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1 **Impact of Combined Alignments on Lane Departure: A**  
2 **Simulator Study for Mountainous Freeways**

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11 **ABSTRACT**

12 Lane departures are responsible for many side-swipe, rear-end and single-vehicle  
13 run-off-road crashes. There is a dearth of research, however, on how lane departures are  
14 impacted by roadway alignments. The objective of this paper is to examine which geometric  
15 design characteristics, including road alignment at the current segment and the adjacent  
16 segments, have significant influence on lane departure. Lane departure data from a total 30  
17 drivers were collected from a driving simulator study of a four-lane (two lanes in each  
18 direction) divided mountainous freeway. Lane departures were classified into *lane keeping*,  
19 *lane departure to the left* and *lane departure to the right* for all-alignments (Dataset I), and  
20 *lane keeping*, *lane departure to the inside* and *lane departure to the outside* for curves-only  
21 (Dataset II). A mixed multinomial logit model for each dataset was employed to examine the  
22 contributory factors. This approach allows for the possibility that the estimated model  
23 parameters can vary randomly to account for unobserved effects potentially relating to  
24 heterogeneous driver behaviors. Fixed parameters that had a significant increase on lane  
25 departure were horizontal curvature at the current segment, and the difference (max-min) in  
26 horizontal curvature within the 300-m adjacent upstream alignment. Downward slope and  
27 upward slope with fixed parameters significantly decreased lane departure. Estimated  
28 parameters related to the direction of the curve, driving lane (bordering median or hard  
29 shoulder) and driving speed had found to have randomly distributed over the drivers. This  
30 indicates that driver behavior is not consistent in the effect of these three variables on lane  
31 departure. These results can assist engineers in designing safer mountainous freeways.

32 **Keywords:** Mountainous Freeways, Combined Alignments, Lane Departure, Mixed  
33 Multinomial Logit Model, Driving Simulator.  
34  
35

## 36 INTRODUCTION

37 Lane departure is a critical safety event that occurs when a vehicle unintentionally  
38 moves out of its current lane. It is considered to be the primary precursor of roadway  
39 departures and single-vehicle run-off-road (ROR) crashes (Transportation Research Board,  
40 2011). An analysis of 2007 to 2013 crash data from the Fatality Analysis Reporting System  
41 (FARS) database reveals that an average of 59% of annual motor vehicle traffic fatalities in  
42 the United States occurred due to roadway departure (NHTSA, 2016). Lane departure can  
43 also lead to rear-end and side-swipe crashes in the case of divided roadways, and to head-on  
44 crashes on undivided roadways. In China, the proportion of traffic crashes associated with  
45 lane departure is about 42% in 2007 (Zhou, 2010).

46 Research on lane departure has mainly focused on the design and development of  
47 warning systems that are capable of detecting whether lane departure is imminent, and then  
48 inform the driver using visual, vibration and sound warnings. There is a dearth of research,  
49 however, on how lane departures are influenced by roadway geometry. Some studies have  
50 shown that certain road alignments increase the likelihood of roadway departure crashes (e.g.  
51 Eustace et al., 2014; Lord et al., 2011; Liu and Subramanian, 2009); and Torbic et al. (2004)  
52 have indicated that approximately 76% of curve-related fatal crashes are single-vehicle ROR  
53 crashes. It can be assumed, then, that some geometric alignments may be correlated with lane  
54 departure. If the combinations of horizontal and vertical alignments at the current segment  
55 (road alignment at the current position of a vehicle) are improperly designed, e.g. a sharp  
56 horizontal curvature with an upward slope, the alignments could lead to unnecessary and  
57 excessive lane departures. In addition to the current segment, the roadway alignments at both  
58 upstream (i.e. road just passed) and downstream (i.e. road ahead) adjacent segments (termed  
59 as '*adjacent alignments*' henceforth) may affect lane departure. For example, when two  
60 curves with small radii are adjacent or a long downhill alignment is followed by a small  
61 radius curve, a vehicle may easily deviate from its lane, especially at a high speed. The  
62 combined horizontal and vertical alignments at the current segment and the adjacent  
63 alignments are here referred to as '*combined alignments*'.

64 Safety assessment of road alignments design has mainly focused on determining of the  
65 threshold values for single horizontal alignments and single vertical alignments  
66 independently. For example, the criteria of the minimum radius and the maximum grade for  
67 appropriate combinations of design speed and terrain type have well established (e.g.  
68 AASHTO, 2010; MOT, 2015). In response to studies that have shown that horizontal and  
69 vertical alignments should be considered together, several qualitative design guidelines for  
70 combined alignments are presented in Design Specification for Highway Alignment  
71 (AASHTO, 2011; MOT, 2006). Safety criteria for combined alignments are, however, not  
72 systematic in current guidelines, and safety criteria for adjacent alignments are not currently  
73 available at all (AASHTO, 2010; MOT, 2015).

74 The objective of this research, therefore, is to examine how the combined alignments  
75 affect the probability of lane departure while controlling for other factors. Since real-world  
76 data on the corresponding occurrences of lane departure with combined alignments are not

77 readily available, a driving simulator study was conducted. Lane departure events, lane  
78 keeping states and other operational data (e.g. speed) were continuously captured by the  
79 simulator software during a varied road alignment scenario of a mountainous freeway.  
80 Factors such as road environment and traffic conditions were kept consistent in the  
81 simulation so as to reduce extraneous impact on lane departure. The mixed multinomial logit  
82 model was employed, which accounts for the possibility that the estimated model parameters  
83 can vary randomly in response to unobserved effects relating to drivers' behaviors.  
84

## 85 **LITERATURE REVIEW**

86 Due to the lack of research on the effects of combined alignments on lane departure, this  
87 section will review and synthesize existing related studies. They include road alignments'  
88 effects on safety and the means of evaluating those effects, and factors that specifically  
89 influence lane departure, particularly vertical and horizontal alignments.  
90

### 91 **Effects of road alignments on safety**

92 Horizontal curvature and vertical grade have been found to be correlated with crash  
93 occurrence in a number of studies. Torbic et al. (2004) reported that the crash rate of  
94 horizontal curves is approximately three times that of tangent sections. A review of crash data  
95 in Iowa between 2001 and 2005 indicated that 12% of all fatal crashes and 15% of all major  
96 injury crashes occurred on curves (Transportation Research Board, 2011). A study by Miaou  
97 and Lum (1993) revealed that as vertical grade increases, accidents involving trucks also  
98 increase. Wang et al. (2015) developed multiple linear regression models to estimate the  
99 effects of combinations of horizontal and vertical alignments on lateral acceleration.

100 Traffic crashes, however, result from the interaction of a complex range of factors such  
101 as driver, roadway, vehicle and weather. The intrinsic complexity of these factors combined  
102 with the often poor quality of traffic crash data results in an insufficient supply of  
103 information about crash causation (Tarko, 2012). Because the shortcomings of this  
104 information can make it difficult to evaluate the impact of single factors such as road  
105 alignment on safety, crash surrogates are therefore commonly used. Good surrogate measures  
106 are directly linked to crash occurrences and are affected by variables known to also affect  
107 safety (Wang et al., 2015).

108 Speed consistency is a commonly and widely used surrogate. For instance, on the basis  
109 of the 50% (median) and the 85% critical values of the sample distribution of  $\Delta V_{max}$  and of  
110  $\Delta V_{mean}$  as thresholds ( $\Delta V_{max}$  is the difference between the minimum speed on a curve and  
111 the maximum speed on a tangent;  $\Delta V_{mean}$  is the difference between the minimum speed on a  
112 circular curve and the mean speed for the entire test course), Cafiso et al. (2009) used a  
113 naturalistic driving experiment to determine good, fair, and poor domains of design  
114 consistency. Similar evaluation criteria were also recommended by *Specifications for*  
115 *Highway Safety Audit* of China, which used speed consistency to evaluate the coordination  
116 between adjacent road segments. Evaluation criteria were divided into three levels: i) good,  
117  $|\Delta V_{85}| < 10 \text{ km/h}$ ; ii) fair,  $10 \text{ km/h} \leq |\Delta V_{85}| \leq 20 \text{ km/h}$ ; and iii) poor,  $|\Delta V_{85}| > 20 \text{ km/h}$ , in which

118  $\Delta V_{85}$  represents the 85th percentile of the distribution of maximum vehicle speed on the  
119 adjacent road alignment segments (MOT, 2015).

120

### 121 **Alignments and other factors influencing lane departure**

122 One way to detect lane departure is to use lateral offset, which is defined as the distance  
123 between the lane's center-line and the vehicle's center-line (Jung and Kelber, 2005). Once  
124 lateral offset reaches the threshold that a vehicle moves out of its current lane, it is termed as  
125 a lane departure behavior, which is identified as a risky lateral offset (NHTSA, 2011).

