

1 Characterising the geomorphological and physicochemical effects of water injection dredging
2 on estuarine systems.

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25 Abstract

26 Dredging is a globally important aquatic system management activity, used for navigation
27 improvement, contamination removal, aggregate production and/or flood risk mitigation.
28 Despite widespread application, understanding of the environmental effects of some dredging
29 types remains limited. Field campaigns in 2016 and 2017 in the River Parrett estuary, UK,
30 therefore investigated the geomorphic and physicochemical effects of Water Injection
31 Dredging (WID), a poorly studied hydrodynamic dredging technology. WID, applied to restore
32 channel capacity for the maintenance of flood water conveyance in the tidal River Parrett,
33 influenced surface elevations but not grain-size characteristics of dredged bed sediments.
34 Topographic alterations due to the 2016 WID operation were short-lived, lasting less than 10
35 months, although benefits of the 2017 WID operation, in terms of volumetric change, outlasted
36 the \approx 12-month study period. Dredging had a significant impact on water physicochemistry
37 (pH, dissolved oxygen, total suspended solids and turbidity) when comparing pre- and during-
38 dredging conditions within the dredge reach, although time-series analysis found dredging
39 effects were comparable in magnitude to tidal effects for some parameters. WID is typically
40 targeted at the thalweg and not the banks, rendering the geomorphic signature of the method
41 different to those of other, often more invasive dredging technologies (e.g. mechanical
42 dredging methods). Further, thalweg not bankside dredging may have potential positive
43 ecological implications, particularly where the majority of biomass is located within the
44 channel margins, as in the tidal River Parrett. Collectively, data suggest WID can be an
45 effective method for sediment dispersal within tidal systems although regular application may
46 be required to maintain cross sectional areas, particularly where management precedes periods
47 of low flows and/or high rates of sediment accumulation. In future, more work is required to
48 better understand both the physical and ecological implications of WID as a flood risk
49 management tool in estuaries and rivers.

50 **1.0 Introduction**

51 Fine sediment deposition in aquatic systems can have significant detrimental impacts on the
52 physical environment, ecosystem health and human life. Subaqueous mining or dredging
53 (dredging hereafter) is an engineering practice used to maintain channel capacity, improve
54 navigation potential (van Maren *et al.*, 2015; Wenger *et al.*, 2017, Wu *et al.*, 2018), remove
55 contaminants (Bormans *et al.*, 2016; Gustavson *et al.*, 2008; Chen *et al.*, 2018) and/or reduce
56 flood risk (Gob *et al.*, 2005) across the world.

57 Numerous dredging technologies exist, the main categories being mechanical, hydraulic and
58 hydrodynamic types, which are ubiquitously applied globally (e.g. China, Luo *et al.* 2007; Jing
59 *et al.*, 2013; Wu *et al.*, 2016, 2018; Egypt, Ismail and Samuel, 2011; France, Petit *et al.*, 1996;
60 Bravard *et al.*, 1997; Gaillot and Piégay, 1999; Liébault and Piégay, 2001; Germany, Reich,
61 1994; Italy, Rinaldi and Simon, 1998; Surian, 1999; Rinaldi, 2003; Surian and Rinaldi, 2003;
62 Poland, Wyzga, 1993; Lach and Wyzga, 2002; United Kingdom, Sear and Archer, 1998;
63 United States of America, Kondolf, 1994, Pinter *et al.*, 2004). Mechanical methods typically
64 involve the removal of sediments using heavy construction equipment, including bucket
65 excavators. Mechanical technologies can be either vessel- or land-based with the latter often
66 associated with bankside habitat degradation (Brooker, 1985). Hydraulic technologies utilise
67 centrifugal pumps and pipelines to raise fine-grained sediments from the bed (Vivian *et al.*,
68 2012). The financial cost of extracted sediment or “spoil” relocation or disposal can be
69 significant (Inman, 1976), rendering both mechanical and hydraulic methods infeasible in some
70 places. Hydrodynamic methods involve sediment displacement through water jetting and
71 unlike mechanical and hydraulic methods, the emphasis is on the downflow displacement, not
72 extraction of sediments, with potential efficiency and cost benefits (Wang *et al.*, 2012).

73 The environmental effects of dredging will vary between sites and as functions of operation
74 extent and persistence (Kondolf, 1994). Further, different dredging technologies utilise
75 different extraction/disturbance mechanisms and will therefore elicit different environmental
76 impacts (Wang et al., 2012), although knowledge of the physical effects of some types and
77 technologies remains rudimentary. Removal of instream sediments through mechanical
78 dredging can have profound impacts on channel morphology, potentially leading to bed
79 degradation (Lagasse and Winkley, 1980; Pinter et al., 2004), water level reductions (Kornis
80 and Laczay, 1988; Ellery and McCarthy, 1998), channel instability, removal of gravel
81 armouring and/or increased channel instability and erosion (Lagasse, 1975), via knickpoint
82 migration (Scott, 1973; Stevens et al., 1990) or incision downstream of the excavated pit.
83 Alternatively, hydraulic technologies are typically applied where high production rates are
84 required (Wenger et al., 2017), and their impacts can be significant, particularly where dredging
85 is spatially and temporally persistent (e.g. Luo et al., 2007). Some hydraulic and mechanical
86 methods may either directly (e.g. via turbation in the direct vicinity of the dredging vessel;
87 Collins, 1995; Mikkelsen and Pejrup, 2000; Smith and Fredrichs, 2011) or indirectly (e.g.
88 through channel deepening and so, tidal amplification; van Maren et al., 2015) cause sediment
89 resuspension. Activities may also release contaminants (Goosens and Zwolsman, 1996, Van
90 Maren et al. 2015), particularly particulate matter and pore water from the bed which may be
91 rich in trace metals (Van Den Berg *et al.*, 2001). Reduced sediment flux through aggregate
92 extraction can have important implications for sediment budgets (Collins and Dunne, 1989;
93 Kondolf and Swanson, 1993) and so, erosion rates (Simeoni and Corbau, 2009).

94 In contrast to the vast literature on mechanical and hydraulic dredging, there is currently little
95 published information on the use of hydrodynamic dredging methods as techniques for
96 subaqueous sediment management. Gravel jetting has been shown to influence surface but not
97 sub-surface size distributions, resulting in coarser and better-sorted surficial sediments with

98 potential benefits for shallow-, not deep-spawning fishes (Basic et al., 2017). Water injection
99 dredging (WID hereafter) is an emerging and relatively novel (Spencer et al., 2006)
100 hydrodynamic dredging technology first developed in the Netherlands in the mid-1980s and
101 brought about by rising costs of spoil disposal (Spencer et al., 2006). The method utilises
102 vessel-mounted pumps that inject high volumes of low-pressure water into channel sediments
103 via a horizontal spraybar located beneath the water surface (Scuria-Fontana, 1994). Hydraulic
104 agitation reduces bed sediment cohesion and resistance to entrainment, resulting in the
105 mobilisation of fine-grained particles (sands and silts) that are transported downstream through
106 persistent jetting under ambient flows (Scuria-Fontana, 1994). The majority of particles are
107 transported within the lower third of the profile and as a fluid mud layer, that varies in thickness
108 – from a few centimetres to a few decimetres – with the thickness of the density current much
109 smaller than the overlying water column (Winterwerp, 2002).

110 Given the nature of hydrodynamic methods and the corresponding sediment dispersal
111 mechanisms, there is significant potential for fine sediment and trace metal resuspension
112 effects, particularly where contaminant-rich sediments are dredged. Some work has considered
113 the potential physicochemical effects of WID on contaminant release and so, water quality,
114 with potential implications for aquatic biota (Spencer et al., 2005). Sediment cores collected
115 from a proposed WID site in South East England were analysed for contaminants and biota
116 were found at risk if native sediments were resuspended, due to elevated porewater ammonia
117 concentrations and bed sediment toxicity (Spencer et al., 2006). WID-induced release of
118 sediment-bound contaminants into the water column and dispersal of contaminated sediments
119 over large spatial scales remain significant environmental concerns (Sullivan, 2000; Oskar
120 Commission, 2004; Spencer et al., 2005), with more work required to assess the net effect of
121 the method.

122 To our knowledge, no other published studies exist on the geomorphic and/or physiochemical
123 impacts of WID. Therefore, the primary objective of this in-situ study was to determine the
124 nature and persistence of WID effects on estuarine channel morphology at the reach scale. WID
125 was selected as an emerging yet poorly studied hydrodynamic dredging method and utilised in
126 the tidal River Parrett (see subsection 2.1) to restore channel capacity for the maintenance of
127 flood water conveyance and in an effort to find a sustainable method of sediment management.
128 These avenues of inquiry are important in the contexts of assessing method efficacy and the
129 intentional and unintentional impacts of the technique, which remain poorly understood.
130 Additionally, there remains significant uncertainty over contemporary rates of sedimentation
131 and how these vary pre- and post-dredging within the tidal River Parrett, hence this study will
132 also generate baseline data for future management. The work had three principle aims:

- 133 1. To investigate the influence of WID on channel (bathymetry and bed sediment grain-size)
134 and flow (bathymetry)_conditions within the dredged reach to assess dredge efficacy and so,
135 suitability.
- 136 2. To assess the temporal persistence of changes in channel condition caused by WID.
- 137 3. To quantify the channel and physicochemical impacts of WID on downstream localities
138 through processes of downstream sediment dispersion.

139

140 **2.0 Materials, methods and data analysis**

141 2.1 Study sites, dredging history and sampling overview

142 The River Parrett is in South West England (Figure 1a) and is characterised by gravel and sand,
143 fine gravel and coarse sand and silt bed typologies in its upper, lower and tidal reaches,
144 respectively. The river is 59 km long and drains a catchment of 1,700 km² (Figure 1b), before
145 discharging into Bridgwater Bay via the Somerset Levels (the Levels hereafter). The Levels

146 are a coastal floodplain and wetland area of Somerset located between the Mendips and
147 Blackdown Hills. The area is low lying and geologically complex, with Triassic formations
148 underlying protrusions of younger lithologies, including upper lias sand, carboniferous
149 limestone and marl (Hardy, 1999). The catchment includes wetlands of international
150 importance (e.g. SPA and Ramsar; Natural England, 2013) and nationally significant
151 floodplain grazing marshes (e.g. SSSI; Natural England, 2013) and there are important
152 differences in land use between the wider catchment and the Levels. Arable and large-scale
153 (often zero graze) dairy farming are prevalent across areas surrounding the Levels. Dairy
154 farming has declined across the peat moors but is still important along the periphery of the
155 Levels. Beef farming is now the main form of grazing in the wetter areas of the catchment and
156 whilst Withy Growing there has declined, it remains an important land use on some Moors.

157 The sediment regime of the upper and middle reaches of the River Parrett is characterised by a
158 significant sand and gravel bed load together with a fine-grained suspended sediment load
159 derived mainly from agricultural land. The estuarine reach of the Parrett is extensive extending
160 through the Levels and some 18 km inland from the Severn Estuary. The reach has a sediment
161 regime characterised in a downstream direction increasingly by a high concentration of marine
162 derived suspended load of silt and clay with a coarser sand and fine gravel bed load (Ambios,
163 2017; Partrac, 2009).

