University of Nevada

Reno

Dendrochronologic Reconstruction of Water Levels v for Pyramid Lake, Nevada 1745 to 1904 A.D.

A thesis submitted in partial fulfillment of the requirements for the degree of Master of Science in Hydrogeology

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1.0 ABSTRACT

The objective of this study is to reconstruct a lake level hydrograph of Pyramid Lake, Nevada over the period of 1745 to 1904 by using tree-ring widths.

Validation of the model reproduced the observed elevations of the lake to within five percent using observed inflows, and to within twenty percent using tree-ring generated inflows.

Modeling the lake between 1745 and 1904 indicates the fluctuations of the lake were relatively subdued, with the maximum range in elevation being only sixteen feet. It is expected from various limitations within the model that this range of elevation should be twenty-five to fifty percent greater. The mean elevation of the lake in this period is the spill elevation of 3863 feet. This mean elevation would largely be determined by the average long term climatic conditions which the tree-ring data suggest were fairly stable.

2.0 ACKNOWLEDGMENTS

To the late Dr. George B. Maxey, Executive Director of the Water Resources Center, Desert Research Institute and Professor of Geology Mackay School of Mines, University of Nevada, Reno, I extend my gratitude for the guidance and support he gave to me throughout my graduate career.

I wish to thank Dr. Paul Fenske for his advice and criticsm throughout this project. I also thank Professor Alfred Cunningham for his suggestion of this topic, as well as his considerable advice and criticism.

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1. Topographic Map of the Truckee River System

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3.0 INTRODUCTION

1

The objective of this study is to reconstruct the history of the surface elevation fluctuations of Pyramid Lake, Nevada prior to the scattered written records that begin in 1844, as well as the complete records which begin in 1932. To achieve this a correlation has been developed between the annual streamflow of the Truckee River and the annual growthring in drought sensitive trees. This tree-ring generated streamflow has been used as input to a mathematical model of Truckee River-Pyramid Lake.

The Truckee River (Figure 1) drains a large area of the east flank of the Sierra Nevadas as well as a portion of the western Great Basin. Precipitation over the area rapidly decreases from west to east because of orographic and rain shadow effects. For instance, the Truckee Ranger Station (No. 10, Figure 1) on the western edge of the basin, receives an average of 2.70 feet of water per year, while at Nixon, Nevada, (No. 1, Figure 1) the average precipitation is only 0.59 feet. Most of the flow of the Truckee River comes from the winter snows on the Sierra Nevadas.

The present terminus of the Truckee River system is Pyramid Lake. Prior to the completion in 1903 of the Truckee Canal (No. 6, Figure 1) as part of the Newlands Project, Pyramid Lake would periodically rise above the lip of its basin, at 3863 feet, and overflow into Winnemucca Lake through Winnemucca (Mud) Slough (near Nixon, No.1, Figure 1).

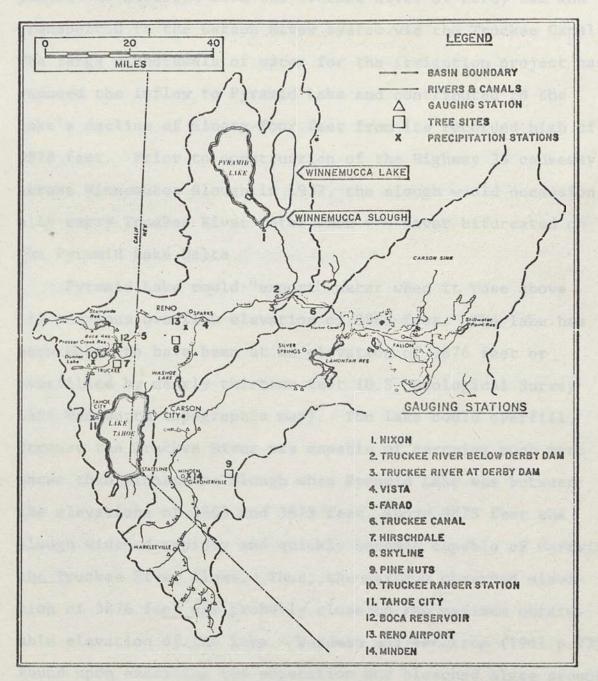


FIGURE 1. Truckee and Carson River System

adapted from BATEMAN, ET AL (1976)

The Newlands Project is an irrigation project which has brought under cultivation thousands of acres of desert surrounding Fallon,Nevada. A large portion of the water used by this project is diverted from the Truckee River at Derby Dam and transported to the Carson River system via the Truckee Canal. The large withdrawals of water for the irrigation project has reduced the inflow to Pyramid Lake and contributed to the lake's decline of ninety-four feet from its recorded high of 3878 feet. Prior to construction of the Highway 34 causeway across Winnemucca Slough in 1937, the slough would occassionally carry Truckee River water when the river bifurcated on the Pyramid Lake Delta.

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Pyramid Lake could "export" water when it rose above its previous overflow elevation of 3863 feet. The lake has been known to have been at an elevation of 3876 feet or overfilled by nearly thirteen feet (U.S. Geological Survey 1894 Wadsworth topographic map). The lake could overfill, because the Truckee River was capable of carrying much more water than Winnemucca Slough when Pyramid Lake was between the elevations of 3863 and 3873 feet. Above 3873 feet the slough widened rapidly and quickly becomes capable of carrying the Truckee River flows. Thus, the maximum observed elevation of 3876 feet was probably close to the maximum obtainable elevation of the lake. Hardman and Venstrom (1941,p.77) found upon examining the vegetation and bleached algae around Winnemucca Lake that the lake "has not exceeded a level of about 3855 feet in recent years". This is eight feet below the lip of the slough. Based upon the same lines of evidence, as well as photographs taken by Russell in 1882 they found "the highest recent elevation of Pyramid Lake was not greater than 3879 feet" (1941, p. 75). It is apparent from Hardman and Venstrom's observations that Winnemucca Lake had not filled up and therefore would have been an infinite sink for any water spilled from Pyramid Lake. Thus, Pyramid was free to establish a mean elevation that is not influenced by Winnemucca Lake. This mean elevation would apply as long as the long term climatic conditions remained stable.

The stability of the climate is a fundamental assumption of this study. It is assumed that the long term climatic conditions which have existed during this century are representative of the conditions which occurred during the period to be reconstructed.

It is known that during the last ice age, about 10,000 years ago, the climate was much different than today because both Pyramid and Winnemucca Lakes were only a small part of Lake Lahonton which covered much of the western Great Basin. There have probably been similar, though smaller magnitude wet periods since this last glacial period, but their frequency and severity are unknown. The tree-ring data, plotted in Appendix A, gives some climatic information about the last 450 years. These data show no major long term changes in the rate of growth of the trees from 1745 to 1972. This implies that the climatic conditions were relatively stable from 1745 through 1972. (The correlation

between tree growth and precipitation will be discussed later). Thus, it is realistic to make the assumption that the mean climatic conditions have been stable over the past 232 years. Prior to 1745, the Skyline chronology suggests a rather radically changing climate existed, but the Hirschdale chronology does not show the same magnitude of shifts.

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Virtually all water that enters these two terminal lakes is ultimately evaporated. Harding (1962) has determined the average evaporation rate for Pyramid Lake to be 4.02 feet per year per acre of surface area. This evaporation rate can be used in conjunction with the U.S. Geological Survey's surface area-elevation rating table for Pyramid Lake to compute the average volume of water that has evaporated. Thus, all of the components are available for developing a mathematical model to compute changes in the lake surface elevation as a function of inflow, evaporation, and overflow.

A tree-ring generated synthetic inflow to the lake can be developed from correlations between tree growth and annual runoff. A regression analysis of the tree-ring widths and runoff volumes gives an equation which allows the tree-rings to be used in the model to generate statistically likely inflows.

This model can be used to reconstruct the fluctuations of Pyramid Lake surface elevation to an accuracy that is largely determined by the statistical correlation between annual growth-ring widths and precipitation.

4.0 DENDROCHRONOLOGY

6

The methodology used for the interpretation of treerings in drought sensitive trees from the Southwest and their conversion into a time series chronology, as well as a growth chronology, has been presented by Schulman (1945b) and others. A short summary of the process is given here.

Most of the growth of the cambrian layer, the wood just under the bark, usually takes place in May, June, and July in the Southwestern United States (Schulman, 1945a, p. 63). The three month period produces the light colored part of a tree-ring or spring wood and is characterized by large, but thin walled cells. The dark colored part of the ring or summer wood has smaller, thicker walled cells (Stallings, 1960, p. 4). The spring wood is the period of rapid growth of the tree and is usually the major part of the ring.

The width of a tree-ring is controlled by the local environment: the length of the growing season, as determined by the temperature; nutrients in the soil; hours of sunlight; and availability of moisture. If one or more of the environmental factors is reduced below normal levels the width of a ring will be less than optimum for that year. The growing season stops when an environmental factor falls below its critical lower limit.

In the forests at the highest latitudes and at the

upper timberline, temperature has been found to be the critical factor for tree growth. In this environment moisture is usually in abundant supply from the deep snow packs, but the season is short and cold.

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In middle zones, Schulman has found the chronologies (width of the tree-rings versus time) tend to have similar size rings, because only rarely will the supply of any one component become short enough to slow growth and it may not be the same factor from year to year.

Only near the lower limit of the forest can a strong correlation of tree-ring growth to precipitation (or streamflow) be found. The growth of the drought-sensitive trees will not show a perfect correlation because: (1) the growth in any particular year could be controlled by the other environmental factors (for instance, a deep snow pack would provide a good water supply but it might be too cold to allow full growth); (2) a difference in the precipitation that fell at the metorological stations and at the trees due to distance between sites, elevation, exposure, slope, etc.; (3) the carryover effects of excess or deficient soil moisture from preceeding years; (4) a difference in the months when precipitation occurred and the growth of the trees took place (for instance, late summer rains affect the growth of a tree as a soil moisture residual carried over for the next year's growth); and (5) the incomplete elimination from the ring chronologies of local effects. such as release from suppression by other trees, insect

injury, erosion, deposition of soil, fire, lightning injuries, etc. (Schulman, 1945a, p. 30).

In addition to studying trees at the dry, lower boundary of the forest, Schulman found by trial and error that drought sensitivity could be enhanced by choosing trees that: 1) were long-lived conifers of non-erratic growth characteristics; and 2) grew on steep slopes underlain by pervious rock and soil so that moisture conservation would not affect the next year's growth (Schulman, 1945a, pp. 62, 64).

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Two erratic growth characteristics of many trees are locally absent rings and false rings. A locally absent ring is caused by an environmental stress so severe that a patch of the cambrian layer within the tree does not grow at all that year. If the core sample, from which the chronology is made, happens to pass through one of these zones there is a missing ring. These can be located only with other chronologies and matching growth patterns. A false ring is a one season growth that goes from early wood to late wood and then repeats itself. These are most easily identified by crossdating and can be recognized in the wood by the fact that the first late wood grades back into the next early wood without the usual sharp break. The false ring is caused by a slowing of growth early in the season, then a reactivation of rapid growth.

The trees in the present study as well as most of the studies cited here were sampled using a Swedish Increment Borer which only extracts a small core from each tree. The trees were bored on a side perpendicular to the downhill direction of the slope to eliminate the distortion a tree undergoes as it bends and grows upright. The cores were dried, then glued to a slotted lath to give the fragile core support. The cores were then sanded to a flat, smooth finish on the exposed face. This facilitates accurate measuring of the rings.

In order to use the tree-ring series for dating and reconstruction purposes a chronology must first be built. First, each core must be crossdated by accurately checking each one against the others for false or missing rings. To facilitate this, a skeleton plot is first made. The plots are made from long strips of graph paper with the horizontal scale representing time in years and the vertical scale reflecting the tree-ring thickness. It has been found that the narrower ring patterns can be more easily recognized than the average, or thick rings. To facilitate the recognition of the patterns on the skeleton plot a thick vertical line is marked on the skeleton plot for each narrow ring such that, subjectively, the narrower the ring the longer the line. Average and overaverage rings are ignored except to mark a "B" (by convention) for those rings that are unusually thick. This process eliminates the tree-ring thickness from the horizontal scale and allows two plots to be slid past one another to match up the thickness patterns on the vertical scale year by year.

When matches are formed in some sections, but are displaced a year or more in other sections, false and missing rings are looked for inbetween. When a match can be made down the entire length of all plots, allowing for recognized false and missing rings, all plots may be dated starting from the known date of the outer ring under the bark.

As a tree grows, the mean tree-ring width becomes smaller, which means the early growth of a tree cannot be directly compared to its later growth. In order to use the tree-rings for reconstruction purposes this individual age trend must be eliminated. This trend is removed by fitting an exponential equation of the type

 $Y_{+} = ae^{-bt} + k$ (1) where

Yt is the observed ring width

t is time in years

a,b,k are positive constants determined by a least squares fit of the measured ring widths (Fritts, Mosimann, and Bottoroff, 1968).

Indicies of growth can be calculated using an equation of the form

 $I_{t} = (Y_{t} / \bar{Y}_{t}) 100$ (2)

where

Y_t and t are as in equation 1 It equals the calculated index \bar{Y}_+ is the expected mean growth as predicted by the equation 1 at time, t.

Thus, each tree-ring can be converted to a percent of the mean growth such that anything between 0 and 100 is less than mean growth, and anything above 100 exceeds the mean growth.

An average chronology can be established for a given type of tree or locality by averaging all of the tree-ring indices of growth from all of the trees within the category to provide a single master time series plot or chronology. This chronology is the most useful for correlation studies because it tends to average out any unusual growth patterns of a single tree.

In the present study twenty-four trees were sampled by Professor Alfred Cunningham and Dr. Richard Bateman of the Water Resources Center between 1972 and 1973. The laboratory of Tree-Ring Research at the University of Arizona analyzed the cores, crossdated the samples, computed the indicies of growth, and developed the following three chronologies by the methods described above.

The trees were sampled in three areas, as shown on Figure 1. The Hirschdale chronology consists of six <u>Pinus ponderosa</u> (yellow pines) from an elevation of 5600 feet near Hirschdale, California above the Truckee River. The Skyline set is made up of seven yellow pines from 4800 feet on the lower slopes of the Carson Range above Skyline Boulevard, Reno, Nevada. The last set is

nine <u>Pinus monophylla</u> (pinyon pines) from an elevation of 4300 feet in the Pine Nut Range east of Gardnerville, Nevada.

to uncert are: 1) the precipitation which falls on the soil must satisfy any soil mailture deficit before substantial cumpt can occur, and 2) the growth of the treas responds to the availability of poil moisture mare

the drivation and hydrolegic factors that effect month and tens growth are precipitation, unsponsition, and changes in this emisture. Precipitation is chylonishy the chalsent factor is controlling the solute of remoti. Therefore, when a solil emission states is way interest there will be tailer functif nulses the states is way interes. Under the conditions of a cold minister derivation of precipitation is there, she particularly if now states of the india on the solar of particularly if now states in ander on the solar and follows endored out only the factor the india on the solar and follows endored out only the month of the water are indianal for periods of any water after it indicates on the solar are indianal for periods of any water after it indicates of the water are indianal for periods of any water after it indicates of the solar are indicated and the model rates are also indicated of high chooses the star upon the built college. This is into all increase the star upon the

5.0 DENDROHYDROLOGY

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Stockton (1971, p.89-93) has expressed the theory of the relationship of tree-ring growth and runoff to dimate in some detail. The following is a summary of his discussion.

The underlying assumptions in relating tree-ring growth to runoff are: 1) the precipitation which falls on the soil must satisfy any soil moisture deficit before substantial runoff can occur, and 2) the growth of the trees responds to the availability of soil moisture more closely than any other environmental factor.

The climatic and hydrologic factors that affect runoff and tree growth are precipitation, evaporation, and changes in soil moisture. Precipitation is obviously the dominant factor in controlling the volume of runoff. However, when a soil moisture storage deficit exists there will be little runoff unless the storm is very intense. Under the conditions of a soil moisture deficit the growth of the trees will similarly be retarded. The form of precipitation is important particularly if snow accumulates to great depth. Evaporation of any water after it has landed on the ground and foliage reduces not only the runoff but the water available for infiltration. Furthermore, during periods of high evaporation, the accelerated rate of transpiration will increase the draw upon the soil moisture. This in turn will increase infiltration as well as reduce ground water outflow.

It is apparent that both the tree growth and the runoff are similarly affected by the precipitation which represents a positive component, and evaporation and transpiration representing negative components. The soil moisture storage is usually considered to be negligible when water years are used (Stockton, 1971).

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6.0 PREVIOUS INVESTIGATIONS

The first attempt to determine a relationship between streamflow and tree-rings is thought to have been done by Kapteyn in the year 1880 (Kapteyn, 1914, cited by Stockton, 1971). Hardman and Reil (1936) were the first to try to extend the hydrologic record based on tree-rings. They collected their samples from the Truckee River Basin. Hardman and Reil recognized that there were probably carryover effects on runoff from the preceeding years precipitation. This would include retarding effects on the ground water component of flow. They smoothed the data with a five year moving average in order to integrate the effects of the preceeding and following years runoff (Table 1). The authors were primarily interested in trends, so they placed the smoothed value in the middle of the group.

Hardman and Reil (1936) ran correlations of tree-rings with six of the major rivers in the area (Table 1). Interestingly, the Feather and American Rivers have the same level of correlation as the Truckee River although they lie on opposite flanks of the mountains. Presumably this is caused by the head waters lying on either side of a common ridge. Hardman and Reil (1936, p. 24) found a poor visual correlation of the water year precipitation from six stations on the west side of the Sierra Nevadas with the combined indices of forty-six trees on the east side. This is

Tree Index*	Correlated . With	Remarks	# of Trees	Done by	Time Base	# of Years	r un- smoothed	r smoothed
T.R.B.	Truckee River	Same Basin	46	Hardman & Reil, 1936	Oct-Sept.	26	-	0.895
"	Feather River	Trees on east side of Sierra Nevada Mtns. and the	46		Oct-Sept.	25	- 1	0.895
п	American River	Rivers on west side	46	11 11	Oct-Sept.	22	-	0.895
	Mokelumne River		46	11 11	Oct-Sept.	22		0.825
	Tuolumne River		46		Oct-Sept.	31	-	0.735
	Yuba River		46	11 11	Oct-Sept.	24	-	0.795
	Bear River		46	11 11	Oct-Sept.	22	- 1	0.545
E.O.	Columbia River		340	Keen, 1937	Oct-Sept.	57	0.56	-
M.V.	Animas River		?	Schulman, 1945a	Oct-Sept.	47	0.73	-9/1
S.J.	Kings River	THE P P R	6		Oct-Sept.	47	0.59	
11	San Gabriel R.	a stand of	6		Oct-Sept.	46	0.57	- ,
C.R.B.	Lees Ferry Flow	Comp.of tree indices	109	Schulman, 1945b	Oct-Sept.	94	0.66	0.814
B.S.	Kings River	From 6 sub-basins	60	Schulman, 1947	July-June	94	0.52	0.621
11	н		60	11	July-June	94	0.52	0.64
	San Gabriel R.		60	11	July-June	49	0.61	0.811
11	11 11	8 8 A	60	11	July-June	49	0.61	0.792
н	1	10 P 2 P 1	60	u e	July-June	49	0.61	0.873
11	11 11	6	60	11	July-June	49	0.61	0.864

TABLE	1	

Runoff Correlations, Past Studies

1)b' = (a+b)/2 2)b' = (a+2b)/3 3)b' = (a+b+c)/3 4)b' = (a+2b+c)/4 5)f' = (d+e+f+g+h)/5

where b' and f' = one years index; a,d,and e = preceding years indices; and c,g,and h = following years indices.

* T.R.B.-Truckee River Basin; E.O.-Eastern Oregon; M.V.-Mesa Verde; S.J.-San Jacinto; C.R.B.-Colorado River Basin; B.S.-Big Cone Spruce

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probably caused by the physiographic location of the precipitation stations, which are well west of the trees and separated from them by the crest of the mountains. The amount of precipitation east of the mountains is substantially less than on the west, because the vast majority of the storms approach from the west-which places the trees in the "rain shadow" of the mountains.

Keen (1937) studied the relationship of precipitation and tree growth in eastern Oregon to determine if the recession of the tree line up the mountains was climatically controlled (Table 2). He examined the tree-ring widths in an attempt to extend the precipitation and streamflow records. From these records he hoped to determine if there had been a shift in the weather pattern. He found the best correlation (0.82) for the historic period by using smoothed precipitation data $(b^{1}=(a+b), b=$ one years data, a=the preceeding year's, and b¹=the smoothed data; Table 1). Keen concluded this indicated a strong influence on one year's growth by the preceeding year's precipitation. The unsmoothed precipitation and tree-ring growth gave a correlation of 0.50. In the case of the correlation of precipitation to streamflow, Keen found a correlation coefficient of 0.56 without smoothing. He did not give a smoothed equivalent.