126 Research on the influence of road alignments on lane departure has mostly focused on  
127 horizontal geometrical parameters, e.g. curve radius, curvature (reciprocal of the radius, unit:  
128 1/km), and curve direction (i.e. left-turn or right-turn). For instance, Jalayer and Zhou (2017)  
129 found that horizontal curvature was one of the most significant variables for ROR crash  
130 frequency. Lin et al. (2011) concluded that a small curve radius led to a large lateral offset.  
131 Wu et al. (2013) constructed a prediction model for the standard deviation of lateral offset  
132 using the multivariate linear regression model and showed that the length of the tangent  
133 alignment, the length of the circular curve, and the curvature change rate were all significant  
134 independent variables. Spacek (2005) showed that drivers maintained a clearly larger  
135 distance from the road edge than to the center line both in left-turn curves and in right-turn  
136 curves, but nearer to the center line on left-turn curves than right-turn curves. Spacek (2005)  
137 then concluded that the variation in lateral offset may be caused by curves, centrifugal  
138 acceleration and speed. Yet none of these previous studies considered the possible impact of  
139 vertical alignments.

140 Adjacent alignments have been found to be related to speed change, and speed change  
141 has been shown to contribute to lane departure. When, for example, the length needed for  
142 deceleration to curve  $n+1$  from curve  $n$  is less than the available length, some speed changes  
143 from the previous curve will occur (Fitzpatrick and Collins, 2000). Xu et al. (2013)  
144 demonstrated a correlation between speed change and trajectory. Therefore, it can be  
145 assumed that speed change can lead to lane departure. Yu et al. (2012) found that when a  
146 vehicle enters a curve, its path has a tendency to shift inward, but that its path tends to shift  
147 outward as it exits the curve. The influence on lane departure of upstream and downstream  
148 adjacent alignments, however, needs a systematic analysis.

149 Other factors extraneous to alignment also affect lane departure and should be  
150 considered. With a driving simulator study, Horst and Ridder (2007) showed that when  
151 roadside trees were introduced in combination with a guardrail, drivers tended to choose a  
152 position away from the guardrail and trees. When trees were introduced solely, without the  
153 guardrail, no effects on lateral position were found. Using video-image detection on a straight  
154 segment, Wang et al. (2016) found a considerable difference in lateral offset depending on  
155 lane: vehicles in the lane closest to the median tend shift to the other side of the lane (to the  
156 right, in China, where driving is on the right side of the road), apparently to keep a safe  
157 distance from the median.

158 In summary, it can be concluded that although research has been conducted on the  
159 influence of single current horizontal and adjacent alignments to lane and/or road departure,

160 there is a lack of research on the joint influence of combined alignments. Horizontal and  
161 vertical alignments complement each other, and poorly designed combinations can be unsafe  
162 and aggravate the deficiencies of each (AASHTO, 2011). For curve with frequent direction  
163 changes or large difference between maximum and minimum curvature on adjacent  
164 horizontal alignments, the scale of lane departure may be severe. Therefore, this study aims  
165 to examine the influence of these combined alignments on lane departure. Due to the limited  
166 availability of real-world data connecting lane departures to combined alignments, this study  
167 used the Tongji University driving simulator.

168

## 169 **DATA PREPARATION**

### 170 **Driving Simulator**

171 With technological developments, innovative technologies for advanced representation  
172 of motion and visual cues, cabin and control equipment, vehicle motion and environmental  
173 factors were adapted for driving simulators (Bhatti et al., 2015). Driving simulators have  
174 increasingly been used to study driving behaviors, road safety and design features (Eryilmaz  
175 et al., 2014). This study has also employed an advanced driving simulator for the purpose of  
176 data collection.

177

178 Figure 1 shows the Tongji University driving simulator used in this study. This simulator,  
179 currently the most advanced in China, incorporates a fully instrumented Renault Megane III  
180 vehicle cab in a dome mounted on an 8 degree-of-freedom motion system with an X-Y range  
181 of  $20 \times 5$  m. An immersive 5-projector system provides a front image view of  $250^\circ \times 40^\circ$  at  
182  $1000 \times 1050$  resolution refreshed at 60 Hz. SCANeR<sup>TM</sup> studio software (OKTAL) is used to  
183 display the simulated roadway environment and controls a force feedback system that  
184 acquires data from the parameters of road alignment, vehicle speed and vehicle position on  
185 the road. The overall performance of the Tongji University driving simulator has been  
186 validated by the manufacturer in three separate tests: simulator sickness, stop distance, and  
187 traffic sign size. Test results showed that the driving simulator satisfied the three criteria: 80%  
188 of drivers reported no sickness; 79% stopped within 2 meters of a designated stop line,  
189 exceeding a frequently used 75% criterion; and 75% of drivers judged traffic sign size as  
190 realistic (Wang et al., 2016).

191

192

**Insert Fig. 1 about here**

193

### 194 **Participants**

195 Drivers were chosen randomly through an open invitation (via posters and internet)  
196 where a cash reward of \$20 per hour was offered to any participant accepted for the study. It  
197 was made clear in the invitation that participants must meet certain criteria in order to qualify  
198 for the experiment. Because driver factors such as age and accumulated driving years may  
199 decrease or increase lane departure behavior, drivers younger than 20 and older than 60 were

200 excluded. They were required to be in possession of a valid driver's license; had a cumulative  
201 driving distance of at least 10,000 km and an average annual driving distance of at least  
202 3,000 km; had no criminal record, nor any record of mental illness or drug use; and had no  
203 physical conditions such as heart disease or frequent headaches. During the experiment's  
204 pre-briefing session, participants were informed of the purpose of the study and their option  
205 to end the experiment if they felt sick when driving.

206 A total of 30 drivers were employed in the analysis, a sample similar to that of most  
207 simulator studies, which had employed fewer than 30 drivers (Richard, 2007; Yu, 2012;  
208 Tarko, 2012). Their ages ranged from 24 to 58 years (with a mean age of 36.3 years and a  
209 standard deviation of 8.7 years), and 3 were female and 27 were male. Wary of the gender  
210 imbalance, we estimated two models: (1) all participants (n=30) and (2) male participants  
211 (n=27). As no difference was found in the results, we retained the n=30 model.

212

### 213 **Experimental procedure**

214 The experimental sessions consisted of three phases: preparation, warm-up, and test.  
215 During the preparation phase, participants were informed of the experiment's content, and  
216 they completed a questionnaire covering their basic demographic information and driving  
217 experience. The warm-up phase entailed a 10-minute dry-run drive to ensure that participants  
218 familiarized themselves with the simulator. The final test phase consisted of two driving tasks:  
219 one for the northbound (outbound) direction of the freeway segment and the other for the  
220 southbound (inbound). In both directions, dry pavement conditions in daylight were ensured  
221 with a free-flow traffic condition. The average duration of the test driving for each driver in  
222 the simulator was 35 minutes. Participants were asked to drive as naturally as possible. After  
223 the experiment, all drivers were asked to complete a second short questionnaire about their  
224 experience during the experiment. Over 85% of the drivers reported that the driving  
225 conditions and road scenarios were realistic.

226

### 227 **Geometric Design**

228 The simulated road was a 24-km four-lane divided mountainous freeway in the  
229 southwest of China. The road was designed under China's 2006 MOT specifications for  
230 highway alignment, with a design speed of 100 km/h. The simulated stretch of the freeway  
231 consisted of horizontal curves with small radii and long downslopes, for a total of 71 vertical  
232 and horizontal combined alignments. The longitudinal grades of these alignments ranged  
233 from -6.0% to +4.0% in the outbound direction (i.e. -4% to 6% inbound) and the values of  
234 the horizontal curvatures ranged from 0 to 2.5 km<sup>-1</sup>. The cross-section was 24.5 meters wide  
235 with a lane width of 3.75 meters and shoulder width of 2.5 meters. These measurements are  
236 schematically shown in Figure 2.

237

238

**Insert Fig. 2 about here**

239

240 To gather all relevant data, the road line was divided into 5-m spatial segments



241 according to the length of the vehicle. Horizontal and vertical geometrical parameters, as well  
242 as vehicle speed, were acquired for every 5-m segment. To determine the relationship  
243 between lane departure event (in every 5-m segment) and the geometric characteristics of the  
244 adjacent alignments, both upstream and downstream adjacent alignments were divided into  
245 50-m, 100-m, 150-m, 200-m, 300-m and 400-m segments as shown in Figure 2. Variables  
246 with different lengths on the upstream or downstream alignments were also acquired, e.g.  
247 difference in curvature within 300-m upstream alignment. A total of 143 independent  
248 variables were explored to determine their relationships with lane departure. Descriptive  
249 statistics for data elements used in this study are shown below in Table 1.

250  
251 **Insert Table 1 about here**  
252

### 253 **Lane Departure**

254 Lane offset is defined as the distance between the lane center line and the vehicle center  
255 line. The width of the vehicle used in the simulator is 220 cm and the lane width is 375 cm.  
256 The offset threshold for lane departure is therefore 77.5 cm, which is shown in Figure 3.

