

1 **Fibre Waviness in Pultruded Bridge Deck Profiles :**
2 **Geometric Characterisation and Consequences on Ultimate Behaviour**

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9 **ABSTRACT**

10 Conventional tests cannot be used to establish the important influence of fibre waviness, a
11 manufacturing legacy at the flange-web joints (FWJs) of pultruded GFRP bridge decks, on the local
12 ultimate behaviour of such decks. Hence a novel, simple and reliable three-step experimental
13 scheme for that purpose is presented herein, using one pultruded deck profile as an exemplar. First,
14 for the given profile, the different individual and bonded deck-deck joint geometries which must
15 be targeted for testing are identified. Second, an effective manual method is put forward to map
16 this waviness at the FWJs. Third, a quasi-static test setup is introduced which enables statically
17 determinate loading of one joint at a time, while also ensuring continuity between this joint and the
18 remaining deck so that the real load paths within the deck are preserved. During the tests failure
19 always occurred by fracture of the wavy fibre-resin interfaces within the FWJs, with a distinct
20 inverse correlation between fibre waviness and failure load, and with the influence of bonding on
21 joint failure behaviour depending on the local flange-web layout. It is concluded that this simple
22 test is sufficiently reliable for extension to assessing local fatigue behaviour at the joints.

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25 **KEYWORDS :** GFRP ; Bridge decks ; Mechanical testing; Pultrusion ; Fibre Waviness

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29 1. INTRODUCTION

30 Pultruded, cellular GFRP units bonded together are increasingly used as road bridge decks. The
31 modularity, superior specific stiffness and strength, low-weight and corrosion-resistance of the
32 units translate into easy assembly, rapid installation, low foundation costs and high durability of
33 the bridges [1]. As traffic loads become more onerous and as the pultruded sections evolve in
34 shape, in material composition and in local detail, the understanding of the ultimate behaviour of
35 the decks must keep pace.

36 A crucial issue is that such decks are typically designed to idealised material property and structural
37 geometry assumptions, but not from an as-manufactured quality perspective. Indeed, Coogler et
38 al. [2] state that the stress limits specified in codes for GFRPs are independent of manufacturing-
39 induced imperfections in the materials, which may reduce these limits. Consequently, the strong
40 link between manufacture and actual deck performance in service remains concealed. This, in turn,
41 means that when the manufactured decks are loaded, process-induced imperfections such as resin-
42 rich zones and out-of-plane fibre waviness (or wrinkles) induce stress concentrations that influence
43 failure in ways not accounted for at the design stage. Insight into the associated high local stress
44 effects is important, because as pointed out by Ellingwood [1], FRPs (unlike steel) do not yield and
45 so can be of limited redistribution capability. The complexity of the problem is compounded by
46 the random – and so uncertain – nature of the imperfections. In addressing this complexity, it is
47 important to develop strategies for characterising the imperfections and for understanding their role
48 in ultimate behaviour.

49 Coogler et al. [2] highlighted the spectrum of process-induced imperfections that can arise in the
50 cross sectional planes of pultruded GFRP decks. They observed that fibre waviness can be
51 particularly pronounced within the flange-web joints, or FWJs, to the extent that the fibres often
52 return upon themselves, essentially describing U-trajectories. Now an impressive feature of these
53 joints is that they successfully transmit large, multi-directional loads between the adjoining webs
54 and flanges through small cross-sectional areas. As a result the FWJs develop high, multi-axial
55 stresses that are exacerbated by the fibre waviness. Moreover, curing conditions unique to the
56 joints induce local residual stresses and microcracks that cause the fibre-resin interfaces in the as-
57 manufactured decks to be of different bond strengths within the joints relative to within the web
58 and flange straights between joints. Unsurprisingly, therefore, fracture of the wavy interfaces
59 within the joints is a key manifestation of failure in GFRP decks loaded to ultimate.