164 The River Parrett is heavily modified in its lower reach as it passes through the Levels. The
165 river is constrained by artificial levees for flood relief purposes and highly affected by tidal
166 processes, with a regular supply of marine sediment delivered from Bridgwater Bay, in the
167 upper Severn Estuary, during penetration of tidal flows upstream. In addition, spillways and
168 sluice gates are located across the Levels and used to divert high flows out of the fluvial Parrett
169 and into a bypass channel (the River Sowey) or onto the inland Moors for short-term storage.
170 High rates of sedimentation within the river's lower estuarine reaches reduce the capacity of

171 the channel and its ability to convey fluvial flood flows from upstream leading to an increased
172 risk of flooding of low-lying land across the Levels. Historically, the impacts of sedimentation
173 on flood water conveyance have been mitigated through human intervention, particularly via
174 mechanical dredging activities. Dredging of the riverbed and banks has predominantly been
175 achieved using boat-mounted or land-based bucket excavators and spoil was used to develop
176 levees and/or improve agricultural land. These methods present a number of challenges as they
177 tend to be expensive and disruptive operations. Further, where the majority of biomass is located
178 within the margins, as in the River Parrett, land-based excavation of sediments and associated
179 bankside degradation can have detrimental ecological effects, reducing for example, faunal
180 abundance and diversity (Brooker, 1985). The frequency by which the method needs to be
181 applied to mitigate high rates of sediment accretion is high, prompting questions over the
182 suitability of dredging by extraction for maintaining flood flow conveyance through the tidal
183 reaches of the River Parrett (HR Wallingford, 2016). Thus, in the winter months of 2016 and
184 2017, the Somerset Rivers Authority and Somerset Drainage Boards Consortium undertook
185 two hydrodynamic dredging trials using WID and two environmental monitoring campaigns
186 spanning these periods to gain better understanding of the impacts of these operations on river
187 geomorphology and water physicochemistry.

188 Environmental monitoring was conducted between 20th October 2016 and 24th October 2018
189 (Figure 2) on a ≈ 7.7 km reach of the River Parrett, located between the M5 road bridge and
190 the village of Burrowbridge (experimental reach hereafter; Figure 1c). Dredging occurred 15th
191 November - 16th December 2016 (WID trial 1; WID 1 hereafter) and 3rd December - 9th
192 December 2017 (WID trial 2; WID 2 hereafter) along a ≈ 1.7 and ≈ 5.0 km reach, respectively,
193 and sampling occurred before, during and after these periods (dredging periods hereafter) at a
194 variety of locations within the experimental reach (details are provided herein; Figure 1c)
195 where morphological, sedimentological and/or physicochemical changes were anticipated.

196 Differences in dredging period were due to differences in campaign objectives and associated
197 environmental monitoring programs. Specifically, Bathymetric and fixed-depth and depth-
198 integrated water physicochemistry sampling was performed to assess effects and longevity of
199 effects of WID 1 and additional measures of bathymetry and bed sediment grain-size were
200 made to gauge long- and short-term effects of WID 2. Additional geomorphological, hydraulic
201 and/or ecological monitoring occurred during both dredging campaigns but the collected data
202 are not considered here.

203

204 2.2 Dredging activities

205 Dredging was completed by Van Ord UK Ltd. using the WID vessel Borr (S1). An articulating
206 spraybar mounted to the stern of the vessel was lowered below the dredge and fed river water
207 via a low pressure (~1 bar) pump. The spray bar was positioned horizontal to and immediately
208 above the bed, with the spray nozzles pointing downwards. Water released from the nozzles
209 agitated bed sediments, reducing bed sediment cohesion and resistance to entrainment and fine
210 sediments mobilised via hydraulic action were transported downstream by the flow. WID was
211 concentrated in a 6m-wide swathe along the dredge reach in the channel centre within the
212 thalweg zone. WID only occurred on ebbing high spring tides in an up-estuary direction and
213 ceased when low river levels prevented safe navigation. Multiple passes of the dredge reach
214 were completed during successive tides over several days within each campaign (Figure 2) to
215 achieve a specific design profile.

216

217 2.3 Characterising and analysing differences in channel bathymetry

218 How much and where material is displaced by WID and where displaced sediments are
219 deposited and for how long, represent important avenues of inquiry in the context of
220 determining the efficacy, sustainability and overall suitability of the method for sediment
221 management in estuarine systems. Thus, the reach between the M5 bridge and Burrowbridge
222 (Figure 1c) was surveyed on five occasions to assess impacts and longevity of impacts of the
223 two dredging campaigns on channel bathymetry (Aims 1 and 2). Specifically, sampling
224 occurred November 1st 2016 (pre-WID 1), February 14th 2017 (post-WID 1a), October 18th
225 2017 (post-WID 1b and pre-WID 2), January 8th 2018 (post-WID 2a) and October 24th 2018
226 (post-WID 2b; Figure 2).

227 Data were collected using a GeoSwath 4R interferometric sonar, mounted on a boat in
228 conjunction with a Real-Time Kinematic (RTK) GNSS positioning system and motion and
229 head referencing unit. The unit sampled at 500 Hz and swathe width was set at 10-15 m,
230 allowing for interrogation of the bed up to the waterline. Typical water depths during sampling
231 were 0.5 - 3.0 m and the bed immediately beneath the transducer head represented a blindspot
232 for the unit. Thus, on each sampling occasion, the boat made two passes of the survey reach –
233 one in the upstream and one in the downstream direction – to ensure full lateral coverage.
234 Sampling always occurred under spring high tides and depth measurements were corrected by
235 an offset of -0.413 m because the transducer was mounted below the RTK and water surface.
236 The Real-Time Kinematic (RTK) GNSS reduced sensor heights into Ordnance Survey levels,
237 ensuring data comparability. Point values of locations and depths (scans hereafter; XYZ.txt
238 format) were exported from Kongsberg Geoaoustics Ltd GS4 software and saved to external
239 storage media for ex-situ analysis.

240 On each occasion the surveyed reach included both a dredged section of channel (dredge reach
241 hereafter; ≈ 1.7 and ≈ 5.0 km during WID 1 and WID 2, respectively) and a section of river

242 located downstream and between the dredge reach and M5 bridge (downstream reach hereafter;
243 ≈ 8.0 and ≈ 4.2 km during WID 1 and WID 2, respectively; Figure 1c).

244 Topographic parameters were extracted from DEMs constructed using scans of the surveyed
245 reach. Scans corresponded to the central region of the channel and a proportion of each of the
246 relatively stable and un-dredged bank regions and consisted of approximately 491,000
247 irregularly spaced x, y and z coordinates with an average x-y spacing of 0.5 m. The point clouds
248 were then converted into raster DEMs (x-y spacing = 0.5 m) using a kriging interpolation
249 algorithm in ArcGIS © v.10.4.1 (Environmental Systems Research Institute, Redlands, CA,
250 USA). Both the dredge and downstream reaches were digitised (polygon) and the areas
251 extracted in ArcGIS for independent analyses (S2).

252 Topographic changes (Δd) to the bed surface were quantified by creating digital elevation
253 models of difference (DoDs hereafter) from the surface DEMs of the dredge and downstream
254 reaches, before and after dredging (Figure 3). The GNSS positioning system had an accuracy
255 of 0.05 m in the vertical plane whereas the interferometric device had an accuracy of
256 approximately 0.1 m. Thus, an error factor of ± 0.30 m was applied during DoD analysis, due
257 to there being two scans per DoD and so, twice the potential for error. Differences in channel
258 topography that exceeded ± 0.30 m were considered a result of dredging or natural
259 erosional/depositional process and not a result of equipment errors. Bed change was partitioned
260 into 11 bins corresponding to differences in elevation in 1 m increments above and below the
261 error factor.

262 Volumetric change of bed sediment through time as a function of dredging and/or natural
263 erosional and depositional processes was also assessed. Data were extracted from each of the
264 dredge reach and downstream reach DoDs using the surface volume (3D Analyst) tool in
265 ArcGIS. During analysis, plane height was set to ± 0.30 m and volumes above and below this

266 threshold were extracted and used to calculate net volumetric change. Negative and positive
267 values imply volumetric sediment losses (net erosion) and gains (net deposition), respectively.

268 The standard deviation of surface elevations was used as a measure of bed surface roughness
269 (Aberle & Smart, 2003) and was compared before and after the dredging periods for both the
270 dredge reach and downstream reach. Other bed elevation parameters including mean,
271 maximum and minimum elevations were extracted from DEMs and compared in the same way.

272 Finally, the effects of dredging on channel cross-section properties and so, river flow
273 characteristics (Aim 1) were assessed at 6 locations along each of the dredge and downstream
274 reaches (WID 1 and WID 2). In ArcGIS, cross sections were digitised (polyline) at the up and
275 downstream limits of and then at regular intervals along dredge and downstream reaches. The
276 number of cross sections did not differ between reaches and indeed, dredging programmes, but
277 the spacing between cross-sections did due to differences in reach lengths. Three subsequent
278 analyses were performed. First, in ArcGIS, 3D analyst interpolate line and point profile tools
279 were used to extract profile elevation data from each of the DEMs. The length of each parabola
280 (channel perimeter length hereafter; proxy for cross-section capacity) was then approximated
281 using the trapezoidal rule. Second, volumetric change of bed sediment through time was
282 assessed for each of the cross sections. Digitised areas corresponding to cross sections were
283 extracted from DEMs in ArcGIS before the cut fill (3D Analysis) tool was used to determine
284 the total volume of sediment eroded/deposited per profile. Third, to investigate the effect of
285 WID on riverflow characteristics, the total volume of water passing through each profile under
286 a simulated flow condition was determined. Prior to analysis, DEMs maintaining identical
287 aerial extents but different elevations to their equivalent cross section DEMs were created using
288 the Raster Calculator in ArcGIS. Importantly, elevations of the newly created DEMs were 6
289 m, slightly above the highest recorded cross section elevation (5.91 m). The Cut Fill tool was
290 then used to determine the volume of water contained within the surveyed channel under

291 simulated flow conditions. Each analysis gave 6 values per occasion and dredging campaign
292 for each of the dredged and downstream reaches ($n = 72$ total). Summary statistics were used
293 during all analyses.

294 2.4 Characterising and analysing differences in bed material grain-size distributions

295 Sieve and particle laser size analysis of sediments recovered from the dredge reach and up and
296 downstream localities were used to quantify the initial impact of WID 2 on bed sediment grain-
297 size distributions (Aim 1). This was an important test as WID induced changes in bed sediment
298 composition will likely have implications for particle entrainment thresholds and so, transport
299 fluxes. Benthic sediment surveys were undertaken at five sites (Figure 1c) and on two and one
300 occasion(s), before and after dredging, respectively (Figure 2b). Sites were located within
301 (dredged; $n = 3$) and downstream (downstream; $n = 2$) of the dredge reach and samples were
302 collected on two occasions before (14th and 15th November 2017) and on three occasions after
303 (26th December 2017, 9th February 2018 and 18th May 2018) dredging. At each site and on each
304 occasion, an Ekman grab sample was collected from the unvegetated inter-tidal sediment
305 surface on the left bank, right bank and channel centre using a boat at low tide. Samples were
306 returned to the laboratory and processed to remove and determine the prevalence of particles $>$
307 1 mm, prior to laser sizing. Specifically, individual samples ($n = 3$ per site and per occasion)
308 were dried and disaggregated using a pestle and mortar before being sieved to 2 mm using
309 sieve stacks and a shaker. A ≈ 4.5 g subsample was then taken from the pan material of
310 individual samples and used during further grain size analysis.