257  
258 **Insert Fig. 3 about here**  
259

260 We considered that there might be two perspectives for analysis. Categories for  
261 all-alignments (straight alignments and curves, termed as Dataset I) are *lane departure to the*  
262 *left*, *lane departure to the right* and *lane keeping*. For subset of curves-only (termed as  
263 Dataset II), they are *lane departure to the inside*, *lane departure to the outside* and *lane*  
264 *keeping*. In some cases the inside of a curve is on the left, but in other cases it's on the right.  
265 Each data set was used to build a separate model. We suspected that combined the results of  
266 the models of two data sets, the impact of alignments on lane departure could accurately be  
267 revealed.

268 The categories of lane departure are shown in Figure 4. The percentage values for  
269 all-alignments behaviors are lane keeping, 90.4%; lane departure to the left, 4.1%; and lane  
270 departure to the right, 5.5%. For curves-only, the values are lane keeping, 87.9%; lane  
271 departure to the inside, 10.1%; and lane departure to the outside, 2.0%.

272  
273 **Insert Fig. 4 about here**

274 **Insert Table 2 about here**  
275

276 There were 697 lane departure behaviors to the left with an average length of 65.5  
277 meters and 750 departure behaviors to the right with an average length of 82.0 meters in  
278 Dataset I (Table 2). A single lane departure behavior was acquired at every 5-m segment (one  
279 event). Therefore, only one lane departure event from a single lane departure behavior was  
280 considered in the development of the model so as to avoid the inherent correlation. Lane

281 departure events were randomly selected by the software, along with a similar proportion (i.e.  
 282 6.87%<sup>1</sup>) of lane keeping for the all-alignments model (Dataset I). For the curves-only model  
 283 (Dataset II), 943 lane departure events to the inside and 217 departure events to the outside  
 284 were randomly selected, with a similar proportion (i.e. 7.39%<sup>2</sup>) of lane keeping.  
 285

## 286 MODELLING METHODOLOGY

287 Since the dependent variable, *occurrence of lane departure*, is a nominal categorical  
 288 variable, the most appropriate statistical method is a multinomial logit model (Horowitz,  
 289 1980). This is the most practical discrete choice model in which we assume a sample of  $N$   
 290 drivers with the choice of  $J$  alternatives on  $T$  choice occasions or making their choices at  $T$   
 291 time periods. The utility that a decision maker  $n$  choosing alternative  $i$  on a choice occasion  $t$   
 292 has two parts: (i) representative or observed utility (i.e.  $V_{nit}$ ) and (ii) a random component  
 293 (i.e.  $\varepsilon_{nit}$ ) denoted as:

$$294 \quad U_{nit} = V_{nit} + \varepsilon_{nit} \quad (1)$$

295 In which the random component captures the unobserved factors that are not included in  
 296 the observed utility. The multinomial logit (MNL) model is therefore derived by assuming  
 297 that each  $\varepsilon_{nit}$  is independently and identically distributed (IID) extreme value known as  
 298 Gumbel and type I extreme value distribution (Train, 2003). The probability that a decision  
 299 maker  $n$  chooses alternative  $i$  on a choice occasion  $t$  can be expressed as:

$$300 \quad P_{nit} = \text{Prob}(U_{nit} > U_{njt}) \quad \forall j \neq i \quad (2)$$

301 The logit choice probabilities are obtained by the following formula:

$$302 \quad P_{nit} = \frac{\exp(V_{nit})}{\sum_{j=1}^J \exp(V_{njt})} = \frac{\exp(\beta X_{nit})}{\sum_{j=1}^J \exp(\beta X_{njt})} \quad j = 1, 2, 3, \dots, J \quad (3)$$

303 As multiple lane departures (i.e. choice occasions  $T$ ) were performed by each of the  
 304 drivers participating in the simulation experiment (average 48 per driver), unobserved  
 305 individual-level correlated effects and heterogeneity (i.e. taste variations) should be taken  
 306 into account. However, the MNL model assumes that the random components of the utilities  
 307 of different choice alternatives are IID and does not allow taste variations. The mixed  
 308 multinomial logit (MMNL) model offers significant advantages over the MNL model (e.g.  
 309 McFadden and Train, 2000) by allowing for random taste variation across drivers in their  
 310 sensitivities to contributory factors such as combined alignments and speed on lane  
 311 departures.

312 The random-parameters formulations of the MMNL model employs integration of the  
 313 standard MNL choice probabilities over the assumed distribution of the random taste  
 314 coefficients in that the probability of  $n$  driver choosing alternative  $i$  on a choice occasion  $t$  is  
 315 given by:

$$316 \quad P(y_{nt} = i) = \int \frac{\exp(\beta_n X_{nit})}{\sum_{j=1}^J \exp(\beta_n X_{njt})} f(\beta|\theta) d\beta \quad (4)$$

<sup>1</sup> 6.87% =  $\left(\frac{5}{65.5} + \frac{5}{82.0}\right) / 2$ ; Length of each segment = 5m; see Table 2 for other values

<sup>2</sup> 7.39% =  $\left(\frac{5}{72.8} + \frac{5}{63.2}\right) / 2$ ; Length of each segment = 5m; see Table 2 for other values

317 where  $f(\beta|\theta)$  is a density function where  $\theta$  is the vector of parameters to be estimated  
318 that represents, for instance, the mean and standard deviation of a contributory factor.

319 The primary drawback of the MMNL model relates to the fact that the integrals  
320 representing the choice probabilities as shown in Equation (4) do not have a closed-form  
321 expression and need to be approximated through simulation. One of the efficient simulation  
322 techniques is the Halton sequence (Bhat, 2003; Halton, 1960). This is a relatively  
323 straightforward type of a quasi-Monte Carlo approach and has the advantage of cost saving  
324 over the use of pseudo-random draws. Therefore the Halton sequence was employed in  
325 estimating the parameters of the MMNL model.

326 Both MNL and MMNL models were initially estimated with statistical package STATA.  
327 As discussed, the dependent variable in both models has three nominal categories for each of  
328 the two data sets in which Dataset I represents all-alignments lane departures and Dataset II  
329 represents the sub-set data related to lane departures at curves only. Although the set of  
330 statistically significant variables was found to be almost the same in both models for the two  
331 data sets, the various goodness of fit statistics (log-likelihood ratio index, log-likelihood at  
332 convergence, and the accuracy of predicted probabilities) suggested that the MMNL model  
333 performs better than the standard MNL model for both data sets. This implies the existence of  
334 a significant level of heterogeneity in tastes, especially with respect to speed at the upstream  
335 and curve direction (i.e. left or right), characterized by fixed (deterministic) and random  
336 driver-level variation. Therefore, model interpretation and further discussion are based on the  
337 findings from the MMNL model.

338

## 339 **MODELING RESULTS**

340 In Dataset I (all-alignments), choice alternatives or lane departure categories are *lane*  
341 *keeping* (i.e.  $Y_{ni}=1$ ), *lane departure to the left* (i.e.  $Y_{ni}=2$ ) and *lane departure to the right* (i.e.  
342  $Y_{ni}=3$ ); In Dataset II (curves-only), the categories are *lane keeping* (i.e.  $Y_{ni}=1$ ), *lane departure*  
343 *to the inside* (i.e.  $Y_{ni}=2$ ) and *lane departure to the outside* (i.e.  $Y_{ni}=3$ ). *Lane keeping* is the  
344 reference category for both data sets.

345 A total of 143 explanatory variables, as discussed in the data preparation section, were  
346 examined. In selecting the final set of variables, many were found to be statistically  
347 insignificant at the 95% confidence interval, then the insignificant variables were taken out  
348 from the final model (a variable was removed if its p-value was more than 0.05). With the aid  
349 of the correlation coefficient matrix, many variables were found to be correlated with each  
350 other (e.g. difference in curvature within 200-m upstream and 300-m upstream, shown in  
351 Table 2). For these correlated variables, we employed the variables one by one respectively  
352 and many models were separately estimated. With the examination of their levels of  
353 statistical significance through the p-values and the models' goodness of fit (i.e. the log  
354 likelihood function at convergence, a larger value of log likelihood indicates a better model),  
355 the final set of explanatory variables was attained. We done this process manually rather than  
356 using a computer program. To ascertain whether the coefficient of an independent variable  
357 was randomly distributed over the observations, a normal distribution was assumed. If the

358 mean and the standard deviation of a coefficient were statistically significant, the variable  
359 was considered to follow a random distribution.

360

### 361 **Dataset I (all-alignments) results for lane departures to the left and right**

362 Six variables in the all-alignments dataset were found to be statistically significant at the  
363 95% confidence interval. These consisted of three categorical variables and three continuous  
364 variables. The three categorical variables were: 1) curve direction at the current segment (left  
365 vs straight; right vs straight); 2) driving lane (Lane 1 borders the median and Lane 2 borders  
366 the hard shoulder); and 3) slope type (upward  $\geq +2\%$  vs flat; downward  $\leq -2\%$  vs flat). The  
367 three continuous variables were: 1) horizontal curvature at the current segment; 2) difference  
368 in horizontal curvature (max-min) within the 300-m upstream adjacent alignment; and 3) the  
369 average speed within the 300-m upstream adjacent alignment. The 300-m adjacent segment  
370 had the best level of significance as compared to the other segment lengths, based on p-values  
371 and the models' goodness of fit. The results are presented in Table 3 below.

