



Design and Construction of Drilled Shafts Wall for Landslides

106th Annual Road School 2020

3/11/2020

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1. Landslides Overview
2. Drilled Shaft Wall Types
3. Construction Sequence
4. Construction Equipment
5. Drilled Shaft Wall
Analysis and Design
6. Drilled Pier Wall Case
History
7. Drilled Shaft Wall Design
Considerations

- Even if foundation conditions are satisfactory, slopes may be unstable at the desired slope angle.
- For new construction, the cost of fill, right-of-way, and other considerations may make a steeper slope desirable.
- Existing slopes, natural or manmade, may also be unstable, as is painfully obvious when they fail.



Example of Landslide



Example of Landslide (2)



Example of Landslide (3)



Terzaghi and Peck (1967, 426) stated: "On shales of any kind, the decrease of the slope angle to its final equilibrium value takes place primarily by intermittent sliding. The scars of the slides give the slopes the hummocky, warped appearance known as "landslide topography"

SR 66 Spencer County, Indiana

Example of Landslide (4)



Drilled Shaft Wall was Installed

SR 237 Perry County, Indiana



Example of Landslide (5)



**Drilled Shaft Wall will be Installed
CR 875 Landslide Jackson County, Indiana**



Example of Landslide (6)



Drilled Shaft Wall will be Installed

**CR 100 Landslide
Jackson County, Indiana**





Example of Landslide (7)



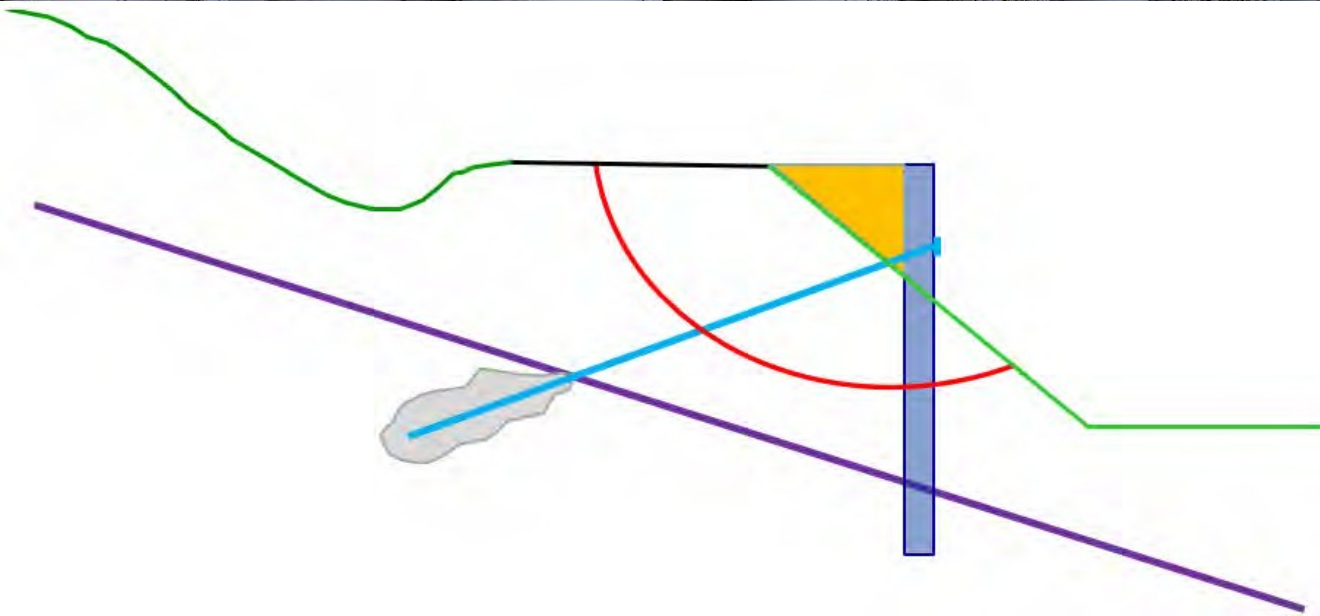
SR 62 Perry County, Indiana

Example of Landslide (8)



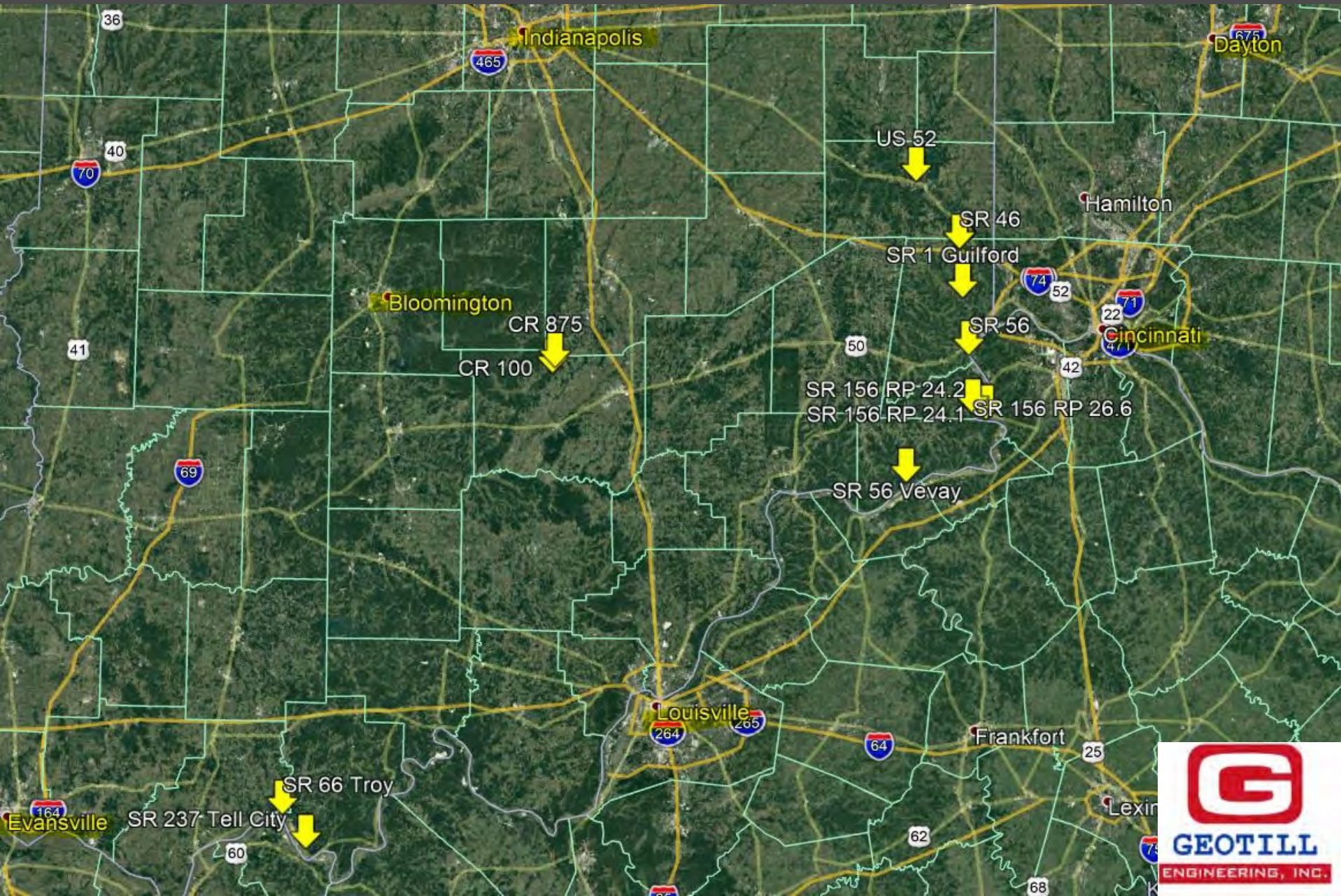
Massive landslides caught on camera

SR 56





Drilled Shaft Walls in Indiana





- Drilled Shaft with Lagging
- Drilled Shaft with Plugin Piles
- Drilled Shaft with Tiebacks
- Secant Drilled Shaft
- Tangent Drilled Shaft
- Drilled Shaft with Soldier Piles and Lagging
- Stub Drilled Shafts



Drilled shafts have been used in **landslide stabilization** schemes. A drilled shaft wall or even **rows of shafts with space between rows** can be constructed **across a slip surface** to provide a **restraining force** to a sliding soil mass.

Drilled Shaft



Drilled Shaft / Pier Wall Types



US 52

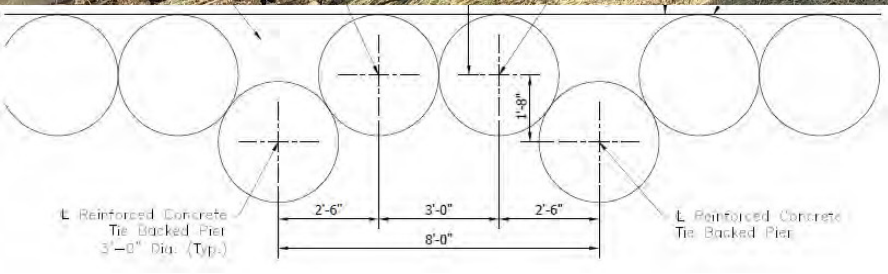
Drilled Shaft Wall with Lagging



Drilled Shaft Wall with Lagging

- Backfill with structural concrete to bottom of lagging.
- Excavate to Install Lagging Panels.
- Reinforced Precast Concrete or Timber Panels.
- Backfill behind wall with free-draining aggregate.
- Regrade slope in front of wall