Schulman (1945a, 1945b, 1947, and 1951), tried to determine the degree of correlation that could be expected between streamflow, precipitation, and tree-rings for

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Precipitation Correlations, Past Studies

			1	1	1		r	r
Tree	Correlated	Demenia	∦ of	Dere her	Time Dee	# of	unsmoothed	
Index*	With	Remarks	Trees	Done by	Time Base	Years	or quality	smoothed
Truckee R. Basin	6 Western Sierra Stations	Trees on east side	46	Hardman & Reil,1936	Oct-Sept	64	poor	-
Eastern Oregon	Eastern Oregon		340	Keen,1937	Oct-Sept	66	0.50	0.821
Mesa Verde	Durango	(Arizona)	?	Schulman, 1945a	Oct-June	48	0.78	12
Okanogan	Okanogan	(Wash.)	26	U	Oct-June	32	0.57	-
So.Calif	San Diego		6		July-June	91	0.44	-
Bigcone Spruce	S. Coast Rainfall		60	Schulman, 1947	July-June	94	0.65	0.822
Bigcone Spruce	S. Coast Rainfall		60	"	July-June	94	0.65	0.85 ³
Fox Mtn.	Jewett		10	Stockton,	Oct-Sept	38		0.89
Alpine	Jewett		20	1971	Oct-Sept	38	1 3	0.82
Luna	Jewett		10	"	Oct-Sept	38	-	0.89
Tularosa Divide	Jewett		10	"	Oct-Sept	38	-	0.95
Rainy Mesa	Reserve		10		Oct-Sept	38	-	0.85

c = the following years index; b' = the smoothed index

several areas of the West (Tables 1 and 2). His 1945a paper was a review of the available literature and a reconnaissance level sampling of trees from various areas to determine if a significant correlation between precipitation and runoff could be established. In general, he found significant visual correlations, but he did not give correlation coefficients as he did in later papers.

Schulman's Colorado River basin study (1945b) used sets of trees from many sub-basins to establish correlations with streamflow for each sub-basin. Each sub-basin was then weighted and integrated into one chronology for the entire basin. The reconstructed streamflow from this chronology was then checked against the streamflow at Lee's Ferry.

In southern California, in a similar investigation, Schulman (1947) sampled bigcone spruce trees in the coast ranges. He correlated the spruce's growth to flows of the San Gabriel River from the San Gabriel Mountains and also for the King's River in the southern Sierra Nevadas. He used precipitation stations from over the entire area but most were not in the mountains (Table 2). The correlations (Table 1) to the King's River are lower than those for the San Gabriel Mountains. This is understandable, because the trees are located within the San Gabriel Mountains and quite removed from the King River. Schulman also found that the correlation of the water year (October-September) precipitation to bigcone spruce, r=0.85, is best with a

three year moving average (b' = (a+2b+c)/4; b= one year's index, a= the previous years, c= the following years, and b'= the smoothed index). A two year average (b'= (a+b)/2) gave only a slightly lower correlation coefficient of r=0.82 and the unsmoothed correlation was lower still at r= 0.71 (Table 2). Schulman believes this to indicate that there is a cumulative effect of precipitation on growth that lasts for more than one season. The correlation coefficients of tree-rings to streamflow are r= 0.86, and 0.81 for the respective smoothing schemes and 0.61 for the unsmoothed data (See Table 1).

In a paper in 1951, Schulman points out that rivers which are subject to flash floods, such as the Gila River in Arizona, have a much lower correlation between treerings and runoff than do the rivers that are dominated by springtime snowmelt runoff.

Stockton (1971, p.8) reports that Potts (1962) in an unpublished paper attempted

to use the time distribution of tree-ring series to improve the estimates of drought recurrences in the upper South Platte River Basin in Colorado. As Water Rights Engineer for the Denver Water Department, he hoped to predict or at least improve the advance estimate of runoff of the South Platte River, which provides the water supply for the city of Denver. His objective was to estimate the storage required to provide a firm water supply for the city. Consequently, the ring-width series were used subjectively in determination of reservoir storage requirements.

Gatewood, Wilson, Thomas, and Kester (1964) used Schulman's published data to test whether a fifty year

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base period (1904-1953) was representative of the runoff in the Southwest for the last 154 years. They concluded that the mean runoff in the 1904-1953 period closely approximated the mean runoff of the last 154 years, 1800-1953.

Stockton (1971) collected tree-ring samples from the Bright Angel Creek watershed on the Colorado River in Arizona and the Upper San Francisco River watershed on the Gila River near the Arizona-New Mexico border. These data, along with the records of precipitation, temperature, and runoff from nearby stations were analyzed statistically by correlation analysis to test whether one value depends strongly on the preceeding value, and spectral analysis was used to identify the frequencies of the various cycles in the data. In addition, he used cross spectral analysis to test the correlation between the two series at each frequency defined above, and analysis of variance of components to give the percent of the total variance that can be ascribed to four sources of variance in the treering width data. Principle component analysis was used to transform the data into orthogonal eigenvectors for use in analyzing the significance of each component used in the multiple linear regression which gives the reconstruction equation.

Fritts, Smith, and Stokes (1965) have shown by use of a physiological model of trees that the most probable climatic season affecting growth is a fourteen-month period

from the June preceeding the growth of an annual tree-ring through the July of the year that the growth takes place (Stockton, 1971, p. 96). Stockton (1971, p. 109) used this period of time for his correlations of precipitation and temperature to growth and found that it gave only a ten percent improvement over a water year (October-September) time base. He concluded that this improvement did not justify the work involved in making the additional computations (Stockton, 1971, p. 109). The correlations of runoff and precipitation with tree-rings in his study are shown in Tables 1 and 2. He has used much more sophisticated statistical methods to improve his correlation, which tends to increase his r values relative to the techniques used by the other workers.

Stockton concludes from his study that dendrohydrology holds great promise, but that it is dependent on carefully selected trees, growing in basins that cause sufficient environmental stress to reflect the hydrologic conditions. Furthermore, each basin must be studied separately, with the equations from one basin having no applicability in any other.

A recent paper on dendrohydrology is by Stockton and Fritts (1973). They attempt to correlate tree-ring width data from six sites along the natural levees of the river channels in the Lake Athabasca Delta of Alberta, Canada, with the 1935-1967 observed lake levels. They were attempting to see if the closure of the W.C.A. Bennett

Dam on one of the main tributaries had significantly affected Lake Athabasca. They found a negative correlation of tree growth to lake levels, which they ascribed to higher river stages when the lake was high. This caused saturation of the root systems of the trees which slowed growth. They worked with three time periods for each year, May 21-30, July 11-20, and September 21-30 corresponding to the times when the lake level was rising, full, and falling. They concluded that the drop in lake levels since closure of the dam upstream has been caused in part by climatic fluctuation, but also by the impoundment of the water at the dam.

7.0 PRESENT INVESTIGATIONS

Due to the yearly growth cycle of the trees, an October through the following September water year is considered by Stockton (1971) to be the most satisfactory time base for dendrohydrology. Thus, the maximum resolution of any tree-ring reconstructed streamflow event will be one year. All precipitation and streamflow records in this paper are in water years with the water year dated in the January-September period.

7.1 INDICES OF PRECIPITATION AND RUNOFF

For use in this paper all precipitation records and streamflow records have been converted to indices by using Equation 2

 $I_{t} = (Y_{t} / \overline{Y}_{t}) 100$

where

- t equals the water year (October through the following September)
- It equals the index of precipitation or flow for year t, dimensionless
- Y_t equals the depth of precipitation or volume of flow at year t, vertical feet per acre per year or acre feet per year

 \overline{Y} equals the mean of the data, acre feet per year.

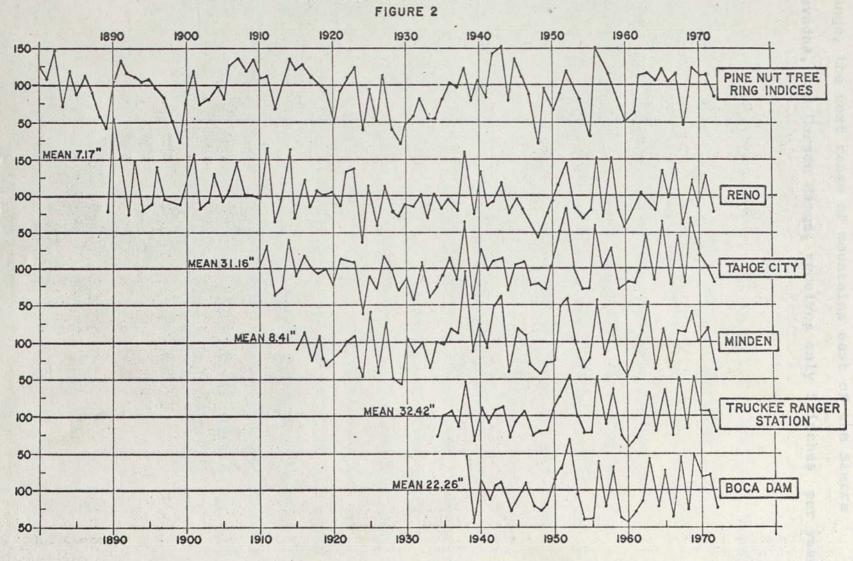
Using the data in this form facilitates interpretation of plots of the data, because all curves are plotted to the same scale. Furthermore, by having both the tree-rings and flows as indices it simplifies the reconstruction equation developed from the linear regression.

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7.2 PRECIPITATION RECORDS

Five relatively long term precipitation stations in proximity to the tree sites, are shown in Figure 1. Their records, shown in Figure 2, were chosen because they represent the range of conditions in the vicinity of the trees. The average precipitation decreases very rapidly as one goes from 2.70 feet at the Truckee Ranger Station to 0.64 feet at Reno, a distance of 24 miles with an elevation loss of only 1329 feet (Figure 1 and Plate 1). This dramatic drop in precipitation is caused by the drying of the air masses as they move from west to east across the Sierra Nevadas. This "rain shadow" is caused by the cooling of the air masses as they rise up the west flank of the mountains. The cooling of the air brings it to saturation and results in precipitation. When the air mass decends the eastern slope, the air is warmed by compression, which results in a loss of saturation and a reduction in the amount of precipitation which falls on the east side of the mountains. The effect of the "rain shadow" is to have high precipitation near the crest of the ridge, i.e. Truckee Ranger Station, with less occurring the further away one is from the crest of the ridge on the leeward side, i.e. towards Reno. There is approximately 15 inches of precipitation at the Hirschdale tree site as compared to the 10 inches at the Skyline site.



PRECIPITATION INDICES AND PINE NUT TREE INDICES

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The Pine Nuts site, which is located on the Pine Nut Range, the next range of mountains east of the Sierra Nevadas, and Carson Range, receives only 5 inches per year.

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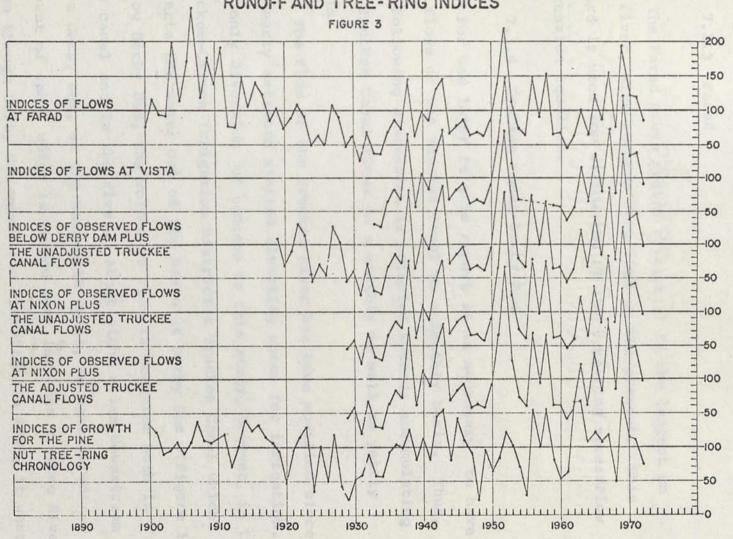
7.3 RUNOFF RECORDS

7.3.1 Nixon

The observed flows at Nixon, Nevada on the Truckee River have been recorded by the Federal Water Master, Reno, Nevada office, between October 1928 and December 1957 (Appendix B). These records are on file in the Water Master's office, Reno, Nevada. The 1955 readings are not complete, therefore, flows below Derby Dam were used for that year. From October, 1957, the U.S. Geological Survey has operated a gauge about two miles above the old site. Data from the new site have been adjusted back to the old site by subtracting the recorded monthly flows in the Bureau of Indian Affairs-Nixon Ditch, which lies in between. The records of these flows are also in the Water Master's office. The diversion records are not accurate, but this does not cause a serious problem. While these flows represent a substantial proportion of the water in the river in low flow summer months, the summer flows are usually a small portion of the yearly flow. Figure 3, which shows a plot of the Nixon plus Truckee Canal flows, approximates the streamflow record if the Truckee Canal had not been built.

7.3.2 Vista

Streamflow records at Vista have been used for determining the average monthly hydrograph in the Pyramid



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RUNOFF AND TREE-RING INDICES

Lake model. This record runs from January 1933 to December 1955 and from October 1958 to the present (Figure 3). The missing data were not recorded.

7.3.3 Farad

The Farad gauge record (Figure 3) is the longest on the river, with records from 1900 to the present. This record is used for validation of the tree-ring streamflow regression equation.

7.3.4 Truckee Canal-Derby Dam

For use later in this report it is necessary to have the flows of the Truckee Canal on a monthly basis. Thus, the following discussion is oriented towards accumulating the Truckee Canal flows on a monthly as well as yearly basis.

The flow of the Truckee River has been modified since the early settlers started diverting water for irrigation. The only diversion of concern to this study, however, is the Truckee-Carson Irrigation District's Truckee Canal which diverts the water out of the basin at Derby Dam (Figure 1). Below Derby Dam, the volume of flow is measured both in the canal and in the river. About eight miles downstream from Derby Dam, at the Wadsworth spill, an unmeasured amount of canal water is dumped back into the Truckee River. Even though Derby Dam went into operation in 1903, it wasn't until January 1916 that flow records were kept for the Truckee Canal and the Truckee River below Derby Dam. The calendar year 1917 for the Truckee River was not recorded. The canal record has three obvious errors in June, July, and August, 1917. In these three months the reported flow exceeds the estimated capacity of the canal by two to three times. For these three cases the appropriate average monthly flow of the canal is computed from all the records less these three months. In 1966 the USGS installed a new gauge on the canal downstream from the Wadsworth spill. and in 1969 the old head gauge was discontinued. The new gauge has been used in this study for Truckee Canal flows since 1966, because it is a more accurate measure of the volume of exported water. The head gauge records shown in Appendix C are on file at the Truckee-Carson Irrigation District offices in Fallon, Nevada.

There is a problem in using the Truckee Canal data because the quantity of water dumped back into the river is unknown. This leads to double counting of the dumped water when the Nixon flows are added to the Truckee Canal flows. An attempt has been made to correct for the double counting of the dumped water by subtracting the flows at the Truckee River below Derby Dam gauge from the flows at the Nixon gauge for each month that both records exist. Each of the positive values represents a gain to the river in the reach and is assumed to be caused by the dumping of the Truckee Canal waters at the Wadsworth spill. Each of these gains in this reach of the river has been subtracted from

the measured flow of the Truckee Canal and is hereafter called the adjusted Truckee Canal flows. Since the smaller losses in the reach of the river can be attributed to consumptive use, they were not added back in.

Truckee River flows at Farad, Vista, below Derby Dam, and at Nixon from 1956 to present, as well as at the new gauge on the Truckee Canal have been published in the USGS Water Supply Papers 1314 and 1734 and in the USGS Stream Flow Records for the State of Nevada.

7.3.5 River Flow Data Requirements

In order to use the flow records at Nixon or Derby Dam in a regression analysis, all of the flow data must be in a form that eliminates the export of water through the Truckee Canal. This is accomplished by adding the yearly unadjusted Truckee Canal flows to the yearly flows at Nixon and Derby Dam. However, adjusted Truckee Canal flows can only be added to Nixon flows. These flows, and the indices of these flows, are called Nixon plus unadjusted Truckee Canal, Derby plus unadjusted Truckee Canal, and Nixon plus adjusted Truckee Canal flows, respectively. In this form these three records reflect the total flow of the river for each water year just as the Vista and Farad gauge records do.

Figure 3 is a plot of all five sets of flow indices plotted along with the Pine Nuts chronology. These five flow patterns are nearly identical. The only significant difference is that high flows tend to increase down river. This is expected, because there would be substantial runoff from the normally dry tributaries to the Truckee River below Reno during extremely wet years.

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7.4 TREE-RING INDICES, PRECIPITATION, AND RUNOFF

Correlation coefficients obtained for comparisons of the tree rings to precipitation and runoff at various locations are shown in Tables 3 and 4. These two tables show clearly that the precipitation and runoff correlate best with the Pine Nuts chronology and least with the Hirschdale chronology. It would seem, at first glance, that the Hirschdale and Skyline chronologies correlate better and more consistently with runoff than they do with precipitation. The numbers are misleading because orographic effects cause substantial variation between the five precipitation stations. On the other hand, the four stream gauges show almost no variation because the bulk of the runoff originates upstream from the highest gauge.

The differences in the correlation coefficient of streamflow and the three tree-ring chronologies reflects how much better the Pine Nuts chronology is for reconstructing streamflow and precipitation, than the other two chronologies. The poor correlation of the Hirschdale chronology would seem to result from the location of the trees in the interior of the Carson Range where the amount of precipitation, 15 inches, is more nearly sufficient to prevent any severe water stress from affecting the trees. The Skyline trees lie on the eastern flank of the Carson Range where the rain shadow effect reduces the precipitation to 10 inches and puts the Skyline trees under a somewhat

TABLE 3

Precipitation Correlations, Present Study Time Base; October-September, Water Year

Tree index	Correlated with	Number of trees	Number of events	r unsmoothed
Hirschdale	Reno	6	84	0.26
н	Tahoe City	6	63	0.45
11	Minden	6	59	0.22
н	Truckee R.S.	6	39	0.31
n	Boca	6	35	0.23
Skyline	Reno	7	84	0.31
u	Tahoe City	7	63	0.45
11	Minden	7	59	0.47
11	Truckee R.S.	7	39	0.39
17	Boca	7	35	0.42
Pine Nuts	Reno	9	84	0.60
and the second	Tahoe City	9	63	0.68
	Minden	9	59	0.72
U	Truckee R.S.	9	39	0.60
	Boca	9	35	0.66

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TABLE 4

Runoff Correlations, Present Study

Time Base; October-September, Water Year

Tree Index	Correlated with	Number of trees	Number of events	r unsmoothed			
Hirschdale	Nixon + UTCan $^{1}_{2}$	6	44	0.49			
IIIISCINALC II	Nixon + $ATCan^2$	6	44	0.49			
11	Derby + UTCan	6	54	0.48			
	Vista	6	40	0.45			
	Farad	6	73	0.47			
Skyline	Nixon + UTCan	7	44	0.57			
	Nixon + ATCan	7	44	0.57			
м	Derby + UTCan	7	54	0.58			
IJ	Vista	7	40	0.55			
U	Farad	7	• 73	0.56			
Pine Nuts	Nixon + UTCan	9	44	0.65			
N	Nixon + ATCan	9	44	0.65			
U	Derby + UTCan	9	54	0.66			
N	Vista	9	40	0.63			
	Farad	9	73	0.64			

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²Nixon plus adjusted Truckee Canal

more severe water stress. The Pine Nut trees lie the farthest east and consequently, receive the least precipitation, 5 inches, of the three sets. The strikingly better correlations of the Pine Nuts chronology with both streamflow and precipitation dictate that only the Pine Nuts chronology will be used in the regression of treerings to streamflow, described next. Inclusion of the other two chronologies would only increase the random error component of the reconstruction equation.

Table 4 indicates that at the accuracy justified by the tree-ring data, there is no difference between the correlations of the tree-ring chronologies to the Nixon plus unadjusted Truckee Canal or Nixon plus adjusted Truckee Canal flows. Since the adjusted data represent the extreme case (subtracting all gains in the river from Derby Dam to Nixon) no further refinement of the adjusted data is warranted, and the unadjusted Truckee Canal data will be used to develop the reconstructive equation.

It is convenient to note at this point that the correlation coefficients between streamflow and treerings would probably be better if it were not for a persistent error during high streamflow years. During very wet years the trees tend to have near average growth due, presumably, to the excess water "drowning" the trees (although other environmental limits, such as temperature, may also come into play). The most obvious case of this occurred in 1890 when the precipitation at Reno was 220

percent of normal, while the Pine Nut tree growth was only 100 percent (Figure 2). If the tree-ring indices could be doubled to match the precipitation during that year, the error would be reduced.

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7.5 SIMPLE LINEAR REGRESSION

In order to use tree-ring indices to reconstruct the flow of the Truckee River at Nixon, a linear regression was used on the forty-four years of Nixon plus unadjusted Truckee Canal flow indices (dependent variable) against the tree-ring indices (independent variable) for the same period. Since only one tree-ring variable was used in this study the principle component analysis procedure suggested by Stockton (1971) was not considered necessary. The regression equation is

 $I_t = 10.6296 + PNT_t (0.9255)$ (4) where

I, equals the synthetic flow index for the year t

PNT_t equals the Pine Nut tree-ring index for year t To convert the synthetic index into a reconstructed flow in acre feet per year, the index must be multiplied by the mean of the Nixon plus unadjusted Truckee Canal flows, 506,570 acre feet per year.

Checking of the tree-ring streamflow equation was accomplished by developing the regression equation over the forty-four years of data at Nixon and testing it against the seventy-three years of data at Farad. There is almost a 1:1 correlation between the historic flows at these stations.

A regression using all forty-four years (1929-1972) of the observed Nixon plus unadjusted Truckee Canal flows against the Pine Nut chronology yields a correlation coefficient of r=0.72. The correlation coefficient between the reconstructed Nixon plus unadjusted Truckee Canal flows and the Farad flows for the period of 1900-1972 is 0.70. This indicates that the reconstructed flows over the longer period have similar variance to the calibration period and thus, the regression equation is valid for the longer period.