372

373

**Insert Table 3 about here**

374

375 For both left and right lane departures, estimated parameters for curve direction at the  
376 current segment and average speed within 300-m upstream segment were found to be  
377 randomly distributed by driver. This indicated that driver behavior was not consistent for the  
378 effect of curve direction and average speed on lane departure to the left or the right.

379 More specifically, in the *lane departure to the left* category, the mean parameter of the  
380 left-turn curve variable was found to be +2.463 with a standard deviation of 1.625, indicating  
381 that the impact of the left-turn curve variable on the probability of lane departure to the left  
382 might have a mixed effect. Since the standard deviation of the coefficient is quite large  
383 relative to the mean value of the coefficient, there is a high possibility that some of the  
384 coefficients would be negative. Since the coefficient was assumed to follow a normal  
385 distribution, the Z-statistic was obtained to calculate the area under the normal curve between  
386 the mean (i.e. 2.463) and 0 as follows:

$$387 \quad Z = \frac{0 - 2.4635}{1.625} = -1.52 \quad (5)$$

388  $Z=1.52$  represents 43.57% of the area under the normal curve. This means that 43.6% +  
389 50% = 93.6%, of drivers show a positive sign, indicating that left-turn curves have positive  
390 influence on probability of lane departure to the left for the 93.6% of drivers whereas 6.4% of  
391 drivers on the left-turn curves exhibit a negative sign, implying that left-turn curves are  
392 negatively associated with the probability of lane departure to the left. Therefore, it can be  
393 said that driver behavior with respect to driving on a left-turn curve is not consistent.

394 Variables with fixed parameters show only positive or negative probability for all  
395 drivers (e.g., driving on a right-turn curve, all drivers showed a negative sign for departing to  
396 left, at the 95% confidence interval).

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**Interpretation of the variables included in the model for Dataset I**399  
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As compared to driving on a straight segment, driving on a right-turn curve was found to be with a fixed parameter that had a significant positive impact on *lane departure to the right* and a negative impact on *lane departure to the left*. The distributed parameter of left-turn curve had a mean of 2.463 and standard deviation of 1.625 for *lane departure to the left*, and a mean of -1.047 and standard deviation 0.794 for *lane departure to the right*. According to the same approach (Equation 5), this implied that driving on a left-turn curve increased *lane departure to the left* for 93.6% of the drivers, with 6.4% exhibiting the opposite behavior, i.e., a decrease in *lane departure to the left*. For 90.6% of the drivers, driving on a left-turn curve decreased the likelihood of *lane departure to the right*, while it was increased for 9.4%.

The parameters of vertical slope were fixed, and negatively associated with lane departure. Downward slope less than -2% (vs flat segment) had a significant negative impact on *lane departure to the left*. Upward slope, as compared to flat segment, was found to decrease the risk for *lane departure to the right*. This suggested that drivers were more cautious when driving on downslope and upslope.

The simulated road was a four-lane (two lanes in each direction) divided mountainous freeway on which Lane 1 borders the median on the left, and Lane 2 borders the hard shoulder on the right. Its estimated parameter was fixed across drivers. The risk of *lane departure to the left* on Lane 2 (bordering the shoulder) was higher than that of Lane 1, while Lane 1's risk for *lane departure to the right* was greater. This finding was expected, as drivers might reasonably avoid fixed impediments such as shoulders and medians.

Curvature is normally a scalar quantity that takes into account the bending of horizontal curve. Horizontal curve that bend more sharply has higher curvature. Driver behavior was consistent for the effect of horizontal curvature at the current segment, i.e. the probability of both *lane departures to the right* and *to the left* were found to be positively influenced by the horizontal curvature.

The difference between maximum and minimum horizontal curvature (1/km) within the 300-m upstream adjacent alignment was also found to be with a fixed parameter. As with horizontal curvature at the current segment, a large curvature difference significantly increased the likelihood of both *lane departure to the right* and *lane departure to the left*.

Average vehicle speed within the 300-m upstream adjacent alignment (*AvgSpeedU300*) was a significant variable for lane departure. Considering the random parameter, it was found that a greater average vehicle speed had a positive effect on *lane departure to the left* for 98.1% of the drivers, while for 1.9% it decreased the probability. Average vehicle speed within the 300-m upstream adjacent alignment had a negative effect on *lane departure to the right* for 91.6% of the drivers, but increased the probability for 8.4%. Since cliffs often appear to the right of the hard shoulder on mountainous freeways in China (where vehicle traffic keeps to the right side of the road), drivers are more likely to depart to the left from their lanes so as to avoid running off the road.

**440 Dataset II (curves-only) results for lane departures to the inside and outside**

441 For lane departures to the inside and outside of a curve, five variables were found to be  
442 statistically significant at the 95% confidence interval. The two categorical variables were: 1)  
443 driving lane, (Lane 1 borders the median and Lane 2 borders the hard shoulder), and 2) slope  
444 type (upward  $\geq +2\%$  vs flat; downward  $\leq -2\%$  vs flat). The three continuous variables were: 1)  
445 horizontal curvature at the current segment; 2) the difference in horizontal curvature  
446 (max-min) within the 300-m upstream adjacent alignment; 3) speed at the current segment.  
447 The results are presented in Table 4.

448 The parameters found to be random were driving lane and vehicle speed at the current  
449 segment. This indicated that the effects of these two variables on drivers' lane departures to  
450 the inside and outside of the curve were not consistent.

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**Insert Table 4 about here**

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**Interpretation of the variables included in the model for Dataset II**

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456 Vertical slope was found to be with a fixed parameter that significantly decreased the  
457 likelihood of *lane departure to the inside*. Both downward slope of less than -2% (vs flat  
458 segment) and upward slope of more than 2% (vs flat segment) had negative impact on *lane  
459 departure to the inside*.

459 The Lane 1 parameter (bordering the median, vs Lane 2) was distributed with a mean of  
460 -0.335 and standard deviation of 0.527 for *lane departure to the inside*, and a mean of -1.152  
461 and standard deviation of 0.637 for *lane departure to the outside*. This implied that on curves,  
462 driver behavior was not consistent with respect to the effect of driving lane.

463 The parameters of both horizontal curvature at the current segment and the difference in  
464 curvature within the 300-m upstream adjacent segment were found to be fixed. Both variables  
465 (parameters) had a positive impact on *lane departure to the inside*.

466 The parameter of speed at the current segment was distributed with a mean of 0.226 and  
467 standard deviation of 0.014, i.e. greater speed at the current segment significantly increased  
468 *lane departure to the outside* for 96.9% drivers. High traveling speed on the curve would  
469 seem to make it easy for a driver to slip to the outside.

**470 DISCUSSION**

471 When the results of the models of two data sets are combined, the impact of alignments  
472 on lane departure can be revealed accurately. In results of two models, horizontal curvature at  
473 the current segment, difference in horizontal curvature in adjacent segments, and downward  
474 and upward slope were all found to be with fixed parameters, indicating that driver behavior  
475 was consistent for the effect of these variables on lane departure. *Curve\_Direction* (left-turn  
476 curve or right-turn curve) had significant effect on lane departure to the left and the right  
477 (model results of Dataset I), but had no significant effects on lane departure to the inside or  
478 outside (model results of Dataset II). Moreover, the proportion of inside lane departures is  
479 81.3%, much larger than departures to the outside (18.7%) and the average speed is the  
480 lowest during inside departures (Table 5). These results suggest that drivers tend to avoid the

481 possibility of running off the curve by decelerating (Yu, et al., 2012).

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**Insert Table 5 about here**

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485 The difference between maximum and minimum horizontal curvature within 50-m,  
486 100-m, 150-m, 200-m, 300-m and 400-m segments of both upstream and downstream  
487 adjacent alignments were correlated with each other, and had similar positive effects on lane  
488 departure. Thus, twelve models for each data set, based on the 6 different adjacent segment  
489 lengths, were separately estimated while other variables in the model were held constant.  
490 With examination of their levels of statistical significance through p-value (should be less  
491 than 0.05) and models' goodness of fit (i.e. the log likelihood function at convergence, in  
492 which a larger log likelihood value indicates a better model fit), the optimum length of the  
493 immediate upstream segment was found to be 300 meters, while the difference in horizontal  
494 curvature on the downstream alignment was not statistically significant.

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Using the same p-value and goodness of fit approach, when vertical grades of upward  
slope and downward slope were defined by  $\pm 2\%$ , the influence of downward slope and  
upward slope decreased the probability of lane departure significantly. The 2015 Chinese  
*Specification for Highway Safety Audit* (MOT 2015) recommends using only two categories  
of vertical grade, namely an 'upward' grade ( $\geq 3\%$ ) and 'not upward' grade ( $< 3\%$ ). However,  
as in both of this study's models, the 2% upward slope and the -2% downward slope were  
found to be statistically significant at the 95% confidence interval, suggesting the MOT  
specifications should be adjusted.