311 Grain size analyses were performed on the subsamples using a Beckman-Coulter LS230 laser
312 particle size analyser with polarisation intensity differential scattering (PIDS). Particles across
313 a range of 0.04 to 2000 μm can be measured across 116 class intervals with the system. The
314 Fraunhofer theory was selected to calculate grain-sizes, from the intensity of the diffracted

315 light, since the samples contained limited material finer than 10 μm (Beckman-Coulter, 1994;
316 de Boer *et al.*, 1987). Prior to grain size analysis, organic matter within samples was removed
317 by loss on ignition (LOI; CEN, 2007). To ensure complete disaggregation, preliminary tests
318 illustrated samples required agitating within the fluid module for 120 minutes (determined by
319 monitoring run time effect on mean, median, mode, D_{10} and D_{90} percentile values; data not
320 shown) until a negligible variability was reached. After this time the particle size distribution
321 was recorded over 10 runs lasting 60 seconds. Output data included statistical summary
322 parameters, which mathematically described the particle size distribution across the 10 runs,
323 including mean, mode, standard deviation, variance, skewness (degree of symmetry), kurtosis
324 (degree of peakedness) and cumulative percentile values (D_{10} , D_{25} , D_{50} , D_{75} , D_{90} ; the particle
325 size at which a specified percentage of the particles are finer), and statistics were calculated
326 geometrically based on a log-normal distribution (Beckman-Coulter, 1994) using equations 1-
327 5 (S2). Sample means were calculated from the ten measurements per sample to provide 6 pre-
328 and 3 post-dredging data points per site, which were used during statistical analyses.
329 Comparisons of pre- vs post-dredging data for each of the sites were made using normality
330 tests (Shapiro-Wilk test) followed by either Oneway ANOVA or the Kruskal-Wallis test, as
331 appropriate.

332

333 2.5 Characterising and analysing differences in water physicochemistry

334 Time series and depth-integrated water physicochemistry data were collected during WID 1
335 and were used to address Aim 3. SI 6600 V2 Sondes were installed at the downstream end of
336 the dredge reach at Westonzoyland and upstream of the dredge reach at Burrowbridge (Figure
337 1c) on the 20th October 2016 and were retrieved 16th January 2017 (Figure 2a). Thus, the time
338 period included 20, 23 and 41 days of pre-, during- and post-dredging data. Probes monitored

339 conditions at approximately 1 m depth and measured optical back scatter (turbidity),
340 conductivity ($\mu\text{S l}^{-1}$) and dissolved oxygen (mg l^{-1} , %) every 15 minutes. Stage data for the
341 river Parrett were recorded within the dredge reach at Northmoor pumping station, and
342 provided by the EA.

343 Fixed-depth sampling prohibits assessment of water physicochemistry through the water
344 column. Thus, and in addition, a Partech 740 turbidity meter and YSI ProDSS sonde, measuring
345 conductivity (used to calculate salinity), pH, turbidity and dissolved oxygen were attached to
346 an Owen tube and used to measure characteristics of the water column before and during WID
347 (Figure 2a). Specifically, 21 profiles were collected within the dredge reach pre-dredging and
348 39 profiles were collected downstream of the active dredging vessel. Profiles consisted of
349 approximately 114 measurements that were approximately equally distributed throughout the
350 water column. Water samples for calibrating the deployed optical turbidity sensors (to convert
351 from nephelometric turbidity units (NTU) to total suspended solids(TSS)) were also collected
352 with the Owen tube.

353 Summary statistics were used to analyse time series data. Pre- and during dredging differences
354 in water physicochemistry were tested using normality tests (Shapiro-Wilk test) followed by
355 either Oneway ANOVA or the Kruskal-Wallis test, as appropriate.

356

357 **3.0 Results**

358 3.1 The effect of WID on channel bathymetry (aim 1)

359 The majority (70%) of the surface area of the dredge reach was shown to be modified (i.e.
360 elevation change $> \pm 0.3$ m) following both WID 1 and WID 2 (Figures 4a and 5a). Within the
361 modified areas (i.e. elevation change < -0.3 and > 0.3 m), the majority of disturbance (82 and
362 83%, respectively) fell within the $-1 < \Delta d \leq -0.3$ m category. Reductions in maximum bed

363 elevation (-13 %) and bed surface roughness (-15 %) and an increase in minimum bed elevation
364 (+296 %) were detected following WID 1 (Table 1). Following WID 2, reductions in mean (-
365 14 %), maximum (-7 %) and minimum (-56 %) bed elevations and an increase in bed surface
366 roughness (+25 %) were recorded (Table 1). Volumetric changes after WID 1 and WID 2 were
367 -6,955 and -17,565 m³ sediment, respectively (Figure 6a and b). In terms of channel cross
368 sections, mean channel perimeter length increased from 14.84 ± 1.34 m³ to 15.36 ± 1.33 m³ (+
369 $3.56 \pm 0.01\%$) and from 14.74 ± 3.31 m³ to 15.08 ± 3.20 m³ ($+ 2.51 \pm 0.02 \%$) following WID
370 1 and 2, respectively (S3, S4). Mean volumetric changes subsequent to WID 1 and 2 were -
371 3.96 ± 1.77 m³ and -3.64 ± 2.29 m³, respectively (S5), and mean water volumes increased from
372 21.99 ± 2.55 m³ to 25.88 ± 4.00 m³ ($+ 17.40 \pm 0.07 \%$) following WID 1 and from 23.92 ± 5.10
373 m³ to 27.56 ± 5.90 m³ ($+ 15.37 \pm 0.09 \%$) after WID 2 (S4). A smaller proportion of the total
374 surface area of the downstream reach was changed following WID 1 and WID 2 in comparison
375 to the upstream dredged reach (38% and 48%, respectively; Figures 4c and 5c). Similarly,
376 volumes of losses of sediment were also much reduced (-2,689 and -2,469 m³ sediment,
377 respectively; Figure 6a and b) in comparison to upstream dredged reaches, and erosion was
378 entirely caused by natural fluvial scour over the winter period rather than including the effects
379 of dredging. After WID 1, bed surface roughness values increased by 96 % (from 0.46 to 0.90)
380 but decreased by 6% (from 0.95 to 0.89) after WID 2 (Table 1). With regard channel cross
381 sections, mean channel perimeter lengths increased from 15.66 ± 1.69 m³ to 15.89 ± 1.73 m³
382 ($+ 1.48 \pm 0.01 \%$) and from 14.54 ± 1.15 m³ to 14.66 ± 1.22 m³ ($+ 0.80 \pm 0.02 \%$) following
383 WID 1 and 2, respectively (S3, S4). Mean volumetric changes subsequent to WID 1 and 2 were
384 -2.45 ± 1.28 m³ and -2.91 ± 2.73 m³, respectively (S5), whereas mean water volumes increased
385 from 27.37 ± 4.23 m³ to 29.82 ± 4.08 m³ ($+ 9.33 \pm 0.06 \%$) following WID 1 and from 26.03
386 ± 5.35 m³ to 28.94 ± 3.06 m³ ($+ 13.91 \pm 0.18 \%$) after WID 2 (S4). During periods of recovery
387 (WID 1: February 14th – October 18th 2017; WID 2: January 18th – October 24th 2018) between

388 WID 1 and WID 2 (period 1) and following WID 2 (period 2), large proportions of the riverbed
389 were modified in both the dredge and downstream reaches (Figures 4 and 5). The majority of
390 change (dredge reach 65%, downstream 67%) during period 1 fell within the $0.3 < \Delta d \leq 1$ m
391 elevation change category whereas during period 2, dominant elevation change categories were
392 $1 < \Delta d \leq 2$ m (dredge reach 46%) and $-0.3 < \Delta d \leq 0.3$ m (downstream 54%). During period 1,
393 mean bed elevation increased from 2.90 to 3.68 m (as did maximum and minimum values;
394 Table 1) in the dredge reach. During period 2, mean and minimum bed elevations increased
395 whereas maximum values decreased (Table 1). Mean channel perimeter length decreased from
396 15.36 ± 1.33 m³ to 15.20 ± 1.30 m³ (-1.06 ± 0.01 %) and from 15.08 ± 3.20 m³ to 14.56 ± 3.14
397 m³ (-3.42 ± -0.02 %) during periods 1 and 2, respectively (S3, S4). Mean volumetric changes
398 during periods 1 and 2 were 6.28 ± 1.43 m³ and 0.76 ± 3.09 m³, respectively (S5). Mean water
399 volumes subsequently decreased from 25.88 ± 4.00 m³ to 19.65 ± 2.85 m³ (-23.95 ± 0.03 %)
400 following period 1 and from 27.56 ± 5.90 m³ to 26.80 ± 7.96 m³ (-3.81 ± 0.10 %) after period
401 2 (S4).

402 The downstream reach also saw mean, maximum and minimum elevations increase during
403 period 1 and channel roughness decreased in the dredge reach from 0.71 to 0.65 but increased
404 from 0.90 to 0.93 in the downstream reach. During period 2, reductions in mean, maximum but
405 not minimum bed elevation values were detected for the downstream reach (Table 1).
406 Volumetric changes of 11,411 and 13,324 and 7,564 and -2,421 m³ sediment were recorded for
407 dredge and downstream reaches following periods 1 and 2, respectively (Figure 6). Mean
408 channel perimeter length decreased from 15.89 ± 1.73 m³ to 15.78 ± 1.62 m³ (-0.61 ± 0.02 %)
409 and from 14.66 ± 1.22 m³ to 13.88 ± 1.27 m³ (-5.34 ± 0.02 %) during periods 1 and 2,
410 respectively (S3, S4). Mean volumetric changes during periods 1 and 2 were 6.40 ± 2.52 m³
411 and -1.08 ± 0.97 m³, respectively (S5). Mean water volumes subsequently changed from 29.82

412 $\pm 4.08 \text{ m}^3$ to $23.42 \pm 5.40 \text{ m}^3$ ($-22.07 \pm 0.10 \%$) following period 1 and from $28.94 \pm 3.06 \text{ m}^3$
413 to $30.01 \pm 3.95 \text{ m}^3$ ($+ 3.49 \pm 0.03 \%$) after period 2 (S4).

414 3.2 The effect of WID on bed material grain-size distributions (aim 2)

415 Dredged and downstream sites did not record significant changes in any of the measures grain-
416 size parameters before and after dredging (Oneway ANOVA or Kruskal Wallace test, $\alpha =$
417 0.05 ; Table 2).

418

419 3.3 The effect of WID on water physicochemistry (aim 3)

420 After initial installation of the static sondes one of the probes became detached 7th - 11th
421 November 2016 and was recovered a short distance downstream. The effect of this was that
422 five days of pre-dredging data were lost at the upstream sampling location. The time-series
423 indicated that turbidity levels were substantially elevated in the dredge reach in comparison to
424 upstream of the dredge during WID 1 (Figure 7a and b). However, because dredging occurred
425 during periods of high tide (Figure 7c), the impact of the dredge could not be isolated from
426 elevated turbidity associated with tidal effects. Indeed, the elevation in turbidity during high
427 tide and dredging in mid-November was of similar magnitude to during high tide with no
428 dredging in mid-December (Figure 7). Dissolved oxygen levels were generally higher at the
429 dredge site than the upstream site but dropped during dredging to well below those of the
430 upstream site (Figure 8a). However, oxygen levels returned rapidly to pre-dredge levels,
431 usually within an hour of dredging being ceased, and also dropped during other periods of high
432 tide. Conductivity is consistently higher at the dredge site and rises substantially during
433 dredging (Figure 8b). Unlike other measures, similar peaks are not observed during later high
434 tides without dredging.

