
A 23	1-58		81415888	TERRY CLOVE KILL NR PEPACTION NEW YORK	42-18	74-58	9.824	9.96	1	B8	1.7	1.88	0.532	0.528
A 24	1-53		93889888	MILL CR NR BERLIN OHIO	48-58	88-58	12.224	12.6	1	B8	1.8	1.73	2.388	1.172
A 25	1-55		93892888	KALE CR NR PRICESTON OHIO	41-88	91-83	14.818	13.4	1	B8	1.8	1.73	2.688	3.818
A 26	1-59		94218588	LITTLE TONAMANDA CR AT LINDEN NEW YORK	42-53	78-18	14.144	14.1	1	B7	1.8	1.59	1.588	1.254
A 27	1-58		81414588	HILL BROOK AT ARENA NEW YORK	42-88	74-39	16.888	16.88	1	B8	1.7	1.68	0.884	0.825
A 28	2-1	42.18		ARS WACO(RIESEL) TEX WATERSHED W-10	31-38	96-38	.828	8.828	2	A6	3.1	3.81	0.821	0.812
A 29	2-2	26.37		ARS COSHOCTON OHIO WATERSHED NO 109	48-33	84-88	.828	8.828	2	B15	2	1.79	0.814	0.812
A 30	2-3	42.8		ARS WACO(RIESEL) TEX WATERSHED W-8	31-38	96-38	.842	8.842	2	A6	3.1	3.81	0.838	0.828
A 31	2-4	26.39		ARS COSHOCTON OHIO WATERSHED NO 103	48-33	84-88	.874	8.874	2	B15	2	1.79	0.832	0.831
A 32	2-5	26.28		ARS COSHOCTON OHIO WATERSHED NO 177	48-22	81-48	.876	8.876	2	B15	2	1.79	0.827	0.827
A 33	2-6	26.31		ARS COSHOCTON OHIO WATERSHED NO 18	48-24	81-48	.122	8.122	2	B15	2	1.79	0.841	0.858
A 34	2-7	42.7		ARS WACO TEX (RIESEL) WATERSHED W-2	31-38	96-38	.138	8.138	2	A6	3.1	3.81	0.859	0.854
A 35	2-8	42.6		ARS WACO TEX (RIESEL) WATERSHED W-1	31-38	96-38	.178	8.178	2	A6	3.1	3.81	0.888	0.185
A 36	2-9		83274188	BLAKE RUN NR REILY OHIO	39-28	84-46	.188	8.188	2	C4	1.8	1.97	0.858	0.897
A 37	2-10	26.36		ARS COSHOCTON OHIO WATERSHED NO 186	48-22	81-47	.383	8.383	2	B15	2	1.79	0.851	0.840
A 38	2-11	26.32		ARS COSHOCTON OHIO WATERSHED NO 8	48-24	81-48	.348	8.348	2	B15	2	1.79	0.861	0.824
A 39	2-14	42.2		ARS WACO TEXAS WATERSHED C	31-38	96-38	.878	8.878	2	A6	3.1	3.81	0.821	0.277
A 40	2-15		83188188	8ELL CR AT MCCONNELLSVILLE OHIO	39-39	81-88	.888	8.888	2	B15	2	1.84	0.133	0.133
A 41	2-16	26.33		ARS COSHOCTON OHIO WATERSHED NO 92	48-24	81-48	.928	8.928	2	B15	2	1.79	0.142	0.846
A 42	2-28	42.2		ARS WACO TEXAS WATERSHED D	31-38	96-38	1.118	1.118	2	A6	3.1	3.81	0.818	0.488
A 43	2-31	26.34		ARS COSHOCTON OHIO WATERSHED NO 94	48-23	81-48	1.588	1.58	2	B15	2	1.79	0.258	0.267
A 44	2-32		83118888	JEFFERSON CR NR JEWETT OHIO	48-24	88-27	1.888	1.88	2	B15	2	1.78	0.377	0.325
A 45	2-33		83148888	OTTER FORK NR CENTERTOWN OHIO	48-19	82-43	2.888	1.98	2	C4	1.8	1.82	0.588	0.888
A 46	2-36		83188888	BARNES RUN NR SUMMITFIELD OHIO	39-47	81-81	2.214	2.26	2	B15	2	1.83	0.391	0.389
A 47	2-37		83238188	88R LITTLE SALT CR AT JACKSON OHIO	39-81	82-39	2.488	2.58	2	B15	2	1.84	0.784	0.683
A 48	2-38	26.35		ARS COSHOCTON OHIO WATERSHED NO 88	48-23	81-49	2.878	2.878	2	B15	2	1.79	0.488	0.413



Table 1. Continued.

14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30
SEC #	USU L	USU NO	USU STR	L/RYS	PHY ZN	PR	Q10	USU Q10	SEVR	ON	SEVR	Q10	Q10	Q10	Q10	Q10/A
A 1	6.86	6.43	7.46	0.0	128	0.808	0.872	0.821	0.887	0.804	0.812	0.869	0.887	0.862	0.893	1.2782
A 2	6.86	6.67	6.85	0.0	128	0.878	0.836	0.867	0.818	0.877	0.866	0.814	0.824	0.886	0.841	1.2518
A 3	7.37	1.02	6.67	12.1	156	120	0.440	0.688	0.232	0.995	1.081	0.238	0.731	0.699	1.062	2.2676
A 4	6.86	6.81	13.83	0.0	142	0.408	0.493	0.138	0.536	0.636	0.683	0.352	0.333	0.566	0.828	1.8687
A 5	7.64	1.08	4.82	0.0	268	130	0.888	0.713	0.203	0.941	1.108	0.282	0.824	0.854	0.951	1.7131
A 6	7.64	5.88	3.89	0.0	374	128	1.060	1.158	0.428	1.063	1.010	0.585	1.180	1.395	1.688	0.6137
A 7	6.36	4.38	2.12	-2	563	120	0.800	0.888	0.625	0.895	0.908	0.681	0.836	0.868	1.355	0.3918
A 8	6.86	4.48	2.56	0.0	674	120	0.808	0.878	0.358	0.875	1.038	0.375	1.038	1.058	2.375	0.3231
A 9	5.86	4.78	2.44	1.1	731	124	0.818	0.718	0.598	0.878	0.872	0.388	0.858	1.588	2.188	0.2553
A 10	5.35	6.88	2.18	0.0	1281	120	1.788	1.388	0.688	1.788	0.848	0.628	1.828	1.748	2.848	0.1887
A 11	5.88	5.18	2.92	-1	693	10	1.888	1.388	0.928	1.888	1.788	0.888	1.888	1.448	1.828	0.1278
A 12	6.85	9.58	4.11	0	844	120	5.888	0.488	0.988	0.428	0.988	2.488	0.938	7.888	12.888	17.888
A 13	5.41	1.48	2.55	0.0	108	80	0.118	0.888	0.037	0.288	0.888	0.834	0.893	0.894	0.898	0.2078
A 14	5.41	1.58	4.38	-4	874	80	0.142	0.888	0.688	0.288	0.388	0.878	0.242	0.288	0.337	0.4158
A 15	5.38	1.98	2.68	20.0	161	80	0.178	0.188	0.884	0.238	0.388	0.688	0.155	0.198	0.228	0.1685
A 16	5.37	8.88	4.86	1.3	1288	80	0.888	0.488	0.235	0.377	0.885	0.288	0.355	0.888	1.188	0.1887
A 17	5.15	5.88	2.11	-1	449	80	0.838	0.888	0.478	0.888	1.888	0.488	0.728	0.788	0.888	0.1882
A 18	5.43	5.88	3.79	4	891	80	1.388	1.078	0.748	1.388	1.888	0.788	1.118	1.158	1.488	0.2887
A 19	5.34	6.88	3.37	0.0	685	80	1.888	1.878	0.688	2.848	3.788	0.688	1.778	1.788	2.888	3.888
A 20	6.49	5.88	3.18	5.3	579	80	0.888	0.818	0.388	1.128	1.388	0.378	0.788	0.788	0.978	1.288
A 21	5.45	5.88	2.59	3	1511	80	0.928	0.688	0.378	0.755	0.682	0.418	0.985	0.688	0.758	0.8877
A 22	5.35	6.88	3.69	0.1	1176	80	1.188	1.388	0.888	1.485	1.588	0.888	1.958	1.888	2.378	0.1477
A 23	5.78	5.88	2.28	-2	426	80	1.588	1.618	0.725	3.188	3.695	0.788	1.788	2.688	3.538	0.1782
A 24	5.49	6.88	3.83	5	1528	80	1.878	1.788	1.148	1.888	2.128	1.888	1.888	1.728	1.825	0.1473
A 25	5.48	2.88	4.85	7	1784	80	2.888	2.428	1.288	3.278	3.928	2.128	2.138	2.488	3.788	4.888
A 26	5.88	9.78	3.87	1.3	1887	124	0.888	1.888	2.818	3.818	3.818	1.888	1.888	2.488	2.888	0.1381
A 27	5.88	18.88	2.37	82	648	80	3.888	3.788	2.888	4.888	5.888	1.748	3.388	3.288	4.148	4.825
A 28	9.88	0.82	2.23	0.0	893	3F	0.187	0.883	0.044	0.185	0.125	0.833	0.872	0.878	0.895	0.114
A 29	5.88	9.28	8.97	0.0	888	8E	0.881	0.888	0.824	0.893	0.118	0.818	0.954	0.875	0.891	1.8831
A 30	9.38	0.24	6.88	0.0	888	3F	0.188	0.134	0.888	0.174	0.288	0.688	0.113	0.111	0.138	0.188
A 31	5.88	9.59	8.25	1.8	188	8E	0.882	0.188	0.833	0.388	0.344	0.833	0.141	0.182	0.211	0.373
A 32	5.88	9.38	12.28	0.0	887	8E	0.888	0.152	0.838	0.348	0.888	0.828	0.118	0.187	0.288	1.888
A 33	5.88	8.85	12.71	8.8	188	8E	0.882	0.182	0.838	0.388	0.387	0.828	0.127	0.185	0.214	1.888
A 34	9.88	0.49	4.23	0.0	114	3F	0.488	0.488	0.187	0.838	0.788	0.131	0.888	0.384	0.537	0.858
A 35	9.88	0.77	4.88	0.8	123	3F	0.888	0.587	0.278	0.788	0.987	0.183	0.815	0.487	0.511	0.888
A 36	6.35	1.88	5.38	-2	111	120	0.188	0.188	0.888	0.828	0.288	0.888	0.178	0.182	0.288	0.8578
A 37	5.88	0.75	18.18	1.5	112	8E	0.885	0.588	0.135	1.888	0.888	0.134	0.467	0.437	0.758	0.888
A 38	5.88	0.47	0.78	0.0	488	8E	0.188	0.888	0.878	0.488	0.955	0.877	0.238	0.218	0.341	0.6782
A 39	9.38	1.58	1.78	0.0	288	3F	0.888	0.684	0.333	0.833	0.788	0.288	0.718	0.758	0.854	1.148
A 40	8.82	1.78	2.82	8.8	124	8E	0.888	0.824	0.348	1.888	1.888	0.388	0.787	0.888	1.288	1.1187
A 41	5.88	0.78	8.38	8.8	188	8E	0.888	0.428	0.182	0.788	1.888	1.188	0.482	0.482	0.648	0.5238
A 42	9.88	2.39	1.82	8.8	398	3F	1.488	1.883	0.548	1.878	1.488	0.448	1.887	1.885	2.815	2.883
A 43	6.88	2.88	18.87	0.0	248	8E	0.888	0.888	0.388	1.888	1.388	0.383	0.843	0.874	1.272	1.888
A 44	6.88	2.38	3.81	9	318	8E	0.888	0.888	0.188	0.987	0.418	0.118	0.888	0.187	0.338	0.418
A 45	5.78	3.58	2.45	1.7	881	120	0.388	0.388	0.883	0.448	0.513	0.138	0.338	0.348	0.348	0.888
A 46	5.78	3.88	5.15	0.0	238	8E	1.488	2.888	0.888	3.188	3.788	0.888	1.837	1.888	2.888	0.888
A 47	6.12	3.78	8.11	8.8	683	8E	0.888	0.888	0.728	1.888	1.888	0.788	1.888	1.888	1.888	0.418
A 48	5.88	3.38	18.12	8.8	338	8E	0.888	1.888	0.888	1.388	1.788	0.544	1.488	1.888	1.718	0.4488

Table 1. Continued.

REQ #	POTTER #	ARG #	USGS #	STATION NAME	LAT	LONG	UBU AREA	PT AREA	PTR ZN	PR AREA	PTR P68	UBU P68	PTR T	USU T
A 49	2-28		83231888	F PAINT CR NR SEDALIA OHIO	38-42	83-29	2,445	8,71	2	C4	1.8	1.98	1,738	8,711
A 50	2-30		83241888	SHANNEZ CR AT ZENIA OHIO	38-40	83-53	2,884	2,77	2	C4	1.8	1.81	8,575	8,822
A 51	2-31		84189188	TIDERISHI CR NR JENEA OHIO	48-55	83-42	2,89	2,89	2	C4	1.8	1.80	1,498	1,531
A 52	2-32		84190888	ROLLER CREEK AT OHIO CITY OHIO	48-48	84-39	3,288	3,16	2	C4	1.8	1.88	1,258	1,891
A 53	2-33		83138888	TOUSY RUN AT ZANESFIELD OHIO	48-45	82-34	3,482	3,31	2	C4	2.0	1.79	8,968	8,978
A 54	2-34		83263788	BRIDGE CR NR GREENVILLE OHIO	48-03	84-38	3,881	3,51	2	C4	1.8	1.92	1,120	1,155
A 55	2-35		83189888	LISON CR AT LISBON OHIO	48-49	86-41	3,862	3,89	2	C4	1.8	1.73	8,698	8,675
A 56	2-36		83230888	MONTY CR AT CIRCLEVILLE OHIO	38-35	82-53	3,832	4,82	2	C4	1.8	1.80	8,785	8,848
A 57	2-37		83868888	MAD RIVER AT ZANESFIELD OHIO	48-22	83-41	4,878	4,18	2	C4	1.8	1.87	8,577	8,639
A 58	2-48	28-38	83234188	ARG CONHOCTON OHIO WATERSHED NO 97	48-22	81-58	4,588	4,588	2	C4	1.8	1.79	8,882	8,712
A 59	2-43		83234188	INDIAN CR AT MARIETTA OHIO	38-14	82-59	6,144	6,22	2	C4	1.8	1.93	8,048	8,748
A 60	2-45		83235888	SALT CR AT FAYLTON OHIO	38-38	82-47	7,388	6,78	2	C4	1.8	1.87	1,328	1,389
A 61	2-48		83147888	TIMBER RUN NR ZANESVILLE OHIO	38-07	82-88	6,784	6,79	2	C4	1.8	1.82	8,027	8,853
A 62	2-48		83288888	SCIOA BIG RUN AT BRIGGSDALE OHIO	38-56	83-87	7,848	7,84	2	C4	1.8	1.67	8,998	1,338
A 63	2-51		83110788	CONNOTON CR AT JEWETT OHIO	48-22	88-88	9,132	9,82	2	C4	1.7	1.78	8,862	1,858
A 64	2-57		81525888	MUNCY CR NR BOWESTOWN PENNA	41-23	78-28	15,232	15,2	2	C4	1.7	1.88	8,984	8,933
A 65	2-59		87183888	COUNCIL CR NR STILLWATER OKLA	38-08	86-54	19,848	19,8	2	C4	2.2	2.78	2,188	2,334
A 66	3-1	13-2	87183888	ARG BLACKBURN VA WATERSHED M-III	38-08	86-52	8,819	8,819	3	C4	2.8	2.17	8,818	8,818
A 67	3-2		87812888	BEHKE BR NR ROLLA MISSOURI	37-55	91-42	8,872	8,872	3	C4	2.3	2.39	8,212	8,248
A 68	3-3		81488888	DILLOON CR NR KING POND PENNA	41-02	75-32	1,538	1,53	3	C4	1.8	1.97	8,219	8,227
A 69	3-4		81588888	SHACKHAM BROOK NR TRUXTON NEW YORK	48-47	78-81	1,888	2,88	3	C4	1.8	1.82	8,178	8,194
A 70	3-6		81808888	SANPIT RUN NR OLDTOWN MARYLAND	38-34	78-35	3,281	3,28	3	C4	1.7	1.99	8,693	8,723
A 71	3-7		86831388	LITTLE BEAVER CR NR ROLLA MISSOURI	37-57	91-49	4,188	4,18	3	C4	2.2	2.39	8,379	8,382
A 72	3-9		81824888	BELL CR AT FRANK HILL STAUNTON VA	38-11	79-88	6,114	6,14	3	C4	1.8	2.15	8,748	1,857
A 73	3-10		81871888	PAXTON CR NR PENBROOK PENNA	48-28	78-58	7,188	7,17	3	C4	1.9	2.83	8,844	8,838
A 74	3-12		81873888	MADADA CR AT MADADA GAP PENNA	48-25	78-42	8,848	9,82	3	C4	1.8	2.84	8,878	1,884
A 75	3-13		86831888	BEAVER CR NR ROLLA MISSOURI	37-51	91-48	8,888	8,88	3	C4	2.2	2.39	8,913	1,885
A 76	3-14		83475888	BEAVER CR NR WALLACE VA	38-41	82-88	8,788	8,77	3	C4	1.8	2.16	1,158	1,147
A 77	3-17		81812888	LITTLE TONOLWAT CR NR HANCOCK MARYLAND	38-43	78-18	18,818	18,8	3	C4	1.8	1.98	8,673	8,617
A 78	3-18		83387588	SP LITTLE BARREN RIVER AT EDMONTON KY	38-56	85-34	11,712	11,6	3	C4	2.0	2.12	2,158	2,383
A 79	3-19		83393888	MF BEARGRASS CR AT LOUISVILLE KY	38-14	85-38	12,888	11,8	3	C4	1.8	2.83	1,718	2,398
A 80	3-20		83392888	MF BEARGRASS CR AT LOUISVILLE KY	38-18	85-39	11,888	12,8	3	C4	1.8	2.83	2,258	2,288
A 81	3-21		87815888	BOURBEUSE RIVER NR ST JAMES MO	38-88	91-48	13,882	13,8	3	C4	2.3	2.59	1,888	1,145
A 82	3-22		81888888	NORTH RIVER NR STOREVILLE VA	38-23	79-17	11,872	15,88	3	C4	1.8	2.13	8,788	8,778
A 83	4-2		81387588	WALNUT BR NR FLEMINGTON NEW JERSEY	41-32	74-53	1,434	1,43	4	C4	2.0	2.11	8,172	8,288
A 84	4-3		81846588	LITTLE FALLS BRANCH NR BETHESDA MO	38-58	77-88	2,884	2,82	4	C4	2.4	2.84	8,288	8,383
A 85	4-6		82888888	DIAL CR NR BAHAMA NORTH CAROLINA	38-13	78-81	3,814	3,14	4	C4	2.2	2.81	8,888	8,888
A 86	4-6		81878888	SABIN RUN AT LIBERTY GROVE MARYLAND	38-48	78-88	3,388	3,48	4	C4	2.2	2.27	8,538	8,522
A 87	4-7		81478888	SHELLPOT CR AT WILMINGTON DEL	38-47	75-32	4,774	4,77	4	C4	2.2	2.85	8,687	8,756
A 88	4-8		83445888	SP HILLS RIVER AT THE PINK BEDS N C	38-22	82-48	8,394	8,39	4	C4	2.1	2.33	8,534	8,438
A 89	4-9		83442888	CRAB CR NR PENROSE NORTH CAROLINA	38-18	82-34	8,878	8,88	4	C4	2.1	2.34	8,585	8,287
A 90	4-10		81868888	PINEY RUN NR SYRESVILLE MARYLAND	38-28	77-81	7,288	7,38	4	C4	2.2	2.89	1,158	1,227
A 91	4-11		83813588	NOLANO CR NR BYSON CITY NORTH CAROLINA	38-31	83-24	8,832	8,83	4	C4	2.2	2.31	8,438	8,418
A 92	4-12		82888888	EF DEEP RIVER NR HIGH POINT N C	38-84	78-58	9,488	9,48	4	C4	2.2	2.38	1,318	1,418
A 93	4-13		83448888	BOYLSTON CR NR MORRISDESH N C	38-21	82-38	8,472	8,47	4	C4	2.1	2.33	2,858	1,888
A 94	4-15		81478888	CHRISTINA RIVER AT COCHS BRIDGE DEL	38-41	78-47	13,188	13,1	4	C4	2.3	2.88	2,498	2,373
A 95	4-18		83118588	FORBURN CR NR YADKINVILLE NORTH CAROLINA	38-11	88-38	13,888	13,8	4	C4	2.2	2.88	1,888	2,888
A 96	4-17		81884888	ACCOITING CR NR ANNANDALE VIRGINIA	38-52	77-16	18,184	18,1	4	C4	2.3	2.88	2,388	1,888

Table 1. Continued.

SEC	#	U0U	P10	U0U	L	U0U	DD	U0U	6TR	L/R	PHY	ZN	PTR	010	USU	010	53VR	OM	56Y	025	56Y	005	05.33	010	010	L00	025	056	010/A
A	49	6.88	2.68	1.39	0.0	.788	120	0.538	0.205	0.337	0.355	0.338	0.230	0.420	0.410	0.560	0.090	0.1718											
A	50	6.88	2.68	1.39	0.0	.788	120	0.538	0.205	0.337	0.355	0.338	0.230	0.420	0.410	0.560	0.090	0.1718											
A	51	5.85	4.38	2.39	.82	.684	120	0.638	0.640	0.900	1.070	0.900	0.210	1.000	1.000	1.450	1.020	0.3712											
A	52	5.94	3.58	2.48	.3	.663	120	0.528	0.335	0.248	0.418	0.458	0.225	0.360	0.330	0.548	0.604	0.1094											
A	53	5.61	4.58	2.76	.5	.585	120	0.648	0.600	0.488	1.138	1.318	0.450	0.825	1.000	5.150	7.560	0.2350											
A	54	6.66	3.56	.76	.84	.761	120	0.928	0.745	0.538	0.688	0.652	0.475	0.660	0.670	0.775	0.625	0.12135											
A	55	5.49	5.48	4.34	.4	.516	9E	1.148	1.348	0.970	1.798	2.118	0.458	1.026	0.988	1.475	1.950	0.3507											
A	56	5.97	5.78	4.19	.1	.587	9E	1.258	1.333	0.970	1.958	0.900	0.650	1.090	2.008	1.675	0.5218												
A	57	5.61	4.78	5.88	.1	.488	120	1.388	1.360	0.648	1.388	1.958	0.500	1.050	1.100	1.377	1.658	0.2245											
A	58	5.06	5.08	1.18	0.0	.562	120	1.108	1.588	0.788	3.598	3.278	0.631	2.317	2.078	2.955	3.553	0.4891											
A	59	6.12	5.48	4.23	.2	.518	120	1.388	1.458	1.188	13.888	26.588	1.288	4.248	4.100	6.138	7.518	0.6081											
A	60	5.91	7.58	3.88	.82	.718	120	2.188	2.625	1.188	3.558	4.278	2.988	2.550	1.388	2.988	5.328	0.3348											
A	61	5.76	5.68	3.36	.2	.515	9E	1.900	1.755	0.990	2.258	2.575	0.500	1.010	1.558	2.270	2.778	0.2373											
A	62	5.91	6.58	2.82	.2	.549	120	2.368	2.688	1.888	4.388	4.100	1.288	2.580	2.488	2.970	3.888	0.3267											
A	63	6.64	5.38	4.24	.5	.648	9E	1.428	0.972	0.538	1.258	1.488	0.555	0.990	0.988	1.155	1.385	0.1082											
A	64	5.98	18.38	2.23	.3	.711	9C	3.988	5.218	1.788	0.388	10.488	1.088	4.488	4.388	8.788	8.458	0.2889											
A	65	8.78	9.08	3.49	.81	1.974	15F	5.688	11.388	1.588	19.088	26.588	2.458	12.550	10.088	20.758	27.758	0.6174											
A	66	6.88	8.24	13.13	0.0	.991	3A	8.014	0.828	0.884	0.841	0.881	0.883	0.927	0.825	8.038	8.647	1.3988											
A	67	7.57	2.21	2.89	.3	.971	11A	0.635	1.198	0.378	1.688	2.848	0.388	0.948	0.948	1.298	1.588	1.4286											
A	68	6.26	2.58	1.28	1.0	.118	8C	0.378	0.348	0.177	0.435	0.485	0.152	0.302	0.378	0.564	0.784	0.2582											
A	69	5.16	2.68	2.81	.83	.384	8C	0.488	0.265	0.218	0.315	0.348	0.185	0.285	0.285	0.382	0.328	0.1435											
A	70	6.32	5.58	4.22	.63	.473	8C	0.718	0.688	0.388	1.888	1.288	0.287	0.588	0.588	0.797	0.888	0.1783											
A	71	7.57	3.58	3.59	.8	.322	11A	3.688	0.688	1.458	1.388	26.288	1.388	4.728	4.488	7.358	9.388	1.1512											
A	72	6.62	6.18	3.38	.2	.287	6A	1.158	1.858	0.855	1.458	1.918	0.188	1.182	1.388	1.207	1.388	0.1934											
A	73	6.46	5.58	1.98	0.0	.376	8C	2.088	1.828	1.458	2.158	2.358	1.425	1.758	1.828	2.825	2.235	0.2441											
A	74	6.48	5.78	1.83	0.0	.584	7C	0.584	0.708	0.358	0.858	0.958	0.258	0.358	0.358	2.358	3.188	0.2731											
A	75	7.57	6.08	3.44	.1	.592	11A	3.988	3.288	1.988	3.688	4.488	1.913	3.588	4.888	4.965	5.525	0.4353											
A	76	6.85	7.88	4.92	.82	.598	6A	0.588	4.888	2.888	0.858	7.788	2.288	1.938	1.428	4.428	5.458	0.4618											
A	77	6.34	6.88	4.24	.84	.346	6B	1.728	1.488	0.888	1.788	1.988	0.588	1.828	1.388	1.458	1.885	0.1288											
A	78	6.73	9.58	3.43	.82	1.158	11A	2.658	2.658	1.558	2.358	2.588	1.548	1.928	1.958	2.188	2.488	0.1638											
A	79	6.45	9.08	2.71	.1	1.144	11B	1.858	1.748	1.188	2.888	2.318	1.258	3.458	3.288	4.888	6.488	0.2832											
A	80	6.45	9.08	2.87	.1	1.096	11B	1.588	2.018	0.938	3.588	3.288	1.288	3.928	3.288	4.218	5.488	0.2697											
A	81	7.57	6.78	4.28	.6	.472	11A	8.788	7.188	4.388	11.288	13.788	3.848	6.728	0.888	8.188	9.488	0.4938											
A	82	6.16	9.58	1.21	0.0	.897	5A	2.588	2.388	0.988	4.288	3.958	0.758	1.725	1.988	7.888	10.558	0.1558											
A	83	6.63	2.78	1.89	0.0	.337	7C	0.498	0.448	0.235	0.858	0.958	0.258	0.358	0.478	0.618	0.3278												
A	84	6.38	2.48	1.69	0.0	.288	4A	1.188	1.988	1.188	2.558	2.648	1.148	2.128	2.128	2.688	3.128	0.8978											
A	85	7.97	5.98	2.88	0.0	.375	4A	0.748	1.958	0.488	1.518	1.878	0.378	1.588	1.188	5.888	9.488	0.3981											
A	86	7.28	3.88	3.87	.1	.418	4A	1.248	1.488	0.678	1.958	2.888	0.878	2.828	2.588	3.418	4.888	0.7182											
A	87	7.15	6.68	2.73	.1	.498	4A	2.988	3.998	1.488	4.588	5.498	1.535	2.778	3.158	3.625	4.858	0.5813											
A	88	7.48	4.88	4.56	0.0	.733	5B	1.888	1.328	0.818	2.888	2.558	0.828	1.955	1.538	2.285	5.795	3.2495											
A	89	7.43	4.88	2.91	.1	1.255	5B	1.188	1.285	0.988	1.958	2.688	0.475	1.375	1.388	1.875	2.878	0.1971											
A	90	8.84	8.08	3.59	0.0	1.089	4A	2.288	2.548	1.938	3.988	5.088	0.688	1.928	2.858	3.948	5.448	0.2632											
A	91	7.32	6.38	3.98	0.0	.374	9B	1.688	1.388	0.888	1.618	1.788	1.018	1.585	1.688	1.988	2.378	0.1795											
A	92	7.49	6.48	3.75	.1	1.322	4A	2.988	3.525	1.888	4.258	5.158	1.888	3.458	3.888	5.388	8.688	0.3687											
A	93	7.48	11.08	24.96	.3	1.188	5B	0.758	0.788	0.588	0.835	0.848	0.578	0.788	0.788	0.885	0.848	0.8882											
A	94	7.18	12.58	1.82	.1	1.698	4B	2.288	2.988	1.458	2.788	3.888	1.428	2.888	2.888	2.528	2.888	0.1585											
A	95	7.17	11.88	3.74	.2	1.782	4A	2.138	2.588	1.818	1.783	4.888	1.928	2.888	2.138	2.728	3.888	0.1584											
A	96	6.58	8.68	3.24	.1	2.282	4B	3.788	3.888	1.418	5.788	6.888	1.928	4.888	4.888	6.788	8.858	0.3589											

Table 1. Continued. Explanation of numbered column headings.

<sup>1</sup>Potter's zone and watershed number.

<sup>2</sup>ARS#. These are the numbers the Agricultural Research Service has given to their experimental watersheds.

<sup>3</sup>USGS#. These are the standard gaging station numbers used in the U.S. Geological Survey water supply papers.

<sup>4</sup>Lat. This column is the latitude of the watershed center. Degrees and minutes.

<sup>5</sup>Long. This column is the longitude of the watershed center. Degrees and minutes.

<sup>6</sup>USU Area. The watershed area as measured by Utah State University personnel. Thousands of acres.

<sup>7</sup>Pt Area. The watershed area as reported by Potter. Thousands of acres.

<sup>8</sup>PTR ZN. Potter zone.

<sup>9</sup>PR Area. Soil Conservation Service problem area.

<sup>10</sup>PTP P60. The 10-year 1-hour precipitation as reported by Potter. Inches.

<sup>11</sup>USU P60. The 10-year 1-hour precipitation as determined by the authors. Inches.

<sup>12</sup>PTR T. The T value as reported by Potter.

$$T = 0.7L/\sqrt{S_1} + 0.3L/\sqrt{S_2}$$

<sup>13</sup>USU T. The T value as determined by the authors.

<sup>14</sup>USU P10. The 10-year 10-minute precipitation intensity in inches per hour.

<sup>15</sup>USU L'. The length in miles up the principal drainage channel from the culvert site to end of the channel delineated as either a solid or dashed blue line on the 7½ minute topographic map.

<sup>16</sup>USU DD. The drainage density as determined by the authors. Miles per 1000 acres.

<sup>17</sup>USU STR. Percent storage. The percentage of the surface area of the watershed occupied by lakes, swamps, playas, etc.

<sup>18</sup>L'/RTS. The length of the principal drainage divided by its mean slope.

<sup>19</sup>PHY ZN. The physiographic section of the watershed center from the map of Fenneman and Johnson.

<sup>20</sup>PTR Q10. The 10-year instantaneous peak runoff as reported by Potter. Thousands of cfs.

<sup>21</sup>USU Q10. The 10-year instantaneous peak runoff as determined by the authors with runoff data prior to 1958. Thousands of cfs.

<sup>22</sup>584 QM. The 2.33-year instantaneous peak runoff from data prior to 1958. Thousands of cfs.

<sup>23</sup>584 Q25. The 25-year instantaneous peak runoff from peaks prior to 1958. Thousands of cfs.

<sup>24</sup>584 Q50. The 50-year instantaneous peak runoff from peaks prior to 1958.

<sup>25</sup>Q2.33. The 2.33 year instantaneous peak runoff from peaks for the entire period of record.

<sup>26</sup>Q10. The 10-year instantaneous peak runoff from peaks for the entire period of record.

<sup>27</sup>Q10 L06. The 10-year instantaneous peak runoff from peaks for the entire period of record and using log Gumbel paper with a graphical method.

<sup>28</sup>Q25. The 25-year instantaneous peak runoff from peaks for the entire period of record.

<sup>29</sup>Q50. The 50-year instantaneous peak runoff from peaks for the entire period of record.

<sup>30</sup>Q10/A. The 10-year runoff peak per unit of area for the entire period of record.

manner and with  $\log q_{10}$  minimized. These curves for Potter group 1 watersheds are shown complete with Potter's data points and USU measured data points in Figures 3 through 6 for Zones I through IV, respectively (p. 13-16). Potter's curve through the mean is also shown for reference. The reader is invited to draw his own conclusions regarding goodness of fit. In an effort to determine exactly how Potter obtained his line USU also minimized the  $\log x$  deviations squared for the same data points. This line was nearer but still did not agree with Potter's line. The  $q_{10}$  vs. area relationships as represented by Potter's curves would tend to overemphasize the parameters other than area below the mean and reverse this affect above the mean. The most plausible explanation for the seemingly poor fit of Potter's line to the data is that Potter used graphical correlation to fit the data points and must have plotted the data to different scales than those used in Figures 3 through 6 (p. 13-16). The fit obtained by graphical correlation is scale dependent and can produce results similar to those in the referenced figures. Figure 7 (p. 17) shows the regression line relating ten year peak flow,  $q_{10}$ , to the watershed area, A, for all Potter watersheds with no zoning.

The watershed parameters and other appropriate data as determined by both Potter and USU are tabulated in Table 1 (p. 5-9). These basic data were either determined from USGS maps or from the publications of the USGS, the Agricultural Research Service (ARS), the Soil Conservation Service (SCS), and various State agencies concerned with water data.

Statistical t tests (Steel and Torrie, ref. 16) show no significant differences between the values of any of the basic parameters such as A, P factor, T factor or  $q_{10}$  determined by Potter or USU. The individual differences on watersheds appear to be higher or lower on an essentially random basis since there were no significant group differences. In the instance of drainage density, Potter's values were not published so could not be compared. However, the high correlation mentioned by Potter between DD and T factors yielded an  $r^2$  value of 0.1602 with a log-log transform on 96 watersheds. This value is significant at the 1 percent level but obviously does not account for a major portion of the variability by itself.

The correlation between DD and area is similar to that between DD and T having an  $r^2$  value of 0.1647 with the same 96 watersheds. Both of the DD relationships are much less correlated with area than the T factor where the  $r^2$  value is 0.5479 using the same watersheds and 0.2405 with 545 watersheds (watersheds from all of the United States).

The relationship between the T factor and  $L/\sqrt{S}$  has an  $r^2$  of 0.9216 using 96 watersheds, and the t value between them is not significant, thus, there appears to be little choice between these parameters. This relationship is similar to the precision obtained for the two sets of measurements.  $L/\sqrt{S}$  is a simpler parameter to visualize so should be a preferred parameter if all else were equal. It appears that the value of T published by Potter for the Zone II Watershed 29 is in error, possibly in printing, since the (1) in front of the decimal should be deleted to agree with the measured value.

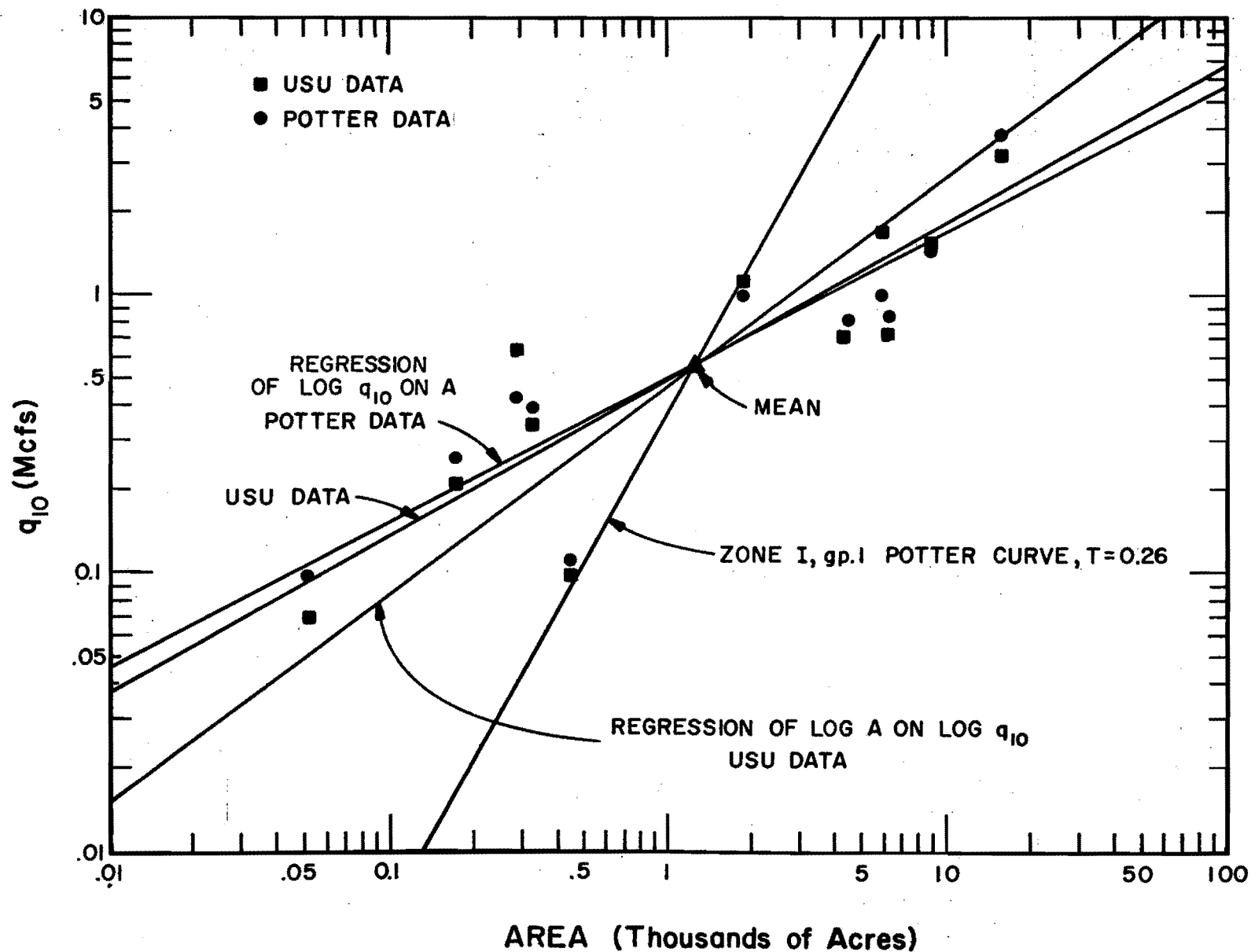


Figure 3. The relationship between 10-year peak flow and area for Potter Zone I, Group 1, watersheds obtained by regression analysis from Potter data and USU data and the graphical correlation curve for  $Q_{10(AT)}$  reported by Potter that goes through the mean of the data.

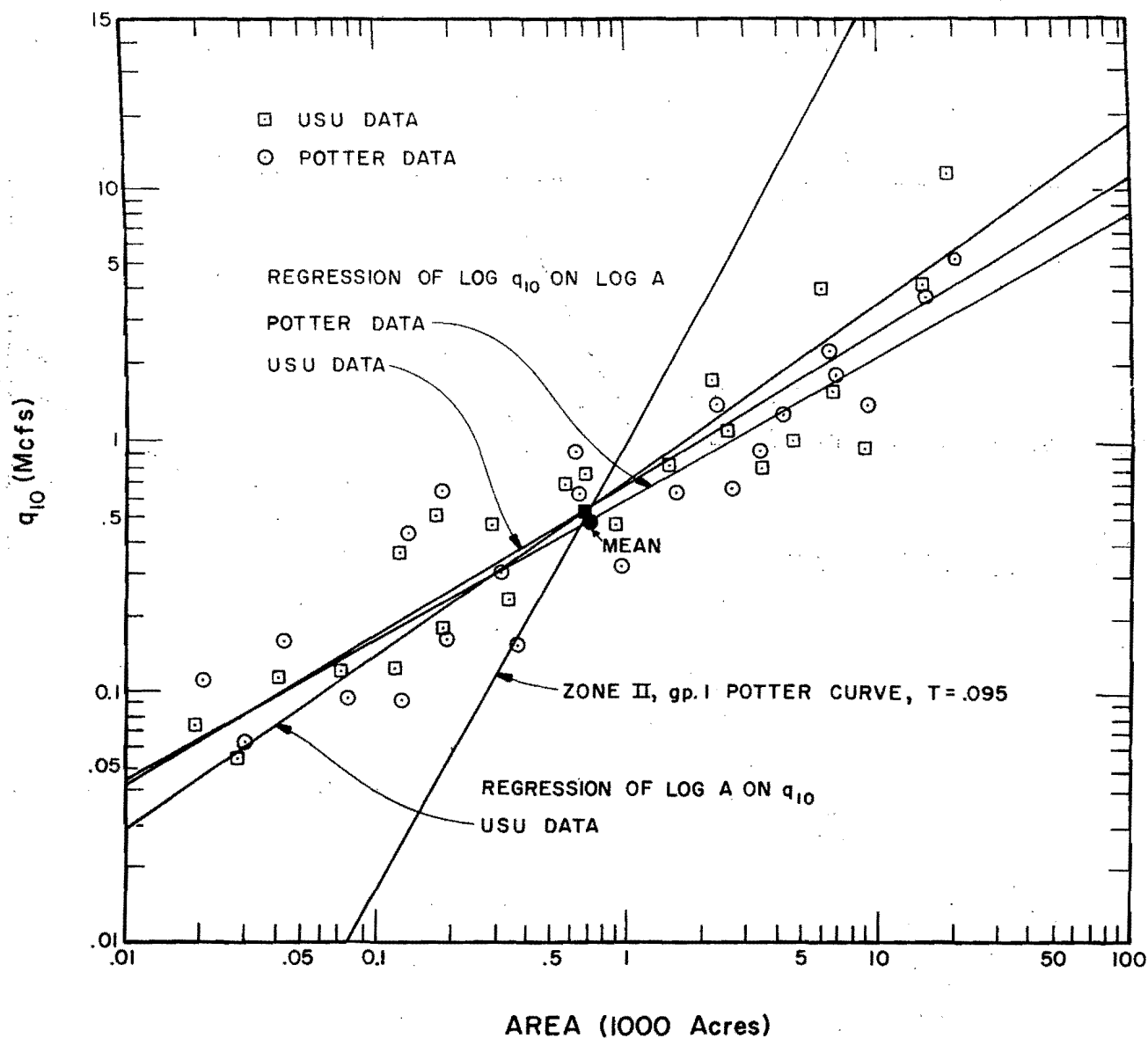


Figure 4. The relationship between 10-year peak flow and area for Potter Zone II, Group 1, watersheds obtained by regression analysis from Potter data, USU data and the graphical correlation between the two data.

Figure

The relationship between 10-year peak flow and area for Potter Zone II, Group 1, watersheds obtained by regression analysis from Potter data, USU data and the graphical correlation curve for  $\hat{q}_{10(AT)}$  reported by Potter that goes through the mean of the data.

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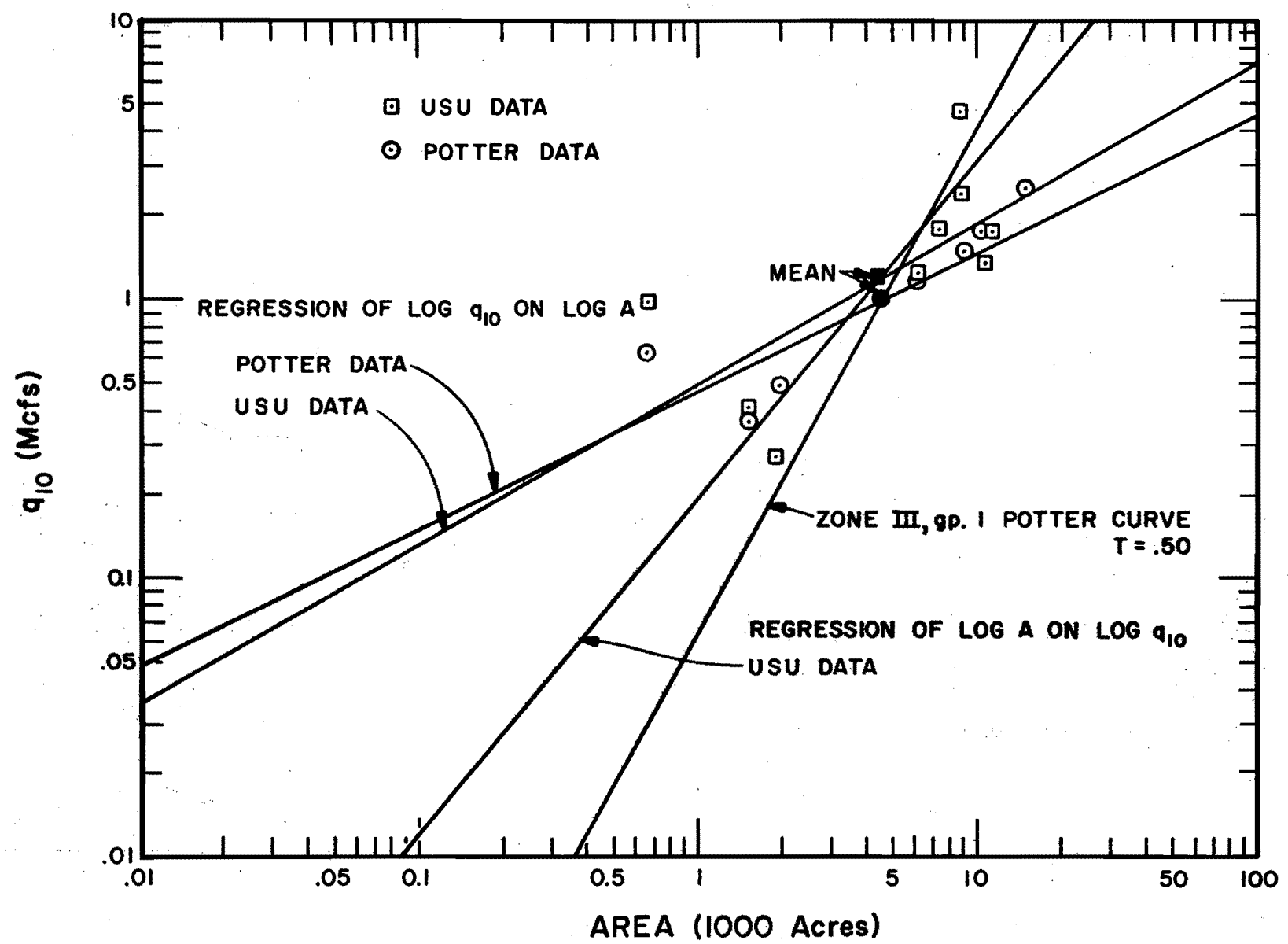


Figure 5. The relationship between 10-year peak flow and area for Potter Zone III, Group 1, watersheds obtained by regression analysis from Potter data, USU data and the graphical correlation curve for  $\hat{q}_{10(AT)}$  reported by Potter that goes through the mean of the data.



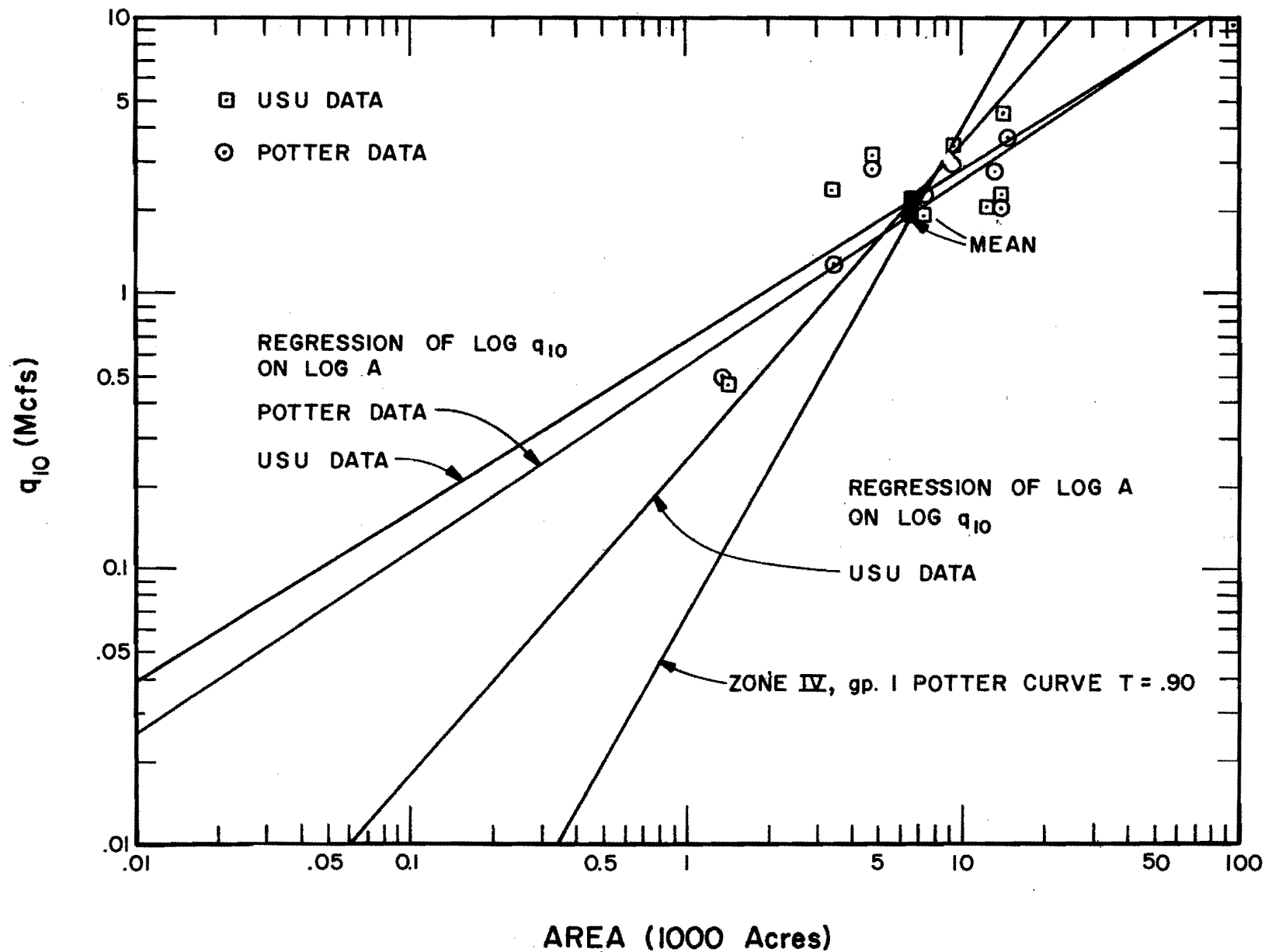


Figure 6. The relationship between 10-year peak flow and area for Potter Zone IV, Group 1, watersheds. The line represents the regression of log  $q_{10}$  on log A for Potter data, USU data and the graphical correlation of log A on log  $q_{10}$  for USU data.

Figure 5. The relationship between 10-year peak flow and area for Potter Zone IV, Group 1, watersheds obtained by regression analysis from Potter data, USU data and the geometric correction curve for  $\hat{q}_{10}(AT)$  reported by Potter that goes through the mean of the data.

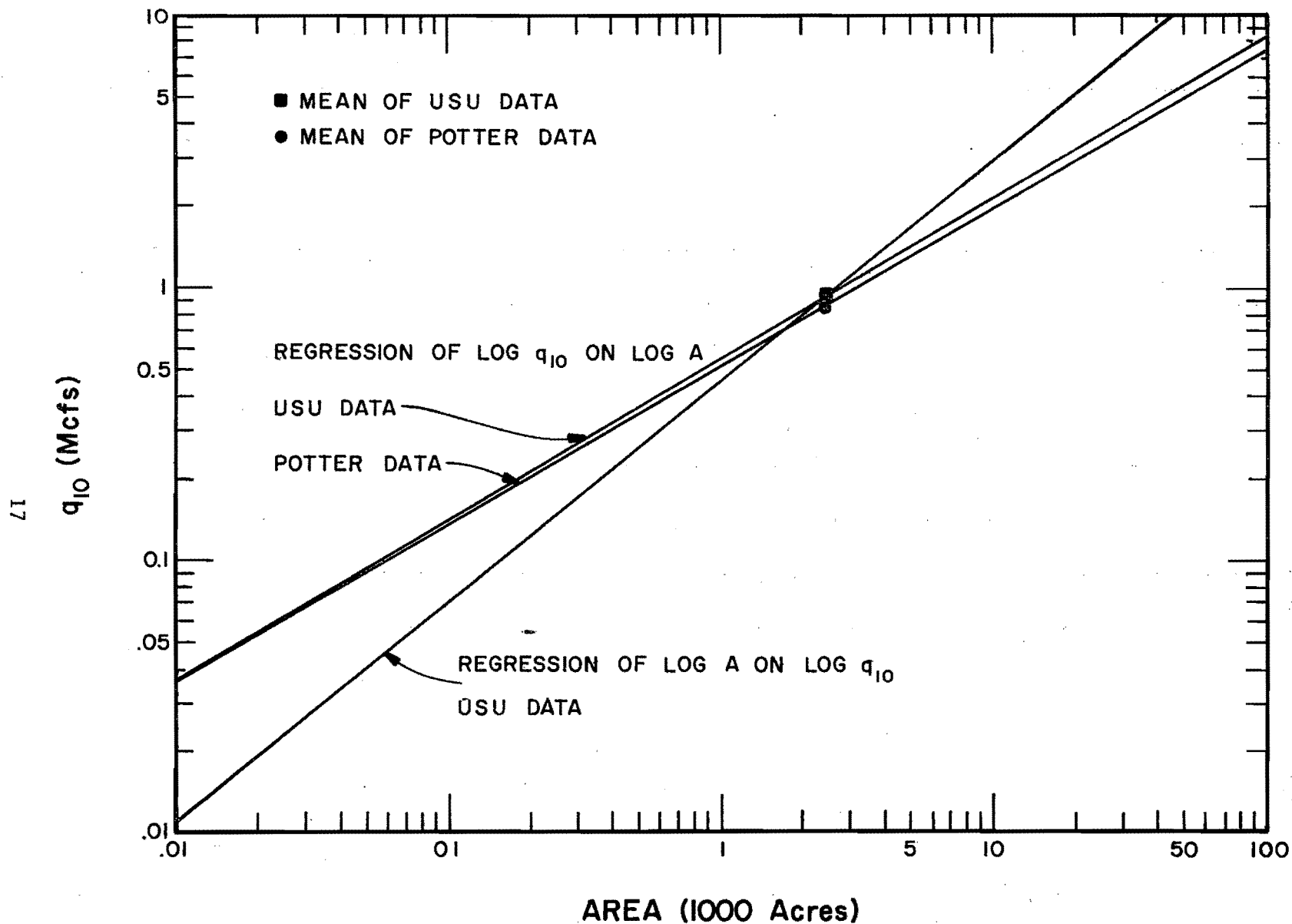


Figure 7. Comparison between the various regression equations relating the 10-year peak flow and area obtained from the USU data and the Potter data for all 96 Potter watersheds.

Potter zones. A study of the performance of Potter's four zones seemed to be in order. The map, "Problem Areas in Soil Conservation," Figure 8 (p. 19), was entered with the centroid location of each of Potter's watersheds to obtain the problem area. The problem areas indicated are:

Zone I	Zone II	Zone III	Zone IV
B-2	A-6	B-8	B-10
B-7	B-5	B-11	B-11
B-8	B-8	B-13	B-12
B-9	B-15	B-14	B-13
B-14	C-4	B-16	
B-15	D-24	B-17	
B-16		B-20	
C-8		B-26	
C-9			
C-10			

Note: B-8 appears in three zones, B-15 appears in two zones, and B-16 appears in two zones.