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Previous studies on the relationship between vehicle speed and roadway geometry have  
mainly focused on the characteristics of horizontal alignments, e.g. length of tangent, length  
of tangent following the curve, horizontal curvature (Fitzpatrick and Collins, 2000; Figueroa  
and Tarko, 2007). However, downgrade was usually associated with higher speed and  
upgrade with lower speed (Montella et al., 2014). Our results found that there was no  
significant difference for speed on downward slope, upward slope and flat grades (Table 6).  
The lower than expected speed on downward slopes may be a result of the horizontal  
alignments.

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**Insert Table 6 about here**

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The coefficients presented in Tables 3 and 4 above have been employed to estimate how  
the probabilities of lane departure change with variation in the corresponding key  
explanatory variables, and thus can assist in formulating recommendations for designing the  
combined alignments common on mountainous freeways. Using the findings in Table 3, the  
probability of *lane departure to the left* can be predicted, for example, for downward slope  
along left-turn curve in lane closest to the median. Figure 5 below shows 2-D probability  
plots indicating how *lane departure to the left* varies by horizontal curvature at the current  
segment (*Curvature\_C*) and the difference in curvature within the 300-m upstream adjacent  
segment (*DiffC\_U300*). Either *Curvature\_C* or *DiffC\_U300* increases, the probability of *lane*

523 *departure to the left* increases. It is notable that lane departures to the left are more frequent  
524 when increasing the vehicle's average speed within the 300-m upstream adjacent segment  
525 (*AvgSpeedU300*) for the same *Curvature\_C* and *DiffC\_U300*. This relationship implies that if  
526 there is a need to increase speed limit or design speed, a design guideline that minimizes the  
527 curvature or the difference in horizontal curvature within the 300-m upstream adjacent  
528 segment should be recommended.

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**Insert Fig. 5 about here**

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## 532 CONCLUSIONS

533 There has been a dearth of research on how potentially dangerous lane departures are  
534 impacted by horizontal and vertical combined roadway alignments at adjacent as well as at  
535 the current segments. The objective of this study was to facilitate the design of safer  
536 combined alignments on mountainous freeways by examining the effects of these alignments  
537 on lane departure. Employing a driving simulator to create a typical four-lane mountainous  
538 freeway in China, this study selected a range of geometric characteristics associated with  
539 horizontal and vertical alignments on current, upstream and downstream segments to build a  
540 mixed multinomial logit model. Two data sets were used to build individual models:  
541 all-alignments data and the subset data of curves-only, which was able to provide a much  
542 better understanding of the effect of combined alignments on lane departure.

543 According to the results of the two data set models, the main influencing factors are  
544 horizontal curvature at the current segment, the difference in horizontal curvature within the  
545 300-m adjacent upstream alignment, and downward and upward slope. These variables have  
546 found to have a fixed effect, indicating that driver behavior is consistent in these conditions.  
547 Specifically, lane departures increase with these horizontal alignments, but decrease with  
548 downward and upward slopes. Additionally, driving in the lane closest to the hard shoulder  
549 increases the probability of lane departure. A left-turn curve has a significant positive impact  
550 on lane departure to the left, and a right-turn curve is likely to cause lane departure to the  
551 right, as drivers commonly tend to depart their lanes toward the inside of a curve.

552 The upstream adjacent segment should be considered interdependently in order to  
553 reduce potentially dangerous lane departure. The optimum length is found to be 300 meters  
554 on the immediate upstream segment. An additional finding that would assist engineers during  
555 the design stage of mountainous freeways is that when the vertical grade is divided by  $\pm 2\%$ ,  
556 the influence of slope on lane departure is significant. The effects of these factors should be  
557 given top priority in designing safer mountainous freeways with respect to lane departure.

558 This research began the study of combined and adjacent alignments on lane departure by  
559 addressing normal conditions, i.e. dry pavement conditions in daylight with a free-flow  
560 traffic condition. Future work shall investigate the problem with different conditions,  
561 including how adverse weather conditions may affect the combined alignments and the  
562 possible influence of gender with a balance of male and female drivers.

## 563 ACKNOWLEDGEMENTS



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## 567 REFERENCES

- 568 AASHTO-American Association of State Highway and Transportation Officials, 2011. A  
569 Policy on Geometric Design of Highways and Streets, 6th Ed., Washington DC.
- 570 AASHTO-American Association of State Highway and Transportation Officials, 2010.  
571 Highway Safety Manual, 1st Ed., Washington DC.
- 572 Bhat, C., 2003. Simulation estimation of mixed discrete choice models using randomized and  
573 scrambled Halton sequences. *Transport. Res. Part B: Methodological* 37 (1), 837–855.
- 574 Bhatti, G., Bremond, R., Jessel, J.-P., Dang, N.-T., Vienne, F., Millet, G., 2015. Design and  
575 evaluation of a user-centered interface to model. *Transp. Res. Part C: Emerg. Technol.*  
576 50, 3-12.
- 577 Cafiso, S., La Cava, G., 2009. Driving performance, alignment consistency, and road safety.  
578 *Transportation Research Record* 2102, 1–8.
- 579 Eryilmaz, U., Tokmak, H.S., Cagiltay, K., Isler, V., Eryilmaz, N.O., 2014. A novel  
580 classification method for driving simulators based on existing flight simulator  
581 classification standards. *Transp. Res. Part C: Emerg. Technol.* 42, 132-146.
- 582 Eustace, D., Almuntairi, O., Hovey, P.W., Shoup, G., 2014. Using decision tree modeling to  
583 analyze factors contributing to injury and fatality of run-off-road crashes in Ohio. 93rd  
584 Annual Meeting Transportation Research Board, National Academies, Washington, DC.
- 585 Figueroa, A.M., Tarko, A.P., 2007. Speed changes in the vicinity of horizontal Curves on  
586 Two-Lane Rural Roads. *Journal of Transportation Engineering* 133(4), 215–222.
- 587 Fitzpatrick, K., Collins, J.M., 2000. Speed-Profile model for two-lane rural highways. 79th  
588 Annual Meeting Transportation Research Board, National Academies, Washington, DC.
- 589 Halton, J., 1960. On the efficiency of certain quasi-random sequences of points in evaluating  
590 multidimensional integrals. *Numerische Mathematik* 2, pp. 84-90.
- 591 Horowitz, J., 1980. The accuracy of the multinomial logit model as an approximation to the  
592 multinomial probit model of travel demand. *Transport. Res. Part B: Methodological*  
593 14B(4), 331-341.
- 594 Horst, R.V., Ridder, S.D., 2007. The influence of roadside infrastructure on driving behavior:  
595 A driving simulator study. 86th Annual Meeting Transportation Research Board,  
596 National Academies, Washington, DC.
- 597 Jalayer, M., Zhou, H., 2017. Exploratory analysis of run-off-road crash patterns. 96th Annual  
598 Meeting Transportation Research Board, National Academies, Washington, DC.
- 599 Jung, C. R., Kelber, C. R., 2005. A lane departure warning system using lateral offset with  
600 uncelebrated camera. 8th International IEEE Conference on Intelligent Transportation  
601 Systems, Vienna, Austria.
- 602 Lin, Y., Niu, J., Ying, X., 2011. Study on characteristics of vehicle path in curves on two-lane  
603 highways. *Journal of Highway and Transportation Research and Development* 28(3),  
604 113-117.