It must be noted that due to climatic cycles of short to moderate length, the data used in the regression must be representative of all of the cycles. In this case it was found that a regression developed from the 1951-1972 Nixon plus unadjusted Truckee Canal data gave a much different set of regression coefficients than the regression developed from all forty-four years of data. However, if a regression was developed from twenty-two years of data selected by using every other year from the forty-four, the coefficients were nearly the same as those from all forty-four years of data. Thus, one must be aware that it is possible, in fact likely, that the historic period is not fully representative of the long term climatic cycles. The 450 years of tree-ring data (Appendix A) used in this study shows that the historic period is similar to the 1745 to 1904 period, and therefore, the assumption that the historic period is representative of that time interval is reasonable.

7.6 PYRAMID LAKE MODEL

The Truckee River empties into the closed basin of Pyramid Lake. The only other inflow to the lake is from small ungauged ephemeral streams and ground water inflow from the surrounding mountains. Both are small compared to the annual flow of the Truckee River. The three modes of water loss from Pyramid Lake are 1) as overflow of Pyramid Lake into Winnemucca Lake through Winnemucca Slough when Pyramid Lake reaches an elevation of 3863 feet or greater; 2) division of the Truckee River on its delta after gauging, but before it enters the lake, sending all or part of its flow to Winnemucca Lake, usually for short periods of time; 3) evaporation directly from the lake which Harding (1962) has computed to be 4.02 vertical feet per acre. Evaporation is by far the most important factor.

Ignoring the fact that division of the Truckee River on its delta takes place (there is no way to include this unrecorded, random phenomanon), Pyramid Lake is for all practical purposes a non-leaky closed basin for lake surface elevations below 3863 feet. This simplifies any outflow equations and allows Harding's evaporation rates to be used in conjunction with volume versus elevation, and surface area versus elevation rating tables to develop an elevation model based on inflow to the lake.

7.6.1 Pyramid Lake Levels

Measured elevations of Pyramid Lake from 1869 -1960 have been published in the U.S. Geological Survey's Water Supply Paper summaries 1314 and 1734. Since 1960 they have appeared in the USGS, Water Supplies for the State of Nevada. All of the published elevations prior to October, 1955 are referenced to the 1929 datum for benchmark N-21 at elevation 3940.04 feet. The supplemental adjustment of 1956 places the elevation at 3940.29 feet to which all published lake elevations since October, 1955 have been adjusted +0.25 feet to the 1956 datum.

Timing of observations of the level of the lake has been rather sporadic. To bring these random observations to the monthly time base of the Pyramid Lake model, the observed elevations were plotted on year by day graph paper and a smooth curve drawn through the points. The value for middle of the month on this curve was used as the observed elevation (Appendix D). When the time between observations was longer than two or three months, interpolation was not attempted. Prior to 1926 the observations were spotty, and only in those cases where two or more observations were relatively close together was interpolation attempted. For many of these early observations the day of the month was not recorded, and in six cases, the month of the year was not even noted. In both situations, the observation was placed in the middle of its period. This should be kept

in mind when examining the errors in the lake model. Interpolated elevations, as well as those placed in the middle of their time period, are listed in Appendix D.

The smoothed lake elevation curves yielded some interesting information on how fast the lake can rise. For instance, in 1969 the lake rose 8.00 feet in five months for an increase in volume of 870,000 acre feet. Only during the times when the lake's elevation rises rapidly does the interpolation of the lake elevations become important. In this case there would have been an 0.85 foot error in the elevation assigned to each of these five months without the interpolations. The recession of the lake through summer and fall plots as a very uniform event. The rate of fall of the lake's elevation is nearly always the same, indicating that summer and fall Truckee River inflows, as well as evaporation rates, are nearly always the same.

A review of the 1844-1960 lake elevations, which includes some reconstructed lake elevations from old records, has been published by S.T. Harding (1965). It is assumed here that all of Harding's elevations are relative to the 1929 datum and are adjusted accordingly. A brief review of Harding's discussion of the observations should give a feel for the quality of each.

In January of 1844, John C. Fremont traveled down the east side of Pyramid Lake. He noted in his diary that "by marks of the waterline along the shores, the spring level is about 12 feet above the present water". The waterline he referred to is the white line which has been caused by recent tuffa deposits and/or mechanical scouring of the older tuffa deposits when the lake stood at those elevations. According to Harding this places the lake elevation at 3860.8 feet. Fremont's report contains a high quality sketch of Pyramid Island and the adjacent inshore rocks. Harding (1965) took the sketch to the island and identified the waterline on the rocks. By surveying this waterline on the rocks he determined, independently, the elevation to have been 3860.8 feet.

The Surveyor General of California in 1856, described his trip down the Truckee River to Pyramid Lake. Harding interprets his description of the division of the Truckee on its delta, with flow into both lakes to mean that Pyramid Lake was at 3860 feet in 1856.

In 1867 and 1871 the King survey (1878) passed by Pyramid Lake. On one of these two occasion a photograph was taken of Pyramid Island and the inshore rocks. Harding as well as Hardman and Venstrom (1941) had determined the elevation of the lake in the photograph to be 3876 feet. The King report states that the lake rose nine feet between 1867 and 1871. Based on other indirect evidence Harding, as well as Hardman and Venstrom, have concluded that the picture was taken in 1871, making the 1867 elevation 3867 feet, and the 1871 elevation, 3876 feet. The USGS Water Supply Paper 1314 lists these elevations at some nine feet higher

on the basis of the photo being taken in 1869. The USGS recently has published a revision stating that the lake could have been at the lower elevation.

Russell (1885) extensively surveyed the lake and took several photographs of the rocks. Harding interprets these photos to give an average elevation of 3867.2 feet during the time Russell worked in the area. Harding also notes that Russell reported a specific elevation for September 9, 1882 which, after allowing for the change in datum, becomes 3867.02 feet.

Harding (1965) gives elevations for 1889 and 1890 that were determined from rocks that Sutcliffe, who was an early settler in the area, said were covered by the rising lake from 1889 to 1890. The elevation difference was estimated at 17 feet after the water had again receded. This apparent 17 foot rise in the lake was during the very wet winter of 1889-1890. Harding (1965) also notes that Mud Slough was closed during much of this period by a brush and rock dam maintained by the Indian Service. This would substantially contribute to this very large change in elevation. Thus, in 1889 and 1890, the elevations were 3861 feet and 3878 feet respectively.

The 1890 elevation is substantiated by the U.S. Geological Survey's 1894 Wadsworth topographic map (topography done in 1890) which shows the elevation of Pyramid Lake as 3880 feet. This reduces to 3876 feet when the -3.93 feet is allowed for the change in datum. Since the USGS probably surveyed the lake's elevation, their elevation will be used for the 1890 observation.

The USGS Water Supply Paper 1314 shows an elevation of 3878.2 feet for 1891. There is no explanation of who made this observation by either the USGS or Harding. Harding does attribute the 1904 elevation at 3861 feet to the U.S. Bureau of Reclamation. Presumably, this was a surveyed elevation.

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7.6.2 Harding's Evaporation Rates

Harding (1962) calculated monthly evaporation rates for Pyramid Lake as shown in Table 5. To do this he used the monthly recorded inflow to Pyramid Lake (1928-1960), the precipitation records at Nixon (1928-1953), or Lahontan Dam precipitation records when the Nixon records were not available (1954-1960), as well as rating curves for elevation versus volume and elevation versus surface area, developed by Russell in 1882 (Figure 4). To derive monthly evaporation, Harding first plotted available lake elevations and drew a smoothed curve through the data. From the curve he used the elevation for the first of each month, then calculated evaporation in feet per acre of surface area for each month that records were available during the years 1928 through 1960. He averaged each month's evaporation for all of the years to get monthly means and totaled the means for the yearly average. These means are shown in Table 5, together with the average

	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	YEARLY
Harding's (1962) ⁴ evaporation rates ¹	.47	. 38	.30.	.24	.23	.20	.22	.25	.32	.39	.50	.52	4.02 ²
Average precipitation ⁵ at Nixon, Nevadal	.05	.06	.07	.07	.07	.05	.04	.05	.06	.02	.02	.03	.59 ²
Average net evaporation ¹	.42	.32	.23	.17	.16	.15	.18 .	.20	.26	.37	.48	.49	3.43 ²
Adjustments made in this report ³	01	01	01	.00	.00	.00	.00	.00	.00	01	01	01	06 ²
Average net evaporation used in the Pyramid Lake Model ¹	.41	.31	.22	.17	.16	.15	.18	.20	.26	.36	.47	.48	3.37 ²

TABLE 5 Evaporation and Precipitation Rates

1 Monthly rates are in feet per month

2 Feet per year

3 See Section 7.6.11 - observed Truckee River Flows as Input.

4 For the period 1928-1960.

⁵ For the period 1928-1953 and 1962-1972.

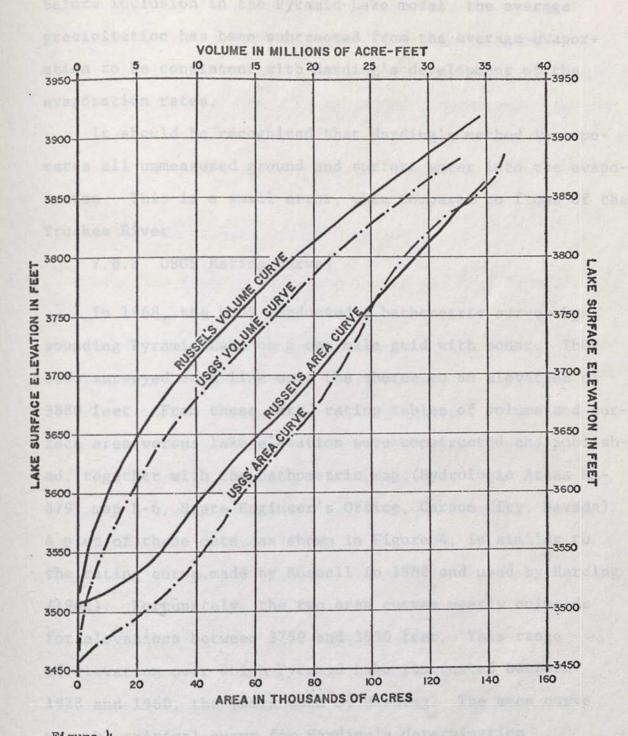


Figure 4 Rating Curves for Pyramid Lake. Russel's (1882) data adapted from Harding (1965). USGS data plotted from Hydrologic Atlas HA-379, Map L-6.

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precipitation for thirty-four years of record at Nixon. Before inclusion in the Pyramid Lake model, the average precipitation has been subtracted from the average evaporation to be consistent with Harding's development of the evaporation rates.

It should be recognized that Harding's method incorporates all unmeasured ground and surface water into the evaporation. This is a small error, when compared to flows of the Truckee River.

7.6.3 USGS Rating Curves

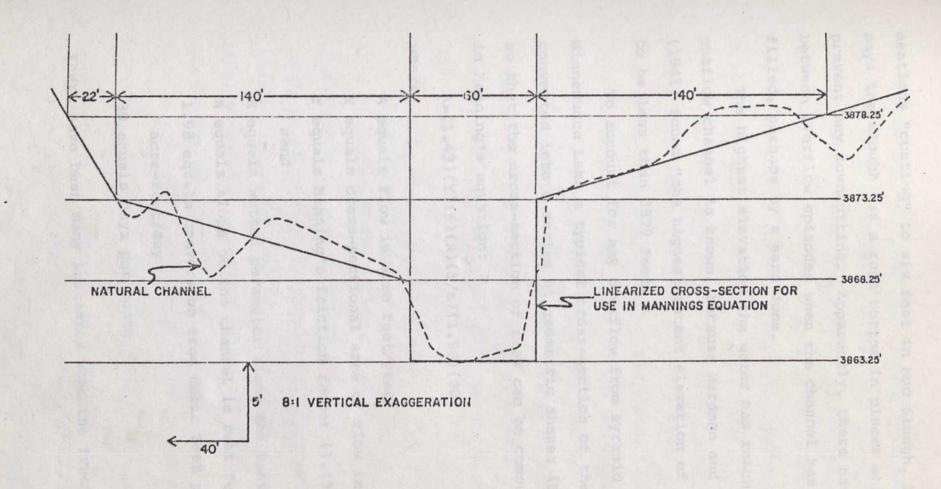
In 1968, the USGS conducted a bathometric survey by sounding Pyramid Lake on a one mile grid with sonar. They also surveyed each line onto the shores to an elevation of 3880 feet. From these data, rating tables of volume and surface area versus lake elevation were constructed and published, together with the bathometric map (Hydrologic Atlas HA-379, map L-6, State Engineer's Office, Carson City, Nevada). A plot of these data, as shown in Figure 4, is similar to the rating curve made by Russell in 1882 and used by Harding (1962). Fortunately, the two area curves nearly coincide for elevations between 3750 and 3850 feet. This range of elevation over which Pyramid Lake fluctuated between 1928 and 1960, the years used by Harding. The area curve was the critical curve for Harding's determination of volume of water evaporated per acre of surface area. If the curves had been as far apart in that elevation zone,

as they are for lower elevations, Harding's evaporation rates would have been considerably less accurate. For the model in this study, a series of fifteen linear equations, joined end to end, were developed to approximate the rating curves. The sixteen segments were chosen so that the shortest segments occurred at the zones of greates curvature and the longest segments, where the curves were nearly straight.

7.6.4 Winnemucca Slough Overflow

Overflow from Pyramid Lake is handled as a function of lake elevation. Hardman and Venstrom (1941) surveyed the ground elevation around Winnemucca Slough and found the elevation of the slough, where it leaves the Truckee River, to be 3863 feet (3863.25, 1956 datum). The slope of the slough was found to be eight feet in 4.5 miles.

A typical cross section of the slough, about two miles down stream of the bifurcation, was measured during this study and is shown in Figure 5. The present channel shows a little natural deterioration since the last reported flow in 1937. The bottom of the essentially rectangular channel is nearly flat and shows no evidence of wind blown siltation, and only minor fluvitile deposition. Thus, the channel has remained essentially clear for the last thirty-eight years. Harding (1965, p. 102) however, notes that the Gibson report to the Indian Service in 1888



A CROSS-SECTION OF WINNEMUCCA SLOUGH

mentions "cutting" to six feet in Mud Slough, but he also says the slough has a rock bottom in places which should prevent any downcutting. Apparently, there have been times between overflow episodes when the channel has been backfilled, perhaps by a sand dune.

The highest elevation the water has reached in the outflow channel is known, because Hardman and Venstrom (1941) found "the highest recent elevation of Pyramid Lake" to be less than 3879 feet.

To account for any overflow from Pyramid Lake to Winnemucca Lake a typical cross-section of the channel was converted into a series of geometric shapes (Figure 5) so that the cross-section of flow can be computed for use in Manning's equation:

A=(1.49)(Y)(f)(R)(1/s)(1.98)(30) (5) where

A equals flow in acre feet/year

- Y equals cross-sectional area of flow in feet²
- f equals Manning's friction factor (1.49) for medium sand
- R equals wetted perameter along the banks in feet
 s equals slope of the channel in feet/feet (8:23760)
 1.98 equals conversion from cubic feet per second to
 acre-feet/day

30 equals days per month

There have been many instances when the Truckee River

divides on its delta, but this cannot be incorporated into the model because the timing and volume of these flows are not known, except for a short period between 1903 and 1905 when a gauge was maintained on the slough.

Hardman and Venstrom (1941) and Harding (1965) note that the Federal Government's Indian Service had a rock and brush dam placed across Winnemucca Slough in 1888 or 1889. This effectively cut off Winnemucca Lake until some time after February, 1891 when the Nevada Legislature complained to Congress. The dam was not maintained after this time and eventually became ineffective. This may have caused higher than normal Pyramid Lake levels for the years of 1890 and 1891, and perhaps several more. For this reason, the model disallows flow through the slough in 1890 and 1891.

7.6.5 Time Base of the Model

Inclusion of the overflow term necessitates building the model to work on a monthly basis in order to limit the error in calculating the Winnemucca Slough overflows. The base period of one year, dictated by the tree-rings, is too long because the volume of water overflowed is a function of the average elevation of the lake over the time period. 7.6.6 Yearly Flows Distributed to Months

Since the time base of the tree-ring flow equation is one year, it is necessary to distribute the yearly tree-ring generated flow of the river over the twelve month period (Table 6). This is done by multiplying the reconstructed yearly flow by each month's average percentage of the annual flow. The month's average percentage of the annual flow was determined from the thirty-six years of flow records at Vista.

The process of breaking the reconstructed yearly flows into monthly flows using the average flow percentages can create errors in some months if the distribution of the flows for a particular year does not match the average hydrograph used in the program. This was found to give only a slight discrepancy between Pyramid Lake model runs using as input the monthly observed flows at Nixon, and runs redistributing the yearly total of the same monthly flows via the average flow percentages.

7.6.7 Truckee Canal Flows

In order to use one regression equation to predict flows before and after 1903 (when the canal went into operation) it is necessary to subtract the monthly observed Truckee Canal flows during the 1903-1972 period. Unfortunately, from 1903-1917 no calculations can be made because Truckee Canal flow was not recorded. Prior to 1903 the reconstructed flows need no adjustment. Thus, for the years 1917-1972, the monthly observed Truckee Canal flows are subtracted from the product of the tree-ring generated Nixon plus unadjusted Truckee Canal flows and the distribution percentages to convert the yearly flows

TABLE 6

Decimal percentages of the yearly flow at Nixon (Nixon plus Truckee Canal). Determined from 44 years of data.

> OCT .0436 NOV .0605 DEC .0768 JAN .0784 FEB .0867 MAR .0924 APR .1494 MAY .1873 JUN .1181 JUL .0407 AUG .0304 SEP .0357

YEARLY TOTAL 1.000

the US66 bating curves rations to a the these services

to the appropriate months.

7.6.8 Seed Values

To make the first calculation of the lake level the model is started on a month with a known lake elevation. This elevation is the "seed value". Prior to 1926, the observed elevations are spotty and often only the month or even the year of the observation is known. This creates a problem because once the model is started with an error this error will grow larger throughout the run.

To start the model in mid water-year, for instance, January 1844, the seed values for volume and area at that elevation must be calculated by the linear equations in the program that approximate the rating curves and inserted as the seed values for October 1844. The gain in volume to the lake can then be internally set equal to zero for the intervening months between the first month of the water year and the month for which the elevation is known. By setting the gain in volume equal to zero, all internal calculations of evaporation and overflow are nullified. If the seed value for volume and area are calculated from the USGS rating curves rather than the linear equations that approximate the rating curves, the model will calculate a different final area each time the gain is set equal to zero. This will give a slightly different elevation from that which would be calculated by using the linear equations over the period when zero change is desired. If the model is run forward in time, searbled from a known

To run the model from 1844 to 1903, when the Truckee Canal began operation, it was decided for convenience to run to 1904 when an observed elevation was available. The error introduced by not removing the 1903-1904 Truckee Canal flows is thought to be small, because the canal was apparently not operated at full capacity for the first several years.

7.6.9 The Program

The model is a straight forward monthly calculation of a new volume contained in the lake based upon the amount of inflow to the lake, less the evaporation and overflow (See flow chart and discussion Appendix F). This final volume is used to calculate the surface area and lake elevation that correspond to that volume. The final volume, area, and elevation from one step become the initial values for the next step. The evaporation is calculated as a function of the average surface area of each step and the overflow is a function of the average elevation for each step (See Appendix F for a further discussion of this point).

7.6.10 Instability in the Model

The Pyramid Lake Model is capable of giving the same artificial elevations when it is run both forward and backward in time, provided that the seed values for the reverse run are the final calculated values from the forward run. If the model is run forward in time, starting from a known

elevation, it will probably produce an elevation at the end of the run which is near, but not exactly the known elevation on that date (as is expected). Running the model backward in time, starting from what was the ending observed elevation (which is different from the last calculated elevation of the forward run) the model will continue to deviate in the direction of the original difference, and the deviation will get larger. This problem arises from the calculation of the evaporation and the volume of water overflowed, both of which are a function of the initial and final volumes in the lake. The model tends to be self-correcting when it is run forward in time because both the evaporation and overflow are abstractive components of the change in volume equation. Let us assume, as an example, that during the course of the forward calculations the reconstructed lake has become slightly too large in comparison to the actual lake in a given time step. Under this situation both the evaporation and overflow tend to take more water out of the lake, because the surface area is larger and elevation is higher. This tends to lower the lake for the next time step which corrects the error. Now, let us use the same example during a run backwards in time, when the evaporation and overflow are additive components of the change in volume equation. The slightly larger lake has a larger surface area and elevation which results again in an increase in evaporation and overflow. In this instance, however, these components

make the lake larger in the next time step. Thus, the error tends to get worse rather than better, as in the forward case.

This lack of stability prevents using the model to go backwards in time with any reliability, particularly when the lake is overflowing through Winnemucca Slough. It is possible however, to start the model prior to 1844 by assuming successive elevations until a fit with little deviation from the data in the 1844-1904 time span is obtained.

7.6.11 Observed Truckee River Flows as Input

The Pyramid Lake Model was validated using the fortyfour years of observed monthly flows at Nixon. These data are completely independent of the tree-ring information, and as such, test the program's ability to reproduce lake levels.