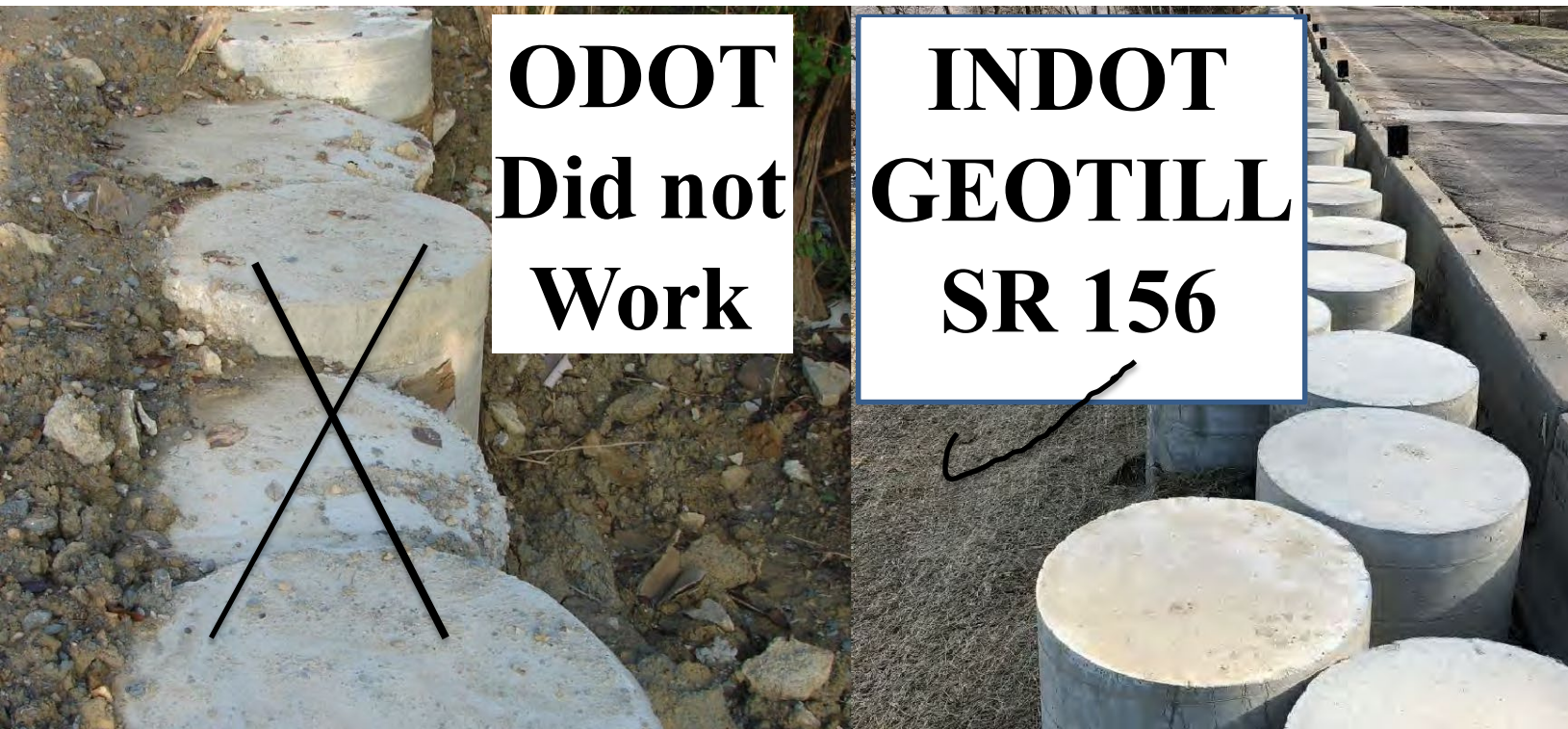
Drilled Shaft Wall with Plugin Piles

SR 156 RP 26.6

Ohio River

Drilled Shaft Wall with Plugin Piles

- Similar to Tangent Pile Wall. However, every other shaft or **every two shafts is reinforced**.
- Unreinforced shafts are generally **shorter** (they do not penetrate into bedrock) and serve the **purpose of lagging**.
- **Quick** and easy wall.





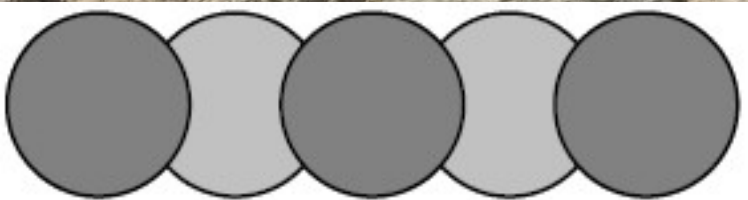
Drilled Shaft Wall with Tiebacks

SR 46



Drilled shaft with tieback 4 ft in diameter - 7 ft center to center – 40 ft deep

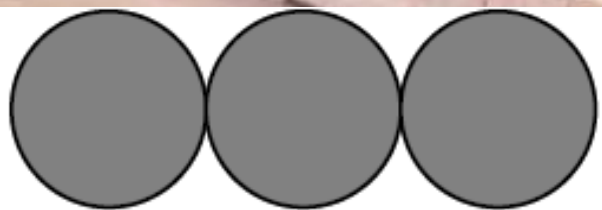
Drilled Shaft Wall with Tiebacks



Drilled Shaft Secant Wall



Drilled Shaft / Pier Wall Types (5)



Drilled Shaft Tangent Wall



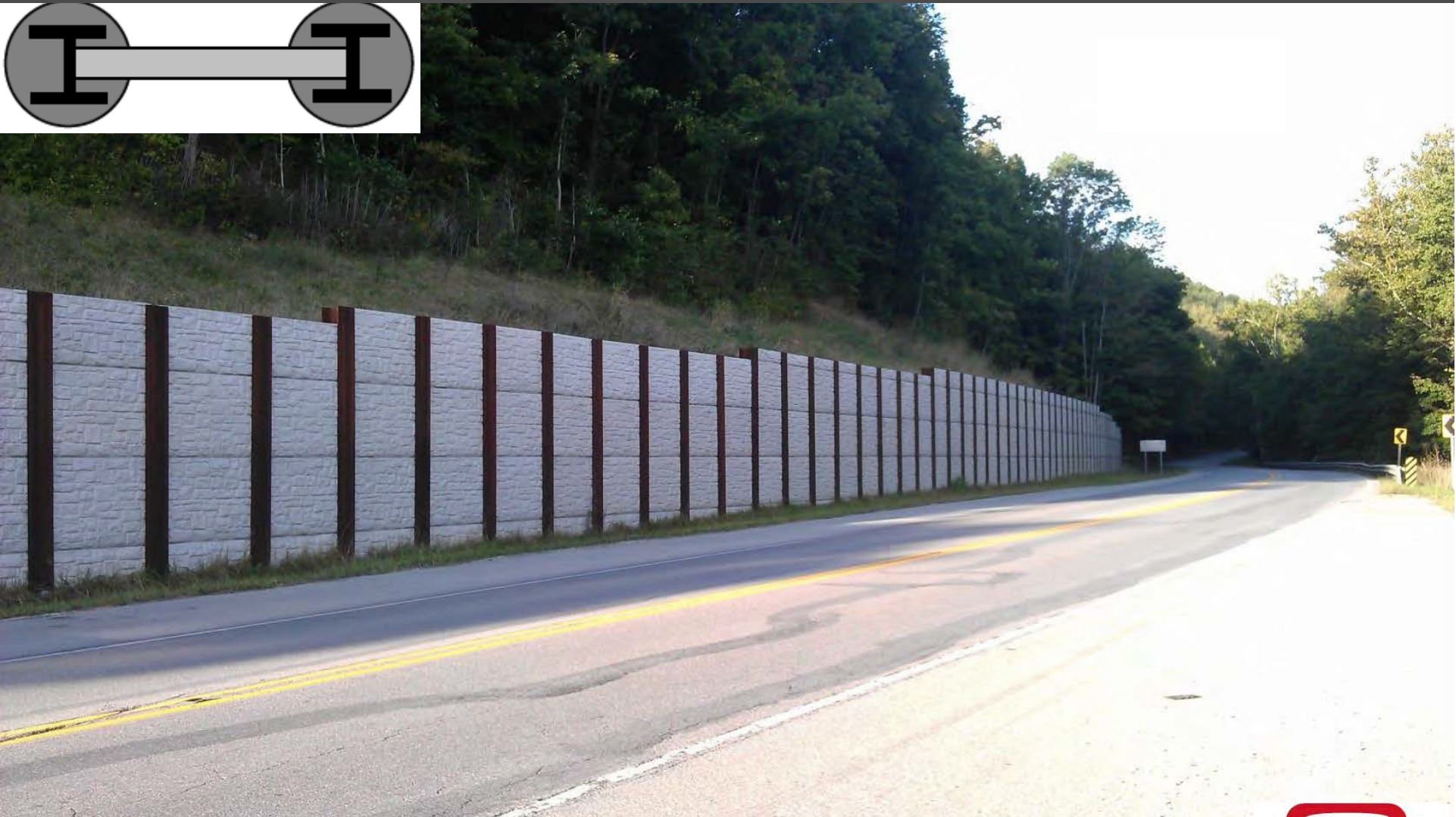
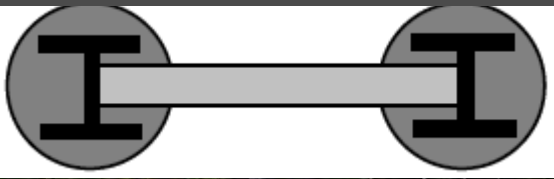
Drilled Shaft Tangent Wall

- Drilled shafts with a **center-to-center** spacing of one shaft diameter.
- With **every shaft reinforced**, this is the strongest type of drilled shaft retaining wall.
- Very **expensive** to construct.





Drilled Shaft / Pier Wall Types (6)



Drilled Shaft with Soldier Piles Wall and Lagging

14 HP - 410 ft long wall- spaced 6 ft center-to-center

Extend 10 ft into sound bedrock, drilled piers





DRILLED PIER WALL CONSTRUCTION PHOTOS



Drilled Shaft Wall Construction Sequence (2)





Drilled Shaft Wall Construction Sequence (3)



SR 66 Landslide, Perry County, Indiana





Drilled Shaft Wall Construction Sequence (4)



SR 66 Landslide, Perry County, Indiana





Drilled Shaft Wall Construction Sequence (5)



SR 66 Landslide, Perry County, Indiana





Drilled Shaft Wall Construction Sequence (6)



SR 66 Landslide, Perry County, Indiana





SR 66 Landslide, Perry County, Indiana



Drilled Shaft Wall Construction Sequence (8)



SR 66 Landslide, Perry County, Indiana





SR 66 Landslide, Perry County, Indiana



Drilled Shaft Wall Construction Sequence (10)



SR 66 Landslide, Perry County, Indiana





Drilled Shaft Wall Construction Sequence (11)



SR 66 Landslide, Perry County, Indiana



Drilled Shaft Wall Analysis and Design

- Past research relevant to the analysis of drilled shaft stabilized slopes include work by Merriam (1960), Andrews and Klasell (1964), Bulley (1965), Gould (1970), Ito and Matsui (1975), Oakland and Chameau at Purdue (1986), Reese (1992), Hassiotis et.al. (1997) Poulos (1999), and Liang and Zeng (2002).
- Slope stability is evaluated at the AASHTO (2014) Service I Load Combination relative to geotechnical resistance factors that are the inverse of the factor of safety (FS) computed by the various software available for slope analysis. In practice, the target geotechnical resistance factors (ϕ) of 0.75 and 0.65, as referenced in 11.6.2.3 of AASHTO (2014), are equal to a factor of safety (FS) of $1/\Phi$, or FS 1.33 and 1.53, respectively.
- Analyses of the overall slope may be performed using a limit equilibrium approach such as the Modified Bishop, Simplified Janbu, or Spencer methods, as available in several different geotechnical analysis software.
- If the existing slope is failing, the computed factor of safety should approximate 1.0, comparable to a geotechnical resistance factor of 1.0 for the Service Limit State, Should the computer simulated surface of failure differ significantly from the estimated shear failure surface based on surface observations and inclinometer data, the engineering properties, soil stratification and/or pore pressures within the slope should be adjusted in iterative “back-analyses” until the output from the computer analysis conforms to
- the observed conditions. A back-analysis that produces a geotechnical factor of safety of 1.0 (geotechnical resistance factor 1.0), but includes a calculated failure surface that is inconsistent with field observations should not be relied upon. All relevant parameters need to be consistent with observations.