- 605 Liu, C., Subramanian, R., 2009. Factor Related to Fatal Single-Vehicle Run-Off-Road  
606 Crashes. Rep. DOT-HS-811-232, U.S. Department of Transportation, Washington D.C.,  
607 pp. 7-8.
- 608 Lord, D., Brewer, M.A., Fitzpatrick, K., Geedipally, S.R., Peng, Y., 2011. Analysis of  
609 Roadway Departure Crashes on Two-Lane Rural Roads in Texas. Rep.  
610 FHWA/TX-11/0-6031-1, Federal Highway Administration, U.S. Department of  
611 Transportation, Washington D.C., pp. 3-5.
- 612 McFadden, D., Train, K., 2000. Mixed MNL Models for discrete response. *Journal of*  
613 *Applied Econometrics* 15(5), 447-470.
- 614 Miaou, S.P., Lum, H., 1993. Statistical evaluation of the effects of highway geometric design  
615 on truck accident involvements. *Transportation Research Record* 1407, 11–24.
- 616 Montella, A., Pariota, L., Galante, F., Imbriani, L.L., Mauriello, F., 2014. Prediction of  
617 drivers' speed behavior on rural motorways based on an instrumented vehicle study.  
618 *Transportation Research Record* 2434, 52-62.
- 619 MOT-Ministry of Transport of the People's Republic of China, 2006. Design Specification  
620 for Highway Alignment, China, Beijing.
- 621 MOT-Ministry of Transport of the People's Republic of China, 2015. Specification for  
622 Highway Safety Audit, China, Beijing.
- 623 NHTSA-US Department of Transportation, 2011. Run-off-road Crashes: an on-scene  
624 Perspective. Rep. American, Washington, D.C., 2011.
- 625 NHTSA-US Department of Transportation, 2016. Fatality Analysis Reporting System  
626 (FARS). <<https://www.nhtsa.gov/FARS>> (accessed May 1, 2016).
- 627 OKTAL, 2017. SCANeRTM Studio Software. <  
628 <http://www.oktal.fr/en/automotive/range-of-simulators/software> > (accessed July 30,  
629 2017).
- 630 Spacek, P., 2005. Track behavior in curve areas: Attempt at typology. *Journal of*  
631 *Transportation Engineering* 131(9): 669-676.
- 632 Tarko, A., 2012. Use of crash surrogates and exceedance statistics to estimate road safety.  
633 *Accident Analysis and Prevention* 45(3), 230-240.
- 634 Torbic, D.J., Harwood, D.W., Gilmore, D.K., Pfefer, R., Neuman, T.R., Slack, K.L., Hardy,  
635 K.K., 2004. Guidance for Implementation of the AASHTO Strategic Highway Safety  
636 Plan- A Guide for Reducing Collisions on Horizontal Curves. Rep. NCHRP Report 500,  
637 Transportation Research Board of the National Academies, Washington, DC.
- 638 Train, K., 2003. *Discrete Choice Methods with Simulation*. Cambridge University Press,  
639 Cambridge, UK.
- 640 Transportation Research Board, 2011. Evaluation of Data needs, Crash Surrogates, and  
641 Analysis Methods to Address Lane Departure Research Questions Using Naturalistic  
642 Driving Study Data. Rep. American, Washington, DC.
- 643 Wang, X., Wang, T., Tarko, A., Tremont, P.J., 2015. The influence of combined alignments on  
644 lateral acceleration on mountainous freeways: A driving simulator study. *Accident*  
645 *Analysis and Prevention* 76(3), 110-117.
- 646 Wang, X., Zhu, M., Chen, M., Tremont, P., 2016. Drivers' rear end collision avoidance

- 647 behaviors under different levels of situational urgency. *Transport. Res. Part C: Emerg.*  
648 *Technol.* 71: 419-433.
- 649 Wang, Y., Yang, S., Pan, B., 2016. Research of vehicle track in highway straight section.  
650 *Journal of Highway and Transportation Research and Development* 33(2): 111-119.
- 651 Wu, X., Wang, X., Lin, H., He, Y., Yang, L., 2013. Evaluating alignment consistency for  
652 mountainous expressway in design stage: A driving simulator-based approach. 92nd  
653 Annual Meeting Transportation Research Board, National Academies, Washington, DC.
- 654 Xu, J., Luo Q., Mao, J., Shao, Y., 2013. Method for horizontal geometry design of  
655 mountainous roads based on trajectory-speed cooperative control. *China Journal of*  
656 *Highway and Transport* 26(04): 43-56+71.
- 657 Yu, H., Zhang, Z., Li, D., 2012. The impact of driving speed on driving track on low-grade  
658 highways in mountain area. *Journal of Transport Science and Engineering* 28(4), 59-64.
- 659 Zhou, X., 2010. Approach and application of steering on freeway curve based on visual edge  
660 rate. Master thesis, Wuhan University of Technology, China.
- 661

- 662 **Fig. 1** Tongji driving simulator and experiment simulation scene
- 663 **Fig. 2** Road cross section and segment length
- 664 **Fig. 3** Schematic diagram of a lane departure scenario
- 665 **Fig. 4** Lane departure classifications for the two datasets
- 666 **Fig. 5** Estimated probabilities for departure to the left with left-turn curve and
- 667 downward-slope on Lane 1 (vertical grade  $\leq -2\%$ )
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669 **Table 1 Descriptive statistics for alignment variables and vehicle operational data**  
670 **Table 2 Lane departure statistics**  
671 **Table 3 Modelling results for Dataset I (all-alignments) for lane departures to**  
672 **the left and right**  
673 **Table 4 Modelling results for Dataset II (curves-only) for lane departures to the**  
674 **inside and outside**  
675 **Table 5 Statistics for speed of lane departure on a curve**  
676 **Table 6 Statistics for speed on slope**

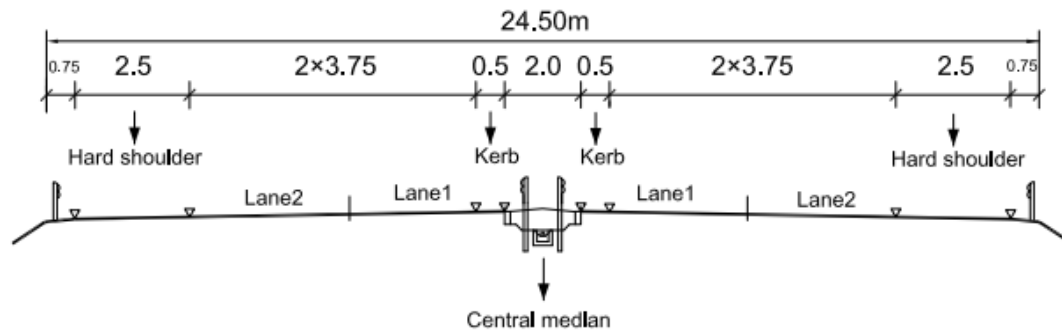
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**Fig. 1 Tongji driving simulator and experiment simulation scene**

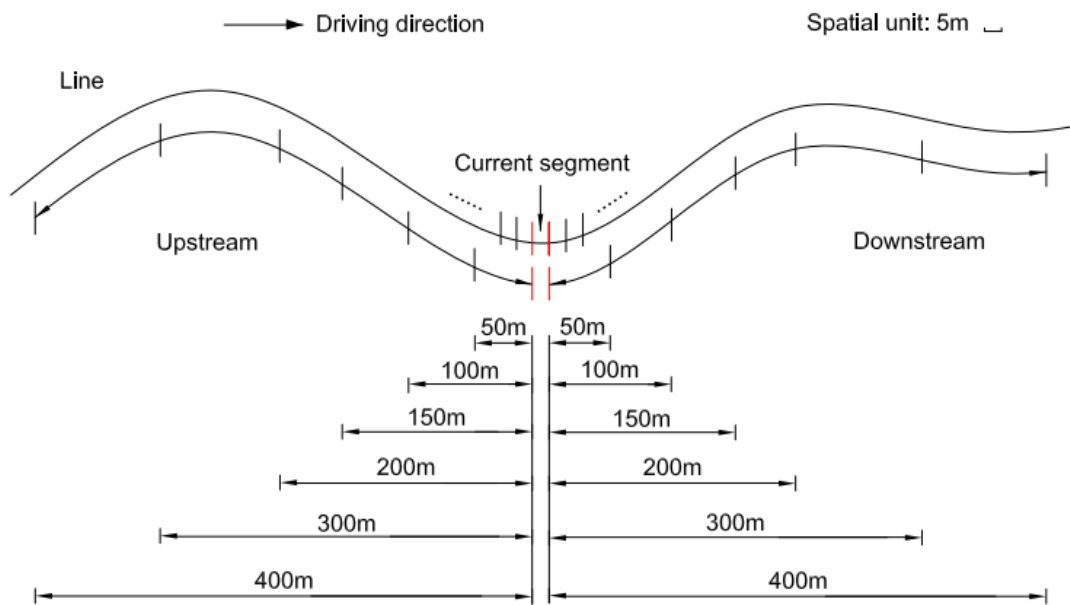
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(a) Cross-section



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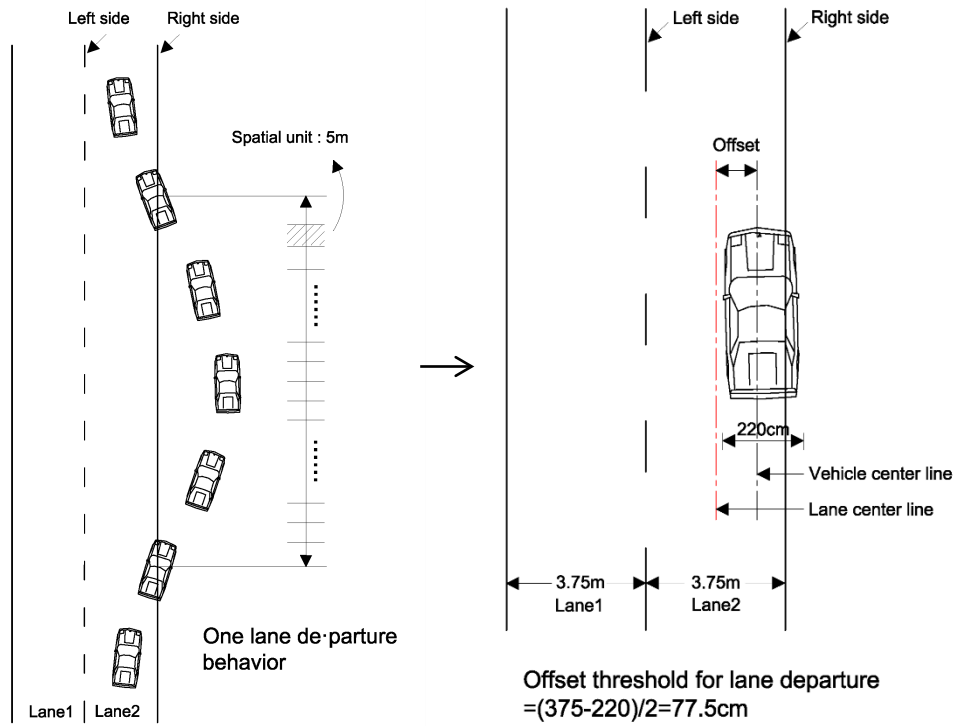
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(b) A bird's-eye view of a stretch of the studied road alignment