60 Notably, a literature survey reveals that these wavy interface fractures occur at FWJs irrespective
61 of structural layout (the deck acting alone or as part of a hybrid), of spatial influence (local or
62 global) of the load, and of directionality (key actions along or transverse to the direction of
63 pultrusion). Indeed, such fractures have dominated ultimate behaviour in GFRP deck-steel beam
64 hybrids (Keller and Gurtler [3]) and GFRP deck-concrete beam hybrids (Sebastian et al. [4]) under
65 global flexure normal to the direction of pultrusion. Wavy interface fractures have also governed
66 failures in isolated decks under local load effects (Gabler and Knippers [5], Sebastian et al. [6, 7]),
67 in an isolated deck system under global flexure along the direction of pultrusion (Zi et al. [8]), and
68 also in deck systems under global flexure normal to the direction of pultrusion (Yanes-Armas et al.
69 [9]). This last case [9] induced both Vierendeel and truss actions in the decks, thereby causing
70 progressive fracture within and associated load redistribution between multiple joints of each test
71 specimen in the approach to failure. Subsequently, Yanes-Armas et al. [10] conducted web
72 cantilever tests on FWJs cut out from one of the decks.

73 Pultruded I-section GFRP profiles also exhibit FWJ failures due to wavy fibre-resin interface
74 fractures. In those cases too, previous studies have highlighted the randomness of the wavy profiles
75 and the consequences of the resulting uncertainties in fracture patterns. For example, in Fig. 14 of
76 [11], Turvey and Zhang provide evidence of strong asymmetry of the fibre architecture and of the
77 associated wavy interface fracture patterns between the FWJs at both ends of the web in a pultruded
78 I-section. From three-point bend tests on portions of the web rotationally restrained at the ends by
79 clamping the FWJs, they surmised that this asymmetry led to inconsistent moment-rotation
80 characteristics between the FWJs of different specimens. Feo et al. [12], Fascetti et al. [13] and
81 Quadrino et al. [14] also observed wavy fractures at the FWJs during vertical pull-out tests on
82 pultruded I-sections. For the publications describing these above cited studies on pultruded decks
83 and I-sections, Table 1 identifies the Figures which show the wavy interface fractures within the
84 FWJs.

85 Other experimental studies have investigated the effects of fibre waviness on the structural integrity
86 of FRPs at a fundamental level. In these studies the test specimens have been manufactured in the
87 laboratory, to enable good quality control on fabrication of simple, but physically meaningful fibre
88 wave geometries. Given this laboratory manufacturing, pultruded FRP specimens have been
89 precluded. The method of generating the waviness has varied between studies. For example
90 Adams and Hyer [15] used copper wire and aluminium foil to artificially generate the wave forms,
91 while Bloom et al. [16] used a manufacturing method which sought to replicate the “organic”

92 mechanism of formation and hence the natural morphology of the wrinkles found in wind turbine
93 blades. The wavy fibre laminates have been subjected to either tensile or compressive axial loads.

94 Note that in the study by Adams and Hyer [15], prepreg tape was employed to fabricate laminates
95 with isolated fibre layer waves. Using optical microscopy, they characterised the wave geometry
96 via the amplitude, δ , the wavelength, λ , and the maximum angle of fibre rotation, θ_{max} , also termed
97 the angle of misalignment. Both θ_{max} and the ratio δ/λ were alternately used to define the severity
98 of the waves. On subjecting these laminates to axial compression, a 36% reduction in static strength
99 was obtained for laminates of severe waviness ($\delta/\lambda \approx 0.06$), although the wavy layers accounted
100 for only 20% of capacity. The observed failure mode was often brooming, namely through-
101 thickness splaying of layers accompanied by several delaminations near the waves.

102 Bloom et al. [16] used aerospace grade glass fibre-epoxy prepregs in a three-stage manufacturing
103 process (layup onto an aluminium tool plate, vacuum bagging, autoclave curing) to produce their
104 laminates. Three sets of specimens were manufactured, namely unwrinkled laminates which served
105 as control specimens, also laminates with 50% wrinkled plies and laminates with 100% wrinkled
106 plies. They found that both visual observation and optical microscopy were useful for
107 characterising the wrinkles in terms of angle of misalignment and height. Where the tows were
108 looped or kinked, two additional angles were measured. On testing the 100% wrinkled ply
109 specimens in axial tension they observed a 38% knockdown of strength, relative to the control
110 laminates, for a 30.5° misalignment angle. Failure was by cracking along the path of misalignment,
111 which fractured the wrinkled plies.