It was found when using Harding's (1962) evaporation rates in the model that a plot of the observed elevations and calculated elevations had slight but distinctly different slopes over the forty-four years. By trial and error, it was found that the October, November, December, July, August, and September evaporation rates could be adjusted -0.01 feet per acre to eliminate the divergence (see adjusted factors in Table 5). These adjustments to Harding's evaporation rates are done simply to correct this apparent discrepancy in the model. This discrepancy could just as well be in another section of the model. It is not known that Harding's evaporation rates are in error, but they are suspect because of his use of Russell's less accurate 1882 curve . It should be noted that Harding's evaporation rates do work very well and are still a good approximation to the evaporation from Pyramid Lake despite these slight changes.

A plot of the synthetic and observed elevations as shown (Figure 6) indicates a generally good reproduction of the lake levels with a maximum deviation of three feet over a gross change in lake level of sixty feet. Figure 6

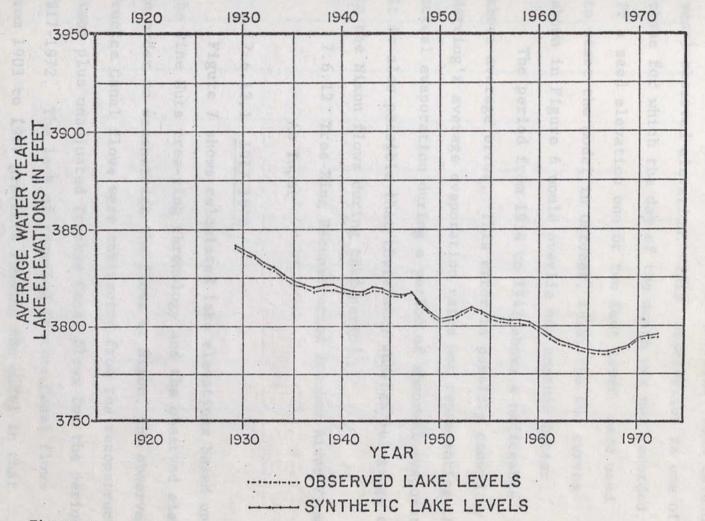


Figure 6

Comparison of the observed Pyramid Lake levels to synthetic lake levels using the observed flows of the Truckee River at the Nixon Dam flow gage as input to the model.

shows that the calculated elevation is consistently above the observed elevation. This appears to be caused by starting the model with the October, 1929 (October, 1928 calendar year) observed elevation. This observation is one of those for which the day of the month was not recorded. If a seed elevation one or two feet lower were used to start the model in October, 1929, the two curves shown in Figure 6 would overlie one another better.

The period from 1934 to 1942 shows a noticeable above average error. This error is possibly caused by Harding's average evaporation rates not representing the actual evaporation during a period of abnormal evaporation. It is also possible that there were substantial gauge errors in the Nixon flows during that period.

7.6.12 Tree-Ring Reconstructed Truckee River Flows As Input

7.6.12.1 1917-1972

Figure 7 shows calculated lake elevations based upon the Pine Nuts tree-ring chronology and the observed elevations. In order to reconstruct the flows at Nixon, the observed Truckee Canal flows were subtracted from the reconstructed Nixon plus unadjusted Truckee Canal flows for the period of 1917-1972. The lack of recorded Truckee Canal flows from 1903 to 1917 prevents running the model in that period. Prior to 1903, the model can be operated with just the reconstructed flows because the Truckee Canal did

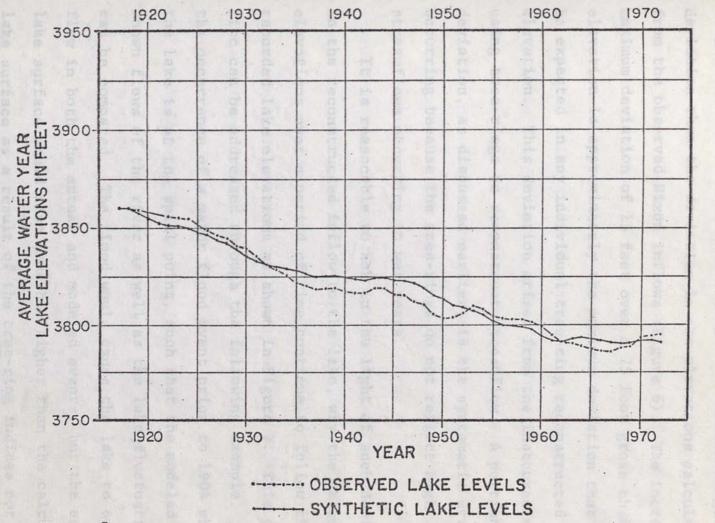


Figure 7

Comparison of the observed Pyramid Lake levels to synthetic levels using the Pine Nut Tree Ring chronology as input to the model.

not exist.

A comparison of Figures 6 and 7 shows that the treering reconstructed elevations (Figure 7) have a larger deviation than the deviation in the elevations calculated from the observed Nixon inflows (Figure 6). The increased maximum deviation of 15 feet over a 75 foot gross change in elevation is approximately the maximum deviation that can be expected in any individual tree-ring reconstructed elevation. This deviation arises from the inaccuracies of using tree-rings to reconstruct streamflow. A portion of this deviation, as discussed earlier, is the systematic error occurring because the tree-rings do not reflect high streamflows occurring in wet years.

It is reasonable to ask in the light of such deviation in the reconstructed inflow to the lake, why the calculated elevations over a period of time continue to follow the recorded lake elevations as shown in Figure 7. This question can be addressed through the following example. Assume the occurrence of a major flood event prior to 1904 when the lake is at the spill point, such that the modeled and known flows of the river as well as the lake fluctuations can be compared. The flood would cause the lake to overflow in both the actual and modeled events, but the actual lake surface would rise somewhat higher than the calculated lake surface as a result of the tree-ring indices not reflecting the high flows. Following the event, the actual lake surface would recede faster than the calculated

because it has a larger surface area and consequently, a larger volume of evaporation. More importantly, the real lake, due to its higher elevation would have a larger outflow cross-section through Winnemucca Slough than the modeled lake. Both of these larger extractions would cause the real lake to fall more rapidly than the modeled one, thus bringing the two to approximately the same elevation. Similarly, for long droughts, which bring the lake below the spill point, the modeled and real lakes will tend to merge. It does not matter in this case whether the real or modeled lake is lower in elevation; the lower of the two will have a smaller volume evaporated each month relative to the other, which will bring them together again. Even with these errors between the calculated and actual lake levels it should be noted that Figure 8 shows that the model is capable of following the actual lake elevations throughout its 78 foot decline, even when eleven years more inflow is modeled than the tree-ring streamflow equation was built upon.

7.6.12.2 1844-1904

The nine observed elevations (Table 7) during the period of 1844 to 1904 have a range of elevation of 18 feet. Although these are widely scattered points they give some feeling for the maximum range of elevation for the actual lake. The modeled lake (Table 7) over this same period has a maximum elevation fluctuation of slightly

Comparison of the reconstructed	elevation and observed elevations
1844 to 1904. The reconstructe	ed elevations are the average of the
12 monthly values. The observe	ed are single observations.

Date E 1844 1845 1846 1847 1848 1849 1850 1851 1852 1853 1854 1855 1856	3860.54 3859.81 3859.35 3859.48 3860.76 3861.70	Observed Elevation 3860.75 ² -0 -0	Difference 21 -0	Date 1874	structed Elevation	Observed Elevation	Difference
1844 1845 1846 1847 1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	3860.54 3859.81 3859.35 3859.48 3860.76	3860.75^{2} -0 -0	21		H. HETOL		Difference
1845 1846 1847 1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	3859.81 3859.35 3859.48 3860.76	-0 -0		1874	0044	that can	be
1845 1846 1847 1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	3859.81 3859.35 3859.48 3860.76	-0 -0		1874	2011		
1845 1846 1847 1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	3859.81 3859.35 3859.48 3860.76	-0 -0		1874	0000 00		
1846 1847 1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	3859.35 3859.48 3860.76	-0 -0	-0		3866.98	-0	-0
1847 1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	3859.48 3860.76			1875	3868.41	-0	-0
1848 1849 1850 1851 1852 1853 1854 1855 1856 1857 1858	386076	~	0	1876	3870.08	-0	-0
1849 1850 1851 1852 1853 1854 1855 1856 1857 1858		-0	-0	1877	3870.69	-0	-0
1850 1851 1852 1853 1854 1855 1856 1857 1858	3861 70	-0	-0	1878	3870.81	-0	-0
1851 1852 1853 1854 1855 1856 1857 1858	2007110	-0	-0	1879	3870.26	-0	-0
1852 1853 1854 1855 1856 1857 1858	3862.75	-0	-0	1880	3869.94	-0	-0
1853 1854 1855 1856 1857 1858	3863.04	-0	-0	1881	3869.87	-0	-0
1854 1855 1856 1857 1858	3863.67	-0	-0	1882	3870.20	3867.15	3.05
1855 1856 1857 1858	3865.57	-0	-0	1883	3869.58	-0	-0
1856 1857 1858	3867.07	-0	-0	1884	3869.64	-0	-0
1857 1858	3867.86	-0	-0	1885	3869.69	-0	-0
1858	3866.68	3860.251	6.43	1886	3869.39	-0	-0
	3865.19	-0	-0	1887	3869.09	-0	-0
1050	3863.61	-0	-0	1888	3868.05	-0	-0
1023	3861.54	-0	-0	1889	3866.46	3861.071	5.39
1860	3860.89	-0	-0	1890	3865.98	3876.07 ¹	-10.09
1861	3861.09	-0		1891	3867.11	3878.15 ¹	-11.04
1862	3861.75	-0	-0	1892	3868.09	-0	-0
1863	3862.52	-0	-0	1893	3868.41	-0	-0
1864	3862.19	-0	-0	1894	3868.54	-0	-0
1865	3862.06	-0	-0	1895	3868.65	-0	0
1866	3862.64	-0	-0	1896	3868.52	-0	-0
1867	3863.71	3867.25 ¹	-3.54	1897	3868.07	-0	-0
1868	3865.55	-0	-0	1898	3866.93	-0	-0
1869	3866.71	-0	-0	1899	3864.94	-0	-0
1870	3866.37	-0 ,	-0	1900		-0	-0
1871	3865.11	3876.25 ¹	-10.96	1901	3864.58	-0	-0
1872	3865.29	-0	-0	1902	3864.83	-0	-0
1873	3866.36	-0	-0	1903	3864.60	-0	-0
				1904	3864.77	3862.07	2.70

¹ The month of the year when the observation was taken is not known.

 $^{\rm 2}$ The day of the month when the observation was taken is not known.

TABLE 7

more than eleven feet. The probable reasons for this reduced range of fluctuation of the modeled lake has been discussed previously. The maximum range of the difference between the calculated and the nine observed elevations (Table 7) is a little more than seventeen feet. This range of deviation, although poorly documented due to few data points, gives some feel for the error that can be expected.

This similarity of the magnitude of the deviation in the tree-ring calculated lake levels and the overall change in the real lake level is expected for several reasons. The deviation of 7 feet is about the same in this time period, using tree-ring inflows only, as the 15 foot range of error found in the 1917 to 1972 period using the reconstructed Nixon minus Truckee Canal flows. This suggests that there is no fundamental difference in the quality of the data. In addition, two lines of evidence point towards Pyramid Lake having its mean elevation under the prevailing climatic conditions near the spill point. First, Winnemucca Lake is known to have had water in it several times prior to 1904. It is not known though, whether the water came from overflow of Pyramid Lake or bifurcation of the Truckee River. The apparently large fluctuations in surface area, and therefore volume, of Winnemucca Lake noted by Hardman and Venstrom (1941), and Harding (1965), would suggest that Winnemucca inflows are sporadic Pyramid Lake overflow events. Therefore, if overflow were occurring, even

occasionally, Pyramid Lake must have been close enough to the spill point to allow the flood water to spill. Secondly, the upper limit of Pyramid's elevation was well established by the rapidly increasing cross-sectional area for outflow. This had kept the lake from rising above 3879 feet according to Hardman and Venstrom (1941). Conversely, the volumes of water that Truckee Canal has prevented from reaching Pyramid Lake (Appendix C), and the resulting drop in elevations (Appendix D), suggest the magnitude and time duration of a drought that would be necessary to drop the lake ten or fifteen feet below the spill elevation.

In short, Winnemucca Lake would have to be filled before Pyramid Lake could rise to an elevation of 3879 feet and a very long and severe drought would be necessary to lower the lake substantially below the spill point. This suggests reasonable limits of perhaps 25 to 30 feet as the maximum range in fluctuation that Pyramid Lake would have had prior to the opening of the Truckee Canal in 1903, and that its mean elevation was near the spill point elevation. The tree-ring information used in this report sheds some light on the time-frame over which these limits apply. The tree-ring data for the three chronologies do not show any long term changes in their growth patterns from 1745 to 1972 which suggests that the climate has remained fairly stable for that period. Assuming this to be true the range of elevations and the mean elevation of the lake would apply to this period also. Between 1742 and the

end of the last ice age they may apply also, but this can not be said with certainty.

7.6.12.3 1745-1904

Under the assumption that the tree-ring streamflow equation is valid prior to 1900, the lake model can be used to test the 1745 to 1904 period for the probable range of lake levels that will keep the computed lake surface within the bounds of the known data between 1844 and 1904. The model can be started in 1745 (the beginning of the Pine Nuts chronology) using various seed values and run forward in time.

Figure 8 shows three runs using a spill elevation of 3863 feet and seed elevations of 3893 feet, 3863 feet, and 3823 feet. The middle trace on Figure 8 used a seed elevation equal to the spill elevation. This can be considered a baseline case that shows an overall fluctuation of sixteen feet from 1745 to 1904. This fluctuation is about the same as the observed range in elevation from 1844 to 1904. The highest trace on Figure 8 represents a run using 3893 feet as a seed elevation for 1745. This clearly shows the rapid discharge of the thirty vertical feet of excess water in Pyramid Lake. When overflowing ceases, the lake levels merge with the baseline case. Without some provision in the model for filling Winnemucca Lake, any higher seed elevation would be pointless. Thus, the 3879 feet noted by Hardman and

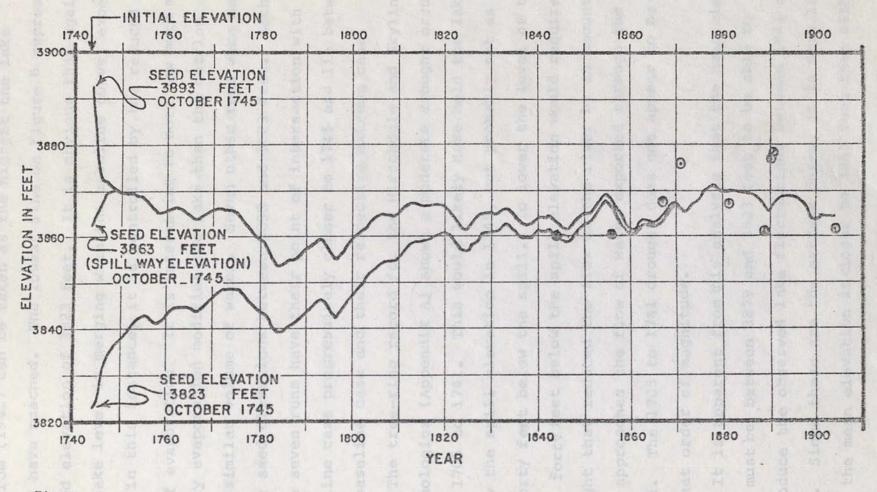


Figure 8

Plots of the tree-ring generated lake elevations starting at October 1745 and running forward in time. The results were checked against known elevations shown with circles from 1844 to 1904.

Venstrom (1941) can be taken as the highest the lake could have reached. The lowest line on Figure 8 represents a seed elevation of 3823 feet. It is obvious that again the lake level is merging with the baseline curve, except that in this instance, it is controlled by the reduced volume of evaporation. It is interesting to note how much more slowly evaporation modifies the lake than the outflow of a similar volume of water. Seven other runs were made using seed elevations between 3893 and 3823 feet. Each of these seven runs have their point of intersection with the baseline case progressively closer to 1745 and lie between the baseline case and their respective extreme case.

The tree-ring record for the Hirschdale and Skyline chronologies (Appendix A) shows a moderate drought occurred from 1705 to 1741. This would likely have held the lake below the spill elevation in 1745, but probably not as low as forty feet below the spill. To lower the level of the lake forty feet below the spill elevation would require a drought that reduced the flow of the river by an amount that approaches the flow of water exported through the Truckee Canal. The 1705 to 1741 drought does not appear to be of that order of magnitude.

It is apparent from this analysis that the mean elevation must be between 3879 and 3823 feet to be able to reproduce the observed lake fluctuations between 1844 and 1904. Since these are the extreme cases, it is more likely that the mean elevation is closer to 3863 feet than either

extreme. Thus, the model, despite its inaccuracies on a short term basis, confirms that Pyramid Lake has its mean elevation near the spill point elevation of 3863 feet due to the physical-evaporative constraints upon the lake. Furthermore, the modeling suggests that during the 1745 to 1904 period, the lake had a limited range of fluctuations of sixteen feet. It is likely, though, that this range should be twenty-five to fifty percent greater (20 to 24 feet), due to the inability of the model to reproduce the high river flows.

8.0. SUMMARY AND CONCLUSIONS

The methodology used in the conversion of tree-rings from drought sensitive trees into indices of streamflow have been presented by Schulman (1945b), Stockton (1971), and others. The trees used in this report are nine Pinus monophylla (Pinyon pine) from an elevation of 4300 feet in the Pine Nut Range east of Gardnerville, Nevada. For this study, the indices of streamflow were computed for the Nixon plus Truckee Canal flows. These flows represent virtually all of the water in the Truckee River as it would have been prior to the construction of the Truckee Canal. A regression equation was developed between the tree-rings and the Nixon plus Truckee Canal flows. In this form the regression equation can be used to estimate the flow history of the river. The regression equation was derived from forty-four years of data at the Nixon gauge and tested against seventy-two years of streamflow records at the Farad gauge, upstream. The correlation coefficient between the reconstructed flows at Nixon and it's historic flows is r=0.72, while the correlation coefficient between the reconstructed flows at Farad and it's historic flows is r=0.70 for the longer period. This demonstrates that the regression equation is valid over the longer period.

A computer model of Pyramid Lake was built that uses as input, the inflow of the Truckee River to the lake at the Nixon gauge and computes the new elevation of the lake after accounting for evaporation and outflow. The model was tested by using historical river inflows as input and comparing the calculated and observed elevations from 1929-1972. The model estimated the lake elevations to within three feet of the actual lake over a total lake level decline of sixty feet. When the tree-ring regression equation was coupled to the model the deviation increased to fifteen over a seventy-eight foot decline during the period of 1917 to 1972. It is apparent that the use of tree-rings to reconstruct streamflow has its limitations, but that this type of data can be useful as an approximation to otherwise unobtainable information on streamflow prior to the written records.

Running the model from 1844 to 1904 allowed the calculated elevation to be checked against nine widely scattered observations of the lake's elevation. The range in elevation of the actual lake is eighteen feet, while the modeled lake fluctuated eleven feet. The maximum range of the difference between the calculated and the nine observed elevations is a little more than seventeen feet. This range of deviation, although poorly documented due to the small number of points, is about the same as found for the 1917-1972 data, but during the 1844-1904 period the lake did not experience the 68 foot decline of the 1917-1972 period. Thus, similarity of the deviation and the total fluctuation of Pyramid Lake is largely a function of the stability of the lake. It is likely that this limited fluctuation of

of Pyramid Lake is caused by the physical evaporational environment of the lake. For instance, whenever the lake is above the spill point elevation of 3863.25 feet, water is discharged into Winnemucca Lake through Winnemucca Slough. This process has effectively kept the lake from rising above the 3879 level since the last time Winnemucca Lake was filled. (Although it is not known when this last occurred, the tree-ring growth data suggest that this was at least 450 years ago). Secondly, whenever the lake level drops below the spill elevation the overflow is stopped and evaporation becomes the only extraction. Assuming that the mean long term climatic conditions remain the same, any short term drought has a limited ability to lower the lake's surface, because as the lake level drops the volume of evaporation is reduced. Therefore, the lake stays near its spill point elevation as long as the mean long term climatic conditions remain the same.

It is possible through the use of the climatic information correlated to the tree-ring record and the lake level model to reconstruct the long term trend in the elevation of Pyramid Lake and see the approximate location of the mean elevation of the lake under the prevailing climatic conditions.

Running the model forward in time from 1745 (the beginning of the Pine-Nuts tree-ring chronology) using various seed elevations, demonstrated that any seed elevation above the spill point elevation very rapidly converges

on the case where the model was started at the spill elevation. Seed values below the spill point converge more slowly. Starting the model in 1745 from an elevation forty feet below the spill elevation only causes a slight disturbance in the 1844-1904 period. Since it is known that Winnemucca Lake received inflows from Pyramid Lake and/or the Truckee River in this later period, it is unlikely that any lower seed elevation could be used in 1745 and have the model stay within the bounds of the known data. Furthermore, if the lake had been strongly depressed in 1745, there should have been a severe drought prior to then. The Skyline and Hirschdale tree-ring data (Appendix A) show a moderately severe drought from 1705 to 1741 which would have had the lake's elevation relatively low in 1745, but it is not likely that it was as much as forty feet below the spill way elevation. Thus, the equilibrium elevation must be near or slightly below the spill point elevation of 3863 feet. The lake model shows the lake as having a maximum range in elevation of sixteen feet. However, due to the inability of the model to reproduce high flows, this could be twenty-five to fifty percent larger. The model can not reconstruct a discreet event, particularly a flood event, but over an extended period of time the model gives a reasonably good approximation to the hydrograph of Pyramid Lake.