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**Fig. 2 Road cross section and segment length**

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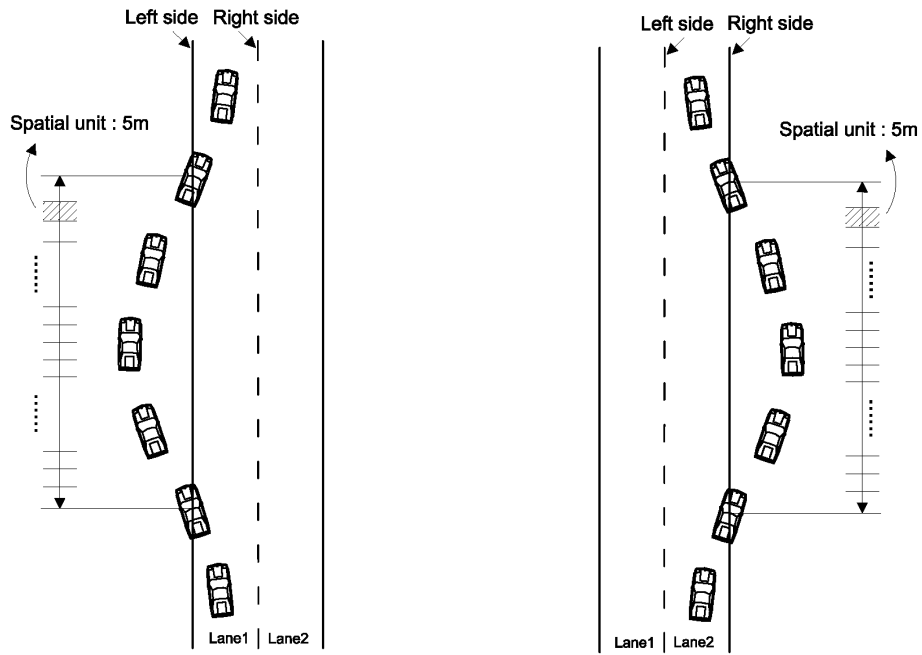
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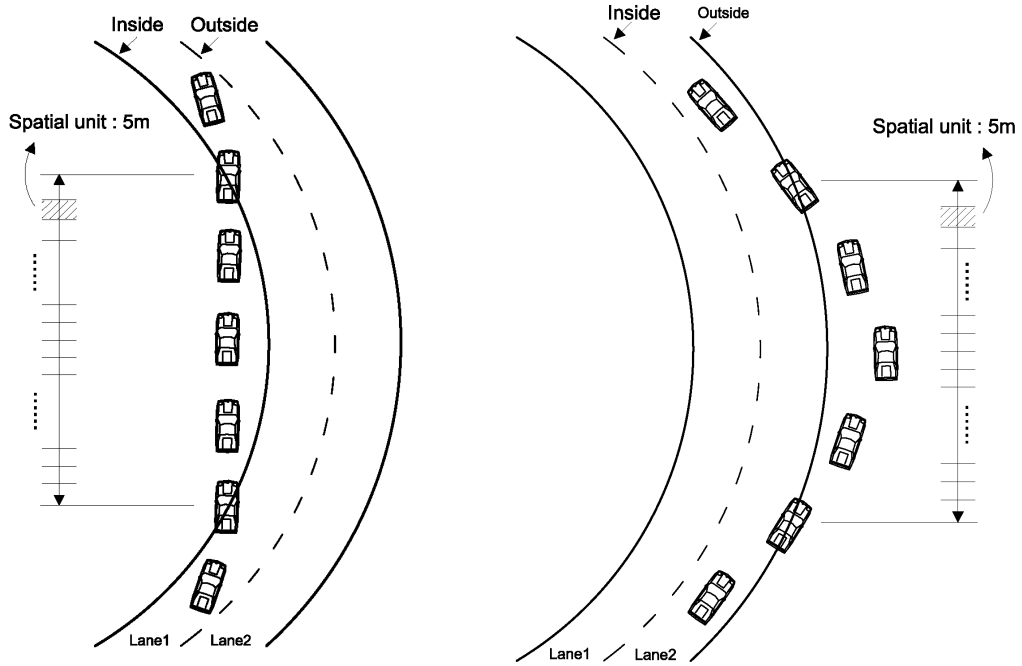
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**Fig. 3 Schematic diagram of a lane departure scenario**





(a) Lane departure to the left (all-alignments) (b) Lane departure to the right (all-alignments)

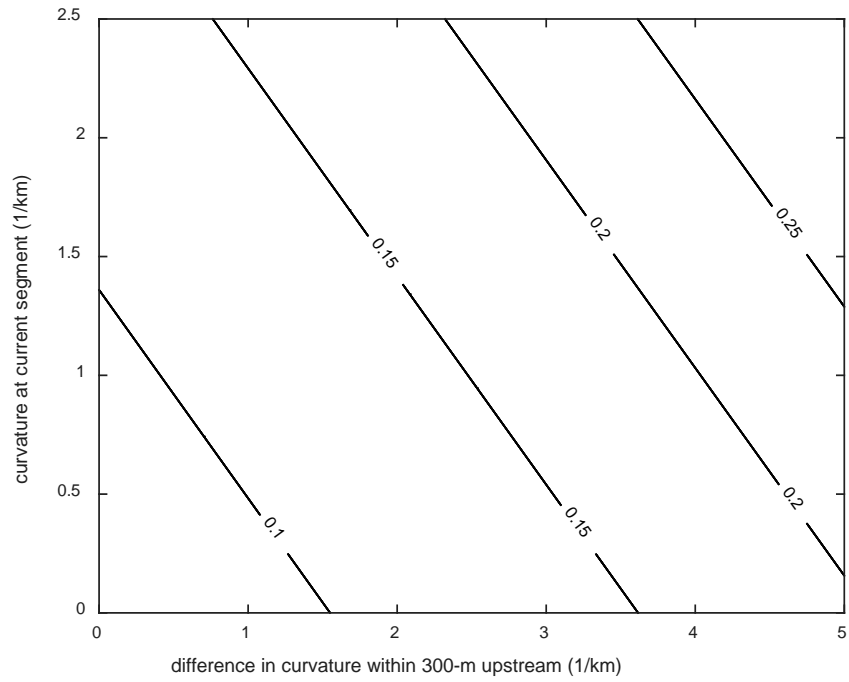


(c) Lane departure to the inside (curves-only) (d) Lane departure to the outside (curves-only)

**Fig. 4 Lane departure classifications for the two datasets**

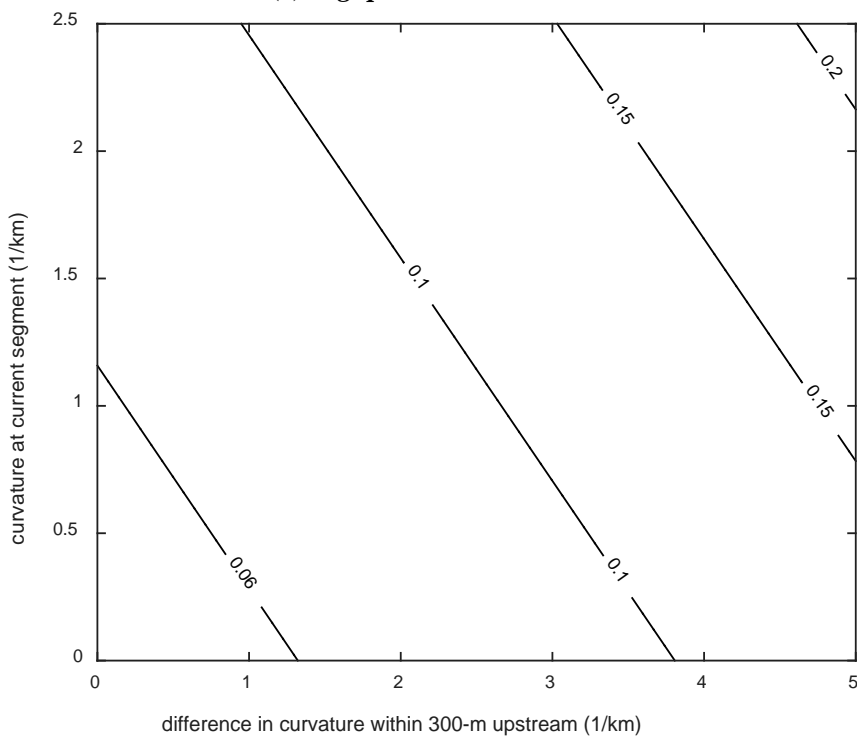
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(a) *AvgSpeedU300*=100km/h