112 Progression from laboratory-manufactured to commercially produced specimens was made by
113 Sutcliffe et al. [17], who characterised fibre waviness for industrial components already
114 manufactured by resin transfer moulding (RTM) and by prepreg/vacuum consolidation. They
115 showed that image analysis with an autocorrelation function could be applied equally to polished
116 sections and to micro-CT X-ray images. This led to the observation that the waviness zones were
117 distinctly longer in the prepreg samples than in the RTM samples.

118 It is now appropriate to build on the successes of these earlier studies by extending the scope to
119 include pultruded GFRP bridge decks. This entails initial investigations into mathematically
120 defining the fibre waviness at the FWJs of such decks, along with acquisition of experimental
121 insight into the way in which this waviness can influence the failure behaviour of the joints when
122 the decks are loaded. The remainder of this paper is dedicated to one such study.

123 In so doing it must be recognised that the above-described coupon fabrication approach and testing
124 methods used to date cannot be applied to pultruded GFRP bridge decks, for the following reasons:

- 125 • The coupons have been flat, whereas the FWJs of pultruded decks are of complex geometries
126 defined by combinations of flats, curves and steps.
- 127 • Simplified, specific waviness profiles have been built into these flat coupons. By contrast, the
128 waviness profiles in pultruded FWJs are complex and random, due for example to the
129 randomness of the vibrations within the fibre-pulling pultrusion machinery.
- 130 • Coupon fabrication has entailed room temperature curing of the resin, which very likely induces
131 only minor residual stresses in the cured coupon. However, pultrusion entails cooling of the
132 resin from over 100°C down to room temperature. Spatial temperature differentials develop
133 through the resin volumes in the FWJs during this large temperature drop, inducing palpable
134 residual stresses and microcracks within the FWJs. These initial stresses and cracks depend on
135 the FWJ geometries and can significantly affect the load-carrying capabilities of these FWJs.
- 136 • In previous tests simple axial tension or compression has been applied to the specimens with
137 embedded waviness. However, as shown in Fig. 1, concentrated tyre loading on pultruded
138 bridge decks induces significant local biaxial flexing of the top flange and introduces high local
139 flexure and shear forces at the nearby flange-joint boundaries. Hence the loading used to test
140 FWJs in the bridge decks must differ significantly from those previously employed.
- 141 • Only one joint type (if any) has required consideration previously. For pultruded decks,
142 however, both the original joints of a unit and the bonded joints must be considered, in the latter
143 case with the added complexity of testing from both sides if the bonded joint is asymmetric.

144 For these reasons a novel testing approach has been developed in the present study. The key
145 ideas which underpinned this study are stated in the next section.

146

147 **2. AIMS AND OBJECTIVES OF THE PRESENT STUDY**

148 The overall aim of the present study is to experimentally characterise fibre waviness at the joints
149 of pultruded GFRP decking and to gain insight, via suitably designed tests, into the failure-inducing
150 effects of this waviness when the joints are loaded. The ASSET deck system, a unit of which is
151 shown in Fig. 2(a), has been chosen for the study. In order to maximise insight into the joint
152 mechanics, each test was designed to satisfy the following criteria, namely :

- 153 • Loading to failure of only one joint.
- 154 • Development of generalised force patterns – local moments and shear forces – on the joint which
155 are reflective of those induced by concentrated tyre loads applied to the flange of the deck.
- 156 • Full determinacy of these generalised forces acting at the entry to the joint from the applied load.
- 157 • Preservation of the natural continuity between the FWJ and its adjoining webs and flanges, to ensure
158 that the natural stresses and crack propagations are induced within the joint under load on the deck.

159 In addition, it was important to test all joint configurations possible within an individual deck unit
160 as well as between bonded deck units. To those ends, the objectives of the study were to :

- 161 • Illustrate an effective manual approach to documenting the fibre waviness at the deck joints.
- 162 • Show how select cuts in the deck enable determinate loading of single joints with web-flange continuity.
- 163 • Highlight the extent to which bonding the deck joints can influence joint failure loads and modes.
- 164 • Quantify any progressive local losses of stiffness due to crack propagation along the wavy interfaces.

165 In what follows the test specimens and procedures are described, then results are presented and
166 discussed, after which conclusions are drawn and suggestions are made for further work.