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9.0 SELECTED REFERENCES

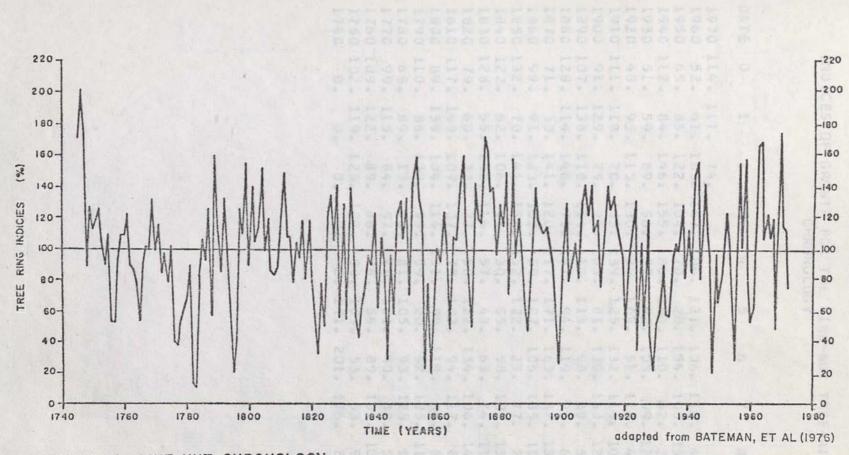
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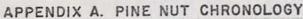
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APPENDICES		
	1.	
CHRONALCET		
PLOT OF HIRSCHIMET COMONOVARY		
INDICKS OF CHARGE ENA COM STREET, AND THE PARTY OF		

10.0 APPENDIX A

- 10.1 PLOT OF PINE NUT TREE-RING CHRONOLOGY
- 10.2 INDICES OF GROWTH FOR THE PINE NUT TREE-RING CHRONOLOGY
- 10.3 PLOT OF SKYLINE CHRONOLOGY
- 10.4 INDICES OF GROWTH FOR THE SKYLINE TREE-RING CHRONOLOGY
- 10.5 PLOT OF HIRSCHDALE CHRONOLOGY
- 10.6 INDICES OF GROWTH FOR THE HIRSCHDALE TREE-RING CHRONOLOGY

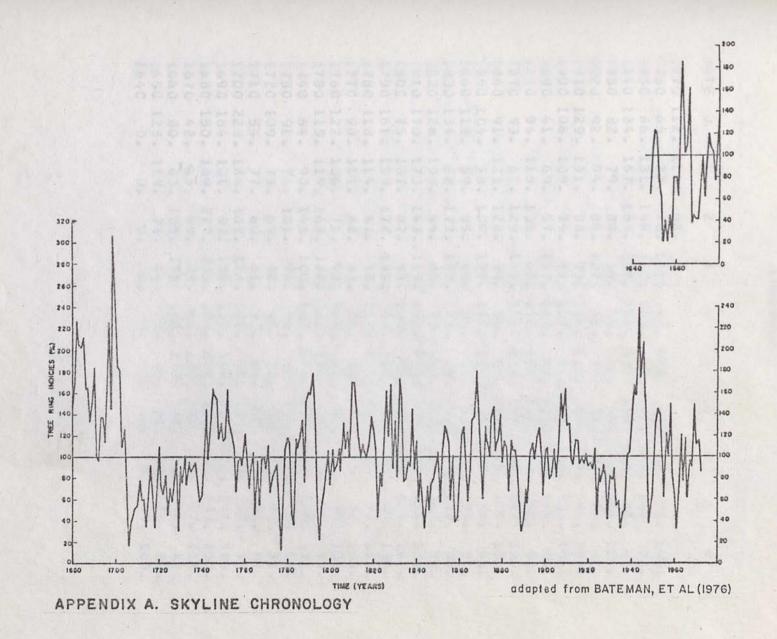




APPENDIX A

INDICES OF GROWTH FOR THE PINE-NUT TREE-RING CHRONULOGY

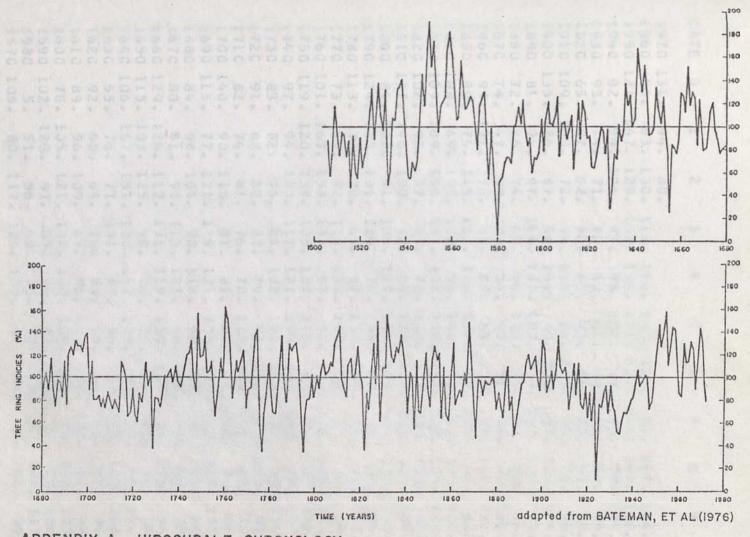
DATE	J	1	2	3	4	5	6	7	8	9
1970	114.	111.	74.							
1960	52.	61.	165.	167.	106.	121.	106.	118.	48.	173.
1950	65.		122.			28.			156.	
1940	112.	84.	146.	155.	80.	140.	110.	92.	20.	96.
1930	51.	59.	89.	58.	57.	92.	103.	99.	127.	81.
1920	48.	95.	115.	130.	35.	103.	51.	118.	43.	22.
1910	111.	118.	70.	103.	139.	125.	132.	114.	105.	95.
1900	91.	129.	79.	93.	103.	81.	130.	14).	121.	140.
1390	107.	138.	118.	114.	110.	113.	98.	.98	53.	26.
1880	128.	114.	148.	71.	157.	94.	119.	95.	64.	47.
1870	71.	50.	141.	120.	117.	171.	163.	137.	139.	96 .
1360	99.	91.	123.	103.	59.	107.	106.	135.	159.	112.
1850	1.32.	70.	141.	158.	132.	122.	23.	77.	20.	34.
1840	122.	62.	106.	91.	30.	\$5.	58.	121.	130.	106.
1830	128.	55.	138.	116.	57.	43.	63.	74.	116.	110.
1820	79.	60.	32.	77.	50.	122.	134.	106.	140.	56.
1910	117.	148.	107.	107.	78.	103.	94.	117.	81.	117.
1800	89.	139.	104.	114.	151.	99.	119.	86.	83.	88.
1790		85.			59.	20.	42.	125.	110.	154.
1780	68.	89.	13.	10.	81.	105.	93.	125.	57.	159.
1770	99.	115.	84.			101.				
1760	109.	122.	89.	86.	78.	54.	89.	101.	101.	131.
1750	102.	119.	125.	102.	90.	109.	53.	53.	90.	108.
1740	0.	0.	0.	0.	0.	171.	201.	176.	89.	126.

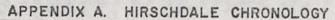


APPENDIX A CONTINUED

INDICES OF GROWTH FOR THE SKYLINE TREE-RING CHRONOLOGY

			4							
DATE	Э	1	2	3	4	5	6	7	8	9
1970	112.	116.	86.							
1960		33.	70.	121.	78.	118.	62.	97.	91.	149.
1950	. 66	131.	145.	140.	103.	62.	122.	102.	149.	83.
1940	134.	167.	158.	238.	174.	207.	158.	116.	38.	63.
1930	82.	79.	89.	54.	59.	34.	48.	50.	102.	104.
1923	92.	94.	89.	137.	46.	89.	76.	92.	84.	42.
1910	129.	131.	88.	93.	116.	115.	93.	99.	94.	103.
1900	106.	109.	79.	83.	107.	81.	102.	159.	136.	164.
1890	41.	69.	57.	103.	113.	\$8.	118.	127.	102.	54.
1880	94.	115.	109.	68.	115.	106.	106.	9).	57.	30.
1870	93.	61.	129.	115.	101.	137.	147.	106.	117.	139.
1860	91.	121.	128.	94.	59.	.88	134.	138.	179.	132.
1850	104.	58.	109.	129.	125.	114.	60.	101.	39.	43.
1840	115.	59.	85.	48.	34.	72.	51.	74.	79.	91.
1830	134.	82.	173.	159.	77.	79.	95.	91.	145.	89.
1820	126.	108.	44.	35.	73.	129.	162.	120.	168.	90.
1810	170.	173.	143.	119.	101.	112.	105.	100.	111.	138.
1800	75.	107.	87.	93.	108.	.88	132.	109.	124.	102.
1790	161.	159.	178.	135.	54.	23.	52.	66.	81.	106.
1780	118.	114.	47.	37.	117.	109.	117.	135.	77.	151.
1770	90.	111.	68.	83.	86.	54.	51.	13.	55.	103.
1760	122.	106.	71.	90.	104.	47.	96.	56.	97.	100.
1750	115.	119.	163.	132.	122.	111.	68.	96.	99.	93.
1740	64.	90.	145.	100.	121.	164.	158.	155.	118.	132.
1730	91.	76.	101.	74.	96.	86.	50.	95.	77.	58.
1720	109.	73.	65.	32.	46.	71.	59.	78.	97.	31.
1710	54.	77.	60.	61.	35.	64.	92.	70.	34.	62.
1700	228.	184.	182.	110.	126.	71.	17.	37.	47.	52.
1690	184.	101.	97.	138.	137.	114.	165.	226.	146.	306.
1680	150.	169.	227.	235.	203.	212.	165.	166.	134.	150.
1670	42.	63.	99.	63.	97.	120.	108.	103.	77.	113.
1660	80.	63.	105.	155.	102.	110.	141.	38.	46.	42.
1650	123.	119.	76.	64.	20.	40.	21.	45.	24.	80.
1640	0.	0.	0.	0.	0.	0.	0.	0.	65.	103.





APPENDIX A CONTINUED

INDICES OF GROWTH FOR THE HIRSCHDALE TREE-RING CHRONOLOGY

DATE	0	1	2	3	. 4	5	6	7	8	9
1970	131.	94.	80.							
1960		83.	130.	113.	114-	132.	125.	90.	117.	144.
1950	121.	147.	128.	139.	158.	111.		144.	141.	108.
1940	82.	94.	106.		98.		99.	72.	79.	104.
1930	93.		71.	52.		61.	69.	68.	72.	79.
1920	65.	87.	62.	92.	10.		74.		93.	69.
1910	109.		72.			85.	113.		73.	93.
1900	133.	126.	87.	94.			106.			104.
1890	81.		97.	114.	121.		112.	119.		89.
1880	72.		74.	65.		110.	71.	83.		66.
1870	74.		99.	97.			94.		99.	101.
1860	99.		76.		89.		104.		149.	110.
1850	89.	74.	115.		116.		64.		66.	62.
1840	108.	69.	105.	100.	55.	116.	70.	68.	105.	122.
1830	109.	109.	156.	137.	118.	113.	127.	122.	138.	121.
1820	102.	95.	37.	87.	72.	110.	131.	95.	150.	62.
1810	137.	146.	100.	92.	84.	111.	117.	101.	104.	128.
1800	84.	102.	91.	106.	117.	104.	106.	114.	110.	130.
1790	124.	129.	131.	114.	78.	35.	68.		82.	104.
1780	113.	94.	70.	69.	97.	118.		94.		130.
1770	73.		92.	115.				35.		89.
1760	101.	163.	153.	133.			115.			125.
1750	119.	120.	137.	105.			67.	80.		118.
1740	97.	94.			121.		133.			157.
1730	83.		75.		102.		104.		97.	111.
1720	91.	68.	62.	87.			114.			38.
1710	82.		81.	76.		115.	110.		67.	70.
1700		90.	115.	81.			75.	82.	74.	88.
1690	115.	77.	121.		121.	133.	129.	129.		137.
1680	84.	98.	104.	90.		92.	76.		97.	84.
1670	80.	81.	99.		121.	96.		76.		82.
1660	129.	126.	112.	151.	119.	130.		84.		84.
1650		103.	125.		74.	25.	84.		76.	.88
1640	106.	167.	151.	130.	152.	129.	93.	94.	99.	134.
		76.								
1620					88.			76.		28.
1610			109.	63.		120.		122.	113.	89.
1600	78.		121.	102.			125.		102.	103.
1590			97.	33.	90.	57.		71.		106.
1580	5.	51.			73.	90.	68. 54.			117.
1570	105.	80.	117.	125.	150.	90.0	54.	90.	66.	39.

APPENDIX A CONTINUED

INDICES OF GROWTH FOR THE HIRSCHDALE TREE-RING CHRCNOLOGY CONTINUED

DATE	0	1	2	3	4	5	6	7	8	9	
1560	164.	135.	105.	107.	157.	133.	115.	118.	127.	123.	
1550	191.	152.	173.	142.	95.	117.	121.	125.	165.	182.	
1540	79.	56.	55.	69.	61.	71.	106.	145.	118.	166.	
			49.								
			74.			133.					
1510	92.	75.	95.								
1500											

11.0 APPENDIX B

OBSERVED NIXON FLOWS

CUBIC FEET PER SECOND

APPENDIX B

OBSERVED NIXON FLOWS. CUBIC FEET PER SECOND.

PPENDIA-R CONTINUED

DATE	OCT	NOV	DEC	JAN	FEB	MAR	APR	МЛҮ	JON	JUL	AUG	SEP
1972	40680	8420	20270	33050	32390	45730	13605	13777	5794	1787	6438	3250
1971	7650	4700	35070	59610	51580	53150	45800	75890	103800	27710	14700	
1970	2900	2180	21210	128300	127300	90450	31447	12228	16716	26684	4799	3818
1969	2860	2580	10430	79140	90570	135100	201800	212400	206400		1650	
1968	25750	19170	27860	33030	51860	51430	13750	4180	312.0	1606	3156	
1967	2246	2960	3620	9650	4410	67280	36310	191600	225800	72547	4983	17290
1966	2522	17280	45340	36360	20040	12950	3777	2976	1602	1119	1488	
1965	1990	2040	95140	73250	55450	35240	34500	81470	29610	2434	18499	2792
1964	2630	3730	2230	1930	1520	1275:)	3180	4195	1682	160	1126	
1963	13648	2030	2510	4250	128600	8990	10910	85510	55080		2006	2520
1962	356	1140	1120	1140	2690	1520	18265	13045	700	225	89	250
1961	1710	1730	1780	1390	1340	1 38 ()	634	328	81		1420	311
1960	1450	1440	1560	1640	4580	2520	1386	2802	0		225	
1959	375	3580	2170	3180	3410	1840	1250	1610	1130	1280	1570	1780
1958	1012	2310	3220	2040	6950	4591	153218	261858	69342	1059	4919	0
1957	1891	3208	3315	3315	8536	2830?	3366	25027	41026	535	0	950
1956	261	8	64940	89282	65399	67906	77531	104102	107963	6789	111	356
1955	1976	2744	2517	307	277	30.7	665	1120	1510	1430	1330	1030
1954	1489	2627	2509	2332	1663	758J	11018	14832	162.4	172	1053	1327
1953	5061	6162	17463	36002	43613	65:10	14268	52310	75761	19652	317	659
1952	1697	2435	9892	47001	113642	107207	211296	329630	178952	22352	1875	8326
1951	2226	94788	198135	113236	64627	41232	11935	10231	5795		673	1301
1950	79	1247	1719	1608	1552	1719	20772	46011	20119	647	0	970
1949	1453	1188	1630	1228	1109	1287	1556	5289	487	57	53	40
1948	1948	1733	1535	1228	1148	122.8	1768	4269	1582.6	297	0	776

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APPENDIX B CONTINUED

DATE	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1947	1918	10807	17117	29379	11039	3122	1168	4651	578	0	311	847
1946	3297	6423	21521	24920	28740	18798	52488	49791	4944	588	327	1000
1945	1616	4378	2629	2184	13139	77:44	23690	67655	18929	1647	662	1436
1944	10452	9639	4283	2629	3789	30033	14927	7429	2610	909	616	584
1943	2726	8528	28207	94054	145201	137764	163675	85813	29938	2515	1790	2079
1942	1536	9043	39117	57161	73260	45655	86436	86259	135596	15198	111	1432
1941	3265	22859	39770 .	36931	22715	3837	1727	33616	26851	1152	1158	774
1940	2107	2269	2299	4445	6158	49245	105999	88158	17790	746	0	1208
1939	8710	28431	31159	14997	23200	25775	14165	1511	1247	0	0	582
1938	5055	2352	68177	11648	35000	55998	145433	248635	142469	19396	1156	1899
1937	3944	3128	2479	3115	31726	37844	61414	42847	4065	0	0	0
1936	744	2231	2059	3154	5914	16923	64831	58669	23463	620	438	2582
1935	1822	2101	1760	1665	1596	1402	40887	54167	20746	0	0	0
1934	618	2823	2968	2382	2121	5340	2831	214	1182	0	0	222
1933	944	2051	2045	1899	1899	2224	1186	3606	16810	186	0	0
1932	0	1148	1208	2039	1946	6675	26092	49401	25597	386	0	0
1931	1701	1657	1597	1725	1018	22.4	174	46	0	0	111	0
1930	737	1226	5423	1523	1178	6690	19778	3546	550	0	0	131
1929	1897	1897	2570	8876	1695	1445	1045	1881	5103	0	0	0

OCTOBER 1928 - DECEMBER 1957 (CALENDAR YEARS), FLOWS WERE MEASURED AT THE PYRAMID DAM GAUGING SITE BY THE FEDERAL WATER MASTER, RENO, NEVADA. THESE RECORDS ARE ON FILE AT THAT OFFICE.

JANUARY 1958 TO PRESENT, FLOWS WERE MEASURED AT THE USGS GAUGING SITE APPROXIMATELY TWO MILES UP STREAM FROM THE OLD SITE. THESE RECORDS HAVE BEEN ADJUSTED TO THE OLD SITE BY PEMOVING THE FLOW OF THE BUREAU OF INDIAN AFFAIRS-NIXON DITCH, WHICH LIES INBETWEEN.

THE 1955 RECORDS ARE INCOMPLETE SO THE DERBY DAM PLOWS ARE USED INSTEAD.

12.0 APPENDIX C

OBSERVED TRUCKEE CANAL FLOWS AT DERBY DAM HEAD GAUGE, 1917-1966. 1967-1972 FLOWS AT TUNNEL NUMBER 3,

NEAR WADSWORTH, NEVADA. CUBIC FEET PER SECOND

APPENDIX C

							20540	23074				
									111114		1000	
DATE	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1972	26710	26829	14710	2520	964	12170	35720	53680	29845	13498	14313.	21109
1971	25450	29790	728	0	0	0	23770	45370	30260	28180	19350	16900
1970	14347	29020	23903	2960	1710	5550	8680	48980	48940	18000	15850	19530
1969	20640	31820	25100	3340	3340	1690	9930	13660	10340	20720	15450	20210
1968	15790	16590	12290	2400	1850	1960	19710	40000	21480	14340	16830	17380
1967	19455	28021	43118	31960	34560	19100	6300	18380	13030	23510	20850	18700
1966	31415	26223	26469	18774	3455	24364	30308	44596	14866	14414	16786	17119
1965	17998	21643	32246	15147	12898	11442	46429	16858	44392	20925	11092	20212
1964	17695	29849	27179	26360	23724	4512	25657	52818	30909	12739	12844	17089
1963	31199	32302	32173	22323	11817	19641	41469	35387	25338	16583	17008	19875
1962	4847	7223	9852	10539	18465	21815	54606	54135	36618	12969	13028	14200
1961	20875	18612	21732	16598	19572	1968 1	19551	19911	17052	11722	12022	6579
1960	21255	24058	25553	23473	30848	37227	34612	29808	22815	15798	16531	16186
1959	28937	31105	33596	32254	29593	34606	20196	21566	12707	13103	13834	18952
1958	29668	31396	33757	30446	30654	38452	17319	40933	47234	25138	19487	25312
1957	30631	37050	36381	30136	29217	40061	37582	47817	38618	16309	13874	18390
1956	19768	22134	30472	36725	9829	33496	32551	38747	26057	28154	15818	19665
1955	20702	27318	30840	32460	26395	30232	24219	29317	30785	13174	11301	16344
1954	31203	33899	32321	30515	28959	3664 1	36152	38176	16220	12266	13733	18041
1953	26250	32307	31210	29385	31038	34371	30957	38838	25508	36727	19574	24978
1952	26282	33226	41435	34505	12369	1716()	24623	41255	44376	50003	21249	22271
1951	25213	30771	5930	2455	7444	14259	34469	60566	53343	13460	15309	19283
1950	14953	16816	20010	31452	31093	33909	51444	52103	46690	16574	14786	23514
1949	21323	23811	27775	23760	22770	25273	36400	48274	16089	11943	13333	10328
1948	25710	26663	27746	29517	22524	19326	23011	37422	41425	14543	13987	15899
	1.000											

OBSERVED TRUCKEE CANAL FLOWS AT DERBY DAM HEAD GAUGE, 1917-1966. 1967-1972 FLOWS AT TUNNEL NUMBER 3, NEAR WADSWOFTH, NEVADA. CUBIC FEET PER SECOND.