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(b) *AvgSpeedU300*=80km/h

**Fig. 5 Estimated probabilities for departure to the left with left-turn curve and downward-slope on Lane 1 (vertical grade  $\leq -2\%$ )**

698 **Table 1 Descriptive statistics for alignment variables and vehicle operational data**699 (a) **Continuous variables**

Variables	Description	Mean	S.D	Min	Max
<b>Characteristics of the current segment</b>					
Curvature_C	Absolute value of the horizontal curve at the current segment (1/km)	0.33	0.62	0.0	2.5
Grade_Northbound	Longitudinal grade of the vertical alignment	-0.0053	0.025	-0.06	0.04
Grade_Southbound	Longitudinal grade of the vertical alignment	0.0053	0.025	0.06	-0.04
Speed	Driving speed at the current segment (km/h), design speed is 100 km/h	96.31	11.04	50.54	143.41
Visibility	Visibility distance at the current segment (m)	274.78	116.65	47.50	420.00
<b>Characteristics of adjacent upstream and downstream alignments</b> (Using 300-m upstream adjacent alignment as an example)					
AvgC_U300	Average horizontal curvature(1/km)	0.33	0.62	0.0	2.5
MaxC_U300	Maximum horizontal curvature(1/km)	0.64	0.79	0.0	2.5
MinC_U300	Minimum horizontal curvature(1/km)	0.28	0.78	0.0	2.4
DiffC_U300	Difference between maximum and minimum horizontal curvature (1/km)	1.17	1.23	0.00	5.00
NumC_U300	Number of successive curves	1.53	1.16	0	5
AvgS_U300	Average vertical grade	-0.5%	2.5%	-6.0%	4.0%
MaxS_U300	Maximum vertical grade	0.4%	2.0%	-6.0%	4.0%
MinS_U300	Minimal vertical grade	-1.5%	2.6%	-6.0%	3.0%
DiffS_U300	Difference between maximum and minimum vertical grade	1.9%	1.9%	0.0%	8.1%
PuS_U300	Proportion of upward slope	48.4%	45.0%	0.0%	100%
PdS_U300	Proportion of downward slope	51.6%	45.0%	0.0%	100%
AvgSpeedU300	Average driving speed within 300-m upstream adjacent segment (km/h)	96.30	10.78	51.07	143.11

700 (b) **Categorical variables**

Variables	Description	Statistic
Curve_Direction	Straight segment, left-turn curve or right-turn curve of the road	Straight: 30.94%
		Left-turn: 31.32%
		Right-turn: 37.73%
Horizontal_Type	Type of horizontal alignment: tangent; circular curve; approach transition curve; departure transition curve	Tangent: 37.7%;
		Circular curve: 32.7%
		Approach transition curve: 14.8%
		Departure transition curve: 14.6%
Lane	Travelling lane: Lane 1 borders the median and Lane 2 borders the hard shoulder	Lane 1: 76.17%
		Lane 2: 23.83%
Slope_Type (outbound)	Slope type of the segment: flat grades	Level or flat: 61%

	(-2% ≤ grade ≤ 2%), downward slope (< -2%), upward slope (> 2%); four additional degrees of slope type were considered, with thresholds ±1%, ±1.5%, ±2.5% and ±3%	Downward slope (< -2%): 25.6%
		Upward slope (> 2%): 13.4%

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702

703 **Table 2 Lane departure statistics**

Statistic		Dataset I (all-alignments)		Dataset II (curves-only)	
		<i>Lane departure to the left</i>	<i>Lane departure to the right</i>	<i>Lane departure to the inside</i>	<i>Lane departure to the outside</i>
Departure behaviors (30 drivers)		697	750	943	217
Average length of lane departure along the road (m)		65.5	82.0	72.8	63.2
Lane offset value (m)	Min	0.775	0.775	0.775	0.775
	Max	2.156	2.285	2.243	2.281
	Mean	0.939	1.010	0.980	1.003

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705

706 **Table 3 Modelling results for Dataset I (all-alignments) for lane departures to**  
 707 **the left and right**

<i>Variables</i>	<i>Parameters</i>	<i>Std. Err</i>	<i>p-value</i>
<b>Category 1: Lane Keeping (reference)</b>			
<b>Category 2: Lane Departure to the Left</b>			
Driving on a left-turn curve (vs straight segment), standard deviation for random parameter	2.463 (1.625)	0.164 (0.102)	0.000 (0.000)
Driving on a right-turn curve (vs straight segment), fixed parameter	-0.351	0.169	0.034
Downward slope <-2%(vs flat segment), fixed parameter	-0.230	0.102	0.024
Driving in Lane 1 (bordering the median) vs Lane 2 (bordering the shoulder), fixed parameter	-2.262	0.109	0.000
Horizontal curvature at the current segment, fixed parameter	0.226	0.078	0.004
Difference in curvature (max-min) within 300-m upstream adjacent segment, fixed parameter	0.257	0.040	0.000
Average speed within 300-m upstream adjacent segment, standard deviation for random parameter	0.025 (0.012)	0.004 (0.001)	0.000 (0.000)
Alternative specific constant, fixed parameter	-5.011	0.449	0.000
<b>Category 3: Lane Departure to the Right</b>			
Driving on a left-turn curve, standard deviation for random parameter	-1.047 (0.794)	0.159 (0.145)	0.000 (0.000)
Driving on a right-turn curve, fixed parameter	0.678	0.120	0.000
Upward slope ">2%" vs flat segment, fixed parameter	-0.205	0.092	0.026
Driving in Lane 1 (bordering the median) vs Lane 2 (bordering the shoulder), fixed parameter	1.691	0.211	0.000
Horizontal curvature at the current segment, fixed parameter	0.211	0.080	0.008
Difference in curvature (max-min) within 300-m upstream adjacent segment, fixed parameter	0.162	0.037	0.000
Average speed within 300-m upstream adjacent segment, standard deviation for random parameter	-0.018 (0.013)	0.004 (0.001)	0.000 (0.000)
Intercept	-4.075	0.424	0.000
<b>Overall</b>			
Number of events	53001		
Log-likelihood	-4893.7139		
LR chiSq	726.76		
Prob> chiSq	0.000		

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710 **Table 4 Modelling results for Dataset II (curves-only) for lane departures to the**  
 711 **inside and outside**

<i>Variables</i>	<i>Parameters</i>	<i>Std. Err</i>	<i>p-value</i>
<b>Category 1: Lane Keeping (reference)</b>			
<b>Category 2: Lane Departure to the Inside</b>			
Downward slope <-2% (vs flat segment), fixed parameter	-0.201	0.088	0.022
Upward slope ">2%" (vs flat segment), fixed parameter	-0.162	0.081	0.047
Driving in Lane 1 (bordering the median) vs Lane 2 (bordering the shoulder), standard deviation for random parameter	-0.355 (0.527)	0.107 (0.063)	0.001 (0.000)
Horizontal curvature at the current segment, fixed parameter	0.192	0.061	0.002
Difference in curvature (max-min) within 300-m upstream adjacent segment, fixed parameter	0.281	0.031	0.000
Intercept	-2.736	0.108	0.000
<b>Category 3: Lane Departure to the Outside</b>			
Driving in Lane 1 (bordering the median) vs Lane 2 (bordering the shoulder), standard deviation for random parameter	-1.152 (0.637)	0.301 (0.131)	0.000 (0.000)
Speed at the current segment, standard deviation for random parameter	0.026 (0.014)	0.009 (0.002)	0.003 (0.000)
Intercept	-5.957	0.860	0.000
<b>Overall</b>			
Number of events	33783		
Log-likelihood	-3989.00		
LR chiSq	260.69		
Prob> chiSq	0.000		

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714 **Table 5 Statistics for speed of lane departure on a curve**

<b>Category</b>	<b>Speed(km/h)</b>					
	<b>Mean</b>	<b>Min</b>	<b>15%</b>	<b>50%</b>	<b>85%</b>	<b>Max</b>
Lane keeping	94.9	51.7	84.0	95.2	105.3	143.0
Lane departure to the inside	92.8	50.5	82.5	93.7	102.0	129.6
Lane departure to the outside	97.3	67.4	89.6	97.3	104.4	123.3

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716 **Table 6 Statistics for speed on slope**

<b>Category</b>	<b>Speed(km/h)</b>					
	<b>Mean</b>	<b>Min</b>	<b>15%</b>	<b>50%</b>	<b>85%</b>	<b>Max</b>
<b>Downward slope (grade&lt;-2%)</b>	95.2	54.1	83.3	95.8	106.2	137.6
<b>Upward slope (grade&gt;2%)</b>	97.2	60.9	86.9	98.0	110.1	142.1
<b>Flat grades (-2% ≤ grade ≤ 2%)</b>	96.9	53.5	86.1	96.8	107.6	143.0

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