167

168 **3. TEST SETUP**

169 *3.1 Identification of Joint Types for Testing*

170 Fig. 2(b) identifies the four flange-web joint (FWJ) types, henceforth termed JA, JB, JC and JD,
171 that exist when two ASSET units are bonded together. Bonding of further ASSET units produces
172 more of these (and no other) joint types. JA and JD are both fundamental joints within the
173 individual ASSET unit. JA joins three members, namely the flange, diagonal web and an external
174 web of the ASSET unit, with 60° angular separations between these members and with a groove
175 at the end of the flange. JD joins two members, namely the flange and the unit's other edge web,
176 again with 60° angular separation between them and with a lip protruding from the flange.

177 JB and JC both refer to the hybrid joint formed by bonding together adjacent deck units, whereby
178 the lip of the “JD” joint from one deck unit fits into and is bonded into the groove of the “JA” joint
179 from the adjacent unit. Since this hybrid joint is asymmetric, it is important to separately consider
180 the effects on the joint of local loading on the flange both from the “JA-side” and the “JD-side”.
181 It is these considerations which have led to the double identification of this hybrid joint as either

182 JB, when it is loaded from the flange on the “JA-side”, or alternately as JC, when it is loaded from
183 the flange on the “JD-side”. There is a loss of alphabetical order in naming the joints from left to
184 right in Fig. 2(b), because the decision was taken that JA and JB should be paired, ditto JC and JD.
185 This will facilitate comparisons between joints within each pair later in this paper.

186

187 3.2 *Layout of Test Specimen and Loading Strategy*

188 The specimen shown in both plan and elevation in Fig. 3 was fabricated to enable testing of each
189 joint type defined above. As can be seen, the specimen comprised six ASSET units, each 200 mm
190 wide, bonded to each other both along their FWJs (the lip of one deck unit fitted and bonded into
191 the complementary groove of the adjoining unit) and along their inclined webs. In addition, the
192 bottom flanges of all six deck units were bonded underneath to a 25 mm thick steel plate, which
193 served as a translationally and rotationally rigid base.

194 JA and JD naturally occurred at the top left and right ends of the specimen. Hence, the local flange
195 span connecting into each of these joints was cut across the 200 mm width, near the next joint
196 inwards. This converted each such flange span into a cantilever, which was loaded near its tip
197 during the test. Given that the cantilever is isostatic, the resulting local shear forces and moments
198 induced at the cantilever’s entry into the joint were fully determinate. Note also, in Fig. 3, the cuts
199 within the flanges on both sides of the middle top layer joint. Clearly, the roots of the resulting
200 cantilevers connect into JB and JC to the left and right respectively of the middle top joint.

201 An important feature of this specimen layout is that, other than the essential flange cuts, the real
202 load paths between adjacent deck units, through the bonded FWJs and webs, were strictly
203 preserved. This is consistent with one of the requirements set out for the test setup as stated earlier.
204 Also, as an aside, Fig. 3 shows that this distribution of flange cuts led to a sequencing of joints
205 from left to right, which follows the alphabetical order JA, JB, JC and JD.

206

207 3.3 *Characterisation of Joint Fibre Waviness*

208 Fibre waviness was documented for each joint type by placing against the cut surface of the joint
209 a transparent plastic strip on which was printed a grid of 10 mm squares. As Fig. 4(a) shows, this
210 was done by making the top horizontal line of the grid flush with the top of the deck. A sharp-

211 pointed red marker was used to highlight on the plastic strip a series of points along each wavy
212 fibre layer of interest, the spacing of these points decreasing with any increased local gradient of
213 the wavy layer under consideration. In Fig. 4(a) the first few such red points can be seen at the
214 early stage of defining a crucial wavy fibre layer in the joint-flange transition zone, with the grid
215 lines also palpable. The abscissae and ordinates of the points were then determined by first placing
216 the 10 mm square grid with highlighted points against another, denser grid of 1 mm squares printed
217 on paper which was used to determine the coordinates of the points to within ± 0.25 mm. These
218 coordinates were entered into Excel or Matlab, which was then used to provide spline fits through
219 the points, resulting in plots of the wavy layers.