The 10-year peak flow per 1000 acres ( $q_{10}/A$ ) was chosen as the parameter to represent runoff. The  $q_{10}/A$  values were tabulated, and  $t$  values were computed for each pair of problem areas where sufficient samples existed. The significant  $t$  values are compiled in Table 2 (p. 20).

The  $t$  values suggest the following conclusions:

1. Problem area A-6 is significantly different from all other problem areas where sufficient data exist to make the  $t$  test.
2. Problem area B-20 is significantly different from problem areas B-10, 11, and 16, and no other significant differences exist.

If this information is applied to Potter's zones, it suggests that Zone I is significantly different from the other zones except for the inclusion of problem areas B-8 and B-15. Because B-13, 16, and 20 are all in Zone III, the differences are between Zones III and IV. The  $t$  value for Zone II was significantly different from the other zones except III and in Zone III the value was just below the 5 percent level. The same weak differences are borne out by the similar slopes of the lines for each zone in Figures 3, 4, 5, and 6 (p. 13-16). Furthermore, if  $\log q_{10}/A$  is plotted against  $\log A$ , the range of difference between the extremes is in the order of 14 percent which is certainly within the point spread about the lines.

Graphical correlation. If we adopt the division of watersheds of Potter (ref. 13) into Group 1 and Group 2, follow the graphical correlation technique as outlined by Ezekial and Fox (ref. 6), and construct a set of curves from Group 1 data for the relation between  $\log q_{10}$  and  $\log A$ , the curves of Figures 3 through 6 (p. 13-16) are obtained for each respective zone. All values are those derived by Potter. The residual  $y$  deviations or  $\Delta y_1$  values from the

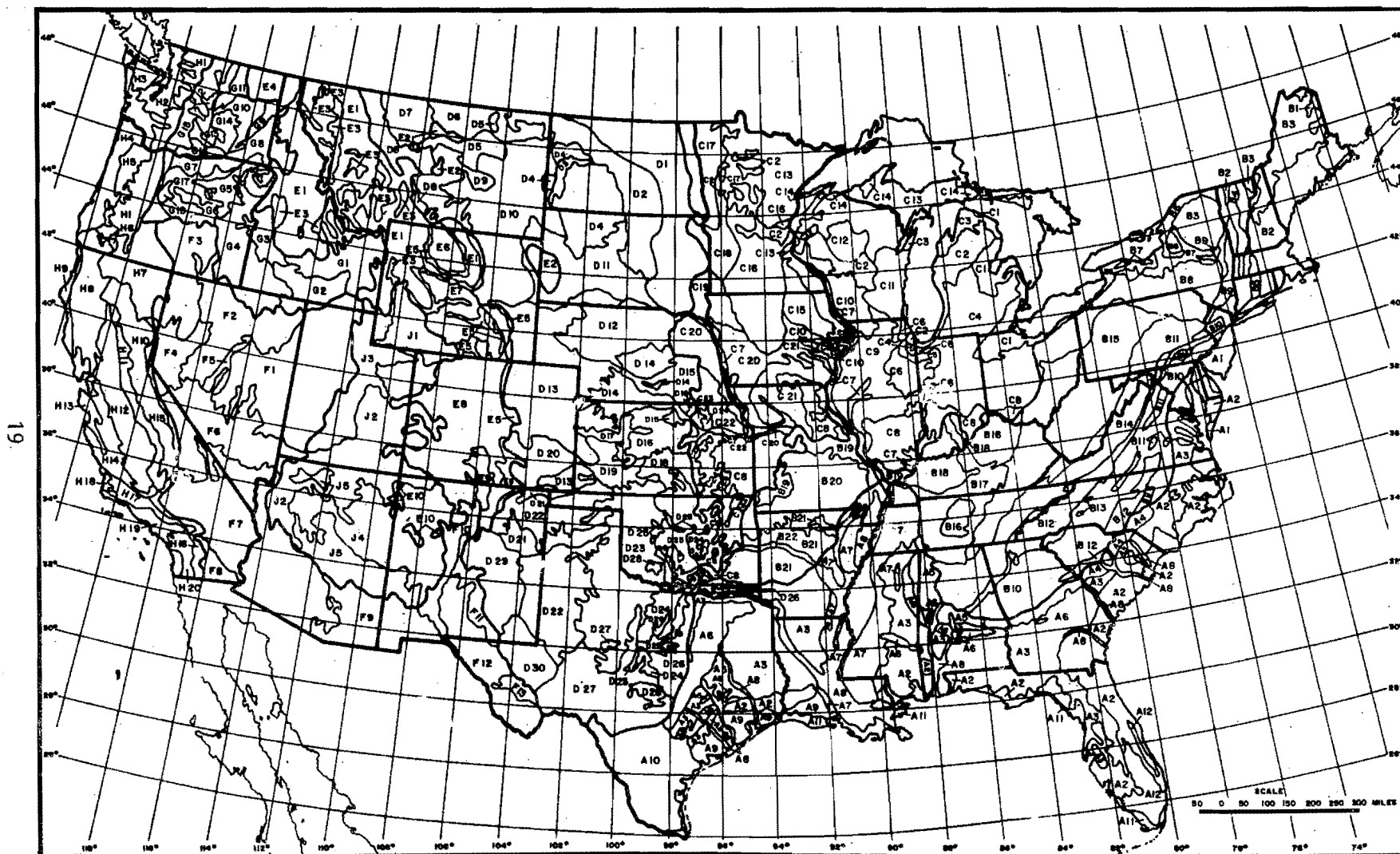


Figure 8. Map of the problem areas in soil conservation. Soil Conservation Service, Washington, D.C.

Table 2. Significance of t values for differences between peak flows per unit area and pairs of problem areas.

Problem Area	B8	B10	B11	B13	B14	B16	B20	C4
A6	**	**	**	**	*	**	*	**
B8								
B10							*	
B11								
B13							**	
B14								
B16							*	

\*t value significant at the 5 percent level.

\*\*t value significant at the 1 percent level.

Blank spaces and missing problem areas indicate non-significant t values or insufficient data to make the t test.

respective curves of Figures 3 through 6 (p. 13-16) are regressed against log T. Table 3 (p. 21) gives the equations derived during this process for each zone along with the correlation coefficient, r, indicating the goodness of fit of each equation. Note that the relative magnitude of the effect of T is small when compared to A as represented by the much smaller exponents and smaller values given by the  $\Delta y_1$  vs. log T relationship.

The residual y deviations from the regression of  $\Delta y_1$  on log T or  $\Delta y_2$  are next regressed against the log of the precipitation factor  $P_{60}$ , to get the relationships also shown in Table 3 (p. 21).

In order to allow some of the beneficial effect of the interaction that may exist among the three independent variables being considered, A, T, and  $P_{60}$ , a correction equation is then obtained by regressing  $q_{10}$  against  $\hat{q}_{10}(ATP)$  as estimated from applying the proper zone equations for each of the 52 Group Potter watersheds. This results in the correction equation also shown in Table 3 (p. 21) and shown graphically as Figure 9 (p. 22).

The results of the above derived equations were then tested against the Group 2 watersheds, data from which were not used in the derivation of the equations and are tabulated in Table 4 (p. 23). The error for the Group 2 data seems to be of the same order of magnitude as that for the Group 1 data, thus indicating the validity of the approach.

When Potter tested his graphically derived equations against the Group 2 data, it became apparent that a correction factor was necessary. His approach was to relate the correction factor, C, to an error in the estimate of the topographic factor,  $\hat{T}(AP)$ , as a function of area and 10-year 60-minute precipitation. He found that the relative errors in the Group 2 data could be

Table 3. Equations relating 10 year peak flow,  $q_{10}$ , to area, A, topographic factor, T, and the 10 year-60 minute precipitation,  $P_{60}$ , derived from Potter Group 1 data.

Zone	Equation	(1) n	(2) r
I	$\hat{q}_{10(A)} = 0.5043 A^{0.5218}$	11	0.893
	$DY_1 = 1.0863 T^{0.0544}$	11	0.147
	$DY_2 = 0.2239 P_{60}^{2.1855}$	11	0.592
II	$\hat{q}_{10(A)} = 0.6187 A^{0.5759}$	24	0.922
	$DY_1 = 1.0790 T^{0.0398}$	24	0.117
	$DY_2 = 0.2643 P_{60}^{1.8489}$	24	0.839
III	$\hat{q}_{10(A)} = 0.4647 A^{0.4933}$	9	0.754
	$DY_1 = 1.0053 T^{0.00805}$	9	0.013
	$DY_2 = 0.2852 P_{60}^{1.9448}$	9	0.323
IV	$\hat{q}_{10(A)} = 0.5450 A^{0.6646}$	8	0.851
	$DY_1 = 1.0018 T^{0.0102}$	8	0.027
	$DY_2 = 0.2515 P_{60}^{1.7523}$	8	0.224
All Zones Correction Equation	$\hat{q}_{10(K)} = 1.02901 \hat{q}_{10(ATP)}^{1.0810}$	52	0.963

Notes explaining the column headings:

- (1) n is the number of watersheds used in the analysis.
- (2) r is the correlation coefficient between two variables x + y. It is calculated by:

$$r = \frac{\sum (x - \bar{x}) (\sum y - \bar{y})}{\sqrt{\sum (x - \bar{x})^2 \sum (y - \bar{y})^2}}$$

where x and y are the independent and dependent variables respectively.

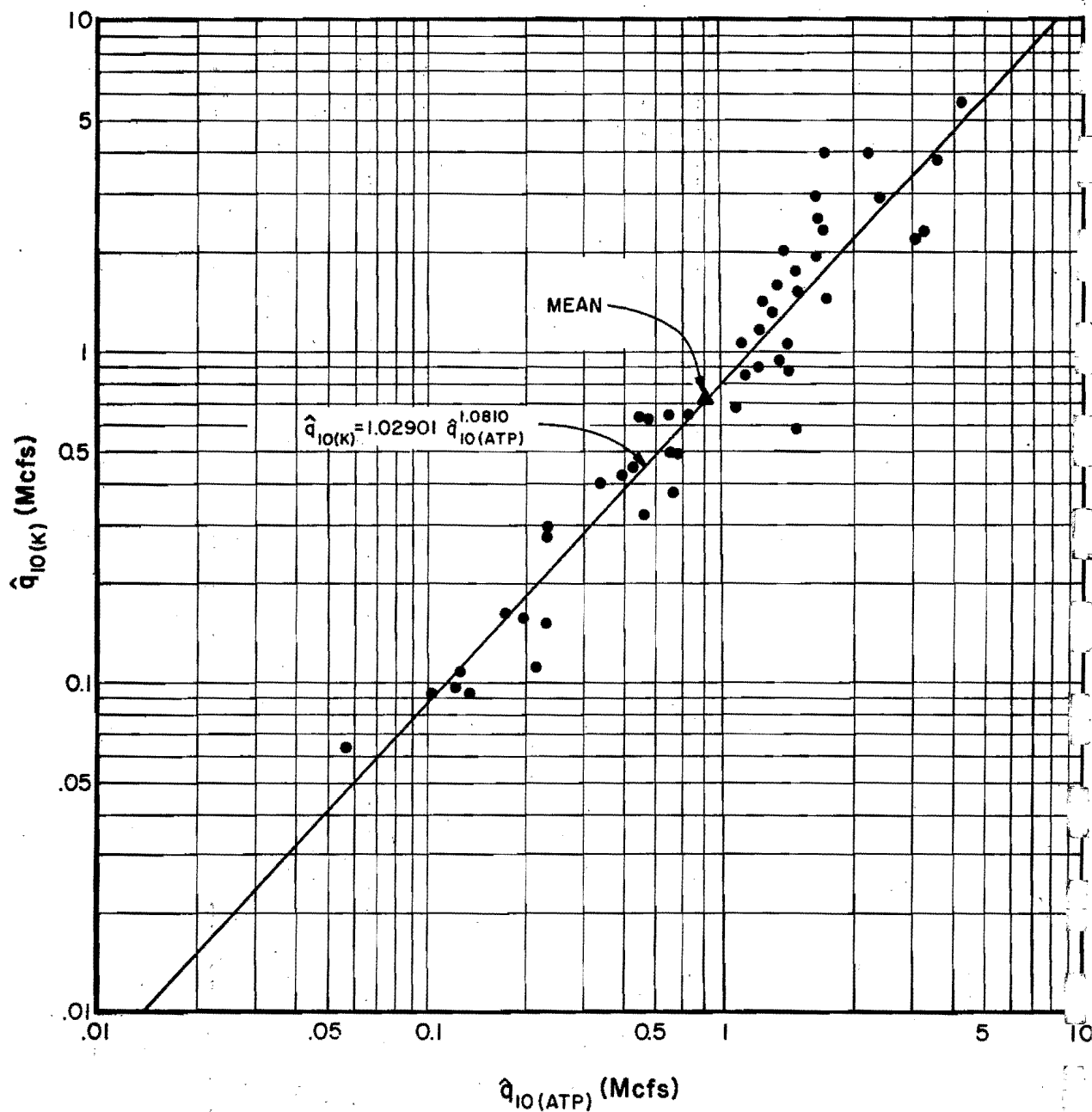


Figure 9. The K correction curve. The relationship between the observed values of the 10-year flow peaks and those estimated by graphical correlation with area, topographic factor, and precipitation factor for Potter Group 1 data.

Table 4. Comparison of 10-year runoff peaks and estimates (Potter methods).

Watershed No.	$\hat{q}_{10}$ Mcfs	$\hat{q}_{10(ATP_{60})}$ Mcfs	$\hat{q}_{10(K)}$ Mcfs	Error % of $\hat{q}_{10(K)}$	Watershed No.	$\hat{q}_{10}$ Mcfs	$\hat{q}_{10(ATP_{60})}$ Mcfs	$\hat{q}_{10(K)}$ Mcfs	Error % of $\hat{q}_{10(K)}$		
Zone I					Zone II (Continued)						
Group 1	1	0.096	0.121	0.105	8.5	Group 2	20	1.420	1.477	1.569	9.5
	2	0.270	0.231	0.211	-27.9		22	0.230	0.657	0.653	64.8
	3	0.440	0.424	0.407	-8.1		23	0.385	0.737	0.740	47.9
	4	0.400	0.340	0.321	-24.8		27	0.918	1.065	1.102	16.7
	6	1.060	0.921	0.941	-12.6		29	0.530	0.949	0.973	45.5
	12a	0.110	0.217	0.197	44.2		30	0.830	0.935	0.957	13.3
	39	0.830	0.938	0.960	13.6		31	0.590	0.979	1.006	41.3
	42	1.050	1.263	1.324	20.7		32	0.520	1.024	1.055	50.7
	44	0.860	1.280	1.344	36.0		34	0.920	1.083	1.121	16.6
	50	1.550	1.189	1.241	-24.9		35	1.140	1.127	1.171	2.7
	58	3.900	1.652	1.770	-120.3		36	1.250	1.149	1.196	-4.5
Group 1 $\hat{q}_{10}$ mean = 0.9605 Mcfs						40	1.100	1.245	1.304	15.7	
Percent Standard Error of Estimate = 77.5						45	2.100	1.587	1.695	-23.8	
Group 2	5	0.690	0.530	0.518	-33.2	48	2.350	1.609	1.721	-36.6	
	8	0.860	0.818	0.828	-3.8	Group 2 $\hat{q}_{10}$ mean = 1.0202					
	9	0.860	0.930	0.952	9.6	Percent Standard Error of Estimate = 36.7					
	10	0.910	0.905	0.924	1.5	Zone III					
	19	1.750	1.523	1.622	-7.9	Group 1	2	0.635	0.546	0.535	-18.7
	20	1.800	0.957	0.981	-83.4		3	0.370	0.566	0.556	33.5
	22	5.600	2.638	2.936	-90.7		4	0.490	0.580	0.571	14.2
	13a	0.142	0.344	0.324	56.1		9	1.150	1.134	1.179	2.4
	18b	0.179	0.344	0.324	44.9		10	2.000	1.225	1.281	-56.1
	38	0.660	0.982	1.009	34.6		12	1.500	1.372	1.448	-3.6
	41	1.300	1.125	1.169	-7.7		14	0.580	1.356	1.430	59.4
	47	0.820	1.256	1.316	37.7		17	1.720	1.347	1.420	-21.1
	49	1.150	1.429	1.514	24.0		22	2.500	1.586	1.694	-47.6
	53	1.870	1.739	1.872	0.1	Group 1 $\hat{q}_{10}$ mean = 1.216					
	55	2.050	1.812	1.956	-4.8	Percent Standard Error of Estimate = 44.3					
	56	2.050	1.396	1.476	-38.9	Group 2	1	0.014	0.070	0.058	75.9
Group 2 $\hat{q}_{10}$ mean = 1.4182							6	0.710	0.662	0.659	-7.8
Percent Standard Error of Estimate = 55.7							7	3.600	1.229	1.286	-180.0
Zone II							13	3.900	1.820	1.965	-98.4
Group 1	1	0.107	0.129	0.112	4.9		18	2.380	1.729	1.860	-28.0
	2	0.061	0.057	0.047	-31.2		19	1.850	1.418	1.501	-23.2
	3	0.155	0.200	0.181	14.2		20	1.550	1.433	1.518	-2.1
	4	0.092	0.102	0.087	-5.5		21	8.700	2.441	2.700	-222.2
	5	0.096	0.103	0.088	-8.9	Group 2 $\hat{q}_{10}$ mean = 2.838					
	6	0.092	0.137	0.120	23.3	Percent Standard Error of Estimate = 97.0					
	7	0.420	0.393	0.375	-12.0	Zone IV					
	8	0.620	0.472	0.457	-35.7	Group 1	2	0.490	0.575	0.566	13.4
	9	0.160	0.176	0.157	-1.7		6	1.240	1.223	1.279	3.1
	10	0.295	0.233	0.213	-38.4		7	2.900	1.536	1.636	-77.2
	11	0.150	0.230	0.210	28.6		10	2.200	2.049	2.235	1.5
	14	0.900	1.011	1.041	13.6		12	2.900	2.427	2.683	-8.1
	15	0.630	0.444	0.428	-47.3		15	2.280	3.293	3.732	38.9
	16	0.320	0.461	0.446	28.2		16	2.130	3.155	3.563	40.2
	21	0.630	0.630	0.624	-0.9		17	3.700	3.616	4.129	10.4
	26	1.400	1.071	1.108	-26.3	Group 1 $\hat{q}_{10}$ mean = 2.230					
	28	0.660	0.868	0.883	25.3	Percent Standard Error of Estimate = 44.8					
	33	0.940	1.234	1.292	27.2	Group 2	3	1.910	1.191	1.243	-55.6
	37	1.300	1.153	1.200	-8.3		5	0.740	1.652	1.214	39.0
	43	2.300	1.627	1.742	-32.1		8	1.060	1.715	1.844	42.5
	46	1.900	1.571	1.677	-13.3		9	1.150	1.818	1.963	41.4
	51	1.420	1.660	1.780	20.2		11	1.600	2.302	2.534	36.9
	57	3.950	2.252	2.475	-59.6		13	0.750	2.258	2.482	69.8
	59	5.600	4.364	5.060	-10.7	Group 2 $\hat{q}_{10}$ mean = 1.202					
Group 1 $\hat{q}_{10}$ mean = 1.008						Percent Standard Error of Estimate = 100.3					
Percent Standard Error of Estimate = 38.7											

Error % of  $\hat{q}_{10(K)}$  is defined as  $\frac{\hat{q}_{10} - \hat{q}_{10(K)}}{\hat{q}_{10(K)}} \times 100$

percent standard error of estimate is defined as  $\frac{100}{\hat{q}_{10 \text{ mean}}}$

$$\sqrt{\frac{\sum (\hat{q}_{10} - \hat{q}_{10(K)})^2}{n-2}}$$



greatly reduced by the development of a correction factor derived from using the Group 2 data. However, this left no possibility of an independent check of the method as this exhausted the data available to him at that time, but gave the best fit he could achieve by graphical methods, which he then published. USU tested an extension of the method described above which may be termed as a modified Potter method and is described in the following sections.

### Improve Potter's Method with Additional Years of Data

#### Determination of 10-year flood peak

Climate, record length, and the procedure for determining the 10-year annual flood peaks influence the values obtained. Schmidt (ref. 14) compared the commonly used distributions for determination of flood frequencies on 167 watersheds within the Great Basin and the Colorado River Drainage Basin. He found that within the range of the data, the inverse cubic polynomial fit the flood data consistently best; and Gumbel, log normal, and log Pearson III were good fitting functions within the range of the data.

It was mentioned earlier, that the Potter values of  $q_{10}$  and those derived by USU using the same upper and lower frequency method were not significantly different by the t test. The  $r^2$  value for this comparison was 0.789 indicating that some differences in the form of "noise" exist but are sufficiently random in nature that the t value is not significant. The  $r^2$  values for Potter  $q_{10}$  versus USU  $q_{10}$  with additional years of record were 0.679 and for USU  $q_{10}$  to 1958 and  $q_{10}$  to date, 0.895. The difference or unexplained variation is random in nature. Note that there is a greater difference between operators than between the two different sets with different numbers of years of record.

#### Other return periods

Table 5 (p. 25) shows the simple correlation coefficients between different return values of annual flood peaks determined from the data through 1958 and to the present. Some watersheds were discontinued during the period between 1958 and the present so the record lengths are variable but most watersheds had additional years of record between 1958 and the present. The distribution of the record lengths may be seen in Figure 10 (p. 26).

The correlations between the various frequencies are all close but there is a consistently closer relation between the  $q_{10}$  and  $q_{50}$  values than between  $q_{10}$  and  $q_{2.33}$ . There is a poorer relation with the  $q_{10}$  values derived by the log extreme method than between values derived from different record lengths. The correlations between the peak flow values to 1958 and the same frequencies of  $q$  to date are as follows: 2.33 yr.,  $r = 0.743$ ; 10 yr.,  $r = 0.946$ ; 25 yr.,  $r = 0.885$ ; and 50 yr.,  $r = 0.765$ . This suggests that Potter showed wisdom in working with  $q_{10}$  values and then calculating other return periods from the  $q_{10}$

Table 5. Correlation coefficients,  $r$ , for various frequencies and record lengths.

	USU $q_{10}$ to 58	Potter $q_{10}$	USU $q_{10}$ to Date	USU Log $q_{10}$ to Date	USU $q_{25}$	USU $q_{50}$
USU to 1958						
$q_{10}$	---	0.888	0.972	0.963	0.921	0.890
$q_{2.33}$	0.835	0.933	0.812	0.856	0.708	0.661
$q_{25}$	0.963	0.786	0.923	0.903	---	0.858
$q_{50}$	0.883	0.682	0.830	0.809	0.970	---
USU to Date						
$q_{10}$	0.972	0.862	---	0.985	0.943	0.916
$q_{2.33}$	0.827	0.871	0.832	0.838	0.714	0.683
$q_{25}$	0.921	0.795	0.932	0.936	---	0.992
$q_{50}$	0.890	0.752	0.916	0.897	0.992	---
log $q_{10}$	0.963	0.888	0.985	---	0.932	0.897

Effects of record length on interrelationships  
among parameters

Table 6 (p. 27) shows a group of correlation coefficients for watershed parameters for the period through 1958 and with the records to date. Consistently, the coefficients are higher as the record gets longer, as one would expect. All relationships have the same number of watersheds (96) in the correlation.

Map scale effects

Map scales effect the physical parameters read therefrom principally because the smaller the map scale ratio, the shorter are the lengths of the drainages drafted in blue lines. If scales are other than the  $7\frac{1}{2}$  minute quadrangles, a correction factor must be used to make values comparable. Figure 11 (p. 28) shows the relationship between L values derived from the 1:24,000 and 1:250,000 scale maps. The  $r^2$  is about 0.9 for this relationship. It appears to be within the errors in the determination of  $q_{10}$ , but note that the curve slope is not 1:1 and that maps with scales nearer the  $7\frac{1}{2}$  minute maps would be between the 1:1 and the line of Figure 11 (p. 28).

Figure 12 (p. 29) gives the relationship between DD from the 1:24,000 and the DD from the 1:250,000 scale maps. The  $r^2$  for this relation is naturally

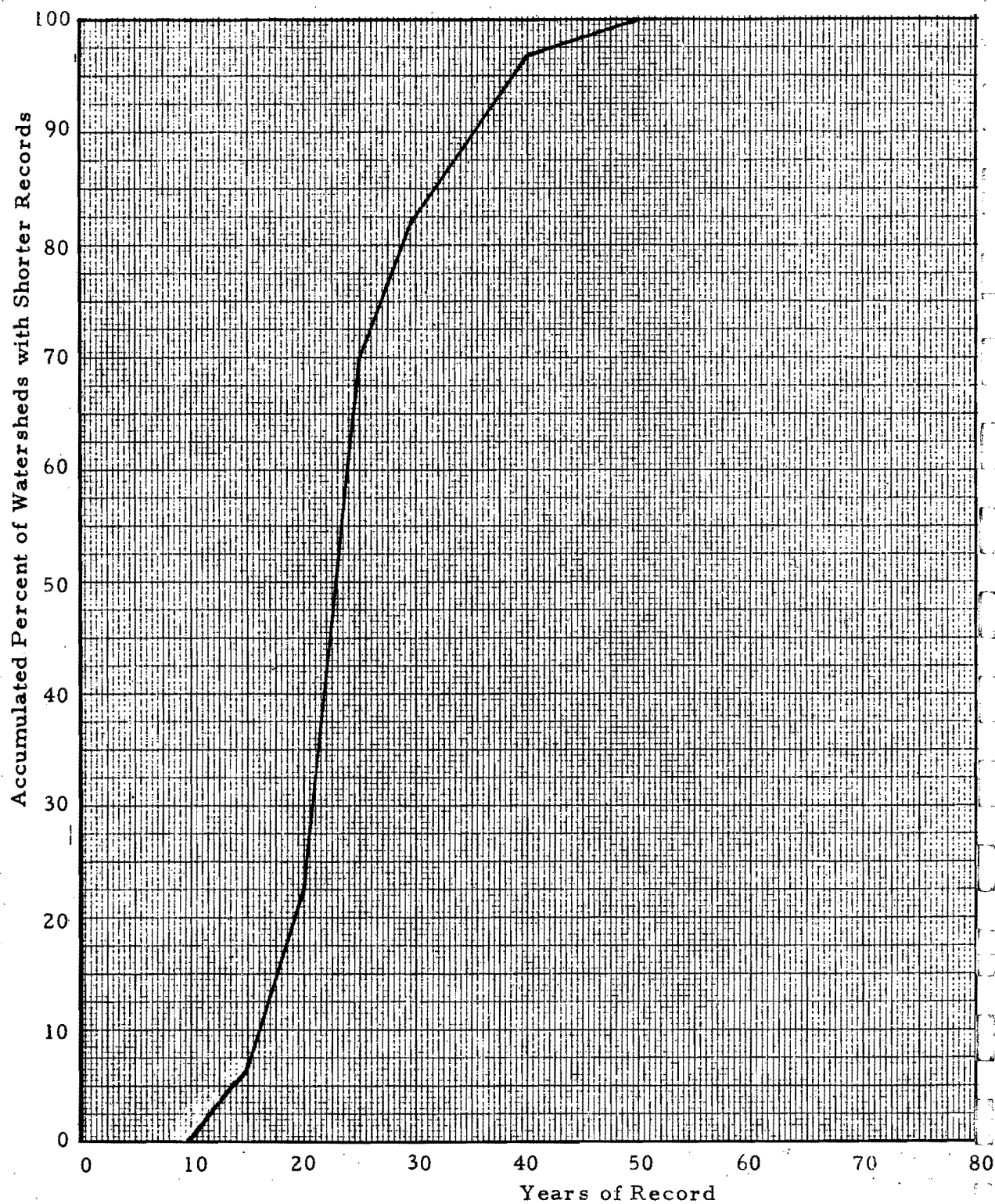


Figure 10. The distribution of record lengths on the Potter watersheds in the "to date" sample.

Table 6. Correlation coefficients,  $r$ , for watershed parameters with 10-year peak flows,  $q_{10}$ , derived from watershed records through 1958 and with additional years to date.

	USU $q_{10}$ to 58	$q_{10}$ to Date	$q_{10}/A$ to Date
<u>Linear</u>			
USU Area	0.691**	0.711**	-0.498**
USU T	0.417**	0.470**	-0.529**
USU P <sub>60</sub>	0.215*	0.266*	0.552**
L	0.523**	0.545**	-0.601**
USU DD	-0.240	-0.179*	0.244**
Storage	-0.173	-0.168	0.518**
P <sub>10</sub>	0.209*	0.234*	0.558**
$q_{50}$	0.883**	0.830**	-0.172
<u>Log-log</u>			
USU T	0.619**	0.641**	-0.727**
USU DD	-0.275**	-0.288**	0.367**

\* is 5 percent probability.

\*\* is 1 percent probability.

(Note that the transformed data correlates better than the untransformed.)

somewhat poorer than for the L values, but the DD values are still usable if no better maps are available.

The mean slope of a watershed can readily be determined even on the 1:250,000 map. Thus an estimate of  $L/\sqrt{S}$  can readily be made. The relation between Potter's T and  $L/\sqrt{S}$  values measured from the same scale maps are given in Figure 13 (p. 30). The  $r^2$  for this relation is 0.9216, so it could be used with confidence.

#### Extend Potter's Method to Additional Areas in States Sampled by Potter

##### Location of Potter and new watersheds

The locations of Potter's watersheds and the additional watersheds in the same States are shown in Figure 14 (p. 31). The descriptions of each of the Potter watersheds are given in Table 1 (p. 5-9) and the data for the additional watersheds considered in this phase are given in Table 7 (p. 32-36). As may be seen in Figure 14 (p. 31), the Potter States are Delaware, Illinois, Iowa, Kentucky, Maryland, Missouri, Nebraska, New Jersey, New York, North Carolina, Ohio, Oklahoma, Pennsylvania, Texas, Virginia, and Wisconsin.

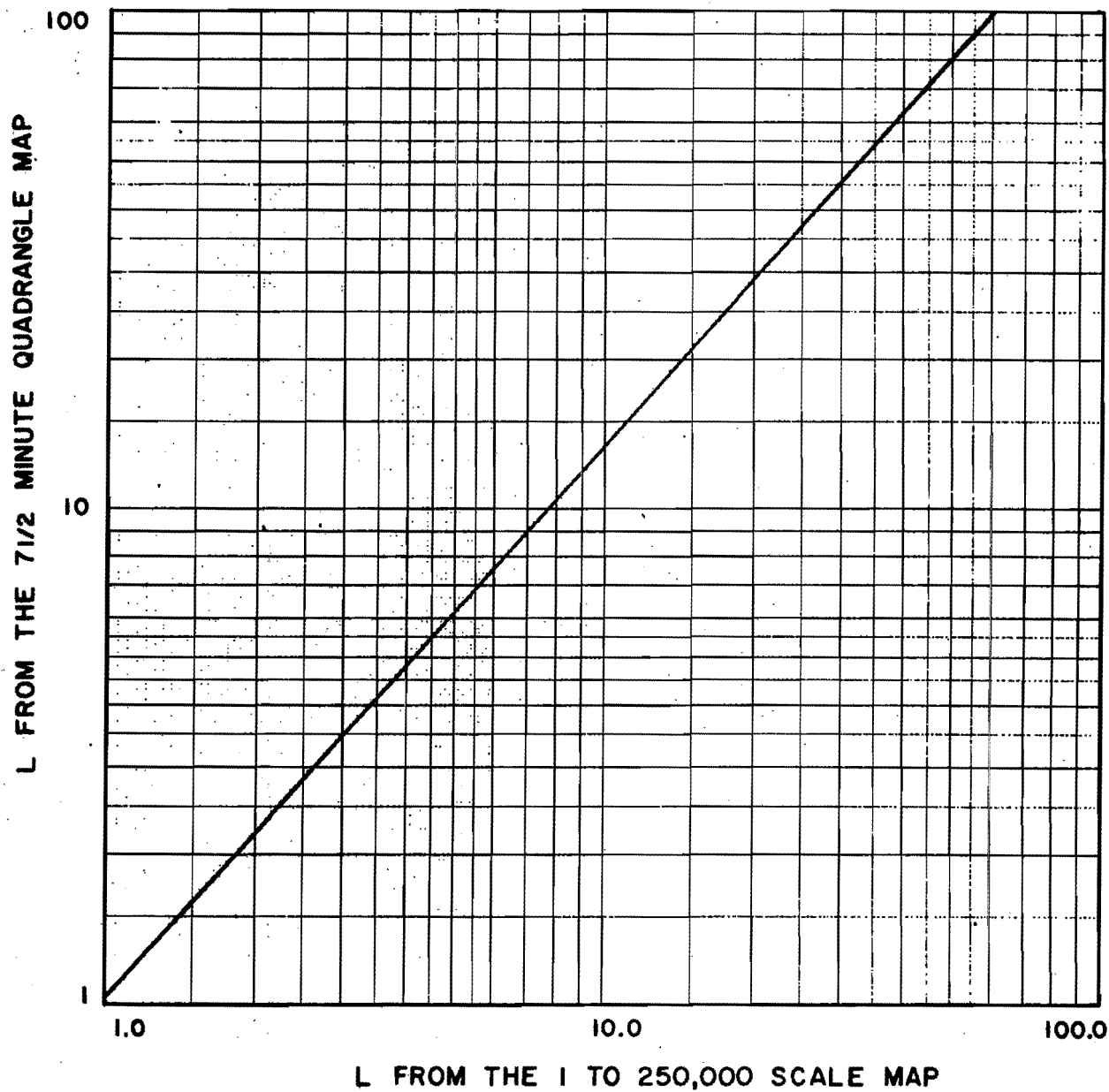


Figure 11. The relationship of L values measured on 1:24,000 maps to those measured on 1:250,000 maps.

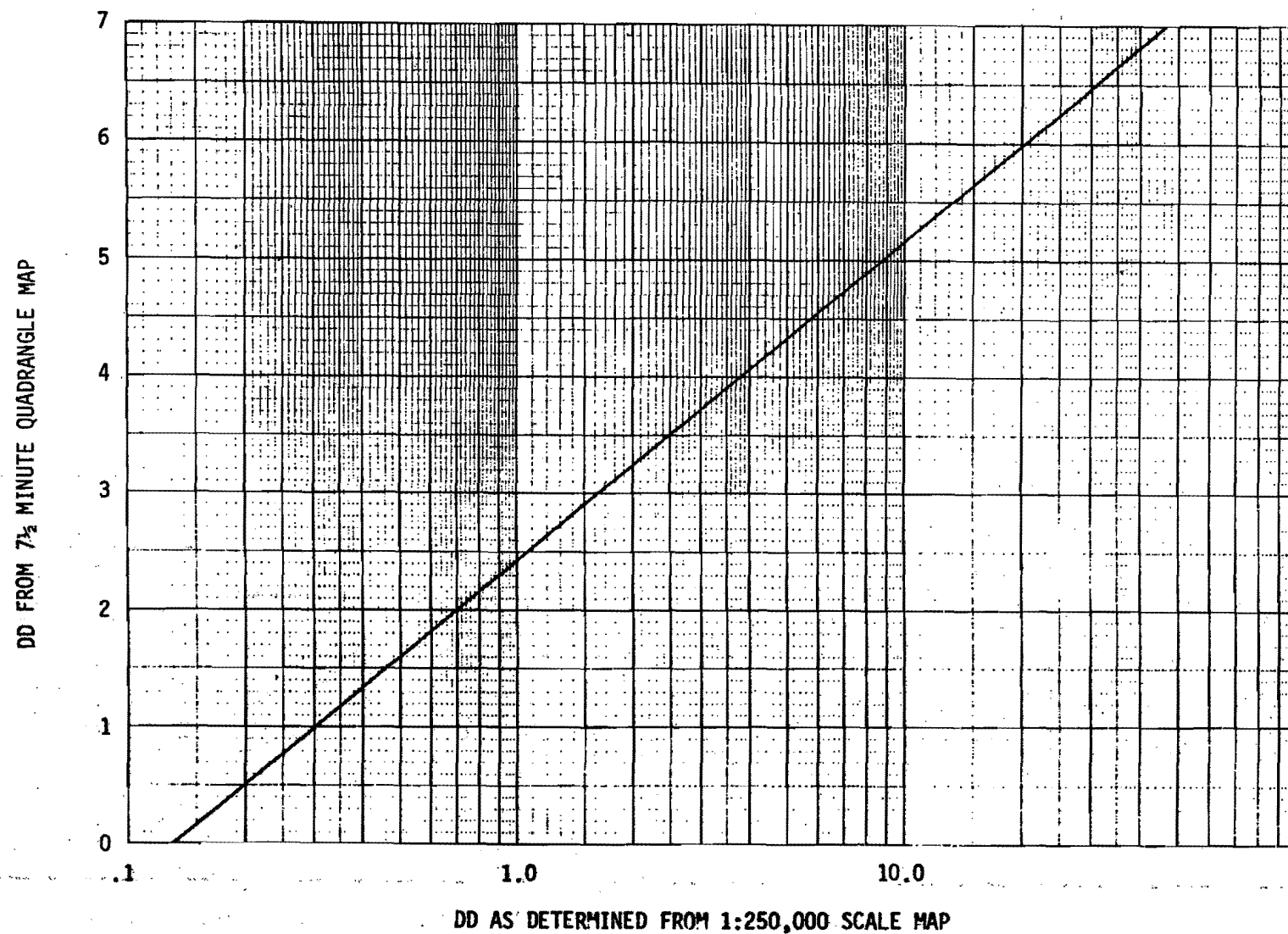


Figure 12. The relationship of DD from 1:24,000 scale maps to DD from the 1:250,000 scale maps.

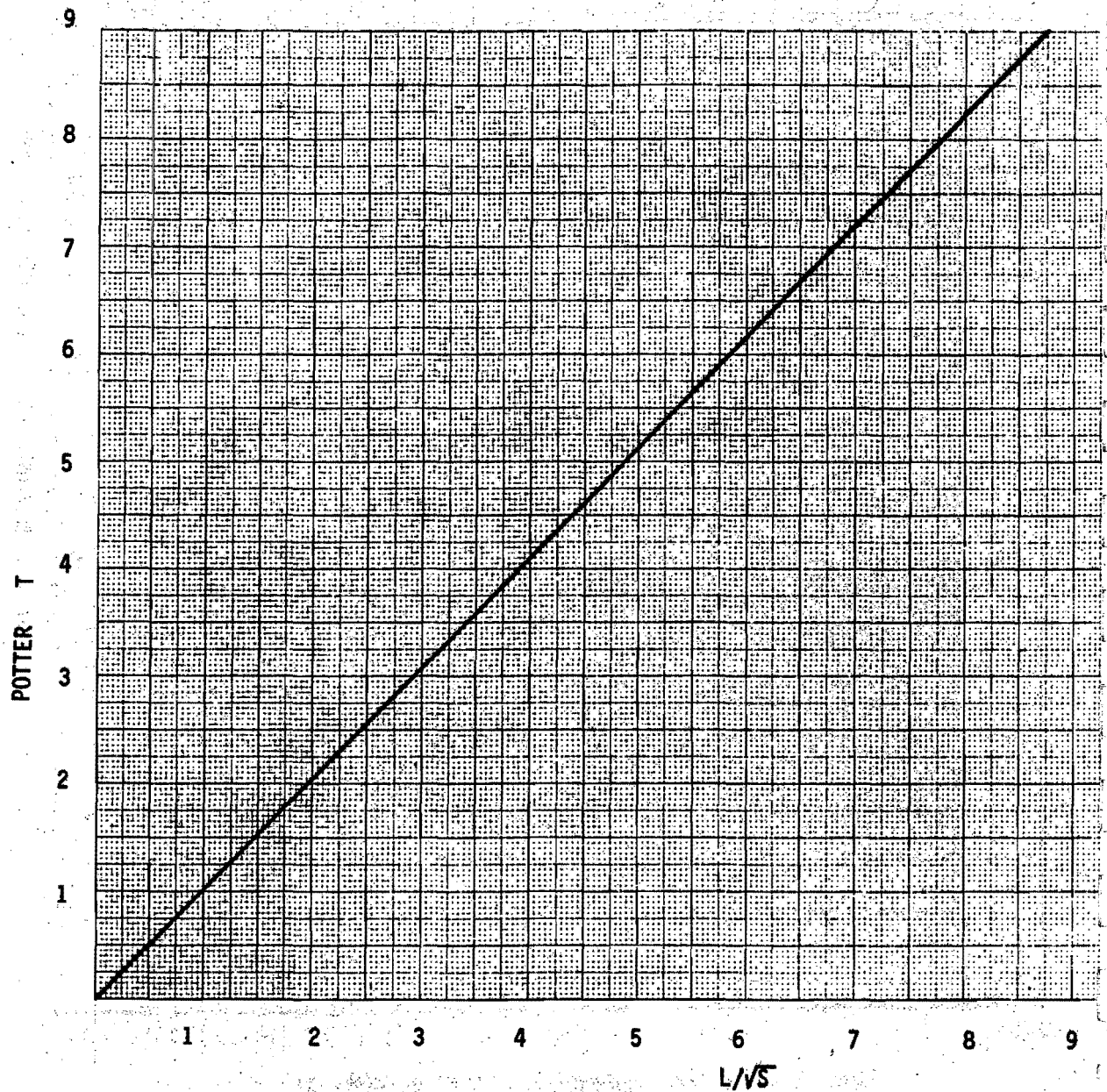


Figure 13. The relationship of Potter's  $T$  to  $L/\sqrt{S}$  for the Potter watersheds.

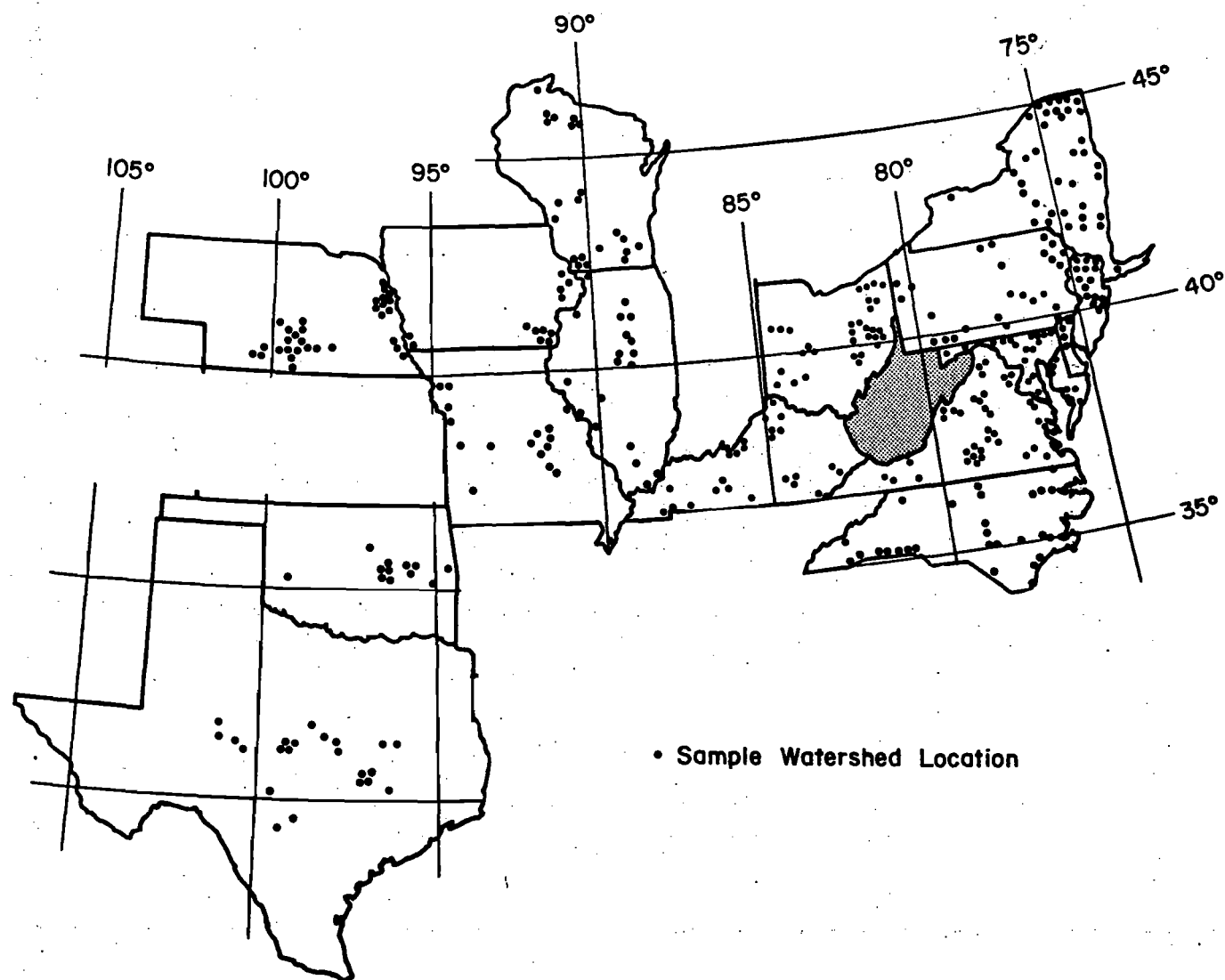


Figure 14. The location of the 270 sample watersheds in the States considered by Potter.



Table 7. Data on additional watersheds in Potter States.

SEQ #	STATION	STATION NAME	LAT	LONG	1	2	3	4	5	6	7	8	9	10	11	12	13	14
					80 MI TH	ACRE	PTR	ZN	PRG	AREA	P60	P10	DD	STORAGE	LENGTH	PHYS	ZN	Q10/A
1	81463500	LETSIC R NR CHENOLD DELAWARE	39-14	873-38	9.35	3,3048	4	A1	2.46	7.77	1.972	0.947	0.0	5.00	3A	8.930	1.108	0.107
2	81463500	BLACKBIRD CR AT BLACKBIRD DELAWARE	39-32	875-48	3.55	2,4548	4	A1	2.42	7.77	1.159	2.151	0.0	5.00	3A	8.930	0.157	0.157
3	81464500	STOCKLEY BR AT STOCKLEY DELAWARE	38-38	875-21	5.24	3,3556	4	A2	2.66	6.35	0.895	0.716	0.0	2.40	3A	0.100	0.133	0.636
4	81464500	TAPPANMANA DITCH NR HARTLY DELAWARE	39-37	875-42	5.53	3,7932	4	A2	2.51	7.27	1.540	0.725	0.0	2.75	3A	0.120	0.156	0.832
5	83344500	RANGE CR NR CISEY ILLINOIS	39-58	888-02	7.68	4,8848	1	C6	2.66	6.87	1.271	0.945	0.0	4.59	120	2.443	3.783	0.502
6	83365000	HAYES CR AT GLENDALE ILLINOIS	37-25	888-40	10.50	12,9986	3	B19	2.12	7.83	2.928	1.536	0.0	11.75	30	4.384	6.166	0.551
7	83365000	BUCK CR NR OXON SPRINGS ILLINOIS	37-24	888-40	7.70	6,2880	3	B19	2.22	7.80	1.148	1.107	0.0	6.08	30	1.692	1.987	0.273
8	83435000	CEGAR CR NR WINSLOW ILLINOIS	42-48	885-54	1.29	1,8256	1	C10	2.06	6.58	0.383	1.880	0.0	2.10	120	0.106	1.136	0.131
9	85526000	TEARY CR NR CUSTER PARK ILLINOIS	41-14	898-00	12.08	7,8668	1	C8	1.98	6.40	2.285	1.888	0.0	6.65	120	0.689	1.187	0.887
10	85530500	TINLEY CR NR PALOS PARK ILLINOIS	41-39	887-46	11.36	7,2328	1	C4	1.94	6.28	1.612	1.280	0.0	7.58	120	0.975	1.233	0.131
11	85537500	LONG RUN NR LEPORT ILLINOIS	41-39	888-00	20.80	13,3128	1	C5	1.95	6.31	3.814	1.027	0.0	8.58	120	1.712	2.864	0.159
12	85549000	BOONE CR NR MCHEWY ILLINOIS	42-19	888-19	15.30	9,7928	1	C11	1.96	6.31	2.359	1.114	0.0	7.68	120	0.228	0.268	0.033
13	85559000	GIMLET CR AT SPARLAND ILLINOIS	41-02	889-20	5.42	3,4688	1	B8	2.08	6.67	0.878	1.858	0.0	4.30	120	1.900	2.893	0.536
14	85560000	EAST BR PANTHER NR GRIOLEY ILLINOIS	48-48	888-54	6.30	4,9328	1	C8	2.06	6.61	0.888	0.944	0.0	2.40	120	0.435	1.085	0.108
15	85574000	SOUTH PK SANGAMON R NR NOKONIS ILLINOIS	39-21	889-19	10.80	6,9128	1	C8	2.17	6.88	1.813	0.941	0.0	5.45	120	4.252	6.838	0.615
16	85580500	HURRICANE CR NR RODHOUSE ILLINOIS	39-29	890-25	2.33	1,4912	1	C9	2.25	7.21	2.833	1.854	0.0	2.35	120	0.566	1.168	0.388
17	85580500	CANTEEN CR AT CASEYVILLE ILLINOIS	38-39	890-01	22.50	14,4888	1	C9	2.27	7.21	2.833	1.854	0.0	10.48	120	5.120	9.868	0.356
18	85590000	KASKASKIA R AT BONOVILLE ILLINOIS	48-07	886-21	12.30	7,8728	1	C8	2.06	6.58	1.284	0.888	0.0	5.00	120	0.893	1.682	0.113
19	85591500	ASA CR AT SULLIVAN ILLINOIS	39-37	888-36	7.93	5,8732	1	C9	2.18	6.73	1.314	0.862	0.0	2.89	120	0.797	1.199	0.137
20	85595500	MARYS R NR SPARTA ILLINOIS	38-06	889-39	17.80	11,5928	1	C10	2.27	7.24	2.582	1.133	0.0	8.40	120	4.454	9.884	0.391
21	85414450	NPK L MAQUOKETA R NR RICKARDSVILLE IOWA	42-35	890-51	22.80	14,5928	1	C10	2.10	6.67	2.793	1.145	0.0	12.24	120	4.131	6.231	0.293
22	85414800	L MAQUOKETA R TRIS AT DURQUE IOWA	42-33	890-42	1.51	0,9664	1	C10	2.09	6.64	0.244	1.850	0.0	2.28	120	1.912	2.978	0.306
23	85453000	RAPID CR TRIS NO 3 NR DAVIS IOWA	41-42	891-27	6.12	5,1988	1	C11	2.21	7.00	1.148	1.362	0.0	4.55	120	1.779	2.748	0.342
24	85453700	RAPID CR TRIS NO 4 NR DAVIS IOWA	41-43	891-25	1.95	1,2488	1	C9	2.21	7.00	0.431	0.883	0.0	2.17	120	0.621	0.958	0.498
25	85453750	RAPID CR SUBTRSD AT MORSE IOWA	41-43	891-20	15.20	9,7288	1	C9	2.21	7.00	1.689	0.815	0.0	8.28	120	2.909	4.419	0.299
26	85453950	RAPID CR TRIS NO 3 NR DAVIS IOWA	41-42	891-27	1.62	1,8368	1	C9	2.21	7.00	0.283	0.888	0.0	1.82	120	2.528	4.138	0.439
27	85454000	RAPID CR NR IOWA CITY IOWA	41-41	891-29	25.30	16,1928	1	C21	2.22	7.02	3.142	1.229	0.0	10.88	120	4.676	6.580	0.299
28	85455000	SP ENGLISH R TRIS NR BARNES CITY IOWA	41-33	892-28	2.51	1,8864	1	C21	2.30	7.21	0.483	0.819	0.0	2.88	120	0.791	1.138	0.493
29	86616000	INDIAN CR AT COUNCIL BLUFFS IOWA	41-18	895-58	7.99	5,1136	1	C20	2.48	7.89	1.075	1.075	0.0	5.35	120	2.116	3.211	0.413
30	86680000	MULE CR NR HALVERN IOWA	48-56	895-35	18.00	6,7840	1	C20	2.49	7.94	1.438	1.094	0.0	8.31	120	1.820	2.682	0.288
31	83203000	STILLWATER CR AT STILLWATER KY	37-45	863-29	24.00	15,3888	2	B15	2.06	6.35	1.515	0.898	0.0	7.13	60	5.321	7.617	0.348
32	83206000	COVE CR NR FORT SPRING KY	38-01	864-36	2.53	1,6192	3	B16	2.04	6.48	0.441	0.947	0.0	2.65	110	0.248	0.405	0.153
33	83208000	SOUTH ELKHORN CR AT FORT SPRING KY	38-03	864-38	24.00	15,3888	3	B16	2.03	6.45	2.678	1.171	0.0	10.13	110	1.695	2.229	0.194
34	83304000	MCGILL CR NR MCKINNEY KY	37-27	864-42	2.14	1,3696	3	B17	2.06	6.61	0.182	0.710	0.0	1.63	110	1.921	1.538	0.749
35	83305000	GREEN R NR MCKINNEY KY	37-25	864-45	22.40	14,3368	3	B17	2.09	6.64	1.387	1.018	0.0	6.48	110	6.944	12.263	0.884
36	83309000	MCDUGAL CR NR HODGENSVILLE KY	37-32	865-48	5.34	3,4176	3	B17	2.06	6.64	1.574	2.198	0.0	4.98	110	2.089	3.847	0.611
37	83313000	WEST BAYS PK AT SCOTTSTOWN KY	38-45	866-12	7.47	4,7688	3	B17	2.14	6.83	0.608	0.908	0.0	4.63	110	3.235	4.818	0.877
38	83348000	ROSE CR AT NEED KY	37-23	867-36	2.10	1,3440	2	B28	2.10	6.68	0.808	0.749	0.0	1.93	110	1.017	1.358	0.737
39	83460000	WOOD CR NR LONDON KY	37-10	864-87	3.80	2,4896	2	B15	2.11	6.69	0.417	0.742	0.0	2.50	80	0.495	0.870	0.199
40	87822000	PERRY CR NR MAYFIELD KY	38-41	866-36	1.72	1,1888	4	A7	2.23	7.85	0.300	0.619	0.0	1.98	30	1.126	1.593	1.085
41	81460000	MANDIN BR NR PRINCEGE ANNE MARYLAND	38-12	875-48	5.00	3,7120	4	A1	2.71	6.61	1.482	0.636	0.0	3.65	3A	0.313	0.461	0.804
42	81460000	CHICAMACONICO R NR SALEM MARYLAND	38-31	875-53	15.00	9,8880	4	A2	2.08	6.42	7.436	0.875	0.0	6.64	3A	0.422	0.592	0.844
43	81402000	BEAVERDAM BR AT MATTHEWS MARYLAND	38-40	875-58	5.05	3,7440	4	A2	2.08	6.26	1.179	0.915	0.0	3.98	3A	1.920	1.619	0.276
44	81403000	UNSCORN BRANCH AT MILLINGTON MARYLAND	39-15	875-52	22.30	14,9720	4	A1	2.46	7.94	4.284	1.242	0.0	10.33	3A	0.740	1.108	0.882
45	81403000	MORGAN CR NR KENNEOVILLE MARYLAND	39-17	876-01	18.50	6,7880	4	A1	2.06	6.10	1.785	0.947	0.0	5.42	3A	1.586	4.682	0.236

Table 7. Continued.