SCHOOL OF
CIVIL ENGINEERING
INDIANA
DEPARTMENT OF HIGHWAYS

JOINT HIGHWAY RESEARCH PROJECT

FHWA/IN/JHRP-86/7

Final Report

DRILLED PIERS USED FOR SLOPE
STABILIZATION

M. W. Oakland
J. L. Chameau



SR 156 Landslide RP 24.1



SR 156 Landslide RP 24.1



SR 156
March, 2019
RP24.1

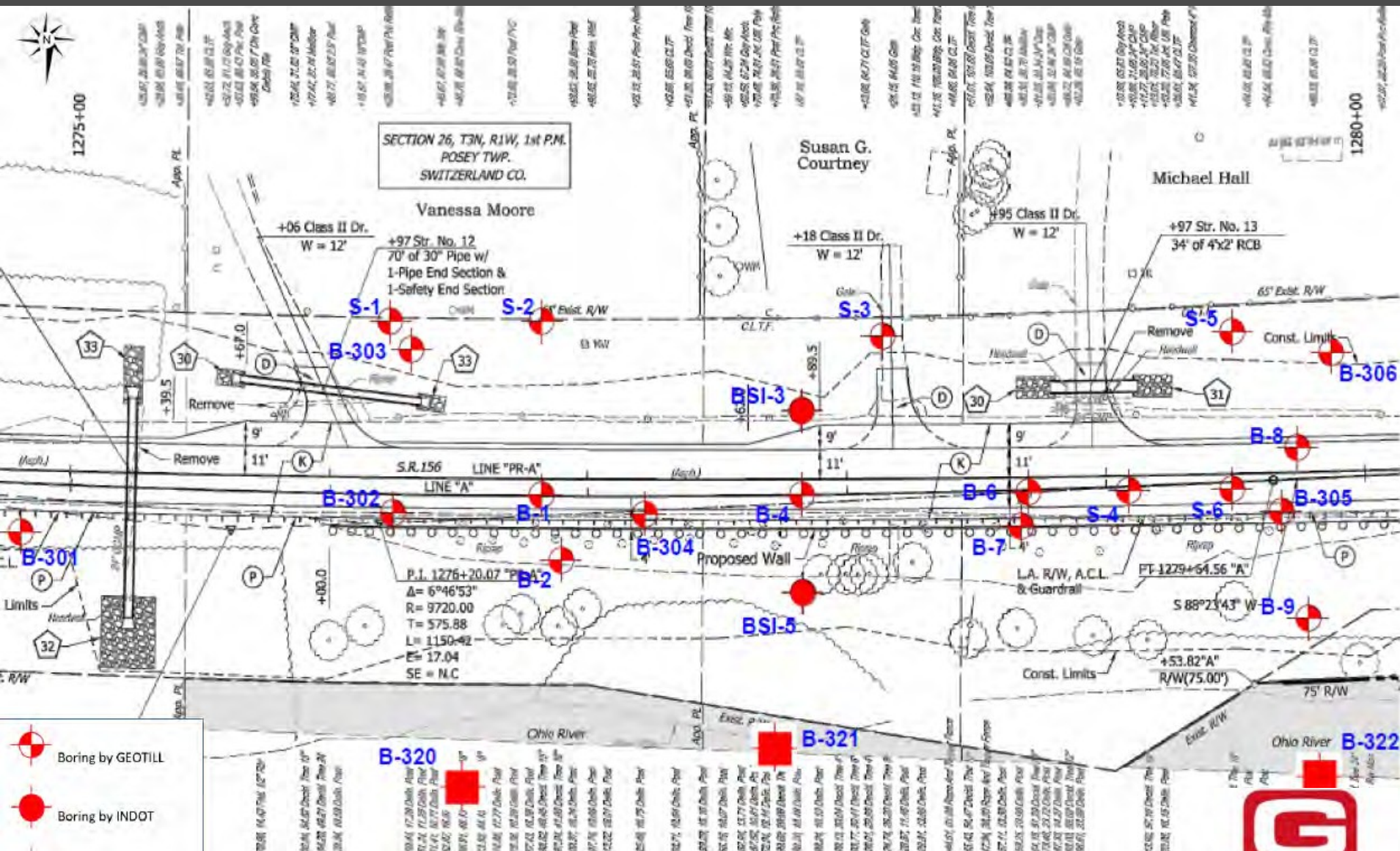
SR 156 Landslide RP 24.1



SR 156 Landslide RP 24.1 (Location of Test Borings)



SR 156 Landslide RP 24.1 (Location of Test Borings)



SR 156 Landslide RP 24.1 (Location of Test Borings)



SR 156 Landslide RP 24.1 (Launching Rig in Ohio River)

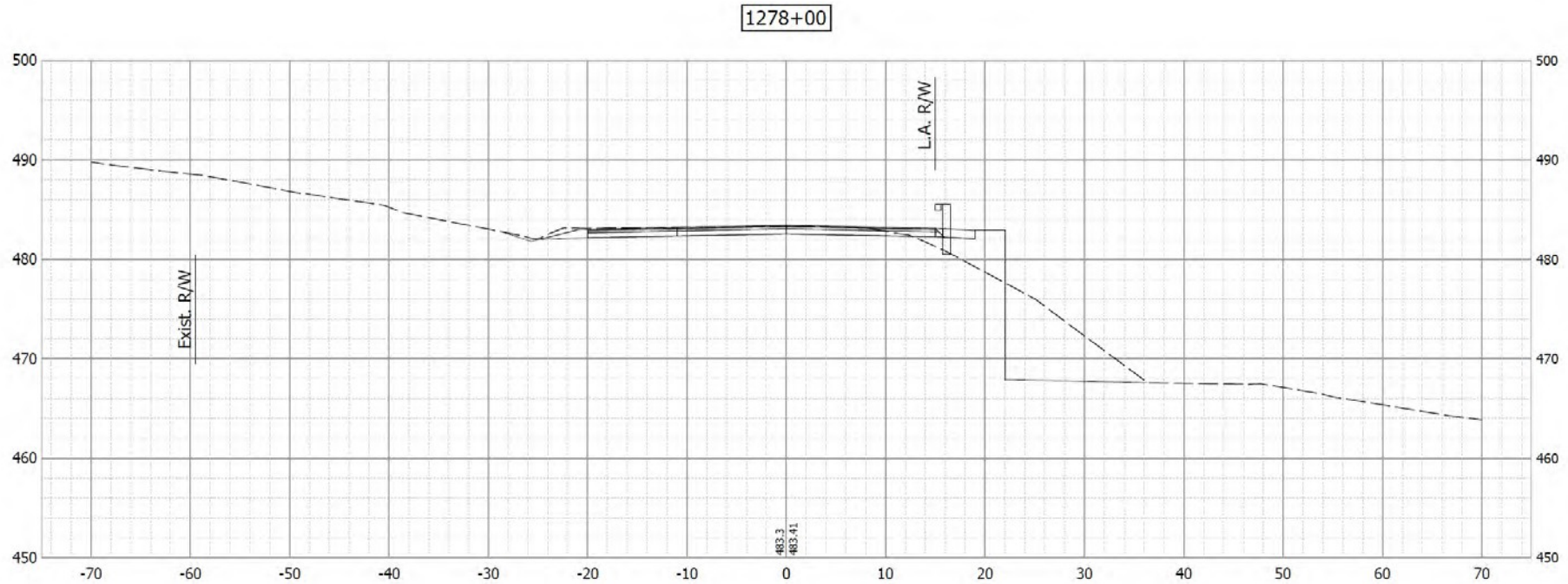


Drilled Shaft Wall Analysis and Design (10)

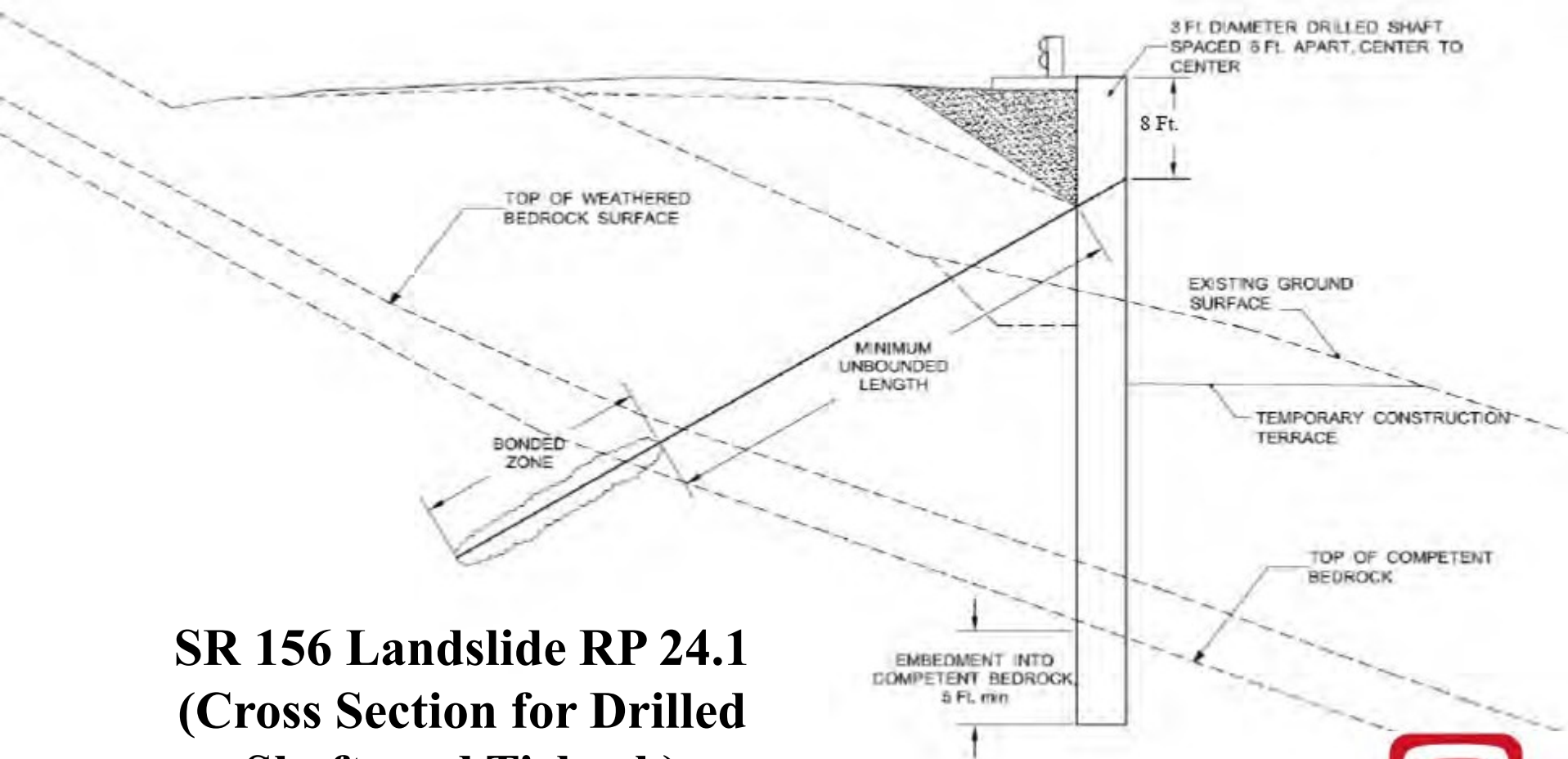


SR 156 Landslide RP 24.1 (Drilling in Ohio River)





SR 156 Landslide RP 24.1 (Cross Section from Plans)



**SR 156 Landslide RP 24.1
(Cross Section for Drilled
Shaft and Tieback)**

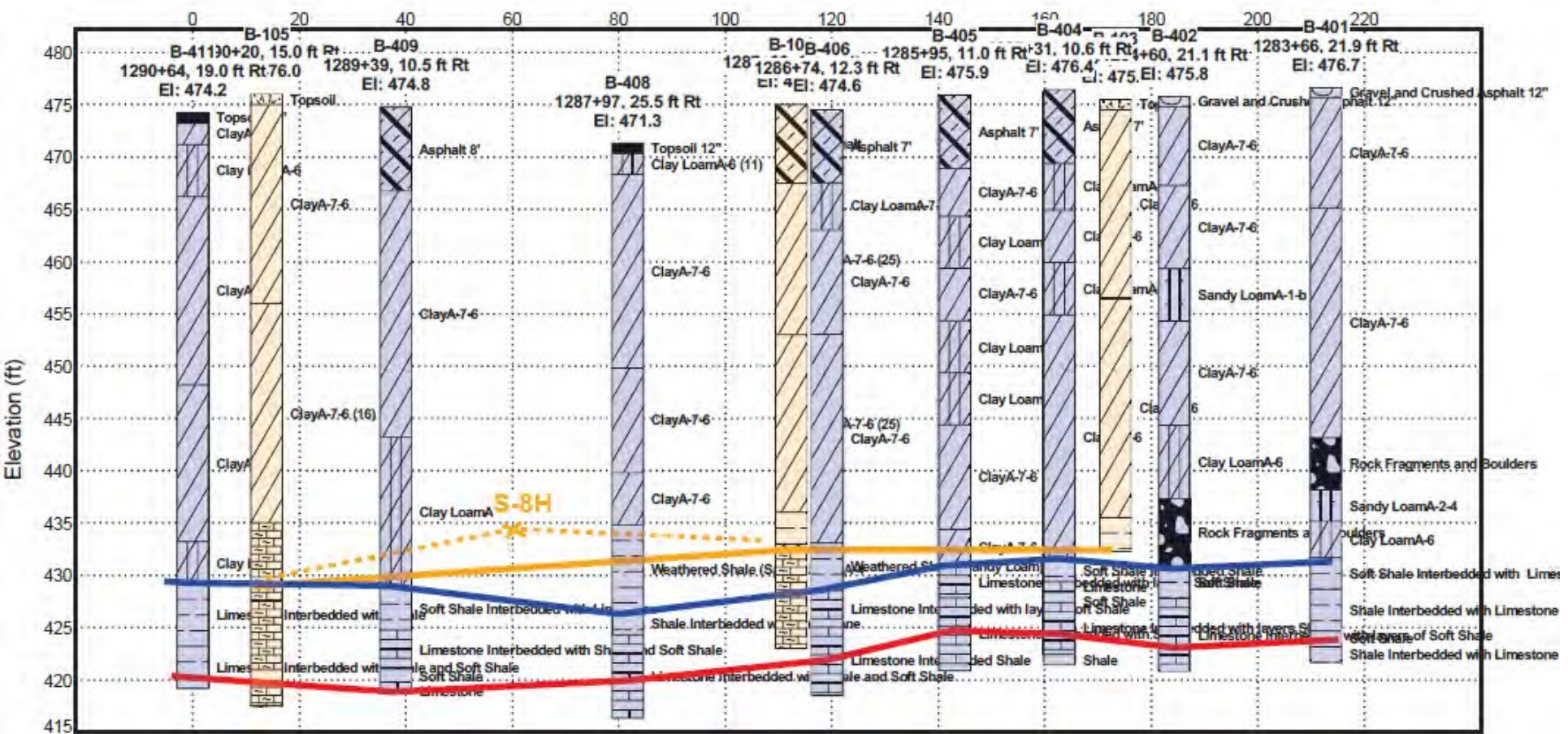
B-401
45.0' - 55.0'
Recovery: 120", RQD: 31" (26%)



SR 156 Landslide RP 24.1 (Soft Rock)



Drilled Shaft Wall Analysis and Design (13)



Auger Refusal (INDOT) 2015

Auger Refusal (GEOTILL) 2019

Proposed Competent Rock (GEOTILL) 2019



SR 156 Landslide RP 24.1 (Soft Rock)





Specimen before running the test



Turbidity of slake apparatus after running 2 cycles



Specimen after running 2 cycles

Durability test
 Sample # 2
 Soft shale
 from SR156
 Mohamed

Elevation (ft)	Rock	Water Content (%)	Slake Durability Index	Type
435	Shale	4.6	32.6%	III

SR 156 Landslide RP 24.1 (Slake Durability Test for Soft Shale)



Specimen before running the test



Turbidity of slake apparatus after running 2 cycles

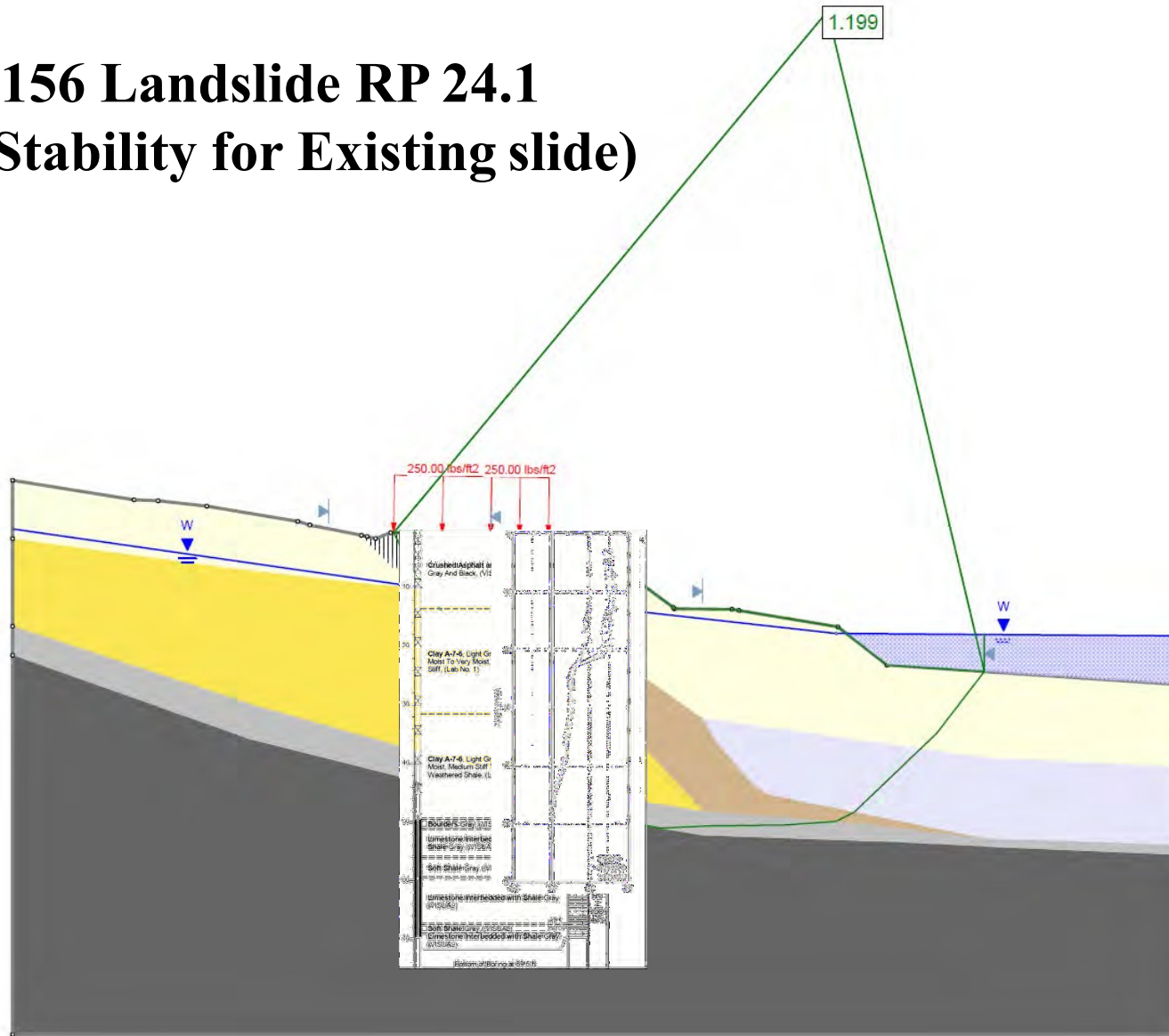


Specimen after running 2 cycles

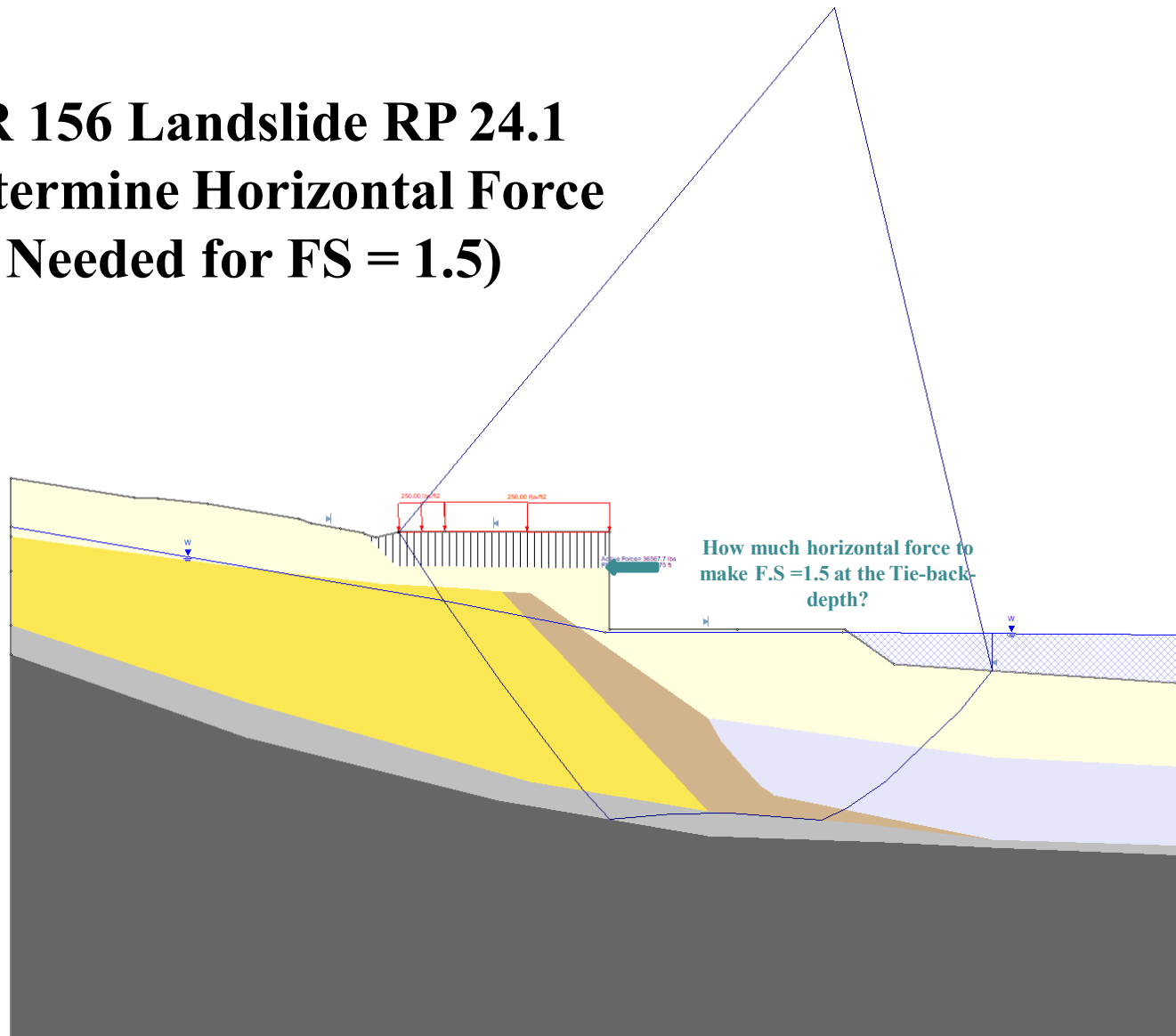
Elevation (ft)	Rock	Water Content (%)	Slake Durability Index	Type
421	Hard Shale	2.4	83.1%	II

SR 156 Landslide RP 24.1 (Slake Durability Test for Hard Shale)

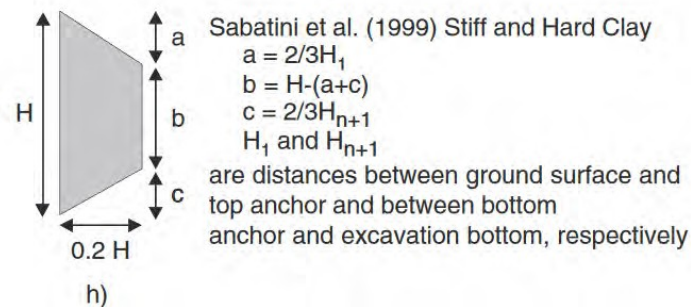
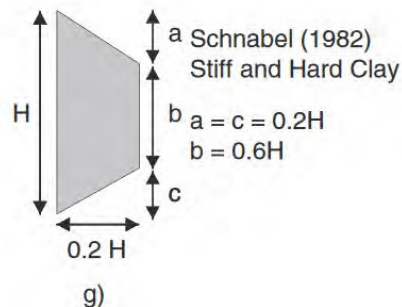
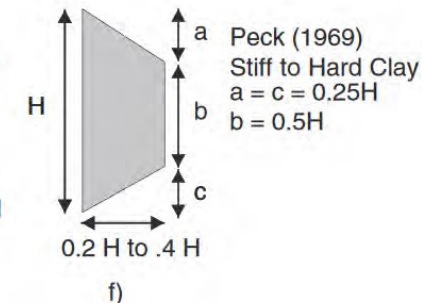
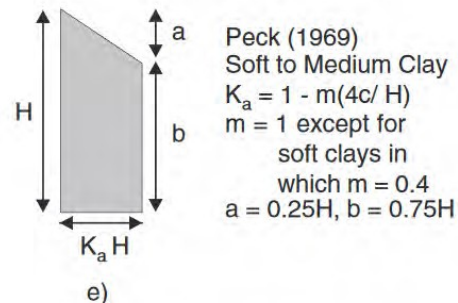
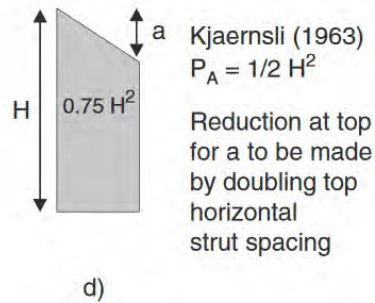
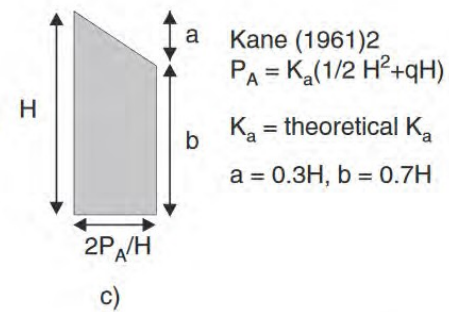
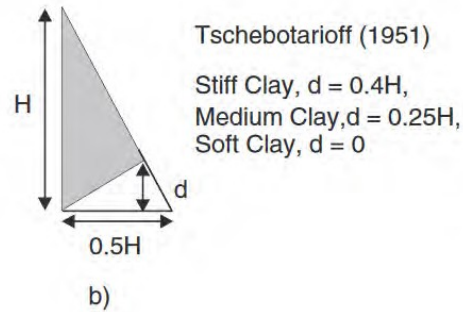
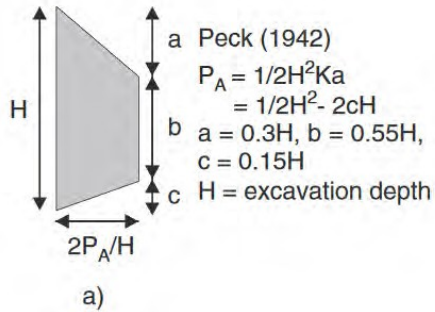
SR 156 Landslide RP 24.1 (Slope Stability for Existing slide)



SR 156 Landslide RP 24.1 (Determine Horizontal Force Needed for $FS = 1.5$)



SR 156 Landslide RP 24.1 (Apparent Earth Pressure Diagrams)



SHORING SUITE V8 - [SR-156 24.1 S2.SH8 - Shoring]

Shoring MENU File Edit Run Modules Help

Select sample from list:

A. General | B. Pressures | C. Braces and Force | D. Options | E. Two Walls

1. Wall Height, H: 60 General Title: SR-156 Landslide Correction 24.1 Station 1278+00

2. Wall Type:
 1. Sheet Pile 4. Secant/Tangent
 2. Soldier Pile, Drilled 5. Concrete/Slurry Wall
 3. Soldier Pile, Driving 6. Trench Box/Int. Strut

3. Shaft Diameter, D (sheet pile=1): 3

4. Pile Spacing, S (sheet pile=1): 8

5. Auto Fill Item 6.7 (table below) based on Item 2.3.4

6. Active Spacing or Width

Z (depth)	Spacing
0	8.00
20	3

7. Passive Arching Width or Width

Z (depth)	Width
60	3

Passive Arching:
 1D: loose sand or silt and clay;
 2D: Medium Sand and silt;
 3D: Dense sand.

For Driving Pile: Additional Multiplier X 1.5

Diagram: Soldier Pile and Lagging, Driving Pile (X 1.5), Drilled Shaft (X 1)

DEPTH: ft, PRESSURE/FRICTION/BEARING: ksf, SLOPE: kcf, FORCE: kip, MOMENT: kip-ft, DEFLECTION: in, I: in⁴, E: ksi

SR-156 Landslide Correction 24.1 Station 1278+00

Geotill Project No. 111902121



Wall Height=60.0 Pile Diameter=3.0 Pile Spacing=8.0 Wall Type: 2 Soldier Pile, Drilled

PILE LENGTH: Min. Embedment=0.92 (8-10 is recommended) Min. Pile Length=60.92
 MOMENT IN PILE: Max. Moment=798.67 per Pile Spacing=8.0 at Depth=28.98

PILE SELECTION

Request Min. Section Modulus = 242.0 in³/pile, Fy = 60 ksi = 414 MPa, Fb/Fy=0.66
 User Input I (Moment of Inertia):

Top Deflection = -0.67(in) based on Elastic Modulo, E (ksi) = 3605.00 and Moment of Inertia, I (in⁴/pile) = 82447.0

BRACE FORCE: Strut, Tieback, Plate Anchor, and Deadman

No. & Type	Depth	Angle	Space	Total F	Horiz. F	Vert. F	L free	Fixed Length
1. Tieback	8.0	30.0	8.0	172.2	149.1	86.1	26.8	23.4

UNITS: Width, Diameter, Spacing, Length, Depth, and Height - ft; Force - kip; Bond Strength and Pressure - ksf

DRIVING PRESSURES (ACTIVE, WATER, & SURCHARGE):

Z1	P1	Z2	P2	Slope
0.0	0.00	5.3	0.90	0.170
5.3	0.90	25.3	0.90	0.000
25.3	0.90	60.0	0.00	-0.026

PASSIVE PRESSURES: Pressures below will be divided by a Factor of Safety = 2

Z1	P1	Z2	P2	Slope
60.0	20.00	999.0	20.00	0.000

ACTIVE SPACING:

No.	Z depth	Spacing
1	0.00	8.00
2	20.00	3.00

SR 156 Landslide RP 24.1 (Moment, shear force and deflection)

SHORING SUITE V8 - [SR-156 24.1 S2.SH8 - Shoring]

Shoring MENU File Edit Run Modules Help

Results Report Diagram Select sample from list:

A. General | B. Pressures | C. Braces and Force | D. Options | E. Two Walls

1. Wall Height, H: 60 General Title: SR-156 Landslide Correction 24.1 Station 1278+00

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4. Pile Spacing, S (sheet pile=1): 8

5. Auto Fill Item 6.7 (table below) based on Item 2.3.4

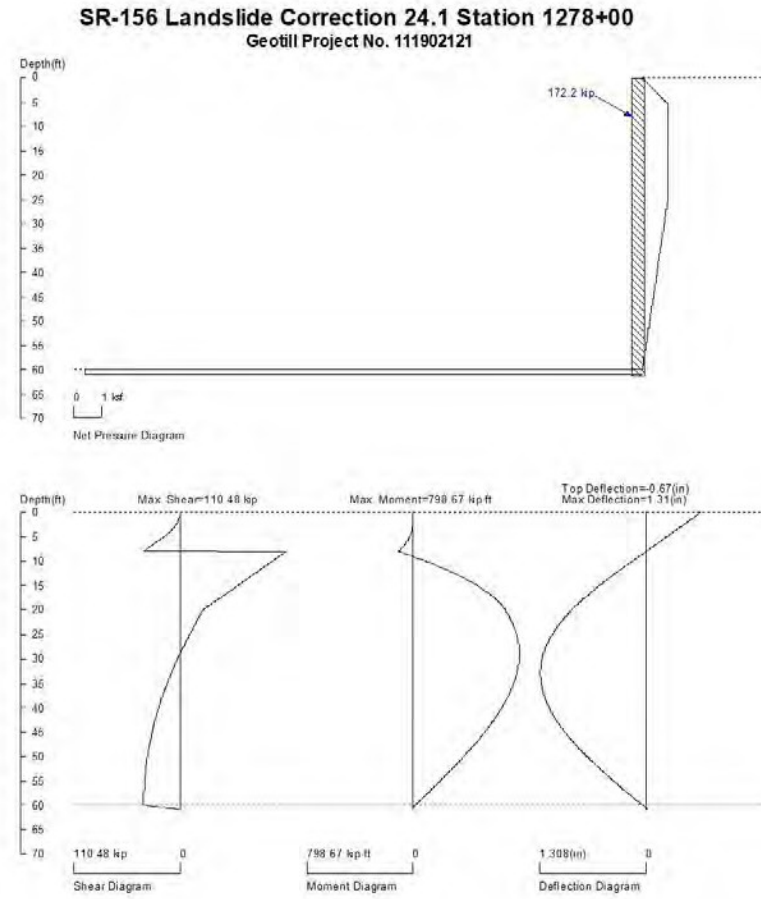
6. Active Spacing or Width 7. Passive Arching Width or Width

Z (depth)	Spacing	Z (depth)	Width
0	8.00	60	3
20	3		

Passive Arching:
 1D: loose sand or silt and clay;
 2D: Medium Sand and silt;
 3D: Dense sand.

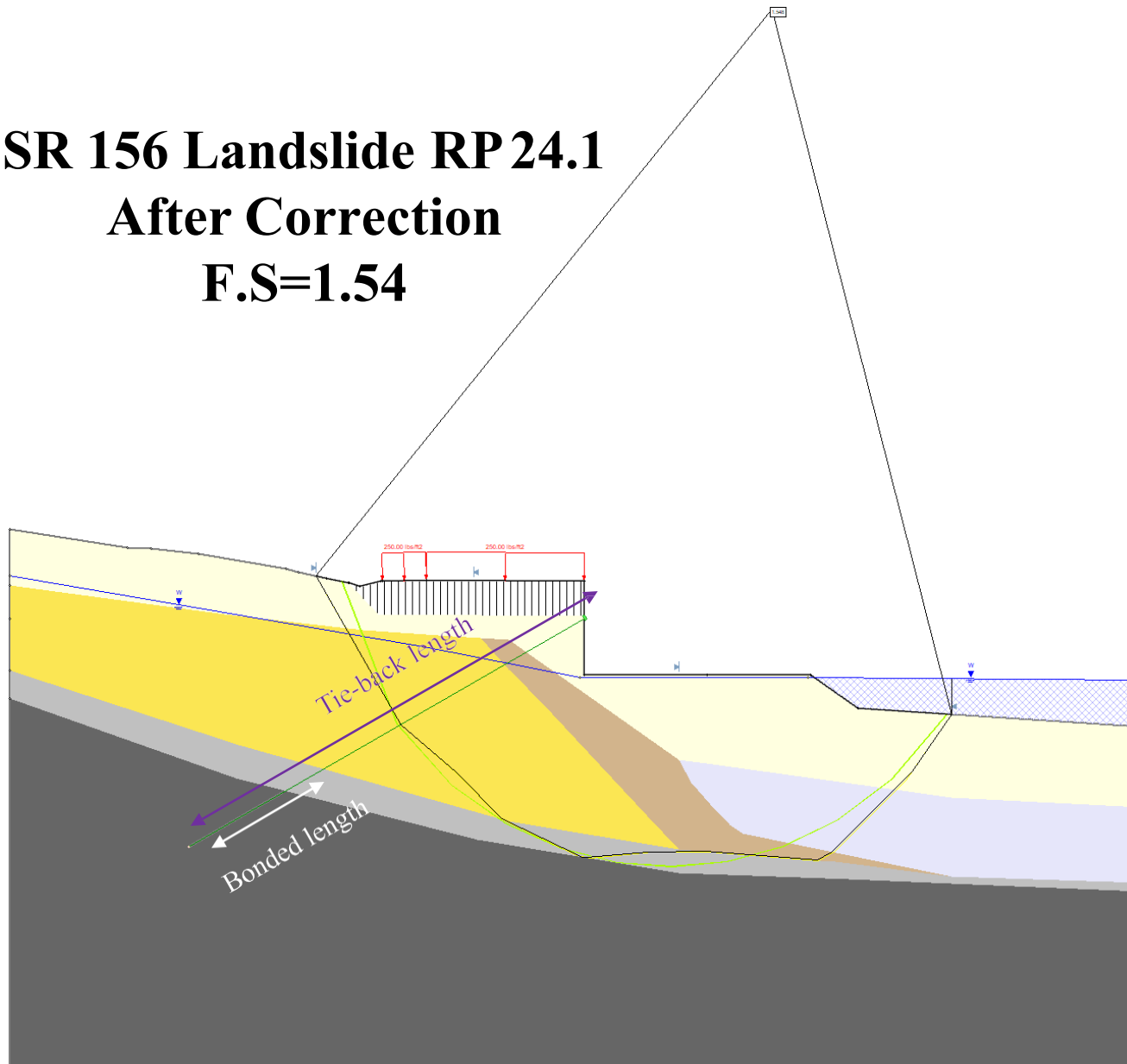
For Driving Pile: Additional Multiplier X 1.5

DEPTH: ft, PRESSURE/FRICTION/BEARING: ksf, SLOPE: kcf, FORCE: kip, MOMENT: kip-ft, DEFLECTION: in, I: in⁴, E: ksi

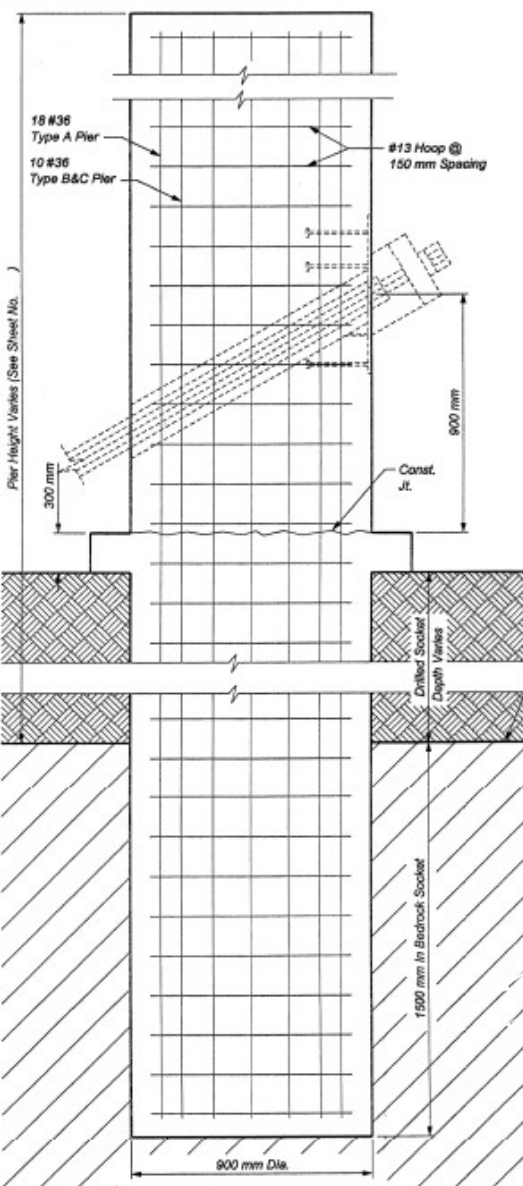


SR 156 Landslide RP 24.1 (Moment, shear force and deflection)

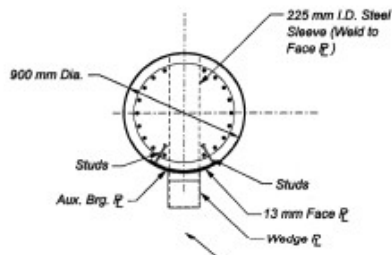
SR 156 Landslide RP 24.1
After Correction
F.S=1.54



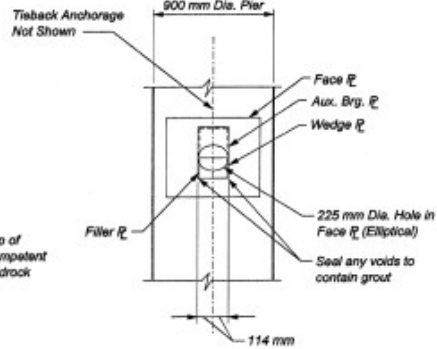
Drilled Shaft Wall Reinforcement



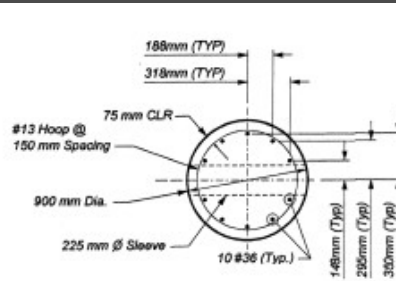
PIER REINFORCEMENT DETAIL
SCALE: 1:10



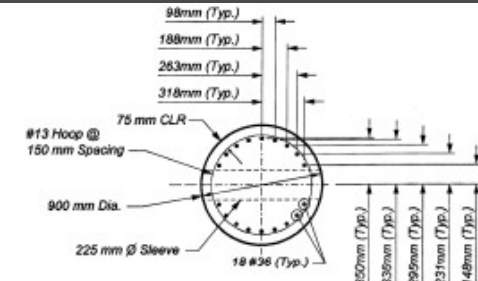
SECTION "A-A"
SCALE: 1:20



SECTION "B-B"
SCALE: 1:20



TYPICAL CROSS SECTION @ PIER B & C
SCALE: 1:20



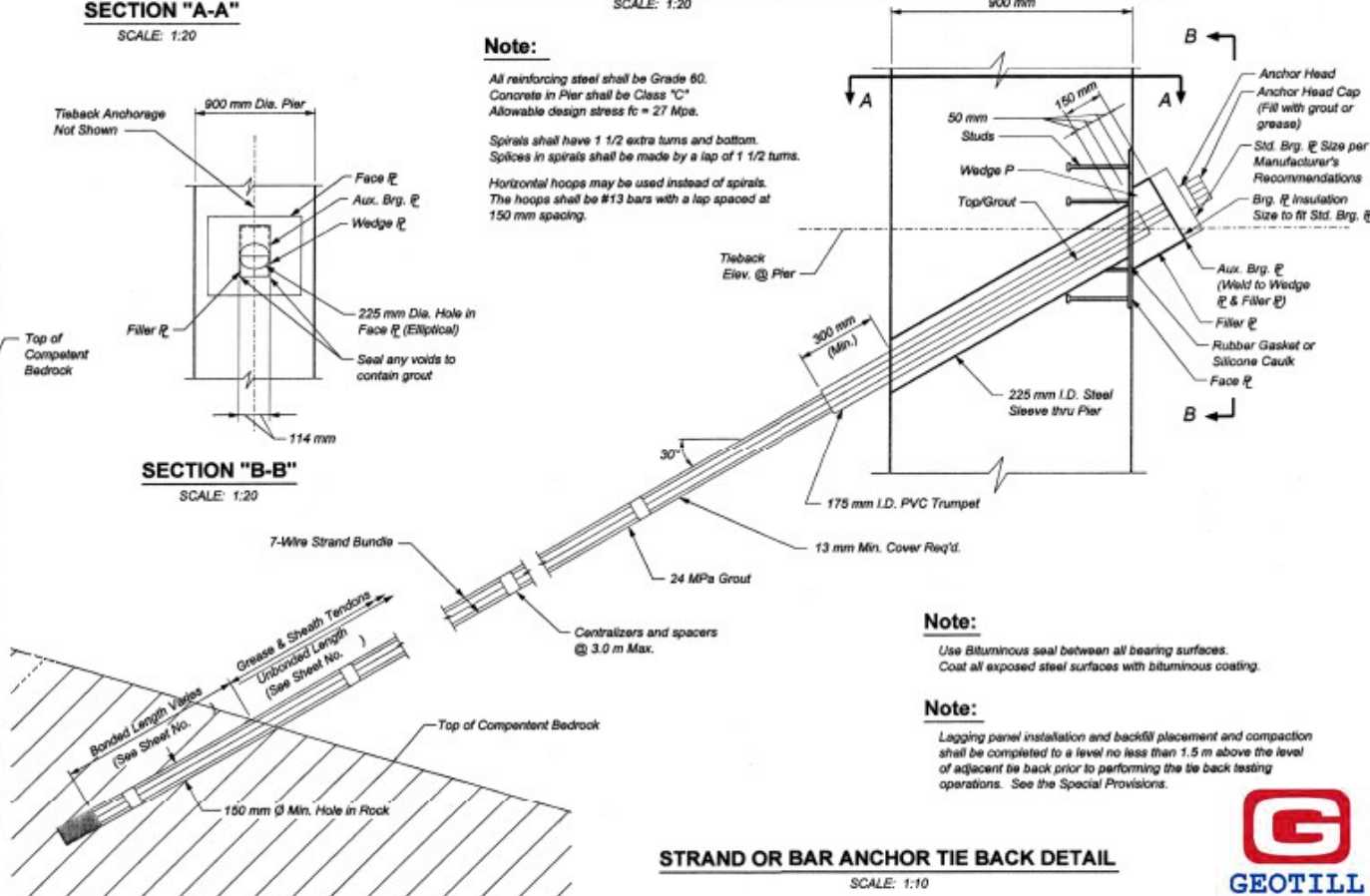
TYPICAL CROSS SECTION @ PIER A
SCALE: 1:20

Note:

All reinforcing steel shall be Grade 60.
Concrete in Pier shall be Class "C"
Allowable design stress $f_c = 27$ Mpa.

Spirals shall have 1 1/2 extra turns and bottom.
Splices in spirals shall be made by a lap of 1 1/2 turns.

Horizontal hoops may be used instead of spirals.
The hoops shall be #13 bars with a lap spaced at 150 mm spacing.



STRAND OR BAR ANCHOR TIE BACK DETAIL
SCALE: 1:10

Note:

Use Bituminous seal between all bearing surfaces.
Coat all exposed steel surfaces with bituminous coating.

Note:

Lagging panel installation and backfill placement and compaction shall be completed to a level no less than 1.5 m above the level of adjacent tie back prior to performing the tie back testing operations. See the Special Provisions.



Drilled Shaft Wall SR 156 Case History



**SR 156
Ohio River**





Drilled Shaft Wall SR 156 Case History



**SR 156
Ohio River**



Drilled Shaft Wall SR 156 Case History (2)



SR 156
Ohio River



Drilled Shaft Wall SR 156 Case History (3)

SR 156
Ohio River





Drilled Shaft Wall SR 156 Case History (4)

SR 156
Ohio River



SR 156
Ohio River





SR 156
Ohio River

SR 156
Ohio River

Gap

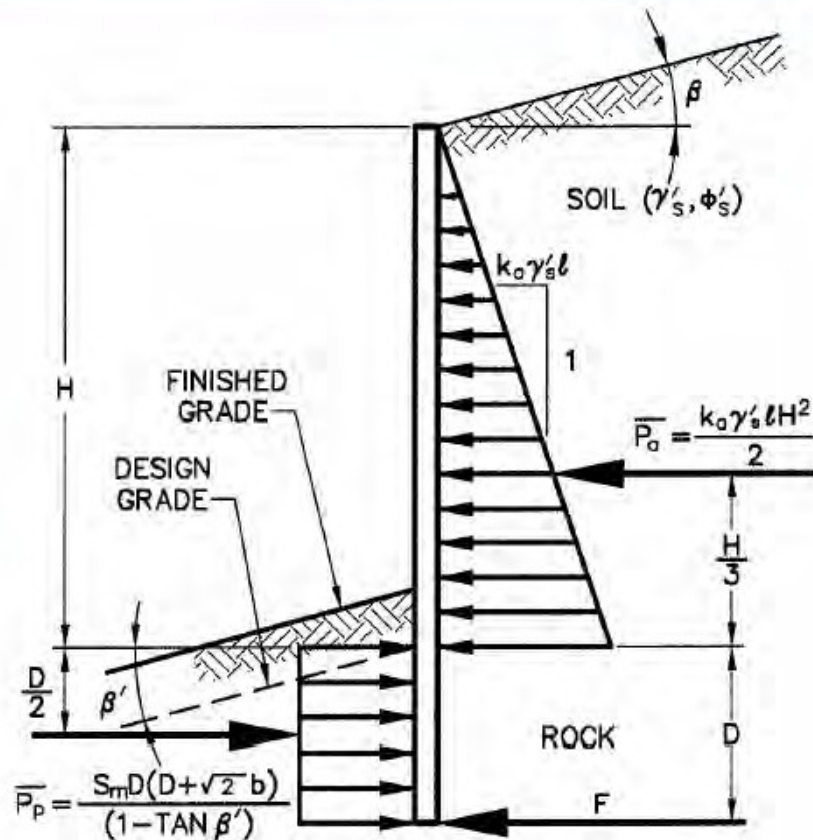
Passive pressure from Bin wall
in front of the Pier wall Yes / No

SR 46 RP 150 Dearborn County Drilled Shaft Wall

In 1952 bin-wall was constructed



**SR 46
RP 150
Dearborn
County
Drilled
Shaft
Wall**



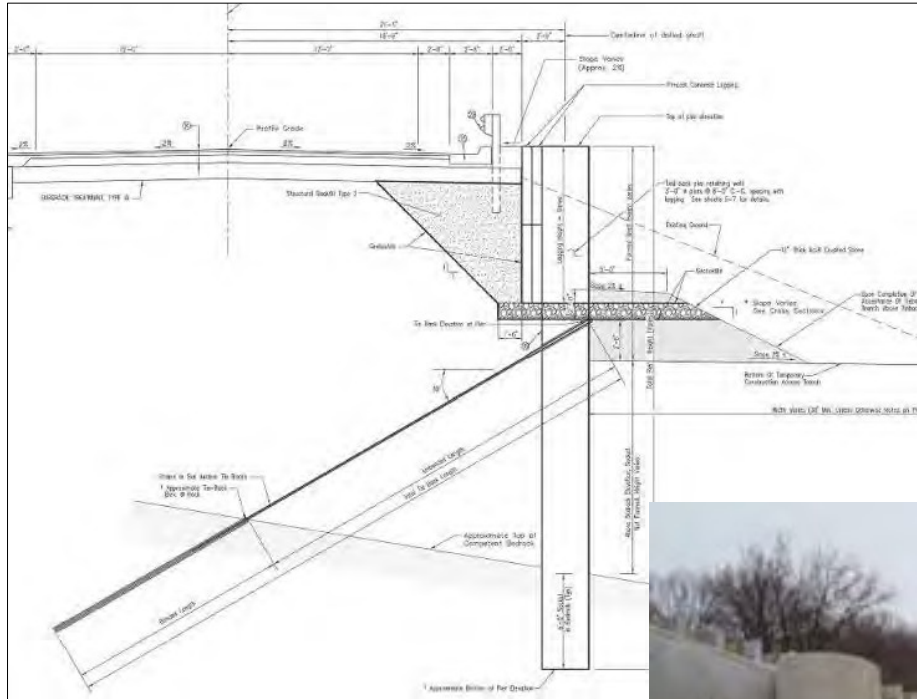
b = ACTUAL WIDTH OF EMBEDDED DISCRETE VERTICAL WALL ELEMENT BELOW DESIGN GRADE IN PLANE OF WALL (FT.).

Figure 3.11.5.6-2—Unfactored Simplified Earth Pressure Distributions for Permanent Nongravity Cantilevered Walls with **Discrete** Vertical Wall Elements Embedded in **Rock**

Passive pressure for drilled shaft wall embedded in Rock



- When piers are spaced 5 pier diameters or less apart, ground loads will tend to arch onto the stiff inclusions, as shown here.
- A semi-circular zone of tension will develop between the piers.



During the design, it has been decided to shift the wall location, what you do about the Geotechnical Investigation that it has been already done?

2000 ft long wall
 300 drilled shafts with
 tied-back
 3 ft in diameter
 8 ft center to center
 5 ft into the bedrock



SR 56
 Switzerland
 County
 Vevay
 Drilled
 Shaft Wall



SR 56 Switzerland County Vevay Drilled Shaft Wall

SR 56 Vevay



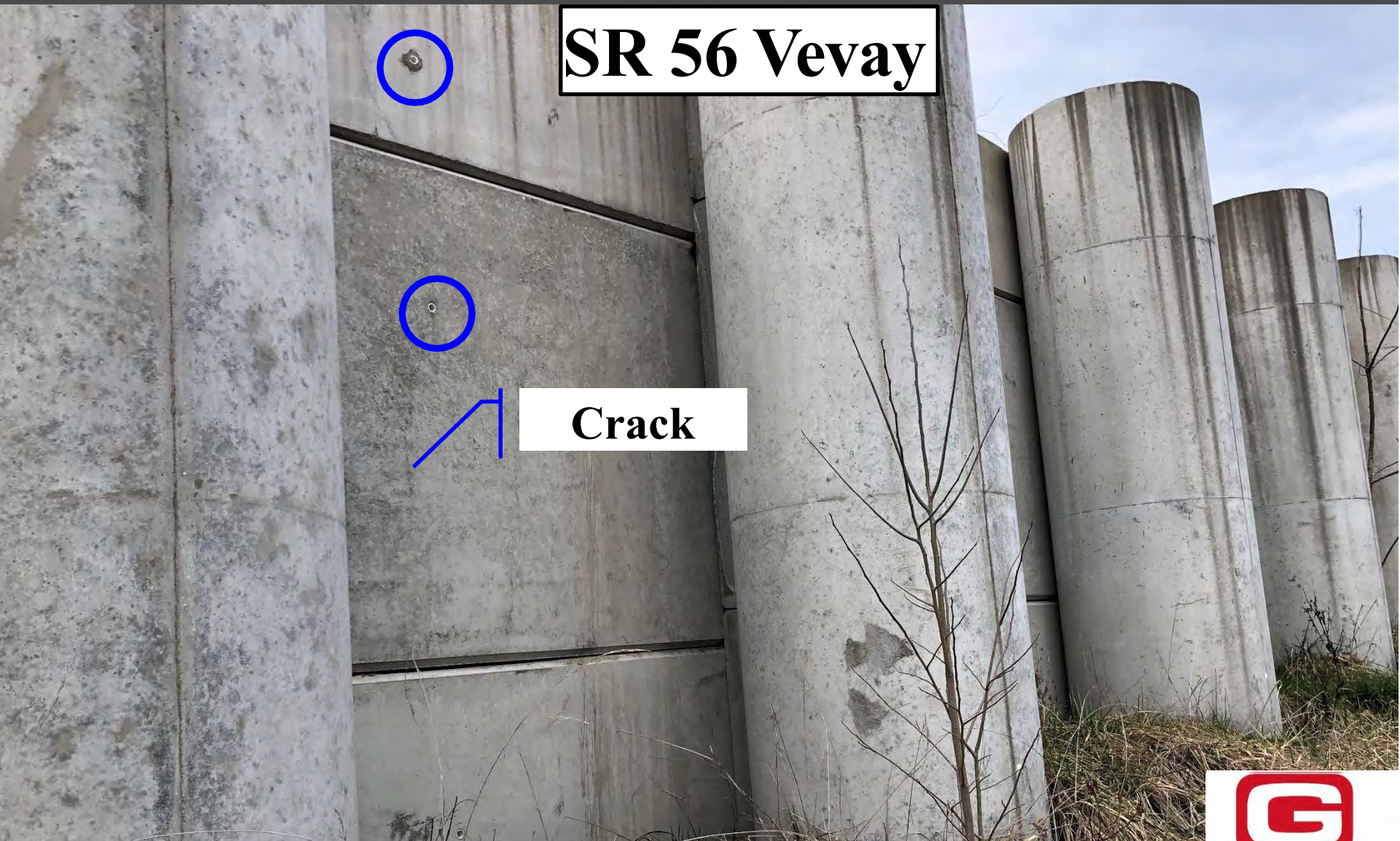
Drainage Design Issues

**SR 56 Vevay
Needs thick layer
of shot rock
riprap**



**SR 56 Vevay
Water drop from
high hill side into
the culvert**





Weep Hole Design Issues

SR 56 Vevay



Crack



Weep Hole Design Issues



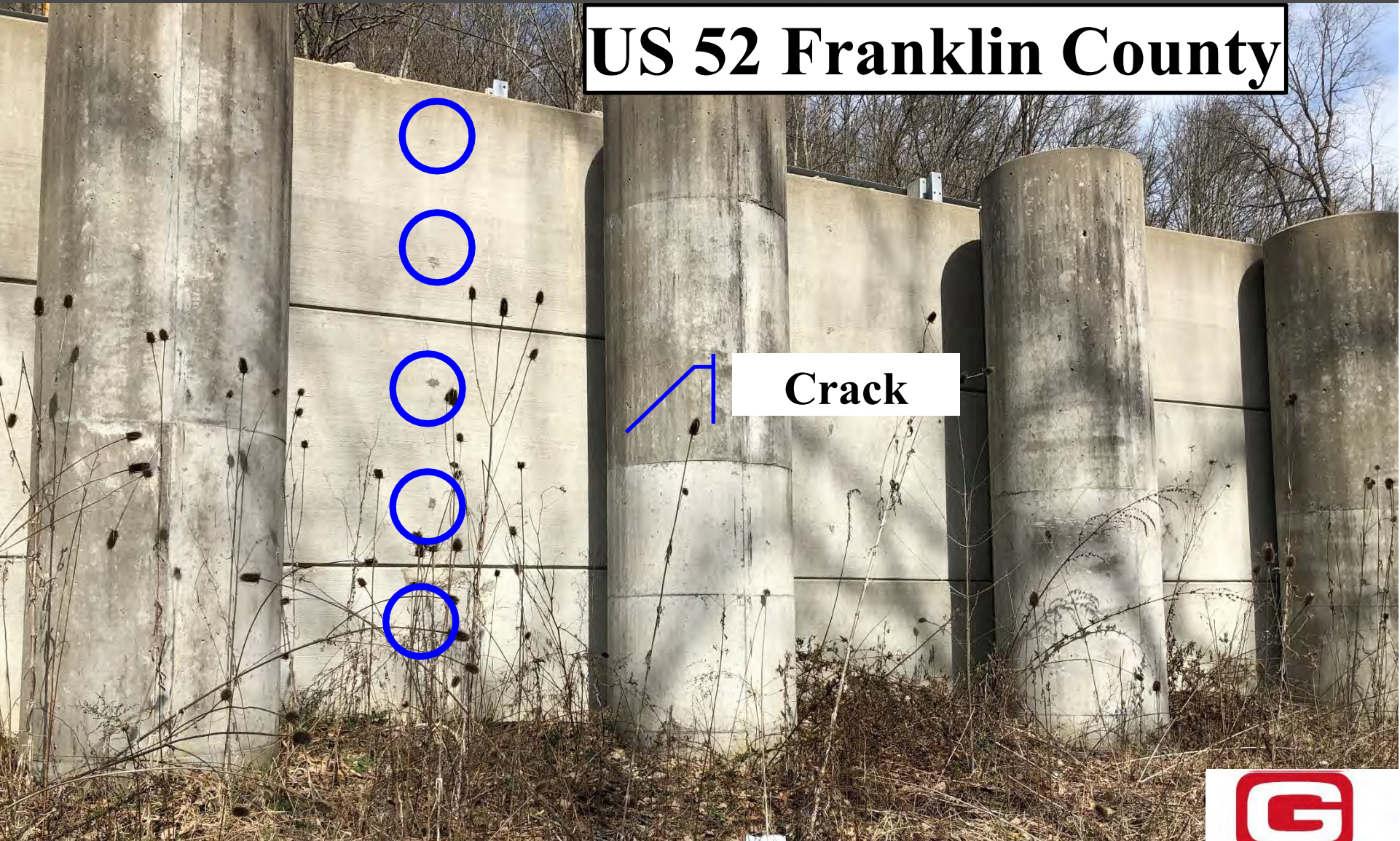
SR 237 Perry County

The way it should be



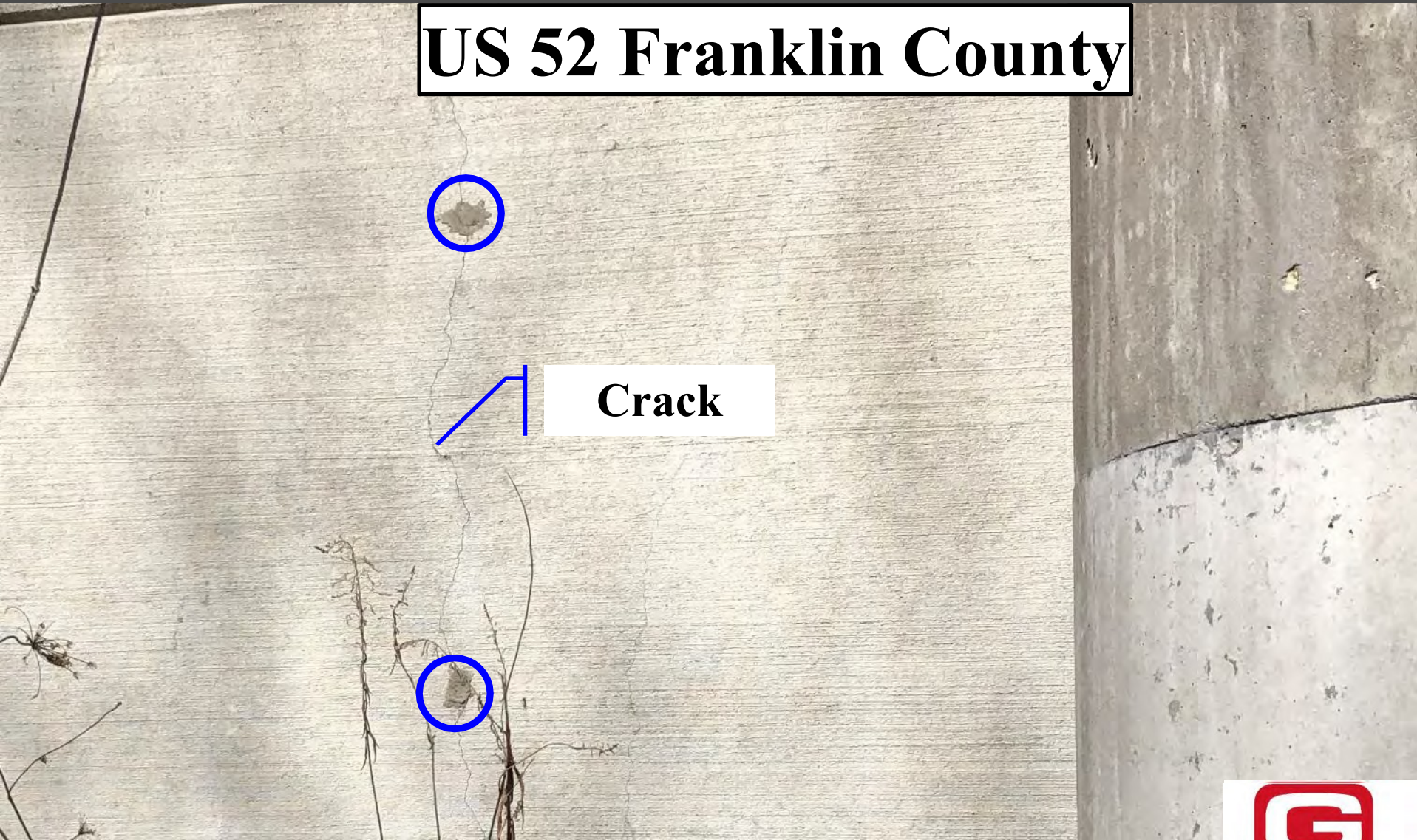
Weep Hole Design Issues

US 52 Franklin County



Weep Hole Design Issues

US 52 Franklin County



Weep Hole Design Issues

Questions ?

Dr. Malek Smadi, P.E.

Principal Engineer - GEOTILL - Fishers, Indiana

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