220 This approach was first trialled for the JA joints within the deck unit shown in Fig. 4(a). As can
221 be seen in Fig. 4(b) this joint occurs as JA1 and JA2, at both ends of the diagonal web for this
222 fundamental unit. It will be shown later that these flange-diagonal web joints include a top,
223 intermediate and base fibre mat layer. This trial focused on the intermediate layer, since the tests
224 (reported later) showed that layer to be quite involved in the local failures.

225 Fig. 4(c) compares this intermediate layer's wavy profiles for JA1 and JA2. The top left image of
226 Fig. 4(c) shows the original marker points dotted on the transparent plastic for the JA1 plot. In
227 developing the plots from these points the x-origin was taken as the flange-joint transition point,
228 namely the location at which the flange suddenly narrowed into the joint to define the starts of the
229 JA grooves which are clearly visible in Figs 2(a), 2(b) and 4(b). The y-origin was taken as the
230 soffit of the flange. As Fig. 4(c) shows the JA1, JA2 wavy profiles are broadly consistent, but
231 differences exist especially in the peak slope and on the flange side of the flange-joint transition.
232 Note that the present paper focuses on the general features of these wavy profiles and their effects
233 on local load response. In future, sinusoidal curve fitting may be attempted through these points.

234 By repeating this approach at one joint of each type, the wavy profiles within JA and JD were
235 recorded more than once. This provided an opportunity to further check the consistency of the
236 waviness profiles recorded for these joints. The results presented later will address this issue.

237

238 *3.4 Testing - Loading Strategy and Instrumentation*

239 Fig. 3(a) shows the load setup used for each test. Each cantilever was vertically loaded at a 95
240 mm lever arm from its connection into the joint. This use of a constant lever arm in all tests

241 enabled useful comparisons to be made between the load responses of the different joints. A square
242 section aluminium bar was used to apply the load uniformly across the 200 mm width of the flange,
243 while a load cell placed between the bar and the loading actuator was used to measure the applied
244 force. The load was applied by an hydraulic jack fed with oil from a reservoir by a hand pump.
245 Potentiometers supported on stands with magnetic bases were used to measure the vertical
246 deflections under the ends of the loading bar. By this means, the deflection at the loading point of
247 the cantilever was measured. Fig. 3(b) shows a close-up of this test setup for joint JC.

248 Strains were recorded from 5 mm long electrical resistance gauges placed on the flange and web
249 members very near the joints, to the layout shown in Fig. 3(a). All gauges were oriented to measure
250 strains along the local spans of the members. For the tests at JA and JD, this included gauges at
251 the roots of the cantilevers on both surfaces, to enable quantification of the effects of the maximum
252 moments developed along the cantilevers. All gauge, potentiometer and load cell readings were
253 recorded continuously during each test at 10 Hz by an electronic data acquisition system.

254

255 **4. WAVINESS PROFILES**

256 The first two parts of Fig. 5 show, for each of JA and JB, a photo of the joint in the deck's cross
257 section with the wavy fibre layers evident, along with the fibre waviness profiles recorded for the
258 joint. Note that the waviness profile is presented in the important context of the local joint geometry.
259 Note from each photo the presence of three fibre mat layers through the flange thickness, namely
260 one layer (labelled Top) near the top of the flange, another layer (labelled Int) at an intermediate
261 level through the flange thickness and a final layer (labelled Base) travelling around the corner very
262 near the base of the flange. Only the top and intermediate layers are here represented.

263 In the plots of Figs 5(a), (b), the marker points which form the basis of the waviness profiles are
264 clearly shown. For the top and intermediate fibre layers the zones of peak waviness are identified
265 as the fibre trajectories between the letters A and B, and C and D, respectively. In both cases, the
266 top layer (A-B) zone displays the more pronounced waviness. Within the hybrid joint JB (Fig. 5(b)),
267 this A-B zone on the "JA-side" of the joint generously exceeds the "JD-side" of the joint in waviness.
268 This pronounced waviness is a direct result of the top layer fibres, which are very near the upper
269 surface of the deck, needing to travel around the corner of tight radius created by the end of the
270 groove where the flange meets the joint.