435 Dredging was associated with statistically significant increases in TSS (Kruskal-Wallis: $H_1 =$
436 12.32; $P < 0.001$) and Turbidity (Kruskal-Wallis: $H_1 = 12.21$; $P < 0.001$) and statistically
437 significant decreases in pH (Kruskal-Wallis: $H_1 = 17.97$; $P < 0.001$) and dissolved oxygen
438 (Kruskal-Wallis: $H_1 = 5.66$; $P = 0.017$) relative to pre-dredging conditions (Figure 9). Salinity
439 data were similar before and during dredging.

440

441 **4.0 Discussion**

442 WID 2 reduced the elevation of the bed surface, as indicated by reductions in mean, maximum
443 and minimum bed surface elevations (Table 1), and topographic alterations corresponded with
444 a volumetric change of $-17,565 \text{ m}^3$ sediment (Figure 6). The extent and nature of disturbance
445 was consistent between dredging operations. Specifically, similar proportions of dredge
446 reaches, by area, were disturbed during dredging operations and in both cases, the majority of
447 topographic alterations involved a 0.3 - 1 m reduction in sediment surface height, suggesting
448 dredge-induced turbation was largely restricted to the uppermost meter of sediment. That the
449 majority of topographic alterations involved surficial sediment rearrangement represents a
450 significant finding given concentrations of contaminants in some systems are elevated in the
451 upper layer (dozens of centimeters) of sediments (Liu et al. 2016). Thus, application of WID
452 in systems where high concentrations of contaminants are detected within surface sediments
453 requires careful consideration of the specific risks associated with potential contaminants.

454 Visual assessments of DEMs (Figure 3; S3) suggest WID was consistently applied within the
455 dredge reach, creating a relatively homogeneous bed with topographic alterations concentrated
456 around the channel centre and not the margins. By contrast, extraction operations typically
457 target much smaller spatial areas, often over longer time scales, resulting in aggregate shoal
458 reduction and formation and/or widening of pits and troughs, respectively (Wu et al., 2016).

459 Findings demonstrate that the WID method was effective in removing the required volume of
460 sediment from the ecologically-poor thalweg without directly impacting the relatively
461 ecologically rich inter-tidal bank face, which is a significant disadvantage of some extraction
462 methods. Some localised loss of inter-tidal bank material and bank slumping was noted
463 following both WID trials (Ambios, 2017) where the toe of the bank had effectively been over-
464 steepened by the deeper thalweg zone created by WID. This was an anticipated outcome and
465 supports one of the aims of the WID trials – that is, the required loss of bank volume is achieved
466 through enhanced fluvial scour processes rather than a more damaging direct impact on habitat
467 and species through physical excavation.

468 Both dredging operations were associated with significant reductions in sediment volume along
469 the dredge reach (Figure 6; S3 and S5), increasing channel perimeter length and channel
470 capacity (S4) and so, potentially reducing flood risk locally. However, following WID 2, an
471 increase in bed surface roughness (+ 25%) was detected which might typically be associated
472 with increased wetted perimeters and flow retardation and so, a theoretical potential to reduce
473 flow conveyance. However, in this case, it is far more likely that detected increases in the
474 standard deviation of surface elevations reflect a deepening of the channel and therefore, an
475 increase in the range of bed elevations rather than net roughness.

476 Temporal persistence of dredging effects within the dredge reach varied between dredging
477 campaigns. Changes in topography were relatively short-lived following WID 1, with
478 significant sediment accretion detected between February and November 2017, as evidenced
479 by increases in surface elevations (Table 1) and an 11,412 m³ increase in sediment volume,
480 which far exceeded initial reductions due to dredging (- 6,955 m³ sediment; Figure 6). By
481 contrast, topographic change due to WID 2 was longer-lasting – with increases in mean and
482 minimum bed elevations corresponding with a 7,564 m³ increase in sediment volume.
483 Importantly, and unlike WID 1, losses due to WID 2 exceeded rates of sedimentation during

484 the period of channel recovery. Thus, WID 2 continued to provide benefit following the study
485 period, whereas benefit of WID 1 was limited to the first 10 months post-dredging. In line with
486 previous studies (e.g. Partrac, 2009), differences in the persistence of WID effects were almost
487 certainly caused by differences in fluvial flows and marine sediment delivery between years/
488 recovery periods. Indeed, gauge data (S6) indicate higher fluvial flows during the second
489 recovery period than the first likely resulting in decreased deposition, increased scour and
490 observed differences in the duration of dredging effects between the two campaigns.

491 It is reasonable to assume that bed disturbance through WID may have influenced natural
492 sediment transport processes in at least three ways. First, particles displaced during WID may
493 be deposited on the bed surface in positions of relative instability, making them more
494 susceptible to entrainment, particularly during ebbing spring tides or under high fluvial flows.
495 Second, turbation of surface sediments during WID might result in the loss of stabilising bed
496 sediment structure and/or disturbance of the coarser surface armour layer. Modification of
497 sediments in this manner, generating weakly structured sediments and/or substrates lacking a
498 coarse surface layer, may promote sediment transport and erosion by reducing critical shear
499 values required for particle entrainment. Third, channel deepening in turbid estuaries like the
500 River Parrett estuary has been found to exacerbate natural rates of sedimentation (e.g. van
501 Maren et al., 2015), potentially through reduced mean channel and nearbed flow velocities and
502 so, shear stresses post-dredging. Thus, one might expect an increase in sedimentation potential
503 and reduced scour following WID, relative to natural levels. Further, it is reasonable to assume
504 reduced basal shear stress may have implications for several other natural processes, including
505 tidal propagation and tidal flats inundation, although these avenues of enquiry were beyond the
506 scope of this study.

507 The benefits of WID over other dredging methods include reduced perceived environmental
508 effects, no dredge spoil disposal and relatively low time costs (Scuria-Fontana, 1994). WID

509 has the potential to deliver maintenance outcomes in a more environmentally sustainable
510 manner and it is more cost effective compared to bankside excavation and land disposal. Costs
511 can be further reduced through recognition that natural fluvial scour can in some years be more
512 effective in channel maintenance than dredging – thus to reduce costs as much as possible,
513 dredging programmes should be evidence-based and adapt to inter-annual variation in accretion
514 rates. Another advantage of WID over extraction methods is that dredged sediment remains
515 within the system. This is particularly important in closed and/or low suspended sediment
516 systems where sediment depletion may have significant long-term implications e.g. inter-tidal
517 habitat degradation and/or loss.

518 Findings demonstrate WID was effective in removing large quantities of fine sediment from
519 dredge reaches. However, detected rates of sedimentation post-dredging indicate changes were
520 short-lived and regular repeat dredging is required to maintain channel size, and flood flow
521 conveyance, in the River Parrett. It is reasonable to assume a need for river sediment and/or
522 water management will change through time for at least three reasons. First, future temperature
523 increases as a function of climate change will likely impact on river flows, reducing baseflow
524 conditions in summer resulting in increased fine sediment accumulation and so, channel
525 aggradation. Second, precipitation patterns may change with the expectation being that in a
526 warmer climate, heavy rainfall will increase with fewer more intense rainfall events, which will
527 have implications for the frequency and magnitude of flood events. Third, future sea level rise
528 is predicted (Natural England, 2013) and expected to cause marine transgression, characterised
529 by the upstream migration of the normal tidal limit and marine-influenced sedimentary
530 environments (Pye and Blott, 2014). In the context of the River Parrett estuary it is reasonable
531 to assume river base levels may be raised, decreasing stream power in the lower reaches and
532 so, increasing sedimentation there and penetration of tidal flows upstream. The potential for a
533 tidal barrier to mitigate against flood risk caused by extreme tidal surges is currently being

534 investigated for Parrett estuary downstream of Bridgwater. Such a scheme could have
535 unintentional implications for sediment dynamics, particularly during spring high tides when
536 the barrier is operational to reduce flood risk, and when peaks in upstream marine sediment
537 transport are expected. The presence of this artificial barrier may influence the process of
538 natural adjustment to sea level rise (Pye and Blott, 2014), potentially resulting in reduced and
539 increased aggradation of intertidal and/or subtidal deposits on the landward and seaward sides
540 of the barrier, respectively. Long-term management strategies will therefore need to account
541 for and be sensitive to both increased rates of anthropogenic changes in sediment and flow
542 regimes and in addition, sedimentation and increased risk of high intensity rainfall and thus,
543 elevated river flows. Furthermore, cost-effective and environmentally sustainable catchment-
544 wide approaches to land management are required to increase field soil-water retention and,
545 reduce fluvial sediment loadings in rivers where this represents a significant source of fine-
546 grained sediment.

547 Surprisingly, dredged and downstream sites (Figure 1c) did not record significant changes in
548 grain-size parameters before and after dredging (Table 2). This suggests WID mobilised the
549 full range of particle sizes, rather than preferentially sorting sediments within the dredge reach,
550 generating a coarse lag there and causing fining downstream. By contrast, other
551 hydromorphological methods such as gravel jetting (see Basic et al., 2017) have been shown
552 to modify the surface but not sub-surface size distribution of sediments, though substrates were
553 coarser in nature and maintained a broad range of size classes.

554 Sediments mobilised from the dredge reach during both campaigns had a negligible impact on
555 bed surface elevations downstream. Within downstream reaches, small proportions of scanned
556 surfaces corresponded to elevation gain categories ($\Delta d > \pm 0.3$; 4 and 3%) and net volume
557 change was negative (-2,689 and -2,469 m³ sediment), following WID 1 and WID 2,
558 respectively. Findings imply erosional processes were dominant and deposition negligible

559 within downstream reaches during the periods between sampling. However, sampling
560 frequency was insufficient to capture any short-term impacts (i.e. in the days and weeks after
561 WID) post-dredging but before winter fluvial flood flows, which may have led to an
562 underestimation of dredging effects downstream. Further, a lack of deposition within
563 downstream monitored reaches prompts questions around the fate of sediments displaced from
564 the dredge reach during WID. It is reasonable to assume these may have been deposited beyond
565 the survey reach within Bridgwater Bay, potentially influencing estuarine sediment dynamics
566 and/or hydraulics there.

567 The nature of disturbance during recovery periods varied between locations, with dredge
568 reaches recording greater proportions of scanned surfaces within higher positive elevation
569 change categories than downstream reaches (Figures 4 and 5). For example, during the post-
570 WID 2 recovery period, 271% more surface elevations were recorded in the $1 < \Delta d \leq 2$ m
571 disturbance category for the dredge reach relative to the downstream reach. This is strong
572 evidence of the dredge reach being taken out of 'regime' (where sediments are in equilibrium
573 with average energy conditions) and placed in a state of change post-dredging (Ambios, 2017).
574 Specifically, the increased cross-sectional dimensions as a function of dredging will have
575 reduced mean flow velocities and so increased potential rates of sedimentation within the
576 dredged reach but not the downstream reach. A similar but less marked trend was observed
577 during the recovery period following WID 1.