SEC #	STATION	STATION NAME	LAT	LONG	SQ MI	TH	ACRE	PTR	ZN	PBS	AREA	PBR	P10	PTR	T	DD	STORAGE	LENGTH	PHYS	ZN	D10	Q50	C10/A
46	81408888	NORTHEAST CR AT LESLIE MARYLAND	39-36 875-87	24-36 15-5328	4	810	2.35	7.31	0.88	1.08	0.8	14.39	3A	3.59	4.48	0.283							
47	81501588	SYNUN RUN AT BELTIE MARYLAND	39-32 876-28	24-35 15-5328	4	810	2.45	7.76	0.89	1.12	0.8	5.77	3A	2.73	4.18	0.562							
48	81503388	SLADE RUN NR GLENDON MARYLAND	39-32 876-48	24-35 15-5328	4	810	2.68	7.88	0.88	0.84	0.8	6.43	4A	6.34	8.94	0.286							
49	81505588	CORNBERRY BRANCH NR WESTMINSTER MO	39-38 876-58	24-39 15-5328	4	815	2.48	7.47	0.451	1.843	0.8	3.33	4A	6.758	1.217	0.388							
50	81508888	NORTH R NR ANNAPOLIS MARYLAND	38-59 876-37	24-35 15-5328	2	811	1.82	6.36	1.129	0.832	0.8	4.87	3A	1.262	2.252	0.232							
51	81507888	CHARTREE CR NR SWANTON MARYLAND	39-38 876-18	24-35 15-5328	2	815	1.95	6.19	0.918	1.254	0.8	9.06	8D	1.535	2.404	0.144							
52	81607888	LITTLE CATOCTIN CR AT HARMONY MARYLAND	39-29 877-38	24-35 15-5328	4	812	2.35	6.88	0.899	0.755	0.8	3.07	5A	2.622	4.249	0.484							
53	81608888	OMENS CR AT LANTZ MARYLAND	39-41 877-28	24-35 15-5328	4	812	2.18	6.95	0.888	0.883	0.8	3.78	5B	1.377	2.181	0.363							
54	81601888	MUNTING CR AT JIMTOWN MARYLAND	39-36 877-24	24-35 15-5328	4	810	2.28	6.99	0.891	1.312	0.8	9.92	3B	1.222	1.678	0.184							
55	81601888	CHAPTICO CR AT CHAPTICO MARYLAND	38-23 876-47	24-35 15-5328	4	812	2.71	6.81	1.429	1.133	0.8	6.54	3A	2.695	4.645	0.304							
56	81601888	ST MARVS R AT GREAT HILLS MARYLAND	38-23 876-38	24-35 15-5328	4	811	2.75	6.87	0.739	0.968	0.8	8.39	3A	3.820	6.697	0.749							
57	81503388	DOUGLAS CR NR EMDEN MISSOURI	39-45 891-85	24-35 15-5328	4	810	2.33	7.39	0.983	0.838	0.8	2.42	12E	1.262	1.766	0.175							
58	86816888	MILL CR AT OREGON MISSOURI	39-59 895-88	24-35 15-5328	1	828	2.58	6.86	0.447	0.742	0.8	2.89	12E	2.307	3.594	0.736							
59	86826888	WHITE CLOUD CR NR MARYVILLE MISSOURI	38-23 894-55	24-35 15-5328	1	828	2.52	7.96	1.136	1.126	0.8	4.87	12E	2.531	3.947	0.693							
60	86826888	BIG BLOUGH NR WILCOX MISSOURI	40-23 894-56	24-35 15-5328	1	828	2.52	7.96	0.981	0.794	0.8	1.88	12E	0.934	1.336	0.133							
61	86804588	EPK FISHING R AT EXCELSIOR SPRINGS MO	39-28 894-13	24-35 15-5328	1	828	2.53	7.99	2.484	1.498	0.8	11.81	12E	7.416	11.453	0.579							
62	86807588	CR BK BLACKWATER R NR ELM MISSOURI	38-49 894-82	24-35 15-5328	3	828	2.54	8.82	2.120	1.286	0.8	9.18	12F	5.091	7.428	0.485							
63	86808888	SHILOH BRANCH NR MARSHALL MISSOURI	39-67 893-85	24-35 15-5328	1	828	2.47	7.78	0.958	0.986	0.8	2.86	12F	0.944	1.279	0.514							
64	86826888	COYLE BRANCH AT HOUSTON MISSOURI	37-19 891-57	24-35 15-5328	3	828	2.45	7.75	0.168	0.619	0.8	1.52	14A	0.541	0.823	0.768							
65	86835088	RUNDO BRANCH AT DANVILLE MISSOURI	38-55 891-32	24-35 15-5328	1	828	2.38	7.48	0.316	0.979	0.8	8.84	12E	0.562	0.884	0.630							
66	87815888	LANES FORK NR VICHY MISSOURI	38-66 891-43	24-35 15-5328	3	828	2.39	7.87	0.882	1.114	0.8	9.85	14A	0.514	0.612	0.432							
67	87185588	STEAL CR NR MILLER MISSOURI	37-12 893-51	24-35 15-5328	3	828	2.62	8.33	0.481	0.819	0.8	2.84	14A	1.291	1.815	0.533							
68	87811588	GREEN ACRE BRANCH NR ROLLA MISSOURI	37-55 891-44	24-35 15-5328	3	828	2.43	7.97	0.134	0.742	0.8	6.85	14E	1.314	2.158	0.478							
69	86808888	SOUTH OMAHA CR TRIS NR WALTHILL NEBRASKA	42-85 896-29	24-35 15-5328	1	828	2.43	7.72	0.893	0.826	0.8	2.36	12E	1.089	1.489	0.597							
70	86808888	SOUTH OMAHA CR NR WALTHILL NEBRASKA	42-87 896-29	24-35 15-5328	1	828	2.43	7.72	1.329	0.896	0.8	6.16	12E	0.147	0.757	0.636							
71	86807788	BRANCH TEKAMAH CR NR CRAIG NEBRASKA	41-58 896-28	24-35 15-5328	1	828	2.45	7.78	0.832	0.832	0.8	2.33	12E	1.282	1.888	0.432							
72	86807888	BRANCH TEKAMAH CR TRIS NR TEKAMAH NEB	41-58 896-28	24-35 15-5328	1	828	2.45	7.78	0.849	1.139	0.8	4.85	12E	2.287	3.644	0.678							
73	86807888	BRANCH TEKAMAH CR NR TEKAMAH NEBRASKA	41-58 896-28	24-35 15-5328	1	828	2.45	7.78	0.826	0.992	0.8	5.43	12E	2.242	4.345	0.472							
74	86808888	TEKAMAH CR NR TEKAMAH NEBRASKA	41-48 896-13	24-35 15-5328	1	828	2.45	7.78	2.421	1.336	0.8	11.32	12E	4.529	6.637	0.308							
75	86808888	NEW YORK CR TRIS NR SPIKER NEBRASKA	41-48 896-17	24-35 15-5328	1	828	2.48	7.82	0.348	0.848	0.8	2.31	12E	1.848	1.647	1.586							
76	86808888	NEW YORK CR NORTH OF SPIKER NEBRASKA	41-48 896-17	24-35 15-5328	1	828	2.48	7.82	0.875	0.941	0.8	4.22	12E	2.819	4.227	0.678							
77	86780188	ELM CR EAST OF SPIKER NEBRASKA	41-48 896-17	24-35 15-5328	1	828	2.48	7.82	1.183	1.818	0.8	6.66	12E	4.896	7.699	0.836							
78	86780188	EAST BUFFALO CR NR BUFFALO NEBRASKA	41-81 899-58	24-35 15-5328	1	828	2.48	7.82	1.889	1.962	0.8	9.89	13D	0.119	0.182	0.836							
79	83738488	WEST BUFFALO CR NR BUFFALO NEBRASKA	48-59 898-32	24-35 15-5328	1	828	2.48	7.82	0.826	0.992	0.8	10.37	13D	0.242	0.388	0.832							
80	82780188	ELM CR TRIS NR OVERTON NEBRASKA	48-53 898-43	24-35 15-5328	1	828	2.48	7.78	0.187	0.723	0.8	9.84	13D	0.124	0.175	0.334							
81	82780288	ELM CR NR SUMNER NEBRASKA	48-52 898-32	24-35 15-5328	1	828	2.48	7.78	2.862	1.414	0.8	9.64	13D	0.030	1.084	0.686							
82	86780188	ELM CR TRIS NR OVERTON NEBRASKA	48-53 898-43	24-35 15-5328	1	828	2.48	7.78	0.875	0.941	0.8	4.19	13D	0.403	0.609	0.112							
83	83723388	WOOD R TRIF NR LODI NEBRASKA	41-12 899-28	24-35 15-5328	1	828	2.39	7.66	0.968	1.248	0.8	3.13	13D	0.832	0.492	0.195							
84	86778788	WOOD R NR LODI NEBRASKA	41-18 899-48	24-35 15-5328	1	828	2.48	7.82	2.844	1.819	0.8	10.24	13D	0.128	0.198	0.615							
85	86777888	LILLIAN CR TRIS NR BROKEN BOW NEB	41-31 899-39	24-35 15-5328	1	828	2.39	7.66	0.348	0.898	0.8	2.24	13D	0.819	0.916	0.906							
86	86777888	LILLIAN CR NR BROKEN BOW NEBRASKA	41-31 899-39	24-35 15-5328	1	828	2.39	7.66	0.984	0.966	0.8	3.72	13D	0.037	1.833	0.285							
87	83778888	LILLIAN CR TRIS NR BROKEN BOW NEB	41-37 899-34	24-35 15-5328	1	828	2.36	7.66	0.894	1.923	0.8	3.83	13D	0.313	0.566	0.248							
88	86780288	S BR MUD CR TRIS NR BROKEN BOW NEB	41-26 899-43	24-35 15-5328	1	828	2.38	7.66	0.187	0.875	0.8	1.24	13D	0.154	0.239	0.588							
89	86780288	MUD CR TRIS NR BROKEN BOW NEBRASKA	41-26 899-43	24-35 15-5328	1	828	2.40	7.66	0.857	1.178	0.8	5.87	13D	0.988	1.536	0.234							
90	86782888	DAVIS CR TRIS NR 2 NR NORTH LOUP NEB	41-26 898-84	24-35 15-5328	1	828	2.43	7.72	1.874	1.274	0.8	9.87	13D	1.898	1.643	0.261							

Table 7. Continued.

SEC #	STATION	STATION NAME	LAT	LONG	80 MI. TH	ACRE	PTR	2N	P88	AREA	P88	P18	PTR	Y	DD	STORAGE	LENGTH	PHYS	2N	Q18	Q18/A
8	01	06708888 E BRANCH SPRING CR TR18 NR WOLBACH NEB	41-27 08-28	1-58 0-378	1	014	2-44	7-76	5-48	1-13	8-9	2-73	130	1-286	8-742	1-286	8-742	130	1-286	8-742	1-286
8	02	06708888 M BRANCH SPRING AT GRAYTON NEBRASKA	41-27 08-28	1-58 0-378	1	014	2-44	7-76	5-48	1-13	8-9	2-73	130	1-286	8-742	1-286	8-742	130	1-286	8-742	1-286
8	03	06806110 STOVE CR AT ELWOOD NEBRASKA	40-45 08-10	10-38 0-8928	1	028	2-58	7-97	1-358	1-618	8-6	15-88	122	1-637	7-942	1-637	7-942	122	1-637	7-942	1-637
8	04	06816100 MOOPER CR TR18 NR PALMYRA NEBRASKA	40-48 08-22	9-88 0-1288	1	028	2-58	7-97	1-358	1-618	8-6	15-88	122	1-637	7-942	1-637	7-942	122	1-637	7-942	1-637
8	05	06816100 LITTLE NEHAJA R TR18 NR BYRACUSE NEB	40-48 08-22	9-88 0-1288	1	028	2-58	7-97	1-358	1-618	8-6	15-88	122	1-637	7-942	1-637	7-942	122	1-637	7-942	1-637
8	06	06816100 FRAZIER CR NR MAYWOOD NEBRASKA	40-48 08-22	9-88 0-1288	1	028	2-58	7-97	1-358	1-618	8-6	15-88	122	1-637	7-942	1-637	7-942	122	1-637	7-942	1-637
8	07	06839950 CUT CANYON NR CURTIS NEBRASKA	40-44 18-32	25-48 15-5888	1	014	2-38	7-37	3-133	1-798	8-6	15-72	130	1-886	1-548	1-886	130	1-886	1-548	1-886	1-548
8	08	06848888 DAY CR NR CURTIS NEBRASKA	40-44 18-32	25-48 15-5888	1	014	2-38	7-37	3-133	1-798	8-6	15-72	130	1-886	1-548	1-886	130	1-886	1-548	1-886	1-548
8	09	01334350 RINGWOOD CR NR MANAQUE NEBRASKA	41-08 07-18	15-18 12-2248	3	02	2-84	6-76	1-228	1-288	8-6	18-82	98	8-861	1-221	1-288	98	8-861	1-221	1-288	98
8	10	01334350 WEST BROOK NR MANAQUE NEBRASKA	41-08 07-18	15-18 12-2248	3	02	2-84	6-76	1-228	1-288	8-6	18-82	98	8-861	1-221	1-288	98	8-861	1-221	1-288	98
8	11	01334350 BLIND BROOK AT RYE NEW YORK	40-50 07-31	1-28 7-5888	4	01	2-12	8-76	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	1-548
8	12	01334350 HILL NECK CR AT HILL NECK NEW YORK	40-50 07-31	1-28 7-5888	4	01	2-12	8-76	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	1-548
8	13	01334350 CHAMPLIN CR AT ISLIP NEW YORK	40-44 07-12	6-58 4-1888	4	01	2-18	8-67	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	1-548
8	14	01334350 PENATAQUIT CR AT BAY SHORE NEW YORK	40-42 07-14	5-88 3-5888	4	01	2-18	8-67	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	1-548
8	15	01334350 SANTIAGOUE R AT LINDENHURST NEW YORK	40-41 07-21	7-88 4-4888	4	01	2-18	8-67	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	1-548
8	16	01334350 CHESTNUT CR AT GRANTVILLE NEW YORK	41-51 07-32	28-98 13-3788	1	06	1-98	5-84	4-484	8-891	8-6	15-88	88	3-833	4-774	3-833	88	3-833	4-774	3-833	88
8	17	01334350 DRYDEN CR NR GRANTVILLE NEW YORK	42-07 07-14	6-88 5-8848	1	06	1-78	5-84	4-484	8-891	8-6	15-88	88	3-833	4-774	3-833	88	3-833	4-774	3-833	88
8	18	01334350 TROUT CR NR ROCK ROVAL NEW YORK	42-19 07-16	28-48 13-8588	1	06	1-78	5-84	4-484	8-891	8-6	15-88	88	3-833	4-774	3-833	88	3-833	4-774	3-833	88
8	19	02001000 SMITHICK CR TR18 NR WILLIAMSTON N C	35-44 07-35	9-92 0-8888	4	03	2-92	8-86	1-428	1-858	8-6	15-88	38	1-398	2-198	1-398	38	1-398	2-198	1-398	38
8	20	02001000 WARTS HILL RUN NR TARBORO NORTH CAROLINA	35-56 07-37	9-38 5-4912	2	018	2-72	8-64	2-836	1-878	8-6	15-88	38	1-398	2-198	1-398	38	1-398	2-198	1-398	38
8	21	02001000 DEEPER NR SCOTLAND NECK N C	36-09 07-26	11-78 7-4888	4	02	2-72	8-64	2-836	1-878	8-6	15-88	38	1-398	2-198	1-398	38	1-398	2-198	1-398	38
8	22	02001000 STEIRUP IRON CR TR18 NR NELSON N C	35-53 07-38	8-28 9-1888	2	011	2-58	8-23	3-378	2-317	8-6	15-88	38	1-398	2-198	1-398	38	1-398	2-198	1-398	38
8	23	02001000 LEE SHANK TR18 NR LUCANA NORTH CAROLINA	35-38 07-02	2-83 1-8112	4	03	2-74	8-74	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	24	02001000 WHITEOAK SHANK TR18 NR WILSON N C	35-42 07-47	2-88 1-8848	4	03	2-78	8-78	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	25	02001000 PALMETTO SHANK NR VANCEBORO N C	35-28 07-18	24-28 10-4888	4	02	2-88	8-68	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	26	02001000 VINE SHANK NR KINSTON NORTH CAROLINA	35-09 07-33	8-84 3-8888	4	03	2-84	8-83	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	27	02001000 SOUTH PRONG ANDERSON CR NR LILLINGTON NC	35-16 07-55	7-28 4-8888	4	04	2-88	8-28	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	28	02001000 MATTHEWS CR NR PINK HILL NORTH CAROLINA	36-06 07-49	9-28 9-4888	4	03	2-83	8-68	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	29	02001000 TURKEY CR NR CASTLE WAYNE N C	34-24 07-55	18-28 9-3888	4	02	2-87	8-88	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	30	02001000 BUCKHEAD BRANCH NR BOLTON NORTH CAROLINA	34-21 07-26	15-38 9-7888	4	02	2-83	8-68	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	31	02001000 MILL BRANCH NR TABOR CITY NORTH CAROLINA	34-11 07-47	3-88 2-4848	4	02	2-82	8-68	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	32	02001000 BIG KNOB CR NR FALLBON NORTH CAROLINA	35-29 08-32	18-48 10-4888	4	026	2-82	7-37	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	33	02001000 BEYER CR NR SHANNANDA NORTH CAROLINA	35-39 08-24	5-48 3-4944	4	012	2-38	7-58	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	34	02001000 ALLEN CR NR HAZELWOOD NORTH CAROLINA	35-25 08-08	14-48 9-2188	2	014	2-31	7-57	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	35	02001000 CULLAJA R AT HIGHLANDS NORTH CAROLINA	36-04 08-14	14-98 9-3888	2	012	2-35	7-53	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	36	02001000 E BR SANDSTONE CR NR ELK CITY OKLAHOMA	35-31 08-32	23-98 14-7888	2	028	2-58	7-23	3-448	1-344	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48
8	37	35.1 GUTHRIE ARS WATERSHED NO N-1 OKLAHOMA	35-46 97-48	.8888 .8334	2	024	2-75	8-71	.864	2-87	8-6	12-8	48	1-886	1-548	1-886	48	1-886	1-548	1-886	48

Table 7. Continued.

BEG #	STATION	STATION NAME	LAT	LONG	80 MI	TH ACRE	PTR	ZN	PFB	AREA	P06	P10	PTR	T	00	STORAGE	LENGTH	PHYS	ZN	019	050	010/1
8 135	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 136	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 137	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 138	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 139	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 140	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 141	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 142	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 143	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 144	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 145	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 146	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 147	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 148	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 149	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 150	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 151	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 152	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 153	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 154	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 155	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 156	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 157	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 158	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 159	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 160	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 161	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 162	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 163	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 164	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 165	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 166	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 167	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 168	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 169	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 170	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 171	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 172	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 173	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 174	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 175	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 176	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 177	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 178	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 179	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 180	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 181	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 182	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 183	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 184	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 185	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 186	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 187	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 188	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 189	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 190	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 191	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 192	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 193	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 194	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 195	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 196	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 197	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 198	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 199	35-48 897-48	35-48 897-48	2	054	2.75	8.71	3262	96.51	0.0	0.003	127	8.807	0.807	2.236								
8 200	35-48 897-48	35-48 897-48	2	054	2.7																	

Table 7. Continued. Explanation of numbered column headings.

- <sup>1</sup>Area in square miles.
- <sup>2</sup>Area in thousands of acres.
- <sup>3</sup>Potter zone as derived from Potter's maps.
- <sup>4</sup>PRB Area. Soil Conservation Service Problem area.
- <sup>5</sup>P60. 10-year 60-minute rainfall. Inches.
- <sup>6</sup>P10. 10-year 10-minute rainfall intensity. Inches per hour.
- <sup>7</sup>PTIR T. Potter T value as computed by the authors.
- <sup>8</sup>DD. Drainage density at the total length of all drainage channels on the watershed in miles divided by the watershed area. Miles/  
thousand acres.
- <sup>9</sup>Storage. The percentage of the watershed covered by lakes, swamps, playas, etc.
- <sup>10</sup>Length. The length of the principal drainage channel as shown as either a solid or broken blue line on the topographic map. Miles.
- <sup>11</sup>Phys ZN. The physiographic section as taken from the Fenneman and Johnson map. Figure 20.
- <sup>12</sup>Q10. The 10-year runoff peak as determined from tabulations of flow data. Thousands of cfs.
- <sup>13</sup>Q50. The 50-year peak runoff as determined from actual measurement data. Thousands of cfs.
- <sup>14</sup>Q10/A. The 10-year peak runoff per unit of area. cfs/acre.

### Watershed parameters

The data showing the additional watersheds and parameters are given in Table 7 (p. 32-36). The values for the parameters in the additional watersheds from the Potter states are considered to be representative of the Potter watersheds themselves except that the additional watersheds represent more physiographic sections than Potter's original watersheds.

### Deviation of the modified Potter method curves for testing the new data

The method described previously relating the 10-year peak flow to area, topographic factor, and precipitation using only Potter Group 1 data was now repeated using the USU extended data from all 96 Potter watersheds. Potter's zonation was retained primarily for testing purposes. The derived equations are given in Table 8 (p. 38) and are shown graphically in Figures 15 through 18 (p. 39-42). Equations for each of the four Potter zones as well as a set lumping all the zones together were derived. The equations were then tested against a sample of 25 watersheds selected randomly from Table 7 (p. 32-36). The sampling was done by first randomly selecting one watershed from each State, then by taking one more sample from each State in which six or more watersheds were shown in Table 7 (p. 32-36) until a total of 25 watersheds were selected.

### Estimation of 10-year peak flow from Potter type curves

Three different estimates of the 10-year peak flow were made for comparison purposes for each of the 25 watersheds. This comparison is given in Table 9 (p. 43). A 10-year peak flow estimate is obtained as follows using Figures 15-18 (p. 39-43): First, enter Figure 15 (p. 39) with the watershed area and read the  $\hat{q}_{10(A)}$  value from the proper curve (zone or lumped). For example, if the area is 5.200 thousand acres in Zone I, read  $\hat{q}_{10(A)}$  as 1.15 Mcfs. Second, enter Figure 16 (p. 40) with the value of T and read  $DY_1$ . To continue the example, if T for the above Zone I watershed was 1.15, read  $DY_1$  as 0.99.  $DY_2$  is similarly determined from Figure 17 (p. 41) using  $P_{60}$  as the argument. For example, if the above watershed had a  $P_{60}$  of 2.20 inches, read  $DY_2$  as 1.70. The fourth step is to multiply the three above numbers together to obtain  $\hat{q}_{10(ATP)} = 1.15 \times 0.99 \times 1.70 = 1.94$  Mcfs. The final step is to enter Figure 18 (p. 42) with  $\hat{q}_{10(ATP)}$  as the argument and read  $\hat{q}_{10(K)}$  which is the estimate of  $q_{10}$ . To conclude the above example, entering Figure 19 (p. 44) with a value of 1.94 on the  $\hat{q}_{10(ATP)}$  axis the value for  $\hat{q}_{10(K)}$  is read as 2.66 Mcfs. Alternatively, the same value could have been obtained by solving the respective Zone I equations given in Table 8 (p. 38) for  $\hat{q}_{10(A)}$ ,  $DY_1$ ,  $DY_2$ , and  $\hat{q}_{10(K)}$ . The values given in the comparison, Table 9 (p. 43), were developed in a manner similar to that described above.

The comparison shown in Table 9 (p. 43) was designed to indicate the value of zoning as opposed to lumping all of the data together to derive a single set of equations. In addition, a third comparison was made to evaluate whether there was much difference between the USU derived equations and those curves published by Potter (ref. 13).

Table 8. Equations relating 10 year runoff peak,  $q_{10}$ , to area, A, topographic factor, T, and 10 year 60 minute precipitation,  $P_{60}$ , derived from all Potter watersheds using USU data.

Zone	Equation	(1) $\bar{q}_{10}$ Mcfs	(2) $PS_{EE}$ percent	(3) n	(4) r
I	$\hat{q}_{10(A)} = 0.45894 A^{0.55730}$	-	-	27	0.835
	$DY_1 = 0.98877 T^{-0.02446}$	-	-	27	-0.054
	$DY_2 = 0.15197 P_{60}^{3.06468}$	-	-	27	0.612
	$\hat{q}_{10(K)} = 1.08688 \hat{q}_{10(ATP)}^{1.34956}$	1.238	54.2	27	0.933
II	$\hat{q}_{10(A)} = 0.59009 A^{0.57097}$	-	-	38	0.862
	$DY_1 = 0.97075 T^{-0.02191}$	-	-	38	-0.061
	$DY_2 = 0.35418 P_{60}^{1.49032}$	-	-	38	0.468
	$\hat{q}_{10(K)} = 1.07015 \hat{q}_{10(ATP)}^{1.15286}$	1.301	85.7	38	0.902
III	$\hat{q}_{10(A)} = 0.47989 A^{0.72245}$	-	-	17	0.879
	$DY_1 = 1.01359 T^{0.02862}$	-	-	17	0.054
	$DY_2 = 0.01752 P_{60}^{5.42379}$	-	-	17	0.820
	$\hat{q}_{10(K)} = 1.00742 \hat{q}_{10(ATP)}^{0.97539}$	2.249	41.0	17	0.962
IV	$\hat{q}_{10(A)} = 0.81093 A^{0.43263}$	-	-	14	0.506
	$DY_1 = 1.00137 T^{0.00638}$	-	-	14	0.010
	$DY_2 = 0.06867 P_{60}^{3.11099}$	-	-	14	0.399
	$\hat{q}_{10(K)} = 1.03877 \hat{q}_{10(ATP)}^{0.93455}$	2.072	45.0	14	0.614
Lumped	$\hat{q}_{10(A)} = 0.54572 A^{0.58863}$	-	-	96	0.854
	$DY_1 = 0.98095 T^{-0.02463}$	-	-	96	-0.059
	$DY_2 = 0.24383 P_{60}^{1.99847}$	-	-	96	0.534
	$\hat{q}_{10(K)} = 1.00959 \hat{q}_{10(ATP)}^{1.09129}$	1.564	69.4	96	0.902

Notes explaining the column headings:

- (1)  $\bar{q}_{10}$  is the mean 10 year peak flow calculated from the observed 10 year peak flows for each zone.
- (2)  $PS_{EE}$  is the standard error of estimate expressed as a percent of the zone  $\bar{q}_{10}$ . It is calculated by the equation:

$$PS_{EE} = \frac{100}{\bar{q}_{10}} \sqrt{\frac{\sum (q_{10} - \hat{q}_{10(K)})^2}{n - 2}}$$

- (3) n is the number of watersheds used in deriving the data.
- (4) r is the correlation coefficient between two variables x and y. It is calculated by:

$$r = \frac{\sum (x - \bar{x})(y - \bar{y})}{\sqrt{\sum (x - \bar{x})^2 \sum (y - \bar{y})^2}}$$

where x and y are any two independent and dependent variables, respectively.

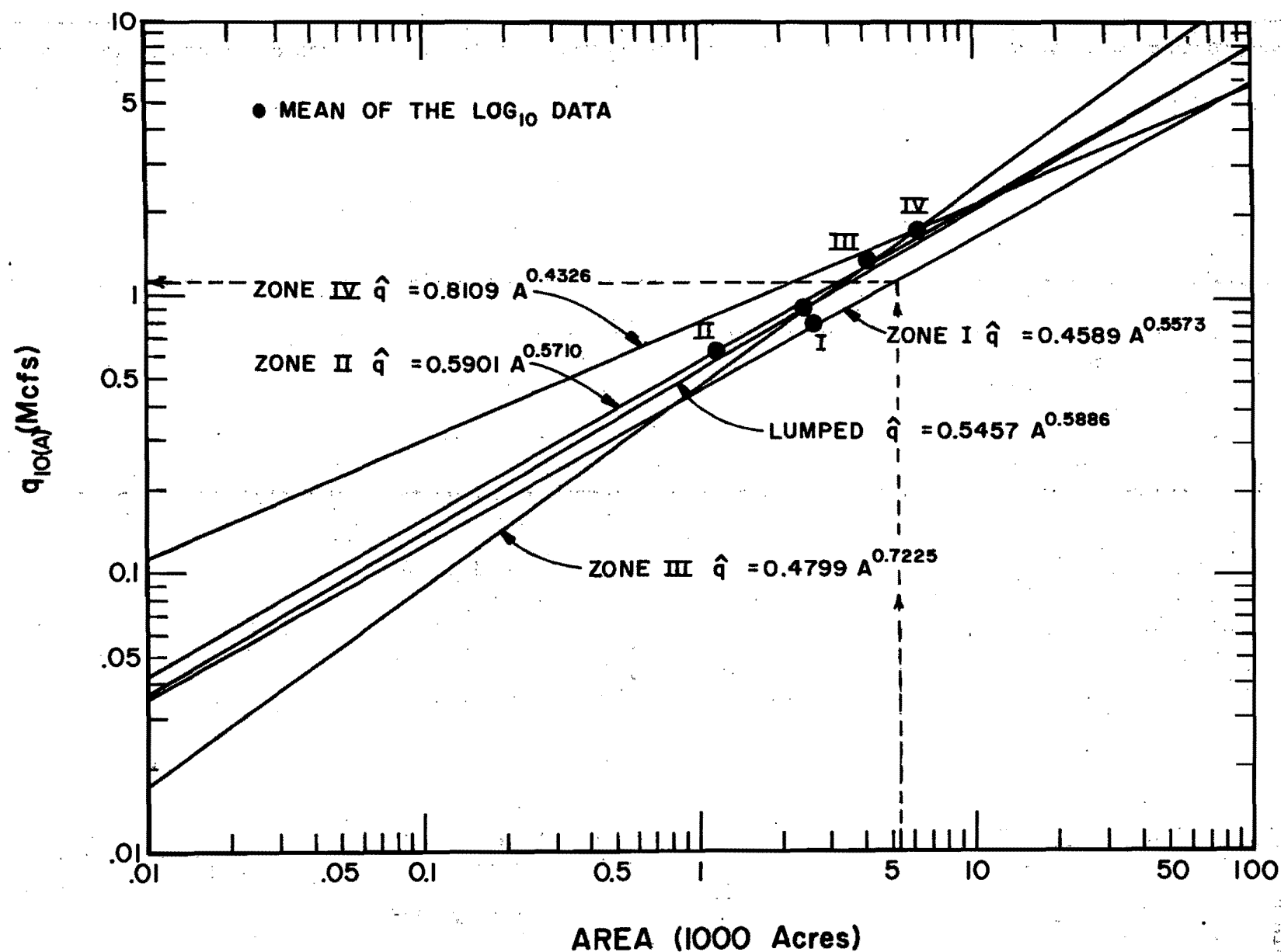


Figure 15. The relationship of the 10-year peak flow to area for Potter watersheds using USU data.



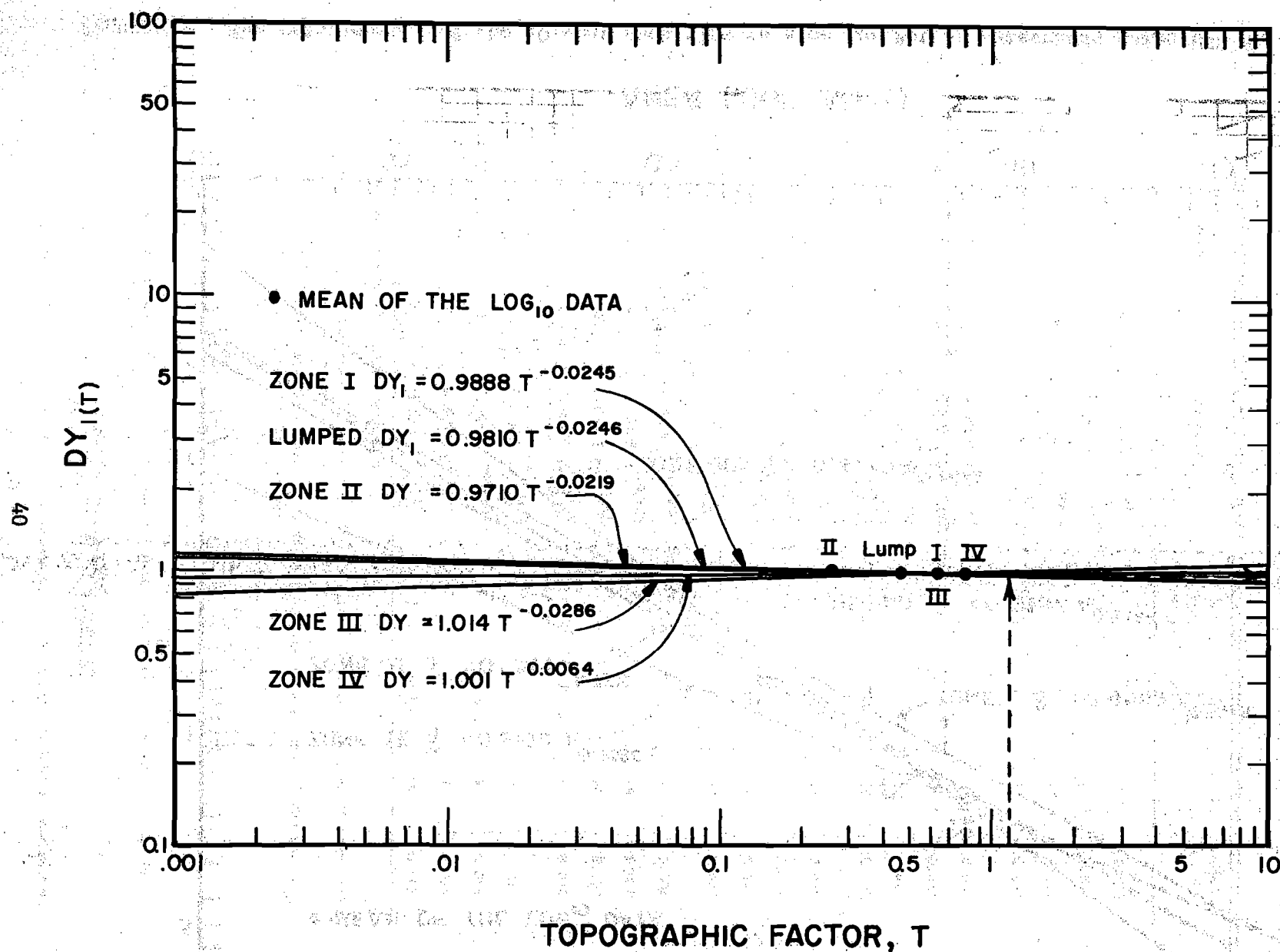


Figure 16. The relationship of the  $y$  deviations,  $\Delta y_1$ , from each line from Figure 9 to its topographic factor,  $T$ .

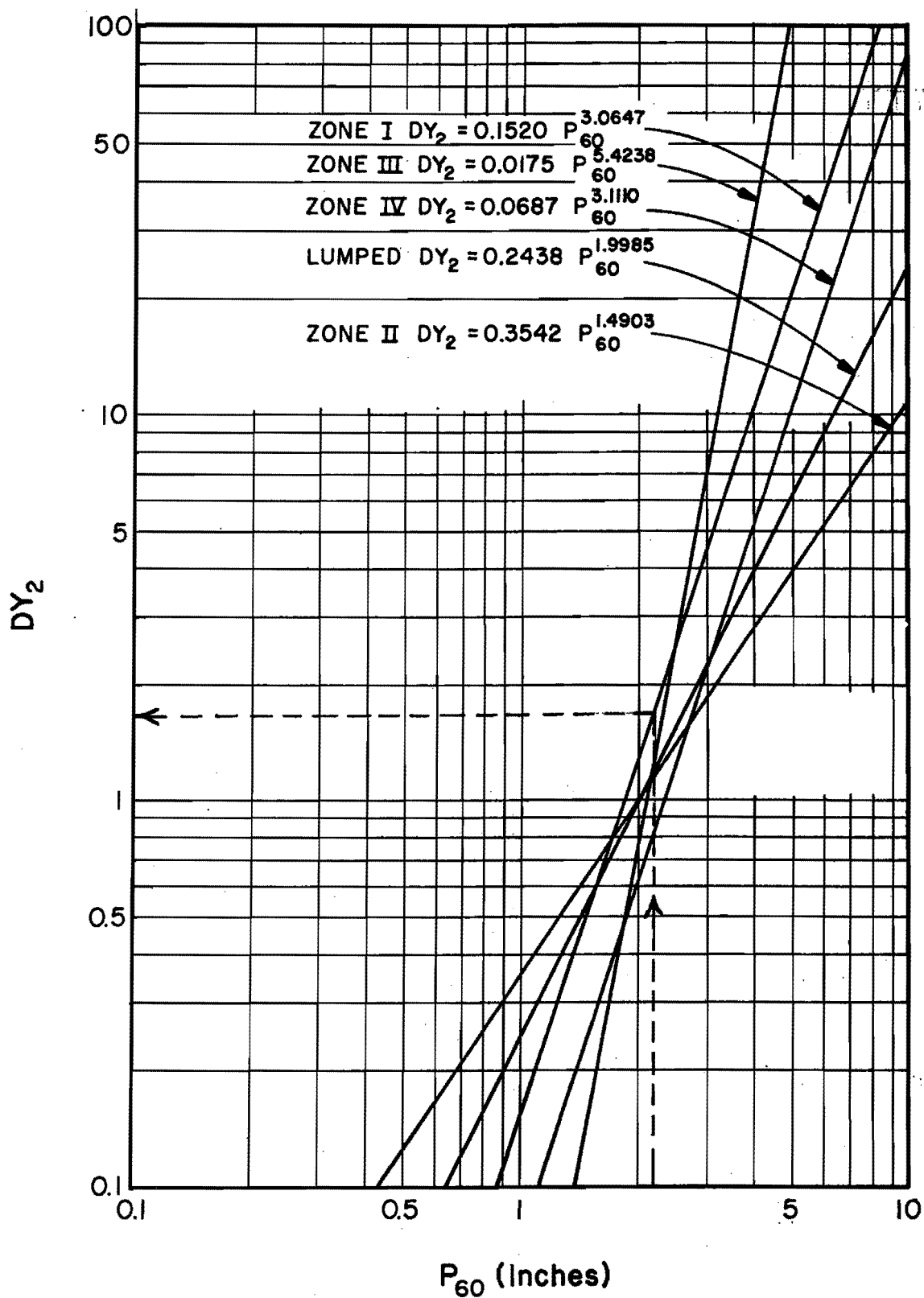


Figure 17. The relationship of the y deviations,  $\Delta y_2$ , from Figure 10 to its  $P_{60}$  value.

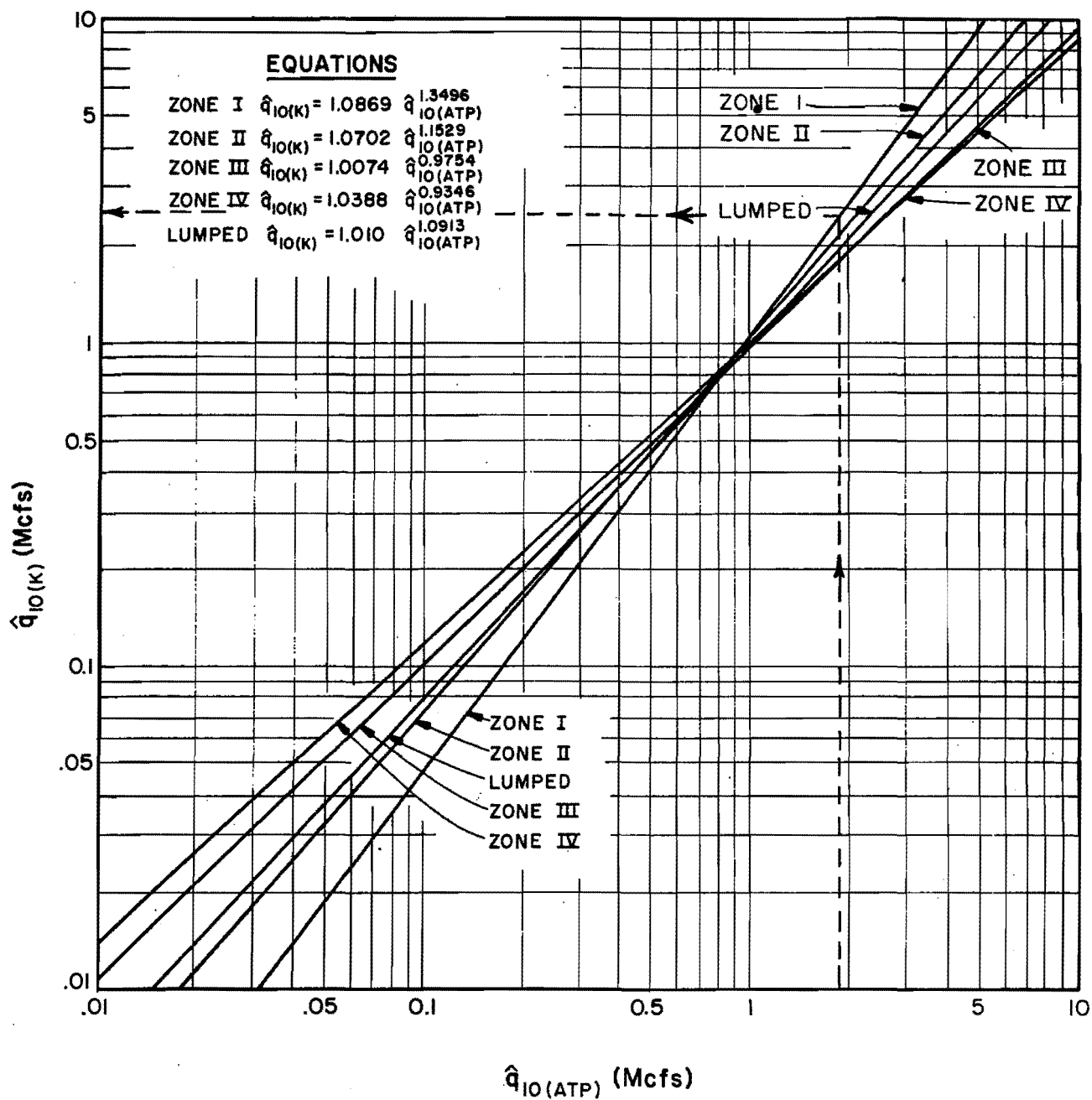


Figure 18. Correction curves, K, for Potter States to be used with Figures 15, 16, and 17.

Table 9. A selected sample of watersheds from Potter's states used to test the areal extension of Potter's method to other watersheds in the original states.

State	Watershed No.	Potter Zone	$\hat{q}_{10}$ Mcfs	A 1000 Acres	T	P 60	Lumped		Zone Eq.		Potter Curves	
							$\hat{q}_{10(K)}$ Inches	1st Error Percent	$\hat{q}_{10(K)}$ Mcfs	2nd Error Percent	$\hat{q}_{10(ATPC)}$ Mcfs	3rd Error Percent
Delaware	2	IV	0.387	2.464	1.159	2.42	1.397	72.3	1.317	70.6	0.126	-207.3
Illinois	12	I	0.228	9.792	2.389	1.96	2.008	88.6	2.572	91.1	1.230	81.5
Illinois	13	I	1.860	3.469	0.576	2.08	1.219	-52.5	1.579	-17.8	1.60	-16.3
Iowa	23	I	1.779	5.197	1.148	2.21	1.771	-0.4	2.689	33.8	1.800	1.2
Iowa	30	I	1.620	6.784	1.436	2.49	2.710	40.2	5.341	69.6	4.100	60.5
Kentucky	35	III	8.944	14.336	1.387	2.19	3.316	-169.7	4.023	-122.3	6.760	-32.3
Kentucky	40	IV	1.128	1.101	0.368	2.23	0.687	-64.1	0.745	-51.4	0.169	-567.6
Maryland	45	IV	1.585	6.72	1.795	2.50	2.701	41.3	2.178	27.2	0.702	-125.8
Maryland	54	IV	1.222	11.776	0.891	2.28	3.229	62.0	2.081	41.3	1.960	37.7
Missouri	60	I	0.934	0.832	0.301	2.52	0.754	-23.9	1.219	23.4	0.830	-12.5
Missouri	68	III	0.847	0.398	0.134	2.39	0.427	-98.3	0.479	-77.0	0.580	-46.0
Nebraska	74	I	4.529	14.72	2.421	2.45	4.243	-6.7	8.792	48.5	7.600	40.4
Nebraska	90	I	1.090	4.346	1.230	2.43	1.939	43.8	3.472	68.6	1.700	35.9
Nebraska	98	I	3.429	13.888	2.193	2.32	3.639	5.8	6.739	49.1	6.000	42.9
New Jersey	101	II	1.909	13.824	2.128	2.06	2.802	31.9	3.255	41.4	5.440	64.9
New York	108	IV	0.068	4.160	0.674	2.18	1.512	95.5	1.198	94.3	0.300	77.3
N. Carolina	118	II	0.120	0.160	0.378	2.58	0.273	56.2	0.266	54.9	0.268	55.3
N. Carolina	129	IV	0.525	2.464	1.300	2.82	1.86	71.8	2.056	74.5	0.050	-950.1
Oklahoma	134	II	0.940	14.720	2.27	2.59	4.799	80.4	5.020	81.3	12.500	92.5
Pennsylvania	141	I	0.934	6.528	0.852	1.74	1.227	23.9	1.199	22.1	1.200	22.5
Texas	147	IV	0.446	0.947	0.251	3.12	1.311	66.0	1.856	76.0	0.600	25.7
Virginia	151	IV	0.884	4.890	3.010	2.60	2.366	62.6	2.153	58.9	0.029	-2948.6
Virginia	169	III	0.325	5.331	0.432	2.16	1.759	81.5	1.803	82.0	1.836	82.3
Virginia	163	IV	0.262	5.472	0.246	2.72	3.000	91.3	2.531	89.7	2.640	90.1
Wisconsin	173	I	0.422	10.688	2.146	1.98	2.178	80.6	2.875	85.3	2.280	81.5
$\hat{q}_{10}$ mean 1.4567							Range absolute error		0.4 to 169.7%		17.8 to 122.3%	
							Mean absolute error		60.5%		62.1%	
							Standard deviation		1.856 Mcfs		2.291 Mcfs	
							Standard error of estimate percent of measured		127.4%		157.3%	
											1.2 to 2948.6%	
											232.0%	
											2.870 Mcfs	

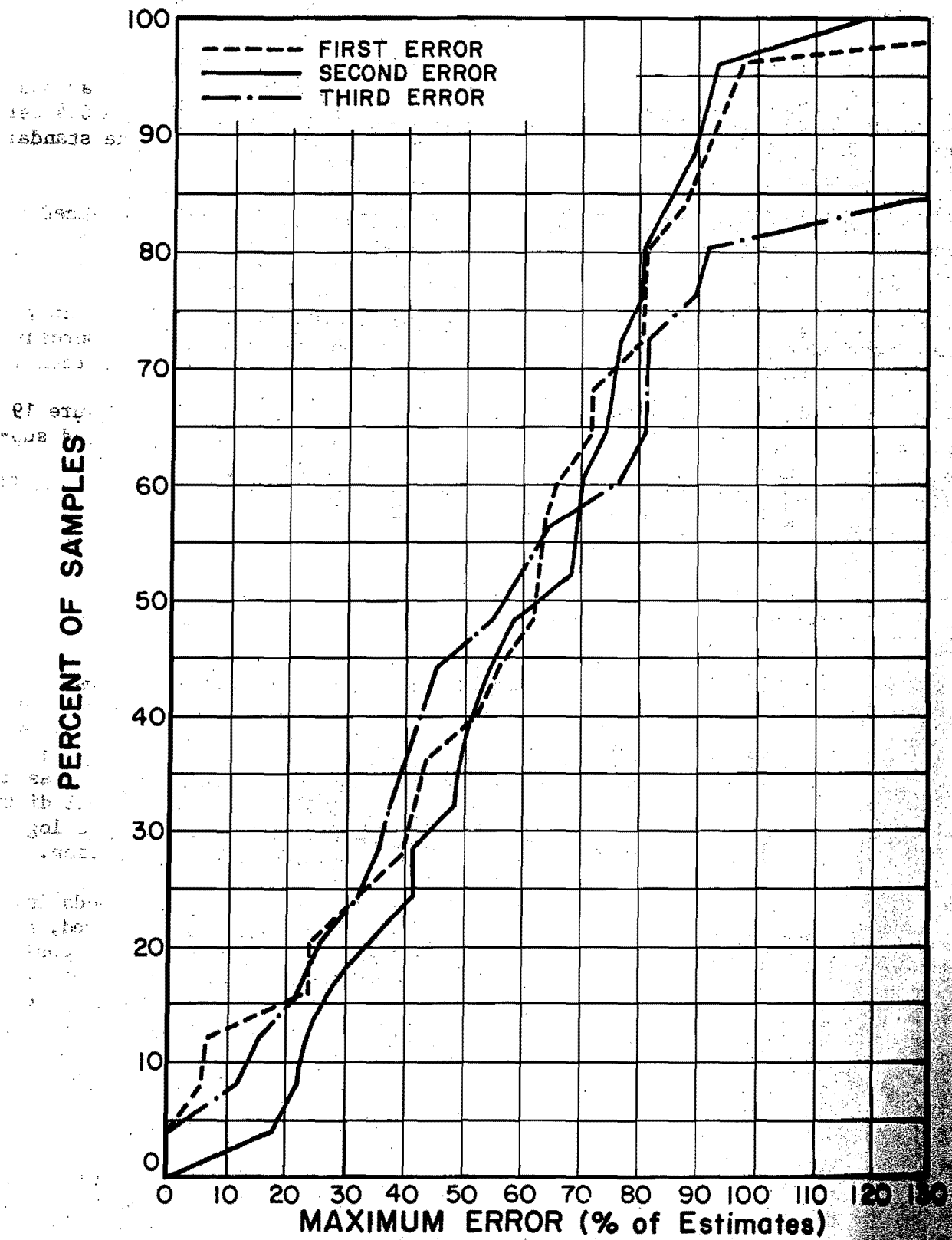


Figure 19. Mass error curves for 25 sample watersheds located in the Pacific States.

The first error estimates were made using the lumped curves or equations. The range in absolute error as a percent of the estimated  $q_{10}$  was from 0.4 percent to 169.7 percent with a mean absolute error of 60.5 percent. The standard error of estimate was 127.4 percent of the mean  $q_{10}$ .

The second error estimates utilized the zone equations. They produced a mean absolute error of 62.1 percent with a range from 17.8 to 122.3 percent and a percent standard error of estimate of 157.3.

The third error estimates utilized the published Potter curves directly. They produced a mean absolute error of 232.0 percent ranging from 1.2 percent to 2948.6 percent. The percent standard error of estimate was 197.0 percent.

The mass error curves for these three comparisons are shown in Figure 19 (p. 44). The general conclusion evident from this phase of the study and supported by the testing on the 25 independently selected watersheds is that a Potter type method is reliable and that the error distribution was not significantly improved by zoning.

## PHASE II

### Refinement of Potter's Method and Other Improved Methods

#### Flood frequency studies

Comparison of methods. The common comparisons between frequency methods generally compare the goodness of fit of a particular set of data to a curve of a known distribution. This criterion was used by Bock, Enger, Malhotra and Chisholm (ref. 3) on 459 watersheds in the United States with Chi-square as a test of the goodness of fit. Their trials indicated that the log normal distribution was superior to Gumbel or log-Pearson Type III distributions and log normal and Gumbel were both superior to log-Pearson Type III distribution.

In the first portion of the present study, data from 167 watersheds in the Great Basin and Colorado River Basin were studied. The data were ranked, normalized and plotted on extremal probability paper against the plotting position,  $(n + 1)/m$ , where  $n$  is the number of years in the array and  $m$  is the position in the array. Selected return period values were computed and tabulated for each watershed and each distribution. The distributions and fitting equations used were the cubic polynomial, the log normal, log-Pearson III, Gumbel, log-Gumbel, gamma, the normal, Pearson III, and log-gamma.

Within the range of the data (11 to 100 years) the cubic gave the best fit. All others except log-gamma fit sufficiently well to be satisfactory. Since the cubic polynomial scored best by every goodness of fit test<sup>1</sup> over the range of the data, it demonstrates the inability of the goodness of fit tests to indicate

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<sup>1</sup>These tests were Chi-square, probability of Chi-square, Smirnov-Kolmogorov, binomial, Cramer-VonMises, and Anderson-Darling.

the extrapolation reliability of any distribution. Because of this, the data were all plotted along with the three most commonly used distributions for flood frequency analysis; the double exponential or Gumbel, the log normal, and the log Pearson Type III distributions to serve as a guide to extrapolation. The flood frequency plots are included as Appendix A to this report.

Ten-year flood peaks from these computations were compared on the Potter watersheds to the Potter values of  $q_{10}$  and Potter's (ref. 12) graphical upper and lower frequency method by USU. The differences were not statistically different by the  $t$  test. The absolute values of the peaks were somewhat higher with the Gumbel distribution though not significantly so.

**Extrapolation.** In the previous section we dealt with the problem of fitting a set of data to a distribution or an equation. In this section we will consider record lengths and extrapolation to return periods beyond the range of the data.

A search was made for very long records which would most likely be a sample of the variability of the local climate. Four such records were located, i.e., the Lake Erie outlet, Logan River, Santa Fe, New Mexico, precipitation and Mississippi River at Cairo, Illinois. The same kinds of results were obtained for the four records so the illustrations presented here will be drawn from the Santa Fe precipitation record. Figure 20 (p. 47) shows curves constructed by dividing the hundred plus years of record into consecutive periods of 5 year, 10 year, 20 year, 50 year, and 100 year records. The highest and lowest 5-year value from the 20, 5-year records are the two points shown at 5 years in Figure 20 (p. 47) as ratios to the value read from the 100 year record. The two points at 10 years were derived in similar fashion from the ten 10-year records and so on until the upper and lower curves are complete.

The third curve is the mean value of all the members of each group derived from the same groups of records from which the extremes were drawn.

Figure 21 (p. 48) has similar curves to those in Figure 20 (p. 47) except the 50-year event is forecast from the different record lengths. Figure 22 (p. 49) is constructed similarly to Figures 20 (p. 47) and 21 (p. 48) except the 100-year event is being predicted from each record length.

Figures 23 through 28 (p. 50-55) show the same error distributions with the log-Gumbel and log-normal distributions. The Gumbel distribution appears to be somewhat superior to either of the log methods for general extrapolation, but appreciable errors may be present when records are shorter than about 25 years on any of the distributions. This observation contrasts with the conclusions found by the fitting of curves within the range of the data where only 11 years of record produced satisfactory data fits to a distribution. This finding is similar to that of Benson (ref. 2) for a synthetic 1000 years of record.

One item observed is that one consecutive record period of even up to 50 years in length may have a widely divergent population from the next 50-year record indicating long term persistence of climatic highs and lows rather than

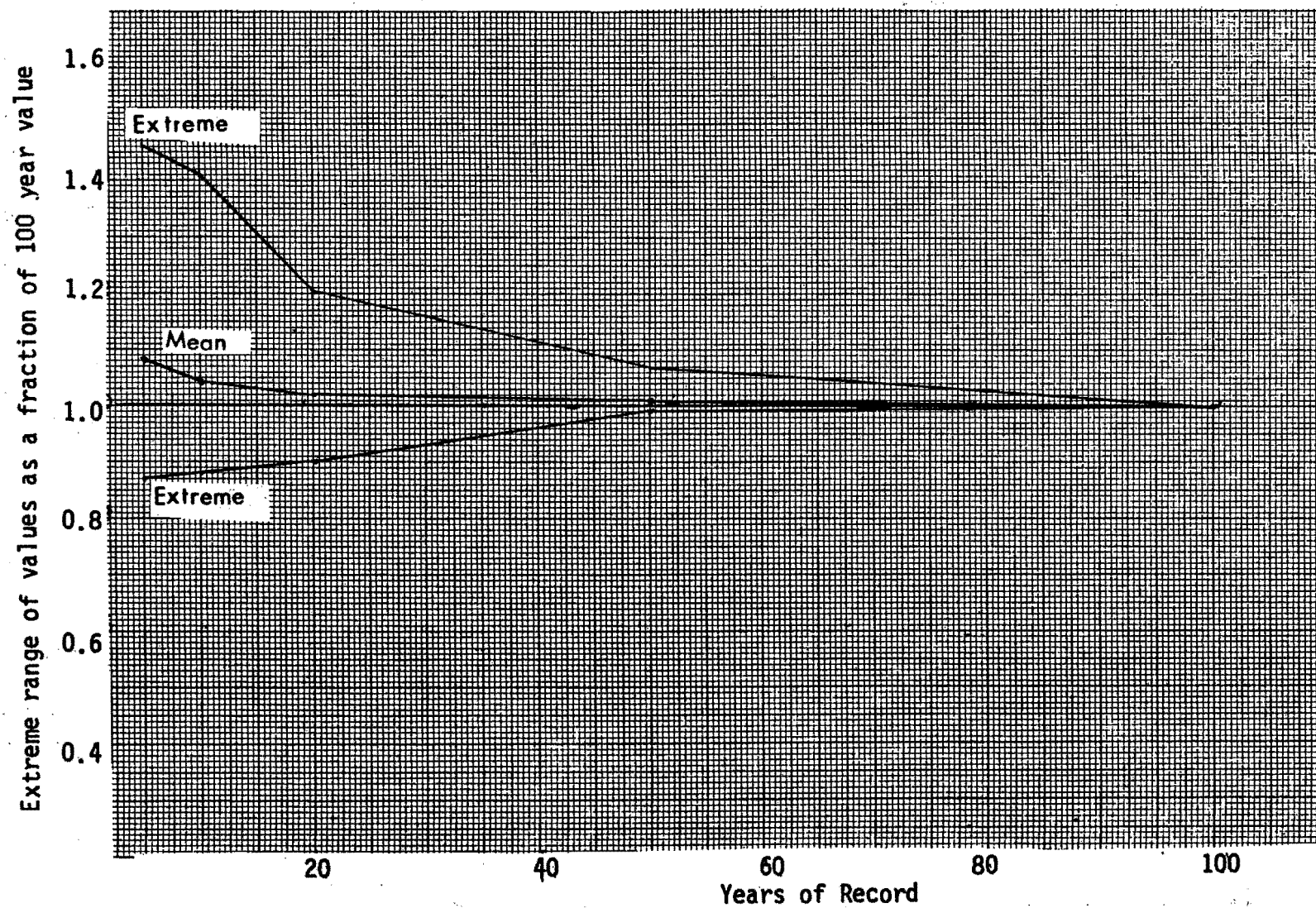


Figure 20. Maximum error distribution in the 10-year storm prediction. Gumbel distribution Santa Fe, New Mexico precipitation record.



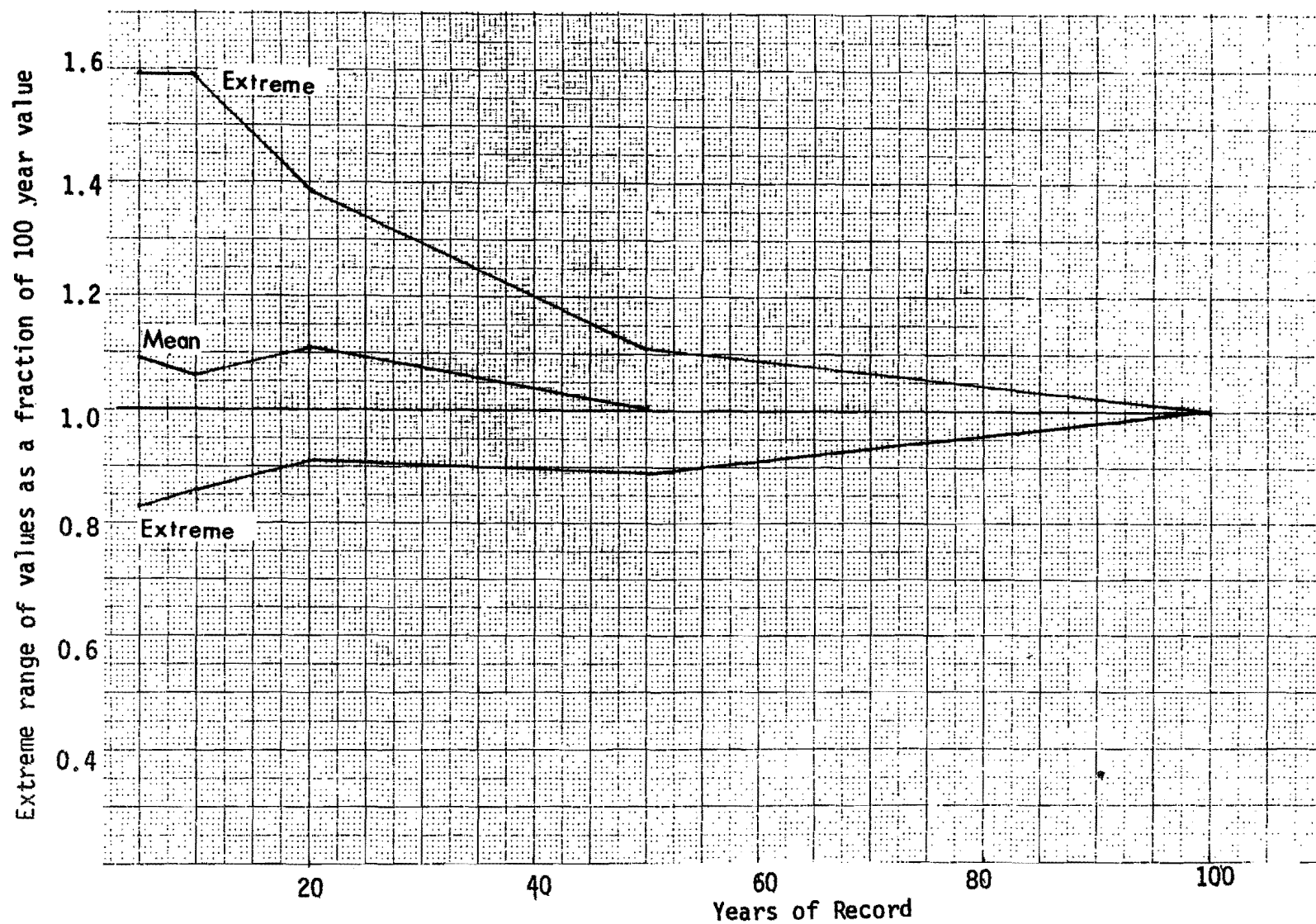


Figure 21. Maximum error distribution in the 50-year storm prediction. Gumbel distribution and Santa Fe, New Mexico precipitation data.

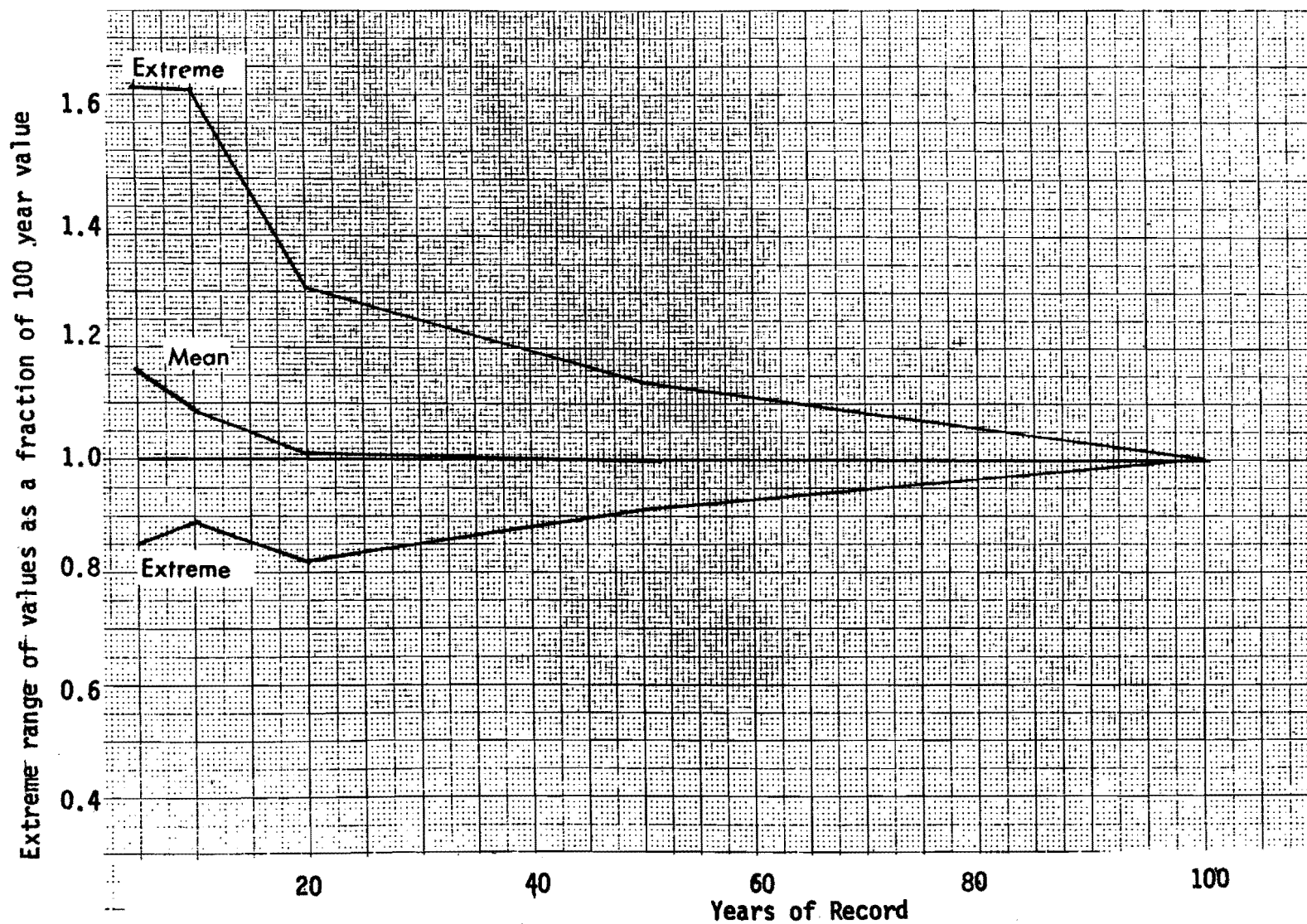


Figure 22. Maximum error distributions in the 100-year storm prediction. Gumbel distribution Santa Fe, New Mexico precipitation data.

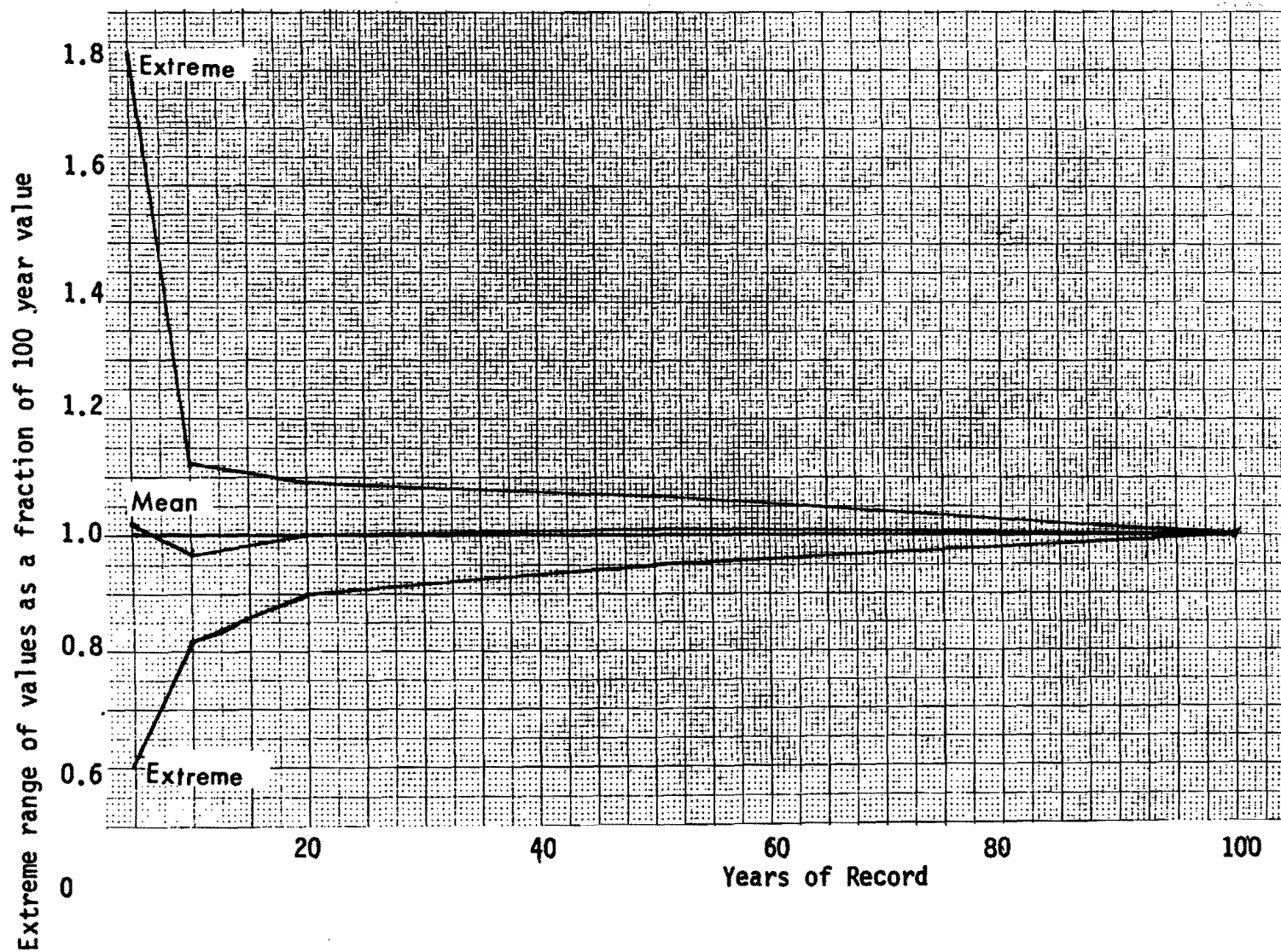


Figure 23. Maximum error distributions in the 10-year storm predictions. Log Gumbel distribution. Santa Fe, New Mexico precipitation.

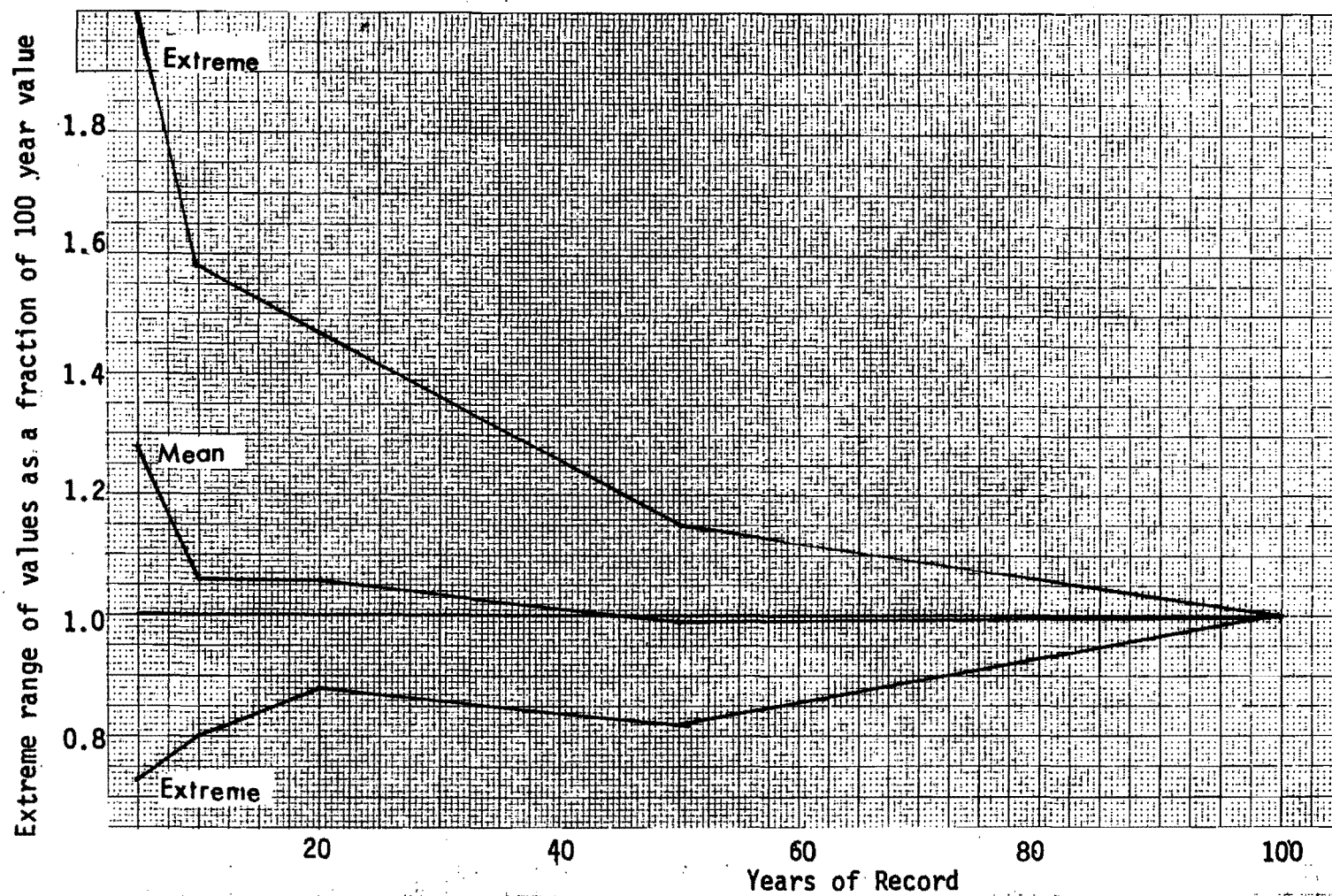


Figure 24. Maximum error distributions in the 50-year storm prediction. Log Gumbel distribution, Santa Fe, New Mexico precipitation.

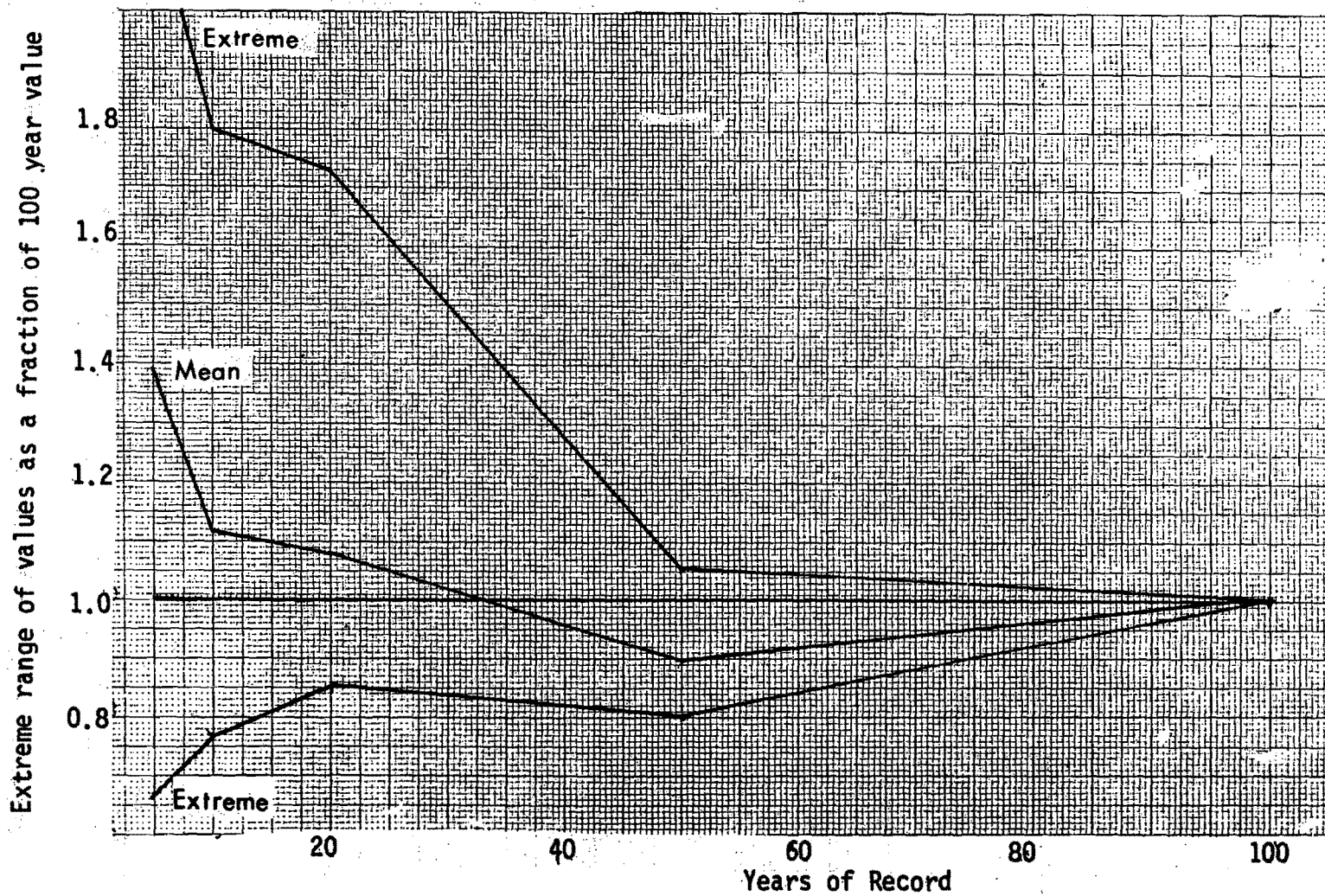


Figure 25. Maximum error distributions in the 100-year storm prediction. Log Gumbel distribution, Santa Fe, New Mexico precipitation.



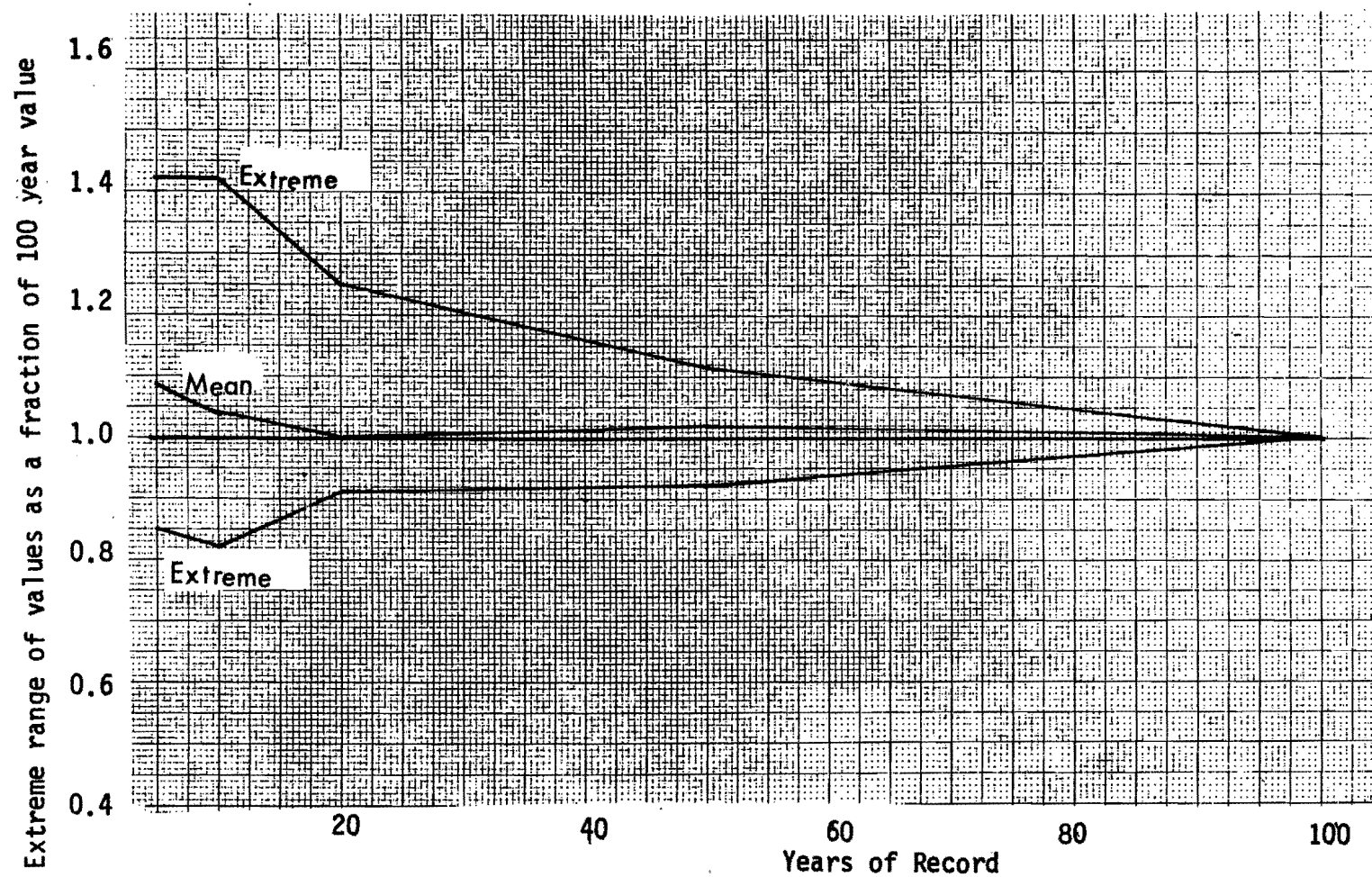


Figure 26. Maximum error distribution in the 10-year storm prediction. Log Normal distribution, Santa Fe, New Mexico precipitation.

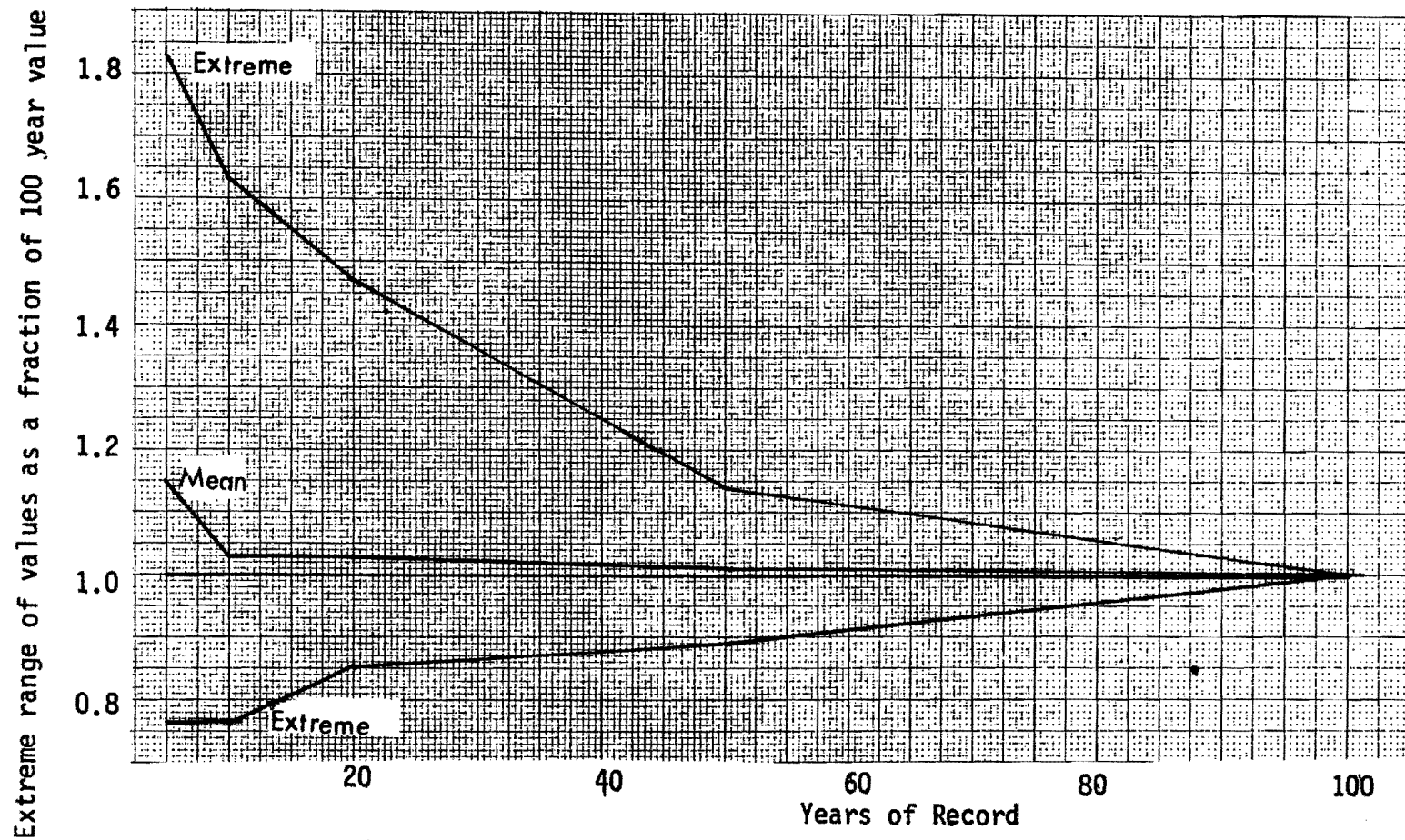


Figure 27. Maximum error distribution in the 50-year storm prediction. Log Normal distribution Santa Fe, New Mexico distribution.

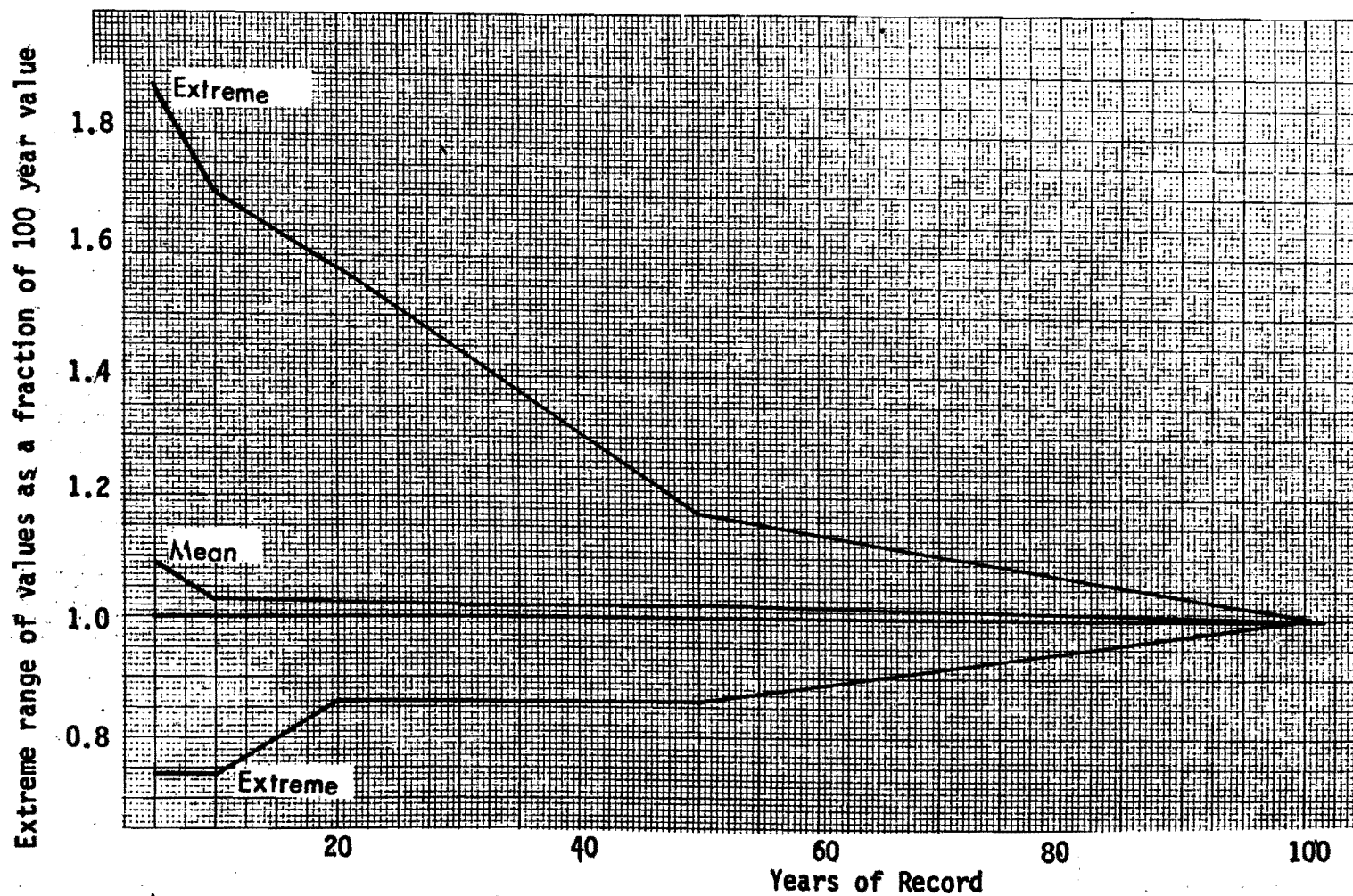


Figure 28. Maximum error distribution in the 100-year storm prediction. Log Normal distribution, Santa Fe, New Mexico precipitation.



short term variability only. Even with 50 years of record, the 50-year annual peak from one period of time might be as much as 200 percent different at the extreme than the next 50-year period. In a practical record, one is never sure where his estimate may lie in the possible range of climatic changes for the site.

Determination of other frequencies from the  $q_{10}$  value. Potter determined the 50-year flood peak,  $q_{50}$ , from a curve made by plotting the log of the  $q_{10}$  against the log of the  $q_{50}$ . The  $\hat{q}_{10}$  value is determined from the estimating procedure. This  $\hat{q}_{10}$  is then entered on the vertical scale and moved horizontally to the curve where the value of the  $\hat{q}_{50}$  is read from the horizontal scale. See Figure 29 (p. 57).

Potter reasoned that greater precision could be had on the 10-year peak from frequency plotting than either more or less frequent returns. USU's present investigation verifies that the greatest prediction accuracy is obtained for the  $q_{10}$ . The curve Potter gives has slightly lower values of  $\hat{q}_{50}$  than USU obtained from the Potter test watersheds only, but agreed very closely when USU used all of the samples from the Potter States. This suggests that perhaps Potter developed his relationship from a larger sample. Other return intervals are related to the 10-year flood peak on Figure 30 (p. 58).

The USU writers believe that no single distribution can be used for all of the United States. Where computers are available, the data points should be plotted on extremal paper and the multiple functions mentioned plotted, thus allowing the engineer to inspect the result to determine whether they conform to any of the distributions or whether a new relation such as Potter's upper and lower frequency should be drawn.

Examples of the data fit from selected stations in each State are given in Appendix A.

Probable maximum peak runoff. All of the "period of record" instantaneous maximum flood peaks for each station with data available (well over 1000 in the proper size range) were plotted against the area of the watershed from which each came on log-log paper. The upper boundary of these points defines a curve known as the probable maximum peak runoff curve. The curve derived for the 50 States and Puerto Rico is shown in Figure 31 (p. 59). For very small drainages where the culvert cost is not prohibitive, this curve might be an acceptable guide to sizing. Normally hydrologists consider this flow peak to be the one used where human lives are involved. This value would almost certainly be adequate for setting maximum watershed sizes for which no other peak flow estimates need be made.

Conclusions. It appears reasonable from this investigation and that reported by Schmidt (ref. 14) as discussed previously to conclude that within the range of the data, there is little choice between the different distributions or methods except the log-gamma distribution.

When extrapolation becomes necessary, all of the methods tested--Gumbel, log-Gumbel, log normal, log-Pearson III, gamma, Pearson III, and normal--gave

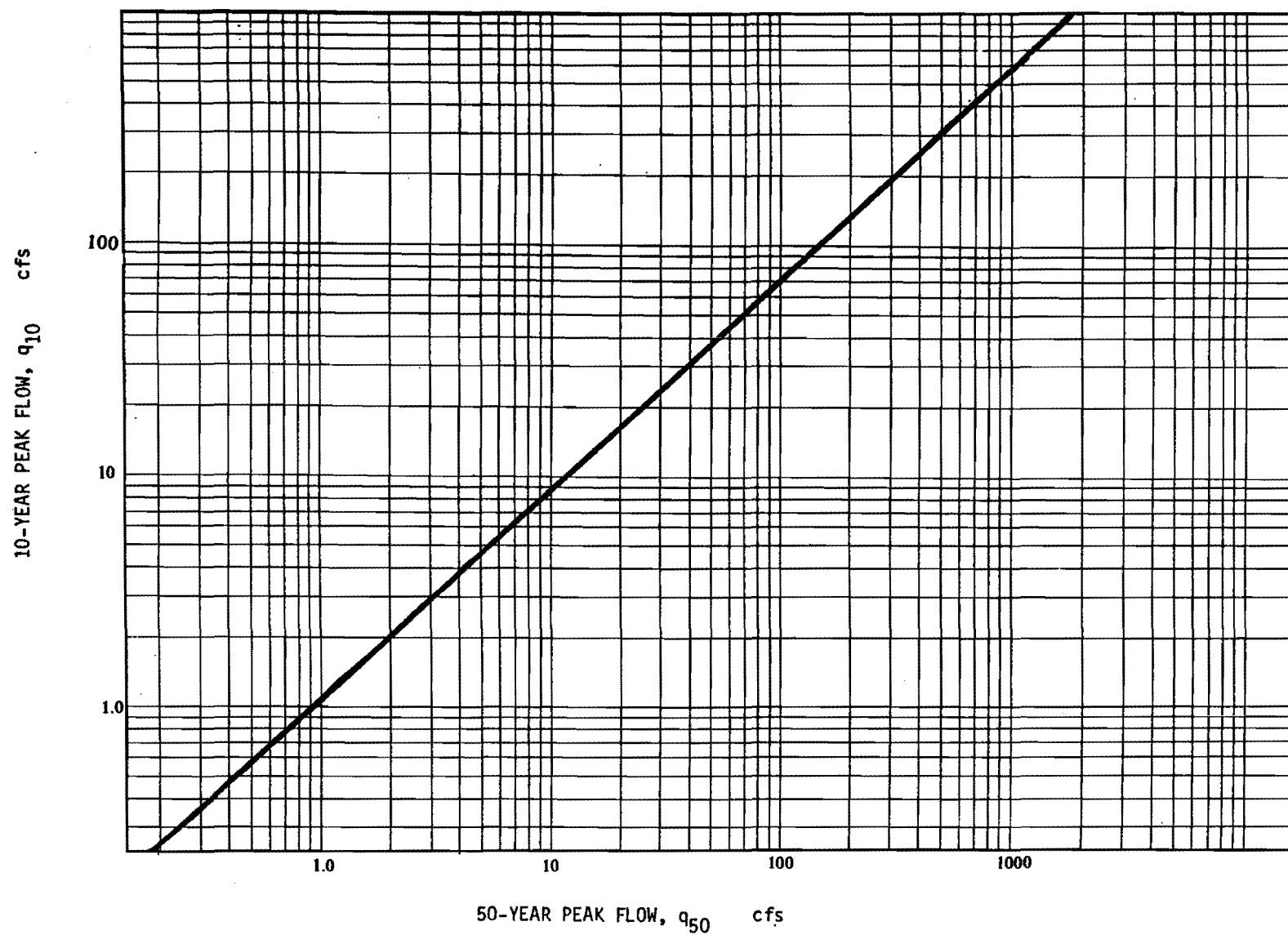


Figure 29. Relationship of  $q_{10}$  to  $q_{50}$  in the Potter States.

closer results than the variability of the climate with time. It would appear to be desirable to establish zonal norms through indirect methods such as tree rings, varves, or some other technique, in order to determine whether or not a time period when measurements were available was characteristic of the climatic range for the area.

These USU writers feel strongly that where records must be extrapolated appreciably beyond the range of the data, no single distribution should be adopted, but all points should be plotted on extremal paper and a visual determination made where the extrapolation line should go. The upper and lower frequency procedure of Potter can be a good guide at this point.

### Extend the Potter Type Procedures

#### Selection and location of watershed samples

Watersheds were selected so there were at least seven watersheds in each physiographic section of the United States as defined by Fenneman and Johnson (ref. 7). Whenever more than the minimum numbers of samples were located in the same section the watersheds with the longest records were retained. The first samples inspected in each section were those compiled by Bock, Enger, Malhotra and Chisholm (ref. 3).

Figure 32 (p. 61) shows the location of all the sample watersheds within the contiguous United States. Figure 33 (p. 62) shows the location of the watersheds in Alaska, Figure 34 (p. 63) shows the location of those watersheds in the Hawaiian Islands, and Figure 35 (p. 64) shows the location of watersheds in Puerto Rico.

### Parameters Investigated

Potter limited the parameters used in the method he recommended to a zoning or geologic parameter, a precipitation factor, area, a topographic factor, and a correction factor "C." In the USU extension of the method to the balance of the United States, it seemed desirable to include parameters which would tend to make the method simpler and more versatile. No attempt will be made to make a literature review on the subject but instead the reader is referred to the reviews of Bock, Enger, Malhotra, and Chisholm (ref. 3), Chow (ref. 4), Hydrocomp, Inc. (ref. 9), and Ben Chie Yen (ref. 22) for references on the subject. Potter considered the parameters: geophysical zone, area, watershed slope or topographic factor, precipitation factor, drainage density and "C" factor. Several of these parameters require manipulation to obtain a usable value after the direct measurement. Each of the basic elements is measured in the field or from maps, charts, etc. Each parameter to be tested in this section is considered in the following subheadings.

#### Area

In this section the area used is that given in the USGS publications or tapes except when their area was labeled "approximate." In this instance, the watershed area was planimetered from the largest scale topographic map available

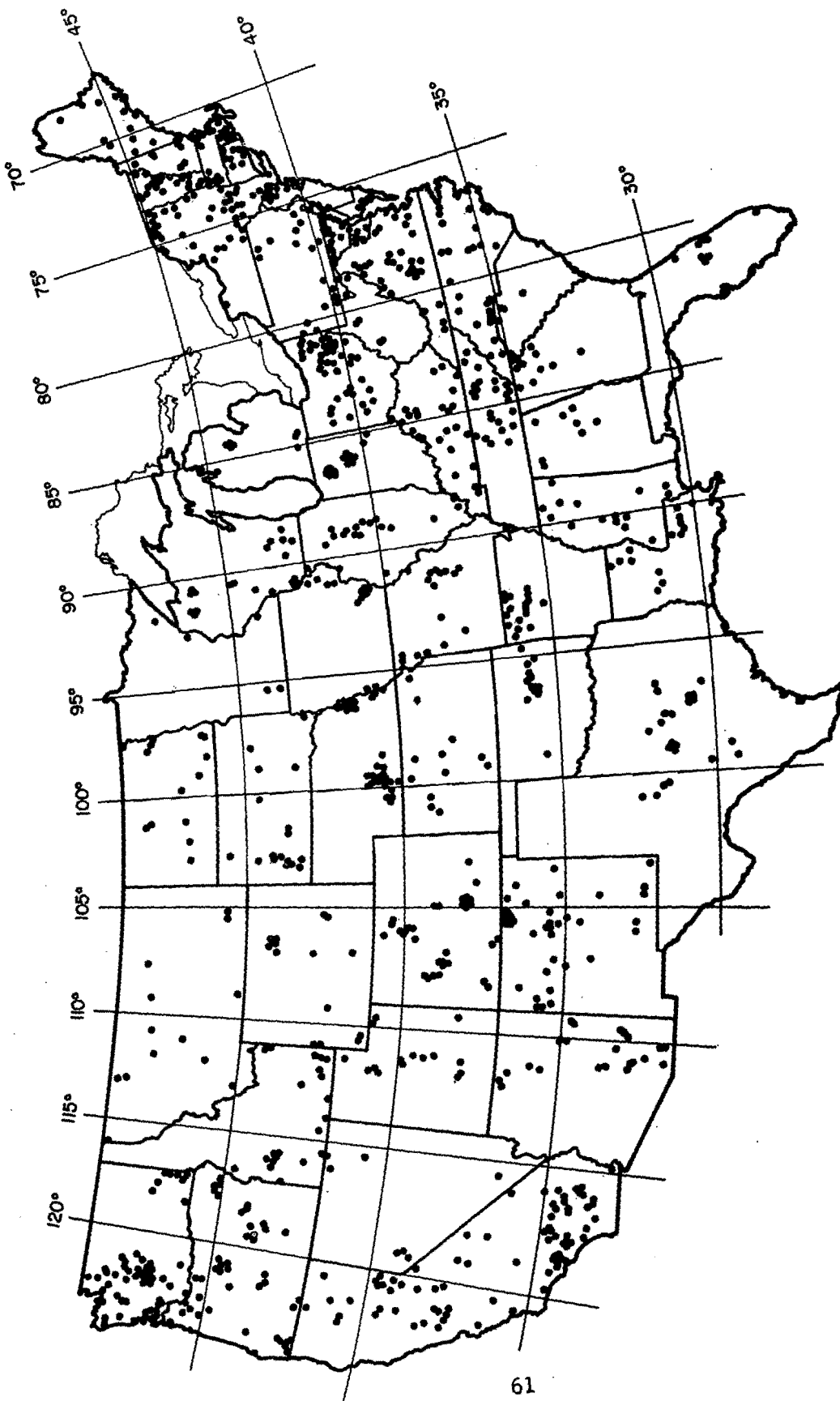


Figure 32. Location of all sample watersheds in the contiguous United States.

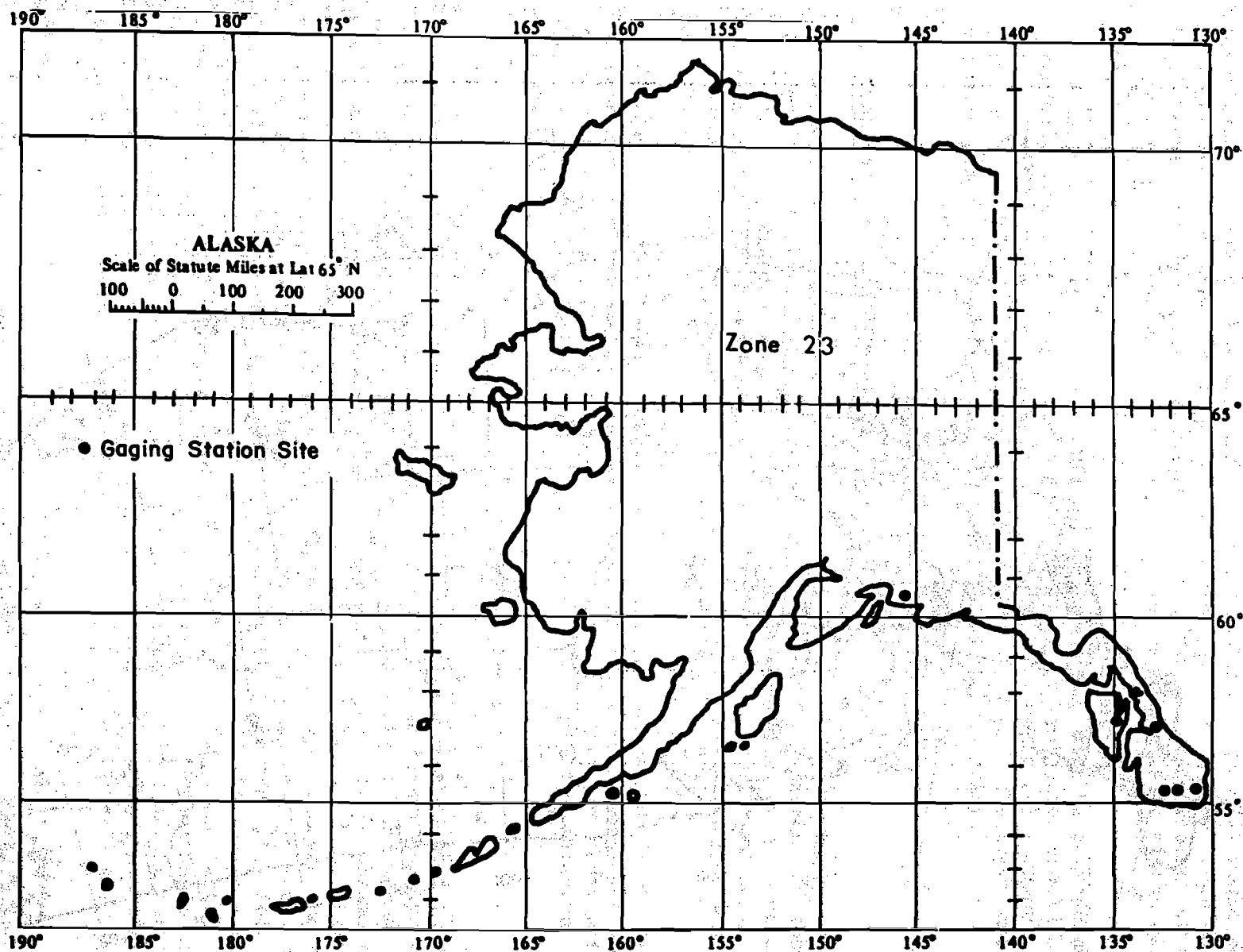


Figure 33. Location of sample watersheds in Alaska.

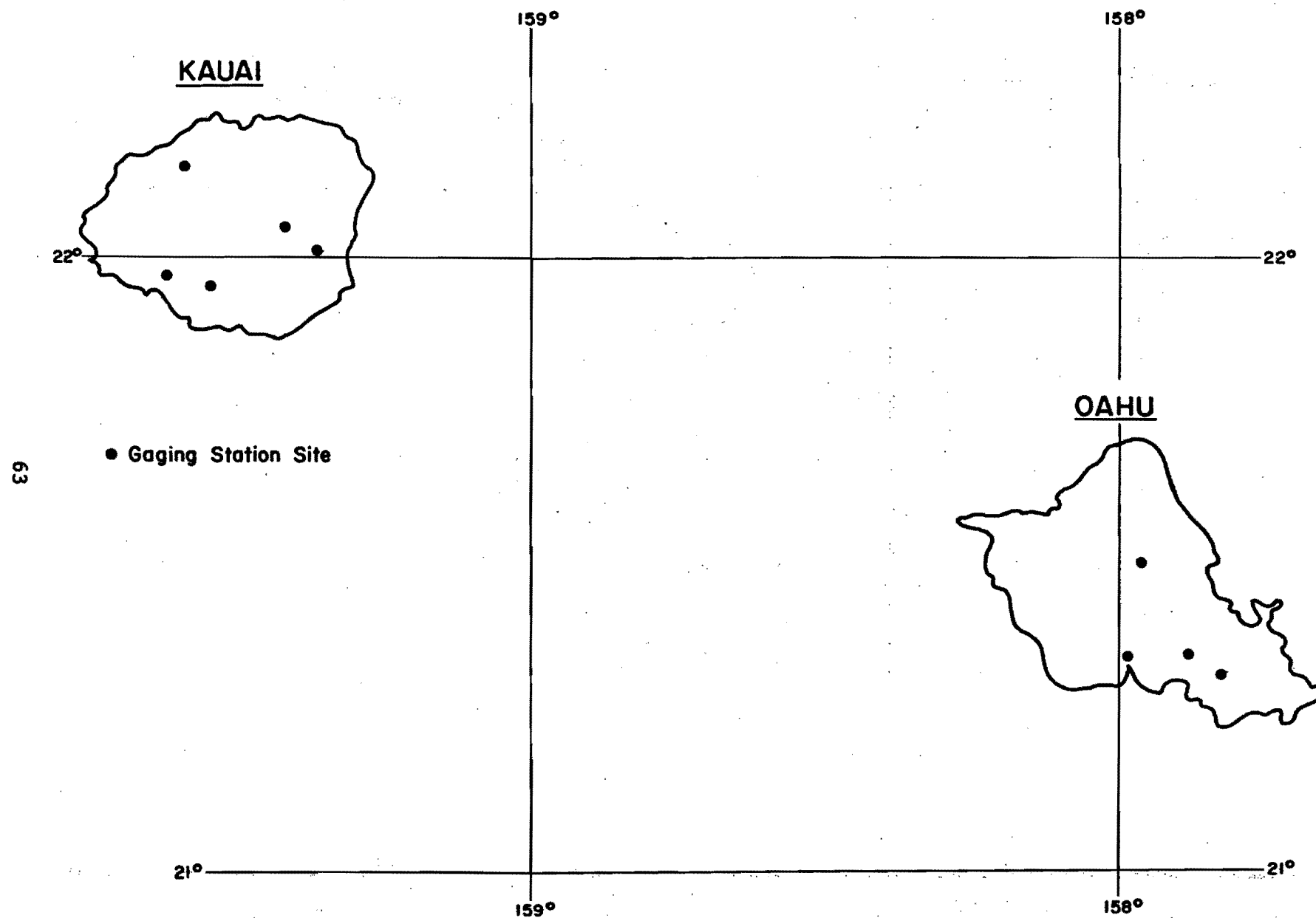


Figure 34. Location of sample watersheds in Hawaii.

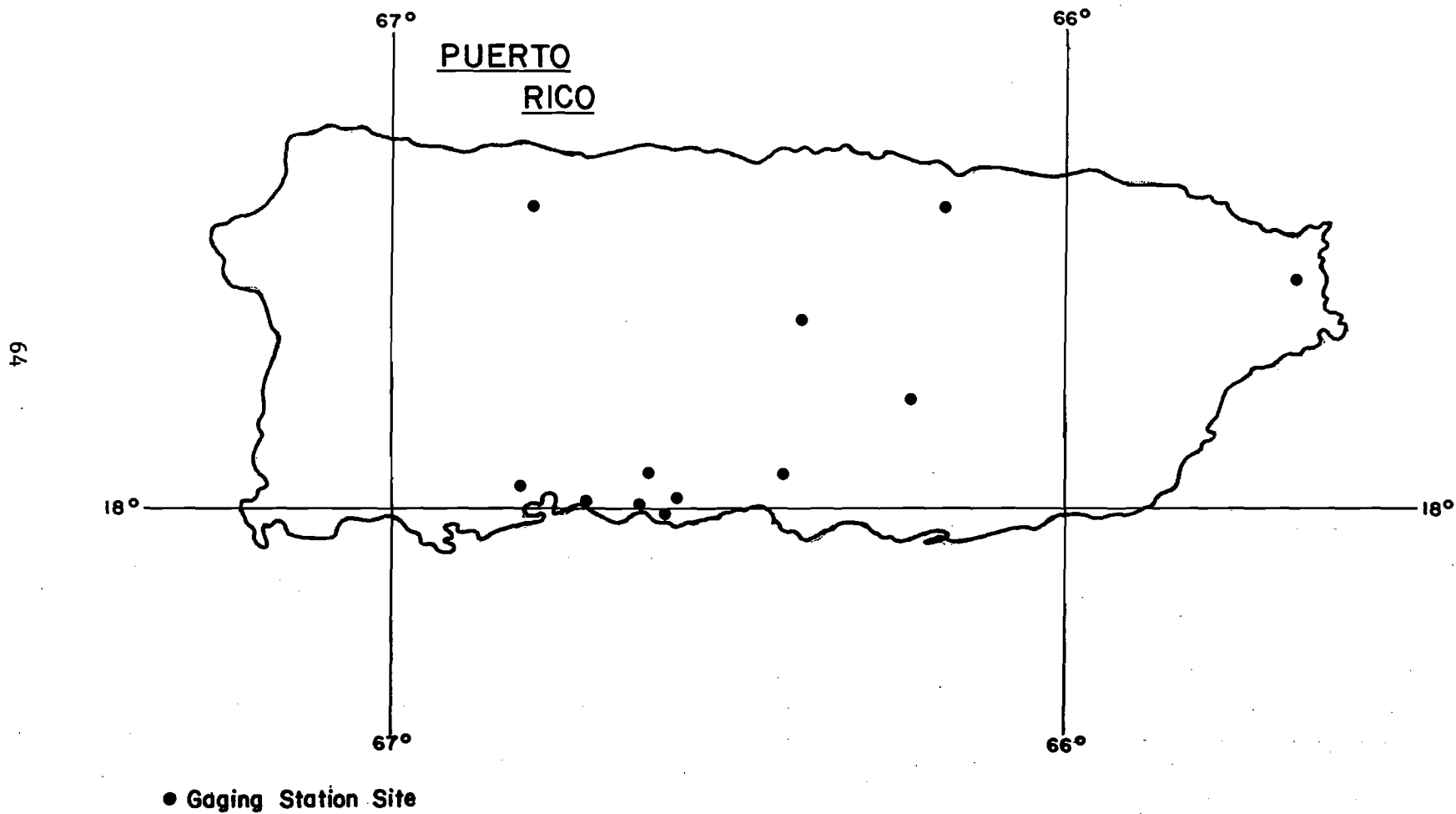


Figure 35. Location of sample watersheds in Puerto Rico.

most commonly this was the 7½ minute quadrangle map. No corrections for map scale were made in the instance of areas. The area is expressed in square miles.

### Snow

In many areas of the United States a large portion of the annual peak runoff events occur as a result of snowmelt. Fletcher and Reynolds (ref. 8) showed that percent normal ( $q_{2.33}$ ) annual runoff peaks were closely associated with the percent normal annual 1 April snow waterequivalent. The 1 April snow water equivalent was thus introduced to supplement the other precipitation factors and to take advantage of the valuable data by the Soil Conservation Service Cooperative Snow Surveys in the western United States. The parameter used herein is the 10-year snow water equivalent in inches of water. The values are read from the appropriate snow water equivalent maps in Appendix F.

### Storage

Potter stated that he carefully avoided surface storage in his selection of the watersheds used to develop his method. It may be seen in Table 1 (p. 5-9), that, out of 96 Potter's watersheds, only two had significant storage. Another 7 watersheds had 1.7% of storage or less. Bar-Kochba and Simon (ref. 1) indicated that small watersheds are much more susceptible to the effects of storage than large watersheds, but that storages smaller than 25 acre-feet per square mile could be considered as negligible for watersheds larger than 70 square miles. If this storage had an average depth of 1 foot, it would be equivalent to less than 4 percent of the total area. In correlations here storages smaller than 1 percent are considered to be negligible. Because so many watersheds have values of storage below this value, storage is handled as a correction after the estimate of  $\hat{q}_{10}$  is made from the other parameters. To make this correction Figure 36 (p.65) is entered with the percent of the area occupied by storage and a multiplier is read off the vertical axis. The  $\hat{q}_{10}$  is multiplied by this multiplier to get the proper 10-year peak flow corrected for storage.

### Slope parameter

During the visits to the States several people complained about the complexity of the Potter T. Consequently, the parameter of channel length up to the crest of the watershed divided by the square root of the main stream slope,  $L/\sqrt{S}$ , was investigated. There was no difference between T and  $L/\sqrt{S}$  as measured by their respective correlations with  $q_{10}$ . In view of this finding USU decided to extend this reasoning one step further and try the difference in elevation between the top and bottom of the watershed,  $\Delta H$ , as a parameter. Since  $\Delta H$  is read directly from a topographic map, it is much simpler than either of the other slope parameters and if anything, the correlations between  $\Delta H$  and  $q_{10}$  proved to be even better than the more complex slope parameters.  $\Delta H$  is in feet.

### Precipitation parameters

During the confirmation of Potter's original method, some of the log  $q_{10}$  values correlated poorly with log P factor,  $P_{60}$ , whereas, log  $q_{10}$  correlated



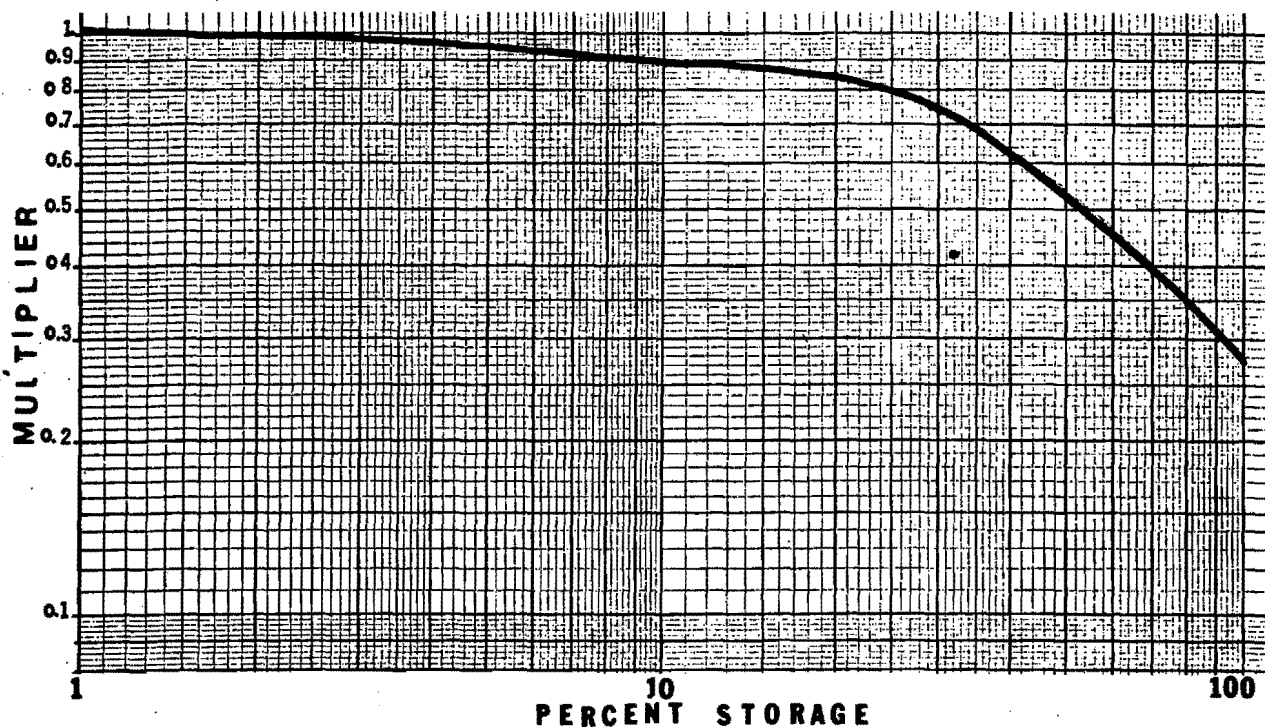


Figure 36. The relationship between percentage of watershed area covered by lakes, ponds, swamps, playas, etc. to the multiplier required to correct a peak runoff estimate for storage.

better with the corresponding arithmetic 10 minute precipitation,  $P_{10}$ , intensity, even though the two parameters are closely correlated with each other ( $r^2 > 0.9$ ) through a log-log transform. The third precipitation parameter, the R value, was included when a national map became available from Transportation Research Board Project 16-3. Each of these three precipitation parameters are read from a map. R is defined as the mean annual rainfall erosivity index, EI, or the total mean annual kinetic energy\* times the annual maximum 30 minute rainfall intensity (see Wischmeier and Smith (ref. 21) and Dragoon (ref. 5)).

#### Drainage density parameters

The so-called drainage density parameter, drainage density, D-factor or DD is a relatively simple parameter. It is derived from the total lengths in miles

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\* $E = \sum_0^{12} (916 + 332 \log I)d$  wherein I = rainfall intensity for a constant period of time and d = depth during the same period. These E values are summed for all storms each year and multiplied by the annual maximum 30 minute rainfall intensity to get the year's EI.

of all drainage channels shown as blue lines within the watershed, LL, divided by the area of the watershed in square miles. The second drainage density parameter is the simple primary unit LL. LL is read directly from a 7½ minute quadrangle sheet, where one is available with a map measure. It is the total length in miles of all blue lines which indicates stream channels within the watershed boundary on the USGS map. If only a 1:250,000 scale map is available the curve in Figure 37 (p. 68) is utilized to correct to the 1 to 24,000 scale value. LL is in miles. DD calculated from a 1:250,000 map may be corrected to the 1:24,000 scale value with Figure 12 (p. 29).

### Shape factors

Potter's length of principal drainage channel  $L'$  is related to shape of the watershed. The method Potter used to determine  $L'$ , however, was map scale dependent. This measurement was therefore modified to extend the length measured past the end of the blue stream channel line to the nearest most remote crest of the watershed. With this modification, it was found that the simple correlations with  $q_{10}$  were actually improved and the  $L$  values were independent of map scale.

The original USU idea was to use one main axis and two minor axes at right angles to this main axis as indices of shape of the watershed. The preliminary correlations, however, indicated that the  $L/A$  parameter was equally well correlated with  $q_{10}$  so the former was discarded in favor of  $L/A$  for simplicity. This ratio was later reduced to just  $L$  since  $A$  was already a parameter. Whenever the different forms of each parameter showed similar correlation values, the simplest form was chosen for further investigation. When 958 watersheds were used in a multiple regression covering all of the states plus Puerto Rico the parameter most pertinent to the 10-year flood peak was the precipitation parameter,  $R$ , followed closely by area,  $\Delta H$  and LL.  $P_{60}$ ,  $L$ , and  $P_{10}$  had somewhat lower weights but were still significant.

The reduction in the percent of explained variation of  $q_{10}$  from these variables with no zonation, is just under 74 percent. Dropping the latter three parameters decreased the reduction by about 1 percent only, and substituting  $L$  for the LL decreased the reduction by about 1 percent additional. The three variables  $A$ ,  $R$ , and  $\Delta H$  give just over 72 percent reduction.

### Zonation

Potter's original zones were deduced from the SCS problem area map (Figure 8, p. 19). The physiographic sections from the map of the physical divisions of the United States prepared by Fenneman and Johnson (ref. 7) were selected for the present zonation which both Potter and USU felt was desirable. The preliminary work which USU did on the Potter watersheds (Phase I) indicated that  $q_{10}/A$  could be used as a parameter to show the effects of zonation. From this and using  $t$  as the statistical test method, Potter's Zone II was significantly different from his other three zones. As the Potter method was extended to other watersheds, the similar physiographic boundaries and the smaller amounts of other data on the map of Fenneman and Johnson made it the most desirable of the two maps to use.

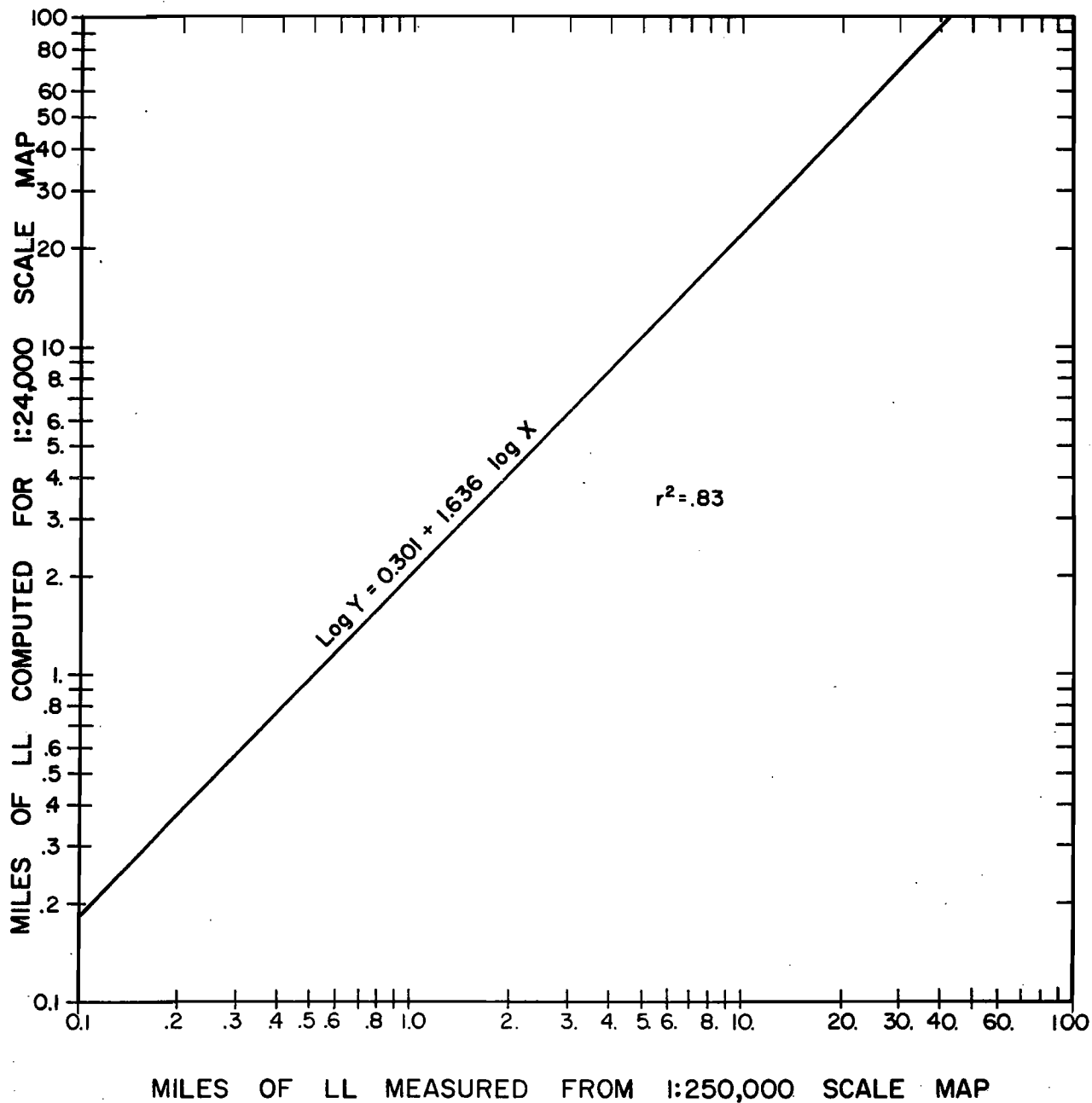


Figure 37. The relation of LL measured on a 1:250,000 scale map to that derived from a 1:24,000 scale map.

When the physiographic sections from the Fenneman and Johnson map were compared to each other on the basis of their  $q_{10}/A$  value utilizing the  $t$  test as the criterion of differences, the physiographic sections were grouped into hydrophysiographic zones in Potter's states. Two or more sections were grouped in the same zone when the  $t$  values between them were not significant and they were in the same general area. These USU zones are as follows:

USU Hydrophysiographic Zone	Physiographic Sections from Fenneman and Johnson
1	1, 3A, 3B, 3C
2	3D, 3E
3	3F
4	4A, 4B, 5A, 5B
5	6A, 6B, 7A, 7B
6	6C, 8A
7	8B, 8C, 8D
8	8E, 8F, 8G
9	9A, 9B, 9C, 9D
10	9E, 10
11	11A, 11B, 11C, 11D
12	12A, 12B, 13C
13	12C, 12D, 12E, 12F, 13B, 13E, 13F, 13G, 13H, 13I, 13K, 14A, 14B, 15A, 15B
14	13A, 13D
19	22B, 22C, 22D, 22E, 21F

These 15 hydrophysiographic zones cover all of the 16 Potter States. The same zonality test was applied to the balance of the test watersheds in the United States and Puerto Rico. The additional sections combined are tabulated as follows:

USU Hydrophysiographic Zone	Physiographic Sections from Fenneman and Johnson
15	16, 17, 18, 19
16	20A, 20B, 23A
17	20C, 20D, 20E, 21A, 21B, 21C, 22A
18	21D, 21E
20	23B, 23D, 24A
21	24B, 24D, 24E, 24F, 24G
22	23C, 24C, 25
23	No zones available--Alaska
24	Hawaii and Puerto Rico combined

It is interesting that no significant differences exist between the watersheds in Hawaii and Puerto Rico even without zoning of either being considered. These two were combined since they were not significantly different even though little physiography was considered. A map delineating the USU hydrophysiographic zones of the contiguous United States is given as Figure 38 (p. 70).

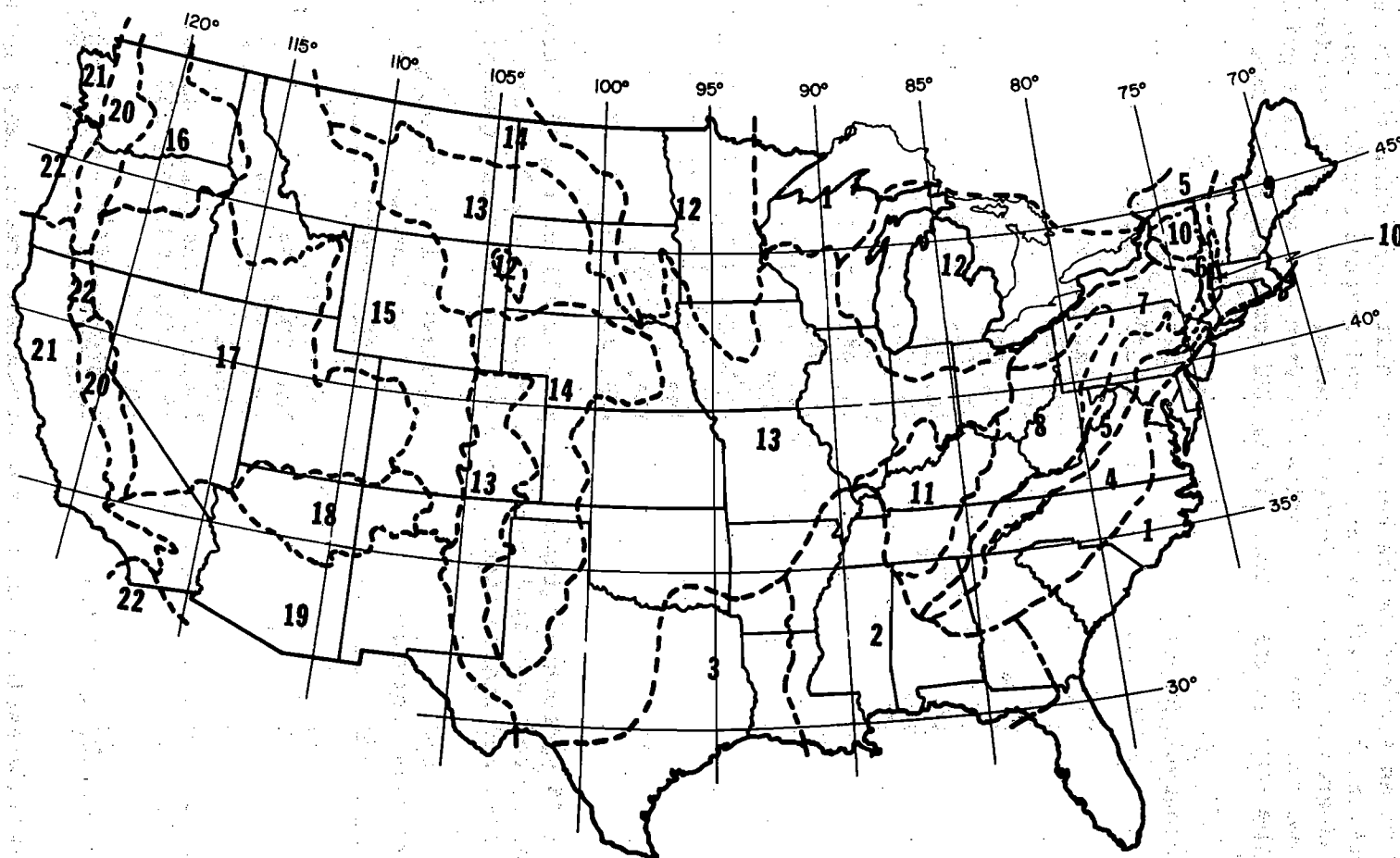


Figure 38. USU hydrophysiographic zones for the contiguous United States as determined by an analysis of the physiographic sections defined by Fenneman and Johnson (ref. 7).

The 958 watersheds and their associated basic data used in this phase of the study are given in Table 10 (p. 72-91) and are grouped within the 24 USU hydrophysiographic zones defined above.

### New Methods

The basic data used in the development and testing of the new methods are compiled in Tables 1 (p. 5-9), 7 (p. 32-36), and 10 (p. 72-91). The watershed numbers are in two categories. First, where available, the U.S. Geological Survey (ref. 19) numbers and second, the USDA Agricultural Research Service (refs. 17, 18) numbers are used to identify the watershed. The two services official names are also given for each watershed. The other data have been discussed earlier under parameters and frequency studies.

The problem of new method development was approached on the basis of supplying directly usable techniques for field use. The methods were investigated starting from the simplest and most direct to those of increasing complexity. They may be summarized as follows: The first procedure was to determine the probable maximum peak runoff for the United States and Puerto Rico by plotting the maximum flood peak of record,  $q_p$ , from Tables 1 (p. 5-9), 7 (p. 32-36), and 10 (p. 72-91) against the area of the watershed in square miles from which it came. The curve enveloping the upper perimeter of these points was determined from the data and is shown in Figure 39 (p. 92).

For many uses the probable maximum peak runoff is needed since it forms the boundary for the upper limit of runoff peaks for each fixed area. Thus all finite frequency runoff peaks are smaller than the values represented by the probable maximum peak runoff curve.

The second category of approaches was to assume that zonation was unnecessary. Combinations of the physical parameters are utilized to obtain a combination which would be at the first portion of diminishing returns between accuracy and simplicity. During the preliminary regressions and throughout, the methods of Steel and Torrie (ref. 16) were used. The reader is referred to this or other standard texts for explanations of the methods and limitations.

The preliminary filtering of variables consisted of simple correlations with all variables. All of the significantly correlated variables were retained for the preliminary multiple regression. The weighting of each variable was determined from the values by the F test. Only those with significant weights were retained for the later work and involved the seven variables A, R, DH, L,  $P_{60}$ ,  $P_{10}$ , and LL. The F tests further revealed that the most important three variables were R, A, and DH in that order and the most important five variables were R, A, DH, L, and  $P_{60}$ .

The first regression equation involving the three most important parameters, A, R, and DH was derived from data from all of the watersheds tabulated in Table 10 (p. 72-91) except those that had 4 percent or more of their area in surface water storage. It is called the 3-parameter all zone equation and is:

$$\hat{q}_{10} = 1.28015 A^{0.56172} R^{0.94356} DH^{0.16887}$$



Table 10. Continued.

1

Zone 01

2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

REQ STATION	STATION NAME	SECT	AREA	Q18	LAT	LONG	R	P18	P08	DM	L	LL	STRG	QMEAN	O2	Q60	Q100	QPEAK
50 128224000	LAKE LULU OUTLET AT ELOISE, FLA.	3C	83.86	185.8	27 59	81 44	558	9.99	3.14	28	5.97	7.88	26.8	78.7	86.1	331.7	483.8	218.8
53 128336000	SHETWATER CREEK NR SULPHUR SPRINGS, FLA.	3C	7.43	186.8	28 02	81 58	18.88	3.48	15	8.17	2.17	7.8	82.1	86.8	239.9	258.4	438.8	
54 128338000	BECHTOLD LAKE OUTLET NR LARGO, FLA.	3C	14.88	184.9	27 56	82 47	588	18.32	3.24	58	8.49	19.43	5.8	82.3	813.3	882.8	1884.8	842.8
55 12848 8.3	VERO BEACH FLORIDA WATERSHED N-3	3C	19.78	2864.8	28 06	80 86	588	9.85	3.88	23	9.36	24.81	9.8	887.2	485.4	9589.8	8831.8	2833.4

Zone 02

56 818248500	MUD CREEK NR MELVA, ALA.	3D	48.48	17974.8	32 14	85 45	388	8.93	2.77	358	13.88	41.88	8.1	7533.8	8441.4	28388.2	33817.1	28188.8
57 818248500	LAKE CREEK NR NORTHPORT, ALA.	3D	3.28	338.8	33 17	87 41	418	8.33	2.81	225	3.25	5.87	8.8	181.5	75.1	472.8	88.2	448.8
58 178338000	MAYOR CREEK AT GLENDALE, ILL.	3D	18.88	4218.8	37 25	88 48	195	7.88	2.22	418	11.75	48.38	9.8	2537.8	2388.3	8488.3	7282.8	8488.8
59 178338000	SUGAR CREEK NEAR GILSON SPRINGS, ILL.	3D	8.78	1785.8	37 24	88 48	195	7.88	2.22	284	8.88	38.88	1.8	1391.5	1391.5	8817.7	2142.8	1878.8
60 218788500	PERRY CREEK NEAR MAYFIELD, KY.	3D	1.72	1188.8	36 41	88 38	248	7.88	2.22	84	1.88	3.28	8.8	873.3	78.8	1978.8	2382.7	1878.8
61 228848888	BOGUE LUGA CREEK NEAR FRANKLIN, LA.	3D	18.18	6487.4	35 52	98 88	928	9.84	3.88	188	5.17	19.48	8.8	2532.3	2283.1	13284.8	18488.8	8758.8
62 228737888	WASHLEY CREEK NEAR ROBERT, LA.	3D	85.38	2888.7	38 38	88 18	718	9.84	3.13	118	8.88	18.48	8.8	1879.8	2828.8	3889.2	3378.8	2888.8
63 228737888	PONCHATOULA CREEK AT NATALGANY, LA.	3D	13.88	2284.8	38 34	88 28	785	9.88	3.18	48	7.88	27.77	8.8	1888.8	1782.8	2728.2	2884.5	2388.8
64 228738818	COLVELL CREEK AT LIVINGSTON, LA.	3D	88.78	3787.1	38 38	88 48	888	9.83	3.11	48	13.87	37.81	8.8	1388.2	1824.8	8878.8	8338.2	4888.8
65 228737888	EUCALYPTUS CREEK AT BALTYLLO, MISS.	3D	18.78	4881.8	34 48	88 48	538	7.78	2.48	117	6.51	48.88	8.8	3888.8	3157.1	8788.4	7817.8	5788.8
66 228737888	SUCK CREEK NEAR RUMBLESTOWN, MISS.	3D	18.18	3834.3	31 28	88 84	895	9.58	3.82	288	11.82	88.73	8.8	1333.7	1333.7	5881.4	8888.2	3888.8
67 228737888	818 CREEK NEAR LUCEDALE, MISS.	3D	88.88	4482.3	38 38	88 38	988	18.28	3.48	118	8.57	18.57	8.8	1888.3	1388.8	1188.8	1188.8	7888.8
68 228737888	MURDER CREEK NEAR POPLAIRVILLE, MISS.	3D	81.88	6833.8	38 47	88 32	748	9.80	3.11	198	18.37	42.37	8.8	1358.8	1358.8	1112.8	1388.2	7888.8
69 228737888	PURPLE CREEK NEAR JACKSON, MISS.	3D	8.18	2488.8	32 22	88 32	548	8.78	2.71	121	8.58	18.57	8.8	1387.8	1387.8	1888.8	4337.8	8888.8
70 228737888	HUNTING HORN CREEK NEAR JACKSON, MISS.	3D	18.88	3888.8	32 22	88 32	548	8.78	2.71	121	8.58	18.57	8.8	1387.8	1387.8	1888.8	4337.8	8888.8
71 228737888	MOORE CREEK NEAR TERRY, MISS.	3D	88.88	4888.8	32 22	88 32	548	8.78	2.71	121	8.58	18.57	8.8	1387.8	1387.8	1888.8	4337.8	8888.8
72 228737888	PICZ CREEK AT ETTA, MISS.	3D	88.88	4888.8	32 22	88 32	548	8.78	2.71	121	8.58	18.57	8.8	1387.8	1387.8	1888.8	4337.8	8888.8
73 228737888	NORTH TIPPAN CREEK NEAR RIPLEY, MISS.	3D	88.88	8178.8	34 44	88 88	848	7.88	2.48	183	8.88	37.88	8.8	8831.8	2483.1	7887.4	8883.8	7188.8
74 228737888	CLARK CREEK NEAR OXFORD, MISS.	3D	18.38	6281.8	34 44	88 88	848	7.88	2.48	183	8.88	37.88	8.8	8831.8	2483.1	7887.4	8883.8	7188.8
75 228737888	TILOA CREEK NEAR CANTON, MISS.	3D	18.38	6281.8	34 44	88 88	848	7.88	2.48	183	8.88	37.88	8.8	8831.8	2483.1	7887.4	8883.8	7188.8
76 228737888	ALEXANDER CREEK NEAR ST. FRANCISVILLE, LA.	3D	13.88	1388.8	38 48	91 28	588	8.88	3.03	188	8.88	19.38	8.8	8888.8	8888.8	8888.8	8888.8	1888.8
77 228737888	BOYD CREEK NEAR ABOVE BAKER, LA.	3D	13.78	3783.7	38 37	81 18	818	8.88	3.07	31	8.83	14.82	8.8	1378.8	822.8	8884.3	8884.3	7388.8
78 228737888	MIDDLE COLVELL CREEK NEAR WALKER, LA.	3D	82.88	18738.8	38 38	88 38	888	8.88	3.18	188	17.18	32.88	8.8	1153.8	1327.8	1913.8	2887.8	1738.8
79 228737888	ABALMORE CREEK NEAR CHARLESTON, MISS.	3D	31.88	1883.1	33 31	88 81	728	8.18	2.58	84	18.88	31.88	8.8	8878.8	8788.8	13514.3	14338.8	18288.8
80 228737888	THOMPSON CREEK AT MCCALL, MISS.	3D	14.48	3881.8	33 31	88 81	548	8.14	2.58	88	8.88	31.88	8.8	8878.8	8788.8	13514.3	14338.8	18288.8
81 228737888	DUNDON CREEK NEAR VICKSBURG, MISS.	3D	8.88	3882.4	38 18	88 88	388	8.88	2.78	288	3.88	8.18	8.8	1878.8	2888.8	3888.8	8888.8	4878.8
82 228737888	OSBERY CREEK NEAR DOLORO, MISS.	3D	8.88	488.8	31 19	91 21	588	8.34	2.88	188	1.38	24.88	8.8	888.8	218.8	488.8	477.8	418.8
83 228737888	LITTLE RIVER DITCH 888 NEAR KENNETH, MO.	3D	88.88	3888.3	38 14	91 88	248	7.88	2.38	27	18.48	44.88	8.8	1888.8	1887.7	4188.8	4584.1	4148.8

Zone 03

84 228737888	MCCAIN CREEK NEAR SHREVEPORT, LA.	3D	13.88	888.3	32 38	83 38	378	8.88	2.81	128	9.84	27.88	8.8	878.8	887.8	1187.8	1888.8	1888.8
85 228737888	LITTLE SANDY CREEK AT KIBATCHIE, LA.	3D	21.88	8881.8	31 24	83 18	478	8.81	2.83	188	18.42	27.83	8.8	8843.1	3831.4	7248.8	8888.8	8888.8
86 228737888	MORRETT CREEK NEAR PROVENAL, LA.	3D	8.88	1888.8	31 24	83 18	478	8.81	2.83	188	18.42	27.83	8.8	8843.1	3831.4	7248.8	8888.8	8888.8
87 228737888	CYPRESS CREEK NEAR VIXEN, LA.	3D	18.88	8328.8	32 18	88 14	388	8.88	2.78	188	8.88	27.88	8.8	1888.8	1888.8	1888.8	1888.8	7888.8
88 228737888	FLORIAN CREEK AT JONESBORO, LA.	3D	8.14	1384.3	38 11	82 44	388	8.88	2.88	128	2.37	2.37	8.8	888.8	888.8	888.8	888.8	1878.8
89 228737888	MAPPOON CREEK NEAR LACAMP	3D	18.78	1383.1	31 11	82 88	488	8.78	3.08	188	8.83	88.74	8.8	884.8	884.8	884.8	884.8	1888.8
90 228737888	MAPPOON CREEK AT HANTY, LA.	3D	88.78	13213.8	31 34	83 88	438	8.78	3.08	148	18.88	38.31	8.8	3884.8	2837.8	2837.8	3118.1	8888.8
91 4888 48.3	MAGO TEXAS WATERSHED 8	3D	8888	7488.8	31 31	88 83	878	8.38	2.83	88	1.88	2.88	8.8	313.8	882.8	1888.8	1888.8	888.8
92 4888 48.3	MAGO TEXAS WATERSHED 9	3D	1728	1488.8	31 31	88 83	888	8.38	2.83	88	8.38	8.48	8.8	888.8	888.8	888.8	888.8	888.8
93 4888 48.3	MAGO TEXAS WATERSHED 10	3D	8878	8888.8	31 27	88 83	288	8.88	3.84	88	8.77	1.818	8.8	888.8	874.8	874.8	874.8	888.8
94 4888 48.3	MAGO TEXAS WATERSHED 11	3D	8888	4817.3	31 27	88 83	288	8.88	3.84	48	8.48	8.817	8.8	184.8	148.1	788.2	888.2	888.2
95 4888 48.3	MAGO TEXAS WATERSHED 12	3D	8888	1888.8	31 31	88 83	288	8.88	3.84	38	8.48	8.817	8.8	88.8	88.8	88.8	88.8	88.8
96 4888 48.3	MAGO TEXAS WATERSHED 13	3D	8888	788.8	31 31	88 83	388	8.88	3.84	28	8.18	8.18	8.8	33.4	33.4	188.8	188.8	88.8
97 4888 48.3	MAGO TEXAS WATERSHED 14	3D	1.48	4888.8	38 38	87 88	888	8.88	3.87	188	1.88	2.78	8.8	145.1	76.8	1878.8	1888.8	788.8
98 4888 48.3	MAGO TEXAS WATERSHED 15	3D	1.48	4888.8	38 38	87 88	888	8.88	3.87	188	1.88	2.78	8.8	145.1	76.8	1878.8	1888.8	788.8



Table 10. Continued.

Zone 04

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

STATION	STATION NAME	SEC	AREA	918	LAT	LONG	R	P16	P68	DN	L	LL	STAG	ONEAN	O2	O30	OPEAK
98 9182414680	HARBURG CREEK NR MACKNEVILLE, ALA.	4A	9.78	8770.6	33 07	88 37	355	9.40	8.85	189	3.85	8.77	8.0	1176.4	1180.2	2055.7	3370.4
98 1601477880	SHELLPOT CREEK AT WILKINSON, DEL.	4A	7.48	3467.7	38 47	75 32	165	7.18	7.25	398	6.89	82.56	8.1	1781.6	182.7	8274.5	13599.1
100 1302177180	ASHLEY CREEK NEAR CLAYTON, GA.	4A	9.98	159.3	34 38	83 86	398	7.47	2.35	848	5.77	18.97	8.0	86.5	60.5	233.7	341.7
101 1302161080	LITTLE PANTHER CREEK NEAR CLARKSVILLE, GA.	4A	8.88	167.0	34 43	83 24	278	7.88	2.41	278	2.43	5.12	8.0	212.7	182.0	1285.6	1874.0
101 1302180880	PANTHER CREEK NEAR TOCCOA, GA.	4A	38.98	8845.7	34 41	83 21	268	7.88	2.48	1888	9.55	27.60	8.0	3815.8	2412.6	17592.1	33824.8
103 1302190880	N F BOARD R SUBWATERED 14 NR AVALON, GA.	4A	1.88	967.1	34 38	83 13	298	7.87	2.41	157	1.62	3.69	8.0	342.1	342.1	1819.5	1163.7
104 1302205880	WILCOAT CREEK NEAR STOCK BRIDGE, GA.	4A	1.88	391.4	34 38	84 88	295	7.88	2.58	115	1.66	3.11	8.0	222.4	240.1	571.8	649.4
106 1302217880	ALLEN CREEK AT TALHO, GA.	4A	17.38	3881.8	34 12	83 43	298	7.87	2.47	388	18.38	48.82	8.0	1977.5	1410.8	5683.2	8841.8
106 1302281880	MURDER CREEK NEAR MONTICELLO, GA.	4A	84.88	8782.3	33 59	83 48	388	8.24	2.68	288	7.85	26.83	8.0	1417.4	1374.9	4823.6	4549.8
107 1302303880	ROCK CREEK NEAR FAIRMOUNT, GA.	4A	8.81	1145.7	34 31	84 48	288	7.78	2.43	188	2.59	7.79	8.0	538.4	468.8	2836.7	2174.6
108 2401579880	BASIN RUN AT LIBERTY GROVE, MD.	4A	9.31	8437.8	38 48	79 08	185	7.83	2.24	188	3.68	18.38	8.1	1889.7	882.6	3695.4	4412.9
108 2401582880	CLONE RUN NEAR ROCKY HILL, MD.	4A	8.88	377.7	38 38	78 48	188	7.86	2.58	275	3.43	1.98	8.0	187.9	163.9	739.9	879.4
109 2401585880	FLYING BRANCH NEAR WESTMINSTER, MD.	4A	9.48	788.8	38 38	78 38	188	7.41	2.41	188	3.43	3.43	8.0	282.9	178.7	1589.4	2436.1
111 8401568880	FLYING RUN NEAR BUNKERVILLE, MD.	4A	11.48	8421.8	38 25	77 81	135	8.82	2.58	438	8.88	46.83	8.0	4893.8	4893.8	8892.5	7889.4
112 2401846580	LITTLE FALLS BRANCH NEAR BETHESDA, MD.	4A	3.24	8887.8	38 08	77 86	188	8.24	2.53	188	2.46	6.93	8.0	1492.4	1313.3	5482.2	6586.9
114 3782867240	STRUP IRON CREEK TRI NR NELSON, N. C.	4A	9.18	1192.7	38 13	78 81	248	8.88	2.53	828	4.78	6.38	8.0	932.3	348.8	2882.7	2427.3
115 3782869880	EAST FORK DEEP R NEAR HIGH POINT, N. C.	4A	14.78	3188.8	38 84	78 38	288	7.84	2.36	188	6.48	55.13	8.1	1883.8	1888.8	8783.8	8158.8
116 3782115580	FORBUSH CREEK NEAR YADKINVILLE, N. C.	4A	21.78	2178.8	38 11	88 36	195	7.11	8.25	235	11.88	98.79	8.2	1274.1	1228.3	2787.4	2848.8
117 3782152420	STE KNOB CREEK NEAR FALLOUTON, N. C.	4A	18.48	2484.1	38 89	81 32	158	7.38	2.32	485	7.76	33.78	8.0	1254.6	1853.0	4787.5	5457.5
118 4882154580	NORTH PACOLEY R AT FINGERSVILLE, S. C.	4A	116.8	8836.1	35 57	81 59	388	7.82	2.31	2495	26.88	128.88	8.0	3685.2	3362.6	18788.8	12378.8
119 4882157880	NORTH TYGER RIVER NEAR FAIRMONT, S. C.	4A	48.88	2886.4	34 86	82 32	285	7.86	2.32	388	3.33	7.25	8.0	1689.9	1885.1	4583.7	5885.3
120 4882164880	NEEDY RIVER NEAR GREENVILLE, S. C.	4A	48.88	3354.2	34 48	82 32	388	7.82	2.34	388	18.67	48.78	8.0	2888.4	2888.4	4485.3	4855.6
121 5181655880	CEAR RUN NEAR WARRENTON, VA.	4A	13.88	3888.8	38 44	77 47	145	7.47	2.34	628	4.84	9.32	8.0	1818.8	1888.8	9999.9	12889.7
122 5181658880	BROAD RUN NEAR WARRENTON, VA.	4A	9.88	1888.8	38 48	77 48	158	7.42	2.38	288	1.58	6.43	8.0	188.8	188.8	281.8	288.8
123 5181658880	SOUP R QUANTICO C NR INDEPENDENT HILL, VA.	4A	7.88	927.8	38 36	77 58	178	7.18	2.88	148	18.33	22.97	8.0	542.6	542.6	1313.1	1427.8
124 5181671880	WUDSON CREEK NEAR FORELLY TAYLOR, VA.	4A	6.18	848.2	38 32	78 11	135	7.55	2.33	125	4.28	8.38	8.0	31.8	23.7	188.3	218.8
125 5181671880	TOTOPOTON CREEK NR MARGARET, VA.	4A	8.88	3888.2	37 57	78 38	188	8.25	2.33	81	14.78	12.33	8.0	118.8	118.8	218.8	218.8
125 5182282880	S N F RANONIA R NR NORTH GARDEN, VA.	4A	9.58	3882.3	37 57	78 38	188	7.18	2.84	81	3.41	13.33	8.0	118.8	118.8	218.8	218.8
127 5182315880	NORTH FORK HOBBAN'S RIVER NR WHITEHALL, VA.	4A	11.48	2255.8	38 38	78 45	155	8.37	2.19	1788	8.87	38.87	8.0	1828.7	714.5	8481.4	7818.4
128 5182315880	PINE CREEK AT PINE CREEK HILL, VA.	4A	83.88	1888.8	37 38	77 45	288	8.38	2.58	188	9.68	51.87	8.0	618.8	482.2	3488.2	4289.7
128 5182315880	NORTH MEHERRIN RIVER NEAR KEYSVILLE, VA.	4A	9.28	1488.6	37 03	78 85	218	7.86	2.53	178	3.84	8.37	8.0	888.7	888.7	2678.1	3177.2
130 5182861380	WININGER CREEK NEAR BEDFORD, VA.	4A	4.77	1411.8	37 16	78 28	148	7.87	2.23	248	3.76	7.49	8.0	785.6	978.6	2489.6	2921.2
131 5182878880	GEORGES CREEK NEAR GRETN, VA.	4A	9.28	1151.4	38 56	79 18	155	7.33	2.32	425	6.92	21.42	8.0	588.6	488.6	1782.8	1877.4
132 1881476880	CHRISTINA RIVER AT COUCHS BRIDGE, DEL.	4A	28.88	2288.7	39 41	75 47	165	6.13	2.25	378	12.58	37.31	8.1	1579.2	1484.4	3883.2	4769.2
133 2481641880	HUNTING CREEK AT JINTOWN, MD.	4A	18.48	1197.8	38 36	77 24	145	7.18	2.27	1258	9.92	32.79	8.0	787.3	936.1	1386.8	1485.5
134 2481641880	BAOULE RIVER AT RIDGEWOOD, N. J.	4A	18.28	2276.7	40 08	74 87	125	8.81	2.86	435	18.68	36.46	8.0	1848.5	936.4	3943.5	4881.4
135 2481641880	HOKORUS BROOK AT HOKORUS, N. J.	4A	18.28	2276.7	40 08	74 87	125	8.81	2.86	435	18.68	36.46	8.0	1848.5	936.4	3943.5	4881.4
137 4813241880	NEAT BRANCH RANNEY RIVER AT MILLBURN, N. J.	4A	7.18	1188.8	40 08	74 87	125	8.81	2.86	435	18.68	36.46	8.0	1848.5	936.4	3943.5	4881.4
137 4813241880	ZACHARIS CREEK NR SHIPPAKE, PA.	4A	7.18	1188.8	40 08	74 87	125	8.81	2.86	435	18.68	36.46	8.0	1848.5	936.4	3943.5	4881.4
138 5181654880	ACCOITON CREEK NEAR ANNANDALE, VA.	4A	23.88	8888.1	38 52	77 18	188	8.88	2.81	218	8.88	78.46	8.1	8284.8	1883.2	18138.3	12343.8
138 5181654880	LITTLE CATOCTIN CREEK AT HARMONY, MD.	4A	8.23	3128.8	38 38	77 38	145	7.31	8.37	838	3.97	18.18	8.0	882.8	478.8	6818.4	8873.8
140 2481648880	OHNE CREEK AT LANTZ, MD.	4A	8.23	1488.8	39 41	77 28	145	7.31	8.37	838	3.97	18.18	8.0	882.8	478.8	6818.4	8873.8
141 4881614880	CONDOCHEAGUE CREEK NR FAYETTEVILLE, PA.	4A	8.88	388.8	38 56	77 28	125	6.48	8.18	778	3.85	8.43	8.0	114.3	68.9	488.6	514.4
148 5181638810	HAPPY CREEK AT FRONT ROYAL, VA.	4A	13.88	1888.8	38 34	78 11	148	8.31	2.81	1888	6.88	38.88	8.0	849.7	713.6	3288.4	3316.2
148 5181638810	RUSH RIVER AT WASHINGTON, VA.	4A	13.88	1888.8	38 34	78 11	148	8.31	2.81	1888	6.88	38.88	8.0	849.7	713.6	3288.4	3316.2
144 5182833880	SOUTH RIVER NEAR STEELS TAYLOR, VA.	4A	18.78	2788.4	37 88	78 18	148	8.31	2.81	1888	6.88	38.88	8.0	849.7	713.6	3288.4	3316.2
145 5182833880	NORTH FORK HARMONY R AT RED HILL, VA.	4A	11.88	3888.8	37 88	78 37	188	7.88	2.24	288	3.41	8.78	8.0	888.7	358.8	1884.2	1484.2
146 3783448880	CATYBETH CREEK NEAR BREVARD, N. C.	4A	11.78	1853.4	38 12	82 48	198	7.44	8.34	1832	5.37	25.34	8.0	849.8	888.4	1782.2	1823.7
147 3783441880	DAYTON RIVER NEAR BREVARD, N. C.	4A	48.48	5883.3	38 12	82 48	198	7.44	8.34	1832	5.37	25.34	8.0	849.8	888.4	1782.2	1823.7
148 3783448880	CRAB CREEK NEAR PENNORE, N. C.	4A	18.88	8778.8	35 18	82 34	198	7.33	2.34	188	4.28	31.72	8.1	1388.8	832.1	9847.8	12482.1
148 3783448880	SOUTH FORK CREEK NEAR HORSBOW, N. C.	4A	18.88	8778.8	35 18	82 34	198	7.33	2.34	188	4.28	31.72	8.1	1388.8	832.1	9847.8	12482.1
150 3783448880	SOUTH FORK HILLS N AT THE PINK, N. C.	4A	9.88	1888.8	38 22	82 48	198	7.38	2.33	188	4.28	31.72	8.1	1388.8	832.1	9847.8	12482.1

Table 10. Continued.

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Zone 04

2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

SEC STATION	STATION NAME	SECT	AREA	G18	LAT	LONG	R	PLS	P88	DH	L	LL	STPO	ONEAN	02	050	0100	ONEAN
152 373458000	SEETREE CREEK NEAR WADSWORTH, N. C.	56	5.46	948.8	36.39	82.24	149	7.39	2.38	1888	3.34	0.47	0.8	285.8	243.8	999.2	1889.8	1378.8
153 373458000	ALLAN CREEK NEAR WADSWORTH, N. C.	58	14.48	1323.8	35.83	83.98	158	7.37	2.32	2348	4.70	41.95	0.8	886.8	886.8	999.2	2121.8	1478.8
154 373458000	CULLAJA RIVER AT KIMMEL, N. C.	58	14.48	1323.8	35.83	83.98	158	7.37	2.32	2348	4.70	41.95	0.8	886.8	886.8	999.2	2121.8	1478.8
155 373458000	NOLAND CREEK NEAR BRYSON CITY, N. C.	58	13.98	1298.8	35.31	83.24	165	7.34	2.31	3928	8.38	53.82	0.8	1819.3	1888.8	999.2	3742.7	4421.1
156 373458000	NORTH INDIAN CREEK NEAR UNICOU, TENN.	58	13.98	1298.8	35.31	83.24	165	7.34	2.31	3928	8.38	53.82	0.8	1819.3	1888.8	999.2	3742.7	4421.1
157 373458000	TURTLE CREEK AT TURTLETON, TENN.	58	13.98	1298.8	35.31	83.24	165	7.34	2.31	3928	8.38	53.82	0.8	1819.3	1888.8	999.2	3742.7	4421.1
158 373458000	DAVIS MILL CREEK AT COPPERHILL, TENN.	58	9.16	875.9	35.98	84.53	258	7.48	2.34	189	8.68	25.37	0.8	885.3	885.3	999.2	1397.2	1417.1
159 373458000	BLACKSBURG VA WATERSHED N-3	58	9.16	875.9	35.98	84.53	258	7.48	2.34	189	8.68	25.37	0.8	885.3	885.3	999.2	1397.2	1417.1
160 373458000	STALEY CREEK NEAR MARION, VA.	58	9.33	368.5	38.49	81.28	148	8.07	2.17	738	4.84	14.04	0.8	288.8	328.8	403.4	944.8	418.8

Zone 05

161 3184872000	FIVEHOLE CREEK AT RETONA, ALA.	6A	28.88	4114.8	33.37	86.43	358	6.16	2.56	458	9.11	88.88	0.8	288.7	182.1	738.1	938.1	4978.8
162 3184872000	FIRST CR. MIN. SPRS. AVE., KNOXVILLE, TENN.	6A	18.76	1228.8	36.91	83.55	178	7.81	2.18	198	7.20	28.07	0.8	874.7	748.7	1248.7	1817.7	1318.8
163 3184872000	WHITE CREEK NEAR SHARPS CHAPEL, TENN.	6A	8.88	281.9	36.21	83.34	178	6.84	2.14	488	2.36	3.88	0.8	153.8	148.8	188.8	328.8	883.8
164 3184872000	SUFFALO CREEK AT MORRIS, TENN.	6A	9.48	1288.4	36.11	84.84	178	6.83	2.17	242	8.96	24.89	0.8	897.4	897.4	999.2	1238.5	1982.7
165 3184872000	NORTH RIVER NEAR STOKESVILLE, VA.	6A	11.38	1878.8	36.19	79.18	123	6.74	2.12	375	5.48	13.42	0.8	889.3	388.8	3716.7	4588.8	8888.8
166 3184872000	COMPASTURE RIVER NEAR NEWATERG, VA.	6A	12.38	1738.7	36.83	79.17	138	6.78	2.12	1878	8.98	184.35	0.8	1153.8	887.7	9248.8	8348.8	11188.8
167 3184872000	CALPAPTURE RIVER NEAR NEWATERG, VA.	6A	12.38	1738.7	36.83	79.17	138	6.78	2.12	1878	8.98	184.35	0.8	1153.8	887.7	9248.8	8348.8	11188.8
168 3184872000	WIDOLE FORK HOLSTON R. AT BRIDGECLOVE, VA.	6A	7.38	328.7	36.33	81.11	145	6.86	2.16	48	3.12	31.32	0.8	887.7	227.8	1887.4	1883.5	813.8
169 3184872000	BEAVER CREEK NEAR VALLACE, VA.	6A	12.78	328.7	36.33	81.11	145	6.86	2.16	48	3.12	31.32	0.8	887.7	227.8	1887.4	1883.5	813.8
170 3184872000	CONVE CREEK NEAR SHELLEY, VA.	6A	12.78	328.7	36.33	81.11	145	6.86	2.16	48	3.12	31.32	0.8	887.7	227.8	1887.4	1883.5	813.8
171 3184872000	SANPIT RUN NEAR OLTON, MO.	6A	5.98	538.8	38.34	78.33	124	6.34	1.88	308	2.56	24.37	0.8	874.8	788.8	2483.4	8938.7	8888.8
172 3184872000	LITTLE TONOLWAY CR NEAR HANCOCK, MO.	6A	10.98	1388.8	39.43	78.15	115	6.33	1.99	1127	6.88	71.48	0.8	1883.1	748.7	2483.4	8938.7	8888.8
173 3184872000	BYLER RUN NEAR LOUISVILLE, PA.	6A	10.98	1388.8	39.43	78.15	115	6.33	1.99	1127	6.88	71.48	0.8	1883.1	748.7	2483.4	8938.7	8888.8
174 3184872000	PAYTON CREEK NEAR PENNSBORO, PA.	6A	11.38	1868.8	48.28	78.88	189	6.44	2.82	928	8.58	32.18	0.8	1883.1	748.7	2483.4	8938.7	8888.8
175 3184872000	MANADA CREEK AT MANADA GAP, PA.	6A	13.08	2247.7	48.28	78.42	98	6.42	2.88	578	4.71	13.91	0.8	883.8	847.8	3388.7	3781.8	8888.8
176 3184872000	HAR BRANCH NEAR MINTON, VA.	6A	9.38	1753.7	38.38	78.99	125	6.75	2.13	1188	5.58	15.38	0.8	738.7	813.8	3781.8	4581.1	2888.8
177 3184872000	TUCANDORA CREEK ABOVE MARTINSBURG, N. VA.	6A	11.38	254.8	38.38	77.88	128	6.84	2.88	888	6.52	13.88	0.8	154.3	177.4	358.4	418.8	388.8
178 3184872000	POULTRY RIVER TRIG. AT EAST POULTRY, VT.	7A	1.13	88.8	43.32	73.18	92	5.21	1.84	488	1.88	3.21	0.8	88.1	139.9	161.7	88.8	88.8
179 3184872000	SHADY BROOK AT SHADY LOAN, VT.	7A	2.44	241.2	43.37	73.88	86	4.83	1.88	1882	2.78	4.24	0.8	128.8	128.8	361.8	417.3	238.8
180 3184872000	BEAVER BROOK AT CORNWALL, VT.	7A	1.11	73.3	43.37	73.13	86	4.86	4.32	288	2.48	4.98	0.8	88.1	82.5	81.1	88.3	73.8
181 3184872000	LITTLE OTTER CREEK TRIG. AT BRISTOL, VT.	7A	5.31	73.8	44.13	73.83	84	4.48	1.88	188	2.88	4.98	0.8	32.3	32.3	88.4	181.8	72.8
182 3184872000	LEWIS CREEK TRIG. NO. 2 MR ROCKVILLE, VT.	7A	1.97	86.1	44.18	73.84	88	4.48	1.88	188	2.88	4.98	0.8	314.8	378.5	1243.8	1448.4	78.8
183 3184872000	STEVENS BRANCH TRIG. AT SOUTH GARRE, VT.	7A	9.38	59.3	44.11	73.31	64	4.38	1.87	288	6.83	1.25	0.8	88.3	33.7	98.8	113.1	42.8
184 3184872000	STONE BRIDGE BROOK NEAR GEORGIA PLAINS, VT.	7A	9.48	246.4	44.12	73.11	70	4.35	1.48	372	5.23	21.70	1.71	142.4	148.1	341.8	384.9	264.8
185 3184872000	TROUT BROOK AT STOCKHOLM CENTER N. Y.	7A	4.08	131.3	44.40	74.40	75	4.88	1.45	325	8.48	71.58	0.8	886.8	838.3	1844.7	8878.4	1388.8
186 3184872000	ALLEN BROOK NEAR GRABER, N. Y.	7A	18.88	828.8	44.40	74.44	75	4.84	1.44	678	7.88	15.18	0.8	438.2	488.1	1788.8	2881.4	1188.8
187 3184872000	LAWRENCE BROOK NEAR MOIRA, N. Y.	7A	28.88	1378.8	44.38	74.38	75	4.83	1.44	698	6.88	24.58	0.8	724.1	753.1	2288.5	2588.8	1478.8
188 3184872000	DEER RIVER AT GRABER IRON HORNS, N. Y.	7A	188.8	878.1	44.34	74.41	77	4.88	1.43	843.8	37.28	185.88	0.8	3788.9	3388.9	10744.2	28324.8	11388.8
189 3184872000	WEST BRANCH DEER CREEK AT FT. COVINGTON CENTER N. Y.	7A	31.48	1887.2	44.37	74.39	77	4.88	1.43	818	11.28	34.58	0.8	872.1	872.1	2537.8	2921.5	1888.8
190 3184872000	EAST BRANCH DEER CREEK AT FT. COVINGTON CENTER N. Y.	7A	28.18	858.5	44.37	74.39	77	4.88	1.43	818	11.28	34.58	0.8	872.1	872.1	2537.8	2921.5	1888.8
191 3184872000	LITTLE SALMON RIVER AT BONGAY, N. Y.	7A	38.08	878.1	44.38	74.33	75	4.88	1.43	1788	33.88	188.88	0.8	1884.7	1743.2	3883.3	4383.9	2788.8
192 3184872000	TRIBUT RIVER AT TRIBUT RIVER N. Y.	7A	87.8	474.5	44.38	74.38	75	4.88	1.43	858	11.88	188.88	0.8	287.5	288.8	7875.1	8244.3	9388.8
193 3184872000	ENGLISH RIVER NEAR HODGES FORKS, N. Y.	7A	48.88	1038.8	44.38	74.38	75	4.88	1.43	858	11.88	188.88	0.8	287.5	288.8	7875.1	8244.3	9388.8

Zone 06

194 3681333000	HEWITTO CREEK NEAR BRADOLIN N. Y.	8C	28.38	1818.8	43.84	73.18	85	5.43	1.78	1188	14.48	73.88	0.8	841.5	888.2	2231.8	2485.8	1848.8
195 3681333000	BOND CREEK AT DUNHAM BASIN, N. Y.	8C	14.78	1188.8	43.84	73.18	188	5.88	1.84	88	8.18	42.94	0.8	747.4	747.4	1821.8	1888.7	1378.8
196 3681333000	MOOREHEAD KILL AT CARLETON-DE-HUDSON, N. Y.	8C	38.88	1188.8	43.84	73.18	88	5.88	1.84	88	8.18	42.94	0.8	747.4	747.4	1821.8	1888.7	1378.8
197 3681333000	QUAKER CREEK AT FLORIDA, N. Y.	8C	8.74	783.2	41.18	74.81	183	6.35	1.88	328	8.52	38.88	0.8	438.1	388.2	1888.8	1188.2	1888.8





Table 10. Continued.

Zone 09

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

STATION	STATION NAME	SECT	AREA	Q10	LAT	LONG	R	Q10	Q90	DN	L	LL	STRO	MEAN	Q2	Q50	Q100	Q100
288 2350101700	FACTORY BROOK NEAR MADANASKA, ME.	88	0.03	248.1	47 81	68 18	51	3.08	1.18	388	4.25	9.54	9.0	158.6	158.6	310.8	330.7	352.6
289 2350101700	HOUTON BROOK NEAR OXON, ME.	88	0.08	217.2	48 08	68 24	74	3.58	1.18	158	3.54	9.54	13.0	123.4	123.4	310.8	330.7	352.6
290 2350101700	NICHOLS BROOK NEAR CARIBOU, ME.	88	0.04	286.8	48 01	67 06	68	3.78	1.23	188	3.42	8.39	9.0	110.0	110.0	304.8	330.7	352.6
301 2350103400	GULLIVER BROOK NEAR HOWARD, ME.	88	11.09	447.7	48 43	68 19	76	4.18	1.32	288	8.78	18.08	21.0	301.5	331.4	689.8	787.1	962.6
302 2350103400	COFFIN BROOK NEAR LEE, ME.	88	0.21	135.2	48 08	68 22	68	4.38	1.35	288	2.24	5.66	21.0	72.0	285.6	233.3	143.0	143.0
303 2350103500	BIG BROOK NEAR BUCKFIELD, ME.	88	10.08	293.8	48 16	78 19	67	4.07	1.56	188	6.06	28.32	9.0	184.8	283.3	389.6	489.6	269.8
304 2350103500	TANBELL BROOK NEAR WINCHESTER, MA.	88	10.88	641.9	48 43	78 08	188	6.48	1.78	288	6.98	24.08	4.0	327.9	268.9	1898.7	2151.0	3888.8
305 2350103500	PRIEST BROOK NEAR WINCHESTER, MA.	88	10.48	687.6	48 41	78 07	188	6.48	1.72	488	12.44	39.72	6.0	484.8	398.3	2588.9	3391.8	2698.8
306 2350103500	HOP BROOK NEAR NEW BALEN, MA.	88	3.36	244.9	48 29	78 28	118	5.38	1.75	482	2.88	5.94	9.0	148.2	148.2	332.7	488.1	289.3
307 2350103500	WATER BROOK AT KNIGHTSVILLE, MASS.	88	1.24	189.3	48 17	78 32	129	6.78	1.51	338	3.22	32.52	9.0	89.1	87.7	1342.5	1350.0	689.8
308 2350103500	MUD BROOK NEAR WILKINSON, MA.	88	7.97	311.8	48 08	72 14	88	4.78	1.74	188	7.97	39.85	2.0	149.1	134.1	813.2	813.2	6106.8
309 2350103500	OTTER BROOK NEAR KEENE, N. H.	88	7.97	311.8	48 08	72 14	88	4.78	1.74	188	7.97	39.85	2.0	149.1	134.1	813.2	813.2	6106.8
310 2350103500	S BR ABHUELOT R AT NEER, NEAR MARLBORO, N. H.	88	7.98	687.7	48 52	78 13	181	4.31	1.78	888	7.98	68.48	1.0	1238.8	984.6	4615.5	5561.7	5060.8
311 2350103500	RINGWOOD CREEK NEAR MANAQUE, N. J.	88	10.18	889.0	41 08	74 16	178	6.78	2.64	688	16.82	24.32	8.0	537.3	537.3	1182.1	1284.2	1198.8
312 2350103500	WEST BROOK NEAR MANAQUE, N. J.	88	11.08	1184.4	41 04	74 16	178	6.78	2.65	688	4.61	22.62	8.0	837.5	596.1	1684.2	1845.2	1880.8
313 2350103500	BLIND BROOK AT AVE, N. Y.	88	0.08	1897.8	48 58	73 41	129	6.78	2.11	935	7.28	12.36	0.0	781.2	652.1	2834.5	3328.0	2328.0
314 2350103500	NEPHUC R AT HARRISVILLE R.1	88	10.08	888.4	41 58	71 41	188	6.68	1.62	418	6.88	21.78	0.0	482.7	561.0	1592.0	1798.4	1020.0
315 2350103500	MOHAWUCK R AT CENTERDALE, R.1	88	30.36	1945.8	41 52	71 29	118	8.18	1.84	488	8.18	28.68	4.0	597.5	552.2	1388.6	1582.8	1440.0
316 2350103500	EAST ORANGE BRANCH AT EAST ORANGE, VT.	88	0.88	443.8	48 28	72 28	85	4.88	1.08	888	9.81	7.88	0.0	277.4	291.3	524.3	554.8	489.8
317 2350103500	AYERS BROOK AT RANDOLPH, VT.	88	30.98	3138.8	43 58	72 38	86	4.97	1.61	1188	18.28	57.98	0.0	1888.8	984.8	7959.8	9889.8	2888.8
318 2350103500	GREEN RIVER AT BUCKFIELD, VT.	88	10.98	978.3	43 56	72 38	86	4.48	1.52	928	9.83	24.14	7.0	528.4	475.8	1743.7	2881.9	2888.8
319 2350103500	MOHAWK BROOK NEAR LANE, VT.	88	3.21	571.4	48 08	78 24	89	4.34	1.38	1888	7.28	11.58	0.0	218.2	188.2	1884.5	813.2	813.2
320 2350103500	MOHAWK BROOK NEAR LANE, VT.	88	3.21	571.4	48 08	78 24	89	4.34	1.38	1888	7.28	11.58	0.0	218.2	188.2	1884.5	813.2	813.2
321 2350103500	NORTH BR. YANING BROOK NEAR RANCHSTER, ME.	88	0.33	178.8	48 21	68 51	82	4.33	1.33	338	5.88	1.83	0.0	82.5	72.4	135.1	135.1	195.8
322 2350103500	FOUR POND BROOK NEAR HOUGHTON, ME.	88	3.41	328.9	44 59	78 42	84	4.78	1.48	588	1.88	4.25	14.0	138.2	128.2	392.9	485.2	340.8
323 2350103500	PATTE BROOK NEAR BETHEL, ME.	88	0.38	635.7	44 51	78 42	84	4.78	1.48	588	1.88	4.25	14.0	138.2	128.2	392.9	485.2	340.8
324 2350103500	ELLIS R NEAR JACKSON, N. H.	88	10.98	4683.4	44 13	71 15	84	4.33	1.58	4788	2.68	12.48	0.0	1788.8	1933.8	9887.8	6333.5	4888.8
325 2350103500	MOHAWK BROOK NEAR CENTER STAFFORD, N. H.	88	0.37	638.8	43 18	71 06	92	4.74	1.71	738	4.88	19.88	16.1	251.1	381.3	989.8	1834.5	888.8
326 2350103500	ROCK BROOK NEAR PITTSBURG, N. H.	88	0.36	438.1	43 08	71 12	86	1.43	0.88	488	4.88	4.18	0.0	879.3	387.2	688.5	978.3	441.8
327 2350103500	MOHAWK R AT MOOSENECK R.1	88	0.33	382.1	41 38	71 38	188	6.48	2.68	448	5.88	16.05	1.5	288.5	227.2	423.3	565.9	318.8
328 2350103500	PHERRIS RIVER TRIO, NEAR ISLAND POND, VT.	88	1.88	148.6	44 38	71 04	94	4.44	1.53	1188	1.88	3.28	0.0	92.2	58.5	188.2	219.1	148.8
329 2350103500	KENT BROOK NEAR SHELBURNE, VT.	88	3.31	1813.4	43 48	72 48	98	4.81	1.53	3178	3.28	6.75	0.0	392.2	285.4	582.7	3189.4	1818.8
330 2350103500	SHACKETS BROOK NEAR PUTNEY, VT.	88	10.88	878.1	43 08	72 32	97	4.77	1.74	1268	6.88	19.65	0.0	334.5	381.3	1583.4	1843.4	1888.8
331 2350103500	FLOOD BROOK NEAR LONGDOCKERY, VT.	88	0.36	1897.6	43 14	72 51	95	4.71	1.99	3924	6.95	29.45	0.0	817.6	481.8	1889.1	1148.8	1888.8
332 2350103500	BEAVER BROOK AT WILMINGTON, VT.	88	0.38	1846.7	43 02	72 51	97	4.81	1.78	387	5.88	11.88	0.0	917.6	421.9	1889.1	1148.8	1888.8
333 2350103500	JAIL BRANCH AT EAST BARRE, VT.	88	48.48	1147.8	44 16	72 27	85	4.35	1.36	1324	6.88	33.88	8.83	587.9	421.9	2388.8	2826.1	1826.8
334 2350103500	STON BROOK NEAR EDEA, VT.	88	0.21	738.2	44 42	72 38	85	4.38	1.08	1488	3.28	6.78	0.0	339.9	352.7	1439.6	1727.5	988.8
335 2350103500	CINON BROOK NEAR EDEA, VT.	88	0.21	738.2	44 42	72 38	85	4.38	1.08	1488	3.28	6.78	0.0	339.9	352.7	1439.6	1727.5	988.8

Zone 10

330 2350103500	WARM BROOK AT LENOX, MASS.	88	2.18	148.8	42 21	73 18	185	6.88	1.78	593	2.93	3.64	10.0	89.3	92.0	287.2	334.0	158.8
331 2350103500	ORY BROOK NEAR ADAMS, MASS.	88	7.83	868.3	42 39	73 07	118	5.82	1.73	1927	4.47	9.16	0.0	594.8	823.7	1217.7	1366.8	984.8
332 2350103500	MOOSE RIVER AT ADAMS, MASS.	88	48.38	2598.2	42 37	73 08	118	5.48	1.73	1188	14.18	84.84	1.0	1287.6	1193.8	4896.6	5943.8	5888.8
333 2350103500	NORTH BRANCH MOOSE R AT NORTH ADAMS, MASS.	88	30.88	8778.8	42 38	73 08	128	6.71	1.93	2268	4.38	16.73	0.0	3788.2	2898.8	11885.8	13448.8	9868.8
334 2350103500	GREEN RIVER AT WILLIAMSTOWN, MASS.	88	48.98	2486.1	42 43	73 18	185	5.48	1.78	3872	11.07	43.12	0.0	1398.7	1320.9	5838.8	3888.2	272.8
335 2350103500	SOUTH STREAM NEAR BENNINGTON, VT.	88	7.78	141.8	42 08	73 19	85	4.81	1.78	1988	4.88	14.88	0.0	774.9	74.8	237.8	388.4	188.8
336 2350103500	METACUE RIVER TRIO, NO. 2 AT EAST RUPERT, VT.	88	1.98	138.9	43 16	73 07	93	4.79	1.79	1188	2.88	4.18	0.0	761.3	96.9	181.8	176.3	138.8
337 2350103500	WEST BRANCH SACANDAGA RIVER AT ARRIETTA, N. Y.	88	8.98	148.8	43 18	73 13	83	6.18	1.68	588	4.88	11.88	1.0	1193.2	137.4	288.8	288.8	1688.8
338 2350103500	SAND LANE OUTLET NEAR RICE, N. Y.	88	1.98	148.8	43 18	73 13	83	6.18	1.68	588	4.88	11.88	1.0	1193.2	137.4	288.8	288.8	1688.8
339 2350103500	LITTLE Sucker BROOK AT WADSWORTH, N. Y.	88	18.38	3418.8	44 58	73 11	75	4.23	1.44	128	7.88	11.38	0.0	1187.7	871.2	6824.5	4258.8	459.8
340 2350103500	EAST BRANCH AT REEDS RIVER NEAR NECHAN LAKE, N. Y.	88	48.48	538.3	44 53	73 18	77	4.74	1.48	1758	14.22	68.98	8.2	333.5	333.5	1817.8	1193.8	762.8
341 2350103500	LAKE BROOK AT REEDS RIVER NEAR NECHAN LAKE, N. Y.	88	18.48	519.7	44 47	73 48	88	4.83	1.98	1518	18.48	83.98	0.0	582.8	582.8	1131.5	1387.4	1888.8
342 2350103500	BOULET RIVER AT NEW RUSSETT, N. Y.	88	32.98	3383.3	44 18	73 37	83	4.33	1.98	2388	18.48	83.98	0.0	1693.9	1643.1	5788.3	6797.8	488.8
343 2350103500	NORTHWEST BAY BROOK NEAR BOLTON LANDING, N. Y.	88	89.48	1884.7	43 48	73 38	87	5.13	1.88	1288	5.48	94.88	0.0	872.9	1148.8	1884.7	1731.8	1888.8

## Zone 10

330 2350107388	WARM BROOK AT LENOX, MASS.	DE	2.18	148.8	42 21	73 18	195	6.88	1.78	593	2.93	3.64	10.0	89.3	92.0	287.2	334.0	158.8
337 2350133148	ORY BROOK NR ADAMS, MASS.	DE	7.83	868.3	42 39	73 07	118	5.82	1.73	1927	4.47	9.16	0.0	594.8	823.7	1217.7	1366.8	884.8
338 2350133168	MOOSE RIVER AT ADAMS, MASS.	DE	48.38	2598.2	42 37	73 08	118	5.48	1.73	1188	14.18	84.84	1.0	1287.6	1193.8	4896.6	5943.8	588.8
339 2350133268	NORTH BRANCH MOOSE R AT NORTH ADAMS, MASS.	DE	30.88	8778.8	42 38	73 08	128	6.71	1.93	2268	4.38	16.73	0.0	3788.2	2898.8	11885.8	13448.8	896.8
340 2350133368	GREEN RIVER AT WILLIAMSTOWN, MASS.	DE	48.98	2486.1	42 43	73 12	195	5.48	1.78	3872	11.07	43.12	0.0	1398.7	1380.9	5838.8	3888.2	278.8
341 2350133388	SOUTH STREAM NR BENNINGTON, VT.	DE	7.78	141.8	42 08	73 19	85	4.81	1.78	1988	4.88	14.88	0.0	77.9	74.8	237.8	388.4	168.8
342 2350420888	METACUE RIVER TRIO, NO. 2 AT EAST RUPERT, VT.	DE	1.98	138.9	43 16	73 07	93	4.79	1.79	1188	2.88	4.18	0.0	76.3	96.9	181.8	176.3	138.8
343 2350131988	WEST BRANCH SACANDAGA RIVER AT ARRIETTA, N. Y.	18	8.98	148.8	43 18	74 31	83	6.22	1.88	2788	9.88	93.88	0.0	1195.3	1374.8	2488.8	2833.8	1688.8
344 2350131998	WANO LAKE OUTLET NR RISCO, N. Y.	18	7.18	2884.4	42 28	74 33	84	6.18	1.88	298	4.98	11.98	1.0	588.8	388.8	388.8	488.8	887.8
345 2350420488	LITTLE BUCKER BROOK AT WADSWORTH, N. Y.	18	18.88	3412.8	44 58	76 11	75	4.83	1.44	128	7.88	15.38	0.0	1116.7	871.2	682.1	8334.3	4458.8
346 2350420888	EAST BRANCH ST. REGIS RIVER NR MECHAN LAKE, N. Y.	18	48.48	358.3	44 33	74 16	77	4.74	1.48	1758	14.22	69.88	3.2	333.8	333.5	4817.2	1193.8	762.8
347 2350427488	BLACK BROOK AT BLACK BROOK, N. Y.	18	49.48	1818.7	44 18	73 43	88	4.88	1.98	2318	18.48	69.88	8.8	588.8	588.8	1193.8	1287.4	1058.8
348 2350427688	BOULEVARD RIVER AT NEW ARADIA, N. Y.	18	37.88	3883.3	44 18	73 37	83	4.83	1.88	2368	18.48	63.88	0.0	1683.8	1843.1	578.3	6787.8	4488.8
349 2350427838	NORTHHEAT BAY BROOK NR BOLTON LANDING, N. Y.	18	83.48	1824.7	43 48	73 38	87	5.13	1.88	1288	9.48	84.88	0.0	972.8	1148.8	1824.7	1731.8	1888.8

Table 10. Continued.

## Zone 11

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
REG STATION	STATION NAME	RECT AREA	D18	LAT	LONG	R	P18	P00	OM	L	LL	STAD	ONEAN	OR	Q50	D100	ONEAK
350 2103304500	MCCELLIS CREEK NEAR MCINNEY, KY.	114 21.14	11336.9	37 28	84 42	175	6.00	2.80	35	1.43	4.70	0.8	585.3	2611.1	2495.4	1480.8	
351 2103305000	GREEN RIVER NEAR MCINNEY, KY.	114 22.40	11335.4	37 28	84 43	188	6.01	2.80	35	1.43	4.70	0.8	422.0	3040.8	1888.7	2343.4	1370.8
352 2103305500	GREEN RIVER NEAR MOUNT SALEM, KY.	114 30.39	11611.4	37 26	84 45	178	6.00	2.80	785	0.48	62.30	0.8	5340.5	5062.0	18418.7	22735.4	13728.8
353 2103306000	SOUTH FORK LITTLE SARREN R AT EDMONTON, KY.	114 18.30	11331.5	36 56	84 33	188	6.07	2.12	281	0.45	30.32	0.82	1083.8	1853.8	3078.1	5324.1	2978.0
354 2103306500	MCDUGAL CREEK NEAR MOOREN VILLE, KY.	114 6.34	11316.9	37 32	85 48	185	6.02	2.80	122	4.86	7.40	0.8	111.8	1141.8	3336.4	3936.2	2890.8
355 2103310000	NORTH FORK NOLIN RIVER AT MOOREN VILLE, KY.	114 31.48	11488.4	37 39	86 44	175	6.07	2.80	288	0.48	61.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
356 2103311000	BEAR CREEK NEAR LEITCHFIELD, KY.	114 38.58	11419.5	37 26	86 17	188	6.08	2.10	218	0.48	26.50	0.8	4730.8	4730.8	9783.8	11889.7	8870.8
357 2103313000	WEST BAY FORK AT SCOTTSVILLE, KY.	114 7.47	11332.2	36 46	86 12	281	6.03	2.14	288	4.83	11.90	0.8	1037.9	1598.5	2839.8	2889.7	7889.8
358 4793060000	810 BIGBY CREEK AT SMOY MOOD, TENN.	114 17.38	11321.2	35 20	87 38	185	6.00	2.30	321	0.48	10.41	0.8	1597.8	1533.9	3131.7	3387.4	2688.8
359 2103305000	CAYE CREEK NEAR DRY SPRING, KY.	114 21.93	11321.2	35 20	87 38	185	6.00	2.30	321	0.48	10.41	0.8	1597.8	1533.9	3131.7	3387.4	2688.8
360 2103305000	SOUTH ELKMAN CREEK AT FORT SPRING, KY.	114 24.99	11321.2	35 20	87 38	185	6.00	2.30	321	0.48	10.41	0.8	1597.8	1533.9	3131.7	3387.4	2688.8
361 2103305000	FLAT CREEK NEAR FORT, KY.	114 25.93	11321.2	35 20	87 38	185	6.00	2.30	321	0.48	10.41	0.8	1597.8	1533.9	3131.7	3387.4	2688.8
362 2103305000	816 EAGLE CREEK AT BAGGINSVILLE, KY.	114 48.98	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
363 2103305000	SOUTH FORK BEAR CREEK AT LOUISVILLE, KY.	114 17.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
364 2103305000	H P BEAR CREEK C, CANNON LA, LOUISVILLE, KY.	114 18.98	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
365 2103305000	SALT RIVER NEAR HARRISBURG, KY.	114 41.48	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
366 2103305000	PLUM CREEK SUBWATERED NO4 NEAR SIMPSONVILLE, KY.	114 1.88	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
367 2103305000	PLUM CREEK NEAR WILSONVILLE, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
368 4793480000	SPENCER CREEK NEAR LEBANON, TENN.	114 3.38	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
369 4793480000	COAL C TRIBUTARY AT GREEN HILL, TENN.	114 6.89	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
370 4793480000	CHICK CREEK ABOVE HENDERSONVILLE, TENN.	114 17.38	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
371 4793480000	BRADLEY CREEK AT LACAS, TENN.	114 37.88	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
372 4793480000	WARTAGE CREEK AT BELL, TENN.	114 10.38	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
373 4793480000	WARTAGE CREEK AT BELL, TENN.	114 10.38	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
374 4793480000	POND CREEK NEAR WILSON OAK, ALA. GAGE NO. 1	114 13.78	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
375 4793480000	POND CREEK NEAR WILSON OAK, ALA. GAGE NO. 2	114 13.78	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
376 4793480000	POND CREEK NEAR WILSON OAK, ALA. GAGE NO. 3	114 13.78	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
377 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
378 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
379 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
380 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
381 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
382 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
383 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
384 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
385 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
386 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
387 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
388 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
389 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
390 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
391 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
392 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
393 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
394 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
395 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
396 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
397 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
398 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
399 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6
400 4793480000	ROCK LICK CREEK NEAR GLEN OAK, KY.	114 18.19	11344.3	38 23	84 33	188	6.07	2.80	283	0.48	62.30	0.8	4897.4	4079.2	18944.9	12188.3	9388.6

## Zone 12

3382 1785630000	TINLEY CREEK NEAR PALOS PARK, ILL.	124 11.30	9717.7	41 38	87 46	123	6.88	1.84	178	7.56	18.64	0.0	511.4	486.9	1534.2	1789.0	1278.8
3383 1785637500	LONG RUN NEAR LEHONY, ILL.	124 20.00	13573.3	41 39	88 00	124	6.33	1.98	71	8.68	30.90	0.0	748.8	589.4	3945.3	4391.8	3189.8
3384 1785639000	BOONE CREEK NEAR MCINNEY, ILL.	124 10.30	8433.8	41 39	88 10	118	6.33	1.93	110	7.68	19.08	0.0	138.5	183.4	313.0	347.6	279.0
3385 2064480000	BLACK RIVER NEAR GARNETT, MICH.	124 20.00	9827.7	46 97	80 22	65	4.50	1.48	388	6.38	18.14	3.0	388.8	246.7	1837.0	1269.2	908.8
3386 2064481100	BLACK RIVER NEAR GARNETT, MICH.	124 19.30	9173.8	46 98	84 19	88	6.00	1.92	61	6.32	20.96	0.0	296.8	296.8	1893.1	1197.2	748.0
3387 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3388 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3389 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3390 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3391 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3392 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3393 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3394 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3395 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3396 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3397 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3398 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000	BLACK RIVER NEAR WILLIAMSTON, MICH.	124 10.30	9547.2	41 41	84 32	82	6.04	1.92	56	4.38	3.71	0.0	711.6	895.3	1853.8	1435.3	943.0
3399 2064413000																	





Table 10. Continued.

Zone 13

REG	STATION	STATION NAME	SECT	AREA	Q10	LAT	LONG	R	P10	P60	DN	L	LL	STRO	MEAN	Q2	Q50	Q100	PEAK
451	308314500	OTTER FORK NEAR CENTERBURG, OHIO	120	3.17	319.3	48 19	82 43	115	5.09	1.83	85	3.38	7.77	1.7	153.9	132.0	354.1	946.2	445.8
452	308320000	OTOOTO BIG RUN AT BRIDGEVILLE, OHIO	120	11.98	327.8	39 38	83 57	125	5.38	1.97	172	8.76	16.19	0.5	1281.1	1208.3	3081.1	4183.8	2088.8
453	308321000	EAST FORK PAINTY CREEK NEAR BEOLA, OHIO	180	3.82	408.8	39 42	83 39	148	6.08	1.91	38	2.68	6.31	0.8	219.8	224.7	703.5	821.1	513.8
454	308323400	INDIAN CREEK AT HARRISVILLE, OHIO	180	9.08	431.6	39 14	82 59	148	6.38	1.92	448	5.48	48.61	0.2	1726.4	1208.5	6985.9	7988.9	5648.8
455	308325000	SALT CREEK AT TAYLTON, OHIO	120	11.88	441.8	39 36	82 47	149	5.08	1.89	215	7.48	34.58	0.82	1776.7	1247.7	9594.2	11726.2	10288.8
456	308324100	SHANNEY CREEK AT XENIA, OHIO	150	4.81	894.1	39 48	83 33	148	6.88	1.92	126	4.88	8.64	0.1	618.8	632.3	1186.8	1317.8	1088.8
457	308324700	PATERSON RUN NR OHNSVILLE OHIO	150	3.34	846.8	39 37	84 07	145	6.82	1.99	98	4.38	7.88	0.2	598.9	598.9	1834.1	1134.8	859.8
458	308325700	BRIDGE CREEK NEAR GREENVILLE, OHIO	180	4.83	868.8	40 03	84 38	158	6.88	1.92	87	3.50	3.87	0.94	488.4	475.8	825.8	918.8	784.8
459	308326000	MAD RIVER AT ZANESVILLE, OHIO	150	7.31	1883.1	40 22	83 41	188	5.88	1.87	276	4.78	84.89	0.1	588.3	588.2	1889.4	1983.1	1388.8
460	308327100	BLAKE RUN NEAR REELT, OHIO	120	9.29	173.2	39 28	84 48	188	6.18	1.89	180	1.58	1.58	0.2	88.8	84.8	317.8	377.8	307.8
461	308328100	YACHT CREEK NEAR LEBANON, OHIO	120	4.06	371.8	40 48	84 38	188	6.88	1.93	45	3.58	11.11	0.82	289.8	217.8	542.8	526.8	488.8
462	308328100	ROLLER CREEK AT OHIO CITY, OHIO	120	6.14	371.8	40 48	84 38	188	6.88	1.93	45	3.58	11.11	0.82	289.8	217.8	542.8	526.8	488.8
463	308328100	NORMAL CREEK NEAR NORMAL, OHIO	120	4.82	872.6	39 35	83 37	92	5.37	1.78	75	4.48	12.68	0.8	442.1	488.7	3318.8	2898.8	1888.8
464	308328100	RAPID CREEK BELOW MORRIS, IOWA	132	8.12	2844.7	41 41	91 58	188	7.81	2.21	81	4.55	17.82	0.8	786.7	758.8	3788.8	4488.8	3888.8
465	198483700	RAPID CR TRI NO. 4 NR OASIS, IOWA	132	1.05	691.9	41 43	91 55	188	7.88	2.21	88	1.97	4.89	0.8	279.8	870.8	1283.4	1534.8	988.8
466	198483700	RAPID CREEK SOUTHWEST OF MORRIS, IOWA	132	15.26	3835.8	41 43	91 58	188	7.88	2.21	188	6.20	32.88	0.8	1378.8	1368.8	5448.8	8488.8	4388.8
468	198483500	RAPID CREEK TRIBUTARY NO. 3 NR OASIS, IOWA	132	1.82	3881.7	41 42	91 57	188	7.81	2.21	88	1.82	3.34	0.8	994.1	995.0	6648.1	6888.8	8888.8
469	198484000	RAPID CREEK NEAR TOWA CITY, IOWA	132	20.38	4488.8	41 41	91 58	188	7.88	2.21	143	18.88	66.88	0.8	2188.8	2149.3	8584.2	7888.7	9188.8
470	198484000	RALSTON CREEK AT TOWA CITY, IOWA	132	11.88	441.8	39 36	82 47	149	5.08	1.89	215	7.48	34.58	0.82	1776.7	1247.7	9594.2	11726.2	10288.8
471	198484300	SPR ENGLISH R TRIS NR JAMES CITY, IOWA	132	2.81	815.3	41 33	92 28	188	7.82	2.29	85	2.28	9.69	0.8	438.8	438.9	994.1	1881.3	988.8
472	198484300	WOLF CREEK NEAR HUNTER, IOWA	132	7.98	2865.5	41 18	92 38	188	7.88	2.21	237	8.12	18.88	0.8	924.8	974.8	3631.8	3289.8	3898.8
473	198484300	WOLF CREEK NEAR HUNTER, IOWA	132	16.88	1824.8	41 30	92 38	188	7.88	2.21	178	11.28	48.38	0.8	886.8	886.8	1882.2	3388.8	1258.8
474	208508000	LITTLE DELAWARE RIVER NEAR HORTON, KANS.	132	2.60	1888.8	39 45	91 53	88	6.37	2.25	178	11.28	48.38	0.8	886.8	886.8	1882.2	3388.8	1258.8
475	208508000	DOUGLAS CREEK NEAR EMERY, MO.	132	2.60	1888.8	39 45	91 53	88	6.37	2.25	178	11.28	48.38	0.8	886.8	886.8	1882.2	3388.8	1258.8
476	208508000	WHITE CLOUD CREEK NEAR MARVILLE, MO.	132	2.60	1888.8	39 45	91 53	88	6.37	2.25	178	11.28	48.38	0.8	886.8	886.8	1882.2	3388.8	1258.8
477	208508000	WHITE CLOUD CREEK NEAR MARVILLE, MO.	132	2.60	1888.8	39 45	91 53	88	6.37	2.25	178	11.28	48.38	0.8	886.8	886.8	1882.2	3388.8	1258.8
478	208508000	GIO SLOUGH NEAR WILCOX, MO.	132	1.36	1813.6	40 23	94 36	178	7.08	2.33	82	1.68	1.88	0.8	537.9	527.9	1073.4	1883.8	1488.8
479	208508000	E PR FISHING R AT EXCELSIOR SPRINGS, MO.	132	8.88	7313.8	39 28	94 13	188	6.88	2.33	888	11.81	68.57	0.8	3324.1	2792.2	1883.8	28118.8	18888.8
480	208508000	RUMBO BRANCH AT DANVILLE, MO.	132	1.48	648.8	39 50	91 32	285	7.44	2.35	85	2.84	2.64	0.8	278.1	289.8	1236.9	1457.8	1699.8
481	318608000	SOUTH OMAHA CR TRIBUTARY NEAR WALTHILL, NEBR.	132	2.64	1168.4	42 06	96 28	138	7.73	2.43	171	2.38	8.93	0.8	522.9	475.8	1781.7	2813.2	1418.8
482	318608000	SOUTH OMAHA CREEK NEAR WALTHILL, NEBR.	132	15.18	7931.8	42 07	96 28	138	7.73	2.43	222	6.18	33.89	0.8	2476.7	1836.4	1836.8	15481.8	18188.8
483	318608000	SOUTH BR TEKANAN CR NR CRAIG, NEBR.	132	2.64	1788.8	41 58	96 28	138	7.78	2.43	164	2.33	11.84	0.8	522.2	786.4	3099.8	4389.8	2888.8
484	318608000	JOHN BRANCH TRIBUTARY NO. 1 NEAR WALTHILL, NEBR.	132	4.98	2853.9	41 58	96 28	138	7.78	2.43	260	6.18	33.89	0.8	1819.8	1819.8	3732.4	5189.8	5888.8
485	318608000	JOHN BRANCH TRIBUTARY NO. 2 NEAR WALTHILL, NEBR.	132	2.78	2744.1	41 58	96 28	138	7.78	2.43	260	6.18	33.89	0.8	1819.8	1819.8	3732.4	5189.8	5888.8
486	318608000	TEKANAN CREEK AT TEKANAN, NEBR.	132	25.88	4783.8	41 48	96 13	138	7.88	2.48	388	11.32	188.87	0.8	2391.9	2487.6	6038.8	7737.7	9188.8
487	318608000	NEW YORK CREEK TRIBUTARY NEAR SPIKER, NEBR.	132	1.88	982.8	41 48	96 17	135	7.81	2.48	137	2.31	8.32	0.8	441.1	432.3	1985.9	1764.4	1588.8
488	318608000	NEW YORK CREEK NORTH OF SPIKER, NEBR.	132	8.88	4838.8	41 48	96 17	135	7.81	2.48	188	4.82	31.78	0.8	1381.8	1113.3	7893.8	9393.3	3828.8
489	318608000	NEW YORK CREEK EAST OF SPIKER, NEBR.	132	13.98	6871.2	41 48	96 17	135	7.81	2.48	838	8.88	68.48	0.8	1743.6	1134.2	11517.8	14847.2	9898.8
490	318608000	STONE CREEK AT ELKHORN, NEBR.	132	18.38	6382.8	40 45	96 15	138	7.88	2.81	111	5.88	21.92	0.8	1817.5	1475.2	11882.8	14436.4	9888.8
491	318608000	MOORE CREEK TRIBUTARY NEAR PALMYRA, NEBR.	132	8.88	3388.8	40 48	96 22	133	7.88	2.81	188	6.78	48.38	0.8	1825.1	922.8	5338.8	9888.7	4818.8
492	318608000	LITTLE NEHAMA R TRIS NR SYRACUSE, NEBR.	132	9.71	7981.1	40 38	96 18	138	8.88	2.82	118	1.88	1.88	0.8	389.8	377.8	1484.1	1788.8	1288.8
493	488647800	SADLEROCK CREEK NEAR CANTON, S. DAK.	132	14.88	7342.8	43 18	98 44	88	8.42	1.88	199	15.32	48.37	0.8	322.3	88.8	1848.8	1157.8	948.8
494	488647800	SADLEROCK CREEK TRI NEAR GRENESBORO, S. DAK.	132	2.38	624.3	43 18	98 44	88	8.42	1.88	199	15.32	48.37	0.8	322.3	88.8	1848.8	1157.8	948.8
495	488648000	LITTLE BEAVER CR TRI NEAR CANTON, S. DAK.	132	9.22	7449.3	43 18	98 38	138	8.17	1.88	188	8.17	8.17	0.8	31.2	25.8	124.8	148.8	18.8
496	208508000	HOLL CREEK NEAR GREEN, KANS.	132	3.98	888.9	39 23	97 08	135	8.17	2.87	188	3.37	7.17	0.8	634.4	330.7	3207.9	2728.4	1788.8
497	208508000	E BR 8 PK BLACKWATER R NR ELMO, MO.	132	18.68	9717.7	38 07	84 82	188	6.88	2.84	82	8.18	48.33	0.8	9723.7	2386.8	9713.8	1819.8	7888.8
498	208508000	SNAILO BRANCH NEAR MARSHALL, MO.	132	2.87	986.3	38 07	84 82	188	6.88	2.84	82	8.18	48.33	0.8	9723.7	2386.8	9713.8	1819.8	7888.8
499	488718300	COUNCIL CREEK NEAR STILLWATER OKLA.	132	31.98	18888.8	38 08	98 64	481	8.02	2.76	248	9.88	188.18	0.81	4891.8	2358.8	27758.8	35958.8	28988.8
500	488732500	SADDLEROCK CREEK NEAR SANDSTONE CR NR ELK CITY, OKLA.	132	8.88	1884.8	38 31	99 38	188	8.24	2.86	871	10.88	63.319	1.8	982.2	943.1	1381.8	1487.4	1138.8
501	4888 35.1	SOUTH OKLAHOMA WATERHED NO 1	132	9.88	84.8	38 47	97 26	218	9.58	2.71	25	8.88	8.88	0.8	48.2	48.2	139.8	154.7	107.8
502	4888 35.3	SOUTH OKLAHOMA WATERHED NO 2	132	9.88	84.8	38 47	97 26	218	9.58	2.71	25	8.88	8.88	0.8	48.2	48.2	139.8	154.7	107.8
503	4888 35.4	SOUTH OKLAHOMA WATERHED NO 3	132	9.88	84.8	38 47	97 26	218	9.58	2.71	25	8.88	8.88	0.8	48.2	48.2	139.8	154.7	107.8
504	4888 37.2	STILLWATER OKLAHOMA WATERHED NO 3	132	9.144	415.8	38 23	97 14	218	9.58	2.71	74	8.88	1.884	21.8	118.8	88.1	810.7	888.1	439.7



Table 10. Continued.

Zone 13																
STATION	STATION NAME	BEY AREA	Q10	LAT	LONG	R	P10	P80	Q10	L	LL	Q100	Q10	Q50	Q100	Q100
595 4848 37.3	STILLWATER OLANHWA WATERSHED NO 4	187 9.81	382.1	38 33	97 14	810	8.80	3.71	259	9.47	10.34	1.4	186.7	166.7	607.4	604.5
596 388810300	WEST BUCKEYE CREEK NEAR BILLINGS, MONT.	138 1.84	488.4	38 190	84	310	8.80	3.71	259	9.47	10.34	1.4	186.7	166.7	607.4	604.5
597 388830000	BARTON CREEK TRI NEAR VOLSBO, MONT.	138 8.14	177.5	48 83	186 48	28	3.46	1.11	188	1.30	2.10	0.8	63.8	21.5	463.5	588.3
598 388830000	BARTON CREEK NEAR VOLSBO, MONT.	138 16.80	683.1	48 33	186 39	36	3.46	1.11	188	1.30	2.10	0.8	63.8	21.5	463.5	588.3
599 388830000	SHEEP CR TRI NEAR MEDORA, N. DAK.	138 6.80	67.5	48 54	183 27	31	6.38	1.07	389	6.99	2.38	0.8	38.7	28.6	184.2	197.6
518 388830000	SHEEP CR TRI NEAR MEDORA, N. DAK.	138 9.48	186.7	48 56	183 28	31	5.20	1.07	115	1.18	1.16	0.8	74.1	58.3	289.8	334.2
511 388830000	HEART RIVER TRI NEAR DICKINSON, N. DAK.	138 1.78	78.8	48 58	182 47	36	5.30	1.79	88	5.78	6.38	0.8	29.2	21.8	126.5	159.1
512 388830000	CLARK CREEK TRI NEAR DICKINSON, N. DAK.	138 14.89	838.8	48 43	182 22	88	5.82	1.84	183	11.31	30.41	0.8	1812.5	1812.5	3847.8	4455.8
513 488840000	PIE RIVER TRI NEAR INTERIOR, S. DAK.	138 9.14	476.2	48 43	181 57	84	5.81	1.87	329	6.48	6.49	0.8	212.6	162.6	744.1	666.9
514 888860000	PRATZIE DOB CR TRI NEAR HORTON KANS.	132 1.80	888.8	38 51	180 53	58	7.13	1.35	188	2.48	3.78	0.8	697.1	214.9	712.2	687.2
515 888860000	SPRING CREEK NEAR KANOPOLIS, KANS.	132 5.81	288.4	38 48	180 18	128	6.15	1.58	288	6.88	1.73	0.8	994.1	894.1	3976.4	4071.1
517 888870000	EAST LESTONE CREEK NEAR IONIA, KANS.	132 25.80	1871.4	38 48	180 28	118	7.88	2.51	388	9.28	28.78	0.8	878.4	858.8	4134.4	4917.8
518 2887148100	RATTLESNAKE CREEK TRI NEAR MULLINVILLE, KANS.	132 19.30	8481.6	37 35	88 25	119	7.87	2.51	315	6.88	13.88	0.8	1189.1	881.8	1648.1	1788.8
519 2887148500	SPRING CREEK NEAR OLLYNN, KANS.	132 14.38	3186.1	37 37	88 88	188	6.87	2.84	188	6.42	9.88	0.8	735.3	338.7	488.5	5944.8
520 2887149100	CHEYENNE CREEK TRI NEAR CLAPLIN, KANS.	132 1.48	248.8	38 27	98 32	119	8.87	2.84	78	1.43	2.88	0.8	138.8	174.1	373.1	458.4
521 3188 44.1	HASTINGS NEBRASKA WATERSHED N-3	132 9.78	788.8	48 21	88 24	85	7.72	2.43	82	1.88	3.88	0.8	313.3	253.8	1283.1	1378.8
522 8887181800	LION CREEK NEAR HALPWAY, COLO.	137 8.80	8.4	38 48	103 88	85	4.71	1.71	888	1.48	1.42	0.8	3.3	3.8	11.8	13.1
523 8887181800	SHEEP CREEK NEAR HALPWAY, COLO.	137 8.73	6.8	38 48	103 88	78	4.84	1.87	888	1.33	1.33	0.8	8.8	2.1	12.8	16.8
524 8887181800	WEST MONUMENT CREEK NEAR PIKEVIEW, COLO.	137 18.48	87.5	38 56	184 84	78	4.78	1.78	888	7.32	48.78	0.8	41.3	38.5	284.5	282.8
525 8887181800	TEMPLETON GAP FLOODWAY AT COLORADO SPRINGS, COLO.	137 8.48	838.8	38 54	184 28	68	4.87	1.81	188	7.41	28.38	0.8	788.4	488.3	488.8	5888.8
526 8887181800	RATON CR AT RATON, N. MEX.	138 14.48	2488.8	38 54	184 28	68	4.87	1.81	188	7.41	28.38	0.8	788.4	488.3	488.8	5888.8
527 8887181800	91X MILE CREEK NEAR EAGLE NEUT, N. MEX.	138 11.88	183.8	38 31	186 16	33	3.86	1.14	488	6.73	23.48	0.8	41.3	33.9	186.9	181.7
528 3887180000	CLEAR CREEK NEAR UTE PARK, N. MEX.	138 7.44	188.8	38 32	186 16	33	4.87	1.81	388	6.71	8.71	0.8	48.8	33.7	383.7	381.8
529 3887180000	CANADIAN RIVER TRI NEAR HILLS, N. MEX.	138 4.88	1388.4	38 18	184 16	88	4.88	1.82	1188	8.21	38.74	0.8	411.3	156.3	258.1	3882.5
530 3887180000	COVOTE CREEK NEAR BLACK LAKE, N. MEX.	138 48.88	488.8	38 18	185 16	78	4.28	1.43	1878	8.25	38.97	0.8	117.8	78.3	722.5	953.8
531 3887180000	DOG CREEK NEAR SHOCHAKER, N. MEX.	138 18.48	3888.8	38 56	184 33	73	4.86	1.82	1188	8.21	38.74	0.8	1388.8	1388.8	7188.1	8788.8
532 3887180000	PLAZA LANGA CR TRI NEAR RASLAND, N. MEX.	138 8.58	538.8	38 56	183 45	88	5.16	2.81	384	1.86	2.86	0.8	278.3	278.3	1642.4	1287.8
533 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
534 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
535 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
536 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
537 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
538 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
539 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
540 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
541 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
542 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
543 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
544 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
545 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
546 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
547 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
548 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
549 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
550 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
551 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
552 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
553 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
554 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
555 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
556 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
557 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
558 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
559 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
560 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
561 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
562 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
563 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
564 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
565 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
566 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
567 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
568 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
569 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
570 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78	4.43	1.46	888	6.51	9.51	0.8	48.3	34.9	284.8	383.8
571 3887180000	PECOS RIVER TRI, N. MEX.	138 8.18	184.8	38 53	183 36	78										

Table 10. Continued.

Zone 13

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

SECT	STATION	STATION NAME	SECT	AREA	D18	LAT	LONG	R	P18	P08	DM	L	LL	STRG	BREAN	Q2	Q50	Q100	QPEAK		
13A	438	DEEP CR SUBWATERED NO. 8 NR MERCURY, TEX.	13A	4.38	3871.9	31	16	99	88	165	8.4	8.71	908	8.99	18.58	0.01	992.1	498.0	9936.8	10028.2	5608.2
14A	1.18	COYLE BRANCH AT HOUSTON, MO.	14A	1.18	483.7	37	19	97	848	7.78	8.18	1.52	2.26	0.0	2.26	0.0	258.6	269.0	1018.4	1366.8	1035.8
14A	0.68	LITTLE BERGER CR TR18 NR MERRIMAN, MO.	14A	0.68	769.8	37	58	91	4	266	7.97	8.39	0.4	1.13	0.0	375.0	333.8	1288.2	1322.5	1068.0	
14A	1.85	SENHME BRANCH NEAR ROLLA, MO.	14A	1.85	907.8	37	55	81	48	288	7.97	2.39	180	8.21	8.94	0.3	486.1	405.9	1532.5	1624.4	1106.0
14A	81.36	BOURBON RIVER NEAR ST. JAMES, MO.	14A	81.36	8978.7	38	00	81	48	800	7.88	2.38	191	6.95	44.95	0.0	3373.1	3420.2	9386.7	10729.9	8258.0
14A	84.16	LANES FORK NR VACHY, MO.	14A	84.16	7368.8	38	00	91	43	880	7.86	2.38	233	9.65	53.84	0.0	3374.8	3078.5	11474.3	13338.5	9488.3
14A	3.88	STANLEY CREEK NEAR MILLER, MO.	14A	3.88	1480.2	37	12	93	31	218	8.36	9.08	180	2.64	5.28	0.0	789.7	873.7	1988.0	2225.0	1468.0
14B	1.28	DOD BRANCH AT ST. PAUL, ARK.	14B	1.28	881.8	35	08	93	48	245	8.48	2.68	888	1.80	3.69	0.0	286.3	314.9	1517.4	1889.8	950.0
14B	0.78	WEST FORK WHITE RIVER TR18, NR GREEN WOOD, ARK.	14B	0.78	1878.8	35	08	94	19	848	8.11	2.67	588	1.80	2.60	0.0	311.3	188.0	2888.0	2375.8	1366.0
14B	0.88	WAL FABLE CREEK NR LITTON, ARK.	14B	0.88	738.4	35	24	93	3	245	8.17	2.68	138	1.80	3.23	0.0	278.8	439.3	1618.0	1788.8	1328.0
14B	0.88	WAL FABLE CREEK NR LITTON, ARK.	14B	0.88	738.4	35	24	93	3	245	8.17	2.68	138	1.80	3.23	0.0	278.8	439.3	1618.0	1788.8	1328.0
14B	0.88	MAXWELL CREEK AT KINGSTON, ARK.	14B	0.88	738.4	35	24	93	3	245	8.17	2.68	138	1.80	3.23	0.0	278.8	439.3	1618.0	1788.8	1328.0
14B	1.88	OSAGE CREEK TR18, AT BERRYVILLE, ARK.	14B	1.88	374.3	35	20	93	34	225	8.39	2.33	588	1.80	8.99	0.0	194.8	174.6	545.1	611.1	486.2
14B	39.3	COVE CREEK NEAR LEE CREEK, ARK.	14B	39.3	1893.7	35	43	84	24	250	8.58	2.39	688	13.00	39.37	0.0	7612.3	4769.4	37886.4	48680.7	33639.2
15A	0.67	N. FORK WHITE OAK CREEK TR18, NR WATULA, ARK.	15A	0.67	568.0	35	36	93	31	250	8.28	0.7	180	0.50	0.90	0.0	280.6	167.7	692.0	1060.0	729.3
15A	3.91	SIX MILE CREEK SUBWATERED NO. 8 NR CHISHOLM, ARK.	15A	3.91	1788.8	35	12	93	33	265	8.68	8.78	180	4.50	6.58	0.0	983.4	1071.9	2271.5	2317.5	1938.0
15A	24.18	SIX MILE CREEK AT CHISHOLM, ARK.	15A	24.18	8978.1	35	12	93	36	265	8.37	2.78	273	9.38	29.50	0.0	1331.4	1971.0	3568.2	3867.6	2612.0
15A	36.78	SIX MILE CREEK NEAR BRANCH, ARK.	15A	36.78	4826.6	35	15	93	36	268	8.56	2.78	150	12.00	45.50	0.0	2515.9	3844.2	6038.2	6892.3	4548.2
15A	8.76	SIX MILE CR SUBWATERED NO. 5 NR CHISHOLM, ARK.	15A	8.76	988.0	35	14	93	35	268	8.57	2.78	235	3.07	3.67	0.0	445.0	458.4	1312.0	1346.4	1078.0
15A	9.81	SIX MILE CR SUBWATERED NO. 2 NR CAULKSHVILLE, ARK.	15A	9.81	1028.2	35	16	93	35	265	8.60	2.98	237	5.08	11.10	0.0	981.1	981.1	2739.3	3859.4	2476.2
15A	4.49	SIX MILE CR SUBWATERED NO. 83 NR BRANCH, ARK.	15A	4.49	816.8	35	21	93	39	235	8.57	2.78	58	2.88	15.94	0.0	597.3	689.9	3484.9	3876.9	2532.0
15B	1.28	MURKIN CREEK NEAR BRANCH, ARK.	15B	1.28	2360.8	35	21	93	36	268	8.58	2.78	480	9.08	66.11	0.0	346.3	795.1	2195.8	2514.5	2846.0
15B	3.19	CHICKASAW CREEK TR18, NR STRINGTOWN, OKLA.	15B	3.19	8667.8	34	36	93	37	288	8.68	2.38	388	9.58	42.98	0.0	842.4	931.8	2393.8	2838.0	2038.0
15B	30.78	CHICKASAW CREEK NEAR STRINGTOWN, OKLA.	15B	30.78	4833.7	34	36	93	38	293	9.18	2.46	460	9.40	32.30	0.0	936.6	936.6	2868.0	3024.8	1888.0
15B	0.68	WHEE CREEK NEAR STRINGTOWN, OKLA.	15B	0.68	9487.3	34	38	93	38	293	9.08	2.46	477	10.20	68.97	0.0	6697.1	7267.1	11897.4	12880.8	10288.0
15B	1.88	KIAHICHI RIVER TR18, NR ALBION, OKLA.	15B	1.88	1334.4	34	37	93	38	285	8.91	2.48	658	2.88	4.10	0.0	395.6	237.6	1462.4	1686.6	1078.0
15B	1.98	SIX BRANCH NR KINGDOLE, OKLA.	15B	1.98	1397.0	34	10	95	85	380	8.99	3.82	220	1.80	3.20	0.0	544.6	499.1	1287.3	2723.0	1468.0
15B	0.98	MOUNTAIN FORK TR18, NR BRITHVILLE, OKLA.	15B	0.98	529.0	34	36	94	40	268	8.65	2.78	380	2.88	4.10	0.0	220.4	233.6	793.4	908.1	508.0

Zone 14

13A	14.88	BEAVER CREEK NEAR ROLLA, MO.	13A	14.88	3892.8	37	51	91	46	285	7.59	2.39	318	6.99	46.16	0.1	2147.0	2254.4	3754.0	6546.4	5880.0
13A	6.41	LITTLE BEAVER CREEK NEAR ROLLA, MO.	13A	6.41	5988.8	37	57	91	49	280	7.59	2.41	122	2.58	13.54	0.1	180.3	370.0	730.0	720.0	720.0
13A	6.41	BLAKE CREEK NEAR ROLLA, MO.	13A	6.41	5988.8	37	57	91	49	280	7.59	2.41	122	2.58	13.54	0.1	180.3	370.0	730.0	720.0	720.0
13A	1.62	MARSH RIVER TRIBUTARY AT LOMA, MONT.	13A	1.62	1781.2	47	37	116	38	28	3.28	1.03	488	2.37	4.37	0.0	83.9	20.3	34.5	412.3	388.0
13A	0.95	MARTAS R TRIBUTARY NO. 2 AT LOMA, MONT.	13A	0.95	39.7	47	44	118	15	25	3.15	1.08	239	1.24	1.24	0.0	9.9	5.9	98.1	66.3	42.0
13A	17.88	SPRING COULEE NEAR HAYNE, MONT.	13A	17.88	385.4	40	25	180	15	21	3.48	1.09	1145	8.08	36.90	0.0	122.8	62.6	583.5	577.2	345.0
13A	13.68	SQUIRE BUTTE C TR1 NO. 2 NR CENTER, N. DAK.	13A	13.68	1214.7	47	07	181	15	43	5.48	1.73	408	4.90	8.437	8.8	484.9	222.7	2631.0	3320.2	2508.0
13A	21.30	JAMES RIVER TRI NO. 3 NEAR HAYNE, N. DAK.	13A	21.30	181.5	47	39	99	48	65	5.40	1.73	39	6.28	16.92	0.0	35.6	16.2	213.6	207.0	158.0
13A	14.68	MUSH CREEK NEAR PIERRE, S. DAK.	13A	14.68	4816.4	44	28	188	18	56	6.23	1.85	360	8.94	16.61	0.0	1382.9	1821.2	9297.4	9868.8	8888.0
13A	5.41	HAYTER CREEK TRI NEAR ORIENT, S. DAK.	13A	5.41	337.8	44	48	90	84	61	6.36	1.97	68	3.92	4.11	0.0	95.9	28.6	786.1	834.3	410.0
13D	4.98	SOUTH FORK BAPPA CREEK TR18 NR GOODLAND, KANS.	13D	4.98	2886.8	39	18	181	58	180	8.96	2.18	120	3.88	9.76	7.0	418.9	180.8	4790.0	5808.8	2888.0
13D	18.28	BEAVER CREEK TRIBUTARY NR LUDWELL, KANS.	13D	18.28	849.4	39	18	180	58	123	7.36	2.38	130	4.92	7.41	0.0	507.6	494.6	938.0	978.4	830.0
13D	7.53	PRAIRIE DOG CREEK TRIBUTARY AT COLBY KANS.	13D	7.53	841.3	39	23	181	83	127	7.31	2.30	65	4.73	16.32	0.0	286.4	169.3	1142.3	1322.6	882.0
13D	5.21	EAST BUFFALO CREEK NEAR BUFFALO, NEBR.	13D	5.21	1321.5	41	51	99	36	98	7.80	2.48	195	8.37	18.35	0.0	57.0	57.0	235.4	286.0	208.0
13D	17.18	WEST BUFFALO CREEK NEAR BUFFALO, NEBR.	13D	17.18	354.8	40	50	99	35	98	7.80	2.48	195	8.37	18.35	0.0	57.0	57.0	235.4	286.0	208.0
13D	6.68	ELM CREEK TRIBUTARY NEAR OVERTON, NEBR.	13D	6.68	1381.4	40	55	99	35	98	7.80	2.48	195	8.37	18.35	0.0	57.0	57.0	235.4	286.0	208.0
13D	4.08	ELM CREEK NEAR SUMNER, NEBR.	13D	4.08	484.8	40	52	99	37	95	7.71	2.44	168	8.64	31.78	0.0	189.9	88.4	917.9	1148.8	668.0
13D	6.68	ELM CREEK TRIBUTARY NO. 3 NEAR OVERTON, NEBR.	13D	6.68	1381.4	40	55	99	35	98	7.80	2.48	195	8.37	18.35	0.0	57.0	57.0	235.4	286.0	208.0
13D	8.82	WOOD RIVER TRIBUTARY NEAR LODI, NEBR.	13D	8.82	915.1	41	59	99	38	95	7.61	2.39	108	3.13	4.98	0.0	44.9	24.7	183.1	253.5	169.0
13D	18.98	WOOD RIVER NEAR LODI, NEBR.	13D	18.98	148.8	41	10	99	48	95	7.68	2.48	225	16.24	28.13	0.0	58.1	18.5	315.0	383.3	188.0
13D	8.82	LILLIAN CR TRIBUTARY NR BROWN, NEBR.	13D	8.82	1117.4	41	31	99	39	85	7.68	2.38	145	3.24	6.53	0.0	5.4	4.8	23.2	26.8	1.0
13D	4.77	LILLIAN CREEK NEAR BROWN, NEBR.	13D	4.77	816.4	41	31	99	39	85	7.68	2.38	145	3.24	6.53	0.0	281.5	211.1	1546.2	1861.6	936.0

Table 10. Continued.

1	Zone 14																		18
	STATION	STATION NAME	RECT AREA	910	LAT	LONG	R	P10	P60	DN	L	LL	STR	SHEAN	Q2	Q50	Q100	QPEAK	
910	3160727500	LILLIAN CREEK TRIBUTARY NEAR WALKHORN, NEBR.	130 9.84	386.2	41 37	89 34	85	7.88	8.33	880	3.23	5.22	0.8	12.3	38.6	941.7	1180.2	953.8	
911	3160732500	S BR HUD CR TRIS NR BROKEN BOW, NEBR.	130 6.48	231.8	41 86	89 43	85	7.84	8.37	877	1.24	1.24	0.8	12.3	38.6	941.7	1180.2	953.8	
912	3160732500	MUD CR TRIBUTARY NR BROKEN BOW, NEBR.	130 9.88	1178.8	41 88	89 38	85	7.88	8.40	816	8.97	10.11	0.8	29.7	148.3	1877.8	2582.3	1583.8	
913	3160732500	DAVIS CR TRIS NO. 2 NR NORTH LOUP, NEBR.	130 6.78	885.7	41 86	89 84	85	7.74	8.43	179	8.85	14.08	0.8	379.1	379.1	1421.8	1611.2	936.9	
914	3160732500	E BR SPRING, CR TRIS NR WOLBACH, NEBR.	130 1.58	888.8	41 27	89 26	85	7.78	8.44	118	2.48	5.18	0.8	238.8	115.8	1817.8	2231.8	1308.8	
915	3160732500	WEST BRANCH SPRING CREEK AT BRAYTON, NEBR.	130 10.58	3848.2	41 27	89 26	85	7.78	8.44	848	18.88	54.83	0.8	1578.1	1578.1	11332.7	13259.2	12858.8	
916	3160835000	FRAZIER CREEK NEAR HAYWOOD, NEBR.	130 11.38	8899.8	48 35	108 38	108	7.35	2.30	332	8.81	25.84	0.8	1885.8	588.8	18893.3	21118.7	11288.8	
917	3160835000	CUT CANYON NEAR CURTIS, NEBR.	130 88.58	947.8	48 44	108 38	108	7.34	2.31	488	18.72	57.85	0.8	464.2	348.1	1818.4	3888.8	1582.8	
918	3160848500	DRY CREEK NEAR CURTIS, NEBR.	130 81.78	3937.2	48 38	108 28	108	7.48	2.33	488	11.64	42.88	0.8	2848.1	1285.4	12319.5	14888.8	25888.8	

1	Zone 15																		18
	STATION	STATION NAME	RECT AREA	910	LAT	LONG	R	P10	P60	DN	L	LL	STR	SHEAN	Q2	Q50	Q100	QPEAK	
919	3160851500	NORTH FORK MICHIGAN RIVER NEAR GOULD, COLO.	18 88.88	328.8	48 33	108 61	17	8.87	8.98	1238	7.38	49.88	0.8	192.1	215.2	332.3	352.2	281.8	
920	3160852500	SOUTH ST. VRAIN CREEK NEAR WARD, COLO.	18 14.48	323.8	48 05	108 31	38	3.83	1.24	2588	7.98	16.51	3.8	243.8	243.8	518.8	582.8	482.8	
921	3160852500	GLACIER CREEK NEAR ESTES PARK, COLO.	18 84.48	342.4	39 45	108 68	28	3.82	1.86	3848	6.88	39.71	2.8	244.8	251.8	353.8	415.8	352.8	
922	3160852500	HALFMOON CREEK NEAR WALT, COLO.	18 83.88	384.8	39 18	108 23	28	3.81	0.95	3888	9.47	48.22	0.8	272.8	311.8	477.4	512.8	458.8	
923	3160852500	SACKETT CREEK NEAR PIKES PEAK, COLO.	18 6.88	81.38	58 16	108 58	78	4.79	1.78	2488	4.88	6.83	0.8	18.7	18.4	38.8	42.4	28.8	
924	3160852500	SOUTH CASCADE CREEK AT CASCADE COLO.	18 3.41	83.8	38 58	108 58	78	4.79	1.78	2488	4.88	6.83	0.8	18.7	18.4	38.8	42.4	28.8	
925	3160853000	HEADON CREEK NEAR TABERNASH, COLO.	18 8.93	273.8	48 03	108 47	38	3.86	8.98	1238	5.83	8.15	0.8	192.4	192.4	361.7	392.8	316.8	
926	3160853000	SLATE CREEK NEAR DILLON, COLO.	18 10.88	274.8	39 43	108 28	28	3.81	8.95	3878	9.42	17.58	0.8	182.8	195.8	331.8	382.1	288.8	
927	3160853000	PINEY A BLN PINEY LAKE, NEAR HINTURN, COLO.	18 13.88	898.8	39 43	108 28	28	3.29	1.95	2578	8.27	14.88	0.8	257.1	282.2	455.1	495.2	386.8	
928	3160853000	GORE CR AT UPPER STA NR HINTURN COLO.	18 14.48	812.8	39 38	108 16	28	3.89	1.88	3888	9.88	16.45	0.8	328.2	328.2	892.2	788.8	588.8	
929	3160853000	BLACK GORE CREEK NEAR HINTURN, COLO.	18 11.88	386.8	39 38	108 16	28	3.29	1.88	1288	5.88	13.13	0.8	216.4	225.8	378.8	488.8	381.8	
930	3160853000	CASAS CREEK NR COSTILLA, N. MEX.	18 18.88	138.8	38 54	108 18	68	4.27	1.87	2848	7.82	38.81	0.8	78.4	73.8	167.8	188.8	181.8	
931	3160853500	ANTHONY CREEK NR COSTILLA, N. MEX.	18 8.18	188.8	38 53	108 17	68	4.27	1.87	2848	7.82	38.81	0.8	78.4	73.8	167.8	188.8	181.8	
932	3160853500	LAKE CREEK NEAR CERRILLO, N. MEX.	18 18.88	218.8	38 37	108 23	53	4.88	1.33	2877	6.33	26.88	0.8	126.8	128.8	286.4	322.8	284.8	
933	3160853500	RED RIVER NEAR RED RIVER, N. MEX.	18 18.88	218.8	38 37	108 23	53	4.88	1.33	2877	6.33	26.88	0.8	126.8	128.8	286.4	322.8	284.8	
934	3160853500	TESQUIE CR AB DIVERGIONS NR SANTA FE N. MEX.	18 11.78	318.1	39 58	108 05	38	3.88	1.14	4488	8.97	24.43	0.8	157.8	117.8	788.8	918.4	632.8	
935	3160853500	MIDDLE CROM CREEK NEAR NECLA, WYO.	18 88.88	188.2	41 18	108 18	14	3.88	1.27	1888	8.91	15.48	0.8	81.8	85.8	388.8	448.3	485.8	
936	3160853500	SOUTH CROM CREEK NEAR NECLA, WYO.	18 13.88	88.4	41 07	108 11	38	3.88	1.27	778	13.26	26.33	0.8	26.3	19.7	143.1	173.6	118.8	
937	3160853500	DEAD MAN GULCH NEAR HONETA, WYO.	17 4.46	1247.1	43 11	107 47	25	2.33	0.76	518	3.94	9.22	0.8	488.8	325.5	2888.2	2475.9	1338.8	
938	3160853500	SHELL CR AB SHELL CR RESERVOIR, WYO.	17 83.18	1388.8	44 38	107 24	22	3.87	1.19	2288	8.18	28.14	0.8	928.8	779.2	2382.4	2788.5	1878.8	
939	3160853500	N PK TONGUE RIVER NEAR DAYTON, WYO.	17 32.48	584.2	44 45	107 37	28	4.28	1.43	1888	9.88	48.73	0.8	883.8	288.8	898.2	1832.3	988.8	
940	3160853500	LITTLE TONGUE RIVER NEAR DAYTON, WYO.	17 88.18	388.8	44 49	107 17	22	3.88	1.27	4281	8.82	17.28	0.8	158.8	135.9	611.8	717.8	838.8	
941	3160853500	WOLF CREEK AT HOLY, WYO.	17 37.88	788.8	44 48	107 14	22	3.88	1.28	5578	7.81	14.31	0.8	378.8	319.4	1544.8	1841.8	1188.8	
942	3160853500	LITTLE SANDY CREEK NEAR ELKHORN, WYO.	17 88.98	891.8	42 32	108 12	28	8.88	8.98	5118	48.32	27.19	10.8	289.5	289.5	383.8	435.8	425.8	
943	3160853500	MIDDLE FORK BEAVER CR NR LONETREE, WYO.	17 88.98	488.4	41 88	108 11	17	2.78	8.88	3888	11.88	17.28	0.8	158.8	135.9	611.8	717.8	838.8	
944	3160853500	WILLOW CREEK NEAR OJON, WYO.	17 18.98	248.8	41 88	108 11	17	2.78	8.88	3888	11.88	17.28	0.8	158.8	135.9	611.8	717.8	838.8	
945	3160853500	GEORGE TOWN CREEK NEAR GEORGE TOWN, IDAHO	18 18.88	137.8	48 38	118 11	28	3.81	8.95	3888	9.44	5.81	0.8	87.8	52.8	281.8	323.8	182.8	
946	3161017800	HOLMES CREEK NEAR KAYSVILLE, UTAH	18 2.49	33.9	41 83	111 53	88	4.88	1.33	3398	8.98	6.28	0.8	19.7	21.7	42.8	44.9	36.8	
947	3161017800	DRY CREEK NEAR ALPINE, UTAH	18 8.88	317.8	48 20	111 47	88	3.85	1.14	4788	8.98	8.98	0.8	817.1	818.8	588.8	597.8		
948	3161017800	MILL CREEK NEAR SALT LAKE CITY, UTAH	18 81.78	113.4	48 41	111 47	88	3.78	1.24	4888	11.38	48.88	0.8	94.8	88.8	188.4	1433.8		
949	31608531800	N PK PONDOR RIVER NEAR HAZELTON, WYO.	18 84.88	937.4	44 62	107 85	28	3.88	1.14	1388	13.48	58.88	0.8	348.7	388.3	849.4	1345.2	888.8	
950	31608531800	LA BARGE C N LA BARGE HEADQUARTERS RGR STA WYO.	18 6.38	188.1	42 38	118 48	28	11.42	3.42	1888	17.38	11.42	0.8	136.3	136.3	288.8	218.1	178.8	
951	31608531800	SWIFT CREEK NEAR APTON, WYO.	18 87.48	738.8	42 44	118 54	16	2.88	0.65	3888	6.38	23.13	0.8	817.2	837.8	861.8	938.8	778.8	
952	3161328500	LONG CANYON CREEK NEAR PORTMILL, IDAHO	18 88.88	888.8	48 57	118 32	28	1.82	8.78	3488	12.88	28.81	0.8	936.4	682.9	1488.9	1837.3	1388.8	
953	3161328500	EAST FORK REIDER RIVER, IDAHO	18 2.88	78.7	44 48	118 31	88	3.81	8.98	888	2.38	8.88	0.8	58.2	38.2	182.3	112.4	77.8	
954	31613315500	MUD CREEK NEAR TANAHA, IDAHO	18 18.18	488.8	48 08	118 51	28	3.28	1.88	2888	18.78	33.78	0.8	291.8	284.3	2188.1	2728.3	2248.8	
955	31608531800	GRINNELL CREEK NEAR MARY GLACIER, MONT.	18 3.47	313.8	48 40	113 42	31	3.89	1.18	888	9.18	31.88	0.8	189.7	179.8	588.2	617.8	558.8	
956	31608531800	EMERY CREEK NEAR MARY GLACIER, MONT.	18 31.48	1487.8	48 48	113 48	31	3.78	1.28	1787	6.88	71.28	0.8	1114.8	1847.7	1858.2	2151.3	8788.8	
957	31608531800	BIRCH CREEK NEAR GLEN, MONT.	18 36.88	1487.8	48 48	113 48	31	3.78	1.28	1787	6.88	71.28	0.8	1114.8	1847.7	1858.2	2151.3	8788.8	
958	31608531800	BOULDER BASIN CREEK NEAR GLEN, MONT.	18 18.48	898.8	48 15	112 38	12	2.88	0.65	1788	6.82	47.24	0.8	891.8	136.7	912.3	1888.4	882.8	
959	31608531800	NECLAND CR NR WHITE SULPHUR SPRINGS, MONT.	18 6.74	77.8	48 44	118 88	28	3.81	8.98	1788	4.57	31.81	0.8	28.8	14.8	184.3	188.3	241.8	

Table 10. Continued.

Zone 16

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

STATION	STATION NAME	SEC	AREA	SIG	LAT	LONG	R	P10	P80	OM	L	LL	STRE	ORAN	Q8	Q50	Q100
800 5133343000	SMITH QUICH TRI NEAR PATANA WASH.	800	1.85	388.8	48 28 117 27	85	8.00	8.00	848	1.01	4.41	8.6	113.7	78.9	710.8	878.1	898.8
801 5133344000	MIGBUDURI PLAT CREEK TRI NE PULLMAN WASH.	801	0.00	128.0	48 48 117 10	85	2.93	8.04	100	1.93	5.43	8.0	85.0	38.0	389.8	358.3	334.8
802 5133345000	MIGBUDURI PLAT CREEK TRI NE PULLMAN WASH.	802	87.10	1178.8	48 48 117 10	85	1372	14.14	78.48	0.0	78.48	0.0	78.48	40.8	244.7	209.3	163.0
803 5133346000	PALOUSE RIVER TRI AT COLFAX WASH.	803	8.10	151.4	48 55 117 25	85	2.88	8.01	488	3.85	4.88	8.0	46.8	46.8	244.7	209.3	163.0
804 5133347000	MARIONAN ORAN TRI AT PLAZA WASH.	804	1.84	187.8	47 19 117 14	88	8.08	8.03	888	8.08	3.08	8.0	134.8	48.8	197.8	163.8	178.8
805 5133348000	CON CREEK TRI NEAR RITVILLE WASH.	805	1.91	119.3	47 11 118 18	88	1.98	8.08	188	8.24	3.84	8.0	48.8	38.8	248.8	289.7	289.8
806 5133349000	BLUE CREEK NE WALLA WALLA, WASH.	806	17.88	718.8	48 53 118 08	48	3.95	1.84	2488	8.08	34.93	8.0	388.8	388.8	1194.8	1299.8	1399.8
807 4133324000	LITTLE CR AT HIGH VALLEY NE UNION, OREG.	807	18.88	297.8	48 13 117 47	38	2.98	8.05	3388	8.08	28.88	8.0	138.8	138.8	389.8	489.8	538.8
808 4133325000	LADON CREEK NE UNION, OREG.	808	18.88	778.8	48 13 117 47	38	2.98	8.05	3388	8.08	28.88	8.0	138.8	138.8	389.8	489.8	538.8
809 4133326000	INDIAN CREEK NE UNION, OREG.	809	21.88	788.8	48 13 117 47	38	3.98	8.05	3788	8.08	38.88	8.0	138.8	138.8	489.8	589.8	638.8
810 4133327000	EAST FORK WALLA RIVER NE JOSEPH, OREG.	810	18.38	888.8	48 48 117 18	81	3.78	1.33	3388	8.08	7.31	8.0	115.8	115.8	388.8	488.8	538.8
811 4133328000	HURRICANE CREEK NEAR JOSEPH, OREG.	811	88.88	888.8	48 48 117 18	81	3.78	1.33	3388	8.08	7.31	8.0	115.8	115.8	388.8	488.8	538.8
812 4133329000	STRAVENS CR AB ELIOE CR NE PRAIRIE CITY, OREG.	812	7.88	138.8	48 51 118 28	83	3.48	1.85	818	4.81	7.81	8.0	93.4	93.4	188.7	284.6	178.8
813 5133330000	CRITCHFIELD ORAN NEAR CLARKSTON WASH.	813	1.88	488.8	48 27 117 05	188	8.08	8.04	743	8.08	8.08	8.0	87.4	39.8	1110.4	1303.8	788.8
814 5133331000	WYCLONE CREEK NEAR ENUNCLAW, WASH.	814	8.38	1888.8	47 18 121 47	148	4.38	1.88	1588	1.88	1.88	8.0	817.8	382.4	848.2	1185.8	1888.8
815 5133332000	SNOW CREEK NEAR LESTER, WASH.	815	11.98	1788.8	47 18 121 47	148	4.38	1.88	1588	1.88	1.88	8.0	817.8	382.4	848.2	1185.8	1888.8
816 5133333000	PRION CREEK NEAR LESTER, WASH.	816	4.87	513.4	47 13 121 27	78	4.88	1.88	8188	3.43	8.21	8.0	387.8	81.8	892.7	1023.3	1378.8
817 5133334000	BOU FORK CEDAR RIVER NEAR LESTER, WASH.	817	8.08	881.8	47 18 121 31	78	4.87	1.81	1848	3.48	10.48	8.0	371.8	882.8	819.8	881.8	834.8
818 5133335000	NEAR RIVER NEAR CEDAR FALLS, WASH.	818	13.48	8848.8	47 51 121 48	88	4.87	1.81	8188	5.78	14.18	8.0	1088.8	1048.4	4458.8	5888.4	4888.8
819 5133336000	CANDYEN CREEK NEAR CEDAR FALLS, WASH.	819	1.97	81.8	47 51 121 48	198	4.88	1.82	188	8.08	8.08	8.0	85.8	85.8	121.8	181.8	181.8
820 5133337000	SUNLEY CREEK NEAR CEDAR FALLS, WASH.	820	1.97	171.8	47 51 121 48	198	4.88	1.82	188	8.08	8.08	8.0	85.8	85.8	121.8	181.8	181.8

Zone 17

STATION	STATION NAME	SEC	AREA	SIG	LAT	LONG	R	P10	P80	OM	L	LL	STRE	ORAN	Q8	Q50	Q100
891 1613180000	BENNETT CREEK NEAR BENNETT, IDAHO	891	81.38	888.8	43 14 118 38	18	2.78	8.07	1888	8.08	17.88	8.0	88.7	88.7	388.4	434.8	884.8
892 1613181000	SUGAR CREEK TRI, NE GRABBER, IDAHO	892	4.88	87.8	43 34 118 34	13	2.41	8.78	248	1.88	1.88	8.0	38.8	38.8	141.7	151.8	184.8
893 1613182000	LITTLE SUGAR CR TRI NE GRABBER, IDAHO	893	1.81	87.8	43 34 118 34	13	2.41	8.78	248	1.88	1.88	8.0	38.8	38.8	141.7	151.8	184.8
894 1613183000	COTTONWOOD CREEK AT ANHODGER NEVERFOR, IDAHO	894	81.48	888.8	43 38 118 48	38	3.87	1.18	4388	8.08	10.88	8.0	112.8	108.8	488.8	588.8	688.8
895 1613184000	WATER CREEK NEAR IDAHO CITY, IDAHO	895	8.78	38.8	43 38 118 48	38	3.87	1.18	4388	8.08	10.88	8.0	112.8	108.8	488.8	588.8	688.8
896 1613185000	RAVINE CREEK NEAR ARROW ROCK, IDAHO	896	18.98	188.8	43 38 118 48	38	3.87	1.18	4388	8.08	10.88	8.0	112.8	108.8	488.8	588.8	688.8
897 1613186000	WATER VALLEY CREEK NEAR EBLE, IDAHO	897	88.88	881.4	43 44 118 18	18	3.81	8.88	8888	18.78	31.78	8.0	88.8	88.8	388.8	488.8	848.8
898 1613187000	BAKER C AT NARROWS, NE BAKER MEV.	898	18.48	188.8	48 08 118 08	18	8.08	8.08	3488	8.08	18.08	8.0	94.4	88.8	338.1	488.8	488.8
899 1613188000	REED CR NE OHVNEE NEVADA	899	8.88	88.8	41 54 118 04	18	8.08	8.78	1888	8.08	7.88	8.0	38.8	38.8	188.8	188.8	88.8
900 1613189000	SUFFALO RIVER NE ISLAND PARK, IDAHO	900	38.78	888.8	44 55 118 83	14	3.81	8.88	888	18.88	41.88	8.0	487.7	418.8	813.8	898.7	838.8
901 1613190000	ELK CREEK NEAR DOWNEY, IDAHO	901	8.48	181.8	44 55 118 83	14	8.08	8.88	178	2.88	3.88	8.0	148.3	138.8	288.8	248.8	184.8
902 1613191000	NORTH FORK SOUTHERN CREEK NEAR SOUTHERN, IDAHO	902	88.88	888.8	48 08 118 38	18	8.08	8.88	1881	8.48	37.18	8.0	184.4	184.4	388.4	448.8	882.8
903 1613192000	LITTLE SAGWYD RIVER AT HENRY, IDAHO	903	8.88	88.8	48 21 118 18	38	3.81	8.88	1888	4.78	18.78	8.0	38.1	38.1	88.8	138.1	88.8
904 1613193000	SOUTH FORK SAGWYD CREEK NEAR	904	8.78	118.8	48 21 118 18	38	3.81	8.88	1888	4.78	18.78	8.0	38.1	38.1	88.8	138.1	88.8
905 1613194000	TRAPPER CREEK NEAR OAKLEY, IDAHO	905	88.88	438.8	48 51 114 18	18	2.88	8.84	3888	17.81	248.33	8.0	848.3	848.3	888.8	888.8	481.8
906 1613195000	TRAPPER CREEK NEAR ROCK CREEK, IDAHO	906	88.88	438.8	48 51 114 18	18	2.88	8.84	3888	17.81	248.33	8.0	848.3	848.3	888.8	888.8	481.8
907 1613196000	SILVER CREEK NEAR SILVER LAKE, OREG.	907	188.88	838.8	43 43 131 84	8	8.33	8.78	8814	8.88	882.88	1.81	883.8	883.8	1848.3	1477.8	1888.8
908 1613197000	SILVER CREEK NEAR SILVER LAKE, OREG.	908	8.88	88.8	43 54 118 04	18	8.08	8.88	888	4.78	8.88	8.0	48.1	48.1	188.8	141.4	74.8
909 1613198000	OTYNE CANYON NEAR BURNS, OREG.	909	4.88	88.8	48 48 118 08	8	8.08	8.88	388	3.88	3.88	8.0	18.7	18.7	48.8	48.7	31.8
910 1613199000	WAGON CREEK NEAR BURNS, OREG.	910	18.47	47.8	43 38 118 15	8	8.13	8.71	1888	18.88	87.38	8.0	21.8	19.1	178.8	288.8	82.8
911 1613200000	WAGON CREEK NEAR FRENCHLEIGH, OREG.	911	888.88	887.8	48 47 118 58	11	1.88	8.88	3348	88.88	188.88	8.0	188.8	188.8	3887.4	3488.8	3888.8
912 1613201000	WAGON CREEK NEAR FRENCHLEIGH, OREG.	912	38.88	831.3	48 56 118 51	8	1.88	8.88	3188	14.38	48.81	8.0	113.4	117.8	381.8	418.8	381.8
913 1613202000	WAGON CREEK NEAR FRENCHLEIGH, OREG.	913	48.88	378.8	48 56 118 43	18	1.88	8.88	4888	28.38	71.88	8.0	219.4	288.8	888.7	887.7	187.8
914 1613203000	SILVER CR NE RILEY, OREGON	914	28.88	1374.1	43 41 118 38	11	8.88	8.74	1878	88.88	88.88	8.0	88.8	88.8	888.8	888.8	418.8
915 1613204000	SUGAR CREEK NEAR SOUTHERN, UTAH	915	18.48	381.8	38 43 111 18	27	3.88	1.14	1188	7.28	21.18	1.8	88.8	88.8	388.8	488.8	488.8
916 1613205000	EAST FORK SOUTHERN CREEK NEAR SOUTHERN, UTAH	916	81.48	381.8	38 43 111 18	27	3.88	1.14	1188	7.28	21.18	1.8	88.8	88.8	388.8	488.8	488.8
917 1613206000	ANTHONY CREEK NEAR ANTHONY, UTAH	917	81.48	381.8	38 43 111 18	27	3.88	1.14	1188	7.28	21.18	1.8	88.8	88.8	388.8	488.8	488.8
918 1613207000	COTTONWOOD CREEK NEAR SALINA, UTAH	918	7.88	188.1	38 56 111 48	88	3.88	1.88	3788	7.88	28.88	8.0	41.8	41.8	388.8	387.7	188.8
919 1613208000	PLEASANT CREEK NEAR MOUNT PLEASANT, UTAH	919	18.88	817.8	38 33 111 83	38	3.81	8.88	3178	4.77	88.87	8.0	88.8	88.8	188.8	234.4	888.8
920 1613209000	THIN CREEK NEAR MOUNT PLEASANT, UTAH	920	8.88	878.8	38 33 111 83	38	3.81	8.88	3178	4.77	88.87	8.0	88.8	88.8	188.8	234.4	888.8
921 1613210000	THIN CREEK NEAR BEAVER, UTAH	921	18.88	838.8	38 18 118 38	88	3.88	1.88	8188	7.88	37.88	8.0	93.8	93.8	388.4	488.8	288.8

Table 10. Continued.

Zone 17

1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18

SECT STATION	STATION NAME	SECT AREA	919	LAT	LONG	R	P18	P09	DN	L	LL	9790	9HEAN	02	950	0100	0PEAN
718 0000000000	BEAVER CREEK NEAR RIPLE COLO.	210 7.00	75.7	35.08	107.50	50	3.10	0.05	3000	0.05	12.70	0.0	48.0	40.0	100.7	127.7	100.0
719 0000000000	SLATE CREEK NEAR BRAND VALLEY, COLO.	210 10.00	101.0	30.00	107.50	50	3.14	1.00	3000	0.37	83.00	0.0	51.2	51.2	107.0	104.0	100.0
714 0000000000	PLATEAU C AT UPPER 97A. MR COLLIER, COLO.	210 84.00	441.0	30.13	107.48	50	3.00	1.00	2010	17.10	40.00	0.0	80.4	80.4	107.0	710.0	400.0
715 0000000000	BRUSH CREEK NEAR COLLIER, COLO.	210 0.07	804.0	30.00	107.51	50	3.00	1.14	8100	4.00	43.00	0.0	100.0	90.0	100.0	100.0	100.0
716 0000000000	SHEEP CREEK NEAR NANTLA, UTAH	210 40.00	700.0	30.00	100.00	50	3.10	0.00	3000	0.10	100.00	1.0	100.0	100.0	100.0	100.0	100.0
717 0000000000	ASHLEY C BELON YOUTU C MR VERNAL, UTAH	210 97.00	640.0	40.00	100.00	50	3.10	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
718 0000000000	WEST FORK OF RIVER NEAR ROBERTSON, WYO.	210 37.00	970.0	41.00	110.00	10	0.00	0.00	8000	11.00	30.00	0.0	510.0	400.0	100.0	100.0	100.0
719 0000000000	SHEEP CREEK NEAR BRAND MESA, COLO.	210 0.07	800.0	30.00	100.00	50	3.10	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
720 0000000000	DIRTY CREEK NEAR BRAND MESA, COLO.	210 14.00	110.0	30.00	107.50	50	3.00	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
721 0000000000	KISER CREEK NEAR BRAND MESA, COLO.	210 0.00	70.0	30.00	107.50	50	3.00	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
722 0000000000	COTTON WOOD CREEK NEAR BRAND MESA, COLO.	210 0.00	70.0	30.00	107.50	50	3.00	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
723 0000000000	SUNFACE CREEK NEAR CEDAREGG, COLO.	210 0.00	70.0	30.00	107.50	50	3.00	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
724 0000000000	MIDDLE MANCOS NEAR MANCOS, COLO.	210 13.00	801.0	30.00	100.00	50	3.00	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
725 0000000000	CATTLE C AS DIVERSIONS NEAR MOAB, UTAH	210 0.00	800.0	30.00	100.00	50	3.00	1.00	1000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
726 0000000000	SORREDO PALM CREEK NEAR SORREDO SPRINGS, CALIF.	220 01.00	300.0	30.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
727 0000000000	SILVER CANYON CREEK NEAR LAWS, CALIF.	220 01.00	6.4	37.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
728 0000000000	INDEPENDENCE CREEK BL PINTON CR MR INDEPENDENCE	220 10.00	100.0	30.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
729 0000000000	CUB RIVER NEAR PRESTON, IDAHO	220 10.00	740.0	40.00	111.00	41	3.00	1.14	2100	5.00	15.00	0.0	070.0	070.0	100.0	000.0	000.0
730 0000000000	DEVIL C AS CAMPBELL C MR MALD C IDAHO	220 13.00	100.0	40.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
731 0000000000	DEAR CREEK NEAR MAP, IDAHO	220 00.00	800.0	40.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
732 0000000000	DEAR CREEK NEAR CANYON CITY, NEV.	220 10.00	110.0	30.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
733 0000000000	LAHOLLE CREEK NEAR LAHOLLE, NEV.	220 00.00	800.0	40.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
734 0000000000	HUNTER CREEK NEAR REBO, NEV.	220 00.00	800.0	40.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
735 0000000000	GALENA CREEK NEAR STEAMBOAT, NEV.	220 00.00	800.0	40.00	110.00	10	0.00	0.00	4000	10.00	04.00	0.0	400.0	400.0	100.0	000.0	000.0
736 0000000000	PAYSON C AS DIVERSION, MR PAYSON, UTAH	220 10.00	300.0	30.00	111.00	41	3.00	1.00	2000	5.00	15.00	0.0	070.0	070.0	100.0	000.0	000.0
737 0000000000	CITY CREEK NEAR SALT LAKE CITY, UTAH	220 10.00	110.0	40.00	111.00	53	3.00	1.00	3000	11.00	07.00	0.0	070.0	070.0	100.0	000.0	000.0

Zone 18

738 0000000000	RITO BLANCO NEAR PAGOSA SPRINGS, COLO.	210 00.00	400.0	37.00	100.00	53	3.00	1.12	3000	11.00	07.00	0.0	210.0	241.0	500.0	070.0	470.0
739 0000000000	LITTLE HAVASO RIVER AT CHRONO, COLO.	210 01.00	300.0	37.00	100.00	51	3.00	1.14	4000	10.00	07.00	0.0	170.0	140.0	500.0	000.0	300.0
740 0000000000	EAST MANCOS RIVER NEAR MANCOS, COLO.	210 11.00	970.0	37.00	100.00	14	3.00	1.00	4000	10.00	07.00	0.0	130.0	230.0	100.0	000.0	000.0
741 0000000000	GALLEGO CANYON TRIBUTARY N MAGEE, N. MEX.	210 00.00	300.0	30.00	100.00	17	0.00	0.00	1000	1.00	1.00	0.0	100.0	100.0	100.0	000.0	000.0
742 0000000000	CHUBCA WASH NEAR MEXICAN SPRINGS, N. MEX.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
743 0000000000	CAYRON WASH NEAR MEXICAN SPRINGS, N. MEX.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
744 0000000000	PUERTO RIVER TRIBUTARY MR GARCIA, N. MEX.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
745 0000000000	MOORE CANYON TRIB. MR MOORE, ARIZ.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
746 0000000000	SALE BRUSH CREEK NEAR PRECONIA, ARIZ.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
747 0000000000	BITTER CREEK NEAR PRECONIA, ARIZ.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
748 0000000000	WEST CANYON CREEK NEAR WILLIAMS, ARIZ.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
749 0000000000	LITTLE RED HORSE WASH MR BRAND CANYON, ARIZ.	210 00.00	400.0	30.00	100.00	51	3.00	1.00	1000	10.00	04.00	0.0	100.0	100.0	100.0	000.0	000.0
750 0000000000	DAK CREEK TRIS. MR CORNVILLE, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0

Zone 19

751 0000000000	LYMAN REBEVOIR TRIS. MR ST. JOHN, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
752 0000000000	LITTLE COLORADO RIVER TRIS. MR ST. JOHN, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
753 0000000000	BLUWATER CR AS BLUWATER DAM BLUWATER N. MEX.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
754 0000000000	LA JENITA CREEK NEAR MASOLENA, N. MEX.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
755 0000000000	SALE BRUSH CREEK NEAR PRECONIA, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
756 0000000000	SALE BRUSH CREEK NEAR PRECONIA, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
757 0000000000	PUERTO RIVER TRIBUTARY MR GARCIA, N. MEX.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
758 0000000000	MOORE CANYON TRIB. MR MOORE, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
759 0000000000	SALE BRUSH CREEK NEAR PRECONIA, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0
760 0000000000	SALE BRUSH CREEK NEAR PRECONIA, ARIZ.	210 00.00	84.0	34.00	100.00	40	3.00	1.00	1000	10.00	04.00	0.0	40.0	40.0	100.0	000.0	000.0

Table 10. Continued.

Zone 19

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REG	STATION	STATION NAME	RECT	AREA	Q18	LAY	LONG	R	P16	P08	OW	L	LL	STRG	QHEAN	Q2	Q50	Q100	OPEAK		
766	3089448088	TROUT CREEK NEAR LUNA, N. MEX.	81P	31.08	1598.6	33	97	109	83	85	4.98	1.33	3088	12.93	47.74	0.8	487.2	883.8	8891.1	3429.0	5198.8
767	6486476808	BURNHAM WASH NR FLORENCE, ARIZ.	82B	18.08	3195.8	32	43	111	87	70	0.78	1.78	3088	10.06	186.4	0.8	1864.4	713.2	5322.6	8857.1	3558.8
768	6486476808	QUEEN CREEK TR29, NO. 3 AT MISTLOW OAK, ARIZ.	82B	9.37	275.8	33	88	111	70	4.78	1.68	308	1.18	1.52	0.8	186.4	181.6	732.6	887.0	508.6	
769	6486476808	QUEEN CREEK TR29, AT APACHE JUNCTION, ARIZ.	82B	9.61	318.1	33	84	111	38	4.88	1.58	70	8.48	8.48	0.8	99.8	79.8	386.1	459.8	822.6	
770	6486461708	CALABAZA CANYON NR MOJAVE, ARIZ.	82B	18.38	3887.7	31	87	110	89	70	0.18	1.88	3348	89.58	376.1	829.4	1632.6	1893.8	1898.8		
771	6486463008	TUCSON ARROYO YINE AVE, TUCSON, ARIZ.	82B	1.88	3489.8	32	85	110	78	0.18	1.88	428	12.38	35.88	0.8	1486.5	187.8	839.8	811.1	9888.8	
772	6486464808	SANJON CREEK NEAR TUCSON, ARIZ.	82B	38.8	348.8	32	88	110	88	88	0.88	1.52	3378	7.98	16.18	0.8	388.5	324.2	1843.1	1847.6	868.6
773	6486464808	TELEPHONE CANYON NR CHARLESTON PARK NEVADA	82B	2.8	788.8	32	88	110	33	38	3.88	1.58	3768	8.88	4.88	0.8	837.7	5.8	887.6	3888.8	288.8
774	6486464808	LIZURTA WASH AT LIZURTA, ARIZ.	82C	1.08	1088.8	38	41	114	18	38	3.78	1.08	3381	3.97	8.83	0.8	447.3	193.8	3182.8	3378.0	1598.8
775	6486464808	SHON CREEK NR WHITE WATER, CALIF.	82C	1.08	881.8	38	42	114	18	38	3.78	1.54	3388	3.88	7.48	0.8	1897.1	888.8	3180.1	18887.4	1388.8
776	6486464808	LONG CREEK NR DEBERT HOT SPRINGS, CALIF.	82C	10.48	6488.8	33	88	110	87	80	3.84	1.14	3888	8.38	37.18	0.8	1174.3	11.7	1888.1	24386.7	9878.0
777	6486464808	TAMUZZI CREEK NEAR PALM SPRINGS, CALIF.	82C	18.88	1188.8	34	48	110	34	18	9.38	9.78	7488	9.18	33.38	0.8	434.7	817.4	4864.4	6888.8	8888.0
778	6486464808	YALNUTZ CREEK NEAR PALM SPRINGS, CALIF.	82C	9.47	88.8	33	34	110	31	88	4.88	1.43	1848	1.38	1.38	0.8	11.6	6.8	48.5	89.4	28.0
779	6486464808	DEER CREEK NR PALM DESERT, CALIF.	82C	38.08	1381.8	33	38	110	83	18	3.88	1.88	3388	7.78	49.87	0.8	383.6	182.2	2841.3	3278.8	1388.8
780	6486464808	COTTONWOOD WASH NR COTTON WOOD SPRINGS, CALIF.	82C	5.61	31.8	32	88	108	48	15	3.88	8.78	3.88	8.78	4.87	0.8	188.8	188.8	538.8	638.8	434.8
781	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
782	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
783	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
784	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
785	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
786	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
787	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
788	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
789	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
790	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
791	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
792	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
793	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
794	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
795	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
796	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
797	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
798	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
799	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
800	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
801	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
802	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
803	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
804	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
805	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
806	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
807	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
808	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
809	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
810	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
811	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
812	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
813	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
814	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
815	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
816	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
817	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
818	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
819	6486464808	ARICUL RESECH SERV SAPPORD WYSHED M-11 ARIZ.	82D	1.18	374.8	32	88	108	48	88	4.88	1.58	338	3.97	11.87	0.8	141.3	188.8	538.8	718.7	897.3
820																					



Table 10. Continued.

Zone 20																
STATION	STATION NAME	SECT	AREA	Q16	LAT	LONG	R	P16	P68	ON	L	LL	STRG	ONEAN	OS	OPEAK
811 0511319688	COLE CREEK NEAR SALT SPRINGS DAM, CALIF.	230	59.48	3888.1	38 31 120 18	98	5.19	0.81	9168	9.79	16.08	0.8	1933.9	1643.1	7388.7	8998.8
812 0511319688	ROCK CREEK AT GOODYEARS BAR, CALIF.	230	9.86	799.3	38 32 120 33	36	0.41	0.28	2656	9.79	16.04	0.8	385.8	364.7	2715.5	2858.9
813 0511418598	GOODYEARS CREEK AT GOODYEARS BAR, CALIF.	230	12.96	1779.2	38 33 120 53	36	0.88	0.39	3345	9.87	26.05	0.8	582.1	481.7	2715.5	2858.9
814 0511448888	ALDER CREEK NEAR WHITE HALL, CALIF.	230	82.18	1779.4	38 48 120 22	98	4.07	1.01	2968	11.51	43.08	0.9	842.3	869.6	8896.1	7496.8
815 4114811888	SALTZMAN CREEK AT PORTLAND, OREG.	244	1.46	839.8	45 34 122 45	118	3.86	1.14	739	1.49	3.98	0.8	119.9	119.9	386.9	411.3
816 5312653488	DOREHALLS R TRIBUTARY NEAR BRINNON, WASH.	244	9.92	863.3	47 43 122 36	114	5.16	0.81	2888	1.37	2.37	0.9	38.6	38.6	78.6	78.4
817 5312653488	DOREHALLS CREEK NEAR BRINNON, WASH.	244	1.01	177.8	47 33 122 45	112	4.71	1.71	58	1.59	1.57	0.8	137.9	146.8	237.9	279.4
818 5312653488	DOREHALLS CREEK NEAR BRINNON, WASH.	244	1.01	177.8	47 33 122 45	112	4.71	1.71	58	1.59	1.57	0.8	137.9	146.8	237.9	279.4
819 5312653488	HUGE CREEK NEAR MUNA, WASH.	244	2.47	534.7	47 23 122 42	78	4.28	1.43	819	3.72	3.72	0.8	185.3	186.7	531.1	391.9
820 5312653488	KAPOHIN CREEK NEAR KAPOHIN, WASH.	244	98.98	588.5	47 08 122 12	31	3.78	1.91	1368	11.95	31.49	0.8	345.5	372.9	821.5	721.2
821 5312653488	KAN CREEK NEAR YACONA, WASH.	244	2.18	191.8	47 11 122 24	36	3.29	1.43	48	3.29	3.29	0.8	33.5	33.5	82.5	82.5
822 5312653488	SHAY CREEK NEAR LESTER, WASH.	244	0.56	228.6	47 15 121 34	78	4.77	1.75	1028	4.78	15.73	0.8	578.8	606.8	1528.8	1788.8
823 5312653488	DEEP CREEK AT CUMBERLAND, WASH.	244	2.17	166.7	47 17 121 35	43	4.98	1.33	1788	3.56	4.47	0.9	68.4	68.4	138.9	151.2
824 5312653488	HILL CREEK NEAR AUBURN, WASH.	244	3.14	52.7	47 15 122 15	25	3.29	1.43	288	3.29	2.42	0.8	47.5	52.8	108.2	124.4
825 5312653488	EVANS CREEK TRIBUTARY NEAR REDMOND, WASH.	244	2.46	54.6	47 46 122 15	17	2.85	0.88	918	1.18	1.18	0.8	31.2	38.1	124.5	151.8
826 5312653488	NORTH CREEK NEAR BOTHELL, WASH.	244	24.68	448.8	47 48 122 12	19	8.08	0.91	365	19.91	19.91	0.8	324.8	328.8	858.8	888.8
827 5312653488	OLNEY CREEK NEAR COLD BAY, WASH.	244	6.31	3614.8	47 45 121 45	155	4.36	1.08	2328	9.38	9.18	0.8	1285.9	1285.9	885.4	8813.3
828 5312653488	MUNSON CREEK NEAR MARYSVILLE, WASH.	244	9.97	41.9	47 05 122 16	39	3.81	0.95	315	1.47	1.47	0.8	86.2	86.2	26.8	56.5
829 5312653488	QUILCOA CREEK NEAR MARYSVILLE, WASH.	244	18.46	332.5	46 58 122 08	38	3.16	1.08	488	9.33	23.11	0.8	109.9	109.9	324.3	351.7
830 5312653488	ALDER CREEK NEAR HARTLEY, WASH.	244	18.78	695.7	46 52 121 37	31	3.78	1.24	1186	9.59	19.19	0.8	344.9	344.9	741.5	772.9
831 5312653488	CANBY CREEK NEAR MOUNT VERNON, WASH.	244	2.59	895.7	46 25 122 26	23	3.18	1.43	88	2.47	9.59	0.5	93.1	363.5	185.1	185.1
832 5312653488	WOODKIN CREEK NEAR BICKERMAN, WASH.	244	53.18	809.4	46 48 122 08	48	3.39	1.43	3459	9.15	21.39	0.8	183.8	183.8	342.3	3978.7
833 5314811988	BURNTRIDGE CREEK AT VANCOUVER, WASH.	244	21.46	133.2	46 39 122 48	6	3.39	1.11	148	9.13	19.19	0.8	66.1	66.1	216.1	241.8
834 5314811988	SALMON CREEK NEAR BATTLE GROUND, WASH.	244	18.36	1498.6	46 48 122 27	48	4.38	1.52	1886	4.12	28.73	0.8	984.2	913.2	1886.4	1871.2
835 5314233588	COLUMBIA RIVER TRIBUTARY AT CARPIS, WASH.	244	1.96	887.7	46 06 122 58	193	4.26	1.43	988	1.84	3.84	0.8	59.3	59.3	129.1	142.7
836 5314233588	NORTH PK LACHANAS CREEK NR ETHEL, WASH.	244	0.38	33.48	46 19 122 55	38	4.16	1.36	138	1.15	1.15	0.8	24.3	26.5	36.0	36.0
837 5314248888	YOUTLE RIVER TR18 NR CASTLE ROCK, WASH.	244	0.84	867.7	46 16 122 05	58	4.16	1.36	155	0.82	0.82	0.8	44.9	41.8	138.0	151.8

Zone 21																
STATION	STATION NAME	SECT	AREA	Q16	LAT	LONG	R	P16	P68	ON	L	LL	STRG	ONEAN	OS	OPEAK
838 5312653488	LANE CREEK NEAR WABE, WASH.	244	2.16	211.3	46 32 123 47	185	5.41	2.26	1878	3.49	15.14	0.8	177.6	184.7	550.4	288.2
839 5312653488	NORTH NEAR R TRIBUTARY NR SOUTH BEND, WASH.	244	2.46	799.3	46 35 123 35	188	5.16	0.81	1779	1.08	1.08	0.8	31.2	34.3	91.1	85.8
840 5312653488	POR CREEK NEAR LEVAN, WASH.	244	86.46	3634.7	46 35 123 35	148	5.16	0.81	1898	0.88	24.28	0.8	233.6	242.4	812.7	888.9
841 5312653488	DEER CREEK NEAR LEVAN, WASH.	244	1.79	281.4	46 38 123 35	148	5.93	2.36	244	1.91	2.93	0.8	135.9	135.9	291.8	324.8
842 5312653488	THE CREEK NEAR CONNOLLY, WASH.	244	6.68	249.6	46 38 123 43	98	4.78	1.81	198	1.54	3.08	0.8	183.3	183.3	348.2	383.8
843 5312653488	WATER HILL CREEK NEAR DE ELL, WASH.	244	1.98	143.6	46 34 123 18	188	4.88	1.43	148	2.38	8.38	0.8	63.8	93.8	172.6	197.6
844 5312653488	516 CREEK NEAR HOQUIAM, WASH.	244	0.96	110.6	47 06 123 03	88	4.07	1.61	78	0.81	0.81	0.8	66.4	78.4	147.4	164.8
845 5312653488	ROCK CREEK AT CEDARVILLE, WASH.	244	84.08	1481.8	46 58 123 15	78	4.88	1.40	988	16.08	35.68	0.8	1148.8	1218.5	1715.8	1827.8
846 5312653488	516 CREEK TRIBUTARY NEAR HOQUIAM, WASH.	244	0.16	53.48	46 58 123 56	86	4.81	1.61	98	0.83	0.83	0.8	19.7	19.8	26.4	24.6
847 5312653488	MAY CREEK NEAR FORKS, WASH.	244	8.93	718.1	47 05 124 18	168	5.89	2.88	358	1.88	1.88	0.8	499.4	499.4	668.8	759.8
848 5312653488	GRAND CREEK NEAR FORKS, WASH.	244	1.87	618.7	47 06 124 24	148	5.41	2.36	338	2.85	8.85	0.8	339.7	339.7	825.8	882.4
849 5312653488	LEES CREEK AT PORT ANGELES, WASH.	244	4.77	316.8	46 06 123 23	48	3.96	1.14	1778	4.82	7.81	0.8	126.3	98.5	624.5	681.5
850 5312653488	DEAN CREEK AT BLYN, WASH.	244	2.96	481.8	46 01 123 51	28	3.26	1.82	2188	3.69	6.43	0.8	38.9	32.3	73.6	81.9
851 5312653488	SHON CREEK NEAR WAYNARD, WASH.	244	11.28	478.3	47 06 123 55	88	3.78	1.24	2888	0.82	8.33	0.8	288.4	282.4	771.4	738.6
852 5312653488	PENNY CREEK NEAR OULICENE, WASH.	244	9.76	449.3	47 46 123 06	114	5.24	2.87	688	4.58	7.78	0.8	339.4	239.4	648.8	587.8
853 5312653488	ANNA CREEK NEAR PORTLETEN, WASH.	244	9.85	318.7	47 01 123 16	188	5.33	2.19	688	1.17	0.91	0.8	95.3	58.8	278.3	328.8
854 5312653488	WADON CREEK NEAR HOODPORT, WASH.	244	1.18	85.3	47 01 123 16	148	5.16	0.81	828	0.78	0.78	0.8	59.1	61.5	129.4	143.8
855 5312653488	WADON CREEK TRIBUTARY NR SHELTON, WASH.	244	1.18	85.3	47 01 123 16	148	5.16	0.81	828	0.78	0.78	0.8	59.1	61.5	129.4	143.8
856 5314246188	RISK CR NR SHANOKANA, WASH.	244	1.13	137.8	46 18 123 38	114	4.87	1.81	338	1.48	8.11	0.8	89.1	101.8	182.2	181.8
857 5314246188	WEST BRANCH GRAYS RIVER NR GRAYS RIVER, WASH.	244	15.86	3686.3	46 23 123 33	148	6.83	2.38	1488	0.88	28.88	0.8	248.8	848.8	588.8	477.8
858 4114361388	JONES CREEK NR GRANTS PASS, OREG.	240	7.41	747.1	42 08 123 17	38	4.88	1.43	1968	3.57	7.83	0.8	375.4	338.4	1288.8	1411.8
859 4114361388	SUTCHKIN CREEK NR HOOPER, OREG.	240	3.67	389.1	42 12 123 19	78	4.18	1.43	1818	2.68	2.78	0.8	248.8	266.1	575.7	659.3
860 4114374888	BOYCE CREEK NR HOOPER, OREG.	240	3.16	389.1	42 12 123 19	78	4.18	1.43	1818	2.68	2.78	0.8	248.8	266.1	575.7	659.3
861 4114371388	GRAVE CREEK NR PEAS BROOK NR PLACER, OREG.	240	82.16	3741.7	42 38 123 13	88	4.88	1.43	2838	16.37	15.28	0.8	1918.8	1918.8	8863.4	7881.2

## Zone 21

Zone 22



Table 10. Continued.

STATION	STATION NAME	Zone 22																
		2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
STATION	STATION NAME	RECT AREA	519	LAT	LONG	R	P10	P80	GM	L	LL	STNG	QMEAN	QR	Q50	Q100	SPEAK	
913 411514300	KENNE CREEK NEAR ASHLAND, OREG.	330 11.86	352.1	41 59 123.58	88	4.88	1.43	1.87	0.38	1.99	0.8	137.5	81.1	79.5	843.5	723.5	751.5	
914 4114152100	GLINN CREEK NEAR SALEM, OREG.	340 2.78	352.1	41 57 123.58	88	4.88	1.43	1.87	0.38	1.99	0.8	137.5	81.1	79.5	843.5	723.5	751.5	
915 4114159000	HARKING CREEK NEAR MCINNINVILLE, OREG.	340 8.78	331.8	48 08 123.28	78	4.88	1.33	1.89	4.48	0.8	0.8	318.5	288.7	288.7	728.8	728.8	816.8	
919 4114233000	BEAVER CREEK NEAR GLENWOOD, OREG.	340 4.31	379.0	45 38 123.15	95	4.88	1.02	3.88	0.88	4.08	0.8	227.7	248.8	253.3	820.2	820.2	472.8	
917 4114241000	BAYMEN CREEK NEAR GLENWOOD, OREG.	340 1.37	350.8	45 37 123.15	95	4.88	1.43	1.88	1.78	1.78	0.8	88.3	86.3	189.1	151.6	145.6		
918 4114289000	ABURNY CREEK NEAR CANNON BEACH, OREG.	340 1.07	353.8	46 46 123.88	78	6.18	0.81	3.28	2.88	3.77	0.8	816.8	811.6	337.8	338.6	314.8		
919 4114312100	PARROT CREEK AT ROSEBURG, OREG.	340 8.48	348.8	43 12 123.21	45	4.88	1.33	4.18	3.18	0.18	0.88	157.8	159.8	288.4	388.2	388.8		
917 4114324000	GETTYS CREEK NEAR FORT ORVING, OREG.	340 1.48	358.8	43 08 124.13	148	0.88	1.91	2.88	1.48	0.8	0.8	147.1	158.8	288.8	314.8	245.8		
911 9011915000	SWEETWATER RIVER NEAR DESCANDO, CALIF.	88 48.08	1418.1	38 56 119.37	48	4.07	1.81	2175	14.38	48.08	0.8	1189.7	451.9	1141.8	14347.2	11889.8		
918 9011927000	GUZITO CREEK NEAR SAN FASUAL, CALIF.	88 88.08	1533.4	33 07 119.57	88	4.88	1.82	2488	10.88	44.88	0.8	435.4	319.7	351.2	4218.8	2888.8		
913 9011933000	RIVER SAN LUIS REY P. MR. WARNER 8045, CALIF.	88 28.88	3088.2	33 10 119.46	188	4.07	1.81	2588	11.88	48.88	0.8	948.8	332.4	712.8	9738.8	2 4888.8		
914 9011944000	DE LUZ CREEK NEAR FALLOON, CALIF.	88 47.88	2485.7	33 22 117.19	88	5.78	1.84	1888	12.11	87.47	0.8	848.8	381.1	431.7	4811.4	2888.8		
925 9011945100	LAS FLORES CREEK NEAR OCEANVIEW, CALIF.	88 35.88	787.3	33 18 117.87	18	3.88	1.33	2888	10.18	95.88	0.8	728.2	277.2	358.8	167.8	288.8		
926 9011946100	HAN OYRE CREEK NEAR SAN ONOFRE, CALIF.	88 28.88	1788.8	33 27 117.34	88	4.38	1.88	2338	8.32	44.18	0.8	888.8	283.3	318.2	3812.8	1888.8		
927 9011948350	CHATTANTON CREEK NEAR SAN CLEMENTE, CALIF.	88 38.78	2884.1	33 32 117.48	88	4.38	1.88	4338	10.88	95.88	0.8	978.8	388.3	8788.8	13881.2	9248.8		
929 9011947800	ARRIVO TRABUCO NR SAN JUAN CAPISTRANO, CALIF.	88 38.78	2884.1	33 32 117.48	88	4.38	1.88	4338	10.88	95.88	0.8	978.8	388.3	8788.8	13881.2	9248.8		
Zone 23																		
930 9015018000	MINSTANLEY CREEK NR KETCHIKAN, ALASKA	ALBK 10.08	6131.8	58 26 138.52	188	2.35	0.92	2888	7.37	19.48	10.8	1383.7	1217.6	2078.3	3379.3	4128.8		
939 9015026000	CASCADE CREEK NR PETERSBURG, ALASKA	ALBK 23.08	2886.8	57 01 132.47	138	1.98	0.85	4888	10.87	16.73	1.8	1781.9	1781.9	3216.8	3588.8	3288.8		
931 9015048000	DOROTHY CREEK NR JUNEAU, ALASKA	ALBK 10.28	1984.8	58 14 134.98	88	1.98	0.84	4881	7.14	19.84	14.8	915.1	888.8	1841.4	1988.4	1788.8		
940 9015048000	SHEEP CREEK NR JUNEAU, ALASKA	ALBK 4.87	748.8	58 16 134.18	77	1.88	0.81	3318	3.38	8.88	9.8	488.8	488.8	923.8	984.8	848.8		
933 9015049000	GOLD CREEK AT JUNEAU, ALASKA	ALBK 9.78	6182.8	58 16 134.24	77	1.88	0.82	4888	5.13	8.81	9.8	1382.8	1388.1	3338.4	3838.2	2888.8		
934 9015080000	PERKINANCE CREEK NR WICKER, ALASKA	ALBK 2.81	988.2	58 20 131.48	188	2.38	0.89	2484	3.08	8.47	8.8	584.2	574.8	888.7	888.7	888.8		
945 9015081000	OLD TONGUE CREEK NR BARANOF, ALASKA	ALBK 10.18	782.8	58 24 132.58	188	2.48	0.88	5182	7.37	9.18	2.8	888.8	877.8	4788.1	5827.8	4178.8		
937 9015081000	POWER CREEK NR CONROVA, ALASKA	ALBK 28.88	4888.8	58 35 145.37	178	1.88	0.78	3888	10.88	87.88	3.8	2873.2	2873.2	8878.8	7284.8	5348.8		
Zone 24																		
938 1510610000	KAWAIKOI STREAM NR MAINEA, HAWAII	MA 44.18	6883.3	22 08 159.37	818	3.98	1.88	1188	5.88	4.34	88.8	4878.8	4189.8	11828.2	13887.8	11388.8		
939 1510610000	KAWAIKOI RIVER NR MAINEA, HAWAII	MA 88.88	13388.8	21 07 159.48	818	3.98	1.88	1188	5.88	4.34	88.8	4878.8	4189.8	11828.2	13887.8	11388.8		
940 1510648000	MANAPEPE R. BELOW KEMANA OTCH INTAKE NR MAINEA, HAWAII	MA 19.48	18816.8	21 07 159.33	808	9.13	3.88	3888	7.38	23.48	8.8	8718.4	8718.4	2438.8	3288.8	3288.8		
941 1510680000	S P MAILUA RIVER NR LIMUE, HAWAII	MA 88.48	34838.8	22 07 159.23	808	18.84	3.98	4888	10.23	83.88	8.8	1788.2	1788.2	5387.8	6888.8	6888.8		
948 1510680000	E BRCH N P MAILUA RIVER NR LIMUE, HAWAII	MA 8.88	6184.8	22 04 159.28	808	11.88	4.48	2888	3.38	16.88	8.8	3888.8	3188.1	7814.4	8787.8	18888.8		
943 1510800000	N P KAUONAHUA STN AB RGT BR NR HANAUANA, HAWAII	MA 1.38	4388.1	21 31 157.87	988	11.18	4.21	388	3.78	8.28	8.8	888.8	888.8	888.8	888.8	888.8		
944 1510800000	NORTH MALANA STREAM NEAR AIEA, HAWAII	MA 3.48	8888.4	21 24 157.84	988	9.18	3.38	388	3.78	8.28	8.8	888.8	888.8	888.8	888.8	888.8		
945 1510800000	HONOLULU STREAM NR HONOLULU, HAWAII	MA 23.78	11848.1	21 25 157.84	988	9.18	3.38	388	3.78	8.28	8.8	888.8	888.8	888.8	888.8	888.8		
946 1510833000	HONOLULU STN, 814 RES 2 HSTNY NR HONOLULU, HAWAII	MA 3.38	2388.8	21 21 157.88	418	8.78	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
947 4358028000	RIO TANAHUA NR UYUADO PUERTO RICO	PR 18.48	8888.8	18 28 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
948 4358031800	RIO SANA MUERTO NR ORCOVIA PUERTO RICO	PR 3.88	1888.8	18 18 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
948 4358043000	RIO DE LA PLATA AT PROYECTO LA PLATA PUERTO RICO	PR 84.88	87844.8	18 18 88.14	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
949 4358043000	RIO LAJAS AT YAO ALTA PUERTO RICO	PR 8.48	8133.8	18 23 88.18	388	10.48	3.18	848	4.18	18.88	8.8	8421.3	1743.3	7778.4	8818.4	8788.8		
951 43580671800	RIO FAJARDO NR FAJARDO PUERTO RICO	PR 14.88	14847.8	18 18 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
952 4358110000	RIO ESCALABRADO NR LOS LLANOS, PUERTO RICO	PR 18.88	18488.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
953 4358114000	RIO CERRILLOS NR PONCE, PUERTO RICO	PR 17.88	18888.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
954 4358114000	RIO BUCANA NR PONCE, PUERTO RICO	PR 28.88	18888.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
955 4358119000	RIO PORTUENOS NR PONCE, PUERTO RICO	PR 8.88	8888.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
956 4358121000	RIO PORTUENOS AT HIGHWAY 14 AT PONCE, PUERTO RICO	PR 14.88	14847.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
957 4358121000	RIO TALLARUA AT PERQUELAS PUERTO RICO	PR 14.88	14847.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		
958 4358124000	RIO GUAYANILLA NR GUAYANILLA PUERTO RICO	PR 18.88	4818.8	18 23 88.48	488	11.08	3.88	1888	8.24	3.37	8.8	1128.8	1813.3	4681.2	5884.8	8888.8		

Table 10. Continued. Explanation of column headings for Table 10.

1. Station. This column has three types of numbers. The first two designate what state the number applies to. The balance of the numbers fall into two categories--the first would be the USGS designation. For example the first station is 2704015500. The 27 says Minnesota. The 04 says the station drains into the Great Lakes basins while the balance of the numbers designate a particular watershed. The second category of numbers are the ARS numbers. For example the last station of Zone 01 is 12 ARS 8.3. The 12 says the state is Florida and the ARS says this is an ARS watershed and the 8.3 says it is watershed No. 3 at Ft. Lauderdale.
2. Sect. This is the physiographic section from the Fenneman and Johnson map.
3. Area. This is the watershed area in square miles.
4. Q10. This is the 10-year runoff peak in cfs.
5. Lat. Latitude in degrees and minutes.
6. Long. Longitude in degrees and minutes.
7. R. This is the mean annual rainfall erosivity index explained in the text.
8. P10. The 10-year 10-minute rainfall intensity in inches per hour.
9. P60. The 10-year 1-hour rainfall in inches.
10. DH. The difference in elevation between the intercept of the main drainage channel with the watershed boundary and the culvert site. Feet.
11. L. This is the length of the principal drainage channel from the culvert site to the watershed rim. Miles.
12. LL. The total lengths of all drainage channels on a watershed. Miles.
13. STRG. The percentage of the watershed covered by lakes, swamps, playas, etc.
14. QMEAN. The arithmetic mean of the annual runoff peaks in cfs.
15. Q<sub>2</sub>. The 2-year or median runoff peak in cfs.
16. Q<sub>50</sub>. The 50-year runoff peak in cfs.
17. Q<sub>100</sub>. The 100-year runoff peak in cfs.
18. Q<sub>PEAK</sub>. The maximum runoff of record in cfs.

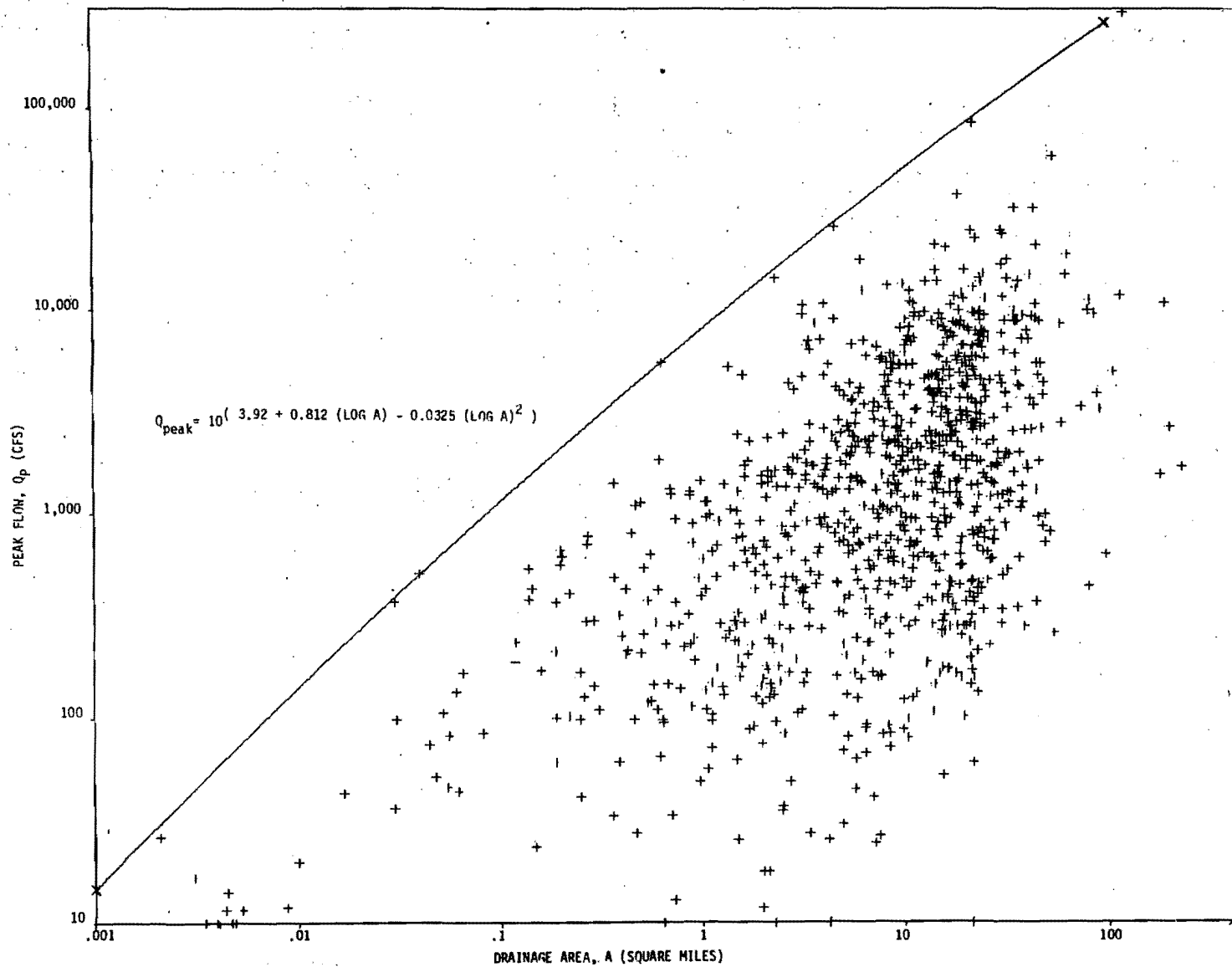


Figure 39. The probable maximum runoff peaks and envelope curve for different sized areas in the United States and Puerto Rico.

In its  $\log_{10}$  linear form, it has a multiple correlation coefficient,  $r$ , of 0.854 with 894 degrees of freedom and a standard error,  $PS_e$ , of 13 percent of  $\log_{10} (\bar{q}_{10})$ . This amounts to a standard error of a point estimate,  $PS_{EE}$ , of 119 percent of  $\bar{q}_{10}$ .

A nomograph for the solution of the equation was constructed and is shown in Appendix H-00. The curve for the relationship between the measured and estimated values of  $q_{10}$  with the 95 percent confidence intervals about the estimate for the standard error and the standard deviation of a point estimate are shown in Appendix G-00.

The second set of regressions used the same three parameters but a separate equation was computed for each of the 24 hydrophysiographic zones. These 24 3-parameter zoned regression equations are shown in Appendix H as Table H-1 along with the standard error of the estimate of  $\hat{q}_{10}(\text{ARDH})$  as a percent of the measured  $\bar{q}_{10}$  for the data used in its development. Nomographs for each equation were constructed and are shown in Appendix H-1 through H-24.

The third regression was a 5-parameter all zone equation. This equation is

$$\hat{q}_{10} = 1.5102 A^{0.4707} R^{0.8386} DH^{0.1718} L^{0.1764} P_{60}^{0.3476}$$

In its  $\log_{10}$  linear form, it has an  $r$  of 0.856 with 892 degrees of freedom and a percent standard error,  $PS_e$ , of 12 percent of  $\log_{10} (\bar{q}_{10})$ . This translates into a percent standard error of a point estimate,  $PS_{EE}$ , of 116 percent of  $\bar{q}_{10}$ .

The fourth set of regressions are the equations obtained by zoning and then determining the individual zone regressions using 5 parameters for each of the 24 hydrophysiographic zones. These equations are given in Appendix H as Table H-2 with their respective percent standard errors of estimate and numbers of watersheds used in their development.

The fifth regression involved the 7 parameters  $A$ ,  $R$ ,  $DH$ ,  $L$ ,  $LL$ ,  $P_{10}$ , and  $P_{60}$  and is called the 7-parameter all zone equation. This equation is

$$\hat{q}_{10} = 1.8816 A^{0.3877} R^{0.8322} DH^{0.1461} L^{-0.0236} LL^{0.2613} P_{10}^{-0.1891} P_{60}^{0.4668}$$

This equation has an  $r$  of 0.860 in its  $\log_{10}$  linear form with a  $PS_e$  of 12 percent with 890 degrees of freedom. This yields a  $PS_{EE}$  of 116 percent of  $\bar{q}_{10}$ .

The sixth set of regressions are the equations obtained when the individual zone data are regressed using the same 7 parameters. The different zone equations are shown with the respective errors of estimate as percentages of the measured  $\bar{q}_{10}$  in Appendix H, Table H-3.

An effort to improve the predicting ability of the 3-parameter all zone equation analogous to that used in the original Potter method was made by

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Appendices A is in Volume III

Appendices B, C, D, E, F, G, and H are in Volume II

regressing  $\log_{10}$  of the observed 10-year runoff peaks,  $q_{10}$ , for each zone against  $\log_{10}$  of the estimated 10-year peaks given by the 3-parameter all zone equation,  $\hat{q}_{10}(3AZ)$ . The correction equations thus derived are given in Appendix H, Table H-4. In addition, a scatter diagram and a plot of the correction curve derived for each zone along with the 95 percent confidence intervals for a mean and for a point estimate are given in Appendix H as Appendixes H-25 through H-48 for zones 1 through 24 respectively. The percent standard error of estimate,  $PS_{EE}$ , was reduced for most zones with the average reducing from 91.5 percent for the uncorrected estimates to 87.1 for the corrected estimates. Although this reduction is not very appreciable, a major reason for using the correction equation and curves with the 3-parameter all zone equation is to reduce the bias that it may have when used in a particular zone.

A summary of the respective prediction errors expected from the various regression equations derived above is tabulated in Table 11 (p. 95). The 3-parameter all zone equation appears to be nearly as good as the other two all zone equations, with the  $PS_{EE}$  being reduced only 3 percent by including more than the three independent variables included in the 3-parameter equations and the  $r$  increasing only from 0.854 to 0.860 with a corresponding decrease of only 1 percent in  $PS_e$ . The same observation is generally true for the individual zone equations. In those zones where significant improvement appears, e.g., zone 6, the number of observations is small; for zone 6 there are only 12 so the degrees of freedom are reduced as the number of variables increases (for zone 6 the degree of freedom goes from 8 for the 3-parameter equations to only four for the 7-parameter equations) and the improvement is not as statistically significant as it may appear from Table 11 (p. 95). Therefore, the three parameter equations, either the all zone or the individual zone equations are preferred for practical use. As mentioned previously all of the equations have been tabulated in Appendix H and nomographs for graphically solving the 3-parameter equations have been prepared and are also included there. In addition, the 95 percent confidence limits for a mean and for an individual point estimate as well as the point scatter are shown in Appendix G for all of the 3-parameter equations derived from the 898 watersheds having negligible storage.

#### Methods test

Fifty-one watersheds, one from each State and Puerto Rico, were selected at random from the watersheds not used in the regressions except where data for the SCS method were unavailable. Here a second choice was made. For these watersheds the measured  $q_{10}$ , and  $q_{10}$  estimates using the SCS method, the 3-parameter all zone and zoned and the 7-parameter all zone and zoned were calculated and are shown in Table 12 (p. 96). The standard errors of estimate for each of the 5 procedures are shown at the bottom of Table 12 (p. 96). The mean error is the algebraic sum of the percentages of the estimate divided by 51. The average error is the average of the magnitudes of the errors. The percent standard error of the estimate is a percentage of the mean value of the measured ten year peaks,  $\bar{q}_{10}$ .

#### Summary and Conclusions

A curve was developed which the writers call the probable maximum runoff peak. It has the equation

Table 11. Summary of the prediction errors associated with estimating 10-year peak runoff from the various regression equations given in Appendix H.

Zone	$\bar{q}_{10}$ cfs	n	3-Parameter Equations			5-Parameter Equations			7-Parameter Equations			Un-corrected 3-Parameter All Zone Eq. Corrected by Zone			
			PS <sub>EE</sub> %	PS <sub>e</sub> %	r	PS <sub>EE</sub> %	PS <sub>e</sub> %	r	PS <sub>EE</sub> %	PS <sub>e</sub> %	r	PS <sub>EE</sub> %	PS <sub>EE</sub> %	PS <sub>e</sub> %	r
All Zone	1922	898	119	13	0.854	116	13	0.856	116	12	0.860	119	119	13	0.854
1	1058	42	84	13	0.774	76	11	0.844	67	11	0.876	97	92	16	0.593
2	4747	28	60	7	0.798	59	7	0.818	59	7	0.831	68	67	8	0.754
3	2295	14	108	9	0.925	110	10	0.930	97	11	0.934	105	105	9	0.912
4	1979	62	56	9	0.795	54	9	0.809	53	9	0.809	63	60	9	0.770
5	1472	35	44	8	0.927	51	8	0.931	45	8	0.942	58	75	8	0.912
6	2014	12	88	7	0.840	32	4	0.970	33	5	0.971	92	92	10	0.622
7	2306	33	76	7	0.918	76	7	0.919	79	7	0.929	103	88	7	0.893
8	2079	39	51	7	0.952	47	6	0.964	44	6	0.968	57	62	7	0.944
9	1170	37	85	8	0.850	87	8	0.865	83	8	0.879	88	88	9	0.800
10	1986	10	67	12	0.882	68	13	0.905	47	17	0.914	76	83	14	0.745
11	4320	32	43	7	0.902	42	6	0.921	39	6	0.923	81	61	9	0.764
12	461	34	115	21	0.672	115	20	0.749	89	19	0.793	106	107	23	0.587
13	2260	166	83	12	0.897	82	12	0.899	85	12	0.901	108	91	13	0.887
14	1304	30	132	17	0.762	134	17	0.789	133	18	0.796	133	121	18	0.704
15	356	37	91	14	0.795	91	14	0.800	97	14	0.808	118	101	21	0.375
16	624	21	95	8	0.897	73	7	0.940	72	7	0.941	88	73	8	0.893
17	368	56	89	15	0.784	71	14	0.809	76	14	0.825	107	98	18	0.622
18	1311	14	107	23	0.643	88	24	0.708	117	20	0.857	143	124	23	0.520
19	1586	40	83	13	0.833	82	13	0.833	82	13	0.838	125	84	13	0.807
20	759	42	103	10	0.926	104	9	0.936	106	10	0.937	103	131	12	0.883
21	1625	68	67	8	0.924	68	7	0.931	69	8	0.931	94	138	11	0.836
22	1013	22	36	5	0.974	34	4	0.979	30	4	0.986	45	38	5	0.966
23	2519	6	35	5	0.961	-	-	-	-	-	-	47	40	6	0.886
24	12277	18	56	5	0.882	42	4	0.917	34	4	0.924	92	72	6	0.772
Average error of estimate			77.3%			73.3%			71.2%			91.5% 87.1%			

Notes explaining the column headings:

$\bar{q}_{10}$  is the mean ten year peak flow calculated from the observed ten year peak flows for each zone.

n is the number of watersheds used in deriving the equation.

PS<sub>EE</sub> is the standard error of estimate expressed as a percent of the zone  $\bar{q}_{10}$ . It is calculated by the equation:

$$PS_{EE} = \frac{100}{\bar{q}_{10}} \sqrt{\frac{\sum (q_{10} - \bar{q}_{10}(K))^2}{n - 2}}$$

PS<sub>e</sub> is the standard error of the log<sub>10</sub> linear equation expressed as a percent of log<sub>10</sub> $\bar{q}_{10}$ . It is calculated by the equation:

$$PS_e = \frac{100}{\log_{10} \bar{q}_{10}} \sqrt{\frac{\sum (\log_{10} q_{10} - \log_{10} \bar{q}_{10}(K))^2}{n - 2}}$$

r is the correlation coefficient between  $q_{10}$  and  $\bar{q}_{10}$ . It is calculated by the equation:

$$r = \frac{\sum (x - \bar{x})(y - \bar{y})}{\sqrt{\sum (x - \bar{x})^2 \sum (y - \bar{y})^2}}$$

where x and y are any two independent and dependent variables respectively.

Table 12. Comparison of 10-year peak flow estimates by the SCS method and four USU methods.

State	Measured $q_{10}$	SCS Method $q_{10}$		USU (3-Parameter All Zone)		USU (7-Parameter All Zone)		USU (3-Parameter Zoned)		USU (7-Parameter Zoned)	
		$q_{10}$	%	$q_{10}$	%	$q_{10}$	%	$q_{10}$	%	$q_{10}$	%
		Estimate	Error 1 <sup>a</sup>	Estimate	Error 2	Estimate	Error 3	Estimate	Error 4	Estimate	Error 5
Alabama	2,070	650	-219	2,162	+ 4	1,461	- 42	1,363	- 52	1,441	- 44
Alaska	2,131	350	-509	2,457	+ 13	1,366	- 56	3,025	+ 30	2,104	- 1
Arizona	265	900	+ 71	124	-112	139	- 90	347	+ 24	329	+ 19
Arkansas	1,790	1,200	- 49	1,289	- 39	1,121	- 59	2,145	+ 17	1,693	- 6
California	385	1,200	+ 68	454	+ 15	369	- 4	214	- 80	322	- 19
Colorado	271	165	- 64	353	+ 23	421	+ 36	666	+ 59	851	+ 68
Connecticut	1,562	350	-346	1,556	+0.3	1,379	- 13	1,172	- 33	925	- 69
Delaware	3,489	150	-2226	1,485	-135	1,470	-137	1,266	-175	1,119	-211
Florida	2,064	3,500	+ 41	3,495	+ 41	2,959	+ 30	1,162	- 77	2,117	+ 2
Georgia	667	70	-853	682	+ 2	569	- 17	425	- 56	405	- 65
Hawaii	6,655	5,200	- 28	6,473	- 3	10,395	+ 36	6,408	- 4	5,465	- 22
Idaho	153	700	+ 78	485	+ 68	639	+ 76	248	+ 39	313	+ 51
Illinois	630	290	-117	249	-100	342	- 84	358	- 76	380	- 65
Indiana	6	10	+ 40	8.8	+ 3	40	+ 84	10	+ 41	19	+ 69
Iowa	1,202	950	- 27	632	- 90	593	-103	1,143	- 5	933	- 29
Kansas	1,142	1,200	+ 5	875	- 31	1,186	+ 4	1,858	+ 39	2,612	+ 56
Kentucky	2,382	380	-527	2,279	- 4	2,213	- 8	4,238	+ 44	3,633	+ 34
Louisiana	6,457	2,000	-223	6,824	- 5	5,035	- 28	4,016	- 61	4,558	- 42
Maine	248	500	+ 50	446	+ 44	368	+ 33	345	+ 28	285	+ 12
Maryland	530	2,400	+ 78	825	+ 36	934	+ 43	430	- 23	421	- 26
Massachusetts	245	350	+ 29	606	+ 60	581	+ 58	436	+ 44	364	+ 33
Michigan	617	1,000	+ 38	872	+ 29	910	+ 32	516	- 20	772	+ 20
Minnesota	2,415	1,000	-142	1,303	- 84	1,116	-116	2,053	- 18	1,900	- 27
Mississippi	4,961	2,200	-126	5,523	+ 10	4,821	- 3	4,294	- 15	4,611	- 8
Missouri	908	1,200	+ 24	460	- 99	572	- 59	681	- 33	640	- 42
Montana	531	300	- 77	244	-118	311	- 71	857	+ 38	893	+ 41
Nebraska	790	390	-103	403	- 96	196	-303	279	-183	353	-124
Nevada	550	2,800	+ 80	551	0	241	-128	189	-191	336	- 64
New Hampshire	487	375	- 30	902	+ 38	762	+ 36	614	+ 21	517	+ 6
New Jersey	726	1,400	+ 48	432	- 68	378	- 92	368	- 97	458	- 58
New Mexico	923	2,000	+ 54	849	- 9	925	0	1,133	+ 18	833	- 11
New York	1,181	300	-294	1,142	- 3	1,221	+ 3	701	- 68	1,177	0
North Carolina	1,596	440	-263	1,881	+ 15	1,362	- 17	1,332	- 4	1,592	0
North Dakota	102	820	+ 88	386	+ 73	276	+ 63	197	+ 48	326	+ 68
Ohio	845	800	- 6	596	- 41	519	- 63	1,124	+ 25	822	- 3
Oklahoma	12,250	28,000	+ 56	12,244	0	8,060	- 52	14,962	+ 18	11,657	- 5
Oregon	200	900	+ 78	232	+ 14	246	+ 19	232	+ 14	313	+ 36
Pennsylvania	1,770	300	-490	1,784	+ 1	888	- 99	1,249	- 42	1,324	- 33
Puerto Rico	8,569	8,900	+ 5	6,567	- 30	6,426	- 33	10,553	+ 19	9,484	+ 10
Rhode Island	652	300	-117	1,869	+ 65	1,698	+ 62	1,593	+ 59	1,076	+ 39
South Carolina	3,354	500	-571	5,382	- 38	4,049	+ 17	4,560	+ 26	3,989	+ 16
South Dakota	98	1,600	+ 94	387	+ 76	406	+ 76	208	- 53	393	+ 75
Tennessee	3,325	610	-445	2,704	- 23	2,994	- 11	3,925	+ 15	4,111	+ 19
Texas	75.2	30	-150	83	+ 10	63	- 19	83.9	+ 10	88.2	+ 15
Utah	25	510	+ 95	230	+ 89	273	+ 91	79	+ 68	36	+ 27
Vermont	972	345	-182	1,218	+ 20	985	+ 1	1,336	+ 27	1,133	+ 14
Virginia	22.8	70	+ 67	35.9	+ 32	41	+ 44	30.7	+ 26	28.1	+ 19
Washington	211	800	+ 74	533	+ 60	723	+ 71	382	+ 45	420	+ 49
West Virginia	1,644	220	-647	1,485	+ 10	1,581	- 4	1,921	+ 14	1,961	+ 16
Wisconsin	68.4	240	+ 72	72	+ 5	76	+ 10	104	+ 34	84	+ 18
Wyoming	537	1,500	+ 65	596	+ 10	650	+ 17	594	+ 9	690	+ 22
Mean error		-145.75		-3.14		-15.08		-8.08		-1.12	
Average error		200.57		40.32		51.24		44.41		37.08	
Percent Standard Error of Estimate		160		39		62		50		38	

<sup>a</sup> Errors 1, 2, 3, 4, and 5 are a percent of the estimate. The standard error of the estimate values are percent of measured  $q_{10}$ .

$$q_{p(\max)} = 10^{[3.92 + 0.812 (\log A) - 0.0325 (\log A)^2]}$$

Besides standard procedures used for sizing of culverts and using rating curves and other factors, the above curve can be used to determine the maximum safe watershed area that can be used with each size of culvert installation. In those States that administratively use a certain minimum culvert size this would give the maximum size watershed which did not require a  $Q_{10}$  estimate of runoff peak.

For practical field use, it appears that a 3-parameter all zone regression may be used to forecast runoff peaks with a mean standard error of the estimate of about 119 percent. Additional parameters up to seven did not appreciably improve these estimates. If zoning is introduced the standard errors of the estimate range between 35 percent and 132 percent with an average value of 77 percent. An appreciable portion of the error comes from zones 3, 12, 14, 18, and 20. It is felt that the map of R might be the source of at least a portion of the errors in these zones.

Nomographs have been constructed for the solution of all of the 3-parameter equations (see Appendix H).

All of the pertinent parameters may be measured from 7½ or 15 minute quadrangle maps. When these maps are not available the 3-parameters can be reliably measured from the 1:250,000 scale maps but the remaining parameters require use of correction curves.

The writers suggest calling these procedures the FHWA method and suggest that it not presently be used on areas larger than 100 square miles.

### PHASE III

#### Field Visits and Evaluation

Contacts were made with at least one person from each State. As a general rule, the personnel of the States were very cooperative and candid. Always, they went to great lengths to supply the information requested.

#### Questions

The questions asked in the interviews always embodied some form of the following:

1. Have you or do you use Potter's method? If so, what do you like about it and what do you dislike about it?
2. What methods do you use for the hydrologic design of culverts for small watersheds? What do you like and dislike about each?
3. Do you utilize the storage provided by fills or borrow areas to decrease peak flows? Do you include such storage in your design?
4. In what form would you prefer to have a design method presented?
5. Have the methods you use given you the kind of results you had hoped for?



6. Do you consider a certain percentage of failures as a normal part of the risk?

7. When there are failures do you or members of your staff have an opportunity to see and study the failure or are failures buried by repairs before you are aware of them?

### Results

The answers to the questions may be summarized as follows:

1. Only three of the specialists queried had ever used Potter's method. Two or three others said they thought the method had been tried in their State prior to the time of the interview. A common academic reaction was that the T factor was too difficult to keep straight whether the upper 0.3 or the lower 0.7 was the division point of the slope, thus introducing blunders. Universally, personnel liked the general format of the Potter method. Generally separate groups were preferred. They liked cycle graphs, alignments charts, or other mechanical procedures or aides.

2. The most common and universally used method was some form of the Rational method. Most States used the Rational method for areas smaller than 200 acres, but one State used the Rational method on all drainages regardless of size even to large rivers. Incidentally, this State reported no knowledge of failures since adopting the Rational method. From personal observations in that State, it appeared their designs approached sizes accommodating floods approaching the magnitude of the maximum probable flood. Another observation in connection with the use of the Rational method was the general failure to observe the lower limit on the time of concentration or lag time whichever was in use.

By far the most widely used forecast methods were those proposed by the local U.S. Geological Survey units for States who had progressed this far in the small watersheds program such as Texas (Schroeder, ref. 15). In general, the States have faith in these methods but would much prefer more computational aids such as those of Schroeder, enabling project engineers to make field checks if questions arise. In Texas, these aids were furnished. In one or two others, States had constructed their own aids.

Two States were using some of the old sewerage formulas for watersheds of all sizes. In contrast to this, 15 States have multiple methods which they use to duplicate each flood design estimate. They adopt the result which seems most logical from engineering judgment.

No State was completely happy with the method or methods they were using and most personnel expressed a desire for improvement. With such a desirable attitude, improved methods should be forthcoming and will be rapidly adopted.

3. Only five States were using fill or borrow storage to reduce the magnitudes of flows and thus to reduce culvert size. The distinct impression was gained, however, that an appreciable number of States were contemplating moves to include storage as a part of the design procedure. In these States, a

complete flood peak hydrograph of the flow becomes desirable or necessary. Only one State was beginning to accumulate hydrograph data at the time of the visit, however.

4. The preferred format in nearly all States was some type of graphical or alignment chart suitable for the use of project engineers in the field. Nearly a dozen States expressed a need to incorporate maximum versatility in any new method.

5. No State was completely satisfied with its present method. Each State felt its method either over or under designed or was too complex. The writers believe the real problem here lies in their concept of a risk factor. Thus it is also involved in question No. 6.

6. The great majority of interviewees felt that some failures should occur, but the same people also felt strongly that failures should only result from unusual events or circumstances. Use of risk factors needs to be formulated in a better way for convenient field use. If a culvert is designed to accommodate a 50-year storm with a 5 percent chance of failure, the structure must actually accommodate a 1000-year flood event. The impression was gained that personnel were not commonly familiar with this concept.

7. Nearly all States had the problem of feedback to the designer. If failures occur on small culverts, the damage is nearly always repaired by maintenance crews before the design people are aware that any damage occurred. Naturally, this is not true of crossings on larger streams or where bridges are involved. It would certainly appear to be beneficial to design people to be informed of any kind of failure in time to examine each incident prior to repair, particularly failures from small area floods.

#### PHASE IV

##### Evaluation and Analysis

A. Conclusions from a study of flood damage. As a direct consequence of the study of flood damage and the literature, the writers feel that flood damage can best be expressed by the stage. This is illustrated by Figure 40 (p. 100) which is computed from the data of Grigg and Helweg (ref. 8a) and fit to a log-extremal distribution. The maximum stage as used here to reduce the data to a common base is the maximum for each group of data and the maximum damage is the damage resulting from this maximum head. The data show considerable scatter depending upon the location but all seem to linearize with the transforms used.

It seems clear that the single most important flood parameter is stage. The peak flows are exponentially related to stage. Thus the advantages of using fills to reduce culvert sizes must be carefully weighed against the increased flood hazard as a result of increasing the stage.

B. Frequency problems. The details of the frequency problems have been discussed in earlier sections so the relative risk involved only, will be

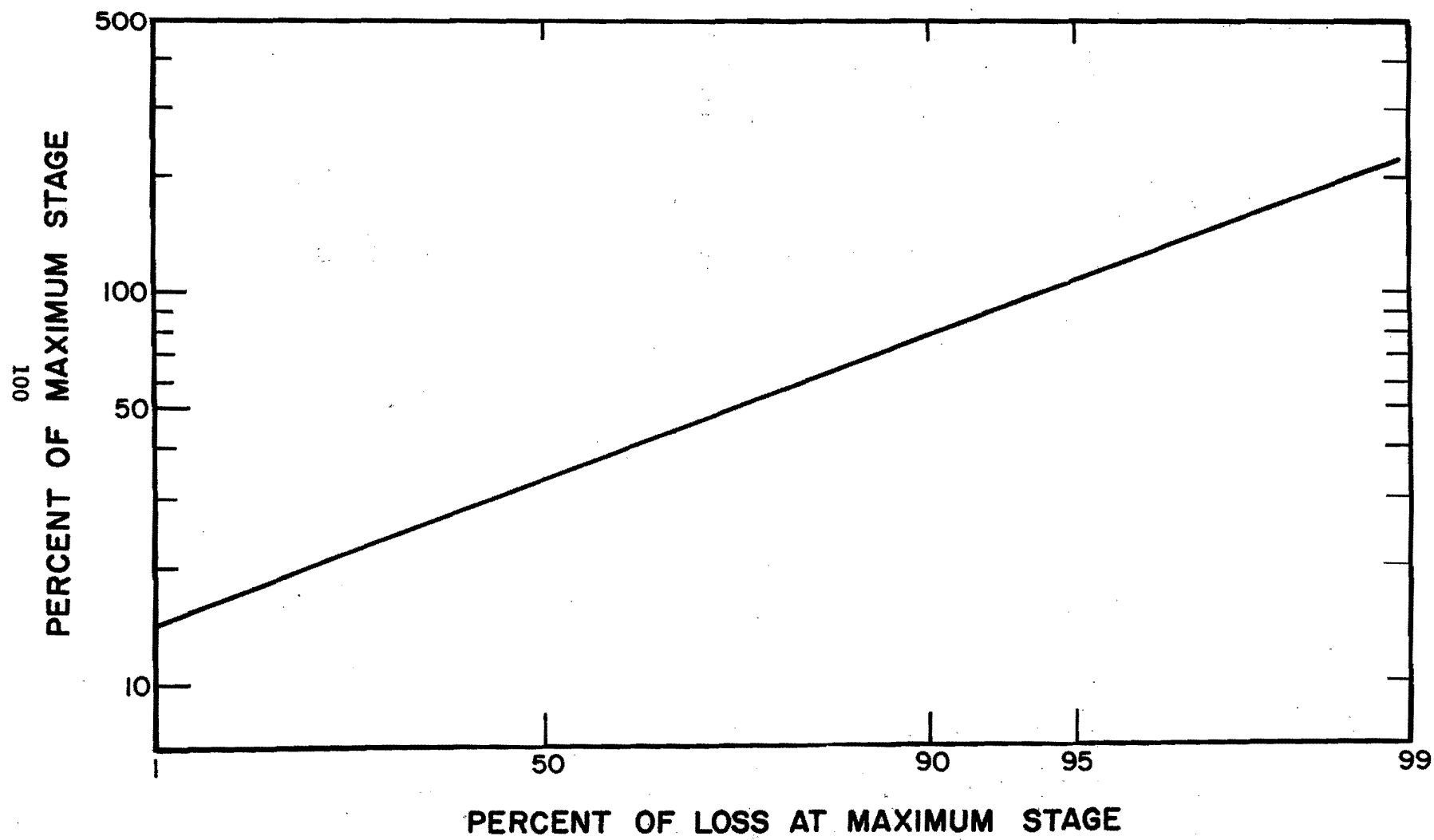


Figure 40. The relationship of percent of maximum stage to the percent of damage at maximum stage.

discussed here. Figure 41 (p. 102) shows the percent chance of having floods in the size for the different return periods occur in different time periods from one year to 100 years. The randomly generated frequency curves do not generally coincide with the curves from actual data. For example, Figure 42 (p. 103) shows a generated set of data compared to the measured data for the USGS watershed numbered 06658500, Laramie River near Jelm, Wyoming.

C. Conclusions from entire study. The validity of Potter's approach was verified and updated equations and graphs were produced using current and corrected data for the 96 watersheds originally selected by Potter for deriving his method.

When 25 watersheds were chosen at random from the same States from which Potter chose his watersheds, the standard errors of estimate were as follows:  $q_{10}(\text{ATPK}) = 127$  percent, zoned into Potter zones 157 percent and  $q_{10}(\text{ATPC})$  (using Potter's published curves) = 197 percent of the measured values.

The parameters used by Potter were simplified into their measured components and one additional parameter, R, introduced. Furthermore, the zoning used by Potter was extended and simplified through the use of the physiographic sections of the United States. Through use of the measured 10-year peak runoff per unit area and the statistical t test, these zones were grouped until 24 covered all of the United States and Puerto Rico. The parameters measured on a large sample of watersheds representing these 24 zones were area (A), 10-year peak runoff ( $q_{10}$ ), location (Latitude and Longitude), iso-erodent value (R), 10-year 10-minute precipitation ( $P_{10}$ ), 10-year 60-minute precipitation ( $P_{60}$ ), difference in elevation between the watershed rim at the extension of the principal drainage and the culvert site (DH), the length of the principal drainage (L), the total lengths of all drainages on each watershed and the storage (S).

Regressions between  $q_{10}$  and these parameters were made using various transforms and each parameter weight evaluated with the statistical F test. These regressions were made both on an all sample basis and by the above hydro-physiographic zones.

It was shown that most of the variation in  $q_{10}$  could be accounted for by area, A, R, and elevation difference DH. The standard error of estimate for the three parameters, when  $\hat{q}_{10}$  estimated was compared to  $q_{10}$  measured as a percentage of the measured for about 900 watersheds, was 119 percent and with seven parameters it was 116 percent. After zoning the same errors as means became 77.3 percent for the three parameters and 71.2 percent for the seven parameters.

When the  $q_{10}$  for each zone's watersheds were estimated with the 3-parameter all zone equation uncorrected for zone and then corrected for zoning, the mean standard errors were 91.5 percent and 87.1 percent respectively.

A 51 watershed sample (one from each State and Puerto Rico), correcting the estimates for storage, and comparing the SCS method on the same watersheds gave the following percent standard errors of estimate; SCS, 160; all zone

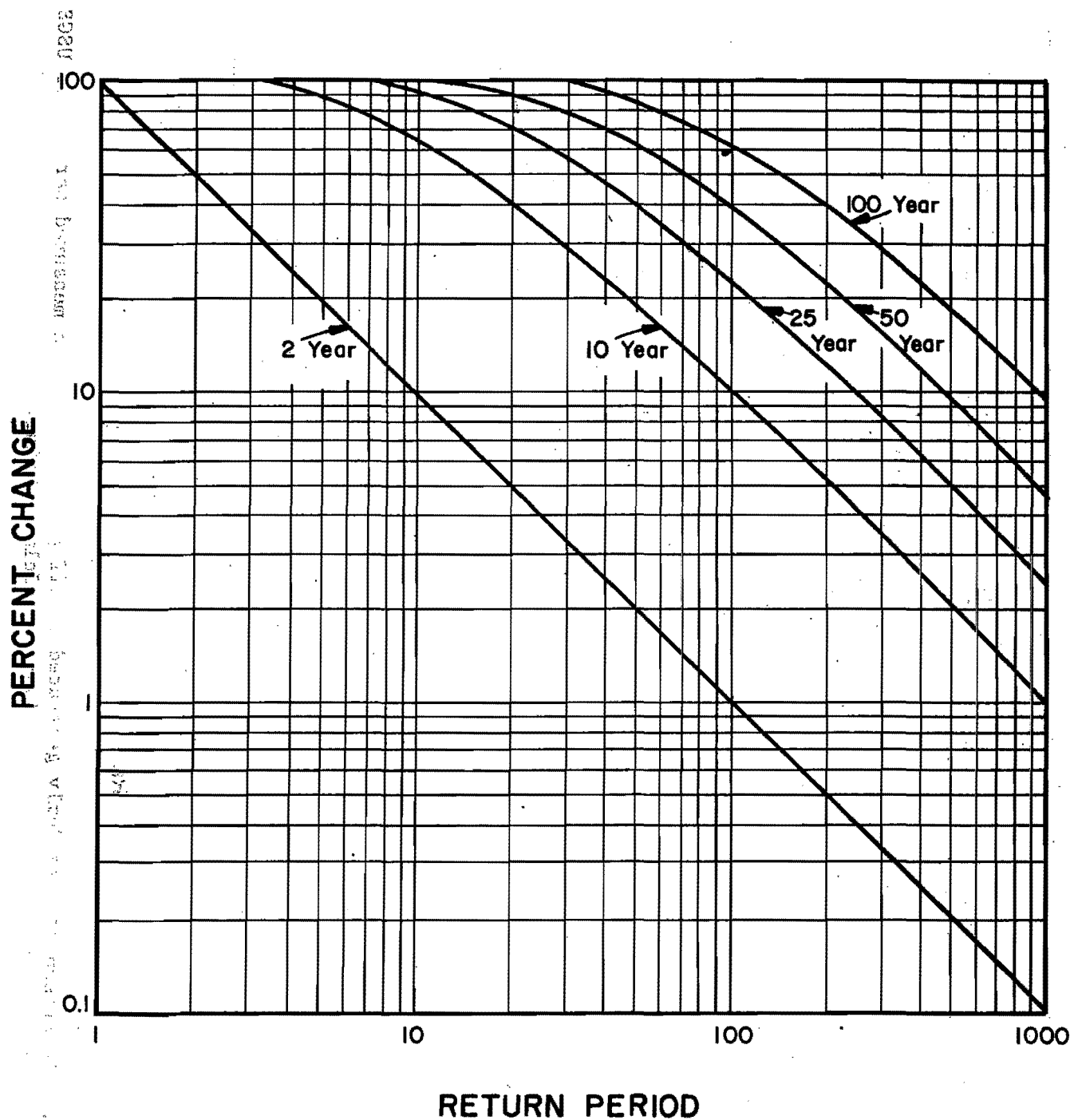


Figure 41. The chance of equaling or exceeding a flood peak for a given return period in periods of 2, 10, 25, 50, and 100 years.

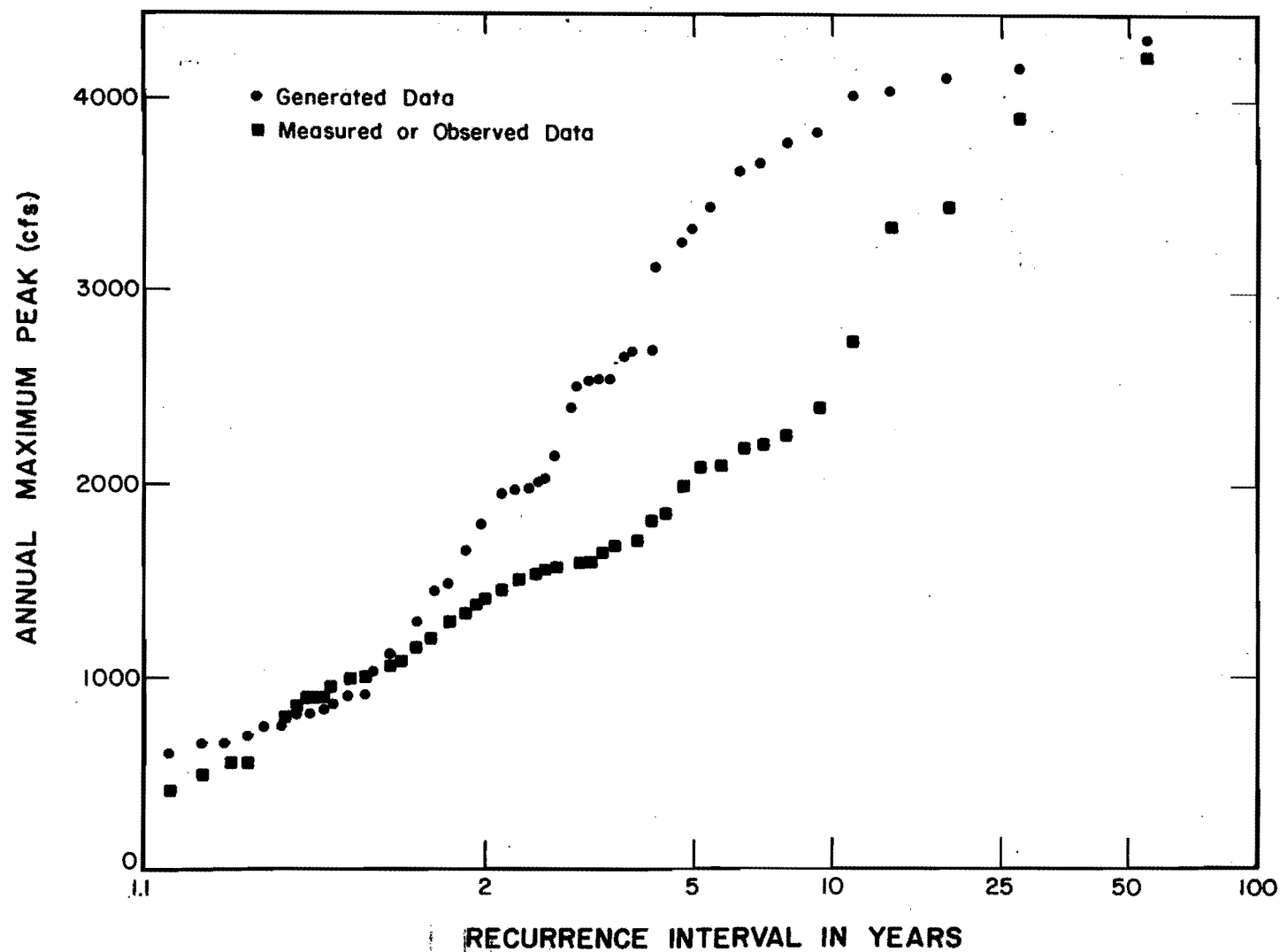


Figure 42. Comparison of a randomly generated runoff peak series and a measured series for USGS watershed 06658500, Laramie River near Jelm, Wyoming.

3-parameter, 39; all zone 7-parameter, 143; 3-parameter zoned, 50 and 7-parameter zoned, 38 as percentages of the measured values. None of these watersheds were used in development of the equations.

not just

Flood risk is related to the flood stage as well as frequencies so must be considered except where using maximum likely flood peaks for the entire USA.

No single distribution will fit all flood peak frequency data but some are more likely to fit than others. The writers recommend that measured points be plotted along with the fitted distributions for extrapolation.

A greatly improved method for forecasting runoff peaks at known recurrence intervals is simple and more reliable than previously existing methods.

present

#### REFERENCES CITED

1. Bar-Kochba, Yoseph, and Andrew L. Simon. 1972. Factors affecting floods from watersheds in humid regions of Northeastern Ohio. Water Res. Bull. 8(6):1235-1245.
2. Benson, Manuel A. 1962. Characteristics of frequency curves based on a theoretical 1000-year record. USGS, Washington, D.C. 21 p. August.
3. Bock, P., I. Enger, G. P. Malhotra, and D. A. Chisholm. 1972. Estimating runoff rates from ungaged small rural watersheds. NCHRP Program Rept. 136. 85 p.
4. Chow, Ven Te. 1964. Handbook of applied hydrology. McGraw-Hill, New York.
5. Dragoon, Frank J. 1962. Rainfall energy as related to sediment yield. Am. Geophys. Union, J. Geophys. Res. 67(4):1495-1501. April.
6. Ezekiel, Mordecai, and Karl A. Fox. 1970. Methods of correlation and regression analysis. John Wiley and Sons, Inc., New York City, New York. 548 p.
7. Fenneman, N. M., and Douglas W. Johnson. 1964. Physical divisions of the United States. USGS 1:7,000,000.
8. Fletcher, Joel E., and George W. Reynolds. 1972. Snowmelt peak flows and antecedent precipitation from watersheds in transition. American Water Resources Assoc. and Colorado State University, 197-199.
9. Grigg, Neil S., and Otto J. Helweg. 1975. State of the Art of estimating flood damage in urban areas. Water Res. Bull. 11(2):379-390. April.
10. Hydrocomp, Inc. 1970. Procedures for estimating flood flow from small rural watersheds. Final report. Bur. Public Roads Contract FH-11-7320. 90 p. September.

10. Kent, K. M. 1968. A method for estimating volume and role of runoff in small watersheds. SCS-TP-149. 40 p. January.
11. Miller, John F., R. H. Frederick, and R. J. Tracy. 1973. Precipitation-frequency atlas of the western United States. Vols. I-XI, NOAA ATLAS 2.
12. Potter, William D. 1958. Upper and lower frequency curves for peak rates of runoff. Trans. Am. Geophys. Union, 39(1):100-105.
13. Potter, William D. 1961. Peak rates of runoff from small watersheds. USDC BPR Hydraulic Design Series No. 2. 35 p.
14. Schmidt, Thomas A. 1975. A comparison of frequency distributions used to fit annual flood series. M.S. Thesis, Civil and Environmental Engineering, Utah State University. 160 p.
15. Schroeder, E. E. 1974. Estimating the magnitude of peak discharges for selected flood frequencies on small streams in East Texas. USGS Interim Rept. 85-8. 16 p. February.
16. Steel, Robert G. D., and James H. Torrie. 1960. Principles and procedures of statistics. McGraw-Hill Book Company, Inc., New York. 481 p. See p. 43.
17. USDA Agricultural Research Service. 1958. Annual maximum flows from small agricultural watersheds in the United States.
18. USDA Agricultural Research Service. 1959-1967. Hydrologic data for experimental agricultural watersheds in the United States. USDA Misc. Pubs. 945, 994, 1070, 1164, 1194, 1216, 1226, and 1262.
19. U.S. Geological Survey. 1971. Index of surface water records to September 1970, parts 1-16. Geological Survey Circulars, 651-666.
20. Utah Water Research Laboratory, Utah State University. 1976. Vol. 2, Manual of erosion control principles and practices during highway construction. Final Technical Report Project 16-3 to Transportation Research Board. February. 103 p.
21. Wischmeier, W. H., and D. D. Smith. 1958. Predicting rainfall-erosion losses from cropland east of the Rocky Mountains. Agric. Handbook 282. U.S. Government Printing Office, Washington, D.C.
22. Yen, Ben Chie. 1973. Methodologies for flow prediction in urban storm drainage systems. Dept. C. E., U. of Illinois, Water Res. Center Res. Rept. No. 72. 150 p. September.



#### CROSS REFERENCES TO APPENDICES

FHWA decided to print Appendix A separately from the other appendices to facilitate a plan to selectively distribute Appendix A in its entirety or in parts applicable to a given State.

## APPENDIX A

Appendix A consists of approximately 220 pages of flood frequency curves and data that were used in the study. This appendix is printed in Vol. III which is report No. FHWA-RD-77-160. Appendix A as a whole will be of interest to researchers who wish to develop new equations and to scientists who wish to validate the equations that have been developed. Otherwise only portions of Appendix A will be of interest to a given State.

In the first printing we will keep Appendix A intact but will make very limited distribution. In subsequent printings, we plan to separate Appendix A by States if it is needed to enhance design procedures promoted by FHWA.

## APPENDIX B

Appendix B consists of a detailed Hydrophysiographic zone map for each State. Appendix B is included in Vol II which is Report No. FHWA-RD-77-159.

## APPENDIX C

Appendix C consists of a detailed Iso-erodent map for each State. Appendix C is included in Volume II.

## APPENDIX D

Appendix D consists of a detailed 10-year, 60-minute precipitation, P60, map for each State. These maps are necessary to use the 5-parameter prediction equations. Appendix D is included in Vol II.

## APPENDIX E

Appendix E consists of a detailed 10-year, 10-minute precipitation, P10, map for each State. These maps are necessary to use the 7-parameter prediction equations. Appendix E is included in Vol II.

## APPENDIX F

Appendix F contains the 10-year, April 1 snow water equivalent maps for each of the Western United States including Alaska. Appendix F is also included in Vol II.

## APPENDIX G

Appendix G contains scatter diagrams for the relationship between the measured and estimated 10-year peak flows with 95 percent confidence intervals for the mean as single samples for each of the 24 hydrophysiographic zones using 3-parameter lumped and zone equations. Appendix G is included in Vol III which is report No. FHWA-RD-77-160.

## APPENDIX H

Appendix H contains the equation and nomographs. The 3-parameter, 5-parameter, and 7-parameter equations are listed for the "all zone" situation in which all the data were lumped together and for each of the 24 zones separately. Nomographs are included for all the 3-parameter equations. Appendix H also contains scatter diagrams and correction curves for the 3-parameter all zone equation with 95% confidence limits for a mean and a point estimate. Appendix H is included in Vol II.



U. S. DEPARTMENT OF TRANSPORTATION

## FEDERAL HIGHWAY ADMINISTRATION

SUBJECT Transmittal of Research Report Nos.  
FHWA-RD-77-158 and FHWA-RD-77-159, "Runoff  
Estimates for Small Rural Watersheds and  
Development of a Sound Design Method,"  
Volumes I & II

FHWA BULLETIN  
September 6, 1978

This bulletin covers the distribution of the subject reports which describe a new method for predicting runoff from small rural watersheds. These reports will be of interest to engineers concerned with highway drainage.

Volume I is the research report which describes the analyses made to develop the method. Volume II describes how to use the method for design but it leaves the designer with several options that appear to have approximately the same reliability. For example, options include using 7-parameter, 5-parameter, or 3-parameter equations, all of which have approximately the same standard error of estimate. To simplify use of this research report, the 3-parameter equations will be extracted and presented in a summary guide in the future.

The Water Resources Council (WRC) is planning to check the validity of this method along with other selected methods for estimating runoff.

Sufficient copies of the reports are being distributed to provide a minimum of one copy to each Regional office, one copy to each Division office, and one copy to each State highway agency. Direct distribution is being made to the Division offices. Additional copies for official use may be requested from Mr. David Solomon, Chief, Environmental Design and Control Division, FHWA, HRS-42, Washington, D.C. 20590. See the attached Report Request Form. These requests will be filled while the supply lasts. Additional copies for the public are available from the National Technical Information Service, U.S. Department of Commerce, 5285 Port Royal Road, Springfield, Virginia 22161. A small charge will be imposed for each copy ordered from NTIS.

*Charles H. Schaffner*  
F. D. Love  
Associate Administrator for  
Research and Development

### Attachments

DISTRIBUTION: Headquarters  
Special: Regions  
Divisions

OPI: HRS-42

Reports FHWA-RD-77-158 & FHWA-RD-77-159  
"Runoff Estimates for Small Rural Watersheds and  
Development of a Sound Design Method"

Frequency analyses of more than 1,000 small watersheds in the United States and Puerto Rico were used to develop the estimation method for design of peak flow for ungaged watersheds. This method, called the Federal Highway Administration (FHWA) method, is conceptually similar to the Bureau of Public Roads (BPR) method developed by W. D. Potter. The FHWA method relates the runoff peak to easily determined hydro-physiographic parameters and is intended for use on watersheds smaller than 50 square miles. The concept of risk is incorporated into the recommended design procedure. The risk is the probability that one or more events will exceed a specified peak flow within the usable lifetime of the drainage structure. The return period of the design flood peak can then be modified according to the risk the designer is willing to take. Another concept dealing with the probable maximum runoff peak derived as a function of watershed area is included. The flow obtained from this relationship is considered to be the upper limit of the design flow that may realistically be expected to ever occur. As such it may be appropriate to use in situations where the consequences of failure are extremely great.

To obtain additional copies of these reports, please send in the request form below.

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REPORT REQUEST FORM

"Runoff Estimates for Small Rural Watersheds and  
Development of a Sound Design Method"

Please send \_\_\_\_\_ copies of FHWA-RD-77-158, Volume I "Research Report" and/or \_\_\_\_\_ copies of FHWA-RD-77-159, Volume II "Recommendations for Preparing Design Manuals; and Appendices B, C, D, E, F, G, & H"

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## FEDERALLY COORDINATED PROGRAM OF HIGHWAY RESEARCH AND DEVELOPMENT (FCP)

The Offices of Research and Development of the Federal Highway Administration are responsible for a broad program of research with resources including its own staff, contract programs, and a Federal-Aid program which is conducted by or through the State highway departments and which also finances the National Cooperative Highway Research Program managed by the Transportation Research Board. The Federally Coordinated Program of Highway Research and Development (FCP) is a carefully selected group of projects aimed at urgent, national problems, which concentrates these resources on these problems to obtain timely solutions. Virtually all of the available funds and staff resources are a part of the FCP, together with as much of the Federal-aid research funds of the States and the NCHRP resources as the States agree to devote to these projects.\*

### *FCP Category Descriptions*

#### **1. Improved Highway Design and Operation for Safety**

Safety R&D addresses problems connected with the responsibilities of the Federal Highway Administration under the Highway Safety Act and includes investigation of appropriate design standards, roadside hardware, signing, and physical and scientific data for the formulation of improved safety regulations.

#### **2. Reduction of Traffic Congestion and Improved Operational Efficiency**

Traffic R&D is concerned with increasing the operational efficiency of existing highways by advancing technology, by improving designs for existing as well as new facilities, and by keeping the demand-capacity relationship in better balance through traffic management techniques such as bus and carpool preferential treatment, motorist information, and rerouting of traffic.

#### **3. Environmental Considerations in Highway Design, Location, Construction, and Operation**

Environmental R&D is directed toward identifying and evaluating highway elements which affect the quality of the human environment. The ultimate goals are reduction of adverse highway and traffic impacts, and protection and enhancement of the environment.

#### **4. Improved Materials Utilization and Durability**

Materials R&D is concerned with expanding the knowledge of materials properties and technology to fully utilize available naturally occurring materials, to develop extender or substitute materials for materials in short supply, and to devise procedures for converting industrial and other wastes into useful highway products. These activities are all directed toward the common goals of lowering the cost of highway construction and extending the period of maintenance-free operation.

#### **5. Improved Design to Reduce Costs, Extend Life Expectancy, and Insure Structural Safety**

Structural R&D is concerned with furthering the latest technological advances in structural designs, fabrication processes, and construction techniques, to provide safe, efficient highways at reasonable cost.

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Maintenance R&D objectives include the development and application of new technology to improve management, to augment the utilization of resources, and to increase operational efficiency and safety in the maintenance of highway facilities.

\* The complete 7-volume official statement of the FCP is available from the National Technical Information Service (NTIS), Springfield, Virginia 22161 (Order No. PB 242087, price \$45 postpaid). Single copies of the introductory volume are obtainable without charge from Program Analysis (HRD-2), Offices of Research and Development, Federal Highway Administration, Washington, D.C. 20590.