271 Clearly, these peak waviness zones are not representable by single sinusoidal curves. Indeed the
272 Matlab spline-fits in these zones were found to be sixth order polynomials, which can be
273 conveniently expressed in normalised (non-dimensional) form. Finally, Fig. 5(c) shows that the
274 waviness profiles are broadly similar, although there are distinct differences in the peak waviness
275 zones especially in maximum tangent angle to the horizontal. This can have important implications
276 for the consistency of the deck's failure behaviour under a given load type and should be considered
277 in more detail in further work.

278 Fig. 6 presents the corresponding results for JC and JD. It is seen that while the waviness in JD or
279 on the JD side of the JC joint is palpable (Fig. 6(a), (b)) and shows a reasonable consistency between
280 the two joints (Fig. 6(c)), it is far less pronounced than that for JA. Indeed, the profiles in Fig. 6(a)
281 suggest that the wavelengths for the "JD-side" of the hybrid joint JC far exceed, and the amplitudes
282 distinctly less than, those for the critical top fibre layer on the "JA-side" of the joint. It is interesting
283 to note how these relative levels of waviness on the two sides of the bond in this hybrid joint translate
284 into wavy interface fracture profiles as the load shifts from the JA side to the JD side of the joint.
285 This issue will be addressed again in the ensuing sections while interpreting the load test results.

286

287 **5. DISCUSSION OF TEST RESULTS**

288 *5.1 Failure Mode and Load Comparisons Between Joints*

289 The test results show that failure never occurred in the adhesive bonds or along the webs common
290 to adjacent deck units. Instead, failure was always confined to the FWJs, where the physical
291 manifestation of such failure was fracture of the wavy fibre-resin interfaces. Fig. 7 shows these
292 fracture profiles for the FWJs of all four joint types. An interesting observation is that the failure
293 mode remained almost unchanged from JA to JB (Fig. 7(a)), but changed sharply from JC to JD
294 (Fig. 7(b)). JA and JB both failed on the "JA-side" of the joint, largely by interface fracture for
295 the intermediate fibre layer within the joint, along with some interface fracture for the intermediate
296 and top layers extending just beyond the joint a short distance into the flange. The wavy interface
297 fractures for JD were within the web side of the web-flange resin rich zone, and were broadly
298 parallel to the inclined web. Fractures in JC were still on the "JD-side" of the joint, but with these
299 fractures now near-horizontal for the top, intermediate and base layer fibres. The base layer
300 fractures, located quite near the lip-adhesive bonded interface, were particularly pronounced.

301 The insensitivity of the JA-JB failure mode to joint make-up was very likely due to the stiff
302 diagonal web support which, even on its own minimised local flexing of the joint and so, with the
303 bond in place, ensured only moderate stress transfer through to the bonded lip of the adjacent deck
304 unit. This might have resulted in similar stress states within and so similar failure modes for the
305 JA, JB joints at ultimate.

306 Fig. 7(b) shows that JD experienced significant rotation of the unstressed lip extension, with
307 considerable tensile stress transfer from the flange to the web clearly having occurred at the outer
308 edge of the resin rich flange-web transition zone. However once the lip and web were bonded to
309 the adjacent unit to give JC, the resulting restraint to lip rotation generated through-thickness
310 tensile stresses in the lip and resin-rich zones that led to the multiple near-horizontal fractures seen
311 for JC in Fig. 7(b). This explains the change in failure mode from JD to JC. Also, the JC failures
312 within the parent GFRP material strongly suggest that the bonded interfaces between the GFRP
313 and the externally applied adhesive enjoyed more favourable stress demand / strength ratios than
314 did the fibre-resin interfaces within the GFRP itself.

315 Table 2 completes the picture on joint strength. It is seen that JA and JB were of nominally
316 identical strengths, while the strength of JC was over four-fold that of JD. Importantly, the
317 strongest joint was JC, over 2.5 times the capacity of JA. This is initially surprising because the
318 lip extension in JC is quite thin and so the pronounced fractures within this lip (Fig. 7(b)) may
319 have been expected to occur at fairly low loads. In this respect the one advantageous feature which
320 puts JC ahead of JA and JB is the low waviness of the fibre mat layers alongside which the fractures
321 occurred. The much more pronounced waviness in JA, JB might have significantly increased the
322 wavy interface stress demand under load and by this means might have triggered the comparatively
323 lower failure loads. This suggests a dominant effect of fibre waviness on local joint failure
324 behaviour.