578 In terms of water physicochemistry, turbidity peaks were relatively unaffected by dredging and
579 comparable in magnitude to pre- and post-dredge high tide peaks (Figure 7). Turbidity was
580 higher at the upstream site during dredging than during subsequent high tides without dredging,
581 indicating that sediment was transported upstream by the tide in greater concentrations in
582 conjunction with dredging than without. In addition, the conductivity was substantially higher
583 during dredging than during high tides alone (Figure 8b). Collectively, this evidence may

584 indicate that bed materials are rendered less stable by dredging and are washed higher into the
585 water column, allowing them to be moved relatively more easily and further upstream than
586 would be expected in the absence of dredging. However, it is clear that the physiochemical
587 impacts of dredging were similar to high spring tides in the study reach and that penetration of
588 the tide is highly variable and strongly influenced by fluvial flows. Dissolved oxygen levels
589 dropped during WID, but these declines were similar in magnitude to those associated with
590 high spring tides and were of short duration (< 1 hour; Figure 8a).

591 At the ecosystem level, a primary concern is water quality and dredging activities capable of
592 detrimentally influencing water physicochemistry may have negative consequences for
593 resident biota. Although significant changes in pH, dissolved oxygen, turbidity and SSC were
594 detected during dredging (Figure 9), it is reasonable to assume the dredging program had a
595 negligible impact on in situ populations for at least three reasons. First, changes in water
596 physicochemistry parameters were short-lived, with individual events lasting less than 60
597 minutes. Environmental impact is at some level proportional to exposure time which was short,
598 especially for highly mobile species like fish that could evade a perceived danger. Second,
599 water physicochemistry was found highly variable within the experimental reach due to tidal
600 influence. Thus, species naturally present are likely to be resilient to rapid changes in water
601 physicochemistry and to sub-optimal conditions, as observed during dredging and under high
602 tides without dredging. Third, dredging operations were scheduled around migrations of
603 potentially sensitive migratory species such as Atlantic Salmon *Salmo salar* and eels *Anguilla*
604 *anguilla*. Appropriate planning can help mitigate impacts on ecological communities. Thus,
605 WID can have considerable physicochemical effects although the ecological consequences of
606 these and other dredging related factors (e.g. vessel noise) for biota and fisheries are poorly
607 understood.

608

609 **5.0 Conclusion**

610 The present study begins to fill the knowledge gap regarding the environmental impacts and
611 sustainability of WID, an increasingly used yet understudied dredging technology. We found
612 strong evidence of a dredging effect, although the effects of WID could not be isolated from
613 those of other processes/factors that affect tidal and indeed, heavily modified systems such as
614 the River Parrett estuary. The paper revealed WID is an effective sediment removal technique
615 for the surveyed reach although geomorphic effects of the method can be short-lived, lasting
616 less than 10 months, meaning regular repeat dredging may be necessary to maintain channel
617 geometries. The study also indicates that the method failed to measurably change bed sediment
618 grain-size distributions and given the highly tidal nature and associated high suspended
619 sediment concentrations typical of the study reach, effects of WID relative to the tide on some
620 water physicochemistry parameters were negligible, although in other less naturally turbid tidal
621 waters the environmental impacts may be more significant. Importantly, the biogenic signature
622 of WID was uniquely different to that of other, better-studied dredging technologies, including
623 mechanical and hydraulic methods, with channel deepening targeted at the thalweg. This paper
624 indicates an urgent need to better understand the geomorphological, physicochemical and
625 ecological effects of WID across a range of environmental conditions and management legacies
626 – from un-dredged to regularly dredged systems. Investigations of the long-term
627 geomorphological and ecological responses to continued WID and differences in effects when
628 applying WID in recently dredged and undredged reaches are required. Until this knowledge
629 is developed, the ability of river managers to fully understand the potential environmental and
630 operational ‘costs’ and ‘benefits’ of WID will remain highly constrained.

631

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643

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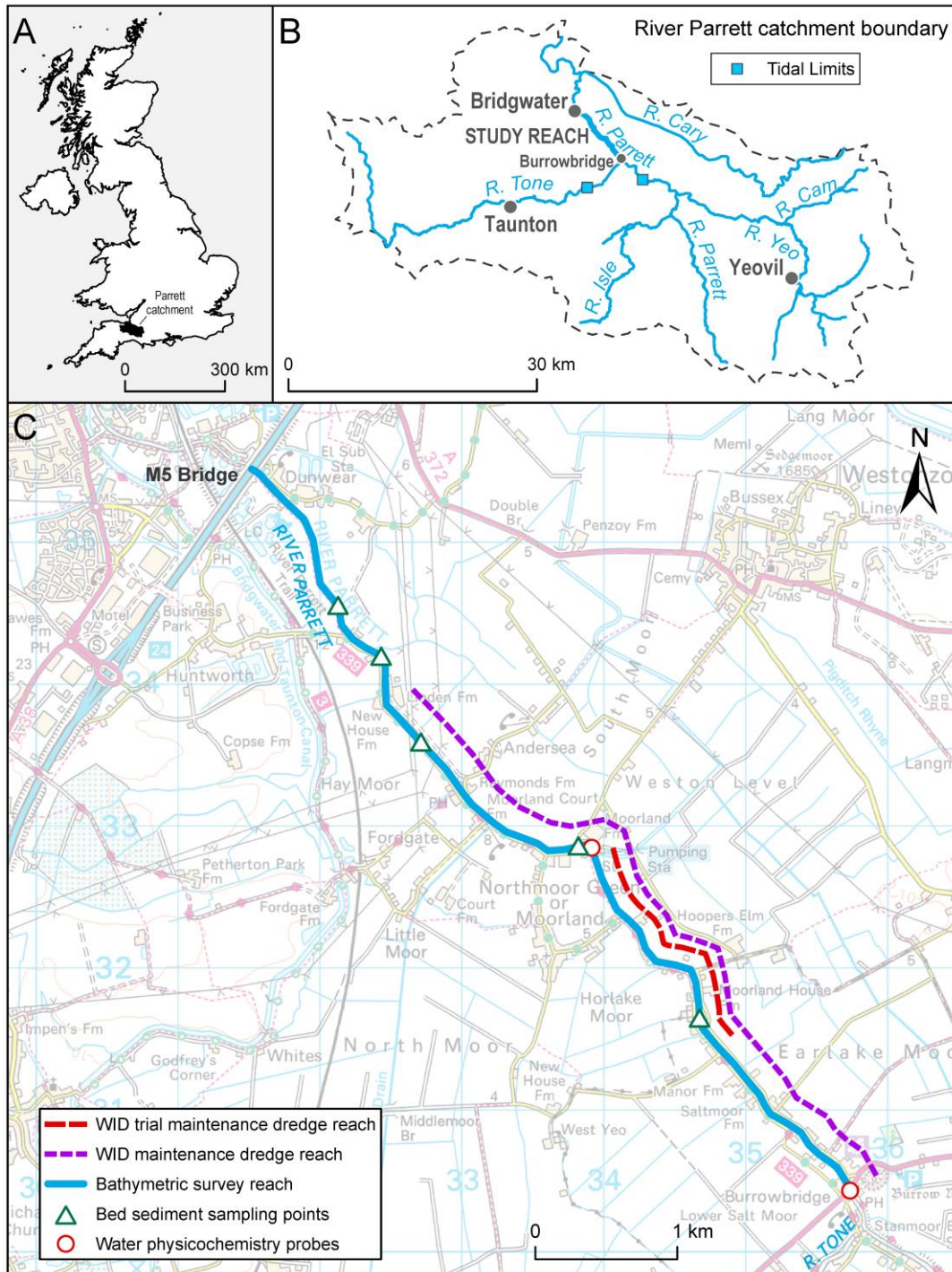
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856 **Figures**

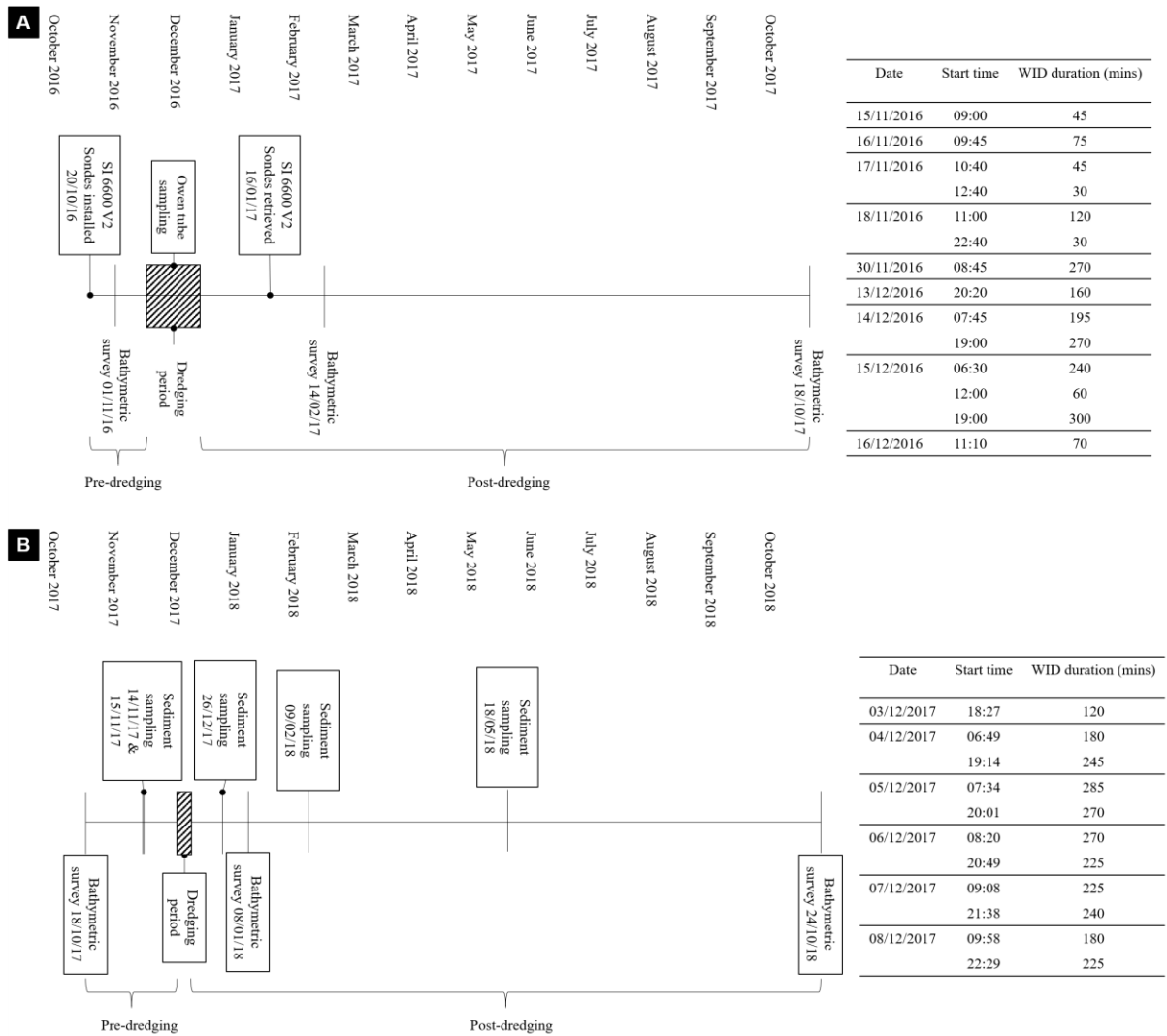
857 **Figure 1:** (A) Location and (B) catchment of the River Parrett and (C) details of the
 858 experimental reach between the M5 bridge and Burrowbridge, where bathymetric, bed
 859 sediment and water physicochemistry sampling occurred. The experimental reach in Figure 1C
 860 is highlighted in Figure 1B.