APPENDIX C CONTINUED

DATE	OCT	NOV	DEC	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP
1947	30021	29898	15850	10955	23984	32528	20544	25074	13613	11601	12674	16551
1946	24501	30223	18349	15990	3483	24019	33107	33575	23394	15030	13397	19549
1945	24629	31371	28989	28257	33830	32082.	21558	17527	18099	14309	13056	17891
1944	20246	23711	29755	30108	27552	9393	17248	34448	23750	13096	13011	16042
1943	21958	29379	14707	10941	280%	3142	8763	23714	28029	18610	13579	17693
1942	25116	21671	4788	1477	1345	1883	25104	29801	16200	17010	12621	18366
1941	23447	11387	1594	2087	21774	41507	32527	50603	19356	15359	15866	18448
1940	29922	23558	24637	36741	33321	22010	12430	22477	23950	15507	12167	21451
1939	20986	4241	3041	20917	6269	10906	14658	19701	12258	12345	13876	21693
1938	5071	15529	16577	19535	4516	2746	5995	11183	12141	19786	13464	16361
1937	13296	12385	11987	12706	5429	2247	13506	51809	29975	13058	10031	6639
1936	1903	5562	7538	16622	21922	40913	43574	46298	31623	11850	11920	9169
1935	5453	8067	6795	8878	10745	12068	44867	51876	39087	6364	1671	1198
1934	1142	6956	10482	12724	13137	30217	20570	6294	3255	3764	8510	6823
1933	3748	4429	5326	7829	7932	11371	20917	26338	33432	5374	871	531
1932	1683	2988	8247	13442	14001	29761	46878	47514	43489	13480	6526	2701
1931	8878	11652	7970	9882	9116	19008	14795	14450	2164	240	0	461
1930	6637	9829	24778	15375	22386	32058	47847	47520	28013	7154	6368	7855
1929	16972	18042	15270	9005	12957	22439	23920	39194	15571	8371	4069	3756
1928	23653	28940	19659	4095	7950	7022	22636	41406	12806	11480	11621	14258
1927	2330	10304	19523	22762	25993	37266	36076	37253	35111	21675	12494	17759
1926	15818	15517	18347	14824	20881	30258	32632	27409	9239	6120	2932	1321
1925	10347	14628	9777	12937	27065	32134	35725	37383	28589	14361	14098	9765
1924	26457	26475	20579	16800	12521	17165	16606	11369	3782	1291	2705	5253
1923	23134	19853	14131	16674	14947	13288	18315	32937	34205	24087	21715	24091
1922	19888	20278	18987	15796	12642	11296	16133	32514	25031	19764	14844	16782
1921	11654	19844	23574	10205	11215	8773	32017	35290	33995	13900	11139	17984
1920	23612	26728	18800	18836	22469	28710	18422	31773	23582	11935	10896	9272
1919	16230	16000	15254	17954	18952	22362	26246	32712	19658	15378	16148	17260
1918	22630	26858	31334	9322	22268	26336	12402	32960	29948	22616	23348	23914
1917	10334	10922	12556	41774	40166	43372	47000	53000	50006	58364	26970	20908

13.0 APPENDIX D

PYRAMID LAKE ELEVATIONS IN FEET, INTERPOLATED TO THE 15TH OF THE MONTH

APPENDIX D

PYRANID LAKE ELEVATIONS IN FEET, INTERPOLATED TO THE 15TH OF THE MONTH.

DATE	OCT	NOV	DEC	JAN	PEB	MAR	APR	KAY	JUN	JUL	AUG	SEP	
1972	3794.90	3794.85	3794.85	3794.95	3795.10	3795.30	3795.50	3795.45	3795.20	3794.90	3794.30	3793.85	
1971	3793.50	3793.10	3793.00	3793.15	3793.75	3794.05	3794.25	3794.75	3795.65	3796.00	3795.80	3795.25	
1970	3792.30	3792.05	3791.80	3792.15	3793.75	3794.70	3795.00	3795.00	3794.90	3794.90	3794.30	3793.75	
1969	3786.70	3786.35	3786.10	3786.15	3787.10	3787.90	3789.50	3791.40	3793.10	3793.95	3793.45	3792.80	
1968	3788.05	3787.75	3787.70	3787.75	3788.20	3788.70	3788.80	3788.80	3788.60	3788.10	3787.55	3787.10	
1967	3784.70	3784.20	3784.05	3784.10	3783.90	3784.00	3784.60	3785.20	3787.60	3789.00	3788.80	3788.35	
1966	3787.10	3787.10	3787.10	3787.10	3787.10	378720	3787.10	3786.80	3786.55	3786.35	3785.90	3785.10	
1965	3786.40	3786.15	3786.25	3786.80	3787.60	3787.85	3787.90	3788.40	3788.75	3788.55	3788.15	3787.70	
1964	3789.85	3789.55	3789.35	3788.90	3788.90	3788.65	3788.50	3788.70	3788.60	3788.30	3787.60	3786.95	
1963	3790.10	3789.70	3789.75	3789.40	3790.20	3790.30	3790.50	3790.75	3791.75	3791.10	3790.50	3790.05	
			3792.15										
			3795.75					575633 (20 Section 21 (1948)					
			3798.80					The second state of the second			12000 2012 2010 2022		
CARLES CARLES			3802.40										
			3801.25						CONTROL 101 T 174 (17)				
1957			3804.20										
			3802.30							the second s			
			3804.60					The second se				Contraction of the second s	
			3807.95										
1953			3808.80										
			3803.05					The second se				A REAL PROPERTY OF A REAL PROPERTY OF	
1951			3803.60										
			3803.95		Contraction of the second s								
1949	0.0	and the second sec	0.0		3805.70		0.0	0.0		172 T 171	3805.60	and the stand of the stand of the	
1948	3011.35	3011.14	3810.64	3810.14	3010.05	3009.80	0.0	0.0	0.0	0.0	0.0	0.0	

APPENDIX D CONTINUED

	-					-												0	0	2								
SEP	3811.64 3814.39	816.0	817.9	820.5	P17.6	816.2	817.5	A18.2	820.6	817.4	819.2	821.5	823.8	R27.7	830.9		0.0	3840.05	3843.95	3845.7								
AUG	3812.14 3814.89	916.5	818.5	821.0	818.1	B16.6	818.1	818.9	821.2	818.1	819.8	821.9	824.3	828.0	831.4		0*0	840.5		846.1								
JUL	3812.89 3815.20	817.0	819.0	821.6	818.6	817.1	818.6	819.3	821.6	818.5	820.2	822.4	824.8	829.2	8.11.8	.0	0.0	11.10		146.50							61.	066.6
NDC	3812.80	817.2	B19.2	821.9	818.0	817.2	819.1	819.8	821.3	818.6	820.6	822.8	825.2	829.3	832.1	0.0	838.25	841.64	. 14	846.70	848.7							
MAY	3812.35	817.0	0.0	821.6	R17.5	R16.3	. 8	820.1	818.9	818.6	820.0	822.4	825.3	829.0	831.7	835.2	0.0	841.85		846.35								
APR	3812.10	816.75	0.0	820.30	817.05	816.14	. 64	820.14	817.55	818.10	819.80	822.05	R25.60	829.14	831.64	0.0	0.	0.	# 2°3300	6.25°								
MAR	3812.25	816.70	819.11,	818.50	816.50	B16.55	816.95	820.10	816.85	817.50	819.80	822.25	825.75	829.20	831.64	0.		0.0	844.	46.16								
FEB	12.60	816.75	819.25	B18.14	816.00	817.25	817.20	820.05	816.85	317.00	819.95	822.45	825.85	829.30	831.75	.0												3864.45
NAL	2.75	816.75	819.35	016.85	815.45	816.95	817.20	820.25	817.14	816.95	820.14	822.60	825.95	829.39	831.95	.0												
DEC	3812.89	816.80	819.45	816.75	815.35	817.14	817.35	819.85	817.14	818.10	820.20	822.85	826.85	829.75	832.05	0.0												
AON	3813.14	817.20	819.80	916.64	815.35	816.95	817.45	819.89	816.85	818.39	820.80	822.95	827.25	829.95	832.25	0.0		.0	0.	2					- C.			
OCT	3813.55	317.50	320.05	817.14	815.45	817.00	817.80	820.20	816.89	818.70	821.30	823.39	827.55	830.39	832.64	0.0		843.5	.55	0.0					.0			
DATE	246	5	116	643	942	941	0110	939	938	126	936	935	934	533	932	131	6	6		6	26	26	6	6	6	6	6	6

APPENDIX D CONTINUED

28.19	U*0	0.0	3868.75	0.0	0.0	0.0	0.0	0.0	3867.15	0.0	0.0	0.0	0.0
AUG	0.0	0.0	3869.	0.0	. 0*0	.0			.0	0		.0	.0
JUL	3864.70	0.0	3869.95	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
NOF	0	85	56	0	0.0	0	0	0	0	0	0	-	0
HAT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
APR	0.0	0.0	0.	869.	0.0	0.	0.	0.	0.	.0	.0	0.	0.
MAR	0.0	0.0	0.0	0.0 3	3862.07ª	3876.57 ⁸	3878.158	3861.07 ^a	0.0	3876.258	3867.25ª	3860.25 ⁸	0.0
FEB	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
JAN	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0*0.	0.0	0.0	0.0	0.0	3860.75 ^b
DEC	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
AON .	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
ocr	0.0	1868.00	0.0	0.0	0.0	0.0	0.0	0-0	0.0	0.0	0.0	0.0	0.0
DATE	1913												

"DATA IS PLACED IN MID-YEAR BECAUSE ONLY THE YEAR OF THE OBSERVATION IS KNOWN.

DDATA IS PLACED IN MID-HONTH BECAUSE ONLY THE MONTH OF THE OBSERVATION IS KNOWN.

COATA IS PLACED IN MID-MONTH BECAUSE ONLY THE MONTH OF THE OBSERVATION IS KNOWN. THIS DATA HAS BEEN INTERPOLATED TO THE 15TH OF THE MONTH TO FILL IN MISSING DATA BETWEEN CLOSELY SPACED POINTS.

S.O APPENDIX E

14.1 THE PYRAMID LAKE ELEVATION MODEL DISCUSSION

The model is a straight forward monthly valculation of a new volume contained in the lake based upon the amount of inflow to the lake, leavethe evaporation and overtlow (see the following flow that, appendix f). This final volume is used to calculate the surface area and lake, elevation that correspond to that walkes. The final volume, area, and elevation from one step become the

14.0 APPENDIX E

14.1 PYRAMID LAKE ELEVATION MODEL DISCUSSION

14.2 PYRAMID LAKE ELEVATION MODEL FLOW CHART

14.3 PYRAMID LAKE ELEVATION MODEL USING TREE-RING GENERATED TRUCKEE RIVER FLOWS AS INPUT

tion ALL(J,M), from AALGA, If AEL(J,M) is greater than 3863 25 feet the model chooses the appropriate equations for Manning's hydraulic redius. B, and cross-sectional area, ASECA These values are then used in Hauning's equation so calculate the volume of overflow through Winnemarca Shough, WSFLD(J,M). With this intocharton on hand, the GAIM, Hinal volume VOLP(J,M) and itigal area ARLAF(J,M) are calculated. The average area, ballA, can then be rested to see if it is less them 0.1 area (an arbitrary outoff for the test) from the true sectors of (AREAL(J,M) + AREAR(J,M)/2. If the test fails, the program books back to perform the

14.0 APPENDIX E

14.1 THE PYRAMID LAKE ELEVATION MODEL DISCUSSION

The model is a straight forward monthly calculation of a new volume contained in the lake based upon the amount of inflow to the lake, less the evaporation and overflow (see the following flow chart, Appendix E). This final volume is used to calculate the surface area and lake elevation that correspond to that volume. The final volume, area, and elevation from one step become the initial values for the next step.

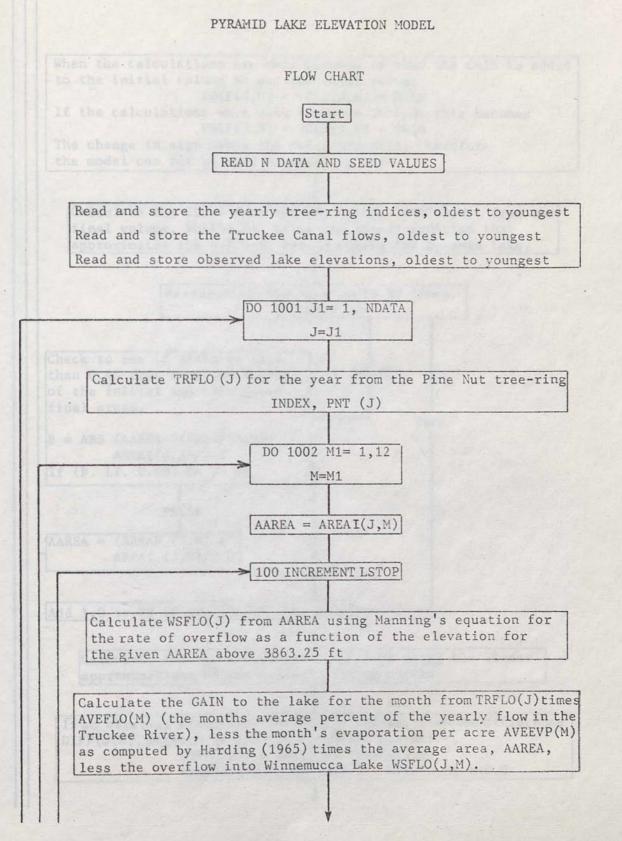
The only iterative loop within the program is for calculating the average area, AAREA, and Winnemucca Slough flows, WSFLO(J,M). At the beginning of each time step AAREA is set equal to the initial area, AREAI(J,M), for that step. The program then calculates the average elevation AEL(J,M), from AAREA. If AEL(J,M) is greater than 3863.25 feet the model chooses the appropriate equations for Manning's hydraulic radius, R, and cross-sectional area, XSECA. These values are then used in Manning's equation to calculate the volume of overflow through Winnemucca Slough, WSFLO(J,M). With this information on hand, the GAIN, final volume VOLF(J,M) and final area AREAF(J,M) are calculated. The average area, AAREA, can then be tested to see if it is less than 0.5 acre (an arbitrary cutoff for the test) from the true average of (AREAI(J,M) + AREAF(J,M)/2. If the test fails, the program loops back to perform the

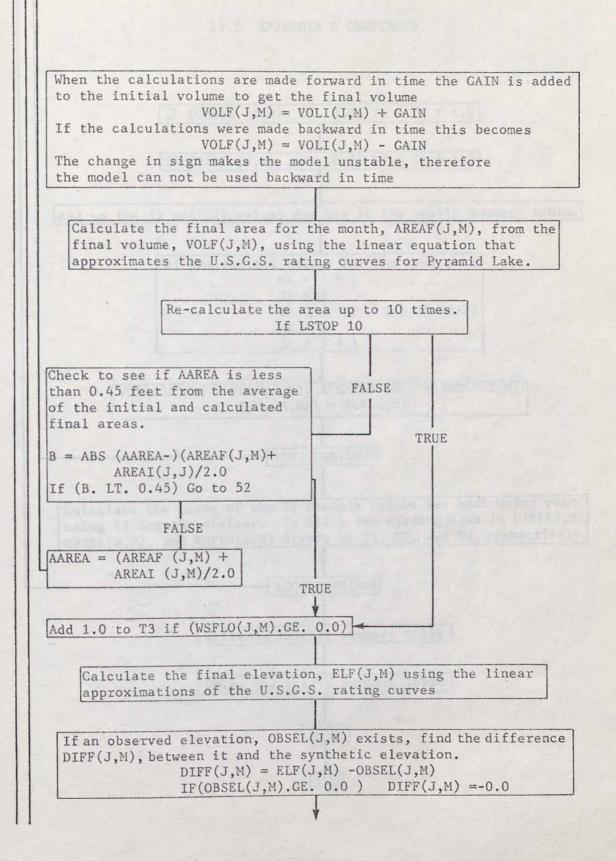
calculations again with AAREA set equal to the average it just calculated. This process rarely takes more than two iterations to close within the allowable error.

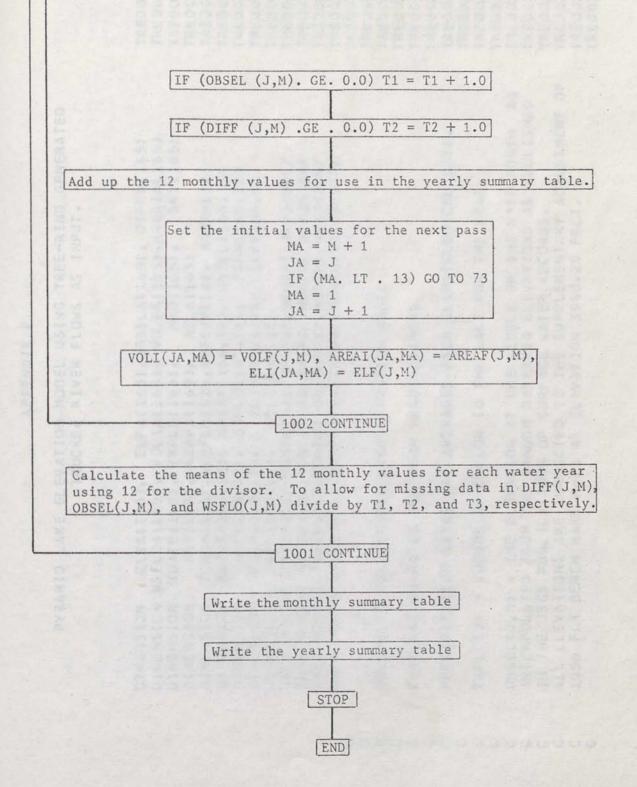
This procedure for calculating evaporation and overflow relative to the average area for the month makes the observed elevation on the middle of the month the most desirable for checking the synthetic elevations.

After all calculations have been made and stored, monthly and yearly summary tables are printed. The yearly values are the average of the twelve monthly calculations. The yearly averages are easier to interpret since the treering data is on a yearly basis, and is the limiting factor of the accuracy of the calculations.

14.2 APPENDIX E







APPENDIX E

PYRAMID LAKE ELEVATION MODEL USING TREE-RING GENERATED TRUCKEE RIVER FLOWS AS INPUT.	
DIMENSION ELIA(165), ELFA(165), OBSELA(165), DIFFA(165)	TRE00010
DIMENSION WSFLUA(165), VOLIA(165), VOLFA(165), AREAIA(165)	TRE00020
DIMENSION NDATE(165), TRFLD(165), HHD(165), SKY(165)	TRE00030
DIVENCION DUTILIES ADEAEALIES AELALIES	TREACOGO
DIMENSION TCANB(166), AVEFLO(12), AVEEVP(12), MONTH(12)	TREOCO50
DIMENSION AREAI(165,12), AREAF(166,12), OBSEL(166,12)	TRE00060
DIMENSION AEL(165,12), WSFL0(165,12), TCAN(165,12)	TREOCO70
DIMENSION VOLF(165,12), ELI(165,12), ELF(166,12)	TREJUJ80
NUMEROLDN DIFFILITE 121 VELITIE 121	TOFOOOO
DATA MONTH(1)/3HOCT/ MONTH(2)/3HNOV/ MONTH(3)/3HDEC/	TRE00100
DATA MONTH(4)/3HJAN/, MONTH(5)/3HFEB/, MONTH(6)/3HMAR/ DATA MONTH(7)/3HAPR/, MONTH(8)/3HMAY/, MONTH(9)/3HJUN/ DATA MONTH(10)/3HJUL/, MONTH(11)/3HAUG/, MONTH(12)/3HSEP/	TRE00110
DATA MONTH(7)/3HAPR/, MONTH(8)/3HMAY/, MONTH(9)/3HJUN/	TRE00120
DATA MONTH(1)/3HJUL/, MONTH(11)/3HAUG/, MONTH(12//3HSEP/	TRE00130
STRATIGENT, UNTIDING, AND ELLISING THE INFTICE AREAS VOLUME, AND	TRE00140
THE THE OF THE LEVE AT THE SECONDER OF THE MEATH LINE	TRE J 0150
VARIABLES FOR PYRAMID LAKE ELEVATION MODEL	TRE00160
	TREJUI70
EVAPORATION RATES ARE FROM HARDING, 1965.	TRE00180
	TRE 00190
PRECIPITATION RATES ARE AVERAGES FROM CLIMATOLOGICAL DATA.	TRE00200
AND	TRE0J210
TRFLO(J) = VOLUME OF INFLOW TO THE LAKE FOR THE YEAR.	TRE00220
	TRE00230
OBSEL(J,M) = THE ELEVATION AT THE MIDDLE OF THE M'TH MONTH AS	TRED0240
INTERPOLATED FROM THE RANDOM OBSERVED ELEVATIONS AS PUBLISHED	TRE00250
IN THE USGS PUBLICATIONS OF SURFACE WATER RECORDS.	TRED0260
ALL ELEVATIONS ARE CORRECTED TO THE SUPPLEMENTARY ADJUSTMENT OF	TRE00270
1956 FOR BENCH MARK N-21 AT ELEVATION 3940.29 FEET.	TRE00280
Average and the second s	TRE00290

5	AAREA = AVERAGE AREA FOR COMPUTING VOLUME OF EVAPORATION.	TRE00300 TRE00310
	AEL(J,M) = THE AVERAGE ELEVATION OF PYRAMID LAKE FOR COMPUTING	TRE00320
С	THE CROSS SECTIONAL AREA OF THE OUTFLOW CHANNEL AT WINNEMUCCA	TREOC330
C	SLOUGH.	TRE00340
C		TREDC350
C	XSECA = CROSS SECTIONAL AREA OF THE OUTFLOW CHANNEL AT	TRE00360
С	WINNEMUCCA SLOUGH.	TRE00370 TRE00380
С		TRE00380
С	K = HTORAULIC RADIOS OF THE DOTILOW CHANNEL AT MINICIPACIE	TREJC400
C	SLOUGH. THIS IS THE R OF MANNING'S EQUATION.	TRE00400
С		
С	WSFLU(J,M) = THE RATE, ACRE FEET PER YEAR, OF OUTFLOW THROUGH	TRE00430
C	WINNEMUCCA SLOUGH.	TRE00440
C	GAIN = INFLOW VOLUME LESS THE AVERAGE AREA (AAREA) TIMES THE	TRE00450
C	IN A INFINI VUIDAL LEGG THE ALLANCE ADEA TOATERS IN THE	TREDC460
C C	MUNTHS CPAPURATION LESS THE BYERLEONY NOT LOTOTING	TRE00470
C	AREAI(J,M), VOLI(J,M), AND ELI(J,M)=THE INITIAL AREA, VOLUME, AND	TRE00480
C	ELEVATION OF THE LAKE AT THE BEGINNING OF THE MONTH (THE	TRE00490
C	BEGINNING OF THE STEP).	TRE00500
c		TREOC510
č	AREAF(J,M), VOLF(J,M), AND ELF(J,M) = THE FINAL AREA, VOLUME, AND	TRE00520
C	FLEVATION AT THE MIDDLE OF THE NEXT MUNTH.	TREJC530
C	THIS IS THE RESULT OF THE AVERAGING OF THE LAKE AREA, AAREA(J,M)	TRE00540
С	TO GET THE EVAPORATION AND THE AVERAGE ELEVATION, AEL(J,M), TO	TREOC550
C	CALCULATE THE OUTFLOW RATE.	TRE00560
С		TRE00570
	AVEFLO(1)=0.0436	TRE00580
	AVEFLU(21-0.000)	TRE30590
	AVEF LUIS 1-0.0100	TREOC6CO
	AVEFLUTTI-0.0104	TRE00610
	AVEFEU() - 0.0001	TRE00620 TRE00630
	AVEFLO(6)=0.0924	I REDUCIDU

	AVEFLO(7)=0.1494	TRE00640
	AVEFLO(3)=0.1873	TRE00650
	AVEFL0(9)=0.1181	TRE00660
	AVEFL0(1)=0.0407	TREJC670
	AVEFLO(11)=0.0304	TRED0680
	AVEFLO(12)=0.0357	TREOC690
	AVEEVP(1)=0.41	TREDCTOD
	AVEEVP(2)=0.31	TREJ0710
	AVEEVP(3)=0.22	TRE00720
	AVEEVP(4)=0.17	TREOC730
	AVEEVP(5)=0.16	TREDC740
	AVEEVP(6)=0.15	TRE00750
		TREOC760
	AVEEVP(7)=0.18	TRE00770
	AVEEVP(8)=0.20	TRE00780
	AVEE VP (9)=0.26	TRE00790
	AVEEVP(10)=0.36	
	AVEEVP(11)=J.47	TRE00810
~	AVEEVP(12)=0.48	TREJC820
C	SEAD THE MUNICE OF DATA DOINTS	TREDC830
	READ THE NUMBER OF DATA POINTS.	TRE00840
	DATE THE TERKS HOST RE MUNERS OF DATA POINTS, SHO THREE SEED	TREOC850
	READ(5,300) NDATA	TRED0860
3000	FORMAT(I4)	TRE00870
C	THE THE ALLS DITL FOR HIL VELOC TUCK THE ODGEDUED	
С	READ THE TREE-RING DATA FOR ALL YEARS THEN THE OBSERVED	TREOC880
С	PYRAMID LAKE ELEVATIONS FOR ALL YEARS.	TREDC890
С		TRE00900
	READ(5,4)(NDATE(J), HHD(J), SKY(J), PNT(J), J=1, NDATA)	TREOC910
4	FURMAT(I4,3F7.0/)	TRE00920
	READ (5,6) ((DBSEL (J, M), M=1, 12), J=1, NDATA)	TRE00930
6	FORMAT(4X,12F6.2)	
С	IVG. 11 NEALASI C.	TRE00950
C	READ THE TRUCKEE CANAL FLOWS (TCAN(J,M)) IF APPLICABLE.	TRE 0 09 60
C	REMOVE THE C IN THE NEXT TWO CARDS AND PUT A C IN FRONT OF	TRE00970

and the second second

000000	THE TCAN(J,M) = -0.0 BELOW. READ(5,7) ((TCAN(J,M),M=1,12),J=1,NDATA) 7 FORMAT(4X,12F6.0)	TRE 00980 TRE0 0990 TRE0 1000 TRE0 1010 TRE0 1010 TRE0 1030
c	READ AND WRITE THE CODE NUMBER AND SIGN THAT INDICATES THE	TRE01040
C	THE DIRECTION THE COMPUTATIONS TAKE PLACE. A NEGATIVE 1	TRE01050
С	MEANS THE COMPUTATIONS ARE BACKWARD WITH RESPECT TO TIME.	TRE01060
C	A POSITIVE 1 MEANS THEY ARE FORWARD WITH RESPECT TO TIME.	TRED1070
C		TRE01080
	READ (5,5) KNOCK	TRE01090
	5 FURMAT(I2)	TRE01100
C		TRE01110
С		TRE01120
	WRITE(6,3001) KNOCK	TRE01130
300		ISTREJ1140
	1 BACKWARD WITH RESPECT TO TIME. A +1 IS FORWARD IN TIME.)	TRE01150
С		TRE 01160
С		TRE01170
С	READ THE YEAR, MONTH, NUMBER OF DATA POINTS, AND THREE SEED	TRE01180
C	VALUES (ELEVATION, AREA, AND VOLUME).	TRE01190
C		TRE01200
C		TRE01210
	WRITE(6,1)	TRE01220
~	1 FORMAT(1H0,46HDATE MONTH NDATA SEED EL SEED AREA SEED VOL)	TRE01230
С		TRE0 1240
	IF (KNUCK.EQ1) GO TO 3040	TRE01250
~	IF (KNOCK.EQ.+1) GO TO 3041	TRE01260
C	A READIC ANNOLTE UNENTH NEATA FLITANDATA 101 LOCAL MOATA 101	TRE01270
304	O READ(5,2)MDATE, MMCNTH, NDATA, ELI(NDATA, 12), AREAI (NDATA, 12),	TRE01280
	1VOLI(NDATA,12)	TRE01290
	2 FORMATII4, 1X, A3, I4, 1X, F7-2, 1X, F7-0, 3X, F9-0)	TRE01300
	WRITE(6,3) MDATE, MMONTH, NDATA, ELI(NDATA, 12), AREAI (NDATA, 12),	TRE01310

	IVOLI (NDATA, 12)	TRE01320
	FURMAT(1H , 14, 3X, A3, 1X, 14, 3X, F7. 2, 3X, F7. 0, 1X, F9. 0)	TRE01330
2		TRE01340
	GO TO 3042	TRE01350
3041	READ (5,2) MDA TE, MMONTH, NDATA, ELI(1,1), AREAI(1,1), VOL I(1,1)	
	WRITE(6,3) MDATE, MMONTH, NDATA, ELI(1,1), AREAI(1,1), VOL1(1,1)	TRE01360
3042	CONTINUE	TRE01370
C		TRE01380
	IDATA = NDATA + 1	TRE01390
	DJ 1001 J1=1, NDATA	TRE01400
	$IF(KNOCK \cdot EQ \cdot -1) J = IDATA - J1$	TRE01410
	$IF(KNOCK \cdot EQ \cdot +1) J = J1$	TRE01420
	T1=0.0	TRE0143J
	T2= 0.0	TRE01440
	T3=0.0	TRE01450
С		TRE01460
2	AREAFA(J) = -0.0	TRE01470
	AREA[A(J) = -0.0	TRE01480
	VOLFA(J) = -0.0	TRE01490
	VOLIA(J) = -0.0	TRE01500
	WSFLOA(J) = -0.0	TRE01510
	AcLA(J) = -0.0	TREJ1520
	DIFFA(J) = -0.0	TRE01530
+ 31	OBSELA(J) = -0.0	TRE01540
	ELFA(J) = -0.0	TRE01550
	ELIA(J) = -0.0	TRE01560
	TCANB(J) = -0.0	TRE01570
С		TRE01580
c		TRE01590
	IF NOTYPE = +1 ALL OF THE WRITE STATEMENTS BETWEEN HERE AND THE	TRE01600
č	MONTHLY SUMMARY TABLE ARE DELEATED. IF ANYTHING ELSE ALL OF	TREJ1610
C	THE TABLES WILL PRINT.	TRE01620
c		TRE01630
C		TRE01640
U III	READ (5,9) NOTYPE	TRE01650
	NEAD (ST) / NOTITE	

	9 FORMAT(I2)	TRE01660
С	WRITE(6,11) NCTYPE	TRE01670 TRE01680
	11 FORMAT(1X,14,2X, 'A POSITIVE ONE MEANS ALL INTERNAL PRINTS	TRE01690
	*FOR CHECKING FOR PERFORMANCE OF THE MODEL ARE DELETED. ',/,	TRE01700
	*7X, 'IF ANYTHING ELSE ALL WILL PRINT.')	TRE01710
С		TRE0 17 20
C		
C	TRUCKEE RIVER USINT TREE-RINGS FOR INPUT.	TRE01740
C		TREJ1750 TRE01760
С	10 TRFLO(J)=(((PNT(J)*(.92550))+ 10.62960)/100.)*506570.	TRED1780
С		TREO1780
~	LSTUP = 0	TRE01790
С		TRE01800
С		TRE0 1810
	IF(NUTYPE.NE.+1) GO TO 400	TRE01820
С		TRE01830
С		TRE01840
	WRITE(6,20) 20 FORMAT(1H0,44HDATE HHD(J) SKY(J) PNT(J) TRFLO(J))	TRE01850 TRE01860
	WRITE(6, 21)NDATE(J), HHD(J), SKY(J), FNT(J), TRFLD(J)	TRE0 1800
	21 FORMAT(1H , 14,4F10.0)	TRE01880
	WRITE(6,22)	TRE0 1890
	22 FORMAT(1H ,124HDATE OCT NOV DEC JAN	TRE01900
	1FEB MAR APR MAY JUN JUL AUG	TRE01910
	2 SEP)	TRE0 1920
	WRITE(6,23)NDATE(J), (OBSEL(J,M), M=1,12)	TRE01930
c	23 FORMAT(1H ,14,12F10.2)	TRE01940
CC		TRE01950 TRE01960
C		TRE01970
0	400 CONTINUE	TRE0 1980
С		TRE01990

DO 1002 M1 = 1,12 IF(KNOCK.EQ1) M = 13 - M1 IF(KNOCK.EQ.+1) M = M1 C	TRE02000 TRE02010 TRE02020 TRE02030
2002 AAREA=AREAI(J.M)	TRE02040
C IFTAAREA.GT. 76600AMD_AMAEALLE. 878301. GD TO 107	TRE02050
C PUT A C IN FRONT OF THE NEXT CARD IF TCAN FLOWS ARE PRESENT	TRE02060
C LE TRIALREA. ST. 94900 M. C. ANG. 4. LE. 103733. F 50 TO 139	TRE02070
TCAN(J,M) = -0.0	TRE02080
C IFINAREA. ST., LEISCOLLAND. AM EALLE. LIFCOD. F SU TO ALL	TRE02090
C SIFTAREA. GT. TIENOS, WRD FAREA. LE. 129100.1 GO TO 112 05 00 00 00 00 00 00 00 00 00 00 00 00	TRE02100
IF(NOTYPE.NE.+1) GO TO 401	TRE02110
C LEFT KAR EXCELLAYDOU TARDIAAR EALE IN 2420.2 GU. TU 119	TRE02120
WRITE(6,24)	TRE02130
24 FORMAT(1H0,117H GAIN= (TRFLO* %FLO) -TCAN - (AAREA* %E) -W.	SFTRE02140
1LO INIT VOL FIN VOL I AREA FAREA AVE EL XSECA	RTRE02150
2)	TRE02160
C 102 MILLING V 30178571024REA+3458.43	TRE02170
401 CONTINUE	TRE02180
C Las all and a sub- and a some all and	TRE02190
C	TRE02200
C 104 And Cable LOBS29670FRAKEAR3419-29	TRE02210
C ASSUME AVERAGE AREA IS EQUAL TO AREAI(J,M) TO START, THEN ITTERA	
C ID MINIMIZE THE DIFFERENCE BETWEEN AAREA AND THE TRUE AVERAGE	
	TRE02240
C FRUM THE AVERAGE AREA FOR EACH PASS CALCULATE THE AVERAGE	
C ELEVATION , AEL(J, M), SO THAT THE OVERFLOW RATE CAN BE CALCULATED	TRE02260
C USING MANNING'S EQUATION.	TRE 02270
C	TRE02280
C 105 ALL CI.MLA 30+22535064 / 42+3325-01	TRE02290
100 LSTOP = LSTOP + 1	TRE02300
C LUS ABLILINALS DUASADAS TAAREA 3298.04	TRE02310
IF (AAREA.GT. OAND.AAREA.LE. 11800.) GO TO 101	IKE02330

110 109 105 103 102 108 107 106 104 101 AEL AEL(J,M) =AEL AEL AEL AEL AEL AEL GO AEL(J,M) =60 GU AEL(J,M) =GU GU GU 60 60 60 IF (AAR EA. GT. 1443JJ.) GO TO 116 TI IF (AAR EA . GT . 142400 . . AND . AAR EA. LE. 144300.) IF(AAREA . GT. 139000 . . AND . AAREA . LE. 142400 . . IFLAAREA.GT. IF (AAR EA. GT. IF (A AR EA . GT . IFLAAREA.GT. IF (AAR EA. GT. 111300. . AND. AAR EA. LE. 118600. IF(AAREA.GT.103700. AND. AAREA.LE.111300. IFLAAREA.GT. IFLAAREA.GT. IF LAAR EA. GT. IFLAAREA.GT. T (AAREA. GT. 129100. AND . AAREA. LE. 139000. TC 199 AAREA.GT.118600. AND. AAREA.LE.129100. 10 (J, M) = TO = (M + L) 10 [J, M) = 10 (J, M) = 10 (J ; M) = (J + M) = TO TO 199 199 661 199 199 661 199 199 .00177966*AAREA+3459.00 .00394737*AAREA+3360.66 .00247934*AAREA+3442.98 . J0178571*AAREA+3458.93 .00422535*AAREA+3329.01 .JJJ335196*AAREA+3411.84 .00329670*AAREA+3414.29 .00454545*AAREA+3298.64 -00275862*AAREA+3448-69 00357143*AAR EA+3386 .43 9490J . AND . AAR EA . LE . 10370 J . 62100 . . AND . AAR EA. LE. 4420J . AND. AAR EA.LE. 11800 . AND . AAR EA .L 87800 .. AND . AAR EA . LE . 76600 . AND . AAREA.LE. 35100 . AND AAREA LE 23000 . . AND . AAR EA. LE 94900. 62100. 35100. 87833.1 44200. 76600. 23000-GO 60 60 50 GO 60 GU GU GO 10 TO TO TO TO TO TU 10 TO TO TO 10 TO 115 113 114 112 111 110 109 108 107 106 105 104 103 102 TRE02670 TRE02660 TRE0 2650 TRE02640 TRE0 2630 TRE02620 TRE 02610 TRE0 2560 TRE02530 TRE02600 TRE02590 TRE0 25 80 TRE02570 TRE 0 2550 TRE02540 TRE 0 2520 TRE0 2510 TRE0 2500 TRE02490 TRE02480 TRE02470 TRE02460 TRE02450 TRE02440 TRE J243 0 TRE02420 TRE02410 TRE02400 TRE 0 23 9 0 TRE02360 TRE02380 TRE02370 TRE0 23 50 TRE 0234 0

	GO TO 199	TRE02680
111	AEL(J,M)= .00273973*AAREA+3495.07	TRE02690
	CO TO 100	TRE02700
112	AEL(J, M) = .00190476*AAREA+3594.10	TRE02710
	GU TO 199	TRE02720
113	AEL(J,M) = .00202020*AAREA+3579.19	TRE02730
**2	GO TO 199	TRE02740
114	AEL(J,M)= .00294118*AAREA+3451.18	TRE02750
114	GO TO 199	TRE02760
115	AEL(J, M)= .00526316*AAREA+3120.53	TRE02770
115	GO TO 199	TRE02780
116	AEL(J,M) = .00202020*AAREA+3588.49	TREJ2790
	WSFL0(J,M)=-0.0	TRE02800
	XSECA=-0.0	TRE02810
	R=-0.0	TRE02820
	IF(NDATE(J).EQ.1891.OR.NDATE(J).EQ.1890) GO TO 34	TRE02830
	A10 = 3863.25	TRE02840
	A11 = 3868.25	TRE02850
	A12 = 3973.25	TRE02860
	IF(AEL(J,M).LE. A10) GO TO 34	TRE02870
		TRE02880
		TRE02890
	IF(AEL(J,M).GT. A12) GO TO 32	TRE02900
30	XSECA=(AEL(J,M)- A10)*60.	TRE02910
50	R=2.*(AEL(J,M)-A10)+60.	TRE02920
	GU FO 33	TRE02930
31		TRE02940
	R=70.+(AEL(J,M)- A11)+SQRT(((AEL(J,M)- A11)**2)+((AEL(J,M)	TRE02950
	1- A11)/0.035714)**2)	TRE02960
	GO TO 33	TRE02970
32	XSECA=(AEL(J,M)- A10)*(60.+140.)+350.00+600.00+(.5*((AEL(J,M)-	
		TRE02990
	R=215.09+SQRT(((AEL(J,M)- A12)**2)+((AEL(J,M)- A12)/0.22727)	TRE03000
	1**2)+SQRT(((AEL(J,M)- A12)**2)+((AEL(J,M)- A12)/0.035714)**2	

	1	1) 1 1 KOLFIJ+K) - GT 470300 - AND - VOLFIJ-MJ -LE- 1330000 -1 60 10 204	TRE03020
C			TRE03030
		WSFLD(J,M)=(1.98)*(XSECA)*(1.49)*(0.7)*(SQRT(0.0003367))*	TRE03040
	1	1(R**0.6666666667)*(30.)	TRE03050
С			TRE03060
С			TRE03070
С		CALCULATE THE GAIN TO THE LAKE, FROM THE TRUCKEE FLOW TIMES	TRE03080
C		THE AVERAGE PERCENT INFLOW FOR THE MONTH, LESS THE AMOUNT	TRE0309C
С		OF EXPORTED WATER, TCAN(J,M) (IF ANY), LESS THE AVERAGE AREA	TRE03100
С		(AAREA), TIMES THE AVERAGE RATE OF EVAPORATION PER ACRE, LESS THE	TRE03110
С		CALCULATED OVERFLOW INTO WINNEMUCCA LAKE, IF ANY.	TRE03120
C		HEAVER PLUTALER, 291 DUEDULA, NEAVING CATALIAN SUBJECTION AND THE STREET	TRE03130
	34	GAIN=TRFLO(J) * AVEFLO(M) - TCAN(J, M) - AAREA* AVEEVP(M) - WSFLO(J, M)	TRE03140
С			TRE03150
		IF (KNOCK.EQ1) GO TO 35	TRE03160
		IF (KNOCK.EQ.+1) GU TO 36	TRE03170
	35	CONTINUE	TRE03180
	36	CONTINUE	TRE03190
	37	CONTINUE	TRE03200
С			TRE03210
С			TRE03220
		IF (KNOCK.EQ1) GO TO 2010	TRE03230
		IF (KNOCK.EQ.+1) GO TO 2011	TRE03240
1	2010	VOLF(J,M) = VOLI(J,M) - GAIN	TRE03250
		GO TO 2012	TRE03260
	2011	VOLF(J,M) = VOLI(J,M) + GAIN	TRE03270
С		7 . Here's (diminist a subject of the subject and the subject as a subject of the	TRE03280
С			TRE 03290
С		COMPUTE AREA AFTER INFLOW USING THE LINEAR EQUATIONS THAT	TRE03300
С		APPROXIMATE THE AREA-CAPITITY CURVE.	TRE03310
С			TRE03320
С			TRE 03330
1	2012	IF(VCLF(J,M).GT. 0AND.VOLF(J,M).LE. 121000.) GU TO 201	TRE03340
		IF(VOLF(J,M).GT. 121000AND.VOLF(J,M).LE. 470000.) GO TO 202	TRE03350

IF(VOLF(J,M).GT. 470000..AND.VOLF(J,M).LE. 1350000.) GO TO 203 TRE03360 IF(VOLF(J,M).GT. 1350000..AND.VOLF(J,M).LE. 2550000.) GD TO 204 TRE03370 IF(VOLF(J,M).GT. 2550000..AND.VOLF(J,M).LE. 5720000.) GU TO 205 TRE03380 IF(VOLF(J,M).GT. 5720000..AND.VCLF(J,M).LE. 8490000.) GO TO 206 TRE03390 IF(VOLF(J, M).GT. 8490000..AND.VOLF(J, M).LE.11790000.) GO TO 207 TRE03400 IF(VOLF(J,M).GT.11790000..AND.VOLF(J,M).LE.14530000.) GO TO 208 TRE03410 IF(VOLF(J,M).GT.14530000..AND.VOLF(J,M).LE.18500000.) GU TO 209 **TRE0 3420** IFIVJLF[J, MJ.GT.18500000..ANE.VOLF(J, M).LE.21720000.) GO TO 210 TRE03430 IF(VCLF(J,M).GT.21720000..AND.VOLF(J,M).LE.24020000.) GO TO 211 **TRE03440** IF(VCLF(J,M).GT.24)20000..AND.VOLF(J,M).LE.26490000.) GO TO 212 TRE03450 IF(VOLF(J,M).GT.26490000..AND.VOLF(J,M).LE.29180000.) GD TO 213 TRE03460 1F(VOLF(J,M).GT.29180000..AND.VOLF(J,M).LE.30580000.) GU TO 214 TRE03470 IF(VOLF(J,M).GT.30580000..AND.VOLF(J,M).LE.32020000.) GU TO 215 TRE03480 TRE J3490 IF(VCLF(J,M).GT.32J20000.) GU TO 216 201 AREAF(J,M)=((.00017355372*VOLF(J,M))+3459.00-3459.00)/ .00177966 TRE03500 TRE03510 GO TO 299 202 AREAF(J, M)=((.00005730659*VOLF(J, M))+3473.07-3458.93)/ .00178571 TRE03520 TRE03530 GO TO 299 203 AREAF(J,M)=((.)0003409091*VOLF(J,M))+3483.98-3442.98)/ .00247934 TRE03540 TRE03550 GO TO 299 204 AREAF(J,M)=((.)00002500000*VOLF(J,M))+3496.25-3414.29)/ .)0329670 TRE03560 TRE03570 GU TO 299 205 AREAF(J, M)=((.J0001892744*VOLF(J,M))+3511.74-3411.84)/ .J0335196 TREJ3580 TRE03590 GO TO 299 206 AREAF(J,M)=((.00001444043*VOLF(J,M))+3537.40-3448.69)/ .00275862 TRE03600 TRE03610 GO TO 299 207 AREAF(J,M)=((.00001212121*V0LF(J,M))+3557.09-3386.43)/ .00357143 TRF03620 TRE03630 GO TO 299 208 AREAF(J, M)=((.00001094891*VOLF(J, M))+3570.91-3329.01)/ .00422535 TRE03640 TRE03650 GO TO 299 209 AREAF(J, M)=((.00001007557*VOLF(J, M))+3583.60-3298.64)/ .00454545 TRE03660 TRE 03670 GU TC 299 210 AREAF(J,M)=(1 .00000931677*VOLF(J,M))+3597.64-3360.66)/ .00394737 TRE03680 TRE03690 GO TO 299

	211		.00000869565*VOLF(J,M))+3611.13-3495.07)/ .00273973	TRE03700 TRE03710
		GO TO 299		
	212	AREAF(J, M) = ((.00000809717*VOLF(J,M))+3625.51-3594.10)/ .00190476	TREUSIZU
		GO TO 299		TRE03730
	213	AREAF(J, M) = ((.00000743494# VOLF(J,M))+3643.05-3579.19)/ .00202020	TRE03740
		GO TO 299		TRE03750
	214	AREAF(J,M)=((.00000714286*V0LF(J,M))+3651.57-3451.18)/ .00294118	TRE03760
		GO TO 299		TRE03770
	215	AREAF(J, M)=((.00000694444*V0LF(J,M))+3657.64-3120.53)/ .00526316	TRE03780
		GO TO 299		TRE03790
	216	AREAF(J, M) = ((.00000743494* VOLF(J, M))+3641.93-3588.491/ .00202020	TRE03800
		CONTINUE		TREJ3810
С				TRE03820
C				TRE 03830
C				TRE03840
-		IFINOTYPE.NE.+		TRE03850
C		War benke state		TRE03860
-		WRITE(6.51)GAI	N, TRFLD(J), AVEFLO(M), TCAN(J,M), AAREA, AVEEVP(M), WSFL	CTRE0 3870
		I(J.M) . VOLI(J.M	1), VOLF(J,M), AREAI(J,M), AREAF(J,M), AEL(J,M), XSECA, R	TRE0388C
	51	FORMAT(1H .2F9	0.0, F6.4, F7.0, F9.0, F4.2, F9.0, 2F10.0, 2F9.0, F8.2, 2F9.2)TRE03890
С				TRE03900
~	402	CONTINUE		TRE03910
С		John and		TRE 03920
č				TRE03930
č				TRE03940
č		CHECK TO SEE I	F THE AVERAGE AREA, AAREA, FROM THE LAST PASS	TRE03950
č			AL TO THE AVERAGE AREA, AAREA, FOR THE NEW PASS.	TRE03960
c			THE AVERAGE AREA, AAREA, TO THE TRUE MEAN,	TRE03970
c			O 100 AND CALCULATE THE EVAPORATION LOSS AND THE	TRE03980
č			DUGH OUTFLOW AGAIN.	TRE03990
C			ARLY EQUAL TO ITSELF PROCEED.	TRE04000
C		HINGIN IT IS NUM		TRE 04010
c				TRE04020
C		IE (I STOP GE.)	LOI GO TO 52	TRE04030
		1. 120101-002-1		SYNERIA CARENT

C C			TRE04040 TRE04050
C		B = ABS(AAREA-((AREAF(J,M)+AREAI(J,M))/2.))	TRE04060
		IF(8.LT.0.45) GD TO 52	TRE04070
		AAREA = (AREAF(J, M) + AREAI(J, M))/2.	TRE04080
		GO TO 100 TO 100 CONTRACTOR CONTRACTOR CONTRACTOR CONTRACTOR STORE	TRE04090
	52	CONTINUE CONTINUE CONTINUES OF A CON	TRE04100
С			TRE04110
		IF(WSFLO(J,M).EQU.0) GO TO 55	TRE0 4120
		T3=T3+1.0	TRE04130
	55	CONTINUE	TRE04140
C			TRE 04150
С			TRE04160
С			TRE04170
		IF (NOTYPE.NE.+1) GO TO 403	TREJ4180
С		HOLTELL FOR	TRE04190 TRE042CC
	52	WRITE(6,53) FORMAT(1H ,50H FIN AAREA F AVE EL FIN XSECA FINAL R FIN WSFLO)	TRE04200
	23	FORMAT(1H ,50H FIN AAREA F AVE EL FIN XSECA FINAL R FIN WSFLO) WRITE(6,54)AAREA,AEL(J,M),XSECA,R,WSFLO(J,M)	TRE04220
	54	FURMAT(1H ,F10.0,3F10.2,F10.0)	TRE04230
С	24	- SKMATTIN 110-0151 10-211 10-01	TRE04240
C	403	CONTINUE	TRE04250
С	TUJ	CONTINUE	TRE04260
c			TRE04270
č			TRE04280
Č		AFTER CORRECT "AAREA" IS DETERMINED, COMPUTE ELEVATION FROM	TRE04290
c		THE EQUATIONS THAT APPROXIMATE THE ELEVATION -VOLUME CURVE.	TRE04300
C			TRE04310
		IF(VOLF(J,M).GT. 0AND.VOLF(J,M).LE. 121000.) GO TO 301	TRE04320
		IF(VOLF(J,M).GT. 121000AND.VOLF(J,M).LE. 470000.) GU TO 302	TREJ433J
		IF(VOLF(J,M).GT. 470000AND.VOLF(J,M).LE. 1350000.) GU TO 303	TRE04340
		IF(VCLF(J,M).GT. 1350000AND.VOLF(J,M).LE. 2550000.) GD TO 304	TRE04350
		IF (VOLF(J,M).GT. 2550000AND.VOLF(J,M).LE. 5720000.) GO TO 305	TR E04360
		IF(VOLF(J,M).GT. 5720000.AND.VOLF(J,M).LE. 8490000.) GO TO 306	TRE04370

	IF(VCLF(J,M).GT. 8490000AND.VOLF(J,M).LE.11790000.1 GO TO 307	TRE04380
	IF(VOLF(J,M).GT.11790000AND.VOLF(J,M).LE.14530000.) GO TO 308	TRE04390
	IF(VOLF(J,M).GT.14530000ANC.VOLF(J,M).LE.18500000.1 GO TO 309	TRE04400
	IF (VOLF(J,M).GT. 18500000. AND. VOLF(J,M).LE. 2172000.) GO TO 310	TRE04410
	IF(VOLF(J, M).GT.21720000. AND.VOLF(J, M).LE.24020000.) GU TU 311	TRE04420
	IF(VOLF(J,M).GT.24020000AND.VOLF(J,M).LE.26490000.) GO TO 312	TRE04430
	IF(VOLF(J,M).GT.26490000AND.VOLF(J,M).LE.29180000.) GO TO 313	TRE04440
	IF(VOLF(J,M).GT.29180000AND.VOLF(J,M).LE.30580000.1 GO TO 314	TRE04450
	IF(VOLF(J,M).GT.30580000AND.VOLF(J,M).LE.32020000.) GO TO 315	TREU4460
	IF (VOLF(J,M).GT.32020000.) GO TO 316	TRE04470
301	ELF(J,M)= .00017355372*V0LF(J,M)+3459.00	TRE04480
201	GG TO 399	TRE04490
302	ELF(J,M)= .00005730659*VOLF(J,M)+3473.07	TRE04500
392	GO TO 399	TRE04510
303	ELF(J,M) = .00003409091*V0LF(J,M)+3483.98	TRE04520
202	GO TO 399	TRE04530
304	ELF(J,M) = .00002500000*V0LF(J,M)+3496.25	TRE04540
201	GD TO 399	TRE04550
305	ELF(J,M) = .00001892744*V0LF(J,M)+3511.74	TRE04560
202	GU TO 399	TRE04570
3.16	ELF(J,M)= .00001444043*V0LF(J,M)+3537.40	TRE04580
200	GU TO 399	TRE04590
307	ELF(J,M)= .0000121212121*VOLF(J,M)+3557.09	TRE 0 4600
501	GO TO 399	TRE04610
308	ELF(J,M)= .00001094891*V0LF(J,M)+357J.91	TRE04620
500	GO TO 399	TRE04630
309	ELF(J,M)= .00001007557*VOLF(J,M)+3583.60	TRE04640
50.	GU TO 399	TRE04650
310	ELF(J,M)= .00000931677*V0LF(J,M)+3597.64	TRE04660
210	GU TO 399	TRE04670
311	ELF(J,M)= .00000869565*V0LF(J,M)+3611.13	TRE04680
JII	GD TO 399	TRE04690
312	ELF(J,M)= .00000809717*V0LF(J,M)+3625.51	TRE047CO
212	GU TO 399	TRE04710

	313		TRE04720
		GI TO 399	TRE0 4730
	314		TRE04740
		GU TO 399	TRE04750
	315		TRE04760
		GO TO 399	TRE04770
	316		TRE04780
		CONTINUE	TRE04790
С		CONTINUE	TRE04800
C			TRE04810
C		CALCULATE THE DIFFERENCE IN THE OBSERVED AND CALCULATED ELEVATION	TRE0 48 20
С			1KE04830
			TRE04840
		IF IUBSELIJIMI.LE.U.I GU IU IU	TRE04850
		DIFE(J,M)=FIF(J,M)-OBSEL(J,M)	TRE04860
	70		TREJ4870
		T TORE T NET	TRE04880 TRE04890
С			TRE04890
		IFIDIFFIJ,MJ.EQ. O.O. GD TO GO	TRE04900
		12=12+1.0	TRE04910
	60		TRE04920
С			TRE04940
C			TRE04950
С			TRE04960
		ELIALUI-ELIALUITELI (UTI)	TRE04970
		ELFA(J) = ELFA(J) + ELF(J,M)	TRE04980
		UBSELATUI-UBSELATUI UBSELTUITT	TRE04990
		DIFFALST-DIFFALST DIFF LOTIO	TREJ5000
		ACLAIDI-ACLAIDIFACLIDINI	TRE05010
		ICANDIJI-ICANDIJI I CANTOJIN	TRE05020
			TREUSUSU
			TRE05040
		AREAIA(J) = AREAIA(J) + AREAI(J,M)	TRE05050
		AUCHTATOT-AUCHTATOTTATCATOTTA	

		AREAFA(J) = AREAFA(J) + AREAF(J,M)	TRE05060
c		AREATAIST-AREATAIST AREATION	TRE0 50 70
C			TRE05080
С		IF(NOTYPE.NE.+1) GO TO 404	TRE0 50 90
~		IFINUITE NE TIT OU TO TOT	TRE05100
С		UDITE(/ 71)	TRE05110
	71	WRITE(6,71) FORMAT(1H,124HDATE MON INIT EL (FIN EL-OBS EL)=DIFF EL AVE EL	TRE05120
		A THE CAR OUT THIT YOU FIN VOL I ADEA	
			TRE05140
	4	2 F AREA) WRITE(6,72)NDATE(J),MONTH(M),ELI(J,M),ELF(J,M),OBSEL(J,M),DIFF(J,M)	
		AEL(J,M), TR FLO(J), TC AN(J,M), WSFLO(J,M), HHD(J), SKY(J), PNT(J), V(TRE0 5160
	-	2LI(J,M), VOLF (J,M), AREAI(J,M), AREAF(J,M)	TRE05170
		FORM AT (1H , 14, 1X, A3, 1X, 5F8. 2, 3F8. 0, 3F5. 0, 2F10. 0, 2F8. 0)	TRE05180
-	12	FURM AT (IH , 14, 1X, A3, 1X, 5F0. 2, 5F0. 0, 5F5. 5. 0, 27 10. 0, 27 0.00	TRE05190
С	101	CONTINUE	TRE05200
	404	CONTINUE	TRE05210
C			TRE05220
C C			TRE05230
5		SET INITIAL VALUES FOR THE NEXT PASS	TRE05240
5		SET INITIAL VALUES FUR THE NEXT FASS	TRE0 52 50
С		IF (KNOCK.EQ1) GO TO 2030	TRE05260
		IF (KNOCK.EQ.+1) GO TO 2031	TRE05270
-	02.0	IF (J.EQ.1. AND.M.EQ.1) GU TO 73	TRE05280
2	030	MA = M - 1	TRE05290
		JA = J	TRE05300
		IF (MA.EQ.0) GO TO 500	TRE05310
		GO TO 73	TRE 05320
	500	MA = 12	TRE05330
	200	JA = J - 1	TRE05340
		GO TO 73	TRE05350
	021	IF(J.EQ.NDATA.AND.M.EQ.12) GO TO 73	TRE05360
4	.051	MA = M + 1	TRE05370
		AA = J	TRE05380
		IF(MA.LT.13) GO TO 73	TRE05390
		TITUMERIETAL OD ID ID	

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	THINK PLANT	DDENNIX E CONTINUED

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	100				000	00		C	502			601	600							2001	2000		73		
WRITE THE MONTHLY SUMMARY TABLE.	AREAFA(J)=AREAFA(J)/12. CUNTINUE			AELA(J)=AELA(J)/12.	ELFA(J)=ELFA(J)/12.	WSFLDA(J)=WSFLDA(J)/T3	GU TU 605	WSEI DA(J)=-0.0	DIFFAUJEUIFFAUJEC TELTS CT 0.01 CO TO 604		DIFFA(J)=-0.0	IF(12.61.0.0) GO TC 602	OBSELA(J)=OBSELA(J)/T1			F	SUMMARI INDEE.	HEARY OF THE IS CONTROL TO THE T	CALCHLATE THE MEANS OF THE 12 MENTHLY VALUES FOR THE YEARLY	CUNITING	CONTINUE		VOLI (JA, MA)=VULFIJ, MJ	JA = J + 1	MA = 1
TRE05710 TRE05720 TRE05730	E0569	E0567	E0 566	E0564	E0563	E0562	E0560	E0559	E0 55 8	E0 55 7		100004	0553	EU 552	0551	E0550	E0549	0548	E0547	E 0546	20545	E0544	E0 54 3	C0 54 2	

C.		TRE05740
L	WRITE(6,78)	TRE0 5750
	78 FURMAT(1HS)	TRE05760
	WRITE(6,74)	TRE05770
	74 FURMAT (1H1, 35HSUMMARY TABLE GIVING MONTHLY VALUES)	TRE05780
	WRITE(6,75)	TRE05790
	75 FORMAT(1H0,124HDATE MON INIT EL (FIN EL-OBS EL)=DIFF EL AVE EL	TRE05800
	1 TRELO TCAN WEFLO HHD SKY PNT INIT VOL FIN VOL I AREA	TRE05810
	2 F AREA)	TRE05820
~	2 F AREA	TRE05830
C	T5=0.0	TRE05840
	SDIFF = -0.0	TRE 05850
c	SUIFF0.0	TRE05860
C		TRE05870
C	DO 1003 J1=1,NDATA	TRE05880
	I = I = I = I = I = I = I = I = I = I =	TRE05890
		TRE0 5900
	$1 + 1 \times 100 \times . = 0.+17 = 31$ DO 1003 M1 = 1,12	TRE05910
	IF(KNOCK.EQ1) M = 13 - M1	TRE05920
	$IF(KNOCK \cdot EQ \cdot +1) M = M1$	TRE0 5930
C	In the bolt of a company of the second sec	TRE05940
v	SDIFF=DIFF(J,M)+SDIFF	TRE05950
	1F(D1FF(J,M).EQ0.0) GO TO 79	TRE05960
	T5=T5+1.0	TRE05970
	79 CONTINUE DEL CONTA	TRE05980
С		TRE05990
	WRITE(6,72)NDATE(J), MONTH(M), ELI(J,M), ELF(J,M), OBSEL(J,M), DIFF(J,	MIREOGUUU
	1), AEL(J,M), TRFLO(J), TCAN(J,M), WSFLO(J,M), HHD(J), SKY(J), PNI(J), V	UIREUOUIU
	2LIIJ, M), VOLF (J, M), AREAI (J, M), AREAF (J, M)	IKE UOUZU
1	1003 CONTINUE	TRE06030
С		TRE06040 TRE06050
	IF(15.GT. 0.0) GO TO 650	TRE06060
	INTEE 2.0	
	GU TO 651	INCOUTO

	650 ADIFF=SDIFF/T5 651 CONTINUE	TRE06080 TRE06090
C		TRE06100
	WRITE(6,76)SDIFF	TRE06110
	76 FORMAT(1H0, 10X, 21HSUM OF THE DIFFERENCE, 3X, F8.2)	TRE06120
	WRITE(6,77)ADIFF	TRE06130
	77 FORMAT(1H ,10X,22HMEAN OF THE DIFFERENCE, 2X, F8.2)	TRE06140
C		TRE06150
C		TRE06160
C		TREO6170
С		TRE0 61 80
C		TRE06190
	WRITE(6,80)	TRE06200
	8J FURMAT(1H1,53HSUMMARY TABLE GIVING YEARLY AVERAGES OF THE 12	MONTHTRE06210
	IS) and I HO, GR. ZINSUM OF THE DEFERENCE IN A STREET OF	TRE06220
	WRITE(6,81)	TRE06230
	81 FURMAT(1H ,119HDATE INIT EL (FIN EL-CBS EL)=DIFF EL AVE EL	
	10 TCAN WSFLO HHD SKY PNT INIT VOL FIN VOL I AREA	F ARTRE06250
	2EA)	TR E0 62 60
C C		TRE06270
С		TRE06280
	SDIFFA= -0.0	TRE06290
	T4=0.0	TRE06300
С		TRE06310
	DO 1004 J1=1, NDATA	TREJ6320
	$IF(KNOCK \cdot EQ \cdot -1) J = IDATA - J1$	TRE06330
	$IF(KNOCK \cdot EQ \cdot 1) J = J1$	TRE06340
C		TRE06350
	SDIFFA=DIFFA(J)+SDIFFA	TRE06360
	IF(DIFFA(J).EQ0.0) GO TO 85	TRE06370
	T4=T4+1.0	TRE0 6380
С	85 CONTINUE	TRE06390 TRE06400
C	WRITE(6,82)NDATE(J), ELIA(J), ELFA(J), OBSELA(J), DIFFA(J), AELA(.	
	WALLETO JOZINDATE (JIJELIA(JIJELFA(JIJOBSELA(JIJDIFFA(JIJAELA))	1) 1 1 1 K -

	ILO(J) ,TCANB(J),WSFLOA(J),HHD(J),SKY(J),PNT(J),VOLIA(J),VOLI	FA(J), ATRE06420
	IREAIA(J), AREAFA(J)	TRE0 6430
	FORMAT(1H ,14,5F8.2,3F8.0,3F5.0,2F10.0,2F8.0)	TRE06440
C		TRE06450
1004	CONTINUE	TRE06460
С		TRE06470
C		TRE06480
	IF(T4.GT.0.0) GO TO 670	TRE06490
	ADIFFA= -0.0	TRE 065 00
	GO TO 671	TRE06510
670		TRE06520
	CONTINUE	TREO£530
C		TRE06540
	WRITE(6,83)SDIFFA	TRE06550
83	FORMAT(1H0, 4X, 21HSUM OF THE DIFFERENCE, 3X, F8.2)	TRE06560
	WRITE(6,84)ADIFFA	TRE 36570
84	FORMAT(1H , 4X, 22HMEAN OF THE DIFFERENCE, 2X, F8.2)	TRE06580
	CUNTINUE	TRE 06590
	STOP	TRE06600
	END	TRE06610
		TRE06620

PLEASE NOTE:

Plate I., "Topographic Map of Truckee & Carson River Drainages", not microfilmed because it will not reproduce well in xerographic copies. Available for consultation at the University of Nevada Library.

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