325 These failure crack patterns and loads show that, while the bonding adhesive layers allowed stress
326 transfers across the hybrid joints, the failure-inducing activity remained on the side of the adhesive
327 layer at which the external load was applied. The nature of this failure crack pattern was
328 unchanged in proceeding from JA to JB, but morphed from JC to JD. This role of the adhesive
329 layer as a buffer between the two sides of a hybrid joint should be explored in future work.

330 Figs 8 - 10 show the progression of fractures for each of joints JA, JB and JC. For JA (Fig. 8),
331 fracture initiated alongside the intermediate wavy fibre mat layer within the joint, followed by

332 further fractures alongside both the intermediate and top layers in the joint-flange transition zones.
333 For JB (Fig. 9), fracture initiated alongside the intermediate layer where the tangent to the wavy
334 profile was at its steepest in the joint-flange transition zone, followed by crack jumping through
335 the resin across to the top fibre mat layer where the fracture continued to propagate along the
336 steepest tangent to that layer. Subsequently, other fractures formed both above and below the top
337 and intermediate layers largely within the joint zone, with short fracture extensions into the flange.

338 Fig. 10 shows that the first fracture in JC occurred alongside the top fibre mat layer in the main
339 body of the flange-web transition zone and so just outside the lip extension, accompanied by crack
340 jumping through the resin down to the intermediate way layer. This was later followed by
341 extension of the fracture alongside the top wavy layer well into the lip extension zone, together
342 with more fracture development alongside the intermediate layer in the resin-rich zone and, most
343 importantly, the main fractures alongside the base fibre mat layer, just above the bonded interface
344 of the lip with the externally applied adhesive. The thin “skin” of parent GFRP material under this
345 main fracture remains fixed to the adhesive, suggesting good integrity of the GFRP-adhesive bond.

346

347 *5.2 Stiffness and Ductility Comparisons Between Joints*

348 Fig. 11(a) compares the load vs cantilever deflection (measured at the loading point)
349 characteristics for the tests including JA and JB, while Fig. 11(b) does the same for JC and JD.
350 It is immediately apparent that the move from the JA test to the JB test led to a significant increase
351 in stiffness, while that from the JD test to the JC test led to a huge increase in stiffness. This is
352 confirmed by Table 1, which shows a 3.14-fold and a 7.26-fold stiffness increase of the JB test
353 over the JA test and of the JC test over the JD test respectively.

354 Hence in proceeding from the JA test to the JB test, the strength increase has been almost zero
355 but the stiffness increase has been considerable. In fact strictly speaking, this stiffness increase
356 was due not only to the bonded FWJ, but also to the entire bonded length of web to the adjacent
357 unit which provided much additional restraint to the rotations and displacements of the
358 cantilevering flange. Certainly this bonding to the adjacent deck unit along the entire length of
359 the web would have accounted for a lot of the huge stiffness increase of the JC specimen above
360 the JD specimen, as the JD specimen had only the one web leg to help restrain rotation and
361 deflection at the root of the flange cantilever (while the JA specimen had two splayed web legs
362 before the additional restraint appeared from the adjacent deck unit in the JB specimen).

363 Fig. 11(a) shows an ability to hold a significant proportion of the load capacity after stiffness
364 drops due to fracture. This suggests ductile behaviour of JA and JB. Clearly, the post-peak load
365 holding as a proportion of the peak load is palpably better for JB than for JA, probably due to the
366 additional material available to help with stable stress redistribution. By contrast, JD and JC
367 show precipitous drops in load-carrying capability beyond peak load, suggesting little ductility.

368 The left plot of Fig. 12(a) shows, for JA, the variations with load of strains recorded from the
369 outermost locations of the flange and each web section at connection into the joint. The strain
370 gauge locations are given in Fig. 3(a). The plots suggest linear behaviour up to failure, when
371 strains of almost $2000 \mu\epsilon$ were recorded. In the right plot of Fig 12(a), these raw strains are then
372 separated out into an axial effect (the average within each pair of strains) and the flexural effect
373 (the difference within each pair). It is seen that the axial effect for the cantilever is almost zero,
374 as required from equilibrium, while that for the two webs remains small. By contrast, the flexural
375 effects are quite pronounced, confirming the bending-dominated behaviours of the members
376 framing into this joint. The two plots of Fig. 12(b) do the same for the cantilever framing into
377 JD. The nonlinear unloading curves may have been due to the pronounced and sudden cracking.