861

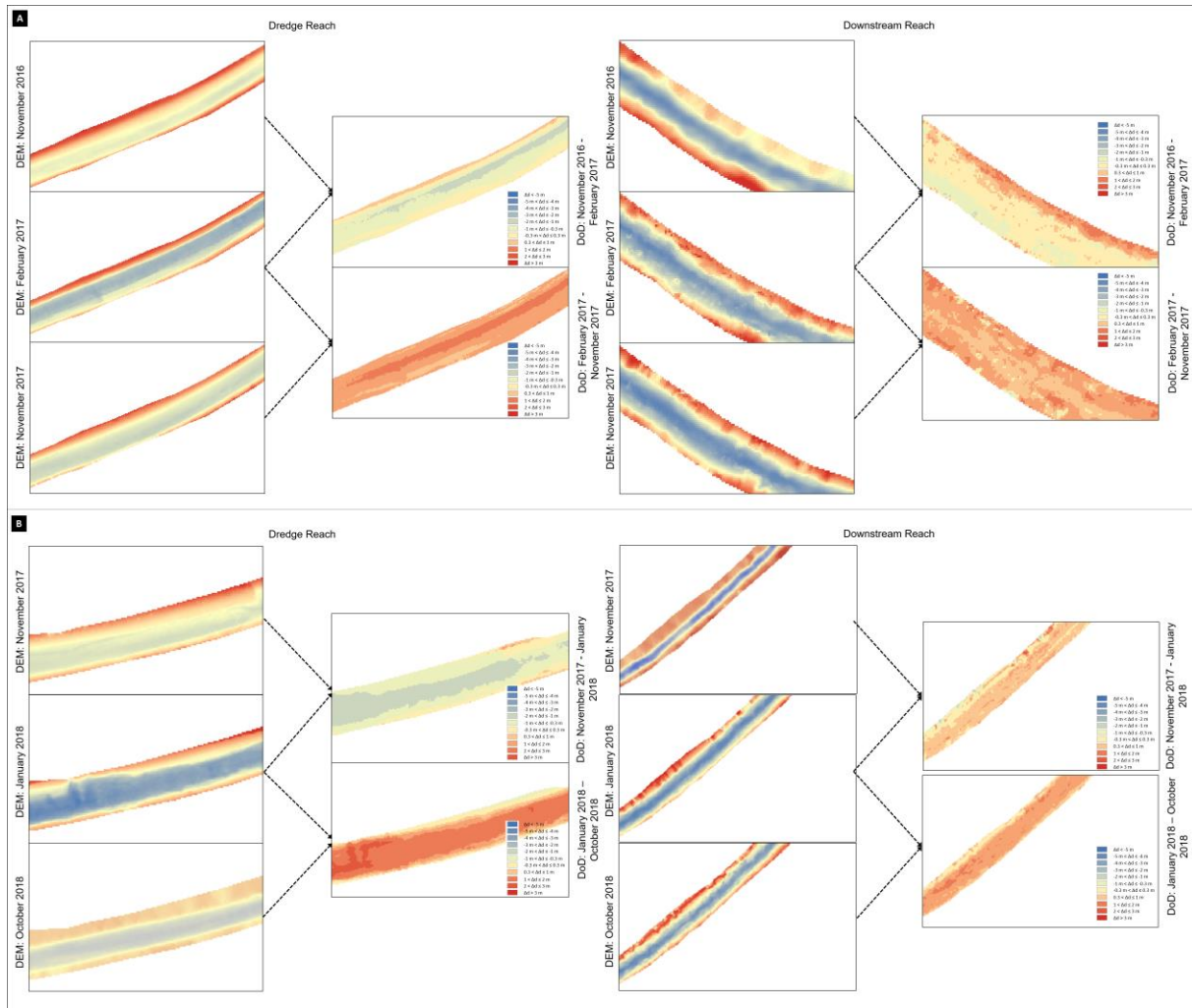
Created using ESRI ArcGIS Pro 2.2, British National Grid. © Crown copyright and database rights 2019 Ordnance Survey (100025252).

862 **Figure 2:** Schematic diagrams presenting the environmental monitoring and dredging
 863 programmes associated with A WID 1 and B WID 2. Timings and durations of dredging appear
 864 in tabular form.

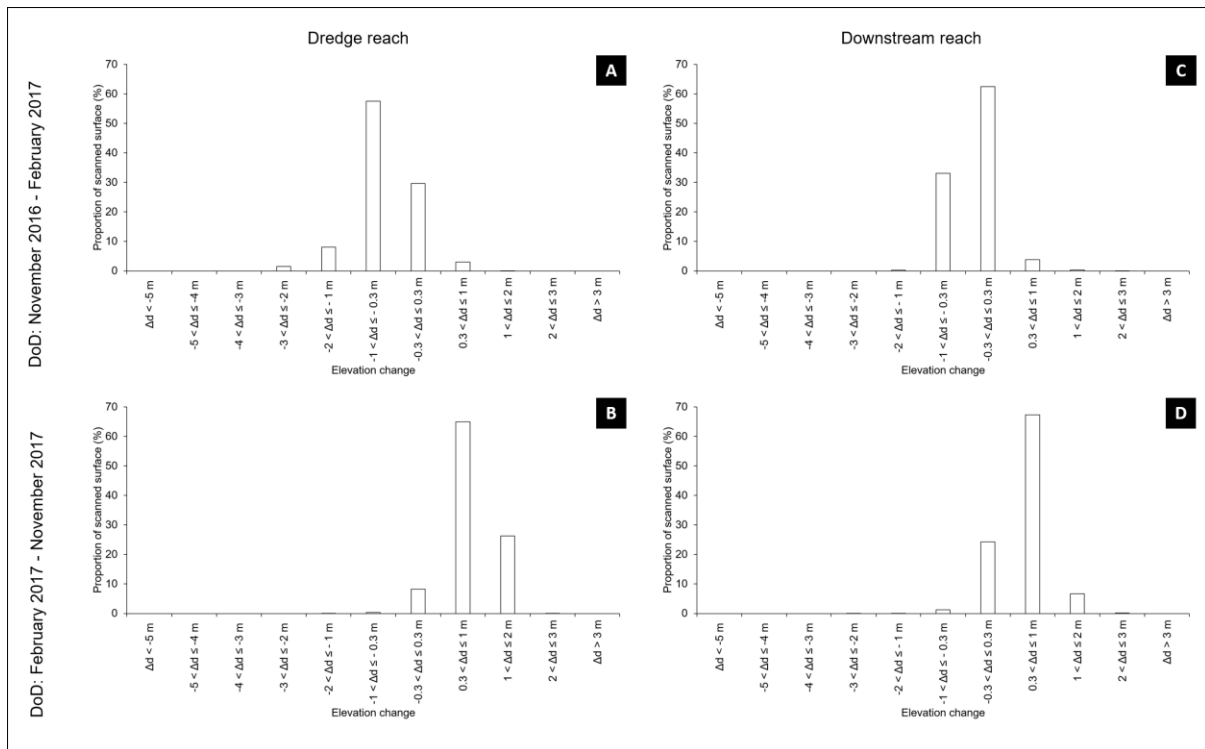


865

866 **Figure 3:** Examples of DEMs collected within and downstream of the dredge reach before and
867 after (A) WID 1 and (B) WID 2, and the resultant DoDs (digital elevation models of difference).
868 Dredging occurred between November 2016 and February 2017 in Figure 3A, and between
869 November 2017 and January 2018 in Figure 3B.

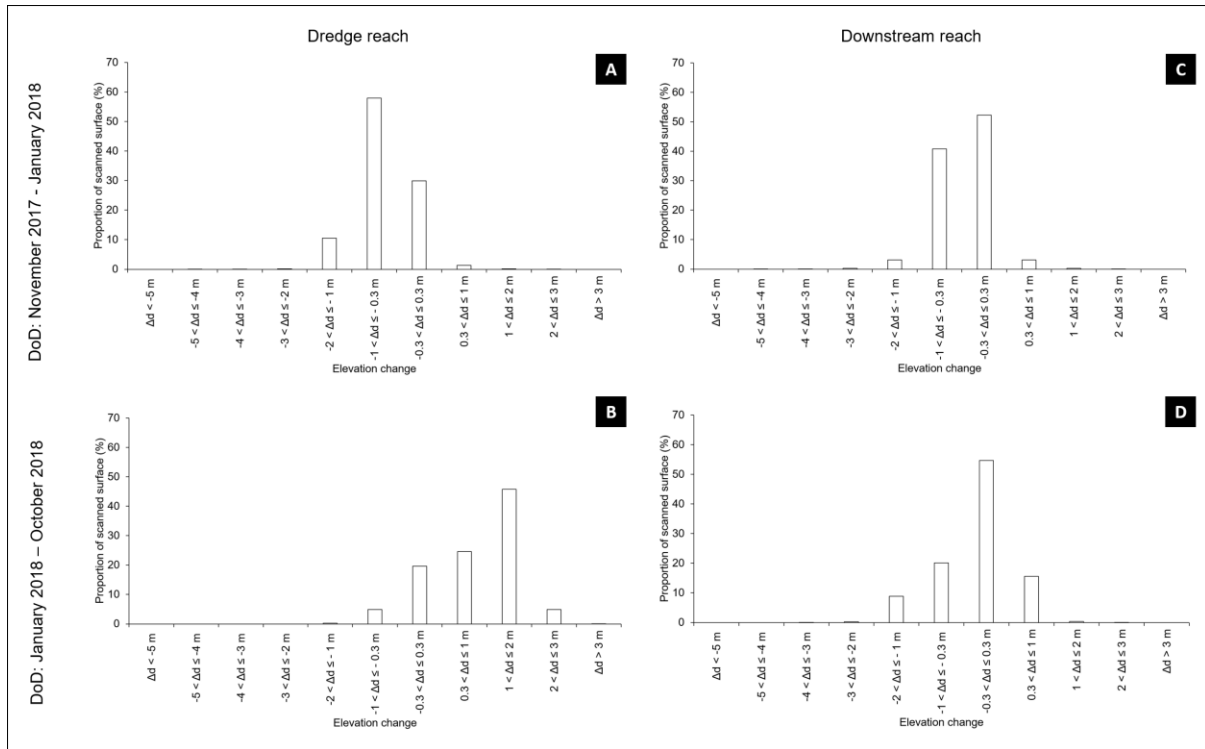


871 **Figure 4:** Surface elevation change as a percentage of the DEM surface area before and after
 872 WID 1, within (A - B) and downstream (C - D) of the dredge reach. A and C correspond to
 873 pre- vs immediately post-dredging comparisons whilst B and D highlight elevation change
 874 during the period of channel recovery. Presented are discrete data.



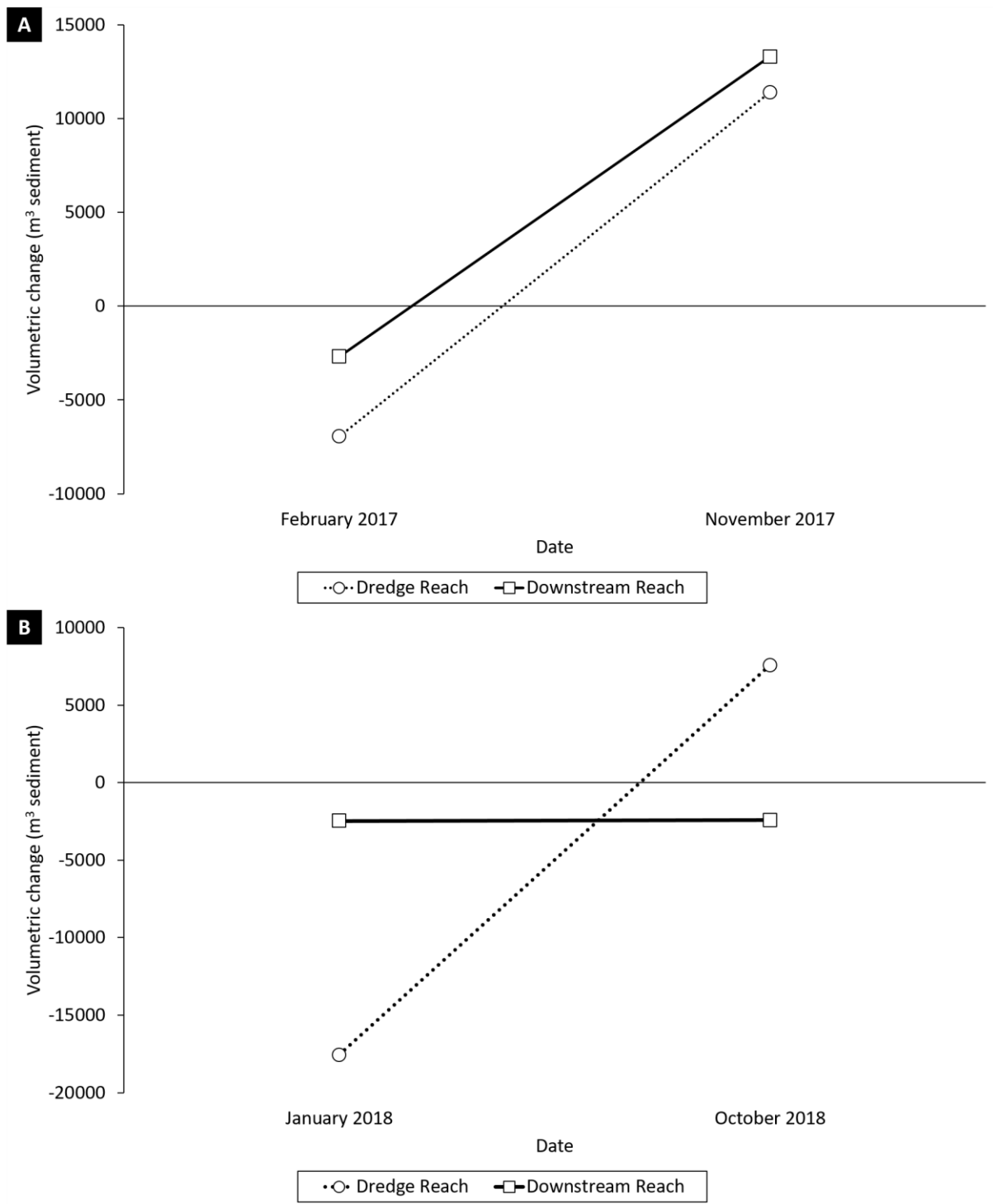
875

876 **Figure 5:** Surface elevation change as a percentage of the DEM surface area before and after
 877 WID 2, within (A - B) and downstream (C - D) of the dredge reach. A and C correspond to
 878 pre- vs immediately post-dredging comparisons whilst B and D highlight elevation change
 879 during the period of channel recovery. Presented are discrete data.



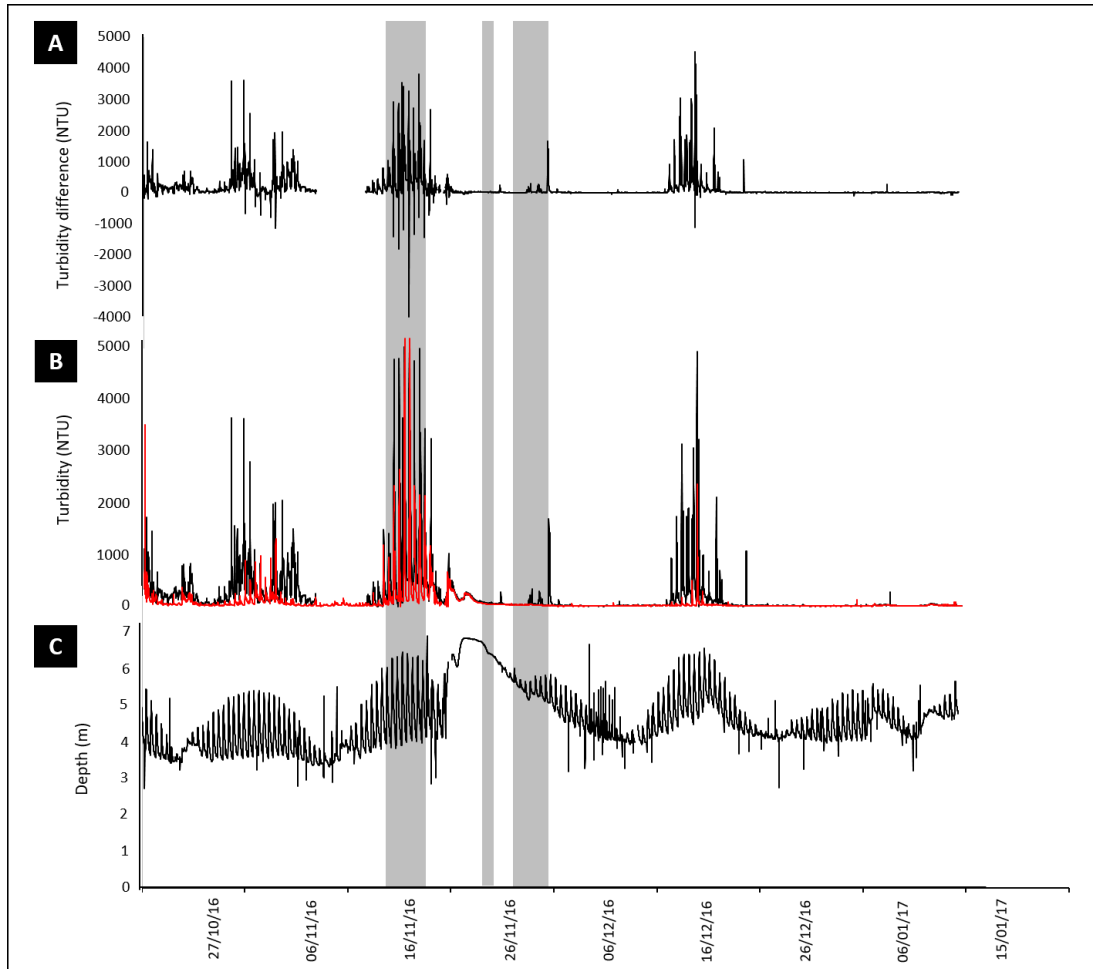
880

881 **Figure 6:** Volumetric change of bed sediment through time within the dredge reach and
 882 downstream of the dredge reach following (A) WID 1 and (B) WID 2. Presented are discrete
 883 data.



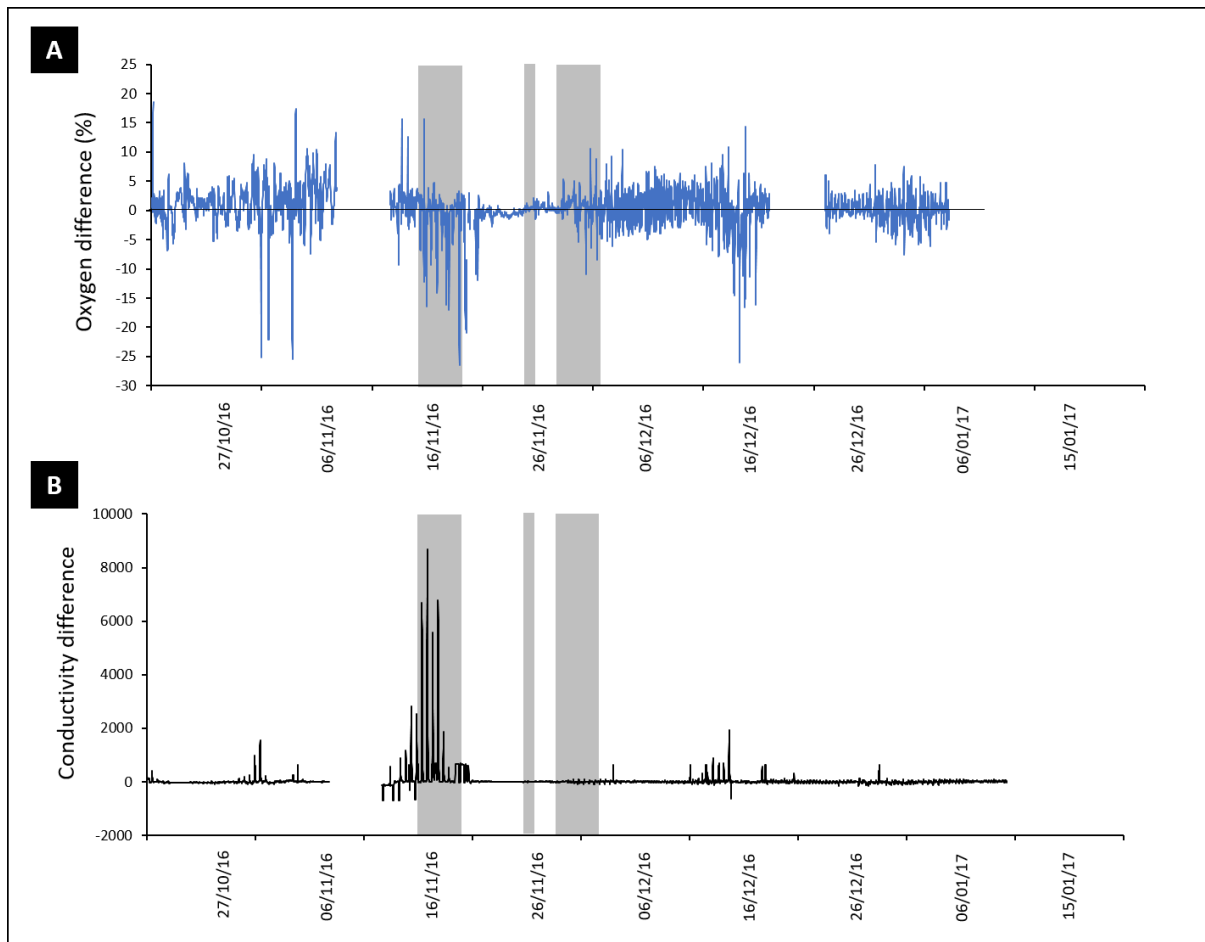
884

885 **Figure 7:** Turbidity recorded between 20th October 2016 and 16th January 2017 (WID 1), with
886 grey regions indicating periods of dredging. Figure 7A shows the difference in turbidity
887 between sites recorded up and downstream of the dredge reach. Turbidity traces are shown in
888 Figure 7B, with the upstream site shown in red and the downstream in black. In Figure 7C, the
889 trace represents water depth and highlights tidal effects on turbidity.