378 Now, recall the elementary beam theory expression $M = EI\kappa$, where M is the section moment, E
379 is the assumed homogenous material modulus, I is the second moment of area of the section
380 about its neutral axis and κ is the section curvature. Further, recall that if d and $\Delta\epsilon$ are
381 respectively the section depth and the difference in longitudinal strains between the outermost
382 locations of (namely the gauge locations on) the section, then $\Delta\epsilon / d$ is the section curvature.
383 Finally, the moment at the gauged section is obtained from statics using the applied load and the
384 lever arm from the load to the gauged section. Hence using these formulae along with the test
385 data, it was possible to deduce the flexural stiffness EI of the cantilever section at the gauged
386 locations.

387 Fig. 13 shows the resulting EI variations with load for the gauged flange cantilever sections
388 framing into JA and JD. This Figure shows that each section flexural stiffness was roughly
389 constant with load increase. The plots start at about 0.2 kN, to avoid the effects of errors in the
390 strain recordings at lower loads. The 2.5 kNm^2 starting value for JD exceeds the corresponding
391 2 kNm^2 starting value for JA by 25%. This is probably due to the lesser waviness of the fibres
392 and the associated further-out locations of the fibre layers on the section at JD than at JA. More
393 importantly, the EI value at JD exceeds double that calculated using the manufacturer's E value
394 (Table 3) for the flange along with an I value for the rectangular section of 200 mm width and

395 15.6 mm depth. This probably has to do with the fact that the I calculation assumes a
396 homogenous section, while in fact the concentrated fibre layers which strongly influence section
397 stiffness are at discrete levels which differ from those assumed when the equivalent homogenous
398 section properties were produced. More work is needed into this effect of fibre waviness.

399 More generally, fibre waviness also occurs along the length of the flange between FWJs. This
400 causes variation along the flange of the GFRP material's effective modulus. Use of strain gauges
401 at regular intervals along the flange cantilever between the loading point and the joint would enable
402 this modulus variation to be estimated.

403

404 **6. SUMMARY AND CONCLUSIONS**

405 Some key conclusions from this study are as follows :

- 406 • The use of a sharp-pointed marker to highlight dots on transparent plastic sheeting placed firmly
407 against the cut face of a pultruded GFRP bridge deck unit enables reliable representation of the
408 fibre waviness profiles in the flange-web joint zones of the deck. Then, using Matlab or Excel,
409 it is possible to define equations which closely fit the particularly wavy profiles, for possible
410 use in further analysis (beyond the scope of this specific study).
- 411 • It is crucial to identify both the fundamental joints within the deck units and the hybrid joints
412 formed by bonding the originals together, taking care to distinguish between the two sides of
413 asymmetric hybrid joints, since the tests can be used to establish the impact of bonding on joint
414 behaviour.
- 415 • A novel experimental strategy was devised in which only one joint at a time was loaded by
416 statically determined generalised forces from the root of the nearby loaded flange cantilever.
- 417 • Under load on the flange cantilever, failure always occurred by fracture of the wavy fibre-resin
418 interfaces within the FWJs, even for the bonded joints, where no failures were observed in the
419 adhesive or along the webs common to adjacent deck units.
- 420 • Fibre waviness also influences the effective section flexural stiffness and so the effective
421 material modulus along the flange. These effective values may be deduced from knowledge of

422 the statically determined moments and the section curvature as deduced from strain gauge
423 readings at regular intervals along the flange cantilever.

424 This study has focused on plotting fibre waviness and experimentally observing its effects on
425 the quasi-static behaviour of the joints. Future work should focus on using statistical approaches
426 (given the likely random nature of fibre waviness) tied to predictive modelling and experiments,
427 to establish the effects of this waviness on local tyre load fatigue of the decks.

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430 7. REFERENCES

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