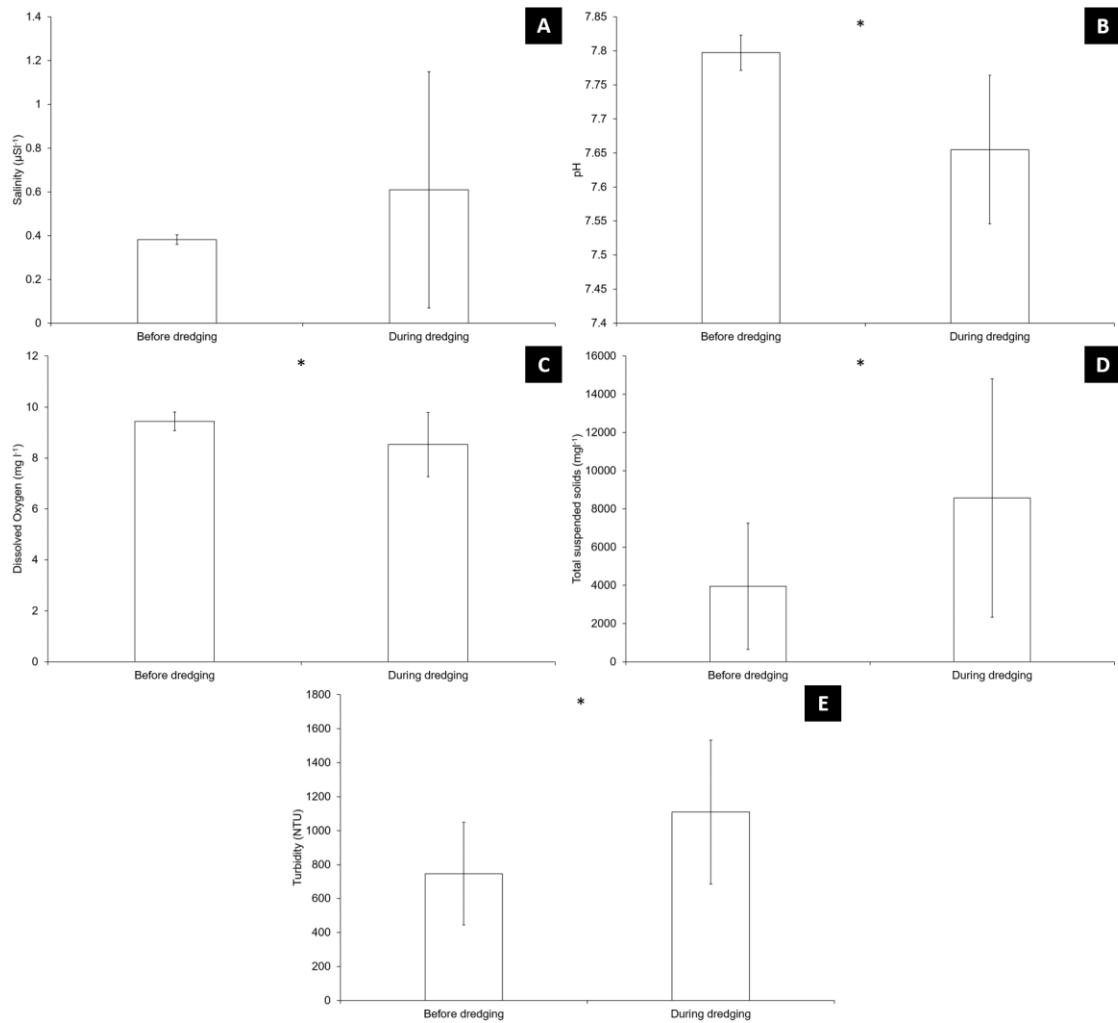
890

891 **Figure 8:** Water physiochemistry between 20th October 2016 and 16th January 2017 (WID 1)
892 with dredging periods shown in grey. Figure 8A indicates the difference in oxygen
893 concentration (%) between up and downstream sites and Figure 8B shows difference in
894 conductivity.



895

896 **Figure 9:** Water physicochemistry within the dredge reach before and during WID 1. Data
 897 derive from depth integrated sampling. Values represent means (\pm SD) and an asterisk above
 898 bars indicates the difference between pre- and during dredging values is significant ($\alpha = 0.05$).



899

900

901 **Tables**

902 **Table 1:** Mean, maximum and minimum bed surface elevations and standard deviations of bed
 903 surface elevations through time within and downstream of the dredge reach, in response to
 904 WID 1 and WID 2.

	Reach	Date	Mean (masl)	Maximum (masl)	Minimum (masl)	Standard Deviation (masl)
WID 1	Dredge	16/11/2016	2.90	5.68	0.59	0.84
	Dredge	14/02/2017	2.90	4.95	1.75	0.71
	Dredge	18/10/2017	3.68	6.22	2.38	0.65
	Downstream	16/11/2016	3.41	5.26	2.34	0.46
	Downstream	14/02/2017	2.70	5.28	0.41	0.90
	Downstream	18/10/2017	3.20	6.55	0.73	0.93
WID 2	Dredge	18/10/2017	3.59	6.47	1.51	0.64
	Dredge	08/01/2018	3.09	6.00	0.82	0.80
	Dredge	24/10/2018	3.34	5.11	1.46	0.46
	Downstream	18/10/2017	2.76	5.84	0.73	0.95
	Downstream	08/01/2018	2.47	6.10	0.19	0.89
	Downstream	24/10/2018	2.33	3.49	0.79	0.50

905

906

907 **Table 2:** D₁₀, D₂₅, D₅₀, D₇₅, D₉₀ and mean, mode, skewness and kurtosis metrics for pre- and
 908 post-WID 2 conditions at individual sites within dredge (n = 3) and downstream (n = 2) reaches,
 909 and details of statistical tests comparing differences between the two. Values represent pre-
 910 and post-dredging site means ± SD (n = 6 and n = 3, respectively).

Reach	Pre-dredging		Post-dredging		T test		Pre-dredging		Post-dredging		T test		Pre-dredging		Post-dredging		T test		
	Mean (mm)	SD (mm)	Mean (mm)	SD (mm)	Test	Result	Mean (mm)	SD (mm)	Mean (mm)	SD (mm)	Test	Result	Mean (mm)	SD (mm)	Mean (mm)	SD (mm)	Test	Result	
D75 (mm)																			
Raach	66.933 ± 21.101	55.967 ± 6.007	Kruskall Wallance	1.067	0.302	90.730 ± 16.374	6.867 ± 14.27	One-way Anova	1.544	0.254	44.125 ± 22.754	31.710 ± 4.125	Kruskall Wallance	1.667	0.197	78.590 ± 40.366	60.400 ± 4.757	Kruskall Wallance	0.267
Dredge	81.467 ± 20.018	76.500 ± 5.797	Kruskall Wallance	0.067	0.796	112.083 ± 14.800	11.333 ± 9.01	One-way Anova	0.006	0.939	49.490 ± 24.723	41.677 ± 3.007	Kruskall Wallance	0.267	0.606	81.467 ± 20.018	76.500 ± 5.797	Kruskall Wallance	0.067
Downstream	75.283 ± 12.551	83.467 ± 33.604	One-way Anova	0.308	0.596	110.900 ± 18.545	10.633 ± 23.85	One-way Anova	0.000	0.986	39.118 ± 8.695	56.360 ± 36.684	One-way Anova	1.356	0.282	75.283 ± 12.551	83.467 ± 33.604	One-way Anova	0.308
Downstream	89.933 ± 16.817	74.100 ± 5.730	Kruskall Wallance	1.667	0.197	122.233 ± 23.233	06.000 ± 3.46	One-way Anova	1.355	0.283	47.758 ± 8.927	41.853 ± 1.746	Kruskall Wallance	1.667	0.197	89.933 ± 16.817	74.100 ± 5.730	Kruskall Wallance	1.667
D90 (mm)																			
Raach	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000	1.000	-1.529 ± 0.615	-1.760 ± 0.165	One-way Anova	0.386	0.554	3.852 ± 2.474	3.670 ± 0.459	One-way Anova	0.003	0.957	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000
Dredge	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073	0.786	-1.455 ± 0.512	-1.715 ± 0.127	One-way Anova	0.703	0.430	2.929 ± 1.957	4.004 ± 0.912	One-way Anova	0.778	0.407	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073
Downstream	55.878 ± 10.184	69.803 ± 41.172	One-way Anova	0.694	0.432	-1.558 ± 0.281	-1.141 ± 0.623	Kruskall Wallance	1.667	0.197	3.152 ± 1.393	1.888 ± 2.094	One-way Anova	1.210	0.308	55.878 ± 10.184	69.803 ± 41.172	One-way Anova	0.694
Downstream	69.592 ± 13.034	39.81 ± 7.474	Kruskall Wallance	3.379	0.066	-1.831 ± 0.218	-1.807 ± 0.038	One-way Anova	0.033	0.862	4.223 ± 1.187	4.657 ± 0.319	One-way Anova	0.363	0.566	69.592 ± 13.034	39.81 ± 7.474	Kruskall Wallance	3.379
Mode (mm)																			
Raach	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000	1.000	-1.529 ± 0.615	-1.760 ± 0.165	One-way Anova	0.386	0.554	3.852 ± 2.474	3.670 ± 0.459	One-way Anova	0.003	0.957	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000
Dredge	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073	0.786	-1.455 ± 0.512	-1.715 ± 0.127	One-way Anova	0.703	0.430	2.929 ± 1.957	4.004 ± 0.912	One-way Anova	0.778	0.407	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073
Downstream	55.878 ± 10.184	69.803 ± 41.172	One-way Anova	0.694	0.432	-1.558 ± 0.281	-1.141 ± 0.623	Kruskall Wallance	1.667	0.197	3.152 ± 1.393	1.888 ± 2.094	One-way Anova	1.210	0.308	55.878 ± 10.184	69.803 ± 41.172	One-way Anova	0.694
Downstream	69.592 ± 13.034	39.81 ± 7.474	Kruskall Wallance	3.379	0.066	-1.831 ± 0.218	-1.807 ± 0.038	One-way Anova	0.033	0.862	4.223 ± 1.187	4.657 ± 0.319	One-way Anova	0.363	0.566	69.592 ± 13.034	39.81 ± 7.474	Kruskall Wallance	3.379
Skewness																			
Raach	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000	1.000	-1.529 ± 0.615	-1.760 ± 0.165	One-way Anova	0.386	0.554	3.852 ± 2.474	3.670 ± 0.459	One-way Anova	0.003	0.957	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000
Dredge	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073	0.786	-1.455 ± 0.512	-1.715 ± 0.127	One-way Anova	0.703	0.430	2.929 ± 1.957	4.004 ± 0.912	One-way Anova	0.778	0.407	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073
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Kurtosis																			
Raach	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000	1.000	-1.529 ± 0.615	-1.760 ± 0.165	One-way Anova	0.386	0.554	3.852 ± 2.474	3.670 ± 0.459	One-way Anova	0.003	0.957	53.870 ± 25.831	44.400 ± 2.356	Kruskall Wallance	0.000
Dredge	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073	0.786	-1.455 ± 0.512	-1.715 ± 0.127	One-way Anova	0.703	0.430	2.929 ± 1.957	4.004 ± 0.912	One-way Anova	0.778	0.407	64.668 ± 25.740	55.300 ± 5.152	Kruskall Wallance	0.073
Downstream	55.878 ± 10.184	69.803 ± 41.172	One-way Anova	0.694	0.432	-1.558 ± 0.281	-1.141 ± 0.623	Kruskall Wallance	1.667	0.197	3.152 ± 1.393	1.888 ± 2.094	One-way Anova	1.210	0.308	55.878 ± 10.184	69.803 ± 41.172	One-way Anova	0